

Storm Water & Erosion Control Calculations For:

# LUMIN TERRACE

### Watertown, WI

Excel Job # 240136200

November 8, 2024



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#### 0.0 Introduction

#### 0.1 Existing Conditions

The proposed development is located on the east side of Johnson Street in the city of Watertown, Wisconsin. The project site is bound by Johson Street to the west, Hoffman Road to the east, proposed development to the north, and vacant land to the south. The existing site currently contains trees. The site currently drains to the east to Hoffman Road. The existing site can be seen in Appendix A.

- Property Area: 9.33 acres

#### 0.2 Proposed Project Overview

The proposed project will include four proposed buildings with parking located primarily east and west of the proposed buildings. The proposed development will drain to inlets that will drain stormwater east to a proposed wet pond and infiltration basin. The stormwater management pond and infiltration basin will reduce peak flows and treat stormwater to meet local and state requirements. The wet pond will drain into an infiltration basin to meet local and state infiltration requirements. The proposed site can be seen in Appendix B.

- Disturbed Area: 8.90 acres

#### 1.0 Design Criteria

#### 1.1 Soils

Soil characteristics were determined using the web soil survey. See Table 1 for a summary of the soils and hydrologic ratings indicated by the web soil survey and Appendix D for web soil survey map.

MAP SYMBOL	SOIL TYPE	HYDROLOGIC RATING
Gth	Grellton fine sandy	B
GLD	loam	D
RtB	Rotamer loam	В
SoP	Sisson fine sandy	D
300	loam	В

#### Table 1: Web Soil Survey

Soil borings were completed for the project site. The boring logs can be seen in Appendix E.

#### 1.2 Rainfall Data

NOAA Atlas 14, city of Watertown rainfall depths with a MSE 3 distribution was used for stormwater calculations.

Table 2: NOAA Atlas 14 24-hour Rainfall Depth

DESIGN	RAINFALL DEPTH
1-YEAR	2.42
2-YEAR	2.73
100-YEAR	6.19

#### 2.0 Stormwater Management Requirements

#### 2.1 Peak Discharge

<u>**City of Watertown-**</u> Maintain or reduce the 1-yr and 100-yr, 24 hour post development peak runoff discharge rates to the 1-yr, and 2-yr, 24 hour pre development peak runoff discharge rates respectively.

**Wisconsin DNR-** Maintain or reduce the 1-yr and 2-yr, 24 hour post development peak runoff discharge rates to the 1-yr and 2-yr, 24 hour predevelopment peak runoff discharge rates respectively.

A wet pond will be used to reduce peak flows to predevelopment flows. Table 3: Runoff Summary

	PREDEVELOPMENT		POS	ST DEVELOPMENT	
DESIGN STORM	Peak Discharge (cfs)	To Pond (cfs)	Offsite (cfs)	Infiltration Basin discharge (cfs)	Peak Discharge (To Pond + Offsite) (cfs)
1YR-24 HR	2.99	16.89	0.60	0.87	0.89
2YR-24 HR	4.16	20.36	0.68	1.04	1.08
100YR-24HR	21.64	60.47	1.56	1.25	2.57

Table 4: Wet Pond Summary

DESIGN	POND RELEASE	STORAGE	MAXIMUM
STORM	RATE (CFS)	VOLUME (C.F.)	ELEVATION (FT)
1YR-24 HR	0.98	21,311	803.34
2YR-24 HR	1.40	25,146	803.54
100YR-24HR	2.12	90,528	806.42

#### Table 5: 100yr-24hr storm pond summary

POND	EMERGENCY SPILLWAY ELEVATION (FT)	CALCULATED POND ELEVATION (FT)	POND DISCHARGE IN (CFS)	DISCHARGE EXIT POINT
WET	806.50	806.42	2.12	INFILTRATION BASIN
INFIL	806.50	804.90	1.25	STORM SEWER

Table 6: Peak Discharge Release Summary

DESIGN STORM	PREDEVELOPMENT (CFS)	POST DEVELOPMENT (CFS)
1 YR- 24 HR	2.99	0.89
2 YR- 24 HR	4.16	1.08
100 YR- 24 HR	21.64	2.57

Table 6 shows that post development release rates will be less than predevelopment release rates for all design storms. See sheet C1.3 and C2.0 of the construction plans for pond design and Appendix C for peak discharge calculations.

#### Therefore, peak discharge requirements are met.

#### 2.2 Stormwater Quality

<u>Wisconsin DNR and city of Watertown</u>- The site is considered a new development project and will be required to remove 80% of total suspended solids (TSS) from site runoff.

The site will treat stormwater using a wet pond and infiltration basin. SLAMM analysis was used to determine the quantity of suspended solids that will be removed by the proposed wet pond and infiltration basin. The proposed site will create 2,423 lbs of TSS and the proposed wet pond and infiltration basin will reduce TSS to 252 lbs, which results in a 89.60% reduction in TSS release.

The proposed wet pond removes 89.60% of suspended solids which is greater than the required 80%.

#### Therefore, stormwater quality requirements have been met.

#### 2.3 Infiltration

<u>City of Watertown and Wisconsin DNR</u>- Infiltrate sufficient runoff volume so that the postdevelopment infiltration volume shall be at least 75% of the pre-development infiltration volume, based on an average annual rainfall. However, no more than 2% of the postconstruction site is required as an effective infiltration area.

The proposed development will be 387,800 sf, which means that a maximum 7,756 sf of area will be required for infiltration onsite. The proposed soils are classified as silt and have an estimated infiltration rate of 0.50 in/hr respectively per the Wisconsin DNR evaluation for infiltration standard (1002). Since the basin must empty within 24 hrs after the storm the basin be 12" deep (0.5 in/hr \* 24 hr= 12 in).

There is one infiltration basin proposed onsite. The infiltration basin will be downstream of the proposed wet pond. The basin will be 15,000 sf and 12" deep.

#### Therefore, Infiltration requirements are met.

### 3.0 Storm Sewer Design

All storm sewer has been designed to convey the 100-year 24-hour post development storm.

See Appendix F, Appendix G, and Appendix H for pipe drainage areas and pipe sizing calculations.

#### 3.1 Emergency Overflow Route

The emergency overflow route is to the east, over the curb and gutter. Maximum ponding onsite will be 9" in drive aisles and 6" in parking stalls.

### 4.0 Erosion Control

The erosion control specifications, construction sequence, site stabilization notes, seeding notes, dewatering notes, and post construction and maintenance plan will be included on sheet C0.2 of the construction plan set.

Appendix A: Pre-Development Basin Area(s)





### ROCK RIVER RIDGE ENDSECTION IE = 1801.00

PRE BASIN	TOTAL (SF)	TOTAL (AC)	BLDG (SF)	BLDG (AC)	PAVEMENT (SF)	PAVEMENT (AC)	OPEN (SF)	OPEN (AC)
Α	373,638	8.58	0	0.00	37,307	0.86	336,331	7.72

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24" RCP INV. = 791.66

SANITARY MANHOLE RIM = 796.99 48" PVC N INV. = 783.30 60" RCP E & S (COULD NOT MEASURE) SANITARY MANHOLE I RIM = 797.00 48" PVC N INV. = 783.00 60" RCP E & S INV. = 780.86



PREDEVELOPMENT BASIN AREA



## Appendix B: Post Development Basin Area(s)



POST BASIN		TOTAL (SF)	TOTAL (AC)	BLDG (SF)	BLDG (AC)	PAVEMENT (SF)	PAVEMENT (AC)	0
Α		312,698	7.18	49,429	1.13	124,161	2.85	139
В		20,550	0.47	8,643	0.20	0	0.00	11,90
	С	32,761	0.75	0	0.00	583	0.01	32,178
	D	7,629	0.18	0	0.00	7,629	0.18	0



POST DEVELOPMENT BASIN AREAS





## Appendix C: Peak Discharge Calculations



#### Area Listing (selected nodes)

	Area	CN	Description
(a	cres)		(subcatchment-numbers)
Ę	5.228	98	(1S, 2S, 4S, 5S, 8S)
(	0.273	74	(2S)
7	7.721	70	(8S)
4	4.002	74	>75% Grass cover, Good, HSG C (1S, 4S)

#### Soil Listing (selected nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
0.000	HSG B	
4.002	HSG C	1S, 4S
0.000	HSG D	
13.223	Other	1S, 2S, 4S, 5S, 8S

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment	
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers	
0.000	0.000	0.000	0.000	13.223	13.223		1S, 2S,	
							4S, 5S,	
							8S	
0.000	0.000	4.002	0.000	0.000	4.002	>75% Grass cover, Good	1S, 4S	

#### Ground Covers (selected nodes)

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: A		Runoff Area=312,698 sf 55.51% Impervious Runoff Depth>1 Tc=6.0 min CN=87 Runoff=15.99 cfs 0.7	1.19" 12 af
Subcatchment2S: B		Runoff Area=20,550 sf 42.06% Impervious Runoff Depth>1 Tc=6.0 min CN=84 Runoff=0.89 cfs 0.04	1.00" 40 af
Subcatchment4S: C		Runoff Area=35,816 sf 1.63% Impervious Runoff Depth>0 Tc=0.0 min CN=74 Runoff=0.92 cfs 0.03	).53" 36 af
Subcatchment5S: D		Runoff Area=7,629 sf 100.00% Impervious Runoff Depth>2 Tc=6.0 min CN=98 Runoff=0.60 cfs 0.03	2.12" 31 af
Subcatchment8S: PRE	Flow Length=433'	Runoff Area=373,638 sf 9.98% Impervious Runoff Depth>0 Slope=0.0600 '/' Tc=37.9 min CN=73 Runoff=2.99 cfs 0.34	).48" 46 af
Pond 3P: WET POND		Peak Elev=803.34' Storage=21,311 cf Inflow=16.89 cfs 0.75 Outflow=0.98 cfs 0.4	52 af 36 af
Pond 6P: INFILTRATION	<b>IBASIN</b> Discarded=0.05 c	Peak Elev=800.66' Storage=2,864 cf Inflow=1.05 cfs 0.4 cfs 0.035 af Primary=0.87 cfs 0.400 af Outflow=0.93 cfs 0.43	72 af 35 af
Link 7L: POST		Inflow=0.89 cfs 0.4	31 af

Primary=0.89 cfs 0.431 af

#### Summary for Subcatchment 1S: A

Runoff = 15.99 cfs @ 12.13 hrs, Volume= Routed to Pond 3P : WET POND 0.712 af, Depth> 1.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1-YEAR Rainfall=2.42"

	Area (sf)	CN	Description							
*	49,429	98								
*	124,161	98								
	139,108	74	>75% Gras	s cover, Go	ood, HSG C					
	312,698	87	Weighted A	verage						
	139,108	108 44.49% Pervious Area								
	173,590		55.51% Imp	ervious Are	ea					
_(	Tc Length min) (feet)	Slor (ft/	be Velocity ft) (ft/sec)	Capacity (cfs)	Description					
	<u> </u>				Dine of Fratma					



Direct Entry,

#### Subcatchment 1S: A



#### Summary for Subcatchment 2S: B

Runoff = 0.89 cfs @ 12.14 hrs, Volume= Routed to Pond 3P : WET POND 0.040 af, Depth> 1.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1-YEAR Rainfall=2.42"



#### Summary for Subcatchment 4S: C

Runoff = 0.92 cfs @ 12.06 hrs, Volume= Routed to Pond 6P : INFILTRATION BASIN 0.036 af, Depth> 0.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1-YEAR Rainfall=2.42"

	Area (sf)	CN	Description
*	583	98	
	35,233	74	>75% Grass cover, Good, HSG C
	35,816	74	Weighted Average
	35,233		98.37% Pervious Area
	583		1.63% Impervious Area



#### Subcatchment 4S: C

#### Summary for Subcatchment 5S: D

Runoff = 0.60 cfs @ 12.13 hrs, Volume= Routed to Link 7L : POST

0.031 af, Depth> 2.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1-YEAR Rainfall=2.42"



#### Summary for Subcatchment 8S: PRE

Runoff = 2.99 cfs @ 12.61 hrs, Volume= 0.346 af, Depth> 0.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 1-YEAR Rainfall=2.42"

	A	rea (sf)	CN I	Description		
*	3	36,331	70			
*		37,307	98			
	3	73,638	73 \	Weighted A	verage	
	3	36,331	ę	90.02% Pei	vious Area	
		37,307	ę	9.98% Impe	ervious Area	a
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	36.1	300	0.0600	0.14		Sheet Flow,
	1.8	133	0.0600	1.22		Woods: Light underbrush n= 0.400 P2= 2.73" <b>Shallow Concentrated Flow,</b> Woodland Kv= 5.0 fps
	37.9	433	Total			

#### Subcatchment 8S: PRE



#### Summary for Pond 3P: WET POND

Inflow Are	a =	7.650 ac, 5	54.68% Imperv	vious, Inflow	Depth > 1	1.18"	for 1-YE	AR event	
Inflow	=	16.89 cfs @	12.13 hrs, Vo	olume=	0.752 a	f			
Outflow	=	0.98 cfs @	13.41 hrs, Vo	olume=	0.436 a	f, Atter	า= 94%,	Lag= 76.6 m	in
Primary	=	0.98 cfs @	13.41 hrs, Vo	olume=	0.436 a	f		-	
Routed	l to Pon	nd 6P : INFILT	RATION BASI	N					

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 803.34' @ 13.41 hrs Surf.Area= 18,345 sf Storage= 21,311 cf

Plug-Flow detention time= 207.2 min calculated for 0.436 af (58% of inflow) Center-of-Mass det. time= 145.1 min ( 929.4 - 784.3 )

Volume	Inve	rt Avail.Sto	rage Storage	Description	
#1	802.0	0' 120,65	50 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevatio	on S	Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
802.0	00	13,000	0	0	
803.0	00	17,500	15,250	15,250	
804.0	00	20,000	18,750	34,000	
805.0	00	22,900	21,450	55,450	
806.0	00	25,500	24,200	79,650	
807.00		28,000	26,750	106,400	
807.50 29,00		29,000	14,250	120,650	
Device	Routing	Invert	Outlet Devices	6	
#1	Primary	802.00'	10.0" Round	Culvert	
	,		L= 546.0' CN	IP, projecting, n	o headwall, Ke= 0.900
			Inlet / Outlet Ir	nvert= 802.00' /	800.00' S= 0.0037 '/' Cc= 0.900
			n= 0.013 Corr	rugated PE, sm	ooth interior, Flow Area= 0.55 sf
#2	Device 1	802.00'	20.0 deg Sha	rp-Crested Vee	e/Trap Weir Cv= 2.69 (C= 3.36)
#3	Primary	806.50'	10.0' long x 1	10.0' breadth B	road-Crested Rectangular Weir
			Head (feet) 0.	.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60
			Coef. (English	) 2.49 2.56 2.	70 2.69 2.68 2.69 2.67 2.64
#4 Device 1 805.00' 24.0" x 36.0" Horiz. Orifice/Grate C= 0.600					
			Limited to weir	r flow at low hea	ads

**Primary OutFlow** Max=0.98 cfs @ 13.41 hrs HW=803.34' (Free Discharge)

-1=Culvert (Passes 0.98 cfs of 1.42 cfs potential flow)

2=Sharp-Crested Vee/Trap Weir (Weir Controls 0.98 cfs @ 3.11 fps)

**4=Orifice/Grate** (Controls 0.00 cfs)

-3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 3P: WET POND



#### Summary for Pond 6P: INFILTRATION BASIN

Inflow Area	ı =	8.473	ac, 49	9.54% Imp	pervious,	Inflow	Depth >	0.67'	' for	1-YE	AR eve	ent
Inflow	=	1.05 cf	s @	13.25 hrs,	Volume	;=	0.472	af				
Outflow	=	0.93 cf	s @	14.24 hrs,	Volume	;=	0.435	af, A	tten= ^	12%,	Lag= 5	9.9 min
Discarded	=	0.05 cf	s @	14.24 hrs,	Volume	;=	0.035	af			-	
Primary	=	0.87 cf	s @	14.24 hrs,	Volume	;=	0.400	af				
Routed	to Link 7	7L : PO	ST									

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 800.66' @ 14.24 hrs Surf.Area= 4,661 sf Storage= 2,864 cf

Plug-Flow detention time= 52.2 min calculated for 0.433 af (92% of inflow) Center-of-Mass det. time= 31.0 min (951.1 - 920.1)

Volume	Invert	Avail.Sto	rage Stora	age Description	
#1	800.00'	62,8	50 cf <b>Cus</b> t	tom Stage Data (P	rismatic)Listed below (Recalc)
Elevation	S.	urf Aroo	Ino Store	Cum Store	
Elevation	51	un.Area			
(teet)		(sq-π)	(cubic-teet	) (cubic-teet)	
800.00		4,000	(	) 0	
801.00		5,000	4,500	) 4,500	
802.00 6,1		6,100	5,550	) 10,050	
803.00		7,500	6,800	) 16,850	
804.00		8,500	8,000	) 24,850	
805.00		10,000	9,250	) 34,100	
806.00		11,000	10,500	) 44,600	
807.00		12,500	11.750	56.350	
807.50		13,500	6,500	) 62,850	
Device F	Routing	Invert	Outlet Dev	vices	
#1 F	Primary	800.00'	8.0" Rou	nd Culvert	
	,		L= 391.0'	CMP, end-section	conforming to fill. Ke= 0.500
			Inlet / Out	let Invert= 800.00' /	794.00' S= 0.0153 '/' Cc= 0.900
			n = 0.020	Corrugated PF cor	rugated interior Flow Area= 0.35 sf
#2 F	Primary	806 50'	10.0' long	x 10.0' breadth F	Broad-Crested Rectangular Weir
<i>"-</i> '	mary	000.00	Head (fee	t) 0 20 0 40 0 60	
			Coef (En	$(10.20 \ 0.40 \ 0.00)$	70 2 60 2 68 2 60 2 67 2 64
#з г	Discarded	800 00'	0 500 in/h	r Exfiltration over	Surface area
#3 L	Jiscalueu	000.00	Conductiv	ity to Groundwater	Elevation = 700.00'
			Conductiv	ity to Groundwater	

