

Have questions about this feature? Call 1-888-293-2339 Mon-Fri: 8AM-7PM | Sat: 10AM-3PM EST

Print Close

diamond pion footings treated 6x6 posts

4/12 pitch roof trusses 2' O.C.

Aluminum soff, + + fasciq



PROJECT: PAVILLIGIA PROJECT NO.: 2023-118 DATE: 8-16-2023 INITIALS: MPT PAGE NO.: 1

PAVILLION STRUCTURE FOR

SCHURZ ELEMENTARY 1508 NEENAH ST. WATERTOWN, WI 53094

SHEET LIST

1 TITLE

2 LOADS, PLAN

3 BEAMS, COLUMNIS, FOUNDATIONS

4 FORTE, TRUSS LOADS

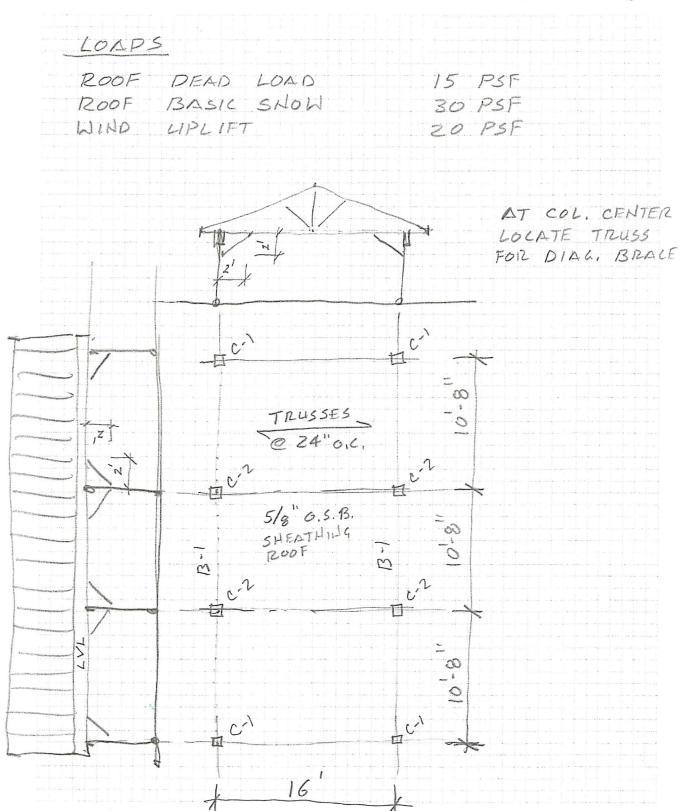
5 FORTE, BEAM DESIGN

6 DIAMOND PIER CHART



Trego ARCHITECTS

PROJECT: PAVILLION PROJECT NO.: 2023-118
DATE: 8-16-2023
INITIALS: M DT PAGE NO.: Z



115 North Maple Street Watertown, Wisconsin 53094 www.tregoarchitects.com

Trego ARCHITECTS

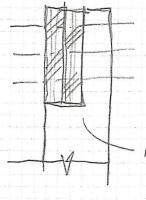
PROJECT: PAVILLIGH PROJECT NO.: 2023-118 DATE: 8-16-2023 INITIALS: MPT PAGE NO.: 3

B-1 (= 10'-8" (MULTH-SPAN) USE (2) 71/4" LVL SEE FORTE OUTPUT C-1 USE 6x6 TREATED P= 2,904# NET UPLIFT = 426# USE DIAMOND PIER DP-50/50

C-2 USE 6 × 6 TREATED

P= 5, 206# NETUPLIFT = 854# USE DIAMOND PIER DP- 75/50"

COLUMN/ BEAM CONNECTION



(3) STRUCTURAL SCREWS 1/4" x 5"

OR

(3) 3/0" THREADED BOLTS.

- NOTCH SFAT IH GXG



#### MEMBER REPORT

### ROOF WITH 3 SPACES, Copy of Roof: TRUSS 1 piece(s) 11 7/8" TJI® 110 @ 16" OC



Member Length: 20' 1/16"

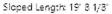
Building Use: Residential

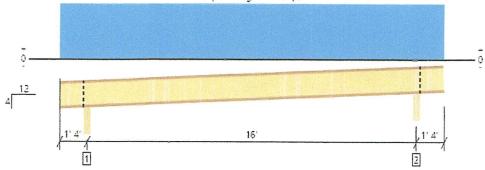
Building Code: IBC 2015

Design Methodology: ASD

Member Pitch: 4/12

System: Roof Member Type : Joist





All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

**Design Results** Actual @ Location Allowed Result LDF Load: Combination (Pattern) Member Reaction (lbs) 2346 (3.50") 571 @ 1' 4" Passed (24%) 1.15 1.0 D + 1.0 S (Adj Spans) Shear (lbs) 464 @ 1' 5 3/4" 1794 Passed (26%) 1.15 | 1.0 D + 1.0 S (Adj Spans) Moment (Ft-lbs) 1918 @ 9' 4" 3634 Passed (53%) 1.15 | 1.0 D + 1.0 S (Alt Spans) Live Load Defl. (in) 0.264 @ 9' 4" 0.843 Passed (L/766) 1.0 D + 1.0 S (Alt Spans) Total Load Defl. (in) 0.402 @ 9' 4" 1.124 Passed (L/504) 1.0 D + 1.0 S (Alt Spans)

- . Deflection criteria: LL (L/240) and TL (L/180).
- Overhang deflection criteria: LL (2L/240) and TL (2L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)			
	Total	Available	Required	Dead	Snow	Total	Accessories
1 - Beveled Plate - SPF	3.50"	3.50°	3.50"	197	374	571	Blocking
2 - Beveled Plate - SPF	3.50"	3.50°	3.50°	197	374	571	Blocking

Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	4' 1" o/c	
Bottom Edge (Lu)	7' 4" o/c	

<sup>•</sup>TJI joists are only analyzed using Maximum Allowable bracing solutions.

Maximum allowable bracing intervals based on applied load.

Vertical Load	Location	Spacing	Dead (0.90)	Snow (1.15)	Comments
1 - Uniform (PSF)	0 to 18' 8"	16"	15.0	30.0	Default Load

### Weyerhaeuser Notes

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

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File Name: ZUBKE PAVILLION



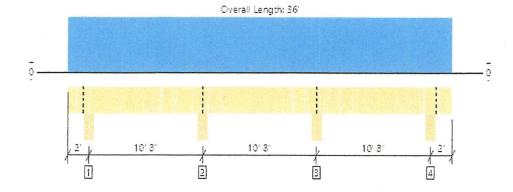
#### MEMBER REPORT

# ROOF WITH 3 SPACES, Copy of Roof: Drop Beam



PASSED

## 2 piece(s) 1 3/4" x 7 1/4" 2.0E Microllam® LVL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	5205 @ 12' 8"	13956 (5.50")	Passed (37%)		1.0 D + 1.0 S (Adj Spans)
Shear (lbs)	2385 @ 11' 10"	5544	Passed (43%)	1.15	1.0 D + 1.0 S (Adj Spans)
Moment (Ft-lbs)	-5123 @ 12' 8"	8182	Passed (63%)	1.15	1.0 D + 1.0 S (Adj Spans)
Live Load Defl. (in)	0.242 @ 6' 11 15/16"	0.533	Passed (L/529)		1.0 D + 1.0 S (Alt Spans)
Total Load Defl. (in)	0.345 @ 29' 11/16"	0.711	Passed (L/371)		1.0 D + 1.0 S (Alt Spans)

System : Roof Member Type : Drop Beam Building Use : Residential

Building Use: Residential Building Code: IBC 2015 Design Methodology: ASD Member Pitch: 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Overhang deflection criteria: LL (2L/240) and TL (2L/180).
- · Allowed moment does not reflect the adjustment for the beam stability factor.

	Be	Bearing Length			to Supports		
Supports	Total	Available	Required	Dead	Snow	Total	Accessories
1 - Column - SPF	5.50"	5.50"	1.50"	1007	1897	2904	Blocking
2 - Column - SPF	5.50"	5.50"	2.05"	1786	3420	5206	Blocking
3 - Column - SPF	5.50"	5.50"	2.05"	1786	3420	5206	Blocking
4 - Column - SPF	5.50"	5.50"	1.50"	1007	1897	2904	Blocking

Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	22' 7" o/c	
Bottom Edge (Lu)	17' 1" o/c	

<sup>•</sup>Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 36'	N/A	7.4		
1 - Uniform (PLF)	0 to 36' (Front)	N/A	147.8	280.5	Linked from: Copy of Roof: TRUSS, Support 1

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator







## RESIDENTIAL DIAMOND PIER LOAD CHART

IAS-Accredited Third-Party Bearing, Uplift, and Lateral Field Tests<sup>2</sup>

# Minimum 1500 psf

Silts/Clays (CL, ML, MH, CH)3

Bearing Load Capacity	☐ Equivalent Base Area	O Cylinder Comparison	♣ Frost Zone	Uplift Load Capacity	Lateral Load Capacity
2700#	1.8 sf	18" dia	24"	600#	600#
* 3000#	2.0 sf	19" dia	36"	* 900#	* 600#
3300#	2.2 sf	20" dia	48"	1200#	600#
* 3750#	2.5 sf	21" dia	48"	* 1400#	* 600#
4200#	2.8 sf	22" dia	60"	1600#	600#
	2700#  * 3000#  3300#  * 3750#	Capacity         □ Base Area           2700#         1.8 sf           * 3000#         2.0 sf           3300#         2.2 sf           * 3750#         2.5 sf	Capacity         Base Area         Comparison           2700#         1.8 sf         18" dia           * 3000#         2.0 sf         19" dia           3300#         2.2 sf         20" dia           * 3750#         2.5 sf         21" dia	Capacity         Base Area         Comparison         SE Zone           2700#         1.8 sf         18" dia         24"           * 3000#         2.0 sf         19" dia         36"           3300#         2.2 sf         20" dia         48"           * 3750#         2.5 sf         21" dia         48"	Capacity         Base Area         Comparison         Strain         Zone         Capacity           2700#         1.8 sf         18" dia         24"         600#           * 3000#         2.0 sf         19" dia         36"         * 900#           3300#         2.2 sf         20" dia         48"         1200#           * 3750#         2.5 sf         21" dia         48"         * 1400#

Equivalency to Traditional Concrete Footings

### Minimum 2000 psf

Sands/Gravels (SW, SP, SM, SC, GM, GC)3

Model / Pin No. / Length	Bearing Load Capacity	☐ Equivalent Base Area	O Cylinder Comparison	♣ Frost Zone	Uplift Load Capacity	Lateral Load Capacity
DP-50/36"	3600#	1.8 sf	18" dia	24"	600#	600#
DP-50/42"	* 4000#	2.0 sf	19" dia	36"	* 900#	* 600#
DP-50/50"	4400#	2.2 sf	20" dia	48"	1200#	600#
DP-75/50"	* 5600#	2.8 sf	22" dia	48"	* 1400#	* 600#
DP-75/63"	6400#	3.2 sf	24" dia	60"	1600#	600#

Equivalency to Traditional Concrete Footings

### Notes:

- This load chart is intended for simple structures supported by columns, posts, and beams loaded up to, but not
  exceeding, the stated capacities. It is not intended for structures with asymmetrical, rotational, overturning, or dynamic
  forces. Intended uses are described in section 2.0 of ICC-ES prescriptive bearing evaluation report ESR-1895. For
  projects that exceed the capacities or limitations defined herein, or the intended uses described in ESR-1895, contact
  PFI for additional information or site-specific capacity evaluation. See also the <u>Use and Applications</u> download at
  <a href="https://www.diamondpiers.com">www.diamondpiers.com</a>.
- 2. Capacities shown are tested to a Factor of Safety of 2, and are applicable in properly drained, normal sound soils only, with minimum soil bearing capacities as indicated. Copies of the field test reports are available from PFI upon request.
- 3. See IRC Table R401.4.1, "Presumptive Load-Bearing Values of Foundation Materials," for a full description of applicable 1500 psf and 2000 psf soil types. For soils below 1500 psf, or soils with unknown characteristics, additional site and design analysis is required. For soils above 2000 psf, the values in this chart shall apply.
- 4. All capacities use four pins of the specified length per foundation. Pin length includes that portion of the pin embedded within the concrete head. See "Check Your Layout" in the Diamond Pier Installation Manual for more information on pin/pier layout and spacing restrictions.
- 5. For professional engineers designing for short-term transient loads, contact PFI for further information.

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<sup>\*</sup>Interpolated from field test values.