

To: City of Watertown Engineering Department  
 From: Aaron Koch  
 Date: April, 10 2024  
 Subject: Consolidated Industrial Addition Stormwater Memo

For the purposes of notification to the City of Watertown, the following is a summary explaining the stormwater runoff for the proposed Consolidated Industrial Addition located at 1207 Boomer St. in Watertown, WI. The proposed site is tributary to the Rock River and the site improvements consist almost entirely of building roof. The total size of the proposed development is 1.5 acres and will require stormwater BMPs as required by the DNR and City of Watertown.

The City of Watertown requires 60% TSS removal rate for the proposed parking and road areas for redevelopment. The proposed addition will not add additional parking or roadway to the existing site. The proposed building addition is 59,290 S.F in size with the remaining disturbed areas being restored as green space.

The City also requires that peak flow from the 1-yr 24-hr storm in post-development conditions does not exceed the peak flow from the 1-yr 24-hr storm in pre-development conditions and that the peak flow from the 100-yr 24-hr storm in post-development conditions does not exceed the peak flow from the 2-yr 24-hr storm in pre-development conditions. These requirements will be met through the use of a blue roof system utilizing 6 Accutrol Weir Flow controlled flow roof drains each with 2 notches.

### Pre-Development Peak Flows

Drainage Area	Area (ac)	CN	Tc (min)	Peak Flows		
				1-year (cfs)	2-year (cfs)	100-year (cfs)
Undeveloped Offsite	1.50	71	6.0*	<b>1.09</b>	<b>1.54</b>	8.21

### Post-Development Peak Flows

Drainage Area	Area (ac)	CN	Tc (min)	Peak Flows		
				1-year (cfs)	2-year (cfs)	100-year (cfs)
Proposed Roof	1.36	98	6.0*	4.49	5.08	11.69
Disturbed Green Area	0.14	71	6.0*	0.10	0.14	0.74
Controlled Flow Watts	---	---	---	0.74	0.75	1.02
Proposed Site Discharge	1.5	---	---	<b>0.83</b>	<b>0.88</b>	<b>1.53</b>

\* A Tc of 6.0min is used since the actual computed Tc is less than the minimum allowed by TR-55

**Comparison of Proposed to Allowable Release Rates**

	<b>1-year Release Rate (cfs)</b>	<b>100-year Release Rate (cfs)</b>
Allowable	1.09	1.54
Proposed	0.83	1.53

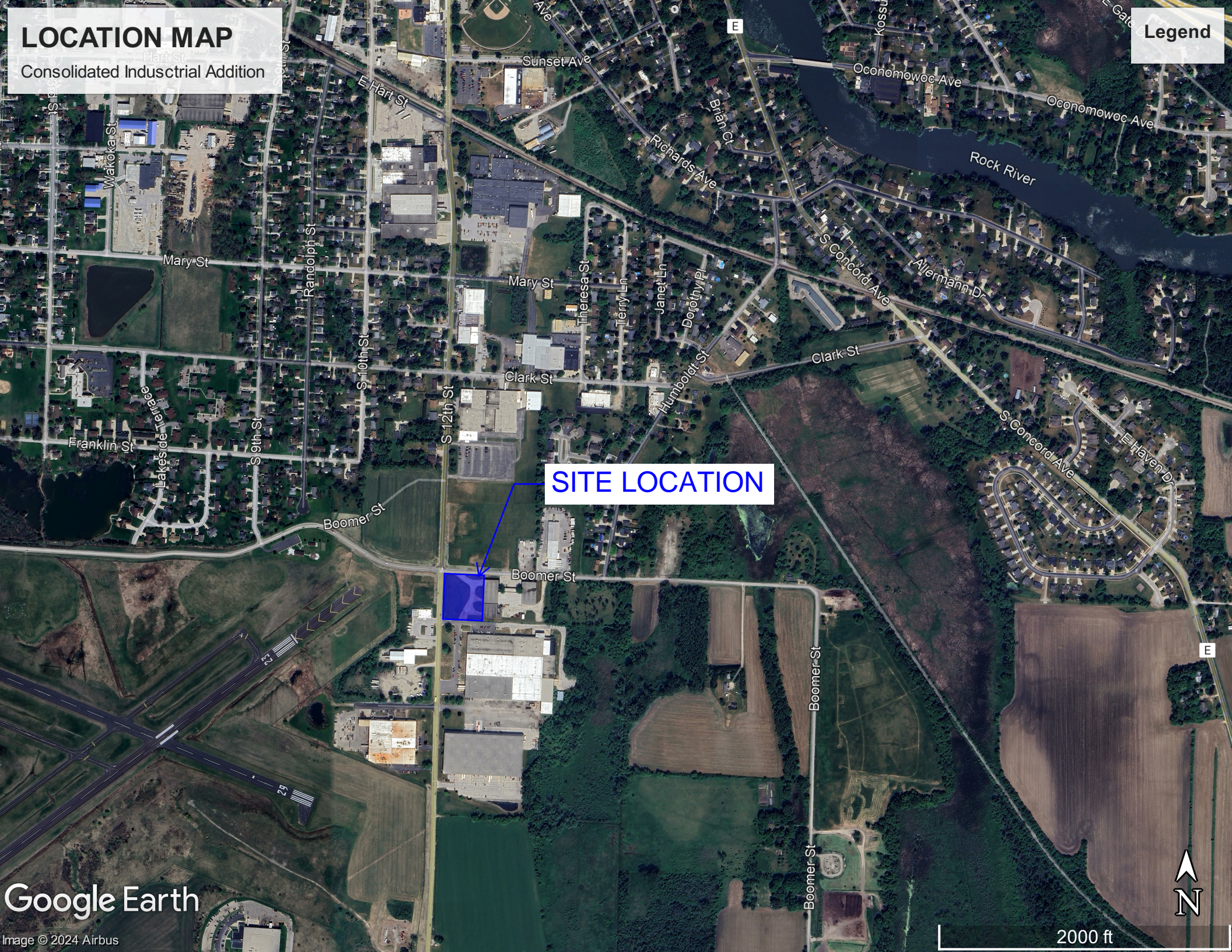
As shown in the tables above, proposed stormwater quantity BMPs will reduce the peak flow from the post-development conditions to meet pre-development conditions per the City of Watertown requirements. The HydroCAD modeling data and the Proposed Hydrology Exhibit have been attached to this memo.



# LOCATION MAP

Consolidated Industrial Addition

Legend



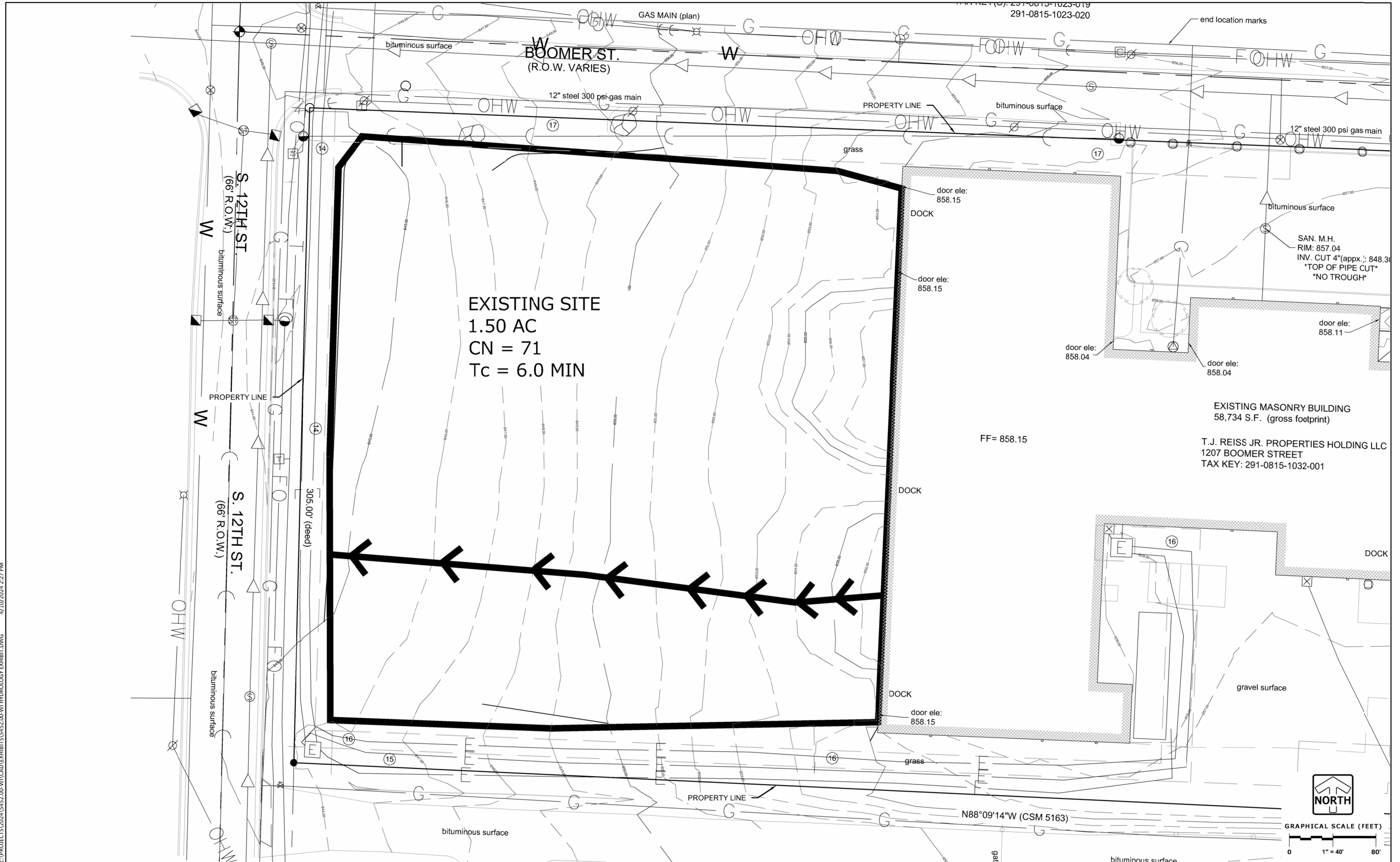
Google Earth

Image © 2024 Airbus

2000 ft



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# EXISTING HYDROLOGY - CONSOLIDATED INDUSTRIAL

PINNACLE ENGINEERING GROUP

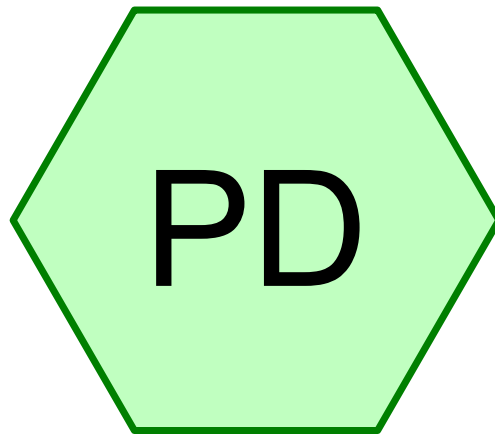
20725 WATERTOWN ROAD | SUITE 100 | BROOKFIELD, WI 53186 | WWW.PINNACLE-ENGR.COM |

PLAN | DESIGN | DELIVER

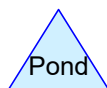
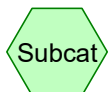
PEG JOB# 5452-WI

4-10-24





# Pre-Development



## Routing Diagram for SITE

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**SITE**

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Type II 24-hr 1-YR Rainfall=2.42"

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Page 2

**Summary for Subcatchment PD: Pre-Development**

Runoff = 1.09 cfs @ 11.99 hrs, Volume= 0.056 af, Depth= 0.45"

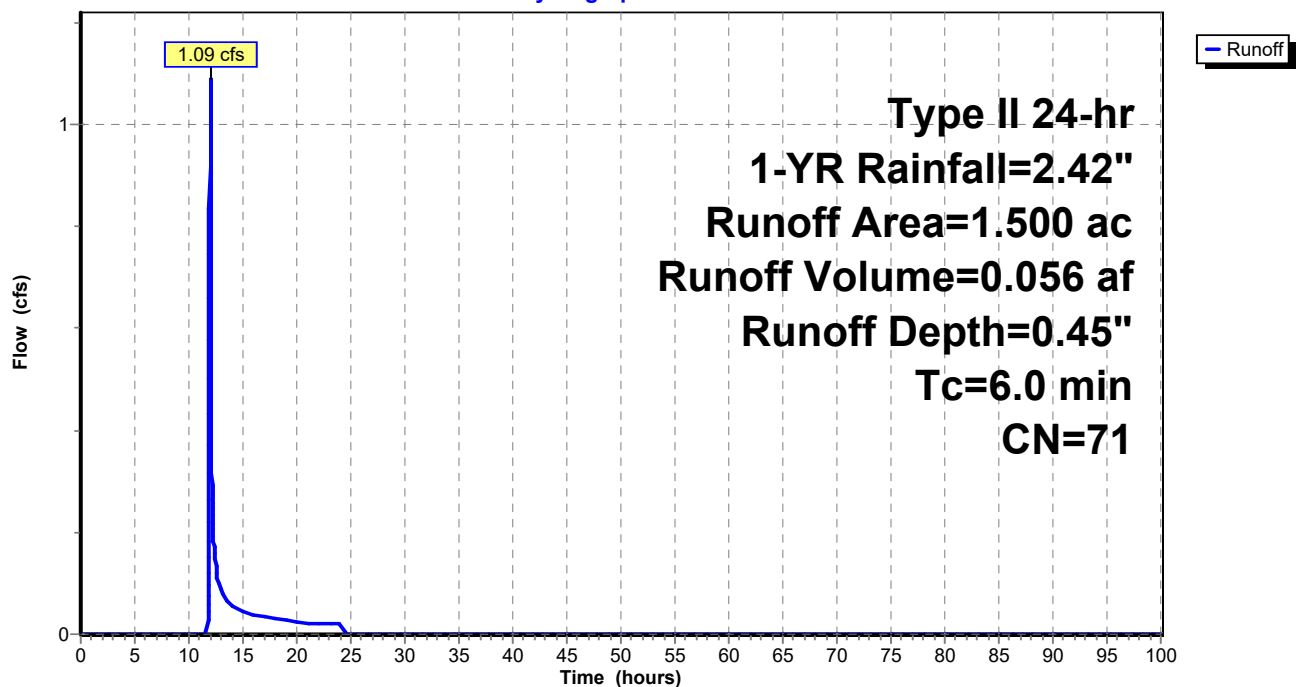
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-100.00 hrs, dt= 0.01 hrs  
Type II 24-hr 1-YR Rainfall=2.42"

Area (ac)	CN	Description
* 1.500	71	Grassland, HSG C
1.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Assumed

**Subcatchment PD: Pre-Development**

Hydrograph





**SITE**

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Type II 24-hr 2-YR Rainfall=2.73"

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**Summary for Subcatchment PD: Pre-Development**

Runoff = 1.54 cfs @ 11.99 hrs, Volume= 0.076 af, Depth= 0.61"

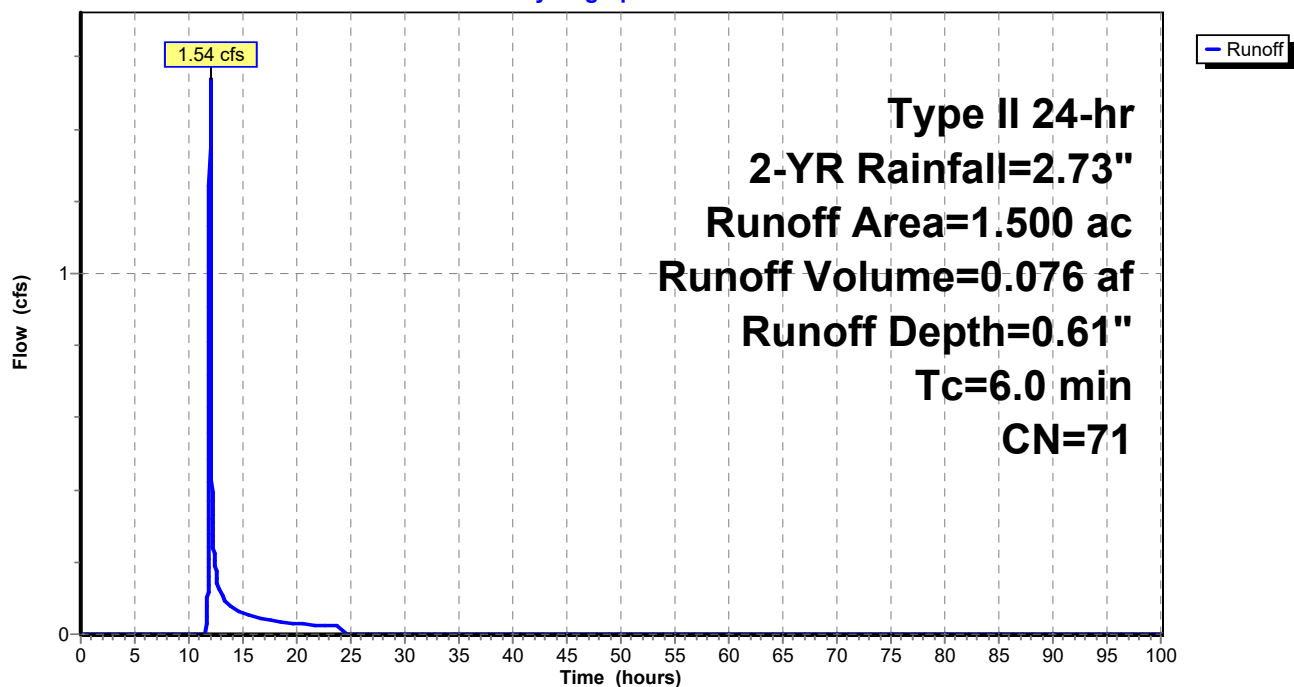
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-100.00 hrs, dt= 0.01 hrs  
Type II 24-hr 2-YR Rainfall=2.73"

Area (ac)	CN	Description
* 1.500	71	Grassland, HSG C
1.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Assumed

**Subcatchment PD: Pre-Development**

Hydrograph



**SITE**

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Type II 24-hr 100-YR Rainfall=6.20"

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Page 4

**Summary for Subcatchment PD: Pre-Development**

Runoff = 8.21 cfs @ 11.97 hrs, Volume= 0.383 af, Depth= 3.06"

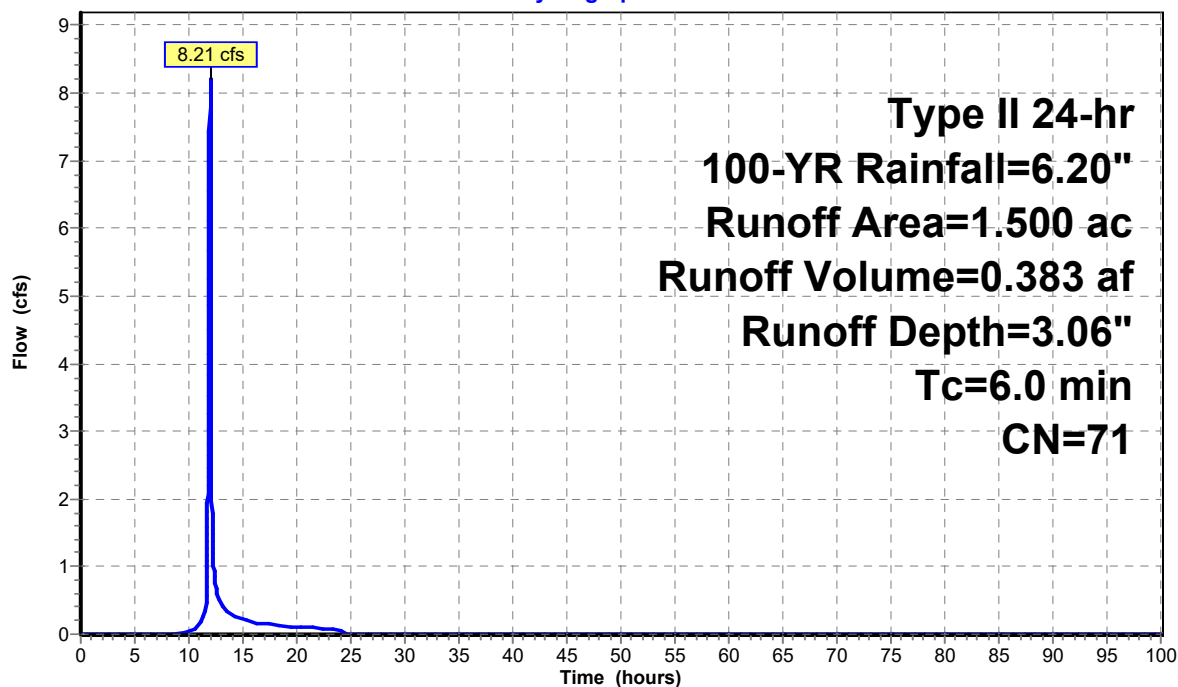
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-100.00 hrs, dt= 0.01 hrs  
Type II 24-hr 100-YR Rainfall=6.20"

Area (ac)	CN	Description
* 1.500	71	Grassland, HSG C
1.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Assumed

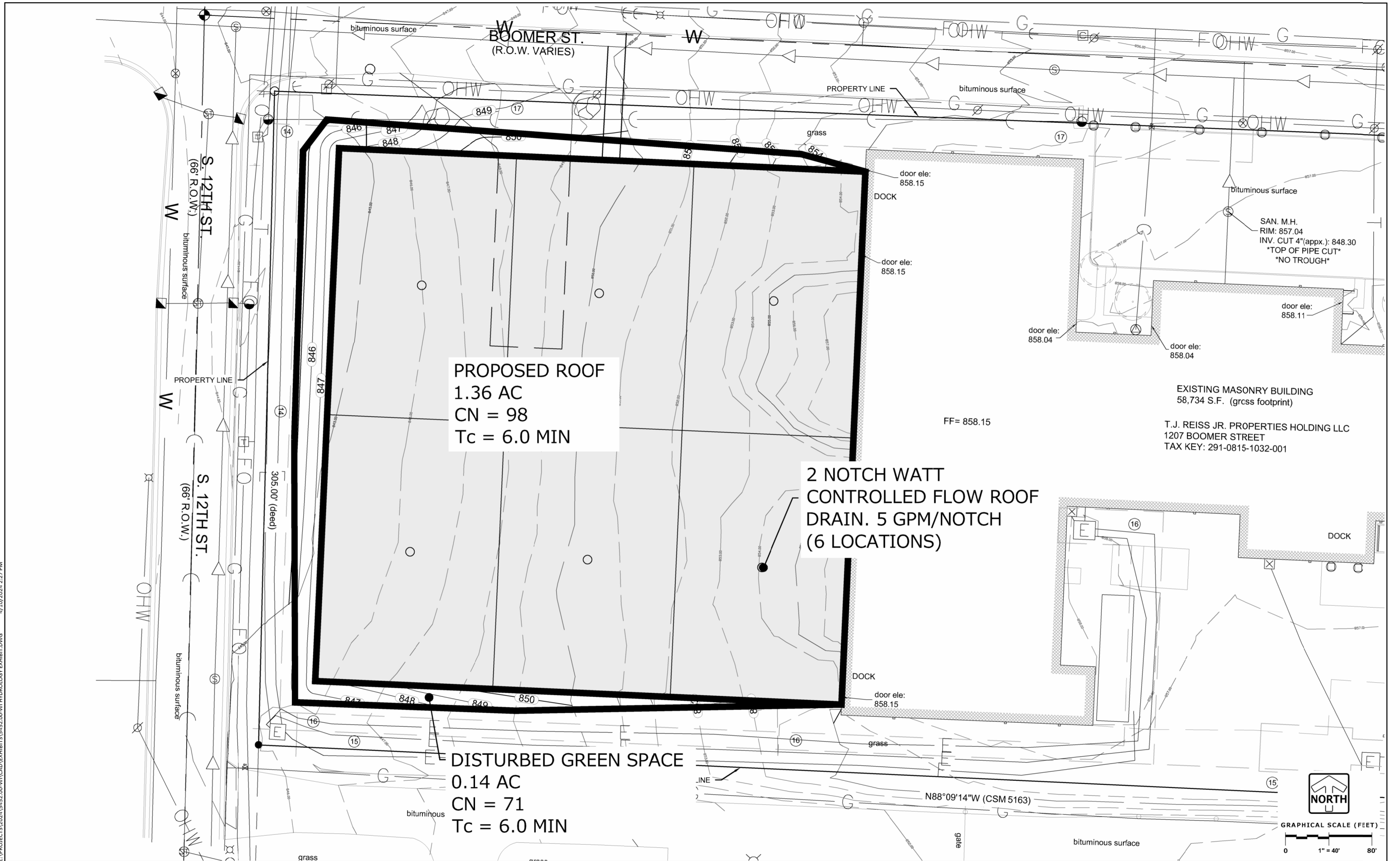
**Subcatchment PD: Pre-Development**

Hydrograph





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# PROPOSED HYDROLOGY - CONSOLIDATED INDUSTRIAL

PINNACLE ENGINEERING GROUP

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PLAN | DESIGN | DELIVER

PEG JOB# 5452-WI

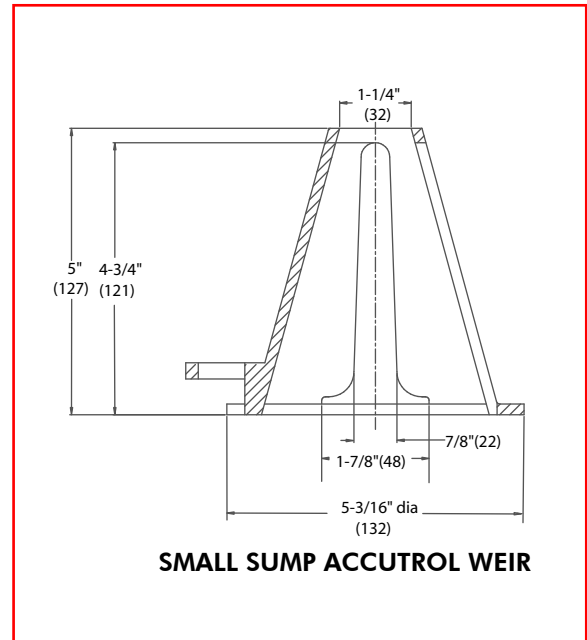
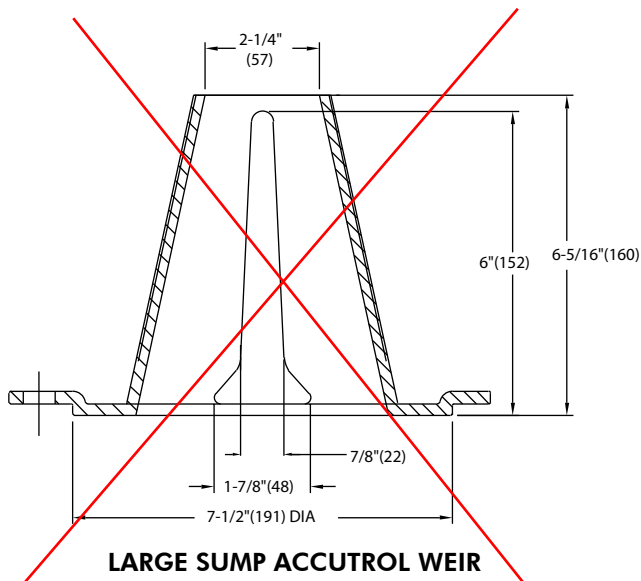
4-10-24

### ACCUTROL WEIR FLOW CONTROL

**SPECIFICATION:** Watts Drainage Products epoxy coated cast iron Accutrol Weir is designed with parabolic openings which limit the flow of rain water off a roof. Each weir slot controls flow to 5 gpm per inch of head to a maximum of 30 gpm at 6" head (for large sump), 25 gpm at 5" head (for small sump). The Accutrol Weir is secured to the flashing clamp of the roof drain. The Accutrol Weir is available with 1 to 4 slots for the large sump drain and up to 3 slots for the small sump drain.

**For Large Sump Roof Drains Specify the "-A" option and number of slots required. (ie. "RD-100-A2" for two slot weir)**

**For Small Sump Roof Drains Specify the "-A" option and number of slots required. (ie. "RD-200-A1" for one slot weir)**



1 GMP = 448.8 CFS

1" HEAD = 5 GPM = 0.011 CFS

4-3/4" HEAD = 18.75 GPM = 0.042 CFS

**\*ABOVE 5" HEAD 1-1/4"-DIA ORIFICE AT TOP OF STRUCTURE IS USED**

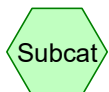
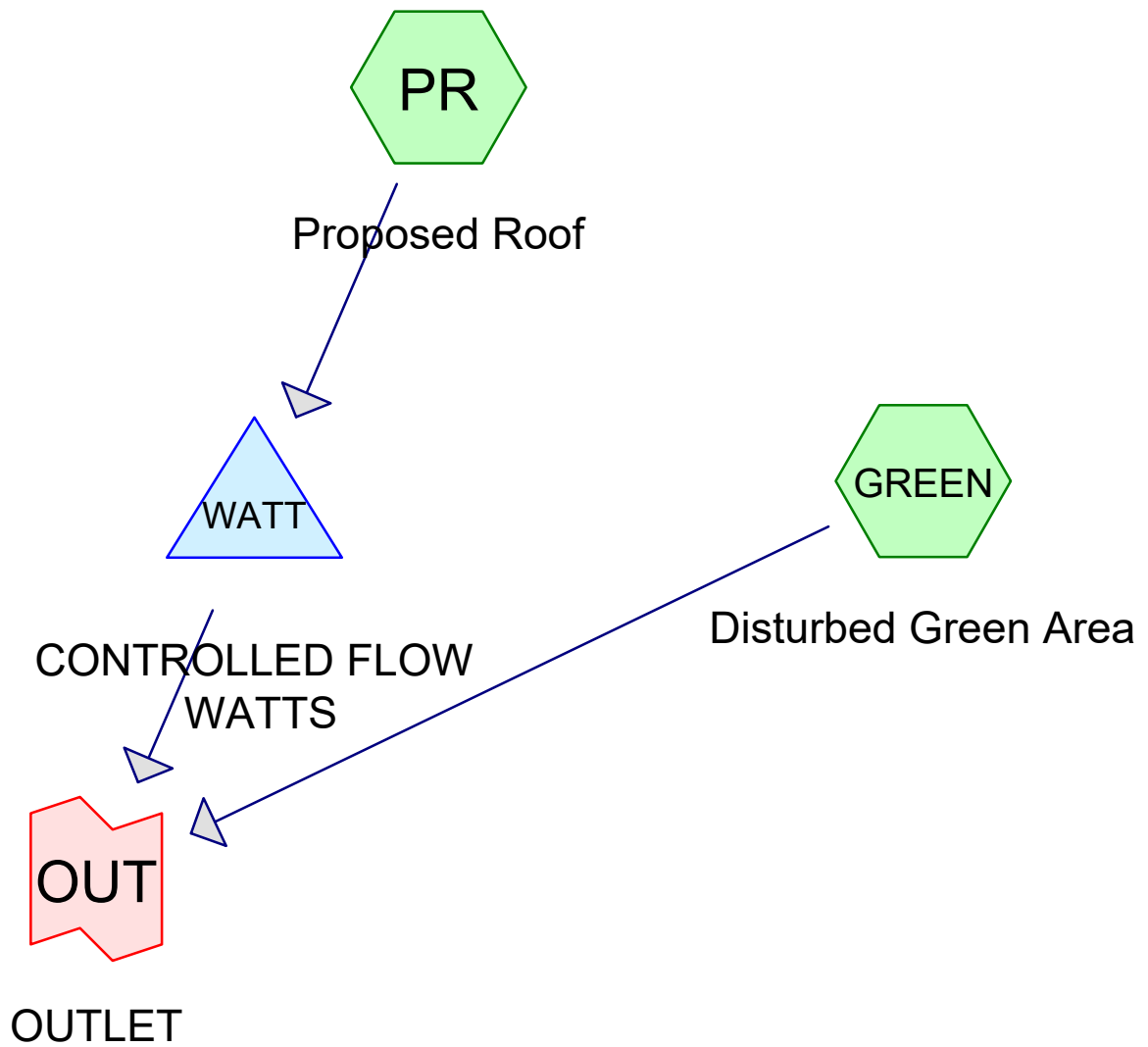
Job Name \_\_\_\_\_ Contractor \_\_\_\_\_

Job Location \_\_\_\_\_ Contractor's P.O. No. \_\_\_\_\_

Engineer \_\_\_\_\_ Representative \_\_\_\_\_

WATTS Drainage reserves the right to modify or change product design or construction without prior notice and without incurring any obligation to make similar changes and modifications to products previously or subsequently sold. See your WATTS Drainage representative for any clarification. Dimensions are subject to manufacturing tolerances.

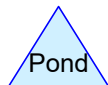




Subcat



Reach



Pond



Link

#### Routing Diagram for SITE

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Type II 24-hr 1-YR Rainfall=2.42"

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Page 2

**Summary for Subcatchment GREEN: Disturbed Green Area**

Runoff = 0.10 cfs @ 11.99 hrs, Volume= 0.005 af, Depth= 0.45"

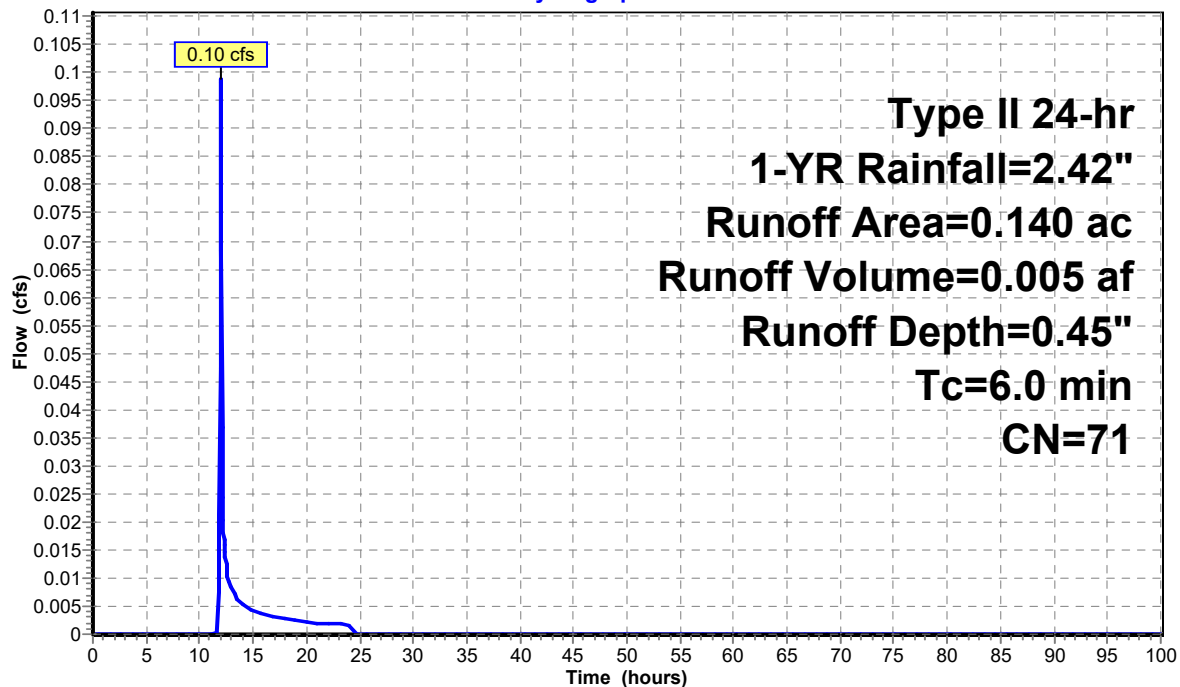
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs  
Type II 24-hr 1-YR Rainfall=2.42"

Area (ac)	CN	Description
* 0.140	71	Grassland, HSG C
0.140		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Assumed

**Subcatchment GREEN: Disturbed Green Area**

Hydrograph





**SITE**

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Type II 24-hr 1-YR Rainfall=2.42"

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Page 3

**Summary for Subcatchment PR: Proposed Roof**

Runoff = 4.49 cfs @ 11.96 hrs, Volume= 0.248 af, Depth= 2.19"

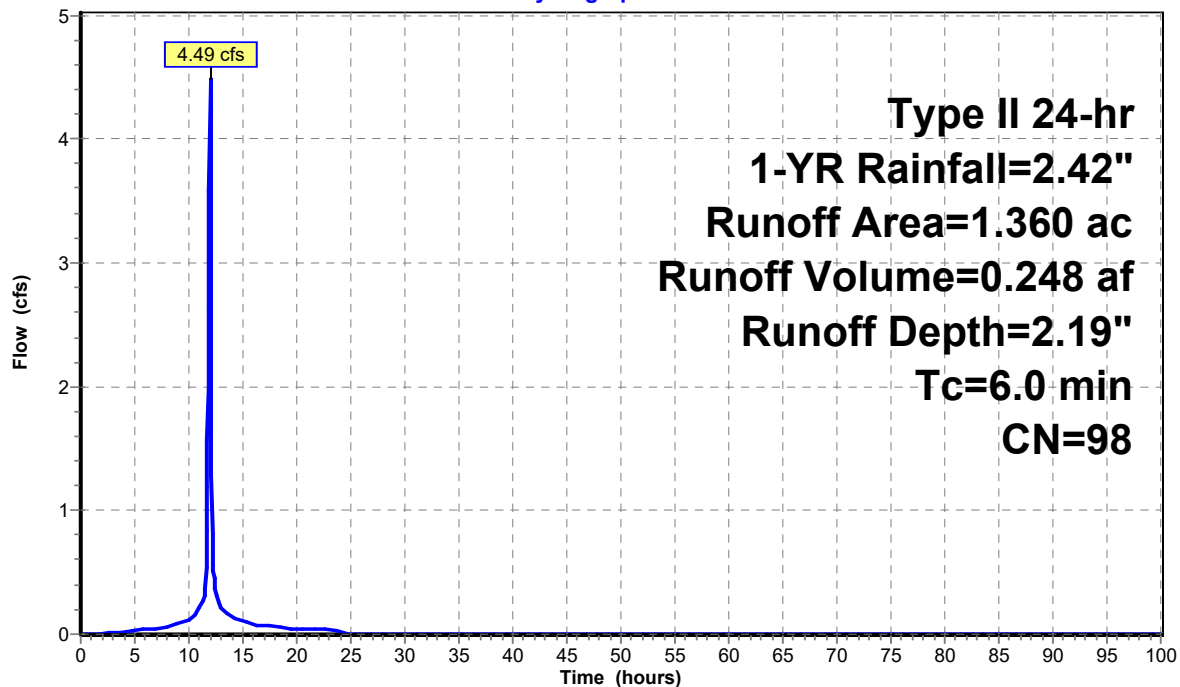
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs  
Type II 24-hr 1-YR Rainfall=2.42"

Area (ac)	CN	Description
* 1.360	98	Building Addition
1.360		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Assumed

**Subcatchment PR: Proposed Roof**

Hydrograph



**SITE**

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Type II 24-hr 1-YR Rainfall=2.42"

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Page 4

**Summary for Pond WATT: CONTROLLED FLOW WATTS**

Inflow Area = 1.360 ac, 100.00% Impervious, Inflow Depth = 2.19" for 1-YR event  
 Inflow = 4.49 cfs @ 11.96 hrs, Volume= 0.248 af  
 Outflow = 0.74 cfs @ 12.17 hrs, Volume= 0.248 af, Atten= 83%, Lag= 12.5 min  
 Primary = 0.74 cfs @ 12.17 hrs, Volume= 0.248 af

Routing by Stor-Ind method, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs  
 Peak Elev= 100.56' @ 12.17 hrs Surf.Area= 18,699 sf Storage= 3,480 cf

Plug-Flow detention time= 28.7 min calculated for 0.248 af (100% of inflow)  
 Center-of-Mass det. time= 28.7 min ( 787.6 - 758.9 )

Volume	Invert	Avail.Storage	Storage Description
#1	100.00'	20,000 cf	<b>1.00'H Prismatic Z=50.0 x 6</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	100.00'	<b>6.0" Horiz. Orifice/Grate X 6.00</b> C= 0.600 Limited to weir flow at low heads
#2	Device 1	100.00'	<b>2 - 5GPM NOTCH PER FIXTURE X 6.00</b> Head (feet) 0.00 0.08 0.42 Disch. (cfs) 0.000 0.022 0.110
#3	Device 1	100.42'	<b>1.2" Horiz. Orifice/Grate X 6.00</b> C= 0.600 Limited to weir flow at low heads
#4	Primary	100.80'	<b>6.0" Horiz. OVERFLOW X 6.00</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.74 cfs @ 12.17 hrs HW=100.56' (Free Discharge)

1=Orifice/Grate (Passes 0.74 cfs of 4.24 cfs potential flow)  
 2=2 - 5GPM NOTCH PER FIXTURE (Custom Controls 0.66 cfs)  
 3=Orifice/Grate (Orifice Controls 0.08 cfs @ 1.79 fps)  
 4=OVERFLOW ( Controls 0.00 cfs)

## SITE

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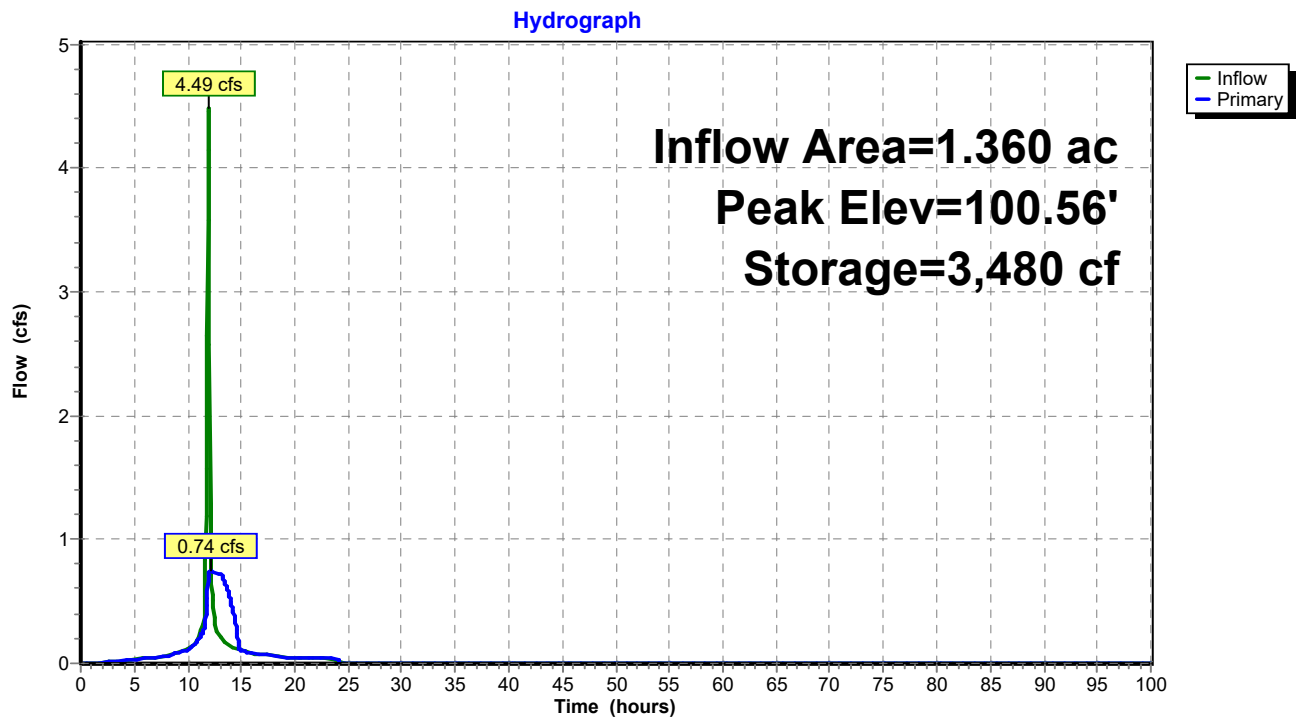
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Type II 24-hr 1-YR Rainfall=2.42"

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Page 5

### Pond WATT: CONTROLLED FLOW WATTS





## SITE

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Type II 24-hr 1-YR Rainfall=2.42"

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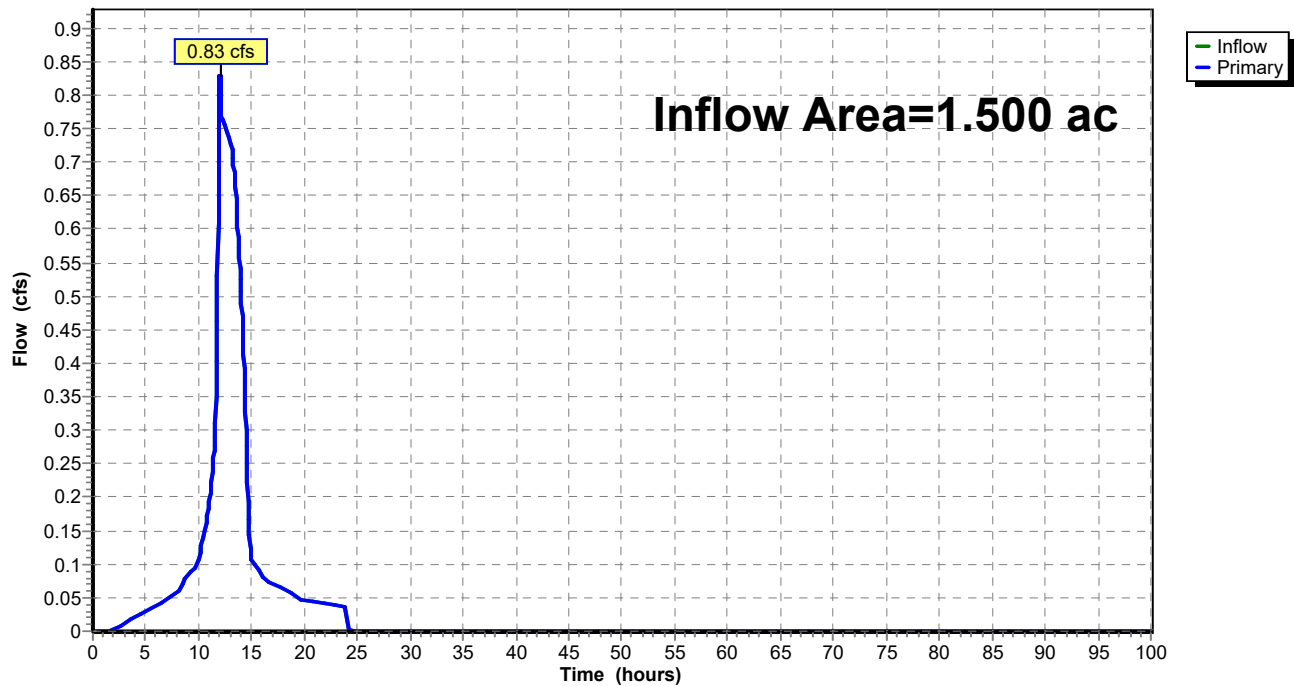
### Summary for Link OUT: OUTLET

Inflow Area = 1.500 ac, 90.67% Impervious, Inflow Depth = 2.03" for 1-YR event  
Inflow = 0.83 cfs @ 12.01 hrs, Volume= 0.254 af  
Primary = 0.83 cfs @ 12.01 hrs, Volume= 0.254 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs

### Link OUT: OUTLET

Hydrograph



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Type II 24-hr 2-YR Rainfall=2.73"

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**Summary for Subcatchment GREEN: Disturbed Green Area**

Runoff = 0.14 cfs @ 11.99 hrs, Volume= 0.007 af, Depth= 0.61"

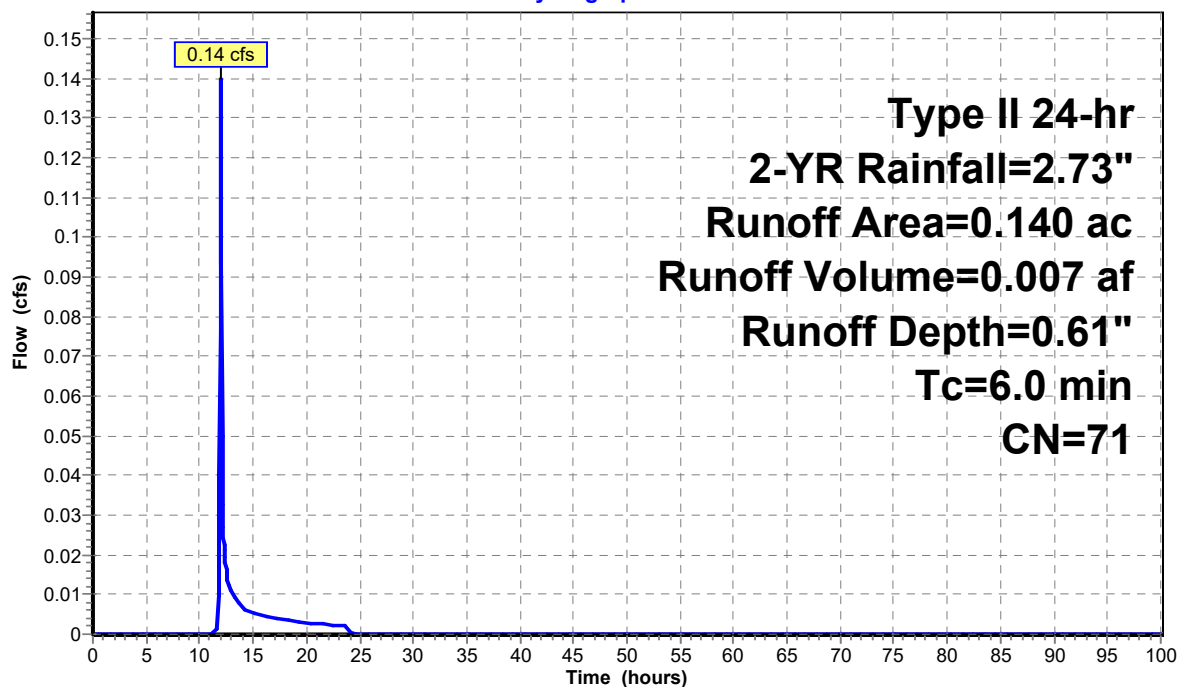
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs  
Type II 24-hr 2-YR Rainfall=2.73"

Area (ac)	CN	Description
* 0.140	71	Grassland, HSG C
0.140		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Assumed

**Subcatchment GREEN: Disturbed Green Area**

Hydrograph



**SITE**

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Type II 24-hr 2-YR Rainfall=2.73"

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Page 8

**Summary for Subcatchment PR: Proposed Roof**

Runoff = 5.08 cfs @ 11.96 hrs, Volume= 0.283 af, Depth= 2.50"

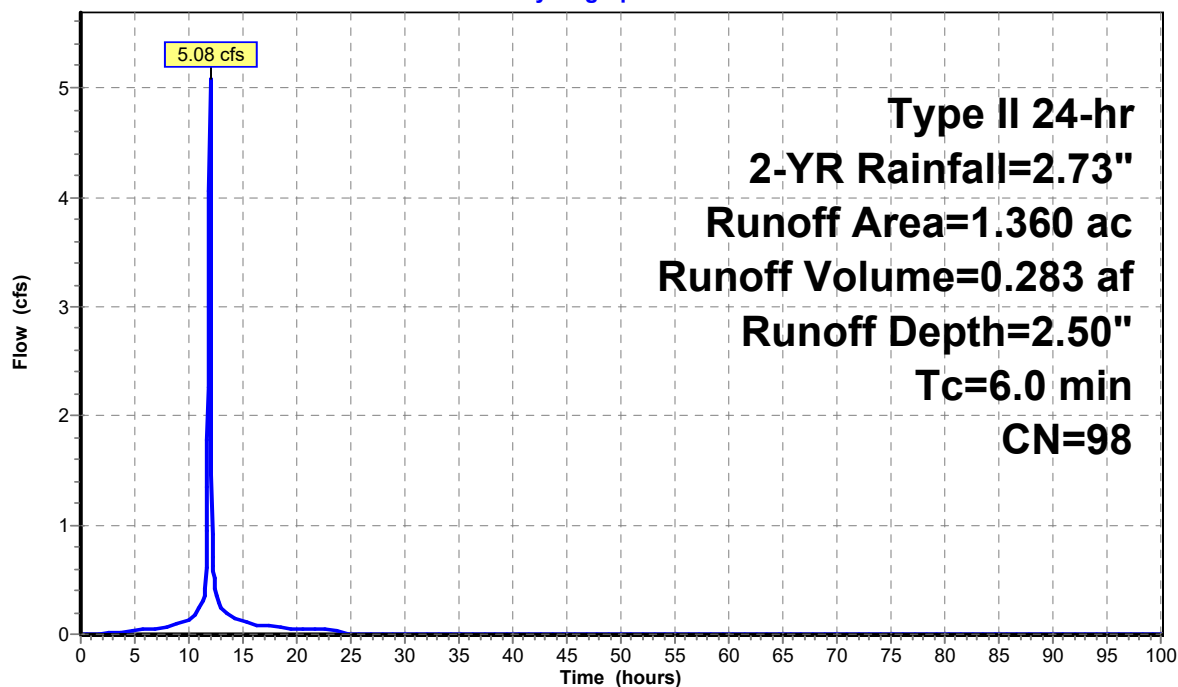
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs  
Type II 24-hr 2-YR Rainfall=2.73"

Area (ac)	CN	Description
* 1.360	98	Building Addition
1.360		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Assumed

**Subcatchment PR: Proposed Roof**

Hydrograph





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Type II 24-hr 2-YR Rainfall=2.73"

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**Summary for Pond WATT: CONTROLLED FLOW WATTS**

Inflow Area = 1.360 ac, 100.00% Impervious, Inflow Depth = 2.50" for 2-YR event  
 Inflow = 5.08 cfs @ 11.96 hrs, Volume= 0.283 af  
 Outflow = 0.75 cfs @ 12.20 hrs, Volume= 0.283 af, Atten= 85%, Lag= 14.0 min  
 Primary = 0.75 cfs @ 12.20 hrs, Volume= 0.283 af

Routing by Stor-Ind method, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs  
 Peak Elev= 100.59' @ 12.20 hrs Surf.Area= 20,829 sf Storage= 4,091 cf

Plug-Flow detention time= 33.8 min calculated for 0.283 af (100% of inflow)  
 Center-of-Mass det. time= 33.8 min ( 789.8 - 756.0 )

Volume	Invert	Avail.Storage	Storage Description
#1	100.00'	20,000 cf	<b>1.00'H Prismatic Z=50.0 x 6</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	100.00'	<b>6.0" Horiz. Orifice/Grate X 6.00</b> C= 0.600 Limited to weir flow at low heads
#2	Device 1	100.00'	<b>2 - 5GPM NOTCH PER FIXTURE X 6.00</b> Head (feet) 0.00 0.08 0.42 Disch. (cfs) 0.000 0.022 0.110
#3	Device 1	100.42'	<b>1.2" Horiz. Orifice/Grate X 6.00</b> C= 0.600 Limited to weir flow at low heads
#4	Primary	100.80'	<b>6.0" Horiz. OVERFLOW X 6.00</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=0.75 cfs @ 12.20 hrs HW=100.59' (Free Discharge)

1=Orifice/Grate (Passes 0.75 cfs of 4.35 cfs potential flow)  
 2=2 - 5GPM NOTCH PER FIXTURE (Custom Controls 0.66 cfs)  
 3=Orifice/Grate (Orifice Controls 0.09 cfs @ 1.98 fps)  
 4=OVERFLOW ( Controls 0.00 cfs)

## SITE

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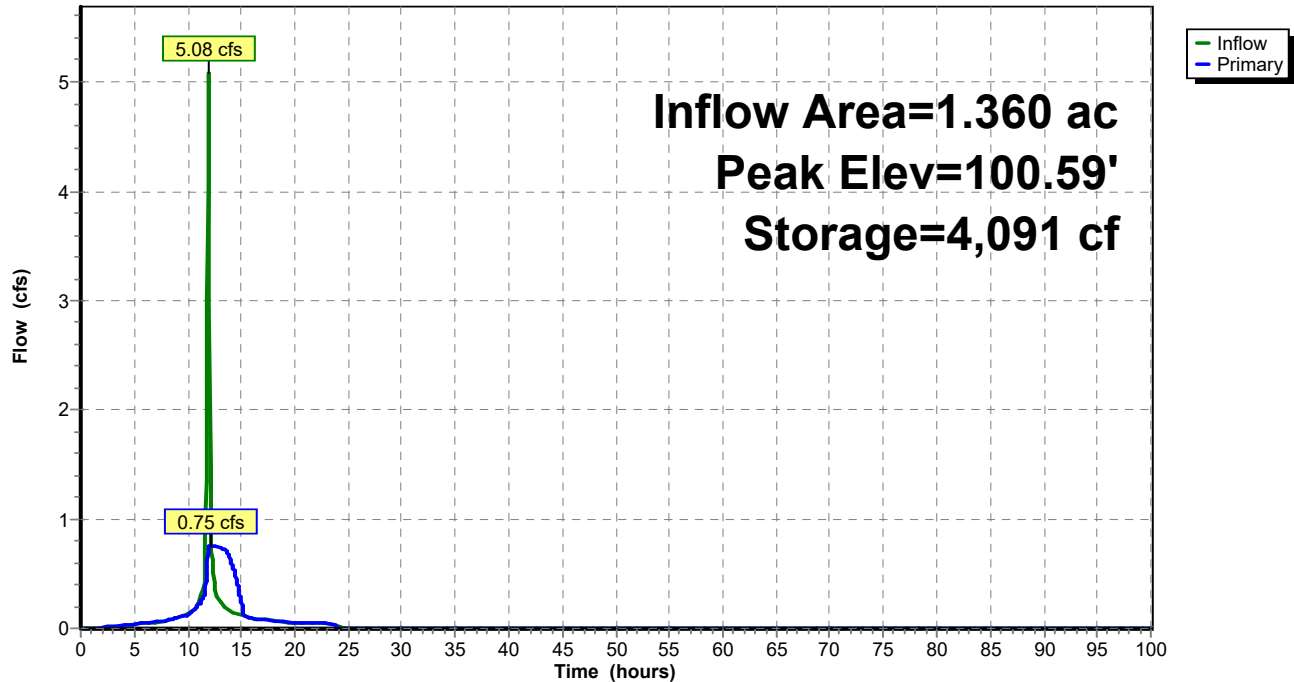
Type II 24-hr 2-YR Rainfall=2.73"

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### Pond WATT: CONTROLLED FLOW WATTS

Hydrograph



## SITE

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Type II 24-hr 2-YR Rainfall=2.73"

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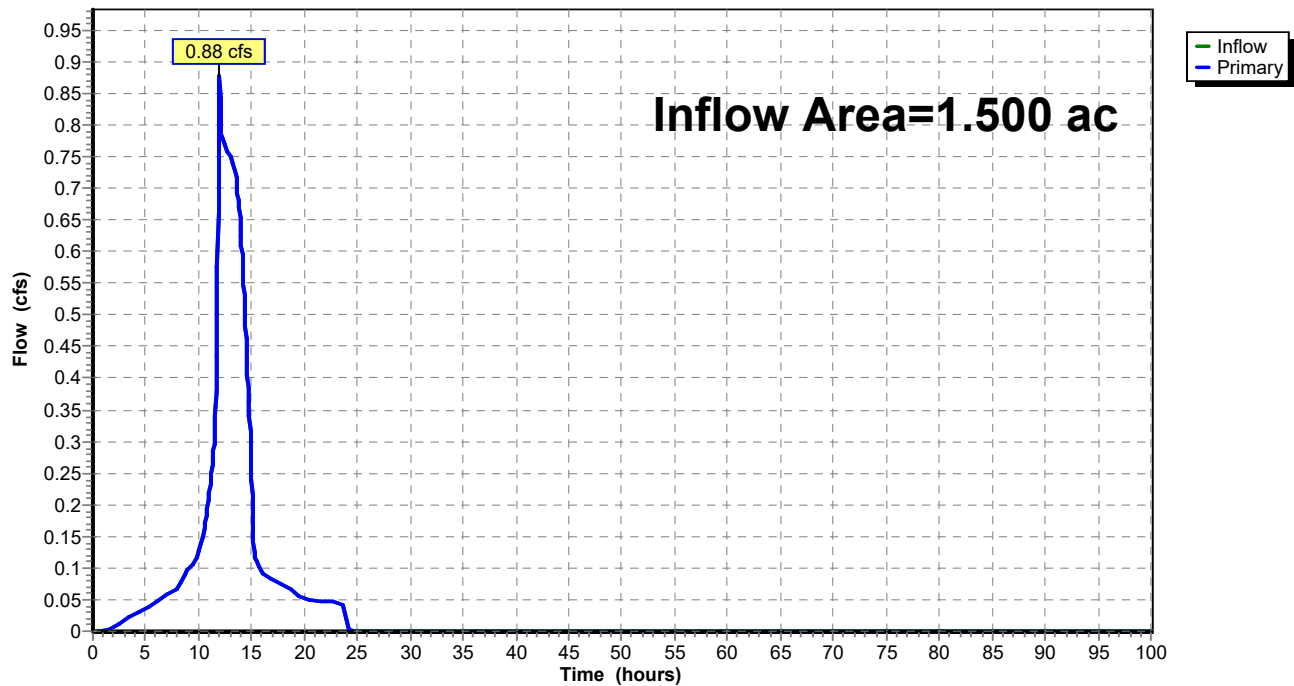
### Summary for Link OUT: OUTLET

Inflow Area = 1.500 ac, 90.67% Impervious, Inflow Depth = 2.32" for 2-YR event  
Inflow = 0.88 cfs @ 12.00 hrs, Volume= 0.290 af  
Primary = 0.88 cfs @ 12.00 hrs, Volume= 0.290 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs

### Link OUT: OUTLET

#### Hydrograph



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Type II 24-hr 100-YR Rainfall=6.20"

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**Summary for Subcatchment GREEN: Disturbed Green Area**

Runoff = 0.74 cfs @ 11.97 hrs, Volume= 0.036 af, Depth= 3.06"

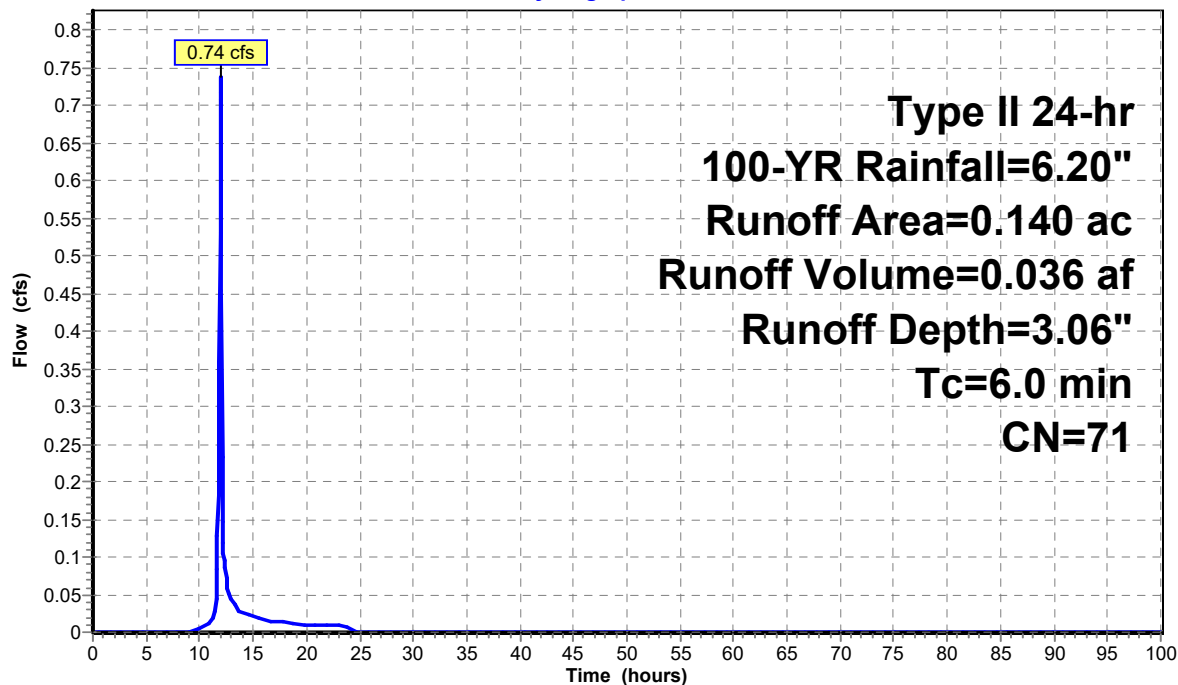
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs  
Type II 24-hr 100-YR Rainfall=6.20"

Area (ac)	CN	Description
* 0.140	71	Grassland, HSG C
0.140		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Assumed

**Subcatchment GREEN: Disturbed Green Area**

Hydrograph





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Type II 24-hr 100-YR Rainfall=6.20"

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**Summary for Subcatchment PR: Proposed Roof**

Runoff = 11.69 cfs @ 11.96 hrs, Volume= 0.676 af, Depth= 5.96"

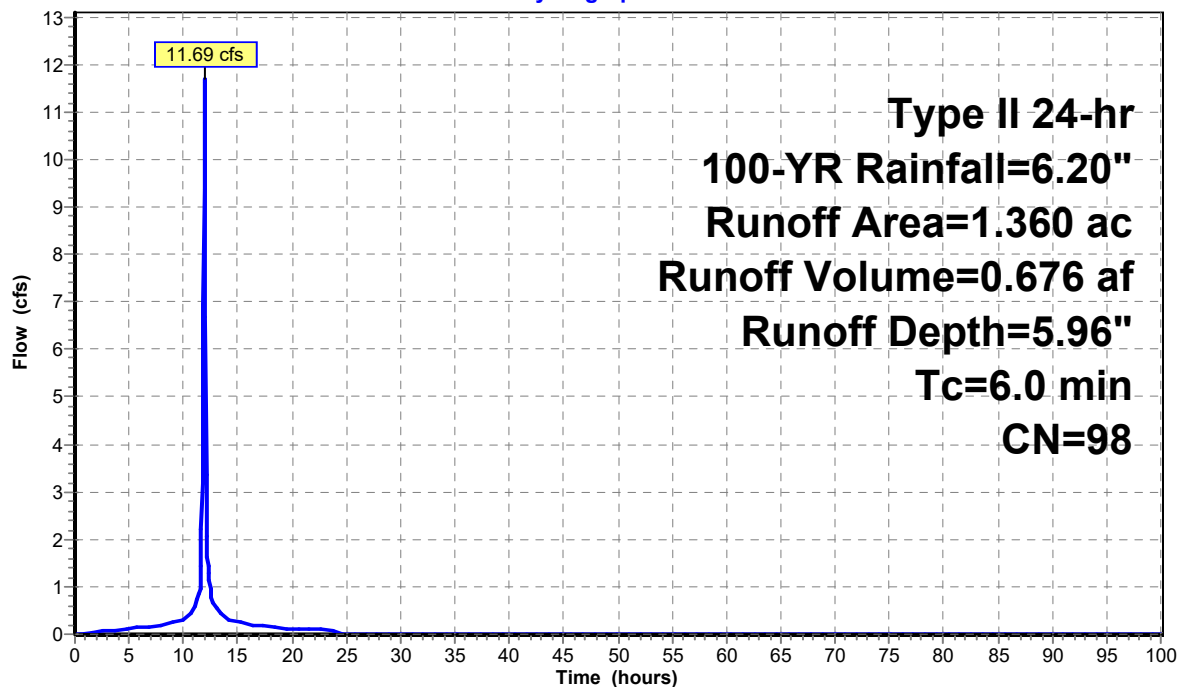
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs  
Type II 24-hr 100-YR Rainfall=6.20"

Area (ac)	CN	Description
* 1.360	98	Building Addition
1.360		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Assumed

**Subcatchment PR: Proposed Roof**

Hydrograph



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Type II 24-hr 100-YR Rainfall=6.20"

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**Summary for Pond WATT: CONTROLLED FLOW WATTS**

Inflow Area = 1.360 ac, 100.00% Impervious, Inflow Depth = 5.96" for 100-YR event  
 Inflow = 11.69 cfs @ 11.96 hrs, Volume= 0.676 af  
 Outflow = 1.02 cfs @ 12.45 hrs, Volume= 0.676 af, Atten= 91%, Lag= 29.1 min  
 Primary = 1.02 cfs @ 12.45 hrs, Volume= 0.676 af

Routing by Stor-Ind method, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs  
 Peak Elev= 100.84' @ 12.45 hrs Surf.Area= 41,984 sf Storage= 11,706 cf

Plug-Flow detention time= 99.2 min calculated for 0.675 af (100% of inflow)  
 Center-of-Mass det. time= 99.2 min ( 839.7 - 740.5 )

Volume	Invert	Avail.Storage	Storage Description
#1	100.00'	20,000 cf	<b>1.00'H Prismatic Z=50.0 x 6</b>

Device	Routing	Invert	Outlet Devices
#1	Primary	100.00'	<b>6.0" Horiz. Orifice/Grate X 6.00</b> C= 0.600 Limited to weir flow at low heads
#2	Device 1	100.00'	<b>2 - 5GPM NOTCH PER FIXTURE X 6.00</b> Head (feet) 0.00 0.08 0.42 Disch. (cfs) 0.000 0.022 0.110
#3	Device 1	100.42'	<b>1.2" Horiz. Orifice/Grate X 6.00</b> C= 0.600 Limited to weir flow at low heads
#4	Primary	100.80'	<b>6.0" Horiz. OVERFLOW X 6.00</b> C= 0.600 Limited to weir flow at low heads

**Primary OutFlow** Max=1.02 cfs @ 12.45 hrs HW=100.84' (Free Discharge)

1=Orifice/Grate (Passes 0.81 cfs of 5.19 cfs potential flow)  
 2=2 - 5GPM NOTCH PER FIXTURE (Custom Controls 0.66 cfs)  
 3=Orifice/Grate (Orifice Controls 0.15 cfs @ 3.11 fps)  
 4=OVERFLOW (Weir Controls 0.21 cfs @ 0.62 fps)

# SITE

Prepared by Pinnacle Engineering Group

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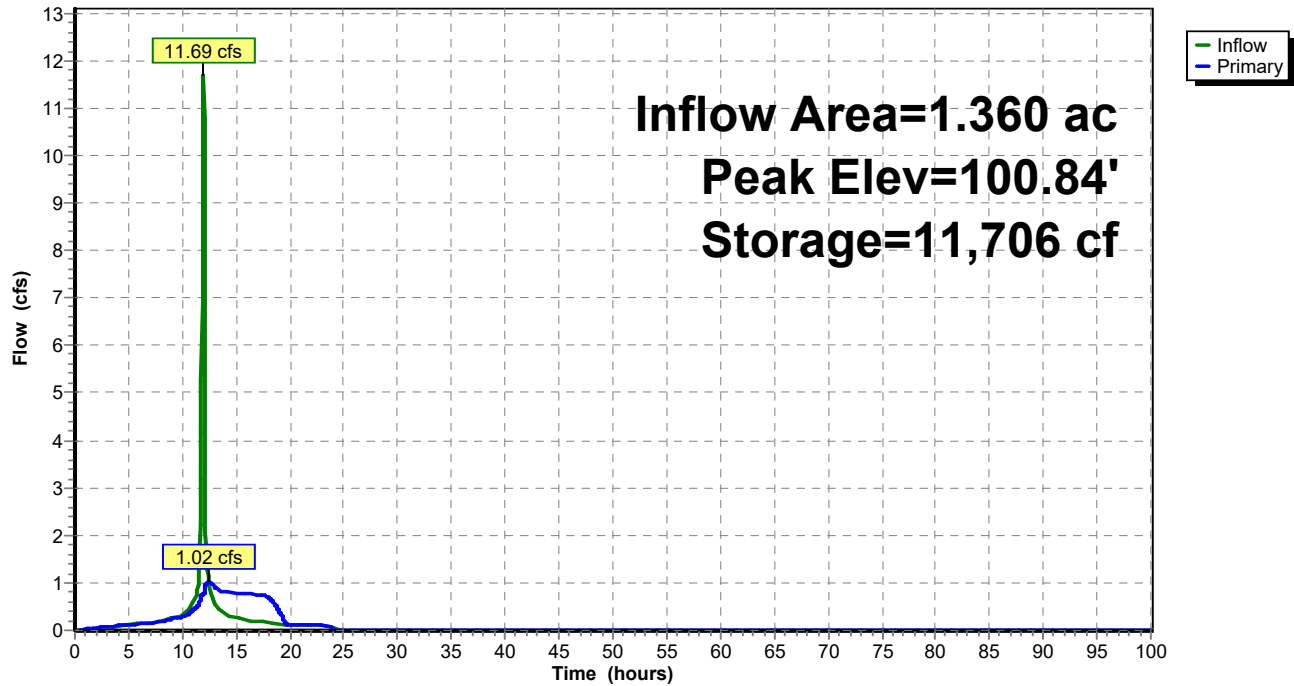
Type II 24-hr 100-YR Rainfall=6.20"

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## Pond WATT: CONTROLLED FLOW WATTS

Hydrograph



## SITE

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Type II 24-hr 100-YR Rainfall=6.20"

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### Summary for Link OUT: OUTLET

Inflow Area = 1.500 ac, 90.67% Impervious, Inflow Depth = 5.69" for 100-YR event  
Inflow = 1.53 cfs @ 11.98 hrs, Volume= 0.711 af  
Primary = 1.53 cfs @ 11.98 hrs, Volume= 0.711 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-100.00 hrs, dt= 0.05 hrs

### Link OUT: OUTLET

Hydrograph