**Discarded OutFlow** Max=0.05 cfs @ 14.24 hrs HW=800.66' (Free Discharge) **3=Exfiltration** (Controls 0.05 cfs)

Primary OutFlow Max=0.87 cfs @ 14.24 hrs HW=800.66' (Free Discharge) 1=Culvert (Barrel Controls 0.87 cfs @ 3.13 fps) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)



Pond 6P: INFILTRATION BASIN

#### Summary for Link 7L: POST

Inflow /	Area =	8.648 ac, 5	50.56% Impervious,	Inflow Depth > 0	0.60" for 1-YEAR event
Inflow	=	0.89 cfs @	14.23 hrs, Volume	= 0.431 a	af
Primar	y =	0.89 cfs @	14.23 hrs, Volume	= 0.431 a	af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



#### Link 7L: POST

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: A		Runoff Area=312	,698 sf 55.5 Tc=6.0 min	1% Impervio CN=87 R	ous Runoff De Runoff=19.27 cf	∍pth>1.44" s_0.863 af
Subcatchment2S: B		Runoff Area=20	,550 sf 42.00 Tc=6.0 min	6% Impervio CN=84 I	ous Runoff De Runoff=1.10 cf	epth>1.24" s_0.049 af
Subcatchment4S: C		Runoff Area=3	5,816 sf 1.63 Tc=0.0 min	3% Impervio CN=74 I	ous Runoff De Runoff=1.24 cf	epth>0.70" s_0.048 af
Subcatchment5S: D		Runoff Area=7,6	629 sf 100.00 Tc=6.0 min	0% Impervio CN=98 I	ous Runoff De Runoff=0.68 cf	∍pth>2.41" s_0.035 af
Subcatchment8S: PRE	Flow Length=433'	Runoff Area=37 Slope=0.0600 '/'	3,638 sf 9.98 Tc=37.9 min	3% Impervio CN=73 F	ous Runoff De Runoff=4.16 cfs	epth>0.65" s_0.462 af
Pond 3P: WET POND		Peak Elev=803.5	54' Storage=2	25,146 cf li C	nflow=20.36 cf Dutflow=1.40 cf	s 0.912 af s 0.573 af
Pond 6P: INFILTRATION	<b>IBASIN</b> Discarded=0.06 c	Peak Elev=80 fs 0.036 af Prima	1.09' Storage ary=1.04 cfs	e=4,971 cf 0.543 af O	Inflow=1.50 cf outflow=1.10 cfs	s  0.621 af s  0.579 af
Link 7L: POST					Inflow=1.08 cf	s 0.578 af

Inflow=1.08 cfs 0.578 af Primary=1.08 cfs 0.578 af

#### Summary for Subcatchment 1S: A

Runoff = 19.27 cfs @ 12.13 hrs, Volume= Routed to Pond 3P : WET POND 0.863 af, Depth> 1.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2-YEAR Rainfall=2.73"

	Area (sf)	CN	Description	
*	49,429	98		
*	124,161	98		
	139,108	74	>75% Grass cover, Good, HSG C	
	312,698	87	Weighted Average	
	139,108		44.49% Pervious Area	
	173,590		55.51% Impervious Area	
(	Tc Length min) (feet)	Slop (ft/	e Velocity Capacity Description t) (ft/sec) (cfs)	



Direct Entry,

#### Subcatchment 1S: A



#### Summary for Subcatchment 2S: B

Runoff = 1.10 cfs @ 12.13 hrs, Volume= Routed to Pond 3P : WET POND 0.049 af, Depth> 1.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2-YEAR Rainfall=2.73"

	Ar	ea (sf)	CN	D	escrip	otion											
*		8,643	98														
*		11,907	74														
	2	20,550	84	W	/eight	ed Av	/erage	;									
		11,907		57	7.94%	Per	vious /	Area									
		8,643		42	2.06%	o Imp	erviou	s Are	а								
	Тс	Length	Slo	ре	Velo	city	Capa	city	Descr	iption	n						
(m	nin)	(feet)	(ft/	′ft)	(ft/s	ec)	(0	cfs)									
(	6.0								Direc	t Enti	ry,						
							6.	haa	tohm	ont f	20. E	•					
							30	inca	lCIIII	ent	23. E	)					
							H	lydrog	raph		1	1					
	ſ		l I	Ì	1				 		1	 					Runoff
								1.10 c	fs		   	   					
				 		 					 +	 	MS	5E 2	4-h	r 3	
	1									2.	-YE/	AR F	Rain	fall	=2.7	3"	
			1		I					D	uno	ff A	root	-20	660	cf	
					1					_ <b>N</b>	uno		iea-	-20,	550	51	
				i	Ì					Rui	noff	Vol	ume	<b>=</b> 0.	049	af	
cfs)					1						Ru	nof	f Dei	oth	>1.2	4"	
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	0- <mark> </mark> - 5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	
								rime	(nours)								

#### Summary for Subcatchment 4S: C

Runoff = 1.24 cfs @ 12.06 hrs, Volume= Routed to Pond 6P : INFILTRATION BASIN 0.048 af, Depth> 0.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2-YEAR Rainfall=2.73"

	Area (sf)	CN	Description
*	583	98	
	35,233	74	>75% Grass cover, Good, HSG C
	35,816	74	Weighted Average
	35,233		98.37% Pervious Area
	583		1.63% Impervious Area

#### Subcatchment 4S: C



#### Summary for Subcatchment 5S: D

Runoff = 0.68 cfs @ 12.13 hrs, Volume= Routed to Link 7L : POST 0.035 af, Depth> 2.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2-YEAR Rainfall=2.73"



#### Summary for Subcatchment 8S: PRE

Runoff = 4.16 cfs @ 12.59 hrs, Volume= 0.462 af, Depth> 0.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 2-YEAR Rainfall=2.73"

	A	rea (sf)	CN I	Description		
*	3	36,331	70			
*		37,307	98			
	3	73,638	73 \	Neighted A	verage	
	3	36,331	ę	90.02% Pei	vious Area	
37,307 9.98%			9.98% Impe	ervious Area	а	
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	36.1	300	0.0600	0.14		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.73"
	1.8	133	0.0600	1.22		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	37.9	433	Total			

#### Subcatchment 8S: PRE



#### Summary for Pond 3P: WET POND

Inflow Area = 7.650 ac, 54.68% Impervious, Inflow Depth > 1.43" for 2-YEAR event   Inflow = 20.36 cfs @ 12.13 hrs, Volume= 0.912 af   Outflow = 1.40 cfs @ 13.21 hrs, Volume= 0.573 af, Atten= 93%, Lag= 64.7 min   Primary = 1.40 cfs @ 13.21 hrs, Volume= 0.573 af   Routed to Pond 6P : INFILTRATION BASIN 0.573 af											
Routing by	Pouting by Stor Ind method. Time Span= 5.00.20.00 brs. dt= 0.05 brs										
Peak Flev=	803 54' @ 13	21 hrs Surf	Area= 18 8	861 sf Stor	$a_{0} = 25146$	cf					
			104 10,0		ugo 20,110						
Plug-Flow c Center-of-N	Plug-Flow detention time= 196.3 min calculated for 0.573 af (63% of inflow) Center-of-Mass det. time= 137.4 min ( 918.5 - 781.2 )										
Volume	Invert A	vail.Storage	Storage	Description							
#1	802.00'	120,650 cf	Custom	Stage Data	(Prismatic)	isted below (Recalc)					
Elevation (feet)	Surf.Are (sq-f	a Inc t) (cubi	c.Store c-feet)	Cum.Sto (cubic-fe	ore et)						
802.00	13,00	00	0		0						
803.00	17,50	· 00	15,250	15,2	50						
804.00	20,00	. 00	18,750	34,0	00						
805.00	22,90	0 2	21,450	55,4	50						
806.00	25,50	0 2	24,200	79,6	50						

Device	Routing	Invert	Outlet Devices
#1	Primary	802.00'	10.0" Round Culvert
			L= 546.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 802.00' / 800.00' S= 0.0037 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.55 sf
#2	Device 1	802.00'	20.0 deg Sharp-Crested Vee/Trap Weir Cv= 2.69 (C= 3.36)
#3	Primary	806.50'	10.0' long x 10.0' breadth Broad-Crested Rectangular Weir
	-		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60
			Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64
#4	Device 1	805.00'	24.0" x 36.0" Horiz. Orifice/Grate C= 0.600
			Limited to weir flow at low heads

106,400

120,650

Primary OutFlow Max=1.41 cfs @ 13.21 hrs HW=803.54' (Free Discharge)

26,750

14,250

-1=Culvert (Passes 1.41 cfs of 1.48 cfs potential flow)

2=Sharp-Crested Vee/Trap Weir (Weir Controls 1.41 cfs @ 3.34 fps)

-4=Orifice/Grate (Controls 0.00 cfs)

28,000

29,000

807.00

807.50

-3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 3P: WET POND



#### Summary for Pond 6P: INFILTRATION BASIN

Inflow Area	a =	8.473 ac,	49.54% Imp	ervious,	Inflow	Depth >	0.88	" for	2-YE	AR ev	ent
Inflow	=	1.50 cfs @	2 13.04 hrs,	Volume	;=	0.621	af				
Outflow	=	1.10 cfs @	) 13.34 hrs,	Volume	;=	0.579	af, A	Atten=	27%,	Lag= <sup>2</sup>	17.9 min
Discarded	=	0.06 cfs @	) 13.34 hrs,	Volume	;=	0.036	af			-	
Primary	=	1.04 cfs @	) 13.34 hrs,	Volume	;=	0.543	af				
Routed	to Link 7	L : POST									

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 801.09' @ 15.10 hrs Surf.Area= 5,103 sf Storage= 4,971 cf

Plug-Flow detention time= 62.0 min calculated for 0.577 af (93% of inflow) Center-of-Mass det. time= 43.2 min (952.9 - 909.6)

Volume	Invert	Avail.Stor	rage Storag	e Description				
#1	800.00	62,85	50 cf Custor	m Stage Data (P	rismatic)Listed below (Recalc)			
Elevation	n S	urf.Area	Inc.Store	Cum.Store				
(feet	)	(sq-ft)	(cubic-feet)	(cubic-feet)				
800.00	)	4,000	0	0				
801.00	)	5,000	4,500	4,500				
802.00	)	6,100	5,550	10,050				
803.00	)	7,500	6,800	16,850				
804.00	)	8,500	8,000	24,850				
805.00	)	10,000	9,250	34,100				
806.00	)	11,000	10,500	44,600				
807.00	)	12,500	11,750	56,350				
807.50	)	13,500	6,500	62,850				
Device	Routing	Invert	Outlet Devic	es				
#1	Primarv	800.00'	8.0" Round	l Culvert				
,, .	, initial y		L= 391.0' C Inlet / Outlet n= 0.020 Cc	CMP, end-section Invert= 800.00' / prrugated PE, cor	conforming to fill, Ke= 0.500 794.00' S= 0.0153 '/' Cc= 0.900 rugated interior, Flow Area= 0.35 sf			
#2 Primary 8		806.50'	<b>10.0' long x 10.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64					
#3 Discarded 800.00'		<b>0.500 in/hr Exfiltration over Surface area</b> Conductivity to Groundwater Elevation = 700.00'						

**Discarded OutFlow** Max=0.06 cfs @ 13.34 hrs HW=800.83' (Free Discharge) **3=Exfiltration** (Controls 0.06 cfs)

Primary OutFlow Max=1.04 cfs @ 13.34 hrs HW=800.83' (Free Discharge) 1=Culvert (Barrel Controls 1.04 cfs @ 3.09 fps) 2=Broad-Crested Rectangular Weir(Controls 0.00 cfs)



#### Pond 6P: INFILTRATION BASIN
#### Summary for Link 7L: POST

Inflow A	vrea =	8.648 ac, 5	50.56% Impervious,	Inflow Depth > 0	.80" for 2-YEAR event
Inflow	=	1.08 cfs @	13.33 hrs, Volume	= 0.578 af	
Primary		1.08 cfs @	13.33 hrs, Volume	= 0.578 af	, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



#### Link 7L: POST

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment1S: A	Runoff Area=312,698 sf 55.51% Impervious Runoff Depth>4.54" Tc=6.0 min CN=87 Runoff=56.92 cfs 2.716 af
Subcatchment2S: B	Runoff Area=20,550 sf 42.06% Impervious Runoff Depth>4.22" Tc=6.0 min CN=84 Runoff=3.55 cfs 0.166 af
Subcatchment4S: C	Runoff Area=35,816 sf  1.63% Impervious  Runoff Depth>3.21" Tc=0.0 min  CN=74  Runoff=5.72 cfs  0.220 af
Subcatchment5S: D	Runoff Area=7,629 sf 100.00% Impervious Runoff Depth>5.70" Tc=6.0 min CN=98 Runoff=1.56 cfs 0.083 af
Subcatchment8S: PRE Flow Length=433'	Runoff Area=373,638 sf 9.98% Impervious Runoff Depth>3.08" Slope=0.0600 '/' Tc=37.9 min CN=73 Runoff=21.64 cfs 2.202 af
Pond 3P: WET POND	Peak Elev=806.42' Storage=90,528 cf Inflow=60.47 cfs 2.882 af Outflow=2.12 cfs 1.400 af
Pond 6P: INFILTRATIONBASIN Discarded=0.12	Peak Elev=804.90' Storage=33,065 cf Inflow=7.36 cfs 1.620 af cfs 0.069 af Primary=1.25 cfs 0.790 af Outflow=1.37 cfs 0.860 af
Link 7L: POST	Inflow=2.57 cfs 0.874 af Primary=2.57 cfs 0.874 af

#### Summary for Subcatchment 1S: A

Runoff = 56.92 cfs @ 12.13 hrs, Volume= Routed to Pond 3P : WET POND 2.716 af, Depth> 4.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100-YEAR Rainfall=6.19"

	Area (sf)	CN	Description		
*	49,429	98			
*	124,161	98			
	139,108	74	>75% Gras	s cover, Go	Good, HSG C
	312,698	87	Weighted A	verage	
	139,108		44.49% Per	vious Area	а
	173,590		55.51% Imp	ervious Are	rea
(	Tc Length min) (feet)	Slop (ft/f	e Velocity t) (ft/sec)	Capacity (cfs)	/ Description
	6.0				Direct Entry,

#### Subcatchment 1S: A



#### Summary for Subcatchment 2S: B

Runoff = 3.55 cfs @ 12.13 hrs, Volume= Routed to Pond 3P : WET POND 0.166 af, Depth> 4.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100-YEAR Rainfall=6.19"

	A	rea (sf)	CN	D	escrip	otion											
*		8,643	98														
*		11,907	74														
		20,550	84	W	/eighte	ed Av	/erage	;									
		11,907		57	7.94%	Perv	vious A	Area									
		8,643		42	2.06%	lmp	erviou	s Are	ea								
	Тс	Length	Slo	pe	Velo	city	Capa	city	Desc	ription	n						
(r	nin)	(feet)	(ft/	ft)	(ft/s	ec)	(0	cfs)									
	6.0								Direc	t Ent	ry,						
							Su	ıbca	tchm	nent	2S: E	3					
							н	lydrog	graph								
	ĺ																Runoff
	-	1	1	i				3.55	cfs	1			RAC				
	-							ľ		1					4-n	r 3	
		/	<u> </u>		<u> </u>					100	-YE/	AR F	Rain	fall	=6.1	9"	
	J				1					R	uno	ff A	rea=	=20.	550	sf	
				1						Ru	noff	Vol	ume	)=0.	166	af	
fs)	• ]		     + -	   	   + +		      +		<b>_</b>		Du	nof	F-Do	nth <sup>.</sup>	- 1-9	ייי סיי	
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	1-							K	A								
	-									   							
						m				$D \pi$							
	0-	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	
								Time	(hours)	)							

#### Summary for Subcatchment 4S: C

Runoff = 5.72 cfs @ 12.05 hrs, Volume= Routed to Pond 6P : INFILTRATION BASIN 0.220 af, Depth> 3.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100-YEAR Rainfall=6.19"

	Area (sf)	CN	Description
*	583	98	
	35,233	74	>75% Grass cover, Good, HSG C
	35,816	74	Weighted Average
	35,233		98.37% Pervious Area
	583		1.63% Impervious Area

#### Subcatchment 4S: C



#### Summary for Subcatchment 5S: D

Runoff = 1.56 cfs @ 12.13 hrs, Volume= Routed to Link 7L : POST 0.083 af, Depth> 5.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100-YEAR Rainfall=6.19"



#### Summary for Subcatchment 8S: PRE

Runoff = 21.64 cfs @ 12.53 hrs, Volume= 2.202 af, Depth> 3.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs MSE 24-hr 3 100-YEAR Rainfall=6.19"

_	A	rea (sf)	CN I	Description		
*	3	36,331	70			
*		37,307	98			
	3	73,638	73	Neighted A	verage	
336,331 90.02% Pervious Area					vious Area	
37,307 9.98% Impervious Are						a
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
	36.1	300	0.0600	0.14		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 2.73"
	1.8	133	0.0600	1.22		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	37.0	133	Total			

#### Subcatchment 8S: PRE



#### Summary for Pond 3P: WET POND

Inflow Area	a =	7.650 ac, 5	54.68% Imp	ervious,	Inflow Depth >	4.5	2" for	100-	YEAR	event
Inflow	=	60.47 cfs @	12.13 hrs,	Volume	= 2.882	af				
Outflow	=	2.12 cfs @	13.60 hrs,	Volume	= 1.400	af,	Atten= 9	96%,	Lag=	88.2 min
Primary	=	2.12 cfs @	13.60 hrs,	Volume	= 1.400	af			-	
Routed	to Pon	d 6P : INFILT	RATION BA	SIN						

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 806.42' @ 13.60 hrs Surf.Area= 26,545 sf Storage= 90,528 cf

Plug-Flow detention time= 250.8 min calculated for 1.395 af (48% of inflow) Center-of-Mass det. time= 187.5 min ( 949.2 - 761.7 )

Volume	Inver	t Avail.Sto	rage Storage	Description	
#1	802.00	0' 120,65	50 cf Custom	Stage Data (P	rismatic)Listed below (Recalc)
Elevatio	on S	Surf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
802.0	00	13,000	0	0	
803.0	00	17,500	15,250	15,250	
804.0	00	20,000	18,750	34,000	
805.0	00	22,900	21,450	55,450	
806.0	00	25,500	24,200	79,650	
807.0	)0	28,000	26,750	106,400	
807.5	50	29,000	14,250	120,650	
Device	Pouting	Invert	Outlet Devices	-	
	Routing			5 Oraliant	
#1	Primary	802.00			
			$L=546.0^{\circ}$ CIV	IP, projecting, n	
					800.00 = 0.0037 / CC = 0.900
#0	Davias 1	902 001		rugaled PE, sm	Ooln Interior, Flow Area = $0.55$ Si
#Z #2	Device I	802.00	20.0 deg Snal	rp-Crested vee	P (C= 3.36)
#3	Primary	00.00			
			Coof (English	20 0.40 0.00	
#1	Dovice 1	905 00'		) 2.49 2.30 2.	70 2.09 2.00 2.09 2.07 2.04
#4	Device I	605.00	<b>24.0 X 30.0</b>	r flow of low box	
					205

**Primary OutFlow** Max=2.12 cfs @ 13.60 hrs HW=806.42' (Free Discharge)

-1=Culvert (Barrel Controls 2.12 cfs @ 3.89 fps)

**2=Sharp-Crested Vee/Trap Weir** (Passes < 19.46 cfs potential flow)

**4=Orifice/Grate** (Passes < 34.40 cfs potential flow)

-3=Broad-Crested Rectangular Weir(Controls 0.00 cfs)

#### hydrocad240136200

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Pond 3P: WET POND



#### Summary for Pond 6P: INFILTRATION BASIN

Inflow Area	a =	8.473 ac,	49.54% Impe	ervious,	Inflow	Depth >	2.2	9" fo	r 100-	YEAR e	vent
Inflow	=	7.36 cfs @	12.05 hrs,	Volume	=	1.620	af				
Outflow	=	1.37 cfs @	20.00 hrs,	Volume	=	0.860	af, J	Atten=	81%,	Lag= 47	7.0 min
Discarded	=	0.12 cfs @	20.00 hrs,	Volume	=	0.069	af			•	
Primary	=	1.25 cfs @	20.00 hrs,	Volume	=	0.790	af				
Routed	to Link	7L : POST									

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 804.90' @ 20.00 hrs Surf.Area= 9,843 sf Storage= 33,065 cf

Plug-Flow detention time= 159.2 min calculated for 0.857 af (53% of inflow) Center-of-Mass det. time= 35.7 min (961.5 - 925.8)

Volume	Invert	t Avail.Sto	rage Storage	Description	
#1	800.00	' 62,85	50 cf Custom	n Stage Data (Pi	r <b>ismatic)L</b> isted below (Recalc)
Elovatio	n c		Inc Store	Cum Stora	
	n 3		(cubic foot)	(cubic foot)	
	<u>,</u>	(SQ-IL)			
800.0	0	4,000	0	0	
801.0	0	5,000	4,500	4,500	
802.0	0	6,100	5,550	10,050	
803.0	0	7,500	6,800	16,850	
804.0	0	8,500	8,000	24,850	
805.0	0	10,000	9,250	34,100	
806.0	0	11,000	10,500	44,600	
807.0	0	12,500	11,750	56,350	
807.5	0	13,500	6,500	62,850	
Device	Pouting	Invert	Outlet Device		
#1	Primary	800.00'	8.0" Round L= 391.0' Cl Inlet / Outlet I	Culvert MP, end-section Invert= 800.00' / rrugated PE_cor	conforming to fill, Ke= 0.500 794.00' S= 0.0153 '/' Cc= 0.900 rugated interior _ Flow Area= 0.35 sf
#2	Primary	806.50'	<b>10.0' long x</b> Head (feet) (Coef. (Englis)	<b>10.0' breadth B</b> 0.20 0.40 0.60 h) 2.49 2.56 2.	road-Crested Rectangular Weir 0.80 1.00 1.20 1.40 1.60 70 2.69 2.68 2.69 2.67 2.64
#3	Discarded	800.00'	0.500 in/hr E Conductivity	<b>xfiltration over</b> to Groundwater I	Surface area Elevation = 700.00'

**Discarded OutFlow** Max=0.12 cfs @ 20.00 hrs HW=804.90' (Free Discharge) **3=Exfiltration** (Controls 0.12 cfs)

Primary OutFlow Max=1.25 cfs @ 20.00 hrs HW=804.90' (Free Discharge) 1=Culvert (Barrel Controls 1.25 cfs @ 3.58 fps) 2=Broad-Crested Rectangular Weir(Controls 0.00 cfs)



#### Pond 6P: INFILTRATION BASIN

#### Summary for Link 7L: POST

Inflow A	rea =	8.648 ac, 5	0.56% Impervious,	Inflow Depth > 1.	21" for 100-YEAR event
Inflow	=	2.57 cfs @	12.13 hrs, Volume	= 0.874 af	
Primary	=	2.57 cfs @	12.13 hrs, Volume	e= 0.874 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



#### Link 7L: POST

## Appendix D: Web Soil Survey Map



United States Department of Agriculture

Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

## Custom Soil Resource Report for Jefferson County, Wisconsin



### Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2\_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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### **How Soil Surveys Are Made**

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

### Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

#### Custom Soil Resource Report Soil Map



	MAP L	EGEND	)	MAP INFORMATION				
Area of In	<b>iterest (AOI)</b> Area of Interest (AOI)	8	Spoil Area Stony Spot	The soil surveys that comprise your AOI were mapped at 1:15,800.				
Solis ~ Special (2) (2) (2) (2) (3) (4) (4) (5) (5) (5) (5) (5) (5) (5) (5	Soil Map Unit Polygons Soil Map Unit Lines Soil Map Unit Points Point Features Blowout Borrow Pit Clay Spot	Ø ♥ ▲ Water Fea ✓ Transport +++	Very Stony Spot Wet Spot Other Special Line Features atures Streams and Canals tation Rails	<ul> <li>Warning: Soil Map may not be valid at this scale.</li> <li>Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.</li> <li>Please rely on the bar scale on each map sheet for map measurements.</li> </ul>				
> X ÷ © <	<ul> <li>Closed Depression</li> <li>Gravel Pit</li> <li>Gravelly Spot</li> <li>Landfill</li> <li>Lava Flow</li> <li>Marsh or swamp</li> <li>Mine or Quarry</li> <li>Miscellaneous Water</li> </ul>		Interstate Highways US Routes Major Roads Local Roads und Aerial Photography	Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857) Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as				
0 > + ∵ <b>=</b>	Perennial Water Rock Outcrop Saline Spot Sandy Spot Severely Eroded Spot Sinkhole Slide or Slip Sodic Spot			<ul> <li>Soil Survey Area: Jefferson County, Wisconsin Survey Area Data: Version 23, Sep 3, 2024</li> <li>Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.</li> <li>Date(s) aerial images were photographed: Aug 4, 2022—Sep 13, 2022</li> <li>The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident</li> </ul>				

### **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI						
GtB	Grellton fine sandy loam, 2 to 6 percent slopes	0.1	2.0%						
RtB	Rotamer loam, 2 to 6 percent slopes, eroded	0.8	12.0%						
SoB	Sisson fine sandy loam, 2 to 6 percent slopes	6.1	86.0%						
Totals for Area of Interest		7.1	100.0%						

### **Map Unit Descriptions**

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or

landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

#### Jefferson County, Wisconsin

#### GtB-Grellton fine sandy loam, 2 to 6 percent slopes

#### **Map Unit Setting**

National map unit symbol: g6zl Elevation: 780 to 1,060 feet Mean annual precipitation: 28 to 35 inches Mean annual air temperature: 36 to 57 degrees F Frost-free period: 135 to 170 days Farmland classification: All areas are prime farmland

#### **Map Unit Composition**

*Grellton and similar soils:* 100 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Grellton**

#### Setting

Landform: Till plains Landform position (three-dimensional): Rise Down-slope shape: Convex Across-slope shape: Linear Parent material: Loamy eolian deposits and/or loamy outwash over eolian deposits

#### **Typical profile**

Ap,BA - 0 to 14 inches: fine sandy loam Bt - 14 to 35 inches: loam 2Bt - 35 to 44 inches: silt loam 2C - 44 to 60 inches: sandy loam

#### **Properties and qualities**

Slope: 2 to 6 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 1.98 in/hr)
Depth to water table: About 36 to 60 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 20 percent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: High (about 9.8 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: B Ecological site: F095XB010WI - Loamy and Clayey Upland Forage suitability group: Mod AWC, adequately drained (G095BY005WI) Other vegetative classification: Mod AWC, adequately drained (G095BY005WI) Hydric soil rating: No

#### RtB-Rotamer loam, 2 to 6 percent slopes, eroded

#### Map Unit Setting

National map unit symbol: 2wpxt Elevation: 790 to 1,070 feet Mean annual precipitation: 33 to 35 inches Mean annual air temperature: 45 to 48 degrees F Frost-free period: 150 to 180 days Farmland classification: All areas are prime farmland

#### Map Unit Composition

Rotamer, eroded, and similar soils: 90 percent Minor components: 10 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Rotamer, Eroded**

#### Setting

Landform: Moraines, drumlins Landform position (two-dimensional): Summit Landform position (three-dimensional): Crest Down-slope shape: Convex Across-slope shape: Convex Parent material: Calcareous loamy till

#### **Typical profile**

Ap - 0 to 9 inches: loam Bt - 9 to 19 inches: clay loam C - 19 to 79 inches: gravelly sandy loam

#### Properties and qualities

Slope: 2 to 6 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.20 to 1.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 40 percent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: Moderate (about 8.2 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: B Ecological site: F095XB007WI - Loamy Upland with Carbonates Forage suitability group: Mod AWC, adequately drained (G095BY005WI) Other vegetative classification: Mod AWC, adequately drained (G095BY005WI) Hydric soil rating: No

#### **Minor Components**

#### Kidder

Percent of map unit: 5 percent Landform: Moraines, drumlins Landform position (two-dimensional): Summit Landform position (three-dimensional): Crest Down-slope shape: Convex Across-slope shape: Convex Ecological site: F095XB010WI - Loamy and Clayey Upland Other vegetative classification: Mod AWC, adequately drained (G095BY005WI) Hydric soil rating: No

#### Lapeer

Percent of map unit: 3 percent Landform: Moraines, drumlins Landform position (two-dimensional): Summit Landform position (three-dimensional): Crest Down-slope shape: Convex Across-slope shape: Convex Ecological site: F095XB007WI - Loamy Upland with Carbonates Hydric soil rating: No

#### Lamartine

Percent of map unit: 2 percent Landform: Moraines, drumlins Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Crest Down-slope shape: Concave Across-slope shape: Linear Ecological site: F095XB005WI - Moist Loamy or Clayey Lowland Hydric soil rating: No

#### SoB—Sisson fine sandy loam, 2 to 6 percent slopes

#### Map Unit Setting

National map unit symbol: 2wsr8 Elevation: 590 to 1,030 feet Mean annual precipitation: 29 to 35 inches Mean annual air temperature: 43 to 48 degrees F Frost-free period: 124 to 193 days Farmland classification: All areas are prime farmland

#### Map Unit Composition

Sisson and similar soils: 90 percent Minor components: 10 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Sisson**

#### Setting

Landform: Lake plains Landform position (three-dimensional): Rise Down-slope shape: Convex Across-slope shape: Linear Parent material: Loamy lacustrine deposits over stratified sandy and silty lacustrine deposits

#### **Typical profile**

Ap - 0 to 8 inches: fine sandy loam
E - 8 to 12 inches: fine sandy loam
Bt - 12 to 30 inches: loam
2C - 30 to 79 inches: stratified fine sand to silt loam

#### **Properties and qualities**

Slope: 2 to 6 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 35 percent
Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Available water supply, 0 to 60 inches: High (about 9.5 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2e Hydrologic Soil Group: B Ecological site: F095XB010WI - Loamy and Clayey Upland Forage suitability group: Mod AWC, adequately drained (G095BY005WI) Other vegetative classification: Mod AWC, adequately drained (G095BY005WI) Hydric soil rating: No

#### **Minor Components**

#### Kibbie

Percent of map unit: 4 percent Landform: Lake plains Landform position (three-dimensional): Dip Down-slope shape: Linear Across-slope shape: Concave Ecological site: F095XB005WI - Moist Loamy or Clayey Lowland Hydric soil rating: No

#### Yahara

Percent of map unit: 3 percent Landform: Lake plains Landform position (three-dimensional): Dip Down-slope shape: Linear Across-slope shape: Concave Ecological site: F095XB005WI - Moist Loamy or Clayey Lowland *Other vegetative classification:* Mod AWC, high water table (G095BY004WI) *Hydric soil rating:* No

#### Plainfield, eroded

Percent of map unit: 3 percent Landform: Lake plains Landform position (three-dimensional): Rise Down-slope shape: Convex Across-slope shape: Linear Ecological site: F095XB009WI - Sandy Upland Hydric soil rating: No

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## Appendix E: Soil Borings



# HARWOOD 255 N 21st Street, Milwaukee, WI 53233 | 414-475-5554

# **GWCHF** Development

Site Exhibit 2/01/2024



CGC Inc.					LOG OF TEST BORING Project GWCHF Residential Development Hoffmann Drive Location Watertown, Wisconsin	Boring No Surface El Job No. Sheet	). levation C 1 c	<b>SB</b> (ft) <b>M24(</b> of	- <b>1</b> 819± 62 1	 	
					356 S. Curtis Rd, West Allis, W1 53214 (414) 443-2000, FAX (414) 443-20						
			1	VISUAL CLASSIFICATION							
No.	Y Rec P E (in.)	Moist	Ν	(ft)	and Remarks	(qa) (tsf)	W	LL	PL	LOI	
				<u>├</u>	18" Dark Brown Clayey TOPSOIL						
1	18	VM	1		Very Loose, Dark Brown Sandy SILT; Little Clay, Trace Gravel (ML)						
2	18	VM	8		Loose, Brown and Gray Mottled Sandy SILT; Little Clay, Trace Gravel (ML)						
3	12	VM	6	+ <u>₹</u> 5– ⊢ └_							
				<u>+</u> ┌─	Daves Deserved Con Method Such SULT, Little						
4	16	VM	47		Clay, Trace Gravel (ML)						
					Very Dense, Light Brown SILT; Little Fine Sand, Few Cobbles (ML)						
5	18	W	91	+ +- 							
				⊢ 15– ∟ ∟							
					Very Dense, Brown and Gray Mottled Sandy SILT; Little Gravel (ML)						
6	12	W	50	┿ ┝ └ ↓ 20-							
					End of Boring at 20 ft Backfilled with Bentonite Chips						
						DENEKA		155	)		
While Time Depti Depti	while Drilling										

	G	CI	n		LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Location         Watertown, Wisconsin       36 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-2	Boring No.         SB-2           Surface Elevation (ft)         809.5±           Job No.         CM24062           Sheet         1         of         1           099					
SAMPLE					VISUAL CLASSIFICATION	SOIL	PRO	PEF	١T	S	
No.	No. TRec Moist N P(in.) (ft)			Depth (ft)	and Remarks	qu (qa)	W	LL	PL	LOI	
1	18	VM	6	+ ⊢ ∔	8" Black Clayey TOPSOIL Medium Stiff, Brown and Gray Mottled Lean	(1.0)					
	10	V IVI			CLAY; Trace Sand and Gravel (CL)	(1.0)					
2	18	VM	10	 ┬ ┝ ↓ 5-	Medium Dense, Brown and Gray Mottled SILT; with Clay, Little Sand, Trace Gravel (ML)						
3	4	VM	16								
4	16	M	23	┌─ ┿ ┝ ┾─ 10-							
					Very Dense, Gray Sandy SILT; Little Gravel, Few Cobbles and Boulders (ML)	_					
5	12	М	70	+ ↓ ↓ ↓ 15- ↓							
					End of Boring and Refusal on Probable Boulder at 16 ft Backfilled with Bentonite Chips						
	WATER LEVEL OBSERVATIONS GENERAL NOTES							μ			
While Time Depti Depti	While Drilling       ✓       14.0'       Upon Completion of Drilling        5 min.       Start       5/2/24       End       5/2/24         Time After Drilling        5 min.       Diller       GeoServe Chief       Eddie       Rig 7822         Depth to Water            Diller       GeoServe Chief       Eddie       Rig 7822         Depth to Cave in										
	G	CI	n		LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Location         Watertown, Wisconsin       336 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-2000	Boring No Surface El Job No. Sheet	evatior C	<b>SB</b> (ft) <b>M24(</b> of	3- <b>3</b> 817± 962 1	 	
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	SA	MPL	E		VISUAL CLASSIFICATION	SOIL	PRO	PEF	۲IE	S	
No.	T Rec	Moist	N	Depth	and Remarks	qu (qa)	W	LL	PL	LOI	
	E(111.)			+ (10) +	12" Black Clayey TOPSOIL	(tsf)					
1	18	W	2		Very Loose, Dark Brown Sandy SILT; Little Clay, Trace Gravel (ML)	(2.0)					
2	18	M	14		Stiff, Brown and Gray Mottled Lean CLAY; Trace Sand and Gravel (CL) Medium Dense, Brown and Gray Mottled Sandy SILT; Little Gravel (ML)						
3	18	VM	22		Medium Dense to Very Dense, Brown Sandy SILT; Trace Gravel (ML)						
4	18	W	45	┌ ┼ ┝ ↓ 10							
5	16	W	95								
	10	XX /	20		Dense, Gray Sandy SILT; Little Clay, Trace Gravel (ML)						
6	18	W	32	⊢_ ⊢							
				L 20. L	End of Boring at 20 ft Backfilled with Bentonite Chins						
					Note: Boring offset 15 ft south due to dense brush.						
			w			FNFRA					
While Time Dept Dept	e Drill After h to W h to Ca	ing Drillin ater ave in	∏ 1 ng	14.0'	Upon Completion of Drilling        5 min.       5.0'       Driller GeoS	/24 End Serve Chief die Editor 2.25" H	5/2/ Edc TA ISA	24 lie F C	• tig <b>78</b>	22	

	G	СІ	n		LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Location         Watertown, Wisconsin         336 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-20	Boring No Surface E Job No. Sheet	). levatior C	<b>SE</b> n (ft) 2 <b>M240</b> of	<b>3-4</b> 807.5 )62 1	±
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL	PRC	PEF	RTIE	S
No.	T Rec	Moist	N	Depth (ft)	and Remarks	qu (qa)	W	LL	PL	LOI
1				<u>+</u> ⊢	24" Black Clayey TOPSOIL	(tsi)				
1	12	VM	4	∔  _ 						
2	18	M	12		Medium Stiff, Brown and Gray Mottled Lean CLAY; Trace Sand and Gravel (CL) Medium Demse, Brown and Gray Mottled Sandy	(1.0)				
			12	┝─ ┝ ┝ ┝	SILT; Little Clay, Trace Gravel (ML)					
3	18	М	51		Little Gravel (ML)					
4	18	М	71	¦⊻ T <b>v</b>	Very Dense, Gray Sandy SILT; Little Gravel (ML)	-				
				⊢ ⊢ ∟ □ □ □						
5	18	M	82							
0	10	IVI	54							
					End of Boring at 20 ft Backfilled with Bentonite Chips					
	-		W	ATEF	R LEVEL OBSERVATIONS	GENERA	LNC	TES	3	
While Time Deptl Deptl	e Drill After h to W h to Ca strat	ing Drillin ater ave in	⊥ 8 ng	3.0'	Upon Completion of Drilling Start 5/2 5 min. 9.0' Driller Geo Logger Ed Drill Method	2/24 End Serve Chief Idie Edito d 2.25" I	5/2/ Edd r TA HSA	24 lie F C	∖ig <b>78</b>	22

	G	CI	n		LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Location         Watertown, Wisconsin       836 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-20	Boring No Surface E Job No. Sheet	o. levatior C	<b>SE</b> 1 (ft) 2 <b>M24(</b> 2 of	<b>8-5</b> 825.5 062 1	 ±
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL	PRO	PEF	۲IE	S
No.	T Rec	Moist	N	Depth (ft)	and Remarks	qu (qa)	W	LL	PL	LOI
				<u>+</u>	10" Dark Brown Clayey TOPSOIL	(USI)				
1	18	VM	2		Stiff to Soft, Dark Brown Sandy CLAY; Trace Gravel (CL)	(1.5)				
2	18	VM	3			(0.5)				
3	16	M/W	15		Medium Dense to Very Dense, Brown Sandy SILT; Little Gravel (ML)					
				Ţ.						
4	18	W	20	┿ ┝ ↓ 10-						
5	12	M	50	+ ⊢ ↓ 15- ∟						
					Very Dense, Gray Sandy SILT; Little Gravel (ML)	-				
	10	NV.	12	' ┝─- +				<u> </u>		
0	18	W	42							
				L 20-	End of Boring at 20 ft Backfilled with Bentonite Chins					
					Backfined with Dentomic Chips					
			W	ATEF	LEVEL OBSERVATIONS	SENERA	LNC	)TES	3	
While Time Dept Dept	e Drill After h to W h to C	Drillin Ater ave in	∏_ ng	7.0'	Upon Completion of Drilling        5 min.       5/2           5 min.       5 min.       5 min.            2 min.       5 min.       5 min.             5 min.	2/24 End Serve Chief Idie Edito d 2.25" 1	5/2/ Edd r TA HSA	24 lie F .C	∖ig <b>78</b>	22

	G	CI	n	<b>c.</b> )	LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Location         Watertown, Wisconsin       836 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-200	Boring No Surface E Job No. Sheet	). levation C 1 (	<b>SB</b> 1 (ft) <b>M24(</b> of	8-6 828⊧ 062 1	 E
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL	PRO	PEF	₹TIE	S
No.	T Y Rec P (in.)	Moist	N	Depth (ft)	and Remarks	qu (qa)	W	LL	PL	LOI
	E ( ,			+	3.5" ASPHALT over 8" Gray Crushed Stone BASE	(tsf)			<u> </u>	
1	16	М	7		Very Stiff to Stiff, Dark Brown Sandy CLAY; Trace Gravel (CL)	(3.0)				
2	4	VM	9	  _  _  -  -	Loose, Brown Sandy SILT; Little Clay, Trace Gravel (ML)	(1.5)				
3	18	VM	11		Medium Dense, Brown Sandy SILT; Little Gravel (ML)	-				
4	18	VM	12	┌─ ┿ ┝─ ↓─ 10-						
5	10	W			Very Dense, Brown and Gray Mottled SILT; Little Fine Sand (ML)	-				
	18		00	⊢ ⊢ ∟ □						
					Very Dense, Gray SILT; Little Fine Sand (ML)	-				
6	3	M	50	+ + -						
					End of Boring at 20 ft Backfilled with Bentonite Chips					
						SENERA				
Whil	e Drill	ing	v v V	лісг 13.0'	Upon Completion of Drilling Start 5/	2/24 End	5/2/	24	-	
Time Dept Dept	h to W h to C	Drillin Vater ave in	ng	lines rottransit	present the approximate boundary between □	Serve Chief ddie Edito d 2.25" 1	Edc r TA HSA	lie F C	€ig <b>78</b>	22

	G	СІ	n		LOG OF TEST BORING Project GWCHF Residential Development Hoffmann Drive Location Watertown, Wisconsin 36 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-20	Boring No Surface El Job No. Sheet	evation Cl	<b>SB</b> (ft) <b>M240</b> f	-7 833± 62 1	 
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL	PRO	PER	TIE	S
No.	T Rec P (in.)	Moist	N	Depth (ft)	and Remarks	qu (qa) (tsf)	W	LL	PL	LOI
				<u>+</u>	FILL: 4" Black Clayey Topsoil					
1	18	М	41	∔ └ └	FILL: Gray Crushed/Reprocessed Concrete					
2	18	М	61	┌ ┌ └ └ ↓ 5-						
3	18	М	25							
		_			FILL: Brown, Gray and Dark Brown Lean Clay,					
4	18	VM	6	↓ ┣ ╋ ╋ ┨0-	Trace Sand and Gravel	(1.0)				
5	18	W	6	⊢ 」∟ └ └ └ └ └ └	Loose, Dark Brown and Dark Clayey SAND (SC)					
6	12	W	21		Medium Dense, Brown Sandy SILT; Little Clay, Trace Gravel (ML)					
					End of Boring at 20 ft Backfilled with Bentonite Chips					
			VV.		LEVEL UBSERVATIONS	DENEKA		IES	)	
While Time Dept Dept	e Drill After h to W h to Ca	ing Drillin ater ave in	$\underline{\nabla}$ 1 ng	14.0'	Upon Completion of Drilling Start 5/3 12.0' 12.0' Logger Ed Drill Method	3/24 End Serve Chief Idie Editor 1 2.25" I	5/3/2 Edd t TA ISA	24 ie R	ig <b>78</b>	22

0	G	СІ	n		LOG OF TEST BO         Project       GWCHF Residential Dev Hoffmann Drive         Location       Watertown, Wisconsin         5 S. Curtis Rd, West Allis, WI 53214 (414) 443	<b>RING</b> /elopment -2000, FAX (414) 443-2	Boring No Surface El Job No. Sheet	evatior C	<b>SB</b> (ft) <b>M24(</b> of	8- <b>8</b> 816± 962 1	 
	SA	MPL	E		VISUAL CLASSIFIC	ATION	SOIL	PRO	PEF	RTIE	S
No.	T Y Rec E(in.)	Moist	N	Depth (ft)	and Remarks		qu (qa) (tsf)	W	LL	PL	LOI
				<u></u> ⊢	FILL: 4" Dark Brown Clayey Top	soil					
1	18	М	7	+  -  _ 	FILL: Brown/Dark Brown Mixed Clay, Trace Gravel	Sandy Silt, Little					
2	16	М	8			$\boldsymbol{\mathcal{A}}$					
				╄── >─ ┝ └──		ace Clay, Little					
3	18	M	11		Gravel						
4	18	M	10								
				⊢ ┾─ 10─							
5	6	M	23								
5	0	111	23								
6	18	M	17	┝ ┝-							
				┝── 15─ └ I	Very Dense, Light Brown Sandy S	SILT; Trace					
7	18	W	61								
8	18	M	54	┝━- ╄-	Very Dense, Gray SILT; Little Fir	e Sand (ML)	-				
				∟ ∟ ⊥ 20−							
					End of Boring at 2 Backfilled with Benton	0 ft .te Chips					
			W	ATEF	LEVEL OBSERVATIONS		GENERA	L NC	TES	5	
While Time Depth Depth	e Drill After h to W h to Ca	ing Drillin ater ave in	∏ 1 ng	7.0'	Upon Completion of Drilling 5 m MV	- Start 5 Driller Gee Logger M Drill Metho	/2/24 End oServe Chief Matt Editor od 2.25" H	5/2/ Edd TA ISA	24 lie F C	Rig <b>78</b>	22

	G	СІ	n	<b>C.</b> )	LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Location         Watertown, Wisconsin         336 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-200	Boring No.         SB-9           Surface Elevation (ft)         821±           Job No.         CM24062           Sheet         1         of         1           3-2099				
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL	PRC	PEF	₹TIE	S
No.	T Rec P (in.)	Moist	N	Depth (ft)	and Remarks	qu (qa) (tsf)	W	LL	PL	LOI
				<u>├</u>	18" Black Clayey TOPSOIL	(031)				
1	18	M	4		Stiff, Brown and Gray Mottled Lean CLAY; Trace Sand and Gravel (CL)	(1.5)				
2	18	M/W	5		Loose, Brown and Gray Mottled Sandy SILT; Little Gravel (ML)					
3	16	W	17		Medium Dense, Brown Sandy SILT; Little Clay, Trace Gravel (ML)					
4	18	W	13	T F + ⊢ ↓ ↓ 10-						
5			1.25		Medium Dense, Brown SILT; with Clay, Little Sand, Trace Gravel (ML)					
3	4	V IVI/ W	25	┝ ┝ ↓ 15- └_ ╹_						
					Medium Dense, Gray Sandy SILT; Little Clay, Trace Gravel (ML)					
6	18	VM	14							
					End of Boring at 20 ft Backfilled with Bentonite Chips					
<b>XX71 '1</b>	- D '''		W		K LEVEL OBSERVATIONS     C				<b>&gt;</b>	
While Time Deptl Deptl	h to W h to Ca strat	Drillin Vater ave in	⊥ 2 lg	<b>+.U</b> <sup>*</sup>	Start     5/3        5 min.        12.0'	S/24 End Serve Chief Idie Edito d 2.25" I	5/3/ Edd r TA HSA	24 lie F C	tig <b>78</b>	22

	G	СІ	n	<b>c.</b> )	LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Location         Watertown, Wisconsin       336 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-200	Boring No Surface E Job No. Sheet	o. levatior C	<b>SB</b> 1 (ft) 2 <b>M24(</b> 2 of	-10 816∃ )62 1	 
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL	PRC	PEF	RTIE	S
No.	T Rec	Moist	N	Depth (ft)	and Remarks	qu (qa)	W	LL	PL	LOI
1	18	M	12	+ +- ↓- ↓-	FILL: Black Clayey Topsoil					
2	18	VM	6		Loose, Brown and Gray Mottled Sandy SILT; Little Clay, Trace Gravel (ML)	-				
3	18	VM	13	⊢ ⊢ ∟ ↓_	Medium Dense, Brown Sandy SILT; Little Clay, Trace Gravel (ML)	-				
4	18	VM	16							
5	18	M/W	48	¥ ∟ ⊢ ⊢ ⊢	Dense to Very Dense, Gray Sandy SILT; Little Gravel (ML)	-				
	10	W	71							
0	18	w	/1							
					End of Boring at 20 ft Backfilled with Bentonite Chips			TE		
While	e Drill	ino	¥¥. ∑ 1	AIE 14.0'	Upon Completion of Drilling Start 5/	2/24 End	5/2/	24	)	
Time Dept Dept	After h to W h to C $\frac{1}{2}$	Drillin Vater ave in	ng	lines r transit	present the approximate boundary between of may be gradual.	Serve Chief ddie Edito d 2.25" 1	r TA HSA	lie F C	Rig <b>78</b>	22

	G	СІ	nc		LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Location         Watertown, Wisconsin       336 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-20	Boring No Surface El Job No. Sheet	evation (f CM	<b>B-11</b> t) 807.5 24062 1	±
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL	PROP	ERTIE	S
No.	T Rec	Moist	N	Depth (ft)	and Remarks	qu (qa)	W I	LL PL	LOI
				<b>├</b>	FILL: 5" Dark Brown Clayey Topsoil	. (LSI)			
1	18	VM	3		FILL: Brown Sandy Clay, Trace Gravel	(1.25-1.5)			
2	18	VM	5	  -  -  -  -  -  -  -	FILL: Gray and Dark Gray Mixed Sandy Silt, Little Clay, Trace Gravel	(1.5)			
3	18	M	5	⊢ └ └_ └_	Stiff, Black Silty CLAY; with Organics (BURIED TOPSOIL) (OL)	(1.5)			
4	4	M	14	⊢ ⊢ ⊢ ⊢	Stiff, Dark Gray Lean CLAY; Trace Sand and Gravel (CL) Medium Dense, Tan and Gray Mottled Sandy SILT; Little Gravel (ML)	(2.0)			
5	18	М	19	└ └ └ └ └ └ └ └ └ └					
					Dense, Gray Sandy SILT; Little Gravel (ML)				
6	18	M	36						
					End of Boring at 20 ft Backfilled with Bentonite Chips				
	_		W	ATER	LEVEL OBSERVATIONS	SENERA	L NOT	EŚ	
While Time Dept Dept	e Drill After h to W h to C	ing Drillin Vater ave in	<u> </u>	NW	Upon Completion of Drilling Start 5/1 5 min. NW Logger M Drill Method	2/24 End Serve Chief latt Editor d 2.25" H	5/1/24 Matt TAC ISA	Rig <b>78</b>	22

	G	СІ	n		LOG OF TEST BORING Project GWCHF Residential Development Hoffmann Drive Location Watertown, Wisconsin 336 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-20	Boring No.         SB-12           Surface Elevation (ft)         824±           Job No.         CM24062           Sheet         1         of         1           2099					
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL	PRO	PEF	₹TIE	S	
No.	T Rec	Moist	N	Depth (ft)	and Remarks	qu (qa)	W	LL	PL	LOI	
	<u>E</u> ()			⊢ ⊢	FILL: Gray Crushed/Reprocessed Concrete	(tsi)					
1	18	М	33	₩- ₩ ₩-							
2	18	VM	10								
				<u>+</u> 5− ⊢	Trace Gravel (CL)	(1.5)					
3	18	VM	10		Very Stiff, Light Brown and Gray Mottled Lean CLAY; Trace Sand and Gravel (CL)	(2.5)					
	10	** *	_		Loose, Brown and Gray Mottled Sandy SILT; Little						
4	12	W		⊢ ⊢	Clay, Trace Gravel (ML)						
					Dense to Medium Dense, Brown Sandy SILT; Little Clay, Trace Gravel (ML)	-					
5	18	W	39								
6	6	W	13	+    -							
				L 20-	End of Boring at 20 ft						
					Backfilled with Bentonite Chips						
<b>X</b> 71- 11	a D.::11				LEVEL UDJERVAIIUNJ						
While Time Deptl Deptl	After h to W h to Ca	Drillin Vater ave in	± 4 ng	Lines recransit:		Siza End Serve Chief Idie Edito d 2.25" 1	5/3/ Edd r TA HSA	24 lie F C	tig <b>78</b>	22	

	G	СІ	nc		LOG OF TEST BORING Project GWCHF Residential Development Hoffmann Drive Location Watertown, Wisconsin 36 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-20	Boring No Surface El Job No. Sheet	o. evatior C	<b>SW</b> (ft) <b>M24(</b> of	/-1 801± )62 1	 E 
	SA	MPL	E			SOIL	PRO	PEF	۲IE	S
No.	T Rec	Moist	N	Depth	and Remarks	qu (ga)	W	T.T.	PT.	LOT
	E(in.)			(ft)	8" Black Clavey TOPSOIL	(tsf)				-
1	6	W	6		Loose, Tan and Gray Mottled SILT; with Clay, Little Sand, Trace Gravel (ML)	-				
2	18	W	20		Medium Dense, Tan and Gray Mottled Fine SAND; Little Silt (SP-SM)	-				
3	18	W/M	18	⊢ ↓_ ↓_ ↓_	Medium Dense, Gray SILT; Little Fine Sand (ML)	-				
4	18	M	23	├── ┼ ┝─ ↓── 10──						
5	18	М	12	⊢ ↓ ↓_ ↓ ↓ ↓						
6	18	M	32	┣━ ╋ ┣━ ┫ ┨ ┚	Dense, Gray Sandy SILT; Little Gravel (ML)					
7	18	M	38							
8	18	М	31	⊢ + ⊢ ↓ 20−						
					End of Boring at 20 ft Backfilled with Bentonite Chips					
				AIER	LEVEL OBSERVATIONS (	JENERA		TES	>	
While Time Dept Dept	e Drill After h to W h to Ca strat	ing Drillin ater ave in	$\frac{\nabla}{\log}$	L.O'	Upon Completion of Drilling Start 5// 5.0' 5.0' Driller Geo Logger M Drill Method	1/24 End Serve Chief Iatt Edito d 2.25" I	5/1/ Ma r TA HSA	24 htt F C	Lig <b>78</b>	22

	G	СІ	Inc		LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Hoffmann Drive         Location       Watertown, Wisconsin         336 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-20	Boring No Surface El Job No. Sheet	o. evatior C	<b>SN</b> (ft) <b>M24(</b> of	<b>/-2</b> 804± )62 1	 
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL	PRO	PEF	۲IE	S
No.	T Rec	Moist	N	Depth (ft)	and Remarks	qu (qa)	W	LL	PL	LOI
	E			<b>├</b>	24" Black Clayey TOPSOIL	(tsi)				
1	18	VM	5	₩ 						
					Stiff, Tan and Gray Mottled Silty CLAY; Trace	(1.0-2.0)		<u> </u>		
2	18	M	18		Medium Dense, Tan and Gray Mottled Sandy SILT; Little Gravel (ML)			<u> </u>		
				⊢ ┾─ 5- ∟				<u> </u>		
3	16	W	31		Dense, Grayish Brown Sandy SILT; Little Gravel (ML)					
		-						<u> </u>	ĺ	
4	18	M	25	-  - 	Medium Dense to Very Dense, Gray Sandy SILT; Little Gravel, Occasional Cobbles and Boulders					
			-	⊢ ∔- 10-	(ML)			<u> </u>		
5	12	M	80/9"							
				   				<u> </u>		<u> </u>
6	12	M	69	┌─ ┼ ┝─				<u> </u>		
				⊢ ├- 15-				<u> </u>		
7	18	M	60							
		-		 				<u> </u>		
8	18	M	66	⊢- + ⊢-						
				⊢ └── 20-	End of Boring at 20 ft			<u> </u>		
					Backfilled with Bentonite Chips					
				⊢- ⊢						
				L 25-						
			W	ATEF	LEVEL OBSERVATIONS	SENERA	LNC	TES	5	
Whil	e Drill After	ing Drilli	<u>⊻ 7</u> ng	7.0'	Upon Completion of Drilling Start	1/24 End Serve Chief	5/1/ Ma	24 ft F	{ig 78	22
Dept	h to W	ater ave in				latt Editor	r TA ISA	C		
The	strat	cificates and	tion l the t	lines re	present the approximate boundary between	u <u>4,43</u> I	197			

	G	СІ	nc		LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Location         Watertown, Wisconsin       336 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-200	Boring No Surface E Job No. Sheet	). levation C 1 c	<b>SN</b> (ft) <b>M24(</b> of	<b>/-3</b> 798± )62 1	·····		
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL PROPERTIES						
No.	T Rec	Moist	N	Depth	and Remarks	qu (qa)	W	LL	PL	LOI		
1	E (111.)			(10) 	FILL: 5" Black Clayey Topsoil	(tsf)						
1	18	M	11	-  -    _	FILL: Brown and Dark Brown Mixed Sandy Silt, Little Gravel, Trace Clay							
2	18	M	12	   ↓ 5−	Black Silty CLAY; with Organics (BURIED TOPSOIL)							
3	18	М	11		Medium Dense, Tan and Gray Sandy SILT; Little Gravel (ML)							
4	4	VM/W	20	└ └ └ └ ↓ ↓ 10-	Medium Dense, Grayish Brown Sandy SILT; Little Clay, Trace Gravel (ML)							
5	18	M/W	36		Dense to Very Dense, Gray SILT; Trace to Some Sand, Few Sand Seams (ML)							
6	16	M	32	├── ┼ ┝─ ↓ 15-								
7	18	W	37									
8	18	W	83	└── ┼ └── └── 20─								
					End of Boring at 20 ft Backfilled with Bentonite Chips							
			W	ATEF	( LEVEL OBSERVATIONS	ENERA		IES	5			
While Time Deptl Deptl	e Drill After h to W h to Ca	ing Drillir ater ave in	⊻ 1 ng	2.0'	Upon Completion of Drilling Start 5/ 5 min. 10.0' ¥ Start 5/ Driller Geo Logger M Drill Metho	1/24 End Serve Chief Iatt Edito d 2.25" I	5/1/ Ma r TA HSA	24 tt R C	Kig <b>78</b>	22		

	G	CI	nc		LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Location         Watertown, Wisconsin       336 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-200	Boring No.         SW-4           Surface Elevation (ft)         801.5±           Job No.         CM24062           Sheet         1         of         1           099						
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL PROPERTIES						
No.	T Rec	Moist	N	Depth (ft)	and Remarks	qu (qa)	W	LL	PL	LOI		
1	18	М	20		FILL: 4" Black Clayey Topsoil FILL: Brown Silty Sand, Trace Clay and Gravel	(USI)						
2	18	М	10	  -  -  -  -  -  -  -  -	FILL: Black and Dark Gray Mixed Sandy Silt, Little Clay, Trace Gravel with Intermixed Topsoil Black Silty CLAY: with Organics (BURIED	-						
3	18	M	12	⊢ ↓ ↓ ↓_ ↓	TOPSOIL)							
4	18	М	8	┌── ┼─ ┝─ ↓── 10-	Loose, Brown and Gray Mottled SILT; Little Clay, Trace to Little Sand, Trace Gravel (ML)							
5	18	M/W	28		Medium Dense, Brown Sandy SILT; Little Gravel (ML)							
6	18	M	38	┝── ┾ ┝─ ↓♥ 15-	Dense, Gray SILT; Trace Fine Sand (ML)							
7	18	М	70	L         +	Very Dense, Gray Sandy SILT; Little Gravel (ML)							
8	18	М	54	┝─ ┿ ┝─ ↓ 20-								
					End of Boring at 20 ft Backfilled with Bentonite Chips							
			W	ATE	LEVEL OBSERVATIONS	GENERA	LNC	DTES	5			
Whil Time Dept Dept	e Drill After h to W h to Ca	ing Drillin ater ave in	$\frac{4}{10}$	12.0'	Upon Completion of Drilling Start _5/ 	1/24 End Serve Chief Iatt Edito d 2.25" 1	5/1/ Ma r TA HSA	/24 att F C	tig <b>78</b>	22		

	G	СІ	n		LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Location         Watertown, Wisconsin       36 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 4	Boring No Surface El Job No. Sheet	). levation ( CM 1_of	SW-5 ft) 823 124062 1	±			
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL	SOIL PROPERTIES					
No.	T Rec	Moist	N	Depth	and Remarks	qu (qa)	W	LL PL	LOI			
	E (111.)			(10) 	FILL: Gray Crushed/Reprocessed Concrete	(tsf)			-			
1	18	М	12	∔ └_					-			
							+					
2	12	W	7	¦≚ ⊤								
2	12		,		Dark Gray and Black Mottled Silty CLAY; with Organics (BUBIED TOPSOUL)	(1.75)						
				+ ⊢ ↓	Very Stiff, Brown and Gray Mottled Silty CLAY;							
3	16	W	10	Ĺ	Trace Sand (CL/CL-ML)	(2.5-3.0)						
4	18	W	11	÷ ⊢	(ML)							
				⊢ ∔10-								
5	18	W	8									
	10	** 7	1.4	¦ +								
6	18	W	14	⊢ ⊢								
				∔ 15- ∟ ।	Soft Sandy CLAY: Trace Gravel (CL)							
7	18	W	6		Sold, Sundy CLATT, Trace Graver (CL)	(0.5)						
8	4	W	9	+ +-								
				∟ ⊥ 20-	End of Boring at 20 ft							
					Backfilled with Bentonite Chips							
				- ⊢ ⊢								
			W		LEVEL OBSERVATIONS	GENERA		res				
While Time Dept	e Drill After h to W	ing Drilliı 'ater <sub>.</sub>	<u>₹</u> 3 ng	<u>3.0'</u>	Upon Completion of Drilling         Start	5/3/24 End GeoServe Chief Matt Edito	5/3/24 Matt r TAC	<b>1</b> Rig 7	822			
Dept The soi	h to Ca strat	ave in ificat	tion 1 the t	lines retransit	resent the approximate boundary between Drill M	ethod 2.25" I	<u>18A</u>					

	G	СІ	n	5.)	33	Pr  Lo 36 S.	LOG OF TEST BORING         oject       GWCHF Residential Development         Hoffmann Drive       Hoffmann Drive         ocation       Watertown, Wisconsin         Curtis Rd, West Allis, WI 53214       (414) 443-2000, FAX (414) 443-200	Boring No Surface El Job No. Sheet	evation C	<b>SW</b> n (ft) 2 <b>M24(</b> of	<b>/-6</b> 822.5 )62 1	 +		
	SA	MPL	E				VISUAL CLASSIFICATION	SOIL PROPERTIES						
No.	T Rec P (in.)	Moist	N	Dept (ft	:h )		and Remarks	qu (qa)	W	LL	PL	LOI		
1	18	М	10	┾─── ┝ ┝ ╷			FILL: Gray Crushed/Reprocessed Concrete	(001)						
2	18	W	4				Black Silty CLAY; with Organics (BURIED TOPSOIL)							
				-  -  -  -  -	5—		Loose, Brown and Gray Mottled Sandy SILT; Some Clay, Few Sand Seams (ML)							
3	18	M	10				Very Stiff, Brown and Gray Mottled Lean CLAY; Trace Sand and Gravel (CL)	(2.5-2.75)						
4	18	W	6		0		Medium Stiff, Gray Lean CLAY; Trace Sand and Gravel, Few Silt Seams (CL)	(1.0)						
5	18	W	9				Loose, Gray Sandy SILT; Little to Some Clay, Trace Gravel (ML)							
6	4	W	8		5—									
7	18	W	20				Medium Dense, Gray Sandy SILT; Little Gravel (ML)	-						
8	18	W	15		0									
					5		End of Boring at 20 ft Backfilled with Bentonite Chips							
<b>XX71 ·1</b>	D '''	•	W		ĸ						Ď			
While Time Deptl Deptl	e Drill After h to W h to Ca	Ing Drillin ater ave in	⊥ 4 ng	H.U'	rep	ores	Start 5/. Start 5/. Start 5/. Driller Geo Logger M Drill Methor ay be gradual.	Serve Chief Serve Chief Iatt Editor d 2.25" F	5/3/ Ma TA ISA	24 htt F C	Rig <b>78</b>	22		

	G	СІ	nc		LOG OF TEST BORING         Project       GWCHF Residential Development         Hoffmann Drive       Location         Watertown, Wisconsin       336 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-20	Boring No.SW-7Surface Elevation (ft) $794 \pm$ Job No.CM24062Sheet1of099							
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL PROPERTIES							
No.	T Rec	Moist	N	Depth (ft)	and Remarks	qu (qa)	W	LL	PL	LOI			
				<u>+</u>	Black Clayey TOPSOIL	(LSI)							
1	18	М	4	∔ └_ └ <b>_</b>		(1.0-2.0)							
2	6	М	41		Dense, Brown and Gray Mottled Silty SAND: Little	-							
				┍ ┾─ 5− ┝_	Gravel (SM)								
3	2	W	24	<u>I</u> ⊈ L I	Medium Dense to Dense, Gray Gravelly, Sandy           SILT; Little Clay (ML)								
4	2	W	32	। †- ┝					-				
		-		⊢ ┾- 10- ∟									
5	18	VM	46		Very Dense to Medium Dense, Gray SILT; Little Sand, Trace to Little Gravel, Trace Clay (ML)								
6	18	M	56	⊢ + ⊢									
				⊢ ├- 									
7	8	W	45										
8	18	W	24	┝ ┾ ╵		(4.5)							
				└ └ └ 20─		()							
					End of Boring at 20 ft Backfilled with Bentonite Chips								
				⊢ ⊢ ⊢									
			W	ATER	LEVEL OBSERVATIONS	SENERA	LNC	TES	5	L			
Whil	e Drill	ing	<u>¥</u> (	5.0'	Upon Completion of Drilling Start	1/24 End	5/1/	24	);~ <b>7</b> 9				
Dept	h to W	ater .	ıg		$\underline{\qquad} \qquad \underline{\qquad} \qquad} \underline{\qquad} \qquad \underline{\qquad} \qquad \underline{\qquad} \qquad \underline{\qquad} \qquad \underline{\qquad} \qquad} \underline{\qquad} \qquad \underline{\qquad} \qquad \underline{\qquad} \qquad \underline{\qquad} \qquad} \underline{\qquad} \qquad \underline{\qquad} \qquad} \underline{\qquad} \qquad \underline{\qquad} \qquad} \underline{\qquad} \qquad \underline{\qquad} \qquad} \underline{\qquad}$	Serve Chief Latt Editor		C	ug /8	<i>44</i>			
The soi	n to Ca strat	ave in cificat es and	tion 1 the t	lines re transiti	present the approximate boundary between	u 2.25 F	15A						

	G	СІ	n		LOG OF TEST BORING Project GWCHF Residential Development Hoffmann Drive Location Watertown, Wisconsin 336 S. Curtis Rd, West Allis, WI 53214 (414) 443-2000, FAX (414) 443-2	Boring No Surface El Job No. Sheet	). levatior C	<b>SV</b> n (ft) 2 <b>M240</b> of	<b>/-8</b> 797.5 )62 1	±		
	SA	MPL	E		VISUAL CLASSIFICATION	SOIL PROPERTIES						
No.	T Y Rec P (in.)	Moist	Ν	Depth (ft)	and Remarks	qu (qa) (tsf)	W	LL	PL	LOI		
				+ +	18" Black Clayey TOPSOIL							
1	18	VM	4		Stiff, Light Gray and Brown Mottled Lean CLAY; Trace Sand and Gravel (CL)	(1.5)						
2	18	VM	6	+    -  - 5-	Loose, Brown and Gray Mottled Sandy SILT; Little Gravel (ML)							
3	2	M/W	20		Stiff, Brown and Gray Mottled Sandy CLAY; Trace Gravel (CL)							
4	12	М	22	└── ┼ ┝─ ↓ 10−	Medium Dense to Very Dense, Gray SILT; Little Sand and Gravel (ML)	_						
5	18	М	75									
6	12	М	36	└── ┼ ┝─ ↓ <b>▼</b> 15−								
7	18	М	18		Medium Dense, Gray Sandy CLAY; Trace Gravel (CL)							
8	4	М	24									
					End of Boring at 20 ft Backfilled with Bentonite Chips							
W/L.1	- D11	ina	W		LEVEL OBSERVATIONS				5			
Time Depth Depth	After h to W h to Ca strat	Drillin ater ave in	<u>+</u> ( ng	D.U  lines re transiti	present the approximate boundary between on may be gradual.  Start 5. 5 min. 15.0' ▼ Start 5. Driller Gen Logger M Drill Metho	Disperve Chief Matt Edito Dd 2.25" I	5/1/ Ma r TA HSA	24 htt I C	≀ig <b>78</b>	22		

Wisconsin De	partment of	Commerce	SOIL AND SITI	E EVALU	JATION - ST	ORM	Page	1		of3	
Division of Sa	itety and Bu	lldings	In accordance with SPS 3	82.365, 385, V	Vis. Adm. Code, and	d WDNR Standard 1002					
Attach comp include, but	olete site p not limited	lan on paper not less than to: vertical and horizontal	8 1/2 x 11 inches in size. I reference point (BM), dire	Plan must ection and		Parcel I.D.	Jefferson	4 001			
percent slop	e, scale or	r dimensions, north arrow, a	and BM referenced to nea	rest road.		Review by	291-0013-001	4-001		Date	
	Developed	Please print all inform	ation.		<b>4</b> ) ())	nonon by				Duio	
Property Ov	vner	ormation you provide may be used	tor secondary purposes (Privac	y Law, s.15.04 (	Property Locat	ion	60	0 and 700	) Hoffman	Drive	
Hoffman Ma	tz I I C										
Property Ov	wner's Ma	iling Address			Govt. Lot Lot #	Block #	SE 1/4	of NE 1/4 Sub	<u>S08 T8N</u> d. Name o	R15E or CSM#	
600 E. Main	Street, Su	ite 200			1				4146-02-1	181	
City		State	Zip Code Phone	Number	X City	Village	Town		Ne	arest Road	
Watertown		WI	53094		,	Watertown			Joh	inson Street	
Drainage area:      sq. ft.      acres         Test Site Suitable for (check all that apply)      Site not su        Bioretention      Subsurface Dispersal System        Reuse      Irrigation      Other				uitable	Hydraulic Appl X Morpholog Double-Rir	ication Test Me ical Evaluation ng Infiltrometer cify)	ethod	Soil Mois Date of s USDA-NF	sture oil boring RCS WET Dry = 1 Normal = Wet = 3	ıs: May 1-3, 2024 S Value: 2	
SW-1 Obs. # X Boring Pit Ground Surface Elev. 801± ft Elevation of limiting factor 801± ft											
Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frag.	% Fines	Hydraulic App. Rate Inches/Hr	
1	0-8	Topsoil (No	t Sampled)								
2	8-36	10YR6/4	c2d 10YR7/2	L	0.m	mfr	a	<5	70	0.24	
3	36-66	10YR5/6	c2f 10YR7/2	LFS	0.sq	mfr	a	<5	15	0.50	
4	66-156	10YR5/1		SICL	0,m	mvfi	q	<5	80	0.04	
5	156-240	10YR6/1		SL	0,m	mvfi	q	<15	50	0.50	
					,		Ŭ				
Comments:					1						
SW-2 O	bs. #	X Boring Pit Ground S	urface Elev. <u>804</u> ±	ft	Elevation of	limiting factor	804±_ft				
Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frag.	% Fines	Hydraulic App. Rate Inches/Hr	
1	0-24	Topsoil (No	t Sampled)								
2	24-36	10YR6/2	c2d 10YR7/4	SIC	0,m	mfi	g	<5	>90	0.07	
3	36-66	10YR6/4	c2d 10YR7/1	SL	0,m	mfr	g	<15	40	0.50	
4	66-96	10YR6/2		SL	0,m	mfi	g	<15	50	0.50	
5	96-240	10YR6/1		SL	0,m	mvfi	g	<15	50	0.50	
Comments:											
CST/PSS Na	ame (Plea	se Print)		Signature	Dud				CS	T Number	
Paul J. Giese, CST					faul full				SP-030800004		
Address					Date Eva	Date Evaluation Conducted Telephone Number					
336 S. Curtis Road, West Allis, WI 53214					5/9/24 (414) 443-2000 				4) 443-2000 SBD-10793 (R.01/17)		

Property C	Owner	Hoffman Matz LLC		Parcel ID#	291-0815-0814-	001 Page 2 of			of	3	
SW-3	Obs. #	X Boring									
		Pit Ground Su	face Elev. 798±	_ft	Elevation of	limiting factor	<u>798±</u> ft				
Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frag.	% Fines	Hydraulic / Inche	App. Rate s/Hr
1	0-5	Topsoil Fill (N	Not Sampled)								
2	5-36	10YR4/2 & 10YR6/4		L*	0,m	mfi	а	<15	60	0.2	24
3	36-66	10YR2/1		SICL**	0,m	mfi	а	<5	>90	0.0	)4
4	66-96	10YR5/6	c2d 10YR7/2	SL	0,m	mfr	g	<5	50	0.5	i0
5	96-126	10YR6/2		SCL	0,m	mfr	g	<5	55	0.1	1
6	126-240	10YR6/2		SIL	0,m	mvfi	g	<15	70-80	0.1	3
Comments	:	* FILL ** Burie	ed Topsoil								
SW-4	Obs. #	X Boring Pit Ground Sur	face Elev. 801.5±	_ft	Elevation of	limiting factor	<u>801.5±</u> ft				
Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frag.	% Fines	Hydraulic / Inche	App. Rate ⊧s/Hr
1	0-4	Topsoil Fill (N	Not Sampled)								
2	4-36	10YR5/4		SL*	0,sg	mfr	а	<15	15	0.5	i0
3	36-54	10YR4/2 & 10YR2/1		SL*	0,m	mfr	а	<5	50	0.5	0
4	54-96	10YR2/1		SICL**	0,m	mfi	а	<5	>90	0.0	)4
5	96-126	10YR6/4	c2d 2.5Y7/1	CL	0,m	mfi	g	<5	70	0.0	13
6	126-156	10YR6/6	c2f 10YR7/2	SL	0,m	mfi	g	<5	40	0.5	i0
7	156-186	10YR6/2		SI	0,m	mfr	g	<5	90	0.1	3
8	186-240	10YR6/2		SL	0,m	mfi	g	<15	50	0.5	0
Comments	:	* FILL ** Burie	ed Topsoil								
SW-5	Obs. #	X Boring Pit Ground Sur	face Elev. 823±	_ft	Elevation of	limiting factor	<u>823±</u> ft				
Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frag.	% Fines	Hydraulic / Inche	App. Rate s/Hr
1	0-48	10YR6/2		GRS*	0,sg	mfr	а	15-35	5-10	3.6	i0
2	48-66	10YR3/1	m2p 10YR2/1	SIC**	0,m	mfr	а	<5	>90	0.0	7
3	66-96	10YR5/6	m2p 10YR7/2	SIC	0,m	mfi	g	<5	>90	0.0	7
4	96-186	10YR6/4		SICL	0,m	mfi	g	<5	50	0.0	14
5	186-240	10YR6/2		SC	0,m	mfr	g	<5	55	0.0	14
Comments	:	* FILL (Crushed/Reproces	sed Concrete) **	Buried Topso	pil						

Property O	wner	Hoffman Matz LLC	I	Parcel ID#	ID# 291-0815-0814-001 Page 3 of					of <u>3</u>
SW-6 C	)bs. #	X Boring Pit Ground Sur	face Elev. 822.5±	_ft	Elevation of	limiting factor	<u>822.5±</u> ft			
Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frag.	% Fines	Hydraulic App. Rate Inches/Hr
1	0-24	10YR6/2		GRS*	0,sg	mfr	а	15-35	5-10	3.60
2	24-48	10YR2/1		SIC**	0,m	mfr	g	<5	>90	0.07
3	48-72	2.5Y6/1	c2f 10YR7/3	SCL	0,m	mfr	g	<5	55	0.11
4	72-96	10YR5/6	m2p 10YR6/3	с	0,m	mfi	g	<5	>90	0.07
5	96-126	10YR6/2		с	0,m	mfi	g	<5	>90	0.07
6	126-186	10YR6/1		SCL	0,m	mfr	g	<15	55	0.11
7	186-240	10YR6/2		SL	0,m	mfi	g	<15	50	0.50
Comments:		* FILL (Crushed/Reproces	sed Concrete) ** I	Buried Topso	il					
SW-7 C	)bs. #	X Boring Pit Ground Sur	face Elev794±	_ft	Elevation of	limiting factor	<u>794±</u> ft			
Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frag.	% Fines	Hydraulic App. Rate Inches/Hr
1	0-48	10YR2/1		SIC*	0,m	mfr	g	<5	>90	0.07
2	48-66	10YR6/8	m2p 10YR7/1	SL	0,sg	mfr	g	<15	15-20	0.50
3	66-126	10YR6/2		GRSCL	0,m	mfi	g	15-35	50	0.11
4	126-240	10YR6/2		L	0,m	mfi	g	<15 60		0.24
Comments:		* Topsoil								
SW-8 C	)bs. #	X Boring Pit Ground Sur	face Elev797.5±	_ft	Elevation of	limiting factor	<u>797.5±</u> ft			
Horizon	Depth in.	Dominant Color Munsell	Redox Description Qu. Sz. Cont. Color	Texture	Structure Gr. Sz. Sh.	Consistence	Boundary	% Rock Frag.	% Fines	Hydraulic App. Rate Inches/Hr
1	0-18	Topsoil (No	t Sampled)							
2	18-48	10YR7/1	m2d 10YR6/4	с	0,m	mfi	g	<5	>90	0.07
3	48-66	10YR6/4	m2d 10YR7/1	SCL	0,m	mfi	g	5-10	50	0.11
4	66-96	10YR6/2	c2d 10YR6/2	SC	0,m	mfi	g	5	50	0.04
5	96-186	10YR6/1		GRL	0,m	mfi	g	15-35	50-60	0.24
6	186-240	10YR5/1		SC	0,m	mfr	g	<15	55	0.04
Comments:										

### Appendix F: Storm Sewer Basin Map



60" RCP E & S (COULD NOT MEASURE) SANITARY MANHOLE RIM = 797.0048" PVC N INV. = 783.00 60" RCP E & S INV. = 780.86

SANITARY MANHOLE RIM = 796.9948" PVC N INV. = 783.30

18" RCP INV. = 791.45

### - STORM SEWER BASIN P

BASIN U

## - STORM SEWER

41' OF 24" HDPE @ 1%

BASIN H

## - STORM SEWER

- STORM SEWER BA\$IN G

## IE 10"= 802.00

BASIN E OUTLET STRUCTURE RIM= 805.00 IE 20° V-NOTCH= 802.00

### BASIN D - STORM SEWER

- DOWNSPOUT LEAD COLLECTOR 5 259' OF 8" HDPE @ 1.25% (100 YR = 597 GPM) - STORM SEWER

CONNECT PROPOSED 8" STORM PIPE TO EXISTING CATCH BASIN AT IE=794.00. CONTRACTOR TO FIELD VERIFY STRUCTURAL INTEGRITY OF EXISTING CATCH BASIN AND REPLACE AS NECESSARY.

\_\_\_\_\_24" RCP INV. = 791.66

### - STORM PIPE Y 289' OF 10" HDPE @ 1.5%

- STORM SEWER BASIN W

### - STORM SEWER BASIN C

#### - STORM MH 1 [4'] RIM = 808.00 IE N = 801.06 IE S = 801.06

STORM PIPE F 260' OF 8" HDPE @ 2% (100 YR = 507 GPM) - STORM SEWER BASIN F

RIM = 815.00 IE S = 811.00

### - STORM SEWER BASIN B

257' OF 10" HDPE @ 0.36%

## - STORM PIPE Z

82' OF 15" HDPE @ 1% (100 YR = 2,724 GPM) SANITARY MANHOLE -RIM = 800.3248" RCP N & S INV. = 785.59

# STORM PIPE C

IE STORM=808.78 IE SAN=805.37

### STORM PIPE AA 391' OF 8" HDPE @ 1.5%

### - STORM SEWER BASIN A

### ROCK RIVER RIDGE ENDSECTION IE = 801.00

![](_page_94_Picture_43.jpeg)

OPEN (AC)

0.03

0.02

0.02

0.28

0.03

0.05

0.01

0.11

0.27

0.21

0.17

0.13

0.45

0.30

0.07

0.00

0.00

0.00

0.00

0.10

OPEN

(SF)

1,371

4,239

734

735

12,385

1,520

2,364

601 4,800

1,947

9,230

7,603

5,511

19,792

12,870

2,872

0

0

0

0

0.15 0 0.00

0 0.00 0 0.00

(AC)

0.26

0.29

0.30

0.19

0.14

0.21

0.06

0.32

0.15

0.15

0.12

0.12

0.15

0.15

0.15

0.15

PIPE BASIN

U

v

W

33.870

5.280

6.498

6,498

6,498

6,498

0.15

0.15

0.15

6,498 0.15 6,498

6,498

6,498

6,498

STORM SEWER BASIN MAP

![](_page_94_Picture_45.jpeg)

### Appendix G: Storm Sewer TR-55 Calculations

![](_page_96_Figure_0.jpeg)

### Area Listing (selected nodes)

Area (acres)	CN	Description (subcatchment-numbers)
3.642	98	(9S, 10S, 11S, 12S, 13S, 14S, 15S, 16S, 17S, 18S, 19S, 20S, 21S, 22S, 23S,
2.263	74	(9S, 10S, 11S, 12S, 13S, 14S, 15S, 16S, 17S, 18S, 19S, 20S, 21S, 22S, 23S, 24S)

### Soil Listing (selected nodes)

Soil	Subcatchment
Group	Numbers
HSG A	
HSG B	
HSG C	
HSG D	
Other	9S, 10S, 11S, 12S, 13S, 14S, 15S, 16S, 17S, 18S, 19S, 20S, 21S, 22S, 23S, 24S, 25S, 26S, 27S
	Soil Group HSG A HSG B HSG C HSG D Other

Ground Covers (selected nodes)												
HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers					
0.000	0.000	0.000	0.000	5.905	5.905		9S, 10S, 11S, 12S, 13S, 14S, 15S, 16S, 17S, 18S, 19S, 20S, 21S, 22S, 23S, 24S, 25S, 26S, 27S					

### Ground Covers (selected nodes)

### hydrocad240136200

 MSE 24-hr 3
 100-YEAR Rainfall=6.19"

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 10/29/2024

 ons LLC
 Page 5

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> Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment9S: SEWER A	Runoff Area=12,555 sf 89.08% Impervious Runoff Depth>5.41" Tc=6.0 min CN=95 Runoff=2.52 cfs 0.130 af
Subcatchment10S: SEWER B	Runoff Area=17,077 sf 75.18% Impervious Runoff Depth>5.09" Tc=6.0 min CN=92 Runoff=3.33 cfs 0.166 af
Subcatchment11S: SEWER C	Runoff Area=10,199 sf 92.80% Impervious Runoff Depth>5.52" Tc=6.0 min CN=96 Runoff=2.06 cfs 0.108 af
Subcatchment12S: SEWER D	Runoff Area=7,944 sf 90.75% Impervious Runoff Depth>5.52" Tc=6.0 min CN=96 Runoff=1.61 cfs 0.084 af
Subcatchment13S: SEWER E	Runoff Area=25,631 sf 51.68% Impervious Runoff Depth>4.43" Tc=6.0 min CN=86 Runoff=4.59 cfs 0.217 af
Subcatchment14S: SEWER F	Runoff Area=13,281 sf 88.56% Impervious Runoff Depth>5.41" Tc=6.0 min CN=95 Runoff=2.67 cfs 0.138 af
Subcatchment15S: SEWER G	Runoff Area=10,820 sf 78.15% Impervious Runoff Depth>5.20" Tc=6.0 min CN=93 Runoff=2.13 cfs 0.108 af
Subcatchment16S: SEWER H	Runoff Area=7,815 sf 92.31% Impervious Runoff Depth>5.52" Tc=6.0 min CN=96 Runoff=1.58 cfs 0.082 af
Subcatchment17S: SEWER I	Runoff Area=10,955 sf 56.18% Impervious Runoff Depth>4.54" Tc=6.0 min CN=87 Runoff=1.99 cfs 0.095 af
Subcatchment18S: SEWER J	Runoff Area=19,472 sf 38.65% Impervious Runoff Depth>4.11" Tc=6.0 min CN=83 Runoff=3.30 cfs 0.153 af
Subcatchment19S: SEWER K	Runoff Area=16,867 sf 45.28% Impervious Runoff Depth>4.33" Tc=6.0 min CN=85 Runoff=2.97 cfs 0.140 af
Subcatchment20S: SEWER L	Runoff Area=16,596 sf 54.19% Impervious Runoff Depth>4.54" Tc=6.0 min CN=87 Runoff=3.02 cfs 0.144 af
Subcatchment21S: SEWER M	Runoff Area=8,078 sf 31.78% Impervious Runoff Depth>4.01" Tc=6.0 min CN=82 Runoff=1.34 cfs 0.062 af
Subcatchment22S: SEWER N	Runoff Area=33,870 sf 41.56% Impervious Runoff Depth>4.22" Tc=6.0 min CN=84 Runoff=5.85 cfs 0.273 af
Subcatchment23S: SEWER O	Runoff Area=19,598 sf 34.33% Impervious Runoff Depth>4.01" Tc=6.0 min CN=82 Runoff=3.25 cfs 0.150 af
Subcatchment24S: SEWER P	Runoff Area=9,415 sf 69.50% Impervious Runoff Depth>4.98" Tc=6.0 min CN=91 Runoff=1.82 cfs 0.090 af

hydrocad240136200 Prepared by Excel Engineering HydroCAD® 10.20-4a s/n 01178 © 2023 Hydro	MSE 24-hr 3 100-YEAR Rainfall=6.19" Printed 10/29/2024 CAD Software Solutions LLC Page 6
Subcatchment25S: SEWER Q	Runoff Area=5,280 sf 100.00% Impervious Runoff Depth>5.70" Tc=6.0 min CN=98 Runoff=1.08 cfs 0.058 af
Subcatchment26S: SEWER R	Runoff Area=5,280 sf 100.00% Impervious Runoff Depth>5.70" Tc=6.0 min CN=98 Runoff=1.08 cfs 0.058 af
Subcatchment27S: SEWER STUVW	Runoff Area=6,498 sf 100.00% Impervious Runoff Depth>5.70" Tc=6.0 min CN=98 Runoff=1.33 cfs 0.071 af

### Summary for Subcatchment 9S: SEWER A

Runoff = 2.52 cfs @ 12.13 hrs, Volume= 0.130 af, Depth> 5.41"

A	rea (sf)	CN [	Descriptio	n								
*	11,184	98										
*	1,371	74										
	12,555	95 V	Veighted	Average								
	1,371	1	0.92% Pe	ervious A	rea							
	11,184	8	39.08% In	npervious	Area							
				-								
Tc	Length	Slope	Velocity	<ul> <li>Capac</li> </ul>	ity Desc	cription						
(min)	(feet)	(ft/ft)	(ft/sec)	) (ct	fs)							
6.0					Dire	ct Entr	у,					
				Subcat	chment	: 9S: S	SEWE	ER A				
				Ну	/drograph							
-												
					2.52 cfc		į.					Runoff
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(cfs					K		Rui	noff [	Depth	>5.4	1"	
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### Summary for Subcatchment 10S: SEWER B

Runoff = 3.33 cfs @ 12.13 hrs, Volume= 0.166 af, Depth> 5.09"

	A	rea (sf)	CN	De	escrip	otion											
*		12,838	98														
*		4,239	74														
		17,077	92	W	eighte	ed Av	verage	е									
		4,239		24	1.82%	Per	vious	Area									
		12,838		75	5.18%	5 Imp	erviou	us Are	ea								
						-											
	Тс	Length	Slop	ре	Velo	city	Capa	acity	Descr	iption							
(m	in)	(feet)	(ft/	ft)	(ft/s	ec)	(	(cfs)									
6	6.0								Direct	t Entr	у,						
						Sı	ubca	tchn	nent 1	0S: 3	SEW	ER E	3				
							1	Hydro	graph								
	ſ						-	-							1		Runoff
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				i	1		i I			Rur	noff	Vol	ume	=0.	166	af	
cfs)	2	1		<sub>l</sub> .	T I	r — — — - I	 	т — — т 1		т — — — Т І	Ru	noff		hth	>5 0	<b>a''</b>	
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### Summary for Subcatchment 11S: SEWER C

Runoff = 2.06 cfs @ 12.13 hrs, Volume= 0.108 af, Depth> 5.52"

A	rea (sf)	CN E	Description	l								
*	9,465	98										
*	734	74										
	10,199	96 V	Veighted A	verage								
	734	7	7.20% Perv	/ious Area								
	9,465	g	2.80% Im	pervious Ar	ea							
Tc	Length	Slope	Velocity	Capacity	Desc	ription						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)								
6.0					Direc	t Entr	y,					
			S	ubcatch	ment '	11S: \$	SEWE	ER C				
				Hydro	ograph							
					1			1	1			<b></b>
-												Runoff
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-						100-	YEA	R Rai	nfall	=6.1	9"	
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-				1 1		Run	OTT	volun	1e=0.	.108	at	
(cfs)					<b>K</b>		Rur	noff D	epth	>5.5	2"	
MC						+		+	TC			
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_										CN=	96	
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### Summary for Subcatchment 12S: SEWER D

Runoff 1.61 cfs @ 12.13 hrs, Volume= 0.084 af, Depth> 5.52" =

	Area (sf)	CN E	Description									
*	7,209	98										
*	735	74										
	7,944	96 V	Veighted A	verage								
	735	ç	9.25% Perv	vious Area								
	7,209	ç	90.75% Imp	pervious Ar	ea							
T (mir	c Length ) (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Descr	iption						
6.	0				Direc	t Entry	Ι,					
			c	ubaatabi	mant d	120.0						
			3	upcatchi	nent	25: 3		RD				
				Hydro	graph			1				
	<b>1</b>											Runoff
	1	i I		1.61	cfs		i I					
								IVI	<b>∋</b> ⊏ 2	(4-N	r 3	
						100+`	YEAF	R Rair	nfall	=6.1	9"	
						R	luno	ff Are	a=7.	944	sf	
						Run	off	olum	e=0	084	af	
(sj	1	+	- +			IXMII			v v.		011	
<u>5</u> ×							Run		ptn	20.0	<b>Z</b>	
Flov					1			Т	c=6	.0 m	in	
										CN=	96	
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					<u> </u>							
	5 6	7 8	9 10	11 12	13 (hours)	14	15	16 17	18	19	20	

### Summary for Subcatchment 13S: SEWER E

Runoff = 4.59 cfs @ 12.13 hrs, Volume= 0.217 af, Depth> 4.43"

	<u> </u>	rea (sf)	CN	Descr	ription											
*		13,246	98													
*		12,385	74													
		25,631	86	Weigh	nted A	verage	;									
		12,385		48.32	% Pe	rvious A	Area									
		13,246		51.68	% Im	perviou	s Area									
	Tc	Length	Slop	e Vel	ocity	Capa	city D	escrip	tion							
(r	min)	(feet)	(ft/f	t) (ft/	/sec)	(0	cfs)									
	6.0						D	irect l	Entry	/,						
					_							_				
					S	ubcat	chme	ent 13	S: 8	5EW	ERE					
						н	lydrogra	iph								
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	5-		1				4.50 of c			1	1	1				Runoff
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	1			!	_ <u> </u>											
	4-							1	00-`	YEA	AK F	kain	fall	=6.1	9"	
	1				i			i	Ri	JNO	ff A	rea≑	25.	631	sf	
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	0	U U	1	0 9	10		Time (h	nours)	14	10	10	17	10	19	20	

### Summary for Subcatchment 14S: SEWER F

Runoff = 2.67 cfs @ 12.13 hrs, Volume= 0.138 af, Depth> 5.41"

* 11,761 98 * 1,520 74 13,281 95 Weighted Average	
* 1,520 74 13,281 95 Weighted Average	
13 281 95 Weighted Average	
1,520 11.44% Pervious Area	
11,761 88.56% Impervious Area	
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)	
6.0 Direct Entry,	
Subcatchment 14S: SEWER F	
Hydrograph	
	Runoff
	r 3
	1 5
100-YEAR Rainfall=6.1	9"
Runoff Area=13-281	ef-
	31
Runoff Volume=0.138	af
Bunoff Denth>5 4	1"
≗   Tc=6.0 m	nin
1 /	95
	1
5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 Time (hours)	20
### Summary for Subcatchment 15S: SEWER G

Runoff = 2.13 cfs @ 12.13 hrs, Volume= 0.108 af, Depth> 5.20"

	A	rea (sf)	CN	De	escrip	otion											
*		8,456	98														
*		2,364	74														
		10,820	93	W	/eighte	ed Av	/erage	;									
		2,364		21	1.85%	Perv	vious	Area									
		8,456		78	3.15%	lmp	erviou	s Are	ea								
(m	Tc	Length	Sloj (ft/	oe ft)	Velo	city	Capa	city	Desc	ription	I						
	6.0	(ieet)	(10	<u></u>	(10.5	60)	(	013)	Direc	t Ent	ry,						
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	0	0		5	Ŭ			Time	(hours)	) . ,	10			.0	10	20	

### Summary for Subcatchment 16S: SEWER H

Runoff = 1.58 cfs @ 12.13 hrs, Volume= 0.082 af, Depth> 5.52"

	Area (sf)	CN D	Description							
*	7,214	98								
*	601	74								
	7,815	96 V	Veighted A	verage						
	601	7	.69% Perv	ious Area						
	7,214	9	2.31% Imp	pervious Ar	ea					
To	Length	Slope	Velocity	Capacity	Descr	iption				
(min)	(teet)	(11/11)	(π/sec)	(CIS)	Diroct	Entry				
0.0					Direct	Entry,				
			S	ubcatchr	ment 1	6S: SE	WER I	4		
				Hydro	graph					
										Runoff
				1.58	3 cfs					
								MSE 2	24-hr 3	
	-					100-YE	EAR F	Rainfall	=6.19"	
						Ru	noff	Aroa=7	815 cf	
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	/					Kunoi	ΤΟΙ	ume=u	.082 at	
(cfs)						R	unof	f Depth	>5.52"	
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### Summary for Subcatchment 17S: SEWER I

Runoff = 1.99 cfs @ 12.13 hrs, Volume= 0.095 af, Depth> 4.54"

A	rea (sf)	CN E	Description								
*	6,155	98									
*	4,800	74									
	10,955	87 V	Veighted A	verage							
	4,800	4	3.82% Pe	rvious Area	l						
	6,155	5	56.18% Imp	pervious Ar	ea						
Та	Longth	Slope	Valaaity	Conocity	Dooo	rintion					
(min)	(feet)	(ft/ft)	(ft/sec)	Capacity (cfs)	Desc	приоп					
6.0	(1001)	(1010)	(10000)	(010)	Direc	t Entry.					
						, <b>,</b>					
			S	Subcatch	ment	17S: S	EWER	I			
				Hydro	graph						
L							1				Pupoff
	,		-i+	-i + - <b>1.9</b>	) cfs	-i		i +			Kunon
2-								MSE 2	24-hr	3	
-		1				100-Y		Rainfall	=6 19	<b>.</b>	
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-		l l				Rui		rea=10	,955 \$	ST	
-						Runc	off Vol	ume=0	.095 a	af	
cfs)					<b>K</b>	: : F	Runof	f Depth	>4.54	L''	
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	5 6	7 8	9 10	11 12 Tim	13 e (hours)	14	15 16	17 18	19	20	

### Summary for Subcatchment 18S: SEWER J

Runoff = 3.30 cfs @ 12.13 hrs, Volume= 0.153 af, Depth> 4.11"

	A	rea (sf)	CN	Desc	cription									
*		7,525	98											
*		11,947	74											
		19,472	83	Weig	ghted A	verage								
		11,947		61.3	5% Pe	rvious A	rea							
		7,525		38.6	5% Imp	pervious	s Area							
,	Τc	Length	Slop	be Ve	elocity	Capac	city De	scription						
(n	nin)	(feet)	(ft/1	ft) (1	tt/sec)	(C	ts)							
	6.0						Dir	ect Enti	r <b>y</b> ,					
					-									
					S	ubcat	chmen	t 18S:	SEW	/ER J				
						H	ydrograp	h						
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### Summary for Subcatchment 19S: SEWER K

Runoff = 2.97 cfs @ 12.13 hrs, Volume= 0.140 af, Depth> 4.33"

	Ar	rea (sf)	CN	Des	scriptior	1										
*		7,637	98													
*		9,230	74													
		16,867	85	Wei	ighted A	Averag	e									
		9,230		54.7	72% Pe	rvious	Area									
		7,637		45.2	28% Im	pervio	us Ar	ea								
	_					_		_								
,	ŢĊ	Length	Slop	e V	/elocity	Cap	acity	Desci	ription	l						
(m	iin)	(feet)	(ft/f	t) (	(ft/sec)		(cfs)									
(	6.0							Direc	t Enti	r <b>y</b> ,						
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					3	SOCIO	itchr	nent	192:	3EM		•				
							Hydro	graph								
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	- 1								R	uno	ff A	rea=	16,8	867 :	sf	
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							Time	e (hours)								

### Summary for Subcatchment 20S: SEWER L

Runoff = 3.02 cfs @ 12.13 hrs, Volume= 0.144 af, Depth> 4.54"

* $3.993$ 98 * $7,603$ 74 16,596 87 Weighted Average 7,603 45.81% Pervious Area 3.993 54.19% Impervious Area Tc Length Slope Velocity Capacity Description (min) (feet) (ft/fs) (ft/sec) (cfs) 6.0 Direct Entry, Subcatchment 20S: SEWER L Hydrograph MSE 24-hr 3 100-YEAR Rainfall=6.19" Runoff Area=16,596 sf Runoff Volume=0.144 af Runoff Volume=0.144 af Runoff Depth>4.54" Tc=6.0 min CN=87 0 0 0 0 0 0 0 0 0 0 0 0 0		A	rea (sf)	CN	Des	scription										
* $7,603$ 74 16,596 87 Weighted Average 7,603 45.81% Pervious Area 8,993 54.19% Impervious Area Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs) 6.0 Direct Entry, Subcatchment 20S: SEWER L Hydrograph 100-YEAR Rainfall=6.19" Runoff Area=16,596 sf Runoff Volume=0.144 af Runoff Depth>4.54" Tc=6.0 min CN=87 0 6.0 CN=87 100-11 12 13 14 15 16 17 18 19 20	*		8,993	98												
16,596 87 Weighted Average 7,603 45.81% Pervious Area 8,993 54.19% Impervious Area Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs) 6.0 Direct Entry, Subcatchment 20S: SEWER L Hydrograph MSE 24-hr 3 100-YEAR Rainfall=6.19" Runoff Area=16,596 sf Runoff Volume=0.144 af Runoff Volume=0.144 af Runoff Depth>4.54" Tc=6,0 min CN=87 0 0 0 0 0 0 0 0 10 10 10 10	*		7,603	74												
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8,993 54.19% Impervious Area Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs) 6.0 Direct Entry, Subcatchment 20S: SEWER L Hydrograph MSE 24-hr 3 100-YEAR Rainfall=6.19" Runoff Area=16,596 sf Runoff Volume=0.144 af Runoff Volume=0.144 af Runoff Depth>4.54" Tc=6.0 min CN=87			7,603		45.8	31% Pe	rvious /	Area								
Tc       Length (feet)       Slope Velocity (ft/sc)       Capacity (cfs)       Description         6.0       Direct Entry,         Subcatchment 20S: SEWER L         Hydrograph         0       MSE 24-hr 3         100-YEAR Rainfall=6.19"         Runoff Area=16,596 sf         Runoff Volume=0.144 af         Runoff Depth>4.54"         Tc=6.0 min         0			8,993		54.1	19% Im	perviou	s Area								
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs) 6.0 Direct Entry, Subcatchment 20S: SEWER L Hydrograph MSE 24-hr 3 100-YEAR Rainfall=6.19" Runoff Area=16,596 sf Runoff Depth>4.54" Tc=6.0 min CN=87 0 0 0 0 0 0 0 0 0 0 0 0 0																
$(\min) (feet) (ft/ft) (ft/sec) (cfs)$ $6.0 Direct Entry,$ Subcatchment 20S: SEWER L $Hydrograph$ $MSE 24-hr 3$ $100-YEAR Rainfall=6.19"$ Runoff Area=16,596 sf Runoff Volume=0.144 af Runoff Depth>4.54" Tc=6.0 min CN=87 $CN=87$		Тс	Length	Slop	be V	elocity/	Capa	city De	escriptio	า						
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Hydrograph MSE 24-hr 3 100-YEAR Rainfall=6.19" Runoff Area=16,596 sf Runoff Depth>4.54" Tc=6.0 min CN=87						S	ubcat	tchme	nt 20S:	SEW	/ER L	-				
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		5	6	7	8	9 10	11	12 Time (ho	13 14 Durs)	15	16	17	18	19	20	

### Summary for Subcatchment 21S: SEWER M

Runoff = 1.34 cfs @ 12.13 hrs, Volume= 0.062 af, Depth> 4.01"

A	rea (sf)	CN	Descriptio	on							
*	2,567	98									
*	5,511	74									
	8,078	82	Weighted	Average							
	5,511		68.22% P	ervious Are	a						
	2,567		31.78% Ir	npervious A	rea						
Tc	Length	Slope	e Velocit	y Capacity	/ Descri	ption					
(min)	(feet)	(ft/ft	) (ft/sec	) (cfs	)						
6.0					Direct	Entry	,				
				Subcatch	ment 2	1S: S	EWER	M			
				Hyd	rograph						
		I				1	I				
											Runoff
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1-		l I									
						Run		olume	e=0.0e	o2 at	
cfs)							Runo	ff De	oth>4	1.01"	
3		1									
Εlo		l					I I		C=6.U	min	
1		l I				1	I		C	N=82	
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5	6	7 8	9	10 11	12 13	14	15 16	6 17	18	19 20	
					me (nours)						

### Summary for Subcatchment 22S: SEWER N

Runoff = 5.85 cfs @ 12.13 hrs, Volume= 0.273 af, Depth> 4.22"

	A	rea (sf)	CN	De	scripti	on											
*		14,078	98														
*		19,792	74														
-		33,870	84	We	eighteo	d Av	erage										
		19,792		58.	.44% F	Perv	ious Ai	rea									
		14,078		41.	.56% I	Impe	ervious	Area									
(m	Tc nin)	Length (feet)	Slop (ft/f	e \ t)	Veloci (ft/se	ty c)	Capaci (cf	ity De	escrip	otion							
(	6.0			,				Di	irect	Entr	у,						
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						Su	bcato	hme	nt 22	S:	SEW	ERI	N				
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	ſ													1			Runoff
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(cfs)	-							T I		1	Ru	nof	f De	pth	>4.2	2"	
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### Summary for Subcatchment 23S: SEWER O

Runoff = 3.25 cfs @ 12.13 hrs, Volume= 0.150 af, Depth> 4.01"

* 6,728 98		
* 12,870 74		
19,598 82 Weighted Average		
12,870 65.67% Pervious Area		
6,728 34.33% Impervious Area		
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)		
6.0 Direct Entry,		
Subcatchmont 23S: SEWER O		
		Runoff
3.25 cfs		
· · · · · · · · · · · · · · · · · · ·	24-hr 3	
<sup>3</sup> 100-YEAR Rainfall	=6.19"	
Runoff Area=19	598 sf	
Runoff Volume=0	150 af	
	>4.01	
ê III	.0 min	
	CN=82	
	+	
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	19 20	

### Summary for Subcatchment 24S: SEWER P

Runoff = 1.82 cfs @ 12.13 hrs, Volume= 0.090 af, Depth> 4.98"

	A	rea (sf)	CN	De	escrip	tion											
*		6,543	98														
*		2,872	74														
		9,415	91	W	eighte	ed Av	/erage										
		2,872		30	).50%	Perv	ious A	rea									
		6,543		69	9.50%	Imp	ervious	s Area	a								
	Тс	Length	Slop	be	Velo	city	Capad	city l	Descr	iption	1						
(m	in)	(feet)	(ft/1	ft)	(ft/se	ec)	(C	fs)									
(	6.0								Direc	t Ent	ry,						
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cfs)	1		 		 						Ru	inof		oth	<u></u>	Q''	
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								Time	(hours)								

#### Summary for Subcatchment 25S: SEWER Q

Runoff = 1.08 cfs @ 12.13 hrs, Volume= 0.058 af, Depth> 5.70"



#### Summary for Subcatchment 26S: SEWER R

Runoff = 1.08 cfs @ 12.13 hrs, Volume= 0.058 af, Depth> 5.70"



### Summary for Subcatchment 27S: SEWER STUVW

Runoff = 1.33 cfs @ 12.13 hrs, Volume= 0.071 af, Depth> 5.70"



## Appendix H: Storm Sewer Manning's Spreadsheet



Excel Engineering Project No.

240136200

#### Project Name Lumin Terrace

	Pij	pe Data			Pipe	Capacity (100-y	r)	
Pipe ID	Diameter (FT)	Slope (FT/FT)	Manning's n	Basin No.	Total Flow (cfs)	Total Flow (gpm)	Full Flow Capacity (cfs)	Full Flow Capacity (gpm)
А	1	0.010	0.012	A	2.52	1131	3.87	1737
В	1.5	0.010	0.012	A,B,Q	6.93	3110	11.41	5121
С	1.5	0.010	0.012	A,B,C,Q	8.99	4035	11.41	5121
D	1.5	0.010	0.012	A,B,C,D,Q	10.60	4757	11.41	5121
E	2	0.010	0.012	A,B,C,D,E,Q,T,W	17.85	8011	24.57	11029
F	0.83	0.020	0.012	F	2.67	1198	3.37	1511
G	2	0.010	0.012	A,B,C,D,E,F,G,Q,T,W	22.65	10165	24.57	11029
Н	2	0.010	0.012	A,B,C,D,E,F,G,H,Q,T,W	24.23	10874	24.57	11029
I	1.00	0.003	0.012	l	1.99	893	2.12	951
J	1.5	0.003	0.012	I,J	5.29	2374	6.25	2805
К	2	0.005	0.012	I,J,K,R	9.34	4192	17.38	7798
L	2	0.010	0.012	I,J,K,L,R	12.36	5547	24.57	11029
М	2	0.010	0.012	I,J,K,L,M,R,S	15.03	6745	24.57	11029
Ν	2	0.010	0.012	I,J,K,L,M,N,R,S	20.88	9371	24.57	11029
0	2.5	0.010	0.012	I,J,K,L,M,N,O,R,S,V	25.46	11426	44.55	19996
Р	1.5	0.010	0.012	I,J,K,L,M,N,O,P,R,S,U,V	28.61	12840	11.41	5121
DS	0.666666667	0.010	0.012	Q	1.08	485	1.31	589
DS	0.666666667	0.013	0.012	S	1.33	597	1.47	659

Full Flow Capacity based off Manning's Equation

 $Q = \frac{1.49}{n} R^{2/3} S^{1/2} a$ 

Where:

Q = Full Flow Capacity of Pipe (cfs)

n = manning's roughness coefficient

R = hydraulic radius (ft) (D/4)

s = hydraulic gradient, slope (ft/ft)

a = flow area (sq. ft.)

Typical Manning's n	
HDPE	0.012
PVC	0.012
Concrete	0.013
CMP	0.024

\*Total Flow calculated via TR-55 hydrologic calculations. Reference Storm Pipe Basin Map & TR-55 Calculations

Appendix I: SLAMM Input/ Output Information



Data file name: \\job-files\2024 Job Files\240136200 Horizon - Lumin Terrace Multifamily - Watertown WI\240136204 Civil\storm water report and calculations\2024-10-10 SUBMITTAL\CALCS\slamm2340136200.mdb WinSLAMM Version 10.5.0 Rain file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WisReg - Madison WI 1981.ran Particulate Solids Concentration file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\v10.1 WI AVG01.pscx Runoff Coefficient file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI SL06 Dec06.rsvx Residential Street Delivery file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI Res and Other Urban Dec06.std Institutional Street Delivery file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI Com Inst Indust Dec06.std Commercial Street Delivery file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI Com Inst Indust Dec06.std Industrial Street Delivery file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI Com Inst Indust Dec06.std Other Urban Street Delivery file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI Res and Other Urban Dec06.std Freeway Street Delivery file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\Freeway Dec06.std Apply Street Delivery Files to Adjust the After Event Load Street Dirt Mass Balance: False Pollutant Relative Concentration file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI GE003.ppdx Source Area PSD and Peak to Average Flow Ratio File: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\NURP Source Area PSD Files.csv Cost Data file name: If Other Device Pollutant Load Reduction Values = 1, Off-site Pollutant Loads are Removed from Pollutant Load % Reduction calculations Seed for random number generator: -42 Study period starting date: 01/01/81 Study period ending date: 12/31/81 Start of Winter Season: 12/02 End of Winter Season: 03/12 Date: 10-30-2024 Time: 08:24:21 Site information: LU# 1 - Residential: A Total area (ac): 7.170 1 - Roofs 1: 1.130 ac. Pitched Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz 13 - Paved Parking 1: 2.850 ac. Source Area PSD File: C:\WinSLAMM Files\NURP.cpz Connected 45 - Large Landscaped Areas 1: 3.190 ac. Normal Silty Source Area PSD File: C:\WinSLAMM

Files\NURP.cpz

LU# 2 - Residential: D Total area (ac): 0.180 31 - Sidewalks 1: 0.180 ac. Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz LU# 3 - Residential: C Total area (ac): 0.750 31 - Sidewalks 1: 0.010 ac. Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz 45 - Large Landscaped Areas 1: 0.740 ac. Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz LU# 4 - Residential: B Total area (ac): 0.470 1 - Roofs 1: 0.200 ac. Pitched Connected Source Area PSD File: C:\WinSLAMM Files\NURP.cpz 45 - Large Landscaped Areas 1: 0.270 ac. Normal Silty Source Area PSD File: C:\WinSLAMM Files\NURP.cpz Control Practice 1: Wet Detention Pond CP# 1 (DS) - DS Wet Pond # 1 Particle Size Distribution file name: Not needed - calculated by program Initial stage elevation (ft): 6 Peak to Average Flow Ratio: 3.8 Maximum flow allowed into pond (cfs): No maximum value entered Outlet Characteristics: Outlet type: V - notch weir 1. Weir angle (degrees): 20 2. Weir height from invert: 0 3. Invert elevation above datum (ft): 6 Outlet type: Broad Crested Weir 1. Weir crest length (ft): 10 2. Weir crest width (ft): 10 3. Height from datum to bottom of weir opening: 10.5 Pond stage and surface area Entry Stage Pond Area Natural Seepage Other Outflow Number (ft) (acres) (in/hr) (cfs) 0 0.00 0.0000 0.00 0.00 1 0.0700 0.00 0.00 0.01 2 1.00 0.0900 0.00 0.00 3 2.00 0.1000 0.00 0.00 4 3.00 0.1300 0.00 0.00 5 4.00 0.1500 0.00 0.00 6 0.00 5.00 0.1600 0.00

7	6.00	0.3000	0.00	0.00
8	7.00	0.4000	0.00	0.00
9	8.00	0.4600	0.00	0.00
10	9.00	0.5300	0.00	0.00
11	10.00	0.5900	0.00	0.00
12	11.00	0.6400	0.00	0.00
13	11.50	0.6700	0.00	0.00

Control Practice 2: Other Device CP# 1 (DS) - DS Other Device # 1 Fraction of drainage area served by device (ac) = 1.00 Particulate Concentration reduction fraction = 1.00 Filterable Concentration reduction fraction = 1.00 Runoff volume reduction fraction = 0

```
Control Practice 3: Biofilter CP# 1 (DS) - DS Biofilters # 1
```

- 1. Top area (square feet) = 15000
- 2. Bottom aea (square feet) = 3300
- 3. Depth (ft): 8.5
- 4. Biofilter width (ft) for Cost Purposes Only: 10
- 5. Infiltration rate (in/hr) = 0.5
- 6. Random infiltration rate generation? No
- 7. Infiltration rate fraction (side): 1
- 8. Infiltration rate fraction (bottom): 1
- 9. Depth of biofilter that is rock filled (ft) 0
- 10. Porosity of rock filled volume = 0
- 11. Engineered soil infiltration rate: 0
- 12. Engineered soil depth (ft) = 0
- 13. Engineered soil porosity = 0
- 14. Percent solids reduction due to flow through engineered soil = 0

```
15. Biofilter peak to average flow ratio = 3.8
```

```
16. Number of biofiltration control devices = 1
```

```
17. Particle size distribution file: Not needed - calculated by program
```

18. Initial water surface elevation (ft): 0

Soil Data Soil Type Fraction in Eng. Soil Biofilter Outlet/Discharge Characteristics:

Outlet type: Broad Crested Weir

```
1. Weir crest length (ft): 10
```

```
2. Weir crest width (ft): 10
```

3. Height of datum to bottom of weir opening: 7.5

Outlet type: Surface Discharge Pipe

- 1. Surface discharge pipe outlet diameter (ft): 0.67
- 2. Pipe invert elevation above datum (ft): 1
- 3. Number of surface pipe outlets: 1

SLAMM for Windows Version 10.5.0 (c) Copyright Robert Pitt and John Voorhees 2019, All Rights Reserved Data file name: \\job-files\2024 Job Files\240136200 Horizon - Lumin Terrace Multifamily - Watertown WI\240136204 Civil\storm water report and calculations\2024-10-10 SUBMITTAL\CALCS\slamm2340136200.mdb Data file description: Rain file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WisReg - Madison WI 1981.ran Particulate Solids Concentration file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\v10.1 WI AVG01.pscx Runoff Coefficient file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI SL06 Dec06.rsvx Pollutant Relative Concentration file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI GE003.ppdx Residential Street Delivery file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI Res and Other Urban Dec06.std Institutional Street Delivery file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI Com Inst Indust Dec06.std Commercial Street Delivery file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI Com Inst Indust Dec06.std Industrial Street Delivery file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI Com Inst Indust Dec06.std Other Urban Street Delivery file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\WI Res and Other Urban Dec06.std Freeway Street Delivery file name: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\Freeway Dec06.std Apply Street Delivery Files to Adjust the After Event Load Street Dirt Mass Balance: False Source Area PSD and Peak to Average Flow Ratio File: J:\Programs\civil\WinSLAMM\v10.5.0\Parameter Files\NURP Source Area PSD Files.csv Cost Data file name: If Other Device Pollutant Load Reduction Values = 1, Off-site Pollutant Loads are Removed from Pollutant Load % Reduction calculations Seed for random number generator: -42 Start of Winter Season: 12/02 End of Winter Season: 03/12 Model Run Start Date: 01/01/81 Model Run End Date: 12/31/81 Time of run: 08:23:55 Date of run: 10-30-2024 Total Area Modeled (acres): 8.570 Years in Model Run: 1.00 Runoff Percent Particulate Particulate Percent Volume Runoff Solids Solids Particulate (cu ft) Volume Conc. Yield Solids Reduction (mg/L)(lbs) Reduction

Total of all Land Uses without Controls:	384604	-	100.9
2423 -			
Outfall Total with Controls:	129292	66.38%	31.23
252.0 89.60%			
Annualized Total After Outfall Controls:	129647		
252.7			

. Percent Solids Reduction due to Engineered Media Not Used

## Appendix J: USLE Map and Calculations



ASSUMED ORDINARY HIGH WATER MARK OF ROCK RIVER

- USLE ROUTE 223'@ 5.7%

+18" RCP INV. = 791.45





## Soil Loss & Sediment Discharge Calculation Tool

for use on Construction Sites in the State of Wisconsin

WDNR Version 2.0 (06-29-2017)



YEAR 1

11/08/24

Project:

Developer:

Date:

County:	Jefferson	-												Version 1.0
Activity (1)	Begin Date (2)	End Date (3)	Period % R (4)	Annual R Factor (5)	Sub Soil Texture (6)	Soil Erodibility K Factor (7)	Slope (%) (8)	Slope Length (ft) (9)	LS Factor (10)	Land Cover C Factor (11)	Soil loss A (tons/acre) (12)	SDF (13)	Sediment Control Practice (14)	Sediment Discharge (t/ac) (15)
Bare Ground	04/01/25	06/08/25	20.4%	140	Sandy Loam 🚽	0.28	5.7%	223	0.94	1.00	7.5	0.923	Inlet Protection	4.9
End 🚽	06/08/25						5.7%	223	0.94			0.000	-	0.0
-							5.7%	223	0.94			0.000	-	0.0
-							5.7%	223	0.94			0.000	-	0.0
-							5.7%	0				0.000	-	0.0
-							0.0%	0				0.000	-	0.0
										TOTAL	7.5		TOTAL	4.9

#### Notes:

See Help Page for further descriptions of variables and items in drop-down boxes.

The last land disturbing activity on each sheet must be 'End'. This is either 12 months from the start of construction or final stabilization.

For periods of construction that exceed 12 months, please demonstrate that 5 tons/acre/year is not exceeded in any given 12 month period.

#### **Recommended Permanent Seeding Dates:**

4/1-5/15 and Thaw-6/30 8/7-8/29 Turf, introduced grasses and legumes Native Grasses, forbs, and legumes

NOTE: THIS TOOL ONLY ADDRESSED SOIL EROSION DUE TO SHEET FLOW. MEASURES TO CONTROL CHANNEL EROSION MAY ALSO BE REQUIRED TO MEET SEDIMENT DISCHARGE REQUIREMENTS.

% Reduction

Required

NONE

Designed By:	
Date	

# Appendix K: Post Construction Operation and Maintenance Plan

The owner of the property affected shall inspect and maintain the following stormwater management systems frequently, especially after heavy rainfalls, but at least on an annual basis unless otherwise specified.

STORMWATER FACILITY		TYPE OF ACTION			
1.	Lawn and	All lawn areas shall be kept clear of any materials that block the			
	Landscaped Areas	flow of stormwater. Rills and small gullies shall immediately be			
		filled and seeded or have sod placed in them. The lawn shall be			
		kept mowed, tree seedlings shall be removed, and litter shall be			
		removed from landscaped areas.			
2.	Rip Rap	All rip rap showing signs of erosion or scour shall be repaired,			
		reinforced, and revegetated immediately. Rip rap should be kept			
		clean of vegetation and sediment. All rip rap shall be repaired to			
		the construction plan requirements.			
3.	Catch Basin/Curb	The grate openings to these structures must be cleared of any			
	Inlet Grates	clogging or the blocking of stormwater flow from getting into the			
		stormwater conveyance system of any kind.			
4.	Retention/Detention	Trash racks, standpipes, outlet structures, inlet and outlet pipes,			
	Basins	and anti vortex devices shall be kept clear of debris. Non-			
		structurally sound devices shall be replaced. Floating litter and			
		algae shall be removed monthly. All grassed areas, embankments,			
		and flow control devices showing signs of erosion shall be			
		repaired, reinforced, and revegetated immediately to the original			
		plan requirements. Dry basins shall be mowed no less than twice			
		per year at a height of no less than 3 inches. Grasses shall not be			
		allowed to grow to a height that permits branching or bending.			
		Mowing shall only take place when the ground is dry and able to			
		support machinery. Every 5 years, the elevations of the pond			
		bottom shall be surveyed to determine the permanent pool depth			
		and sediment depth in the pond. When silt has accumulated three			
		feet from the original design depth elevation of the pond, the			
		pond shall be cleaned out and restored back to the original design			
		depth of a minimum of 5' from the normal water elevation.			
		Cleaning, removal, and deposit of silt from the detention pond			
		shall be done by means and methods acceptable to the Wisconsin			
		Department of Natural Resources.			
5.	Infiltration Basin	Inspections shall occur at minimum every 3 months. Inspections			
		shall include the spreader, overflow spillway, and the condition of			
		vegetation. To maintain vegetation, the first mowing of newly			
		planted seed shall occur once it reaches a height of 10 to 12			
		inches. Mowing shall reduce the height of plants to 5 to 6 inches.			
1		After establishment, if burning cannot be accommodated, mowing			
		shall occur once in the fall after November 1 <sup>st</sup> . Mowing shall			
		reduce the height of plants to 5 to 6 inches. If burning can take			

		place, beginning the second year, burning shall occur in the early spring prior to May 1 <sup>st</sup> , or in late fall after November 1st. Burning shall be done two consecutive years and then up to three years can pass before the next burning. Under no circumstances shall burning occur every other year. If standing water is observed over 50% of the basin floor 3 days after rainfall, the basin is considered clogged. If this ever occurs, remove the top 2 to 3 inches, chisel plow and add topsoil and compost. If deep tilling is used, the basin shall be drained and soils dried to a depth of 8 inches. Replant with turf grass. If clogging again occurs, the basin shall be replanted with prairie style vegetation. During winter conditions, all draw down devices in the pond shall be opened to discourage the infiltration of high levels of chlorides. For enclosed basins, the use of chloride deicers shall be limited in the upland areas of the basin. Trash shall be removed as quickly as possible once observed.
6.	Record of Maintenance	The operation and maintenance plan shall remain onsite and be available for inspection when requested by WDNR. When requested, the owner shall make available for inspection all maintenance records to the department or agent for the life of the system.