

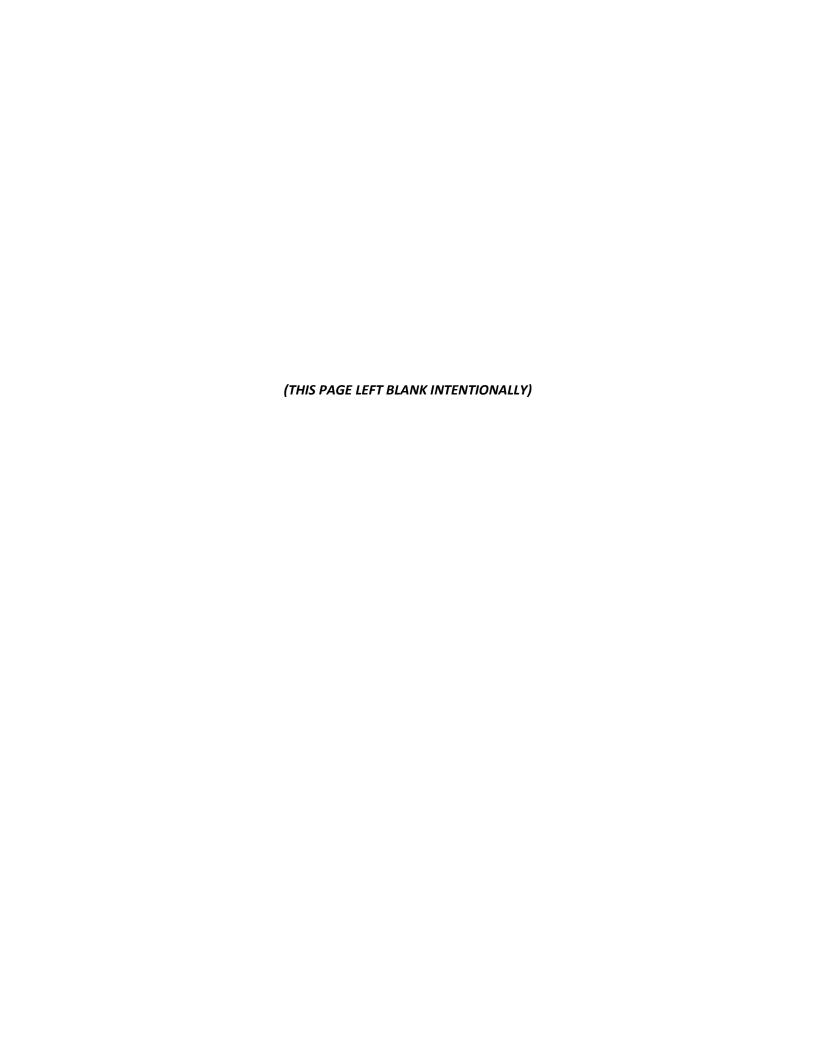
CITY OF SWEET HOME

CONTRACT DOCUMENTS FOR

MAHLER WATER RECLAMATION FACILITY IMPROVEMENTS PROJECT PHASE 1

SUPPLEMENTAL INFORMATION
GEOTECHNICAL DATA REPORT

AUGUST 2022

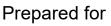




City of Sweet Home Mahler Water Reclamation Facility Improvements

Geotechnical Data Report

Final Submittal





July 2022



This page intentionally left blank.

Table of Contents

1.0 Introd	duction	1
1.1	General	1
1.2	Project Description	1
1.3	Purpose and Scope of Work	3
2.0 Site C	Conditions	4
2.1	Site Description	4
2.2	Regional Geology	4
3.0 Site Ir	nvestigations	5
3.1	General	5
3.2	Exploratory Borings	5
3.3	Piezometers	6
3.4	Borehole Backfill and Abandonment	6
4.0 Labor	ratory Testing	7
5.0 Subsi	urface Conditions	9
5.2	Groundwater	12
6.0 Limita	ations	13
7.0 Refer	ences	14
7.0 Kelek	611065	
List of T	ables	
Table 4-1. L	aboratory Test Results Summary	8
Table 5-1. D	Definition of Rock Strength Descriptions	10
Table 5-2. D	Depth of Fill / Depth to Rock Summary	11
	Groundwater Level Measurements	12
List of F	igures	
Figure 1	Vicinity Map	
Figure 2	Site Plan	
Figure 3A	Geologic Cross Section – Building A	
Figure 3B	Geologic Cross Section – MEB Building	
Figure 3C	Geologic Cross Section – Aeration Basin No. 3	

Figure 3D Geologic Cross Section – Primary Clarifier

Figure 3E Geologic Cross Section – Secondary Clarifier 90 (SC90)

Figure 3F Geologic Cross Section – Influent Pump Station

Appendices

Appendix A Boring Logs

Appendix B Rock Core Photos

Appendix C Laboratory Test Results

Acronyms and Abbreviations

ASTM American Society for Testing and Materials

bgs below ground surface

bpf blows per foot

GDR Geotechnical Data Report

I_{S(50)} point load index

LBVS Little Butte Volcanic Series
MJA McMillen Jacobs Associates

N-value standard penetration test blows to advance final foot

OAR Oregon Administrative Rules
PGA peak ground acceleration
RQD Rock Quality Designation
SPT Standard Penetration Test

USGS United States Geological Survey

West Yost Associates, Inc.

WRF Wastewater Reclamation Facility

Distribution

To: Preston Van Meter, P.E.

West Yost Associates, Inc.

From: Wolfe Lang, P.E., G.E.

McMillen Jacobs Associates

Prepared By: Jeff Quinn, P.E.

McMillen Jacobs Associates

Reviewed By: Wolfe Lang, P.E., G.E.

McMillen Jacobs Associates

Revision Log

Revision No.	Date	Revision Description
Final Submittal	July 22, 2022	Final Submittal to West Yost

1.0 Introduction

1.1 General

McMillen Jacobs Associates (MJA) has been retained by West Yost Associates, Inc. (West Yost) to provide geotechnical engineering services for the City of Sweet Home Mahler Water Reclamation Facility Improvements project. The project owner is the City of Sweet Home. The Project site location is shown in the Vicinity Map, Figure 1. This Geotechnical Data Report (GDR) presents the results of our field explorations and laboratory testing, as well as a discussion on the site geology.

1.2 Project Description

The City of Sweet Home Mahler Water Reclamation Facility (WRF) is located at 1357 Pleasant Valley Road, in Sweet Home, Oregon. The WRF has been in service since 1947 with two major upgrades completed in 1974 and 1994 (Brown & Caldwell, 2016). Our project understanding is based on our communication with West Yost and the 90 percent submittal plans entitled *Mahler Water Reclamation Facility Improvements Project – Phase 1*, dated June 2022, prepared by West Yost. This GDR addresses both the Phase 1 and Phase 2 project improvements. However, Phase 2 of the project is only in the preliminary design stages and several details are still being worked out by West Yost. The proposed Phase 1 and Phase 2 improvements are shown in Figure 2.

1.2.1 Phase 1 WRF Improvements

The proposed Phase 1 WRF improvements will include the following:

- Main Electrical and Blower (MEB) Building: The 22- by 67.5-foot MEB Building will be located within the north-central part of the WRF site. The interior of the building will be supported on a 6-inch-thick slab on grade foundation, while the perimeter of the structure will be supported on a 1.5-foot-wide continuous spread footing. A new generator, which will be installed on the north side of the MEB Building, will be supported on 12- by 28-foot, 12-inch-thick reinforced concrete slab on grade with thickened edges. Site grades will be raised up to approximately 6 feet in this area to facilitate construction of the MEB Building, with approximately 4 to 6 feet of fill placed below the building footprint. The approximately 6-foot-tall fill slope on the east side of the MEB Building will descend at 2H:1V (horizontal:vertical).
- Influent Pump Station (IPS): The new, 40- by 56-foot IPS will be constructed just north of the existing IPS, within the southeast corner of the WRF site. The approximate southern half of the IPS structure, which supports above-grade piping, will be supported on a 12-inch-thick reinforced concrete slab on grade with thickened edges. The approximate northern half of the IPS structure, which houses five submerged pumps, will be supported on a 28-inch-thick reinforced concrete slab with a base elevation of approximately 491.2 feet (e.g., about 26.2 feet below grade).
- Temporary Generator: A temporary electrical generator, supported on an 18-inch-thick reinforced concrete slab, will be installed on the south side of existing Secondary Clarifier SC60.

- Existing Bathroom Relocation: The existing bathroom, currently along the northern fence line and east of the existing entrance gate, will be moved to the north side of the boat ramp access road.
- Retaining Walls: To support new fills placed for site grading, as well as proposed cuts, there are six retaining walls (designated Wall 1 through Wall 6) proposed as part of Phase 1 improvements. The retaining walls will be cast-in-place, reinforced concrete cantilevered walls ranging in height from 2.5 to 6.75 feet.
- Existing Structures/Facilities to Remain In Use: Secondary Clarifier Nos. 1 through 3; RAS/WAS Pump Station; Aeration Basin; Chlorine Contact Chamber; Aerobic Digestor; Administration and Operations Building; Solids Building; Sand Filters; Lime Silo; and the electrical building on the south side of existing SC60.
- Proposed Piping Improvements: 12-inch storm drain; 10-inch storm drain; 8-inch to 36-inch sanitary sewer; 12-inch IPS forcemain to headworks (HDPE pipe); 24-inch RS IPS to Headworks (HDPE pipe); 6-inch PS forcemain; 12-inch RDPS; 6-inch SL-WAS; 6-inch SL-D forcemain; 4-inch PSC; and 10-inch DIP waterline relocation.
- Demolition of the Following Existing Structures: Backwash Storage; Waste Backwash Storage; the approximately 25- by 50-foot storage building on the west side of the WRF site; and several buildings clustered together in the central portion of the WRF site including the carport, an approximately 40- by 40-foot CMU building, an approximately 25- by 50-foot Quonset Hut storage building, and an approximately 15- by 35-foot shed.
- Other Improvements: The existing asphalt pavement and concrete curbs along the main entrance road and within the approximate western half of the site. The approximate western half of the WRF site, an area north of the boat ramp access drive, and a narrow area along the south site boundary will be surfaced with gravel as part of the Phase 1 improvements.

