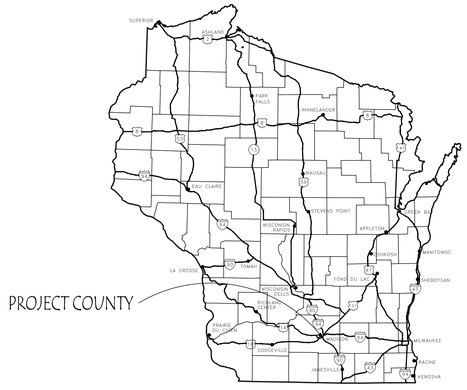
COLD SPRING DESIGN, LLC

222 SOUTH MAIN STREET - FORT ATKINSON, WI 53538
(920)568-9530 - WWW.COLDSPRINGDESIGN.NET

OCTAGON HOUSE

DRAWING INDEX:

SEAL



LOCATION MAP

OWNER:

xxx
919 Charles St.
Watertown, WI 53094

CONTACT:

xxx
xxx@xxx.com
608-xxx-xxxx

ARCHITECT / ENGINEER:

COLD SPRING DESIGN, LLC
222 SOUTH MAIN STREET
FORT ATKINSON, WI 53538
PHONE: (920)568-9530
CONTACT: CONOR NELAN

PROJECT INFORMATION:

All requirements per 2015 IBC

Building Information:

Building Height:	1-STORY, SEE ELEVATIONS
Use & Occupancy Classification:	OCCUPANCY TYPE B - 720 sq. ft. conditioned space
Construction Type:	Type VB - NON-SPRINKLERED

Cold Spring Design

222 South Main Street
Fort Atkinson, WI 53538
P (920)568-9530
F (920)568-9531

ISSUE

#	DATE	DESCRIPTION

OCTAGON HOUSE
919 CHARLES ST.
WATERTOWN, WI 53094

CSP PROJECT #: .

SCALE:	AS NOTED
DATE:	7/31/2024
DRAWN BY:	ATF
CHECKED BY:	CFN

TITLE SHEET

TS0.1

GENERAL NOTES

1. USE CURRENT COMMERCIAL BUILDING CODE ADOPTED BY WISCONSIN.

2. CONSULT ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR LOCATION AND DIMENSIONS OF CURBS, PADS, INSERTS, SLEEVES, DRIPS, REGLETS, REVEALS, DEPRESSIONS, AND OTHER PROJECT REQUIREMENTS NOT SHOWN ON STRUCTURAL DRAWINGS.

3. SIZE AND LOCATION OF ALL ROOF, FLOOR, AND WALL OPENINGS TO BE VERIFIED WITH MECHANICAL AND ELECTRICAL CONTRACTORS REQUIRING SUCH OPENINGS.

4. CONSULT ARCHITECT FOR ANY NECESSARY DIMENSIONS WHICH ARE NOT SHOWN ON PLANS. SCALING OF DRAWINGS IS NOT PERMITTED.

5. SIMILAR PORTIONS OF THE BUILDING SHALL HAVE SIMILAR DETAILING UNLESS NOTED OTHERWISE.

6. ELEVATIONS SHOWN ON PLAN ARE BASED ON 100'-0" AS FINISHED FIRST FLOOR ELEVATION.

7. ELEVATIONS SHOWN ON PLANS ARE TO TOP OF STEEL, CONCRETE, OR PLYWOOD SHEATHING, UNLESS NOTED OTHERWISE.

8. ALL WORK SHALL CONFORM TO OSHA REQUIREMENTS.

9. STRUCTURAL MEMBERS INCLUDING JOISTS, SLABS, BEAMS, TRUSSES, COLUMNS, AND WALLS ARE DESIGNED FOR "IN PLACE" LOADS. CONTRACTOR IS RESPONSIBLE FOR BRACING, WITHOUT OVER STRESSING, ALL STRUCTURAL ELEMENTS (AS REQUIRED AT ANY STAGE OF CONSTURCTION) UNTIL COMPLETION OF THIS PROJECT..

10. IN NO CASE SHALL STRUCTURAL ALTERATIONS OR WORK AFFECTING A STRUCTURAL MEMBER BE MADE, UNLESS APPROVED BY THE ARCHITECT.

11. SUBMIT REPRODUCIBLE COPY OF ALL STRUCTURAL SHOP DRAWINGS.

WOOD TRUSS NOTES

WOOD TRUSS SHOP DRAWINGS SHALL SHOW THE FOLOWING INFORMATION

1. INFORMATION WHICH THE RESPONSIBLE BUILDING DESIGN PROFESSIONAL WILL CHECK FOR COMPLIANCE WITH CONTRACT DOCUMENTS.
   a. ERECTION PLAN SHOWING DIMENSIONED LOCATIONS AND TRUSS IDENTIFICATION.
   b. BEARING DETAILS: SHOWING BEARING LENGTH, WIDTH, AND DEPTH INDICATING CONFORMANCE TO TRUSS CALCULATIONS.
   c. DESIGN LOADS: ALL DEAD AND LIVE LOADS SHALL BE SHOWN ON THE FRAMING PLAN AND/OR TRUSS ELEVATION INDICATING CONFORMANCE TO TRUSS CALCULATIONS.
   d. ALL PERMANENT BRACING: SHOW TOP CHORD, BOTTOM CHORD, AND WEB MEMBER BRACING ON FRAMING PLAN AND TRUSS ELEVATION. SUPPLIER AND INSTALLER OF THIS BRACING SHALL ALSO BE INDICATED.
   e. TRUSS DIMENSIONS: SHOW DEPTH, SPAN, BEARING, HEIGHT, AND SLOPES AT ALL CRITICAL POINTS.

2. INFORMATION THAT SHALL BE THE RESPONSIBILITY OF THE FABRICATOR AND TRUSS DESIGNER AND SHALL BE PROVIDED FOR INFORMATION WITH THE SHOP DRAWING SUBMITTAL.
   a. MEMBER DESIGN: INCLUDING WEB CONFIGURATION, MEMBER SIZE, GRADE OF LUMBER, FABRICATED SPLICES, REACTIONS, AND MEMBER BRACING REQUIRED BY TRUSS DESIGN.
   b. INTERIOR CONNECTIONS: DESIGN AND SHOW DETAIL OF WEB AND CHORD CONNECTIONS INCLUDING PLATE AND BOLT SIZES.
   c. MEMBER CONNECTIONS: DESIGN AND INDICATE ALL NECESSARY HARDWARE FOR PROPER INSTALLATION OF TRUSSES INCLUDING, BUT NOT LIMITED TO, GIRDER PLY CONNECTIONS, TRUSS-TO-GIRDER CONNECTIONS, TIE-DOWNS, AND FIELD SPLICES.
   d. STRUCTURAL DESIGN OF TRUSSES: SUBMIT COMPLETE TRUSS CALCULATIONS AND OBTAIN ALL APPROVALS NECESSARY FOR CONFORMANCE TO BUILDING CODE. VERIFY SUBMITTAL AND APPROVAL BY SENDING COPY TO BUILDING DESIGN PROFESSIONAL.
   e. PROVIDE CONTRACTINSTALLER WITH ALL DATA NECESSARY FOR PROPER INSTALLATION.
3. ROOF TRUSS SUPPLIER TO SEE ARCHITECTURAL BUILDING SECTIONS & REFLECTED CEILING PLANS FOR LOCATIONS WHERE ROOF TRUSSES NEED TO BE ADJUSTED FOR CEILING HEIGHT REQUIREMENTS.

ROOF TRUSS BRACING NOTES

1. ALL BRACING SHOWN OR DESCRIBED SHALL BE MINIMUM 2x4 WITH 2- 16d IN EVERY TRUSS IT CROSSES.

2. ALL TRUSS TOP CHORDS SHALL BE CONTINUOUSLY BRACED BY THE ROOF DECKING.

3. ALL TRUSS WEB MEMBERS SHALL BE BRACED AT 4'-0" OC, UNLESS CALCULATIONS SHOW OTHERWISE.

4. ALL HORIZONTAL BRACING SHALL BE STIFFENED AT 20'-0" OC WITH EITHER:
   a. DIAGONAL BRACING EXTENDED TO A SHEAR WALL PARALLEL TO THE ORIGINAL BRACING. SEE BRACING DETAIL 6/S3.1 FIGURES 1(a) THROUGH 1(d).
   b. A 3/8" PLYWOOD SHEET EXTENDED TO ROOF DECK OR SHEAR WALL.

5. ALL TRUSS BOTTOM CHORDS SHALL BE BRACED AT 6'-0" OC UNLESS CALCULATIONS SHOW OTHERWISE. CONTINUOUS SHEATHING APPLIED TO BOTTOM CHORD WILL SATISFY THIS BRACING REQUIREMENT.

TRUSS DESIGN LOADS

BUILDING OCCUPANCY CATEGORY	RESIDENTIAL
SNOW LOADS	
IMPORTANCE FACTOR - Is	1.0
EXPOSURE FACTOR - Ce	1.0
THERMAL FACTOR - Ct	1.1
GROUND SNOW LOAD - Pg	SEE DESIGN CRITERIA
FLAT ROOF SNOW LOAD - Pf	SEE DESIGN CRITERIA
APPLY DESIGN DRIFT LOADS TO ROOF TRUSSES WHERE REQUIRED BY CODE.	
WIND LOADS	
IMPORTANCE FACTOR - Iw	1.00
BASIC WIND SPEED - V	115 mph
EXPOSURE CATEGORY	B
DEAD LOADS	
ROOF DEAD LOAD	20 psf (10 TOP CHORD & 10 BOTTOM CHORD)
FLOOR DEAD LOAD	20 psf (10 TOP CHORD & 10 BOTTOM CHORD) 15 psf PARTITION LOAD
DEFLECTION LIMITS	
ROOF	
LIVE LOAD	L/360
TOTAL LOAD	L/240
FLOOR	
LIVE LOAD	L/480
TOTAL LOAD	L/360

WOOD FRAMING NOTES

1. ARCHITECT & CONTRACTOR SHALL DETAIL & CONSTRUCT BUILDING FINISHES TO ACCOMMODATE AN EXPECTED BUILDING SHRINKAGE OF APPROXIMATELY 3/4" TO 1" PER FLOOR OF WOOD CONSTRUCTION. PROPER CARE SHALL BE TAKEN TO PREVENT STORED & INSTALLED LUMBER FROM THE ELEMENTS. DO NOT ALLOW LUMBER TO REST IN STANDING WATER.

2. FRAMING MEMBERS

VERTICAL MEMBERS:	SEE BEARING WALL SCHEDULE, KD, MOISTURE CONTENT SHALL BE BETWEN 15% AND 19%
JOISTS:	2x No 1 / No 2 SPF, UNO SIZE & SPACING PER PLANS
JOISTS (EXPOSED TO WEATHER):	2x NO 1 / NO 2 TREATED SOUTHERN YELLOW PINE, UNO SIZE & SPACING PER PLANS
POSTS:	NO 2 SPF (INTERIOR), UNO NO 2 TREATED SOUTHERN YELLOW PINE (EXTERIOR), UNO

3. SEE WOOD BRG WALL SCHEDULE FOR BOTTOM PLATE & DOUBLE TOP PLATE INFORMATION..

4. FLOOR SHEATHING SHALL BE 3/4" APA RATED, T&G SHEATHING, GLUED & NAILED TO FLOOR FRAMING w/ 8d COMMON OR BOX NAILS @ 6" OC ALONG EDGES AND 12" OC ALONG INTERMEDIATE MEMBERS. STAGGER PANEL EDGES.

5. ROOF SHEATHING SHALL BE 3/8" APA RATED OSB SHEATHING ATTACHED TO THE ROOF FRAMING MEMBERS w/ 8d COMMON OR BOX NAILS @ 6" OC ALONG EDGES AND 12" OC ALONG INTERMEDIATE MEMBERS (1" MINIMUM EMBEDMENT INTO FRAMING MEMBER). STAGGER PANEL EDGES.

6. EXTERIOR WALLS SHALL BE SHEATHED w/ 7/16" APA RATED SHEATHING. ATTACH DIRECTLY TO THE OUTSIDE FACE OF EXTERIOR STUD WALLS WITH 8d COMMON OR BOX NAILS @ 6" OC ALONG EDGES AND 12" OC ALONG INTERMEDIATE MEMBERS, UNO.

7. ALL INTERIOR DEMISING WALLS, CORRIDOR WALLS, & LOAD BEARING WALLS NOT SPECIFICALLY DESIGNATED AS A SHEAR WALL, SHALL BE CONSTRUCTED WITH A MINIMUM OF 1 LAYER 5/8" GYPSUM BOARD ATTACHED w/ 6d COOLER NAILS @ 8" OC ALONG EDGES & 12" OC AT INTERMEDIATE MEMBERS, UNO.

8. DESIGN UPLIFT ON ROOF TRUSSES AS INDICATED IN THE DESIGN CRITERIA. PROVIDE TIE DOWN CLIP AT EACH TRUSS, AT EVERY POINT OF BEARING.

9. TYPICAL TRUSS SPACING = 2'-0" EXCEPT WHERE SPECIFICALLY NOTED.

10. COORDINATE WALL STUD LOCATIONS TO ALIGN WITH TRUSS BEARING LOCATIONS @ ALL WALLS.

11. PROVIDE EQUIVALENT SIZE SOLID BLOCKING & VERTICAL MEMBERS THROUGH UNDERLYING FLOORS / WALLS BELOW MULTIPLE MEMBERS OR POSTS CARRYING CONCENTRATED LOADS.

12. COLUMN SIZES SHOWN ARE MIN. CONTRACTOR MAY USE LARGER SECTION IF REQ'D TO FULLY SUPPORT MEMBERS.

13. AS A MINIMUM, ALL CONNECTIONS SHALL CONFORM TO FASTENING SCHEDULE TABLE 2304.9.1 WECBC 2006. DRAWING DETAILS SHALL GOVERN IF THEIR CONNECTION CAPACITY IS GREATER THAN THOSE SPECIFIED IN TABLE 2304.9.1

14. WHERE BUILT-UP SECTIONS OF DIMENSIONAL LUMBER ARE INDICATED, FASTENING SHALL BE IN ACCORDANCE WITH NDS 15.3.3. MULTI-PLY LVL SECTIONS SHALL BE FASTENED IN ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS.

15. USE JOIST HANGERS DESIGNED FOR GIVEN MEMBER SIZE TO SUPPORT ALL JOISTS/HEADERS FRAMING INTO SIDES OF OTHER MEMBERS.

16. PROVIDE CROSS BRIDGING/BLOCKING BETWEEN FLOOR JOISTS PER NATIONAL DESIGN SPECIFICATION 4.4.1.

17. DO NOT CUT, NOTCH, OR DRILL HOLES IN MIRCOLAM LVL OR JOISTS WITHOUT ENGINEER APPROVAL.

18. ALL HEADERS NOT ABOVE DOORS OR WINDOWS TO BE FLUSH WITH CEILING, UNO.

19. COORDINATE WALL & FACE BRICK DIMENSIONS w/ ARCHITECTURAL DRAWINGS.

20. JOIST MANUFACTURER SHALL NOTIFY STRUCTURAL ENGINEER IF FRAMING PLANS TO BE DIFFERENT THAN SHOWN.

21. ALL CONNECTORS (I.E., SIMPSON HANGERS, ETC.) TO BE GALVANIZED WHEN USED FOR EXTERIOR PURPOSES.

22. GENERAL CONTRACTOR TO COORDINATE WOOD TRUSS, PLUMBING, AND HVAC LOCATIONS.

FOUNDATION PLAN NOTES

1. CONTRACTOR SHALL PROVIDE FROST AND MOISTURE PROTECTION FOR FOOTINGS EXPOSED DURING CONSTRUCTION.

2. REFER TO ARCHITECTURAL DRAWINGS OR PLUMBING DRAWINGS FOR SPECIFIC FLOOR DRAIN LOCATIONS AND ELEVATIONS.

3. REFER TO FOUNDATION DETAILS SHEET FOR MISCELLANEOUS DETAILS NOT CUT ON PLAN.

4. FOOTING EXCAVATIONS SHALL BE EXAMINED BY THE GEOTECHNICAL ENGINEER TO CONFIRM THAT THE SOILS AT THE BOTTOM OF THE EXCAVATION ARE CAPABLE OF PROVIDING THE ALLOWABLE BEARING PRESSURE NOTED IN THE DESIGN CRITERIA. CONTACT THE ARCHITECT OR ENGINEER IF UNABLE TO ATTAIN THIS SOIL BEARING PRESSURE.

5. NO PROVISION HAS BEEN MADE FOR FUTURE EXPANSION.

6. VERIFY SIZES OF ALL STOOPS WITH ARCHITECT PRIOR TO CONSTRUCTION.

STRUCTURAL ABBREVIATIONS

AB - ANCHOR BOLT	ID - INSIDE DIAMETER
ALT - ALTERNATE	IF - INSIDE FACE
AGG - AGGREGATE	INSUL - INSULATION
ALUM - ALUMINUM	INT - INTERIOR
ARCH - ARCHITECT/ ARCHITECTURAL	JBE - JOIST BEARING ELEVATION
BLDG - BUILDING	JT - JOINT
BLK - BLOCK (CMU)	L - STEEL ANGLE DESIGNATION
BM. - BEAM	LG - LENGTH/LONG
BOT - BOTTOM	LLH - LONG LEG HORIZONTAL
BRG - BEARING	LLV - LONG LEG VERTICAL
	LP - LOW POINT
C - CHANNEL DESIGNATION	LVL - LAMINATED VENEER LUMBER
CB - CATCH BASIN	
CI - CAST IRON	MAX. - MAXIMUM
CIP - CAST-IN-PLACE	MBW - MASONRY BEARING WALL
CJ - CONSTRUCTION/ CONTROL JOINT	MIN - MINIMUM
CL - CENTER LINE	MSW - MASONRY SHEAR WALL
CLG - CEILING	MTL - METAL
CLR - CLEAR DISTANCE	NIC - NOT IN CONTRACT
CMU - CONCRETE MASONRY UNIT	NOM - NOMINAL
COL - COLUMN	NTS - NOT TO SCALE
CONC - CONCRETE	N-S - NORTH-SOUTH DIRECTION
CONT - CONTINUOUS	
CONTR - CONTRACTOR	OC - ON CENTER
	OD - OUTSIDE DIAMETER
D - DEPTH	OF - OUTSIDE FACE
DBA - DEDICATE/ARING ANGLE	OH - OVER HEAD
DIA - DIAMETER	OPP - OPPOSITE
DIM - DIMENSION	
DN - DOWN	PERIM - PERIMETER
DP - DRILLED PIER	PC - PRECAST / PRESTRESSED
DTL - DETAIL	PL - STEEL PLATE DESIGNATION
DWG - DRAWING	PT - POINT
DWL - DOWEL	P/T - POST TENSIONED
EA - EACH	R - RADIUS
EC - ELECTRICAL CONTRACTOR	RD - ROOF DRAIN
EJ - EXPANSION JOINT	REINF - REINFORCING / REINFORCEMENT
EL - ELEVATION	REQ'D - REQUIRED
ELEV - ELEVATOR	
ENG - ENGINEER	SCHED - SCHEDULE
EQ - EQUAL	SIM - SIMILAR
EW - EACH WAY	SHT - SHEET
E-W - EAST-WEST DIRECTION	SPA - SPACE / SPACES
EXIST - EXISTING	SPEC - SPECIFICATION
EXP - EXPANSION	SQ - SQUARE
EXT - EXTERIOR	SS - STAINLESS STEEL
	STL - STEEL
FD - FLOOR DRAIN	STR - STRUCTURAL
FDN - FOUNDATION	
FE - FIRE EXTINGUISHER	THK - THICK
FF - FINISH FLOOR	TL - TOP OF LEDGE ELEVATION
FT - FLOOR TRUSS	TP - TOP OF PIER ELEVATION
FTG - FOOTING	TS - SEE HSS DESIGNATION
FUT - FUTURE	TYP - TYPICAL
FV - FIELD VERIFY	TW - TOP OF WALL ELVATION
GA - GAUGE	UD - URINAL SLAB DEPRESSION
GALV - GALVANIZED	UNO - UNLESS NOTED OTHERWISE
GC - GENERAL CONTRACTOR	
GL - GRID LINE/COLUMN LINE	VERT - VERTICAL
	VIF - VERIFY IN FIELD
HC - HVAC CONTRACTOR	
HK - HOOK	W - WIDTH
HM - HOLLOW METAL	W/ - WITH
HORIZ - HORIZONTAL	W/O - WITHOUT
HP - HIGH POINT	WD - WOOD
HSS - HOLLOW STRUCTURAL SECTION (REPLACES "TS" DESIGNATION)	WF - WIDE FLANGE DESIGNATION
	WP - WORKING POINT
HT - HEIGHT	WSBW - WOOD STUD BEARING WALL
HVAC - HEATING, VENTILATING, & AIR CONDITIONING	WWF - WELDED WIRE FABRIC

REINFORCING NOTES

1. REINFORCING SHALL BE DETAILED IN ACCORDANCE WITH ACI 315 (CURRENT EDITION).

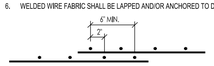
2. ALL LAPS SHALL BE CLASS 'B' PER ACI 318 UNLESS OTHERWISE NOTED ON THE DESIGN DRAWINGS, OR UNLESS THE DETAILER TAKES SPECIAL CARE TO PROVIDE STAGGERED LAPS. USE TOP BAR LAP LENGTHS FOR ALL HORIZONTAL WALL BARS AND FOR TOP BARS IN SLABS AND BEAMS OVER 12 INCHES DEEP.

3. LAP LENGTH SHALL BE SPECIFICALLY NOTED ON PLACING DRAWINGS WHERE MORE THAN ONE BAR MAKES UP A CONTINUOUS STRING.

4. CORNER BARS WITH CLASS 'B' PER ACI 318 LAPS SHALL BE PROVIDED AT ALL WALL CORNERS AND INTERSECTIONS PER DETAIL 1/S2.1.

5. HORIZONTAL BARS, EXCEPT FOR CONTINUOUS STRINGS FROM ONE CORNER OF OPENING TO ANOTHER, SHALL BE DETAILED TO SHOW THE DISTANCE FROM AT LEAST ONE END OF THE BAR TO THE NEAREST BUILDING GRID LINE OR WALL.

6. WELDED WIRE FABRIC SHALL BE LAPPED AND/OR ANCHORED TO DEVELOP Fy PER ACI 315.



7. PROVIDE MINIMUM COVER PER ACI 318, 7.7.1.

8. PROVIDE REINFORCING AT CONCRETE OPENINGS PER DETAIL 2/S2.1.

9. PROVIDE TYPICAL VERTICAL WALL JOINTS PER DETAIL 3/S2.1.

10. PROVIDE ISOLATION BOARD WHERE SLABS ABUT VERTICAL SURFACES PER DETAIL 4/S2.1.

11. PROVIDE FOOTING STEPS PER DETAIL 5/S2.1.

12. PROVIDE SLAB ON GRADE CONSTRUCTION AND CONTROL JOINTS PER DETAILS 6 & 7/S2.1.

MILD REINFORCING STEEL MINIMUM CLEAR COVER

CONCRETE CAST AGAINST EARTH AND PERMANENTLY EXPOSED TO EARTH	
FOOTINGS	3" MIN
CONCRETE EXPOSED TO EARTH OR WEATHER	
WALLS, COLUMNS, & BEAMS	
BARS UP TO #5	1½" MIN
#6 BARS AND UP	2" MIN
CONCRETE NOT EXPOSED TO EARTH OR WEATHER	
WALLS	
BARS UP TO #11	¾" MIN
#14 BARS AND UP	1½" MIN
ELEVATED SLABS	
TOP BARS	¾" MIN
BOTTOM BARS	1" MIN
BEAMS	
ALL BARS	1½" MIN
COLUMNS	
ALL BARS	1½" MIN

DESIGN CRITERIA

DESIGN CODE
INTERNATIONAL BUILDING CODE 2015 w/ WISCONSIN AMENDMENTS

DESIGN LOADS	
LIVE LOAD INFORMATION	
CORRIDOR	100 psf
DECK - RESIDENTIAL	40 psf
MECHANICAL	125 psf
PUBLIC AREA	100 psf
STAIR	100 psf
STORAGE	125 psf
SNOW LOAD INFORMATION	
GROUND SNOW LOAD - Pg	30.0 psf
SNOW EXPOSURE FACTOR - Ce	1.00
SNOW LOAD IMPORTANCE FACTOR - Is	1.00
THERMAL FACTOR - Ct	1.10
FLAT ROOF SNOW LOAD - Pf	25.0 psf
DRIFT LOAD	SEE SNOW DRIFT DIAGRAM ON UPPER ROOF PLAN
SOIL LOAD INFORMATION	
ALLOWABLE NET SOIL BEARING PRESSURE - Qa	2.000 psf (ASSUMED)

WIND LOAD INFORMATION

BASIC WIND SPEED			115 mph
BUILDING CODE OCCUPANCY CATEGORY			R
WIND LOAD IMPORTANCE FACTOR - Iw			1.00
WIND EXPOSURE			B
INTERNAL PRESSURE COEFFICIENTS			±0.18
COMPONENTS AND CLADDING WIND PRESSURES			
WIDTH OF PRESSURE COEFFICIENT ZONE - a			17.1 ft
TRIBUTARY WIND LOAD AREAS	10 SF	100 SF	500 SF
ROOF			
NEGATIVE ZONE 1	-16.6 psf	-15.2 psf	-
NEGATIVE ZONE 2	-27.9 psf	-18.0 psf	-
NEGATIVE ZONE 3	-27.9 psf	-18.0 psf	-
POSITIVE ALL ZONES	10.0 psf	10.0 psf	-
WALLS			
ZONE 4	-16.5 psf	-	-12.7 psf
ZONE 5	-20.3 psf	-	+12.7 psf

SEISMIC LOAD INFORMATION	
SEISMIC USE GROUP - OCCUPANCY CATEGORY	II
SEISMIC LOAD IMPORTANCE FACTOR - Ie	1.00
SEISMIC SITE CLASS	D
MAPPED SPECTRAL RESPONSE ACCELERATION - Ss	0.1170
MAPPED SPECTRAL RESPONSE ACCELERATION - S1	0.0470
SPECTRAL RESPONSE COEFFICIENT - Sds	0.125
SPECTRAL RESPONSE COEFFICIENT - Sd1	0.075
SEISMIC DESIGN CATEGORY	B
BASIC SEISMIC FORCE RESISTING SYSTEM	LIGHT FRAME WALLS w/ SHEAR PANELS
RESPONSE MODIFICATION FACTOR	2
SEISMIC RESPONSE COEFFICIENT - Cs	0.062
DESIGN BASE SHEAR	0.062W
ANALYSIS PROCEDURE	EQUIVALENT LATERAL FORCE

DESIGN PROPERTIES

REINFORCING STEEL STRENGTHS	
BARS - ASTM A615, GRADE 60	Fy = 60,000 psi
WWF - ASTM A185	Fy = 65,000 psi
BOLT STRENGTHS	
ANCHOR BOLTS - ASTM A307 OR A36	
HIGH STRENGTH BOLTS - ASTM A325N	
EXPANSION BOLTS - WEDGE TYPE	
CAST-IN-PLACE CONCRETE STRENGTHS	
FOOTINGS	f'c = 3,000 psi
WALLS	f'c = 4,000 psi
SLAB ON GRADE	f'c = 4,000 psi
PRECAST CONCRETE TOPPING	f'c = 4,000 psi
SITE PAVEMENT - CURBS	f'c = 3,000 psi
STRUCTURAL STEEL STRENGTHS	
WF SHAPES - ASTM A992	Fy = 50,000 psi
C SHAPES, L SHAPES, PLATES, & BARS- ASTM A36	Fy = 36,000 psi
TS OR HSS SHAPES - ASTM A500, GRADE B	Fy = 46,000 psi
CONCRETE MASONRY STRENGTHS	
CMU - ASTM C90, GRADE N	f'm = 1,500 psi
CONCRETE BRICK - ASTM C55, GRADE N	f'm = 2,500 psi
CLAY HOLLOW BRICK - ASTM C652, GRADE SW	f'm = 3,000 psi
MORTAR - ASTM C270	
TYPE M - BELOW GRADE	fu = 2,500 psi
TYPE S - ABOVE GRADE	fu = 1,800 psi
GROUT - ASTM C476	
BOND BEAMS	f'c = 3,000 psi
WALLS AND PIERS	f'c = 3,000 psi

CANADIAN DIMENSIONAL LUMBER STRENGTHS - BASE VALUES
2" TO 4" THICK AND WIDER
BASE VALUES IN psi - TO BE USED WITH ADJUSTMENT FACTORS

SPECIES	GRADE	EXT FIBER STRESS IN BENDING Fb	TENSION PARALLEL TO GRAIN Ft	HORIZ SHEAR Fv	COMP ⊥ TO GRAIN $F_{c\perp}$	COMP ∥ TO GRAIN Fc		COMP MODULUS OF GRAINELASTICITY E
SPRUCE	SEL STR	1250	675	70	425	1400	1.5	
PINE FIR	NO 1/NO 2	875	425	70	425	1100		1.4
	NO 3	500	250	70	425	625		1.2
	STUD	675	325	70	425	675		1.2
DOUGLAS	SEL STR	1300	800	95	625	1900	1.9	
FIR LARCH	NO 1/NO 2	825	500	95	625	1350	1.6	
(N)	NO 3	475	300	95	625	775		1.4
	STUD	650	375	95	625	850		1.4
HEM FIR	SEL STR	1300	775	75	370	1650	1.7	
(N)	NO 1/NO 2	1000	550	75	370	1450	1.6	
	NO 3	575	325	75	370	850		1.4
	STUD	775	425	75	370	925		1.4
NORTHERN	SEL STR	950	450	65	350	1100		1.1
SPECIES	NO 1/NO 2	575	275	65	350	825		1.1
	NO 3	350	150	65	350	475		1.0
	STUD	450	200	65	350	525		1.0

SHEAR WALL SHEATHING ATTACHMENT SCHEDULE

MARK	REQUIRED ATTACHMENT
SW1	1/2" 16 GAGE STAPLES @ 6" OC EDGE AND 12" OC FIELD
SW2	1/2" 16 GAGE STAPLES @ 4" OC EDGE AND 12" OC FIELD

HEADER SCHEDULE

MARK	TYPE	SHOULDER STUDS, UNO		REMARKS
		BASEMENT	1st	
H1	SPF NO 1/NO2 2-2x8	2	1	
H2	2-9½" LVL	3	2	
H3	2-11⅞" LVL	3	2	
H4	3-11⅞" LVL	3	2	

HEADER SCHEDULE NOTES
1. WHERE HEADER PLIES ARE LESS THAN THE CORRESPONDING WALL WIDTH, THE HEADER SHALL BE MODIFIED ACCORDING TO I/S3.0.
2. SEE DETAIL 2/S3.0 FOR MULTI-PLY CONNECTION ASSEMBLIES.
3. SEE DETAIL 5/S3.0 FOR TYPICAL HEADER FRAMING ELEVATION.
4. SEE SHEET S0.1 FOR WOOD DESIGN PROPERTIES & MINIMUM STRESS REQUIREMENTS.
5. HEADERS DENOTED AS YA-R' TO BE RECESSED (I.E. TOP OF HEADER @ UNDERSIDE OF SHEATHING).
6. MINIMUM REQUIRED LVL BENDING STRESS TO BE 2400 psi.
7. SHOULDER STUDS TO BE OF SAME SPECIES AND GRADE AS BEARING WALLS, UNO.
8. WALL OPENINGS SHOWN WITHOUT HEADERS ARE CONSIDERED NON-BEARING WALLS. MINIMUM 2-2x6 HEADER REQUIRED w/ 1- SHOULDER STUD EA SIDE.
9. PROVIDE FULL HEIGHT SPF KING STUDS ADJACENT TO EACH HEADER LOCATION ACCORDING TO THE FOLLOWING:

OPENINGS ≤ 6'-0"	1 KING STUD
6'-0" < OPENING ≤ 10'-0"	2 KING STUDS
10'-0" < OPENING ≤ 16'-0"	3 KING STUDS

10. REFER TO ARCHITECTURAL DOOR SCHEDULE SHEET FOR DOOR AND FRAME HEIGHTS AT HEADERED OPENINGS. REFER TO ARCHITECTURAL WINDOW TYPES SHEETS FOR WINDOW HEIGHTS AT HEADERED OPENINGS. NOTE THAT SOME EXTERIOR WALL OPENINGS ARE HIGHER THAN TYPICAL AND TOP OF HEADER HEIGHTS WILL CREATE TRUSS BEARING HEIGHTS HIGHER THAN THE BOTTOM CHORD.

WOOD STUD BEARING WALL SCHEDULE

LOCATION	TYPE		REMARKS
	BASEMENT	1st	
EXTERIOR WALLS	–	SPF NO1/NO2 2x6 @ 12" OC	
DEMISING WALLS (NON-BRG LOCATIONS)	SPF STUD GR 2x @ 24" OC	SPF STUD GR 2x @ 24" OC	SEE ARCH FOR WALL SIZES & LOCATIONS
1	SPF NO1/NO2 2x6 @ 12" OC	SPF NO1/NO2 2x6 @ 12" OC	
2	SPF NO1/NO2 2x6 @ 16" OC	SPF NO1/NO2 2x6 @ 16" OC	

WOOD STUD BEARING WALL SCHEDULE NOTES
1. SEE SHEET S0.0 FOR WOOD DESIGN PROPERTIES & MINIMUM STRESS REQUIREMENTS.
2. ALL PLATES IN CONTACT WITH CAST-IN-PLACE CONCRETE OR CMU TO BE PRESSURE TREATED.
3. BOTTOM PLATES & DOUBLE TOP PLATES TO BE SPF MATERIAL - MINIMUM BENDING STRESS TO BE 2,325 psi.
4. ALL STUD WIDTHS NOTED ARE MINIMUM. CONTRACTOR CAN UPSIZE FRAMING MEMBERS & KEEP SAME STUD SPACING AS NOTED IF REQUIRED. COORDINATE w/ ARCH PRIOR TO INSTALLATION.
5. CORING OF STUD BEARING WALLS IS NOT PERMITTED. IF NECESSARY, CONTACT ENGR FOR FURTHER BRACING INFORMATION.

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ISSUE

#	DATE	DESCRIPTION

OCTAGON HOUSE

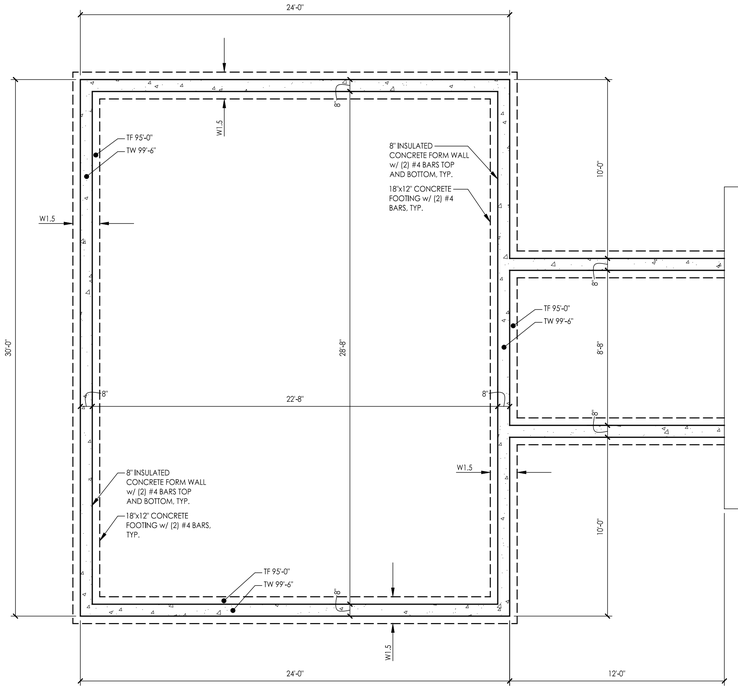
919 CHARLES ST.
WATERTOWN, WI 53094

CSP PROJECT #: .

SCALE: AS NOTED
DATE: 7/31/2024
DRAWN BY: ATF
CHECKED BY: CFN

GENERAL NOTES AND SCHEDULES

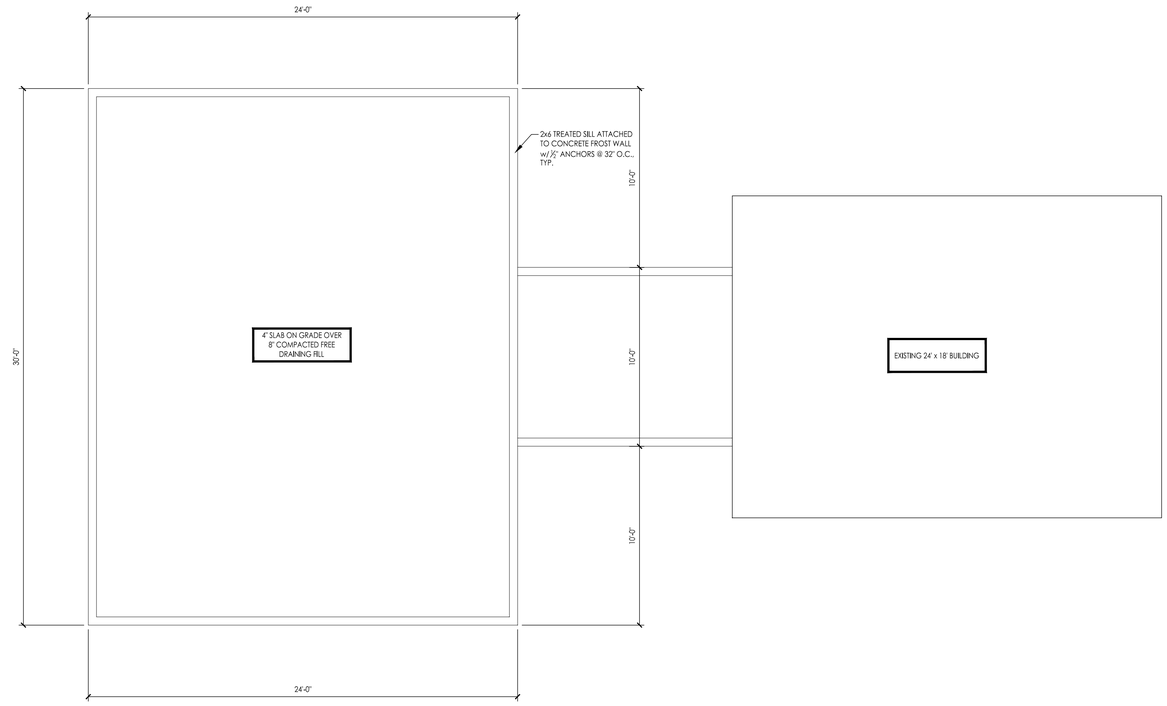
S0.1



1 S1.1 PROPOSED FOUNDATION PLAN

SCALE: 1/4" = 1' - 0"

NOTES:
1. ALL DIMENSIONS ARE FROM FACE OF CONCRETE OR STUD TO FACE OF CONCRETE OR STUD, UNLESS NOTED OTHERWISE.



2 S1.1 PROPOSED FLOOR FRAMING PLAN

SCALE: 1/4" = 1' - 0"

NOTES:
1. ALL DIMENSIONS ARE FROM FACE OF CONCRETE OR STUD TO FACE OF CONCRETE OR STUD, UNLESS NOTED OTHERWISE.

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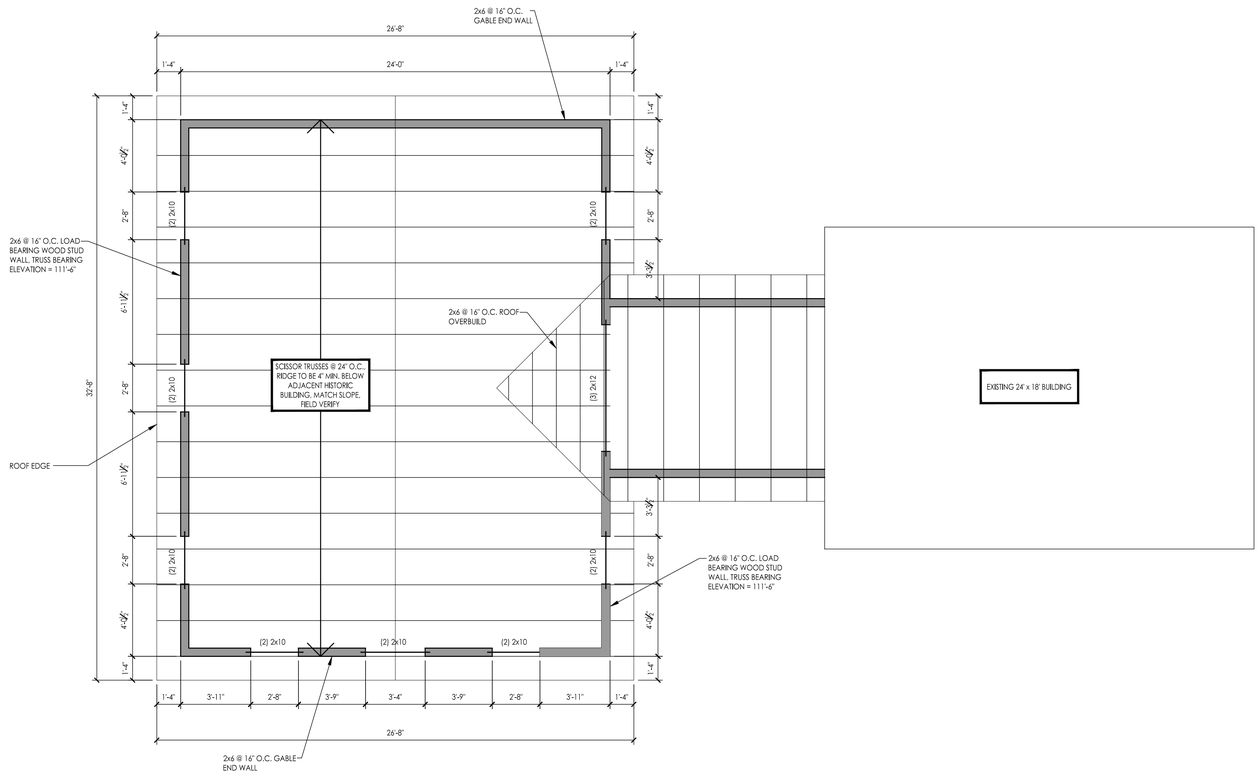
FOUNDATION AND FLOOR FRAMING PLAN

S1.1



1 S1.2 PROPOSED ROOF FRAMING PLAN

SCALE: 1/4" = 1' - 0"

NOTES:

1. ALL DIMENSIONS ARE FROM FACE OF CONCRETE OR STUD TO FACE OF CONCRETE OR STUD, UNLESS NOTED OTHERWISE.

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ROOF FRAMING PLAN

S1.2

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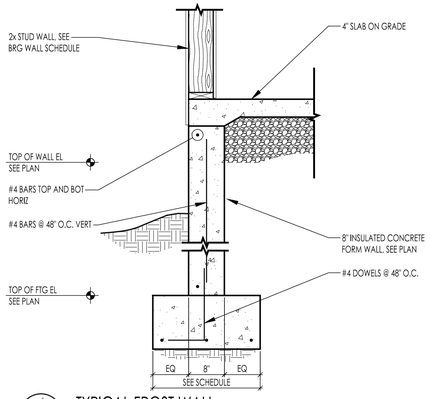
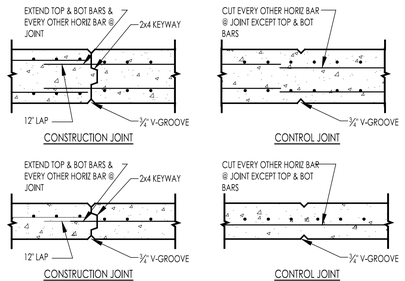
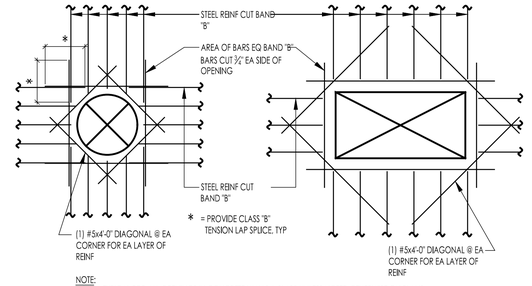


4 S2.1 TYPICAL FROST WALL
SCALE: NTS



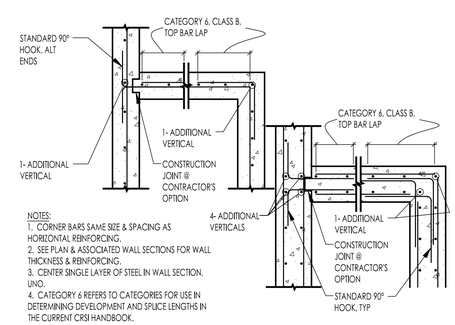
NOTE:
1. VERTICAL WALL JOINTS SHALL OCCUR @ INTERVALS OF 25 FEET.
2. KEYED EXPANSION JOINTS SHALL OCCUR @ EVERY FOURTH JOINT.

3 S2.1 TYPICAL VERTICAL WALL JOINTS
SCALE: NTS



NOTE:
1. TYPICAL FOR ALL OPENINGS IN CONCRETE WALLS & SLABS UNLESS NOTED OTHERWISE ON PLANS.
2. DO NOT WELD REINFORCEMENT TO PIPE SLEEVE AND INSERTS UNLESS NOTED OTHERWISE ON PLANS.

2 S2.1 REINFORCING @ CONCRETE OPENINGS
SCALE: NTS



NOTES:
1. CORNER BARS SAME SIZE & SPACING AS HORIZONTAL REINFORCING.
2. SEE PLAN & ASSOCIATED WALL SECTIONS FOR WALL THICKNESS & REINFORCING.
3. CENTER SINGLE LAYER OF STEEL IN WALL SECTION, UNO.
4. CATEGORY 6 REFERS TO CATEGORIES FOR USE IN DETERMINING DEVELOPMENT AND SPLICE LENGTHS IN THE CURRENT CRSI HANDBOOK.

1 S2.1 REINFORCING @ CONC WALL CORNERS
SCALE: NTS

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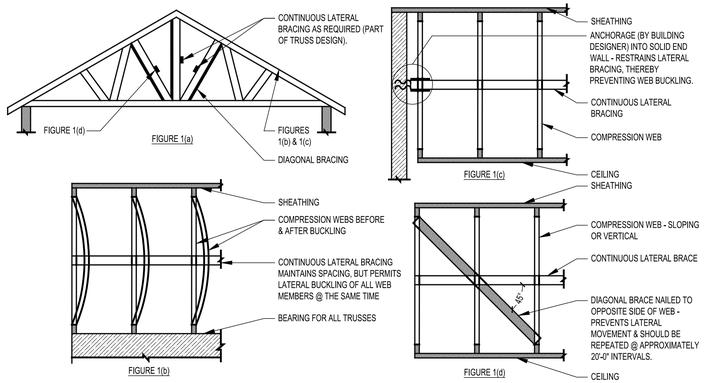
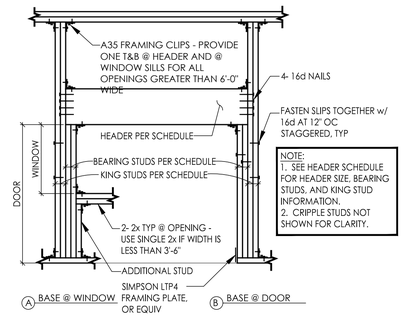
FOUNDATION DETAILS

S2.1



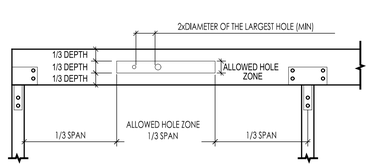
6 S3.1 BRACING DETAILS

SCALE: NTS

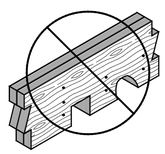


5 S3.1 TYP ELEVATION DETAIL @ OPENING IN WOOD FRAMED WALL

SCALE: NTS



NOTES:
1. THE ALLOWED HOLE ZONE IS SUITABLE ONLY FOR UNIFORMLY LOADED BEAMS USING MAXIMUM LOADS FOR ANY TABLES LISTED IN THIS BROCHURE. FOR OTHER LOAD CONDITIONS OR HOLE CONFIGURATIONS, PLEASE CONTACT YOUR TRUSS JOIST REPRESENTATIVE.
2. RECTANGULAR HOLES ARE NOT ALLOWED.
3. HOLES IN CANTILEVERS REQUIRE ADDITIONAL ANALYSIS.
4. IF LARGER HOLES ARE REQUIRED, CONTACT STRUCTURAL ENGINEER FOR EVALUATION.

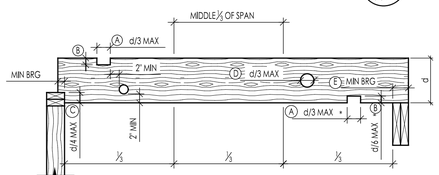


DO NOT CUT, NOTCH OR DRILL HOLES IN MICROLLAM LVL EXCEPT AS INDICATED IN TABLE BELOW AND ILLUSTRATION ABOVE.

BEAM DEPTH	MAXIMUM ROUND HOLE SIZE
5½"	1½"
7¼" TO 18"	2"

4 S3.1 ALLOWABLE LVL MODIFICATION DETAIL

SCALE: NTS



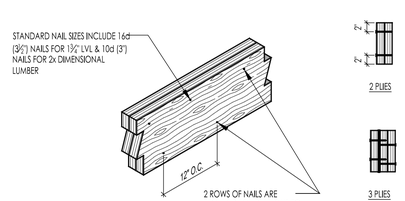
* NOTE: IF b ≥ 3½", THEN NO NOTCHES ON TENSION SIDE EXCEPT AT ENDS (b = MEMBER THICKNESS).

JOIST SIZE	(A) MAXIMUM NOTCH LENGTH	(B) MAXIMUM NOTCH DEPTH	(C) MAXIMUM END NOTCH DEPTH	(D) MAXIMUM HOLE DEPTH	(E) MINIMUM (1) BEARING LENGTH	
2x6	1'-3⅛"	3/8"	1⅛"	1'-3⅛"	1½"	3"
2x8	2'-1⅝"	1'-3⅛"	1'-3⅛"	2'-1⅝"	1½"	3"
2x10	3'-1⅛"	1½"	2'-3⅛"	3'-1⅛"	1½"	3"
2x12	3'-10¾"	1¾"	2'-3⅛"	3'-10¾"	1½"	3"

NOTE:
1. MINIMUM BEARING: 1-1/2" ON WOOD OR STEEL; 3" BEARING ON MASONRY.

3 S3.1 ALLOWABLE JOIST HOLES & NOTCHES

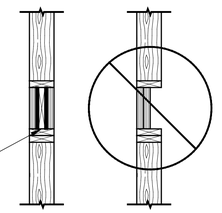
SCALE: NTS



NOTES:
BOLTED CONNECTIONS SHALL BE ASTM GRADE A307 (OR BETTER) ½" BOLTS @ 24" OC WITH WASHERS ON BOTH FACES, UNO

2 S3.1 BEAM/HEADER CONNECTION ASSEMBLIES

SCALE: NTS



NOTES:
ACCEPTABLE MODIFICATIONS TO ACHIEVE REQUIRED HEADER THICKNESS INCLUDE THE ADDITION OF LVL, 2x DIMENSIONAL LUMBER, AND OR PLYWOOD OF MATCHING (OR BETTER) GRADE & SPECIES OF THE CORRESPONDING HEADER AND WALL STUDS.

1 S3.1 HEADER FRAMING DETAIL

SCALE: NTS

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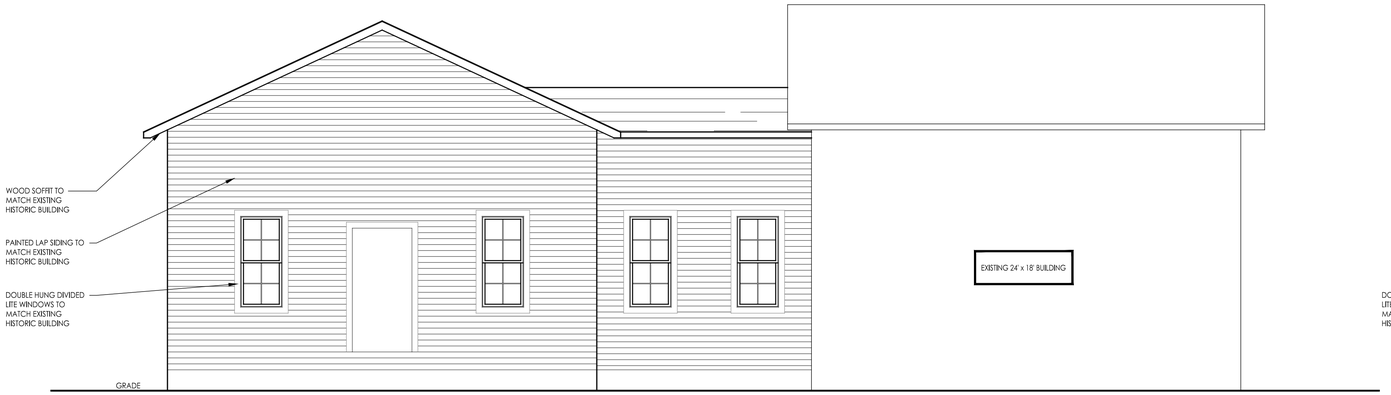
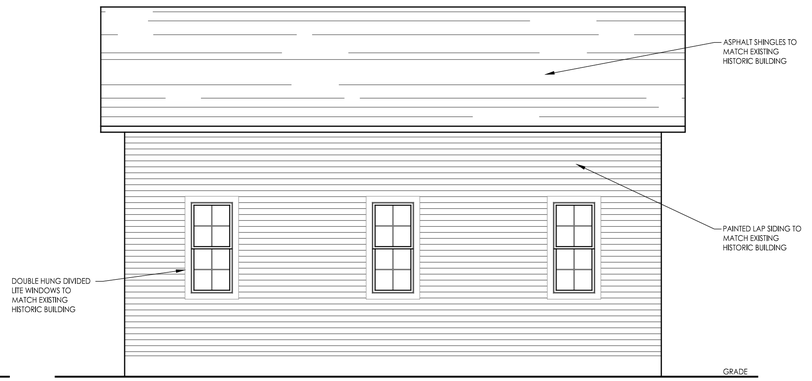
WOOD FRAMING DETAILS

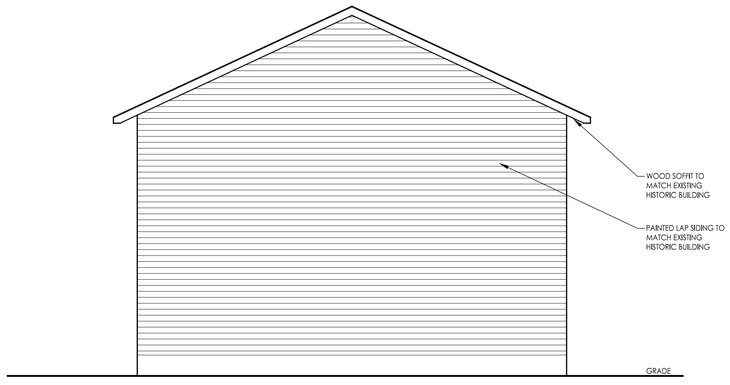
S3.1



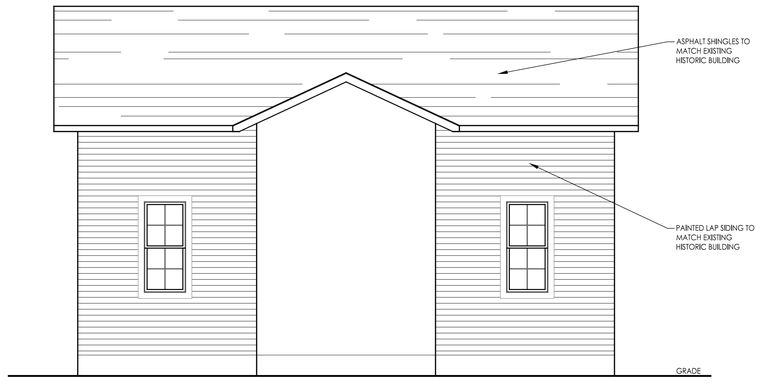
1 A2.1 PROPOSED WEST ELEVATION
SCALE: 1/4" = 1' - 0"



2 A2.1 PROPOSED NORTH ELEVATION
SCALE: 1/4" = 1' - 0"



3 A2.1 PROPOSED EAST ELEVATION
SCALE: 1/4" = 1' - 0"



4 A2.1 PROPOSED SOUTH ELEVATION
SCALE: 1/4" = 1' - 0"

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EXTERIOR ELEVATIONS

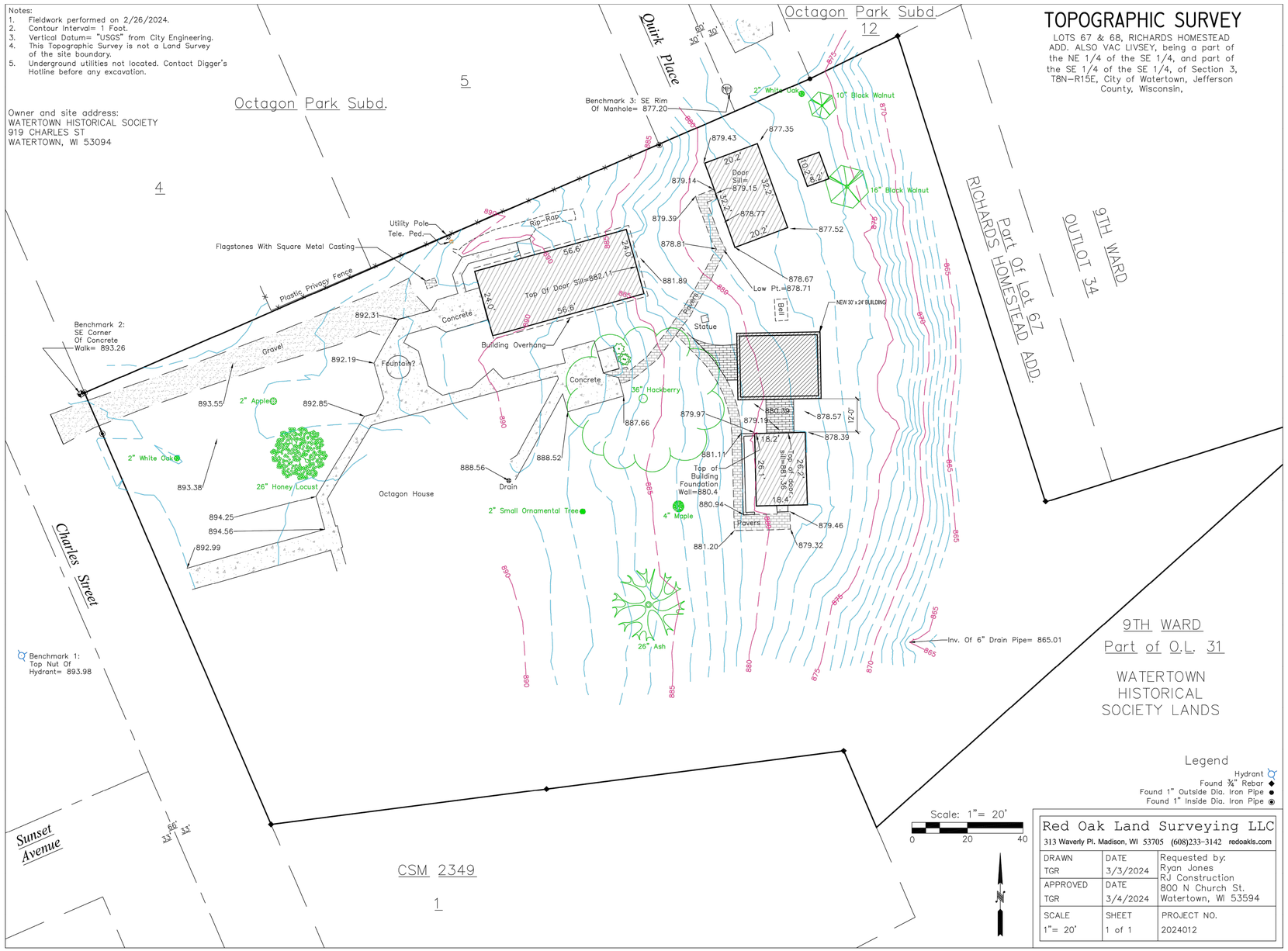
A2.1

TOPOGRAPHIC SURVEY

LOTS 67 & 68, RICHARDS HOMESTEAD ADD. ALSO VAC LIVSEY, being a part of the NE 1/4 of the SE 1/4, and part of the SE 1/4 of the SE 1/4, of Section 3, T8N–R15E, City of Watertown, Jefferson County, Wisconsin,

Notes:
1. Fieldwork performed on 2/26/2024.
2. Contour Interval= 1 Foot.
3. Vertical Datum= "USGS" from City Engineering.
4. This Topographic Survey is not a Land Survey of the site boundary.
5. Underground utilities not located. Contact Digger's Hotline before any excavation.

Owner and site address:
WATERTOWN HISTORICAL SOCIETY
919 CHARLES ST
WATERTOWN, WI 53094



Red Oak Land Surveying LLC		
313 Waverly Pl. Madison, WI 53705 (608)233-3142 redoaks.com		
DRAWN TGR	DATE 3/3/2024	Requested by: Ryan Jones RJ Construction 800 N Church St. Watertown, WI 53594
APPROVED TGR	DATE 3/4/2024	
SCALE 1"= 20'	SHEET 1 of 1	PROJECT NO. 2024012

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SITE PLAN