1.2.2 Phase 2 WRF Improvements

Although Phase 2 plans are not available as of the time of report issuance, West Yost provided us with preliminary information, the details of which are provided in the following:

- Primary Clarifier: The new primary clarifier will consist of an approximately 65- by 85-foot reinforced concrete tank structure. The base depth of the new primary clarifier will be approximately 9.5 to 22.5 feet below existing site grades and will be supported on an 18-inch-thick, reinforced concrete mat foundation system. The primary clarifier will be expanded to the north (designated Primary Clarifier 4) in a future project phase.
- Secondary Clarifier 90 (SC90): SC90 will consist of an approximately 95-foot diameter, 17-foot-tall reinforced concrete tank structure. SC90 will be a primarily below-grade structure, with the tank walls being about 2 feet above finish grade. The base depth of the SC90 will be approximately 15 feet below existing site grades and will be supported on a reinforced concrete mat foundation.
- Building A (O&M and Controls): This 40- by 120-foot building, oriented northeast-southwest, will be constructed at the west end of the WRF site and will be an at-grade structure supported on an interior slab-on-grade. The perimeter of the structure and interior load-bearing walls will be supported on shallow spread footings.

- Building A (Headworks and Dewatering): This 40- by 120-foot building, oriented southeast-northwest, will abut the southeast corner of the O&M and Controls building to form the leg of the L-shaped Building A. This will be an at-grade structure supported on an interior slab-on-grade. The perimeter of the structure and interior load-bearing walls will be supported on shallow spread footings.
- *SC90 WAS and RAS Pump Stations*: Each of these two pump stations will be an approximately 10-foot diameter reinforced concrete vault structure with a base depth of approximately 16.5 feet below existing site grades and will be supported on a reinforced concrete mat foundation.
- Existing Secondary Clarifier SC60 RAS/WAS Pump Station: This pump station will be an approximately 10- by 20-foot reinforced concrete vault structure with a base depth of approximately 12.5 feet below existing site grades and will be supported on a reinforced concrete mat foundation.
- Aeration Basin No. 3: Aeration Basin No. 3 will consist of an approximately 40- by 75-foot reinforced concrete structure. The base depth of Aeration Basin No. 3 will be approximately 13 to 18 feet below existing site grades and will be supported on a reinforced concrete mat foundation. Aeration Basin No. 3 will be expanded to the north (designated Aeration Basin No. 4) in a future project phase and will likely be founded at the same depth as Aeration Basin No. 3.
- Tertiary Filters: The tertiary filters will be housed in an approximately 25- by 35-foot reinforced concrete tank structure. The base depth of Aeration Basin No. 3 will be approximately 18 feet below existing site grades and will be supported on a reinforced concrete mat foundation.
- Tertiary Building, UV Disinfection, and UW Storage: These facilities will be located adjacent to one another within an approximate 35- by 40-foot area. The UW Storage and UV Storage facilities will be constructed within the existing chlorine contact chamber. The portion of the Tertiary Building housing the utility water pumps will likely be approximately 7 feet below existing site grades.
- *Outfall*: The outfall pipe leads from the UV disinfection chamber to the South Santiam River. This will either be part of the Phase 2 improvements or in a future project phase.
- Other Improvements: New asphalt pavements along the entrance road from Pleasant Valley Road
 and withing most of the WRF site; new concrete curbs and sidewalks; two new entrance gates;
 and a new chain-link fence.

1.3 Purpose and Scope of Work

The primary purpose of this GDR is to compile sufficient geologic and geotechnical data at the project site to support the design and construction of the proposed improvements. This GDR presents a summary of geologic and geotechnical data collected from field explorations and testing, laboratory testing, a review of published geological sources, and other historical geotechnical information related to the project. Information contained in this GDR will be used to characterize subsurface conditions at the site and to develop recommendations for design and construction of the Project.

2.0 Site Conditions

2.1 Site Description

The existing WRF site is located on the south bank of the South Santiam River near its confluence with Ames Creek. The site is bordered by the Albany and Eastern railroad tracks to the south, a boat ramp access road and the South Santiam River beyond, private property to the west, and Ames Creek to the east. In general, the site slopes gently from the west to the east (i.e., towards Ames Creek) and gently from the south to the north towards the South Santiam River. The eastern two-thirds of the site, where most of the WRF is located, is situated on a relatively level terrace that generally sits 5 to 7 feet lower than the western one-third of the site and approximately 10 feet above Ames Creek to the east.

A paved access road runs along the north side of the site, from the Pleasant Valley Road WRF entrance at the west end of the site to the boat ramp and recreation use area at the north-northeast end of the site that provides public access to the South Santiam River. The paved WRF road, heading south from the access road from the main entrance gate, provides access to the main areas within the WRF. Bedrock is exposed along the South Santiam River and Ames Creek approximately 100 feet to the north-northeast of the site. Exposed bedrock on the riverbank slopes steeply into the river.

2.2 Regional Geology

The project site is located in the foothills of the Western Cascades, a north-south trending physiographic region that stretches from northern California to British Columbia, tucked between the Willamette Valley to the west and the younger High Cascades to the east. The Western Cascades in Oregon were formed by a series of volcanic events from approximately 35 to 17 million years ago. The region is marked by densely forested hills dissected by the region's many rivers (Madin, 1990; Schlicker and Deacon, 1967; Wilson, 1998; Popowski, 1996).

The Paleogene structural basement of this region of the Western Cascades is composed of non-marine volcaniclastic sedimentary rocks, tuff, basaltic andesite, andesite, and dacite of the Late Eocene to Oligocene Fisher Formation. The Fisher Formation is overlain by basalt lavas, ash-flow tuff, tuff, and non-marine sedimentary rocks of the Little Butte Volcanic Series. A subducting plate below the Eocene shoreline resulted in a volcanic chain that produced the volcanic activity responsible for the Fisher Formation and the Little Butte Volcanic Series. As the angle of the subducting plate shifted, the volcanic activity gradually shifted east of the region. The bedrock in the project area is composed of tholeitic basalt from the Little Butte Volcanic Series.

Over the span of geologic time, quaternary sedimentary deposits of alluvium, colluvium, landslide deposits, and terrace deposits have accumulated on the volcanic rock surfaces and in the valleys formed by the rivers. The sediments that directly overlie the bedrock in the project area have been defined as alluvium. Regionally, the alluvium consists of unconsolidated gravel, sand, silt, and clay deposited in modern stream channels, adjoining flood plains, overbanks, and low bedrock terraces.

Across the project area, fill materials are present near the ground surface. These materials were generally placed to re-grade the site prior to development.

3.0 Site Investigations

3.1 General

The subsurface exploration was completed in three phases, between 2018 and 2022, and consisted of five boreholes (designated B-1 through B-5) and 35 probe-holes (designated P-01 through P-35).

The site investigations were performed under the direction of a geologist or geotechnical engineer from MJA, who examined and logged the soil and rock conditions encountered in the borings. The soils were classified in the field in accordance with the Visual-Manual Procedure (ASTM D 2488). Sample depths, stratigraphy, groundwater occurrence and soil engineering characteristics were also recorded. The stratigraphic contacts indicated on the logs represent the approximate boundaries between soil and rock; actual transitions may be more gradual. The boring logs are presented in Appendix A. The soil and rock samples were transported to the MJA Portland office for further examination. The rock core samples were transported, stored, cataloged, and disposed of in accordance with the Standard Practices for Preserving and Transporting Rock Core Samples (ASTM D 5079). The rock core photos are included in Appendix B

3.2 Exploratory Borings

Phase one was completed between April 30 and May 2, 2018 and included the advancement of five boreholes (B-1 through B-5) and fifteen probe-holes (P-01 through P-15). The Phase one borings were completed by Western States Soil Conservations Services of Hubbard, Oregon using a truck-mounted CME-75 drill rig. The boreholes were advanced to depths ranging from 5.8 to 23 feet bgs using mudrotary drilling techniques within soil materials, and HQ-triple-tube wireline coring in rock materials in accordance with ASTM D 2113. The probe-holes were advanced using hollow-stem auger drilling techniques and extended to depths ranging from 2.5 to 15 feet bgs at which point drilling refusal was encountered at the top of the bedrock surface.

Phase two was completed on October 29, 2019 and consisted of the advancement of 12 probe-holes (designated P-16 through P-27). The phase two probe-holes were completed by McCallum Rock Drilling of Salem, Oregon using a track-mounted Furakawa 900 drill rig. The probe-holes were advanced to depths ranging from 11 to 26 feet bgs using air percussion drilling methods.

Phase three was completed on June 20, 2022 and consisted of eight probe-holes (designated P-28 through P-35) advanced by PLI Systems of Hillsboro, Oregon using a truck-mounted Mobile B-75 drill rig. The probe-holes were advanced using hollow-stem auger drilling techniques and extended to depths ranging from 3.7 to 19 feet bgs.

In borings B-2 through B-5 and probe-holes P-28 through P-35, disturbed soil samples were collected in conjunction with a SPT at selected intervals using a standard 2-inch diameter split-barrel sampler, and automatic safety hammer system. In each test, the sampler was advanced 18 inches by dropping the 140-pound hammer a distance of 30 inches for each strike in accordance with ASTM D 1586. The number of hammer blows for each six inches of penetration was recorded. The standard penetration resistance (N-value) of the soil is calculated as the sum of the number of blows required for the final 12 inches of sampler penetration. The SPT N-values of greater than 50 blows per six inches of penetration is referred

to as refusal. The N-value is an indication of the relative density of granular soils and the relative consistency of cohesive soils. Uncorrected N-values are presented on the boring logs. In addition to the SPT sampling, relatively undisturbed samples were obtained in selected borings by pushing a 3-inch inside diameter thin wall sampler (Shelby Tube) in accordance with ASTM D 1587.

In the probe-holes in which SPT sampling was not performed, soil samples were taken from borehole cuttings. These sampling locations are shown in boring logs.

3.3 Piezometers

Two-inch diameter piezometers were installed in two boreholes to allow for long-term, stabilized groundwater level measurements. Piezometers were installed in the following boreholes:

- Boring B-3, screened between 9 and 12 feet bgs.
- Boring B-5, screened between 5.5 and 8.5 feet bgs.

3.4 Borehole Backfill and Abandonment

Piezometer construction and borehole backfilling and abandonment was performed in accordance with the requirements of the Oregon Department of Water Resources (OAR 690-240, Construction, Maintenance, Alteration, Conversion, and Abandonment of Monitoring Wells, Geotechnical Holes, and Other Holes in Oregon).

4.0 Laboratory Testing

Field samples obtained from exploratory borings were delivered to the MJA Portland office for further examination and storage. Each of the samples was re-examined and compared to the field boring log description to confirm the field classifications and maintain consistency. Representative samples were then selected for laboratory testing. Testing was performed on soil and rock samples from boreholes B-1, B-3, and B-5. The laboratory test results are provided in Appendix B. The laboratory testing included the following index and strength property tests, performed in accordance with relevant ASTM standards:

- Moisture content analyses (ASTM D 2216).
- Atterberg limits tests (ASTM D 4318).
- Point load tests of rock core (ASTM D 5731).
- Unconfined compressive strength of intact rock core (ASTM D 7012).

Point load testing was performed by MJA. All other laboratory testing was performed by Northwest Testing, Inc. of Wilsonville, Oregon. Laboratory test results are presented in Appendix B. A summary of the laboratory test results is provided in Table 4-1 on the following page. Comprehensive laboratory test results are presented in Appendix C.

Table 4-1. Laboratory Test Results Summary

Sample Location or ID										Strength Testing			
			Soil Description		% e.u	Atterberg Limits		Point Load Strength Index, PLI	PLI Test Type	UCS ¹			
					stur	Liquid	Plastic	Plasticity	macx, r Er	<u> </u>			
Boring	Sample, No.	Top Depth (feet)	Bottom Depth (feet)	Geologic Unit	USCS Soil Type	Soil or Rock Description	Moisture	Limit (LL)	Limit Limit		Corrected Point Load Strength Index, I _{s(50)} (PSI)	Diametral or Axial	PSI
B-1	R-1	5.2	6.3	LBVS Basalt		BASALT							25,302
B-3	S-2	5	6.5	FILL	СН	SANDY FAT CLAY	44.8	51	28	23			
B-5	S-1	2.5	4	FILL	SM	SILTY SAND with Gravel	26.3	44	28	16			
B-5	R-1	11.7	12.4	LBVS Basalt		BASALT							29,919
B-5	R-2	18.0	19.0	LBVS Basalt		BASALT							6,932
B-5	R-2	16.8	17.2	LBVS Basalt		BASALT					513	Diametral	12,563 ²
B-5	R-2	16.3	16.8	LBVS Basalt		BASALT					304	Diametral	7,4372
B-5	R-2	16.8	17.0	LBVS Basalt		BASALT					653	Diametral	13,7172
B-5	R-2	17.2	18.0	LBVS Basalt		BASALT					268	Diametral	5,098 ²
B-5	R-2	17.6	18.0	LBVS Basalt		BASALT					569	Diametral	13,951 ²
B-5	R-3	22.5	23.0	LBVS Basalt		BASALT					579	Diametral	13,315 ²
B-5	R-1	9.0	9.5	LBVS Basalt		BASALT					407	Diametral	9,778²
B-5	R-2	12.4	12.8	LBVS Basalt		BASALT					714	Diametral	17,138 ²
B-5	R-2	15.8	16.7	LBVS Basalt		BASALT					383	Diametral	9,185 ²
B-5	R-2	16.3	16.7	LBVS Basalt		BASALT					690	Diametral	14,493²

¹ UCS = Unconfined compressive strength. ² UCS results from point load strength index testing are based on a correlation factor of 24.5, which was calculated by plotting the relationship between UCS and point load strength index test results.

5.0 Subsurface Conditions

The materials encountered in the explorations were grouped into three geotechnical units: Pavement, Fill, and Basalt (Little Butte Volcanic Series – Tholeiitic Basalt). These units have been defined by their geologic origin, stratigraphic position, engineering properties, and their distribution in the subsurface. Variations in subsurface conditions may exist between the locations of the borings. Contacts between the units may be more gradational than shown in the boring logs in Appendix A. The SPT N-values, shown on the boring logs and discussed below, are reported as counted in the field (uncorrected). Photos of rock core samples are provided in Appendix C. Cross sections of subsurface conditions are shown in Figures 3A through 3F. The following sections describe each geotechnical unit in greater detail.

5.1.1 Pavement

Asphalt concrete (AC) pavement was encountered in 12 borings (P-01, P-03, P-04, P-16, P-17, P-19, P-23, P-25, P-29, P-31, P-32, and P-34). The AC pavement ranged from about 4 to 6 inches in thickness and was typically underlain by about 4 inches of crushed rock base. Portland Cement Concrete (PCC) pavement was encountered in two borings (B-1 and P-27). In B-1, the PCC pavement was about 18 inches thick and underlain by about 12 inches of crushed rock base. In P-27, the PCC pavement section was about 6 inches thick.

5.1.2 Fill

Undocumented fill materials were present in all the explorations and were likely placed for site grading and development. We observed highly variable fill depths across the site, extending from the existing ground surface to depths ranging from 2 to 16.5 feet bgs. Additionally, the fill depths often varied considerably over short distances. The fill was underlain by bedrock, and generally consisted of silt with variable amounts of organics, lean to fat clay with variable amounts of sand and gravel, sand and gravel with variable amounts of silt and clay, and organic soil with variable amounts of sand. Undocumented fill refers to materials placed without (available) records of subgrade conditions or evaluation of compaction. Standard Penetration Tests (SPTs) conducted within the fill yielded N-values ranged from 0 to 45 blows per foot (bpf). The higher N-values were generally recorded at the fill-bedrock contact.

5.1.3 Basalt - Little Butte Volcanic Series Tholeiitic Basalt

The Little River Butte Volcanic Series (LRBV) in the project area consists of tholeitic basalt and is bedrock in this area. The basalt was encountered beneath the fill in all the explorations. The contact between fill and LBVS was generally selected based on auger refusal, drilling resistance, drill cuttings, and/or SPT refusal (more than 50 blow counts for 6 inches or less of penetration). The depth to the LRBV ranged from 2.0 feet to 16.5 feet bgs.

The strength of the basalt rock generally varies from strong to very strong (R4 to R5). Rock strength descriptors are presented in Table 5-1. The basalt is slightly weathered to fresh, moderately to highly fractured (i.e., joint spacing from 3 feet to 2 inches), and gray. The joints were generally sub-horizontal or high angle, smooth to rough, planar, curved, and irregular in shape. The occasional joint had a less than

0.1-inch-thick light blue green coating. Core recovery ranged from 80 to 100 percent. The Rock Quality Designation of the cored rock (RQD) ranged from 42 to 86 percent, with an average value of 67 percent.

Table 5-1. Definition of Rock Strength Descriptions

Grade ¹	Approximate Uniaxial Compressive Strength (psi)	Qualitative Description
R0	35 – 150	Extremely Weak
R1	150 – 700	Very Weak
R2	700 – 3,600	Weak
R3	3,600 – 7,200	Medium Strong
R4	7,200 – 14,500	Strong
R5	14,500 – 36,000	Very Strong
R6	>36,000	Extremely Strong

¹ Rock strength grades from Brown (1981).

Three unconfined compressive strength tests were completed on selected rock core samples. The unconfined compressive strength of the rock cores ranged from 6,932 psi to 25,919 psi with an average value of 19,384 psi. The results of point load tests indicate the Point Load Index ($I_{S(50)}$) of the basalt ranged between approximately 268 psi and 714 psi, with an average value of 508 psi. Depth to rock at each exploration is summarized in Table 5-2. The depth to bedrock is also included on the geologic cross sections shown in Figures 3A through 3F.

Table 5-2. Depth of Fill / Depth to Rock Summary

Exploration ID	Associated Structure(s)	Ground Surface Elevation (feet)	Depth to Rock (feet)	Rock Surface Elevation (feet)
B-1		516.5	2.5	514.0
B-2	SC90 WAS Pump Station	518.3	16.5	501.8
B-3	Primary Clarifier	529.9	11	518.9
B-4	Building A – O&M & Controls	538.6	4.5	534.1
B-5	MEB Building	528.8	8.5	520.3
P-01		517.3	11	506.3
P-02		526.1	11.5	514.6
P-03	Primary Clarifier	530.4	16	514.4
P-04	Building A – Headworks & Dewatering	530.8	7.5	523.3
P-05	Building A – Headworks & Dewatering	533.1	3.5	529.6
P-06	Building A – O&M & Controls	536.2	3.5	532.7
P-07		538.0	2.5	535.5
P-08	Building A – O&M & Controls	535.3	7.5	527.8
P-09		530.5	4	526.5
P-10		530.1	5.5	524.6
P-11	MEB Building	529.8	5.5	524.3
P-12	MEB Building	526.4	5	521.4
P-13	Future Aeration Basin No. 4	523.6	8.5	515.1
P-14	Aeration Basin No. 3	521.7	5.5	516.2
P-15		519.1	2.5	516.6
P-16	IPS	517.1	12.0	505.1
P-17		516.5	6.0	510.5
P-18	SC90, SC60 RAS/WAS Pump Station	518.5	9.0	509.5
P-19		518.0	6.0	512.0
P-20	SC90 RAS Pump Station	525.4	8.0	517.4
P-21	Primary Clarifier, Aeration Basin No. 3	521.8	6.0	515.8
P-22	Aeration Basin No. 3	518.5	7.0	511.5
P-23	Primary Clarifier	530.4	15.0	515.4
P-24	MEB Building	529.7	3.0	526.7
P-25	Future Outfall	506.4	2.0	504.4
P-26	Future Outfall	507.2	3.0	504.2
P-27		516.6	10.0	506.6
P-28	IPS	516.5	11.0	505.5
P-29	IPS	517.3	12.5	504.8
P-30	SC90	519.8	11.0	508.8
P-31	SC90	520.8	10.0	510.8
P-32	Primary Clarifier, Future Primary Clarifier No. 4	530.4	10.0	520.4
P-33	Future Outfall	529.6	3.5	526.1
P-34	Tertiary Filters	516.9	15.0	501.9
P-35	-	517.5	6.0	511.5

5.2 Groundwater

Two-inch diameter, PVC groundwater observation wells were installed in borings B-3 and B-5. Initial groundwater levels recorded on May 2, 2018, with subsequent levels recorded on October 29, 2019, and on June 20, 2022. We also observed groundwater in nine other exploration locations, as summarized in Table 5-3. Groundwater levels are noted on the boring logs in Appendix A.

Groundwater levels may vary with precipitation, the time of year, and/or other factors. Generally, groundwater highs occur near the end of the wet season in late spring and groundwater lows occur near the end of the dry season in the early fall.

Table 5-3. Groundwater Level Measurements

		Borehole	Depth to	Groundw	ater (feet)	Groundwater Elevation (feet)			
Boring ID			Oct 29, 2019	June 20, 2022	May 2, 2018	Oct 29, 2019	June 20, 2022		
B-2	No	518.3	2.5	-	-	515.8	-	-	
B-3	Yes	529.9	2.9	3.6	8.2	527.0	526.3	521.7	
B-4	No	538.6	2.0	-	-	536.6	-	-	
B-5	Yes	528.8	5.0	7.0	6.8	523.8	521.8	522.0	
P-03	No	530.4	8.5	-	-	521.9	-	-	
P-09	No	530.5	3.0	-	-	527.5	-	-	
P-13	No	523.6	6.0	-	-	517.6	-	-	
P-15	No	519.1	2.5	-	-	516.6	-	-	
P-29	No	517.3	-	-	10.5	-	-	506.8	
P-31	No	520.8	-	-	9.2	-	-	511.6	
P-32	No	530.4	-	-	9.7	-	-	520.7	

6.0 Limitations

This Geotechnical Data Report has been prepared for the City of Sweet Home Mahler Water Reclamation Facility Improvements Project located in Sweet Home, Oregon. This report contains a compilation of information from field explorations, laboratory test data, and published literature. The professional judgments and characterizations presented herein are based on this information. McMillen Jacobs Associates is not responsible for errors and omissions that may appear in the field exploration and laboratory test results performed by others.

It shall be noted that the geotechnical data were obtained at specific exploration locations at specific times. It must be acknowledged that variations in soil/rock conditions exist between exploration locations, and this report does not necessarily reflect the variations between explorations. The nature and extent of variation may not become evident until exposed during construction. No analyses, interpretations between exploration locations, conclusions, or design recommendations are contained in this report. This report should be made available to prospective contractors for use as factual data only, and not as a warranty of subsurface conditions.

The scope of our geotechnical services has not included an environmental evaluation regarding the presence or absence of hazardous or toxic materials in the soil, surface water, groundwater, or air, on or below the site, or for evaluation of disposal of contaminated soils or groundwater, should they be encountered.

This report has been completed within the limitations of the agreement between the Owners and West Yost Associates, Inc. and the West Yost Associates, Inc.-approved scope of work, schedule, and budget with McMillen Jacobs Associates. The services rendered have been performed in a manner consistent with the level of care and skill ordinarily exercised by members of the profession currently practicing under similar conditions in the same area. McMillen Jacobs Associates is not responsible for the use of this report in connection with anything other than the project at the project location.

MCMILLEN JACOBS ASSOCIATES

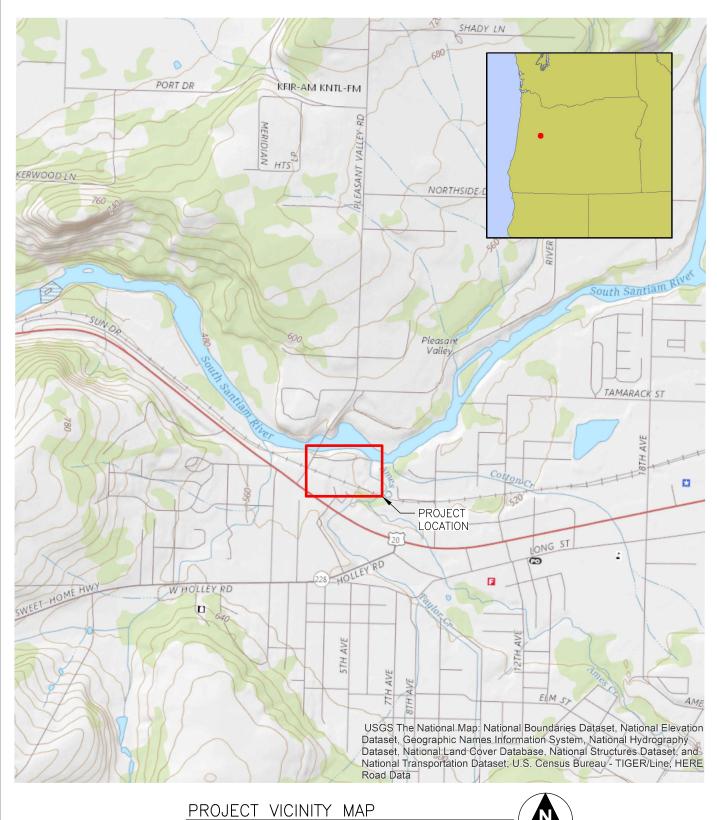
Jeff Quinn, P.E.
Senior Project Engineer

7.0 References

- ASTM Standard, 2017, ASTM International, West Conshohocken, PA, 2017, www.astm.org.
- Brown E.T. (Ed). 1981, Rock characterization, testing and monitoring ISRM suggested methods, 171-183. Oxford, Pergoman.
- Brown and Caldwell, 2016, City of Sweet Home Wastewater Facilities Plan, December 2016.
- Madin, I.P., 1990. Earthquake-Hazard Geology Maps of the Portland metropolitan Area, Oregon: Oregon Department of Geology and Mineral Industries Open-File Report O-90-02.
- Popowski, T.A., 1996. Geology, Structure, and Tectonic History if the /Tualatin Basin, Northwestern Oregon: Corvallis, Oregon, Oregon State University Master's Thesis.
- Schlicker, H.G. and Deacon, R.J., 1967. Engineering Geology of the Tualatin Valley Region, Oregon: Oregon Department of Geology and Mineral Industries Bulletin 60.
- Wilson, D.C., 1998, Post-middle Miocene geologic evolution of the Tualatin basin, Oregon: Oregon Geology, v. 60, no. 5, p. 99-116.

14

Figures



SCALE: NTS



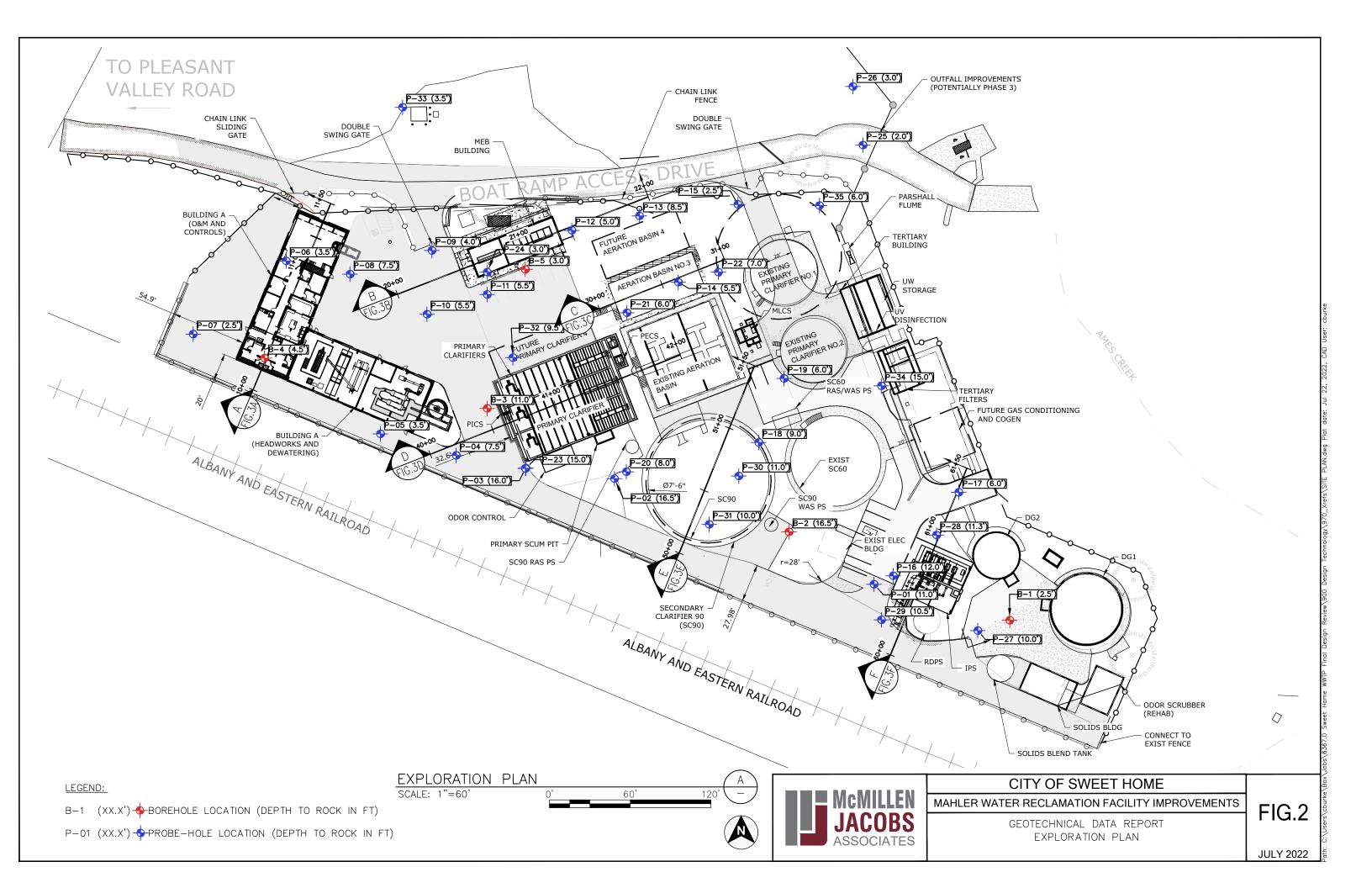


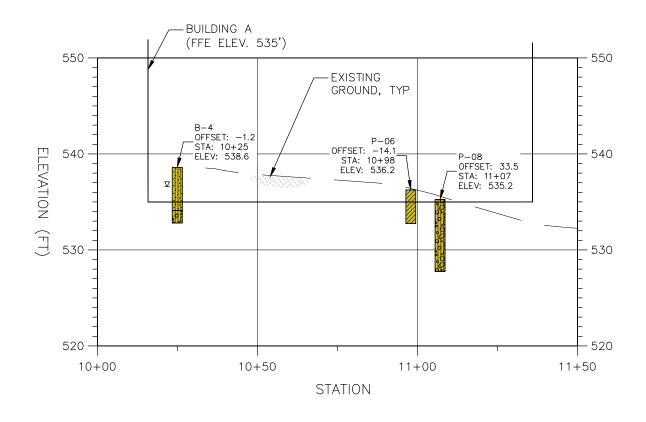
CITY OF SWEET HOME

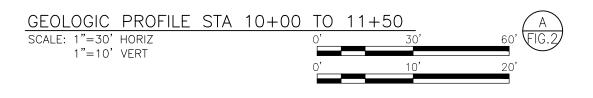
MAHLER WATER RECLAMATION FACILITY IMPROVEMENTS

GEOTECHNICAL DATA REPORT PROJECT VICINITY MAP

FIG.1







NOTES:

- - 1. BOREHOLE LOCATIONS ARE APPROXIMATE. BOREHOLE LOCATIONS ARE PROJECTED PERPENDICULAR TO ALIGNMENT.
 - OFFSETS ARE NEGATIVE LEFT OF ALIGNMENT AND POSITIVE RIGHT OF ALIGNMENT WHEN TRAVELING IN THE DIRECTION OF INCREASING STATION.
 - 4. ALIGNMENT IS BASED ON 90% DRAWINGS PROVIDED BY WEST YOST ASSOCIATES, DATED JUNE 2022.

STRATIGRAPHIC LEGEND:

WELL-GRADED GRAVEL (GW)

LEAN CLAY (CL)

SILTY GRAVEL (GM) CLAYEY GRAVEL (GC)

LOW PLASTICTIY ORGANIC CLAY (OL)

BASALT

SILTY SAND (SM)

CLAYEY SAND (SC)

FAT CLAY (CH)

BASALT



CITY OF SWEET HOME

STANDPIPE

BORING/PROBE-HOLE LEGEND:

B-3 OFFSET: -16.0 _ STA: 40+52 ELEV: 529.9

MAHLER WATER RECLAMATION FACILITY IMPROVEMENTS

GROUNDWATER LEVEL

GEOTECHNICAL DATA REPORT GEOLOGIC PROFILE BUILDING A

FIG.3A

JULY 2022

OFFSET FROM ALIGNMENT CENTERLINE, STATION ALONG ALIGNMENT, AND BORING USCS GRAPHIC SYMBOL ENGINEERING SOIL UNIT

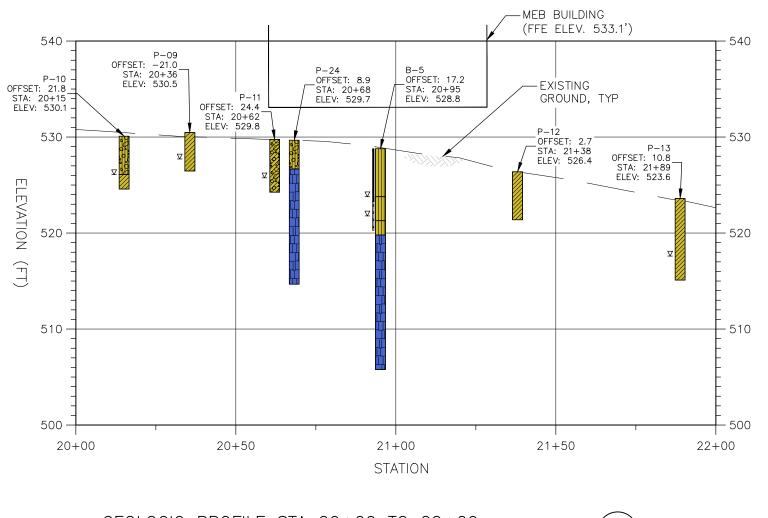
BOREHOLE ID

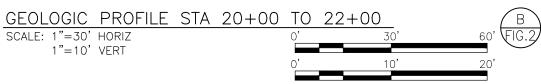
SPT N-VALUE

(BLOWS/FT)

END OF BORING

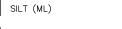
50/3"





STRATIGRAPHIC LEGEND:

WELL-GRADED GRAVEL (GW)





SILTY GRAVEL (GM)

CLAYEY GRAVEL (GC) LOW PLASTICTIY ORGANIC CLAY (OL)

SILTY SAND (SM)

CLAYEY SAND (SC)

FAT CLAY (CH) BASALT

BASALT

NOTES:

- 1. BOREHOLE LOCATIONS ARE APPROXIMATE.
- BOREHOLE LOCATIONS ARE PROJECTED PERPENDICULAR TO ALIGNMENT.
- OFFSETS ARE NEGATIVE LEFT OF ALIGNMENT AND POSITIVE RIGHT OF ALIGNMENT WHEN TRAVELING IN THE DIRECTION OF INCREASING STATION.
- 4. ALIGNMENT IS BASED ON 90% DRAWINGS PROVIDED BY WEST YOST ASSOCIATES, DATED JUNE 2022.

CITY OF SWEET HOME

GROUNDWATER LEVEL

MAHLER WATER RECLAMATION FACILITY IMPROVEMENTS

BORING/PROBE-HOLE LEGEND:

OFFSET: -16.0 _ STA: 40+52

ELEV: 529.9

GEOTECHNICAL DATA REPORT GEOLOGIC PROFILE MEB BUILDING

FIG.3B

JULY 2022

OFFSET FROM ALIGNMENT CENTERLINE, STATION ALONG ALIGNMENT, AND BORING USCS GRAPHIC SYMBOL

BOREHOLE ID

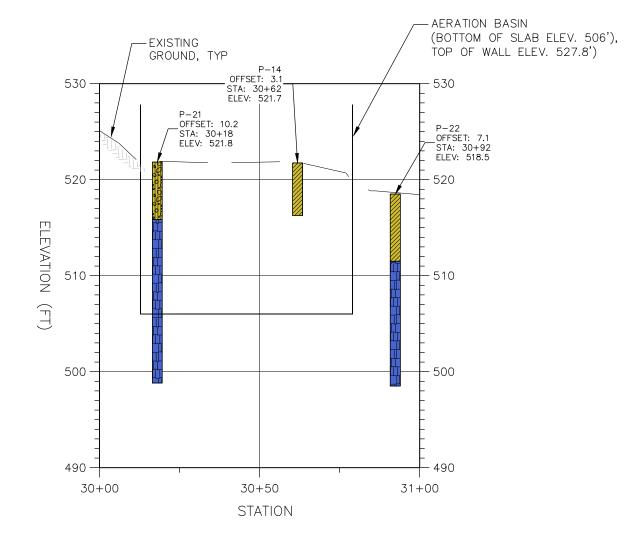
SPT N-VALUE

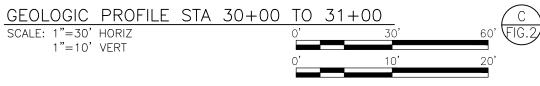
ENGINEERING SOIL UNIT

(BLOWS/FT)

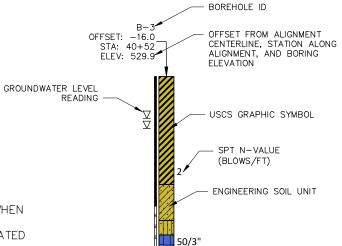
END OF BORING

50/3"





BORING/PROBE-HOLE LEGEND:



STRATIGRAPHIC LEGEND:



SILTY GRAVEL (GM) CLAYEY GRAVEL (GC)

LEAN CLAY (CL)

LOW PLASTICTIY ORGANIC CLAY (OL)

SILTY SAND (SM)

CLAYEY SAND (SC)

BASALT

FAT CLAY (CH)

NOTES:

BASALT

- 1. BOREHOLE LOCATIONS ARE APPROXIMATE.
- BOREHOLE LOCATIONS ARE PROJECTED PERPENDICULAR TO ALIGNMENT.
- OFFSETS ARE NEGATIVE LEFT OF ALIGNMENT AND POSITIVE RIGHT OF ALIGNMENT WHEN TRAVELING IN THE DIRECTION OF INCREASING STATION.
- 4. ALIGNMENT IS BASED ON 90% DRAWINGS PROVIDED BY WEST YOST ASSOCIATES, DATED JUNE 2022.



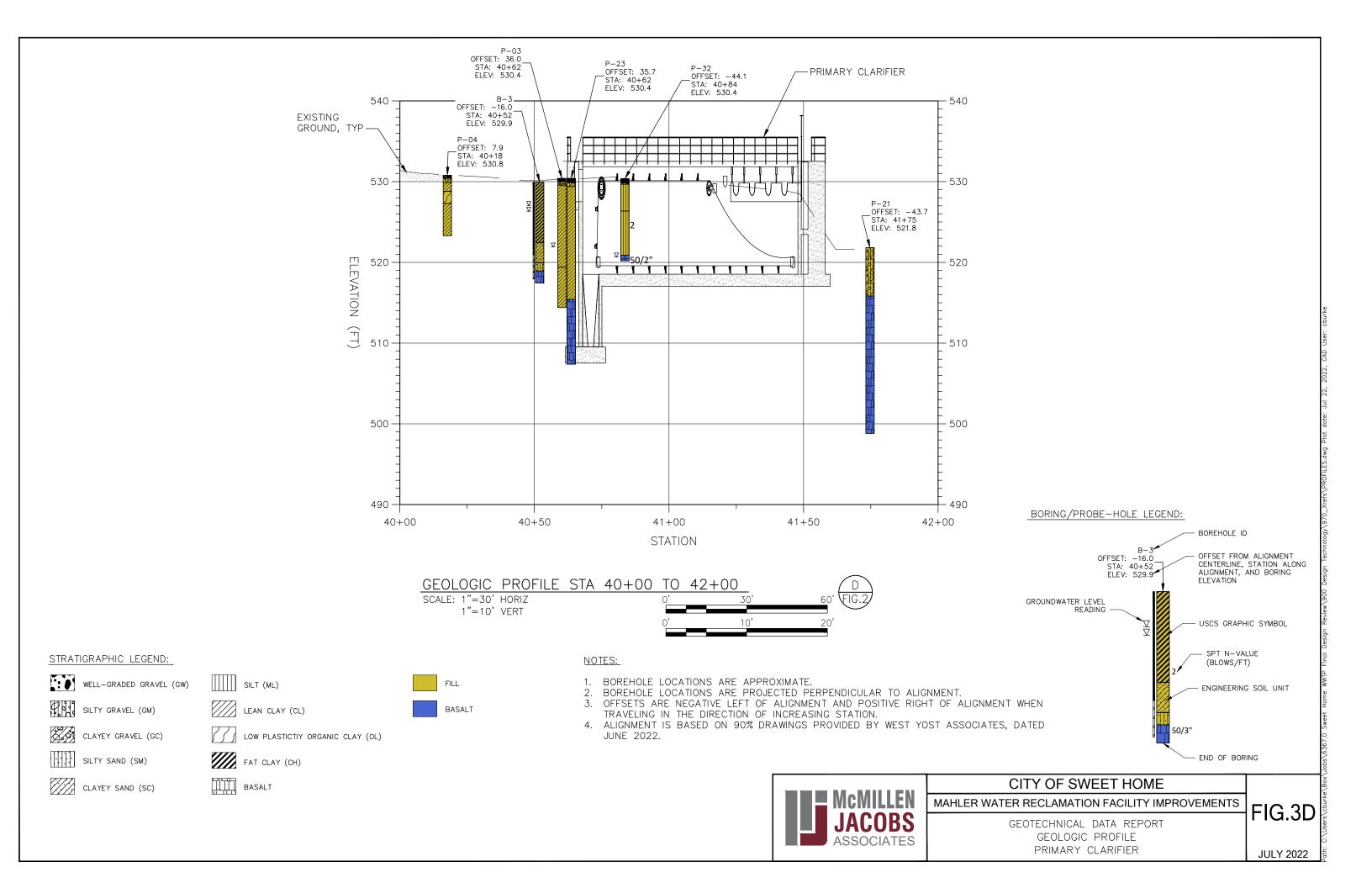
CITY OF SWEET HOME

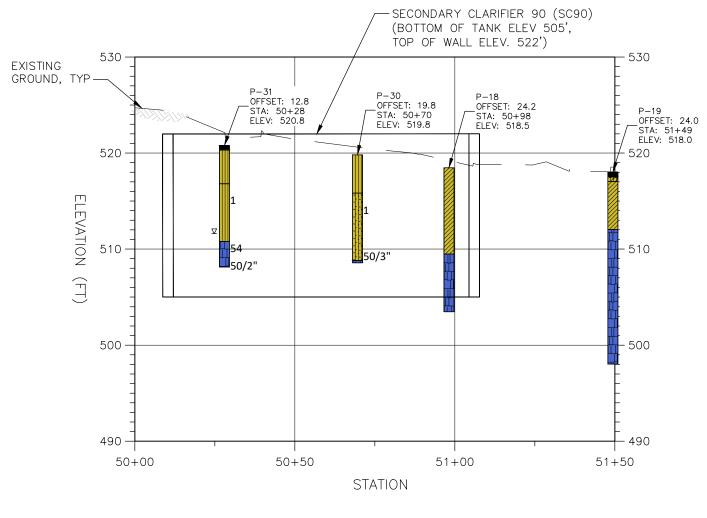
MAHLER WATER RECLAMATION FACILITY IMPROVEMENTS

GEOTECHNICAL DATA REPORT GEOLOGIC PROFILE AERATION BASIN NO. 3

FIG.3C

END OF BORING

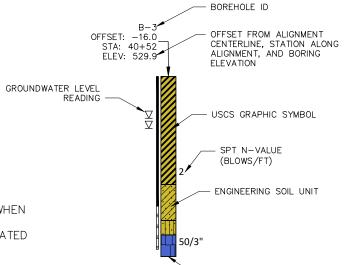








BORING/PROBE-HOLE LEGEND:



STRATIGRAPHIC LEGEND:

WELL-GRADED GRAVEL (GW)

BASALT

SILTY GRAVEL (GM) LEAN CLAY (CL)

LOW PLASTICTIY ORGANIC CLAY (OL)

SILTY SAND (SM)

CLAYEY SAND (SC)

CLAYEY GRAVEL (GC)

FAT CLAY (CH)

BASALT

NOTES:

1. BOREHOLE LOCATIONS ARE APPROXIMATE.

BOREHOLE LOCATIONS ARE PROJECTED PERPENDICULAR TO ALIGNMENT.

OFFSETS ARE NEGATIVE LEFT OF ALIGNMENT AND POSITIVE RIGHT OF ALIGNMENT WHEN TRAVELING IN THE DIRECTION OF INCREASING STATION.

4. ALIGNMENT IS BASED ON 90% DRAWINGS PROVIDED BY WEST YOST ASSOCIATES, DATED JUNE 2022.

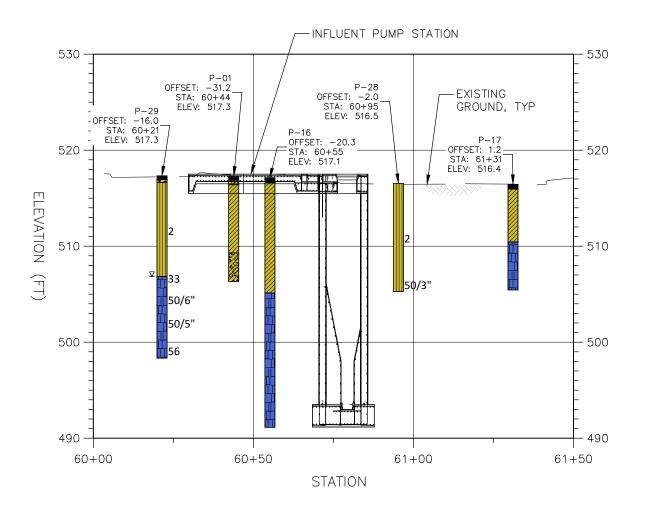


CITY OF SWEET HOME

MAHLER WATER RECLAMATION FACILITY IMPROVEMENTS

GEOTECHNICAL DATA REPORT GEOLOGIC PROFILE SECONDARY CLARIFIER 90 (SC90) FIG.3E

END OF BORING



NOTES:

GEOLOGIC PROFILE STA 60+00 TO 61+50

- 1. BOREHOLE LOCATIONS ARE APPROXIMATE.
- BOREHOLE LOCATIONS ARE PROJECTED PERPENDICULAR TO ALIGNMENT.
- OFFSETS ARE NEGATIVE LEFT OF ALIGNMENT AND POSITIVE RIGHT OF ALIGNMENT WHEN TRAVELING IN THE DIRECTION OF INCREASING STATION.
- 4. ALIGNMENT IS BASED ON 90% DRAWINGS PROVIDED BY WEST YOST ASSOCIATES, DATED JUNE 2022.

STRATIGRAPHIC LEGEND:

WELL-GRADED GRAVEL (GW)

FAT CLAY (CH)

LEAN CLAY (CL)

SILTY GRAVEL (GM)

CLAYEY GRAVEL (GC)

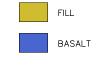
LOW PLASTICTIY ORGANIC CLAY (OL)



SILTY SAND (SM) CLAYEY SAND (SC)



BASALT



SCALE: 1"=30' HORIZ

1"=10' VERT



CITY OF SWEET HOME

BORING/PROBE-HOLE LEGEND:

OFFSET: -16.0_ STA: 40+52

ELEV: 529.9

MAHLER WATER RECLAMATION FACILITY IMPROVEMENTS

GROUNDWATER LEVEL

GEOTECHNICAL DATA REPORT GEOLOGIC PROFILE INFLUENT PUMP STATION

FIG.3F

BOREHOLE ID

OFFSET FROM ALIGNMENT CENTERLINE, STATION ALONG ALIGNMENT, AND BORING

USCS GRAPHIC SYMBOL

ENGINEERING SOIL UNIT

SPT N-VALUE

(BLOWS/FT)

END OF BORING

Appendix A Boring Logs

Project: Sweet Home WWTP Schematic Design Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring B-1

Boring B-1

Sheet 1 of 1

Date(s) 04/30/2018	Geotechnical Consultant	IcMillen Jacobs Asso	ciates	Logged J. Irizarry		Checked	K. Elliott	
Drilling Method/	Rotary and HQ Wireline/CME 75	Drilling Western States Sail Conservation Inc.			Total Depth 42.5%			
Rig Type Hole Diameter 4.00 in	•	Hammer Weight/Drop //b/ip \/Type 140 lb / 30 ip / Automatic Groul			Ground Surface	round Surface 546 5 ft		
Location Survey	•				Elevation/Datum Site Survey			
	DENIE						-,	
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE RECOVERY (%)	# RES BLO BLOV	ISTANCE S SWS/FT S		MATERIAL DESCR		BACKFILL INFORMATION	REMARKS AND TESTS	
				Concrete - 18" thick (Paveme	nt)			
-512	RUN 1			Dense, moist, gray Silty Grave coarse angular gravel, low plas Aggregate) BASALT, very strong (R5), slig to fresh, moderately to highly f stepped, smooth to rough joint narrow apertures (Little Butte VTholeititc Basalt) Run 1: 3.5-8.5 feet. RQD = 59% Run 2: 8.5-13.5 feet: RQD = 42% Planar and irregular, smooth rough joints.	sticity silt (Base htly weathered ractured, planar, s with very /olcanic Series -	J	At 2.5 feet very slow, very rough drilling. At 3.5 feet switch to rock coring. From 5.20 feet to 6.30 feet, UCS = 25,300 psi.	
-502 - - 15 - 			##				Borehole completed at 13.5ft. below ground surface (bgs).	
-497								
- 25								

Project: Sweet Home WWTP Schematic Design Project Location: Sweet Home, OR Log of Boring B-2 Project Number: 5834.0 Geotechnical Date(s) Checked By K. Elliott Logged 05/01/2018 McMillen Jacobs Associates J. Irizarry Drilled Drilling Method/ Drilling Total Depth Mud Rotary/CME 75 Western States Soil Conservation, Inc. 16.5 ft Rig Type Contractor of Borehole Ground Surface Hole Diameter 4.00 in Hammer Weight/Drop (lb/in.)/Type 140 lb / 30 in / Automatic 518.2 ft Elevation/Datum ocation Survey 7617232.28 E, 275385.44 N Elevation Source Site Survey PENETRATION ELEV. (FT) WATER LEVEL SAMPLE TYPE RECOVERY (%) **GRAPHIC LOG** BACKFILL INFORMATION DEPTH (FT) **RESISTANCE** SAMPLE# BLOW **USCS REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS** 40 60 Э ис LL/Pl Organic Soil (OL/OH); Mulch (Fill) Very stiff, moist, brown to gray brown, Sandy LEAN CLAY with Gravel (CL); Medium plasticity, medium to low toughness, fine to ______12/2/2018 1**|** coarse sand, fine angular gravel (Fill) 7-5-11 47 S1 (N=16)Loose, moist to wet, brown, Silty SAND with 2-4-4 27 S2 Gravel (SM); Fine to coarse sand, fine angular (N=8)gravel, medium plasticity, slow dilatancy (Fill) Very loose, wet, brown, Silty SAND (SM); Fine 2-3-1 53 S3 to medium sand, low plasticity fines, rapid (N=4)dilatancy (Fill) -509 At 10.0 grades to orange-brown, 2-0-0 73 S4 occurrence of trace, fine, angular gravel, (N=WOR) SM and slow dilatancy. 0-0-1 33 S5 (N=1)-504 Very dense, moist, gray and olive-brown, 15 CLAYEY GRAVEL with Sand (GC); Fine to 17-18-27 100 S5 Ă, coarse angular gravel, fine to coarse sand, (N=45)medium plasticity fines (Fill) Borehole completed at 16.5ft. below ground surface (bgs). 20 494 25 489



30

Project: Sweet Home WWTP Schematic Design Project Location: Sweet Home, OR Log of Boring B-3 Project Number: 5834.0 Geotechnical Date(s) Checked By K. Elliott Logged 05/02/2018 McMillen Jacobs Associates J. Irizarry Drilled Drilling Method/ Total Depth Mud Rotary/CME 75 Western States Soil Conservation, Inc. 12.5 ft Rig Type Contractor of Borehole Ground Surface Hole Diameter 4.00 in Hammer Weight/Drop (lb/in.)/Type 140 lb / 30 in / Automatic 529.9 ft Elevation/Datum Survey 7617007.93 E, 275477.18 N Elevation Source Site Survey PENETRATION BACKFILL INFORMATION ELEV. (FT) WATER LEVEL SAMPLE TYPE RECOVERY (%) **GRAPHIC LOG** DEPTH (FT) **RESISTANCE** SAMPLE# BLOW **USCS REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS** ∖и́с LL/Pl Soft, moist, gray with trace orange mottles, Sandy FAT CLAY (CH); High plasticity, medium toughness, fine sand, occasional organics (Fill) 3-2-1 53 S1 (N=3)СН At 5.0 feet grades to scattered woody 0-1-1 60 S2 organics. (N=2)Very loose, moist to wet, gray, CLAYEY SAND 1-1-1 100 S3 (SC); Fine to medium sand, medium plasticity (N=2)and medium toughness fines, occasional 1inch sandy lenses of slow dilatancy (Fill) -520 10 Medium dense, wet, red-brown, SILTY SAND 0-16-49 87 S4 (SM); Fine to medium sand, low plasticity fines, (N=65)slow dilatancy (Fill) At 11.0 feet very rough, Dark gray basalt chips in cuttings (Little Butte very slow drilling. 50/0" 0 Volcanic Series - Tholeiitic Basalt) S5 (Refusal) Borehole completed at 12.5ft. below ground surface (bgs). 515 15 -510 20 505 25 -500 30



Project: Sweet Home WWTP Schematic Design Log of Boring B-4 Project Location: Sweet Home, OR Project Number: 5834.0 Geotechnical Checked By K. Elliott Date(s) Logged J. Irizarry 05/01/2018 McMillen Jacobs Associates Drilled Drilling Method/ Total Depth Mud Rotary/CME 75 Western States Soil Conservation, Inc. 5.8 ft Rig Type Contractor of Borehole Ground Surface Hole Diameter 4.00 in Hammer Weight/Drop (lb/in.)/Type 140 lb / 30 in / Automatic 538.6 ft Elevation/Datum 7616842.35 E, 275514.57 N ocation Survey Coordinates Elevation Source Site Survey PENETRATION BACKFILL INFORMATION SAMPLE TYPE **GRAPHIC LOG** ELEV. (FT) WATER LEVEL RECOVERY (%) DEPTH (FT) **RESISTANCE** SAMPLE# BLOW COUNTS USCS **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS** ⊃ мс LL/PL Medium dense, moist to wet, dark gray, SILTY SAND with Gravel (SM); Fine to medium sand, fine angular gravel, low plasticity fines, slow dilatancy, scattered organics (Fill) SM 1 534/2 1 534/2018 1 7-10-7 20 S1 (N=17) At 4.5 feet very slow, Very dense, moist, gray and orange, SILTY very rough drilling. 28-50/3" GM 120 S2 GRAVEL with Sand (GM); Fine to coarse (Refusal) gravel, fine to coarse sand, medium plasticity Borehole completed at fines (Fill) 5.8ft. below ground surface (bgs). -529 10 -524 15 519 20 -514 25 -509 30



Boring B-4

Project: Sweet Home WWTP Schematic Design Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring B-5

Boring B-5

Date(s) Drilled 05/02/	2018		Geote	echnical ultant	IcMillen Jacobs	Associ	ates	Logged J. Irizarry		Checked By	K. Elliott
Drilling Method/ Rig Type	Mud Ro	tary and	HQ Wireline/C	ME 75	Drilling Contractor	Wester	n States	Soil Conservation, Inc.	Total Depth of Borehole 23.	O ft	
Hole Diameter	4.00 in				Hammer Weigh	t/Drop (II	b/in.)/Typ	pe 140 lb / 30 in / Automatic	Ground Surface Elevation/Datum	528.8 ft	
Location Sur	vey				Coordinates	761703	6.16 E, 2	275580.52 N	Elevation Source	Site Surv	еу
ELEV. (FT) WATER LEVEL DEPTH (FT)	RECOVERY (%)	SAMPLE#	BLOW	RES BLO	ETRATION ISTANCE DWS/FT VS/FT 40 60 80	GRAPHIC LOG	nscs	MATERIAL DESCR	RIPTION	BACKFILL INFORMATION	REMARKS AND TESTS
-524 <u></u> 5-	53	S1 S2	1-5-6 (N=11) 1-1-1				ML	Stiff, moist, gray, Sandy SILT (plasticity, medium toughness, coarse sand, occasional organical organical states of the same sand, occasional organical organical states of the same same same same same same same sam	fine sand, trace nics (Fill)		
10/29/201 9 /(/2016 1	73	S3	(N=2)			-	ML ML	SILT (ML); Low plasticity, low to medium sand, occasional organisms. Stiff, wet, gray and red-brown, (ML); Low plasticity, medium to the same street or the same street.	anics (Fill) Sandy SILT	0	
- 519 10	100	RUN 1	(N=15)					ine sand, trace medium and occasional organics (Fill) Run 1: 9.0 - 13.0 feet, RQD = very strong (R5), slightly weat moderately to highly fractured curved, smooth to rough, high horizontal narrow joints (Little Series - Tholeiitic Basalt) Run 2: 13.0-18.0 feet, RQD addition of irregular joints ar	75%; BASALT, hered to fresh, planar and angle and sub-Butte Volcanic = 86%,		At 8.5 feet, more difficulty drilling, driller remarks likely weathered rock. At 9.0 feet, switch to rock coring. RQD = 75%. From 11.7 feet to 12.4 feet UCS = 29,919 psi. At 13.0 feet RQD = 86%.
-514 15 - 	100	RUN 2						light blue green staining/coa occasional joints.	tting of		At 18.0 feet RQD = 74%.
-509 20 - 	100	RUN 3						Run 3: 18.0-23.0 feet, RQD to medium strong based on			From 18.0 feet to 19.0 feet UCS = 6,932 psi.
-504 25 - 											Borehole completed at 23ft. below ground surface (bgs).
-499 30 - - - - - - -	- NAII I	I I I I I I I I I I I I I I I I I I I				-					

Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-01

Boring P-01

Date(s) Drilled 04/30/2018	Geotechnical Consultant	AcMillen Jacobs	Millen Jacobs Associates Logged By J. Irizarry Checked By K. Ell					
Drilling Method/ Rig Type 4-1/4" Hollow ste		Drilling Contractor	Western	States	s Soil Conservation, Inc.	Total Depth of Borehole	0 ft	
Hole Diameter 4.25 in		Hammer Weigh	nt/Drop (lb/i	in.)/Typ	pe	Ground Surface Elevation/Datum	517.3 ft	
Location Survey		Coordinates	7617295.	47 E, 2	275346.48 N	Elevation Source Site Survey		ey
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE RECOVERY (%) SAMPLE #	SENUL BEON	ETRATION ISTANCE OWS/FT VS/FT 40 60 80	GRAPHIC LOG	nscs	MATERIAL DESCR	RIPTION	BACKFILL INFORMATION	REMARKS AND TESTS
-513			119	GM	Asphalt (Pavement) Dense, gray, Silty GRAVEL (GAggregate) Very soft to soft, moist, brown with Sand (CL); Low plasticity toughness, fine sand, trace moccasional organics (Fill) Very loose to loose, moist to v CLAYEY GRAVEL with Sand coarse angular basalt gravel, sand, low plasticity fines (Fill)	LEAN CLAY medium nedium sand, wet, gray brown, (GC); Fine to		At 9.0 feet driller remarks that the material stiffens. At 10.0 feet, driller remarks that the material feels like rock but the auger is able to continue to spin. Auger refusal at 11 feet. Borehole completed at 11ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on reactions while drilling.

Project: Sweet Home WWTP Schematic Design Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-02

Date(s) 05/01/2	018 - 01/05/2018	Geotechnical Consultant	IcMillen Jacobs	Associat	tes	Logged By J. Irizarry		Checked By	K. Elliott	
Drilling Method/ Rig Type	4-1/4" Hollow stem auge		Drilling Contractor	Western	States	s Soil Conservation, Inc.	Total Depth of Borehole	5 ft		
Hole Diameter	4.25 in		Hammer Weight	t/Drop (lb/	/in.)/Typ	pe	Ground Surface Elevation/Datum	526.1 ft		
Location Surve	э у		Coordinates	7617102	.54 E, 2	275424.97 N	Elevation Source Site Survey			
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE	SAMPLE #		ETRATION ISTANCE DWS/FT VS/FT 40 60 80	GRAPHIC LOG	nscs	MATERIAL DESCR		BACKFILL INFORMATION	REMARKS AND TESTS	
-522 - -522 - -521 - -517 - 517 - 512 - 512 - 507 - 502 - 502 - 						Soft to very soft, moist, brown Sandy LEAN CLAY with Grave plasticity, fine angular gravel, organics (Fill)	el (CL); Low		Auger refusal at 11.5 feet. Borehole completed at 11.5ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on reactions while drilling.	
	'									



Project: Sweet Home WWTP Schematic Design Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-03

Boring P-03

Drilling Method/ Rig Type 4-1/4" Hollow stem auger. Hole Diameter 4.25 in Location Survey	/CME 75	Drilling Contractor	Mastana		By 0. IIIZarry		Ву		
			western	States	Soil Conservation, Inc.	Total Depth of Borehole 16.0	ft		
Location Survey		Hammer Weigh	t/Drop (lb/i	in.)/Typ	20	Ground Surface	530.4 ft		
		Coordinates	7617036.	.27 E, 2	275432.54 N	Elevation Source Site Survey			
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE RECOVERY (%) SAMPLE # BLOW COUNTS		ETRATION ISTANCE DWS/FT vs/FT 40 60 80	GRAPHIC LOG	USCS	MATERIAL DESCR	IPTION	BACKFILL INFORMATION	REMARKS AND TESTS	
				CL	Asphalt (Pavement) Base rock (Fill) Very soft to soft, moist, dark gr CLAY with Sand (CL); Fine san Very soft to soft, moist to wet, LEAN CLAY (CL); Medium pla medium sand (Fill) At 13.0 feet, grades to brown decrease in sand content.	gray, Sandy sticity, fine to		Auger refusal at 16 feet. Borehole completed at 16ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on reactions while drilling.	

Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-04

Boring P-04

Date(s) Drilled 04/30/2018 - 04/30	0/2018 Geotechnical Consultant	IcMillen Jacobs Ass	ociates	Logged By J. Irizarry		Checked By	K. Elliott	
Duilling Mathad/	llow stem auger/CME 75	Drilling Wes	stern States	Soil Conservation, Inc.	Total Depth of Borehole 7.5 f	•		
Hole Diameter 4.25 in		Hammer Weight/Dro	pp (lb/in.)/Typ	pe	Ground Surface Elevation/Datum			
Location Survey		Coordinates 761	6984.89 E, 2	275442.28 N	Elevation Source Site Survey			
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE RECOVERY (%)	# RES BLO BLOV BLOV	STRATION CONTROL OF CO	USCS	MATERIAL DESCR	RIPTION	BACKFILL INFORMATION	REMARKS AND TESTS	
	S1 S2		OL/ OH	Asphalt - 5" thick (Pavement) Dense, moist, gray Silty GRAV coarse angular gravel, low pla inches thick (Base Aggregate) Very soft to soft, moist, dark gr CLAY (CL); (Fill) Hard, moist, brown, Organic S (OL/OH); Frequent hard wood Very soft to soft, moist, green- brown-orange mottles, LEAN or plasticity, fine sand, trace med	sticity silt, 5 ray-brown, LEAN Soil with Sand in clay (Fill) brown with CLAY (CL); Low		At 2.5 feet driller remarks that material becomes stiffer, woody material began smoking so driller added water to hole, very slow drilling. Organic chemical odor in the wood fiber of Sample 1. Auger refusal at 7.5 feet. Borehole completed at 7.5ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on reactions while drilling.	

Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-05

Date(s) Drilled 05/01/2018 - 05/01/2018	Geotechnical Consultant	IcMillen Jacobs	Associa	ites	Logged By J. Irizarry		Checked By	K. Elliott
Drilling Method/ Rig Type 4-1/4" Hollow stem auge	r/CME 75	Drilling Contractor	Western	n States	s Soil Conservation, Inc.	Total Depth of Borehole 3.5 ft		
Hole Diameter 4.25 in		Hammer Weigh	t/Drop (lb	/in.)/Ty	ре	Ground Surface Elevation/Datum 533.1 ft		
Location Survey		Coordinates	7616928	3.58 E,	275458.29 N	Elevation Source	Site Surv	еу
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE RECOVERY (%) SAMPLE #		ETRATION ISTANCE DWS/FT VS/FT 40 60 80	GRAPHIC LOG	nscs	MATERIAL DESCF		BACKFILL INFORMATION	REMARKS AND TESTS
-529	LL/PL			CL	Very soft to soft, moist, brown LEAN CLAY with Sand (CL); I medium toughness, fine sand angular gravel, occasional org	ow plasticity, trace coarse		Auger refusal at 3.5 feet. Borehole completed at 3.5ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on reactions while drilling.



Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-06

Boring P-06

Date(s) Drilled 04/30/2018 - 04/30/2018 Geotechnical Consultant	McMillen Jacobs	s Associat	tes	Logged J. Irizarry	C	Checked	K. Elliott
Drilling Method/ Rig Type 4-1/4" Hollow stem auger/CME 75	Drilling Contractor	Western	States	s Soil Conservation, Inc.	Total Depth of Borehole 3.5 ft	-	
Hole Diameter 4.25 in	Hammer Weigh	ht/Drop (lb/	/in.)/Ty	pe	Ground Surface Elevation/Datum 5	36.2 ft	
Location Survey	Coordinates	7616858.	.52 E,	275586.88 N	Elevation Source S	ite Surv	ey
7. (FT) 7. (EVE) 7. (FT) 7. (FT) 8. (FT) 8. (FT) 8. (FT) 9. (F	ETRATION SISTANCE COWS/FT WS/FT 40 60 80	GRAPHIC LOG	nscs	MATERIAL DESCR		BACKFILL INFORMATION	REMARKS AND TESTS
S1 S1 S32 S S S S S S S S S S S S S S S S S S			CL	Stiff, moist to wet, brown, LEA Low plasticity, medium toughn medium sand, fine gravel, occ organics (Fill)	IN CLAY (CL); less, trace fine to easional woody	11/2 11/2	Auger refusal at 3.5 feet. Borehole completed at 3.5ft. below ground surface (bgs).
-527 - 10							Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on reactions while drilling.
-517							
-512							
-507							

Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-07

Date(s) Drilled 04	1/30/2	018		G	eotechnical onsultant	IcMillen Jacobs	s Associa	ates	Logged By J. Irizarry	ı	Checked By	K. Elliott	
Drilling Meth Rig Type	od/	4-1/4" H	lollow s	tem auger/C	ME 75	Drilling Contractor	Wester	n States	s Soil Conservation, Inc.	Total Depth of Borehole 2.5 f	-		
Hole Diamet	er	4.25 in				Hammer Weigh	nt/Drop (It	o/in.)/Ty	pe	Ground Surface Elevation/Datum 538.0 ft			
Location	Surve	Э у				Coordinates	761678	9.32 E,	275532.46 N	Elevation Source	Site Surv	еу	
ELEV. (FT) WATER LEVEL DEPTH (FT)	SAMPLE TYPE	RECOVERY (%)	SAMPLE#	BLOW	RES BLO	ETRATION ISTANCE DWS/FT VS/FT 40 60 80	GRAPHIC LOG	nscs	MATERIAL DESCF		BACKFILL INFORMATION	REMARKS AND TESTS	
- 	-							GM	Very loose to loose, moist, da GRAVEL with Sand (GM); And coarse gravel with cobbles, lo (Fill)	gular fine to		Auger refusal at 2.5 feet	
- - - -533 5	-											Borehole completed at 2.5ft. below ground surface (bgs).	
- - - - - - - -	-						-					Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on reactions while drilling.	
	-						- - - -						
- - - - -	-												
518 20 -													
	-												
- - -	-												
-508 30 -													



Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-08

Boring P-08

Date(s) Drilled O4/30/2018 Geotech Consulta		Associates	Logged By J. Irizarry	Checked By	K. Elliott	
Drilling Method/ Rig Type 4-1/4" Hollow stem auger/CME 75	Deilling	Western States Soil Conservat	Total De	Total Depth of Borehole 7.5 ft		
Hole Diameter 4.25 in		nt/Drop (lb/in.)/Type	Ground	Ground Surface Elevation/Datum 535.2 ft		
Location Survey	Coordinates	7616905.96 E, 275577.01 N		n Source Site Sur	vey	
ELEV. (FT) WATER LEVE DEPTH (FT) SAMPLE TYP RECOVERY (9 SAMPLE # BLOW COUNTS	PENETRATION RESISTANCE BLOWS/FT BLOWS/FT 20 40 60 80 T T T T T T T T T T T T T T T T T T T	GRAPH	FERIAL DESCRIPTIO	BAC		
5 0		Very loose to GRAVEL with angular to su	loose, moist, dark brown Sand (GM); Fine to coabangular gravel, fine to sticity fines, occasional)	n, SILTY arse coarse woody		
-511						

Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-09

Boring P-09

A gray around, scattered organics Columbia Columbi	Inter Members 4.4 in Medicary statem augmor/CME 75 Committed Control (Control (Con	Date(s) Drilled 05/01/2018	1115011 000-	Geotechnical .	McMillen Jacobs	s Associat	tes	Logged J. Iriza	arry	Checked	K. Elliott	
Hammer Weight/Drog (Drin,Trype Clark Stating Control Stating	The complete of the complete o		" Hollow stem auge	Consultant	Drilling			БУ	Total Depth	By Oft		
Coordinate Coo	State Survey Coordinates 7664964.80 E, 275994.59 N Elevation Source Site Survey							·	Ground Surface			
PENETRATION RESISTANCE BLOWS/FT 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	PENETRATION RESISTANCE BLOWNSFT BEOWEST PLAN CLAY with LIPL WHITE PLAN CLAY with LIPL									Elevation/Datum		
RESISTANCE BLOWS/FT B	RESISTANCE DELOWISH TO DELOWISH TO DELOWISH TESTS RESISTANCE DELOWISH TO DELOWISH TESTS MAINTERIAL DESCRIPTION MATERIAL DESCRIPTION MATERIAL DESCRIPTION DELOWISH TESTS REMARKS AND TESTS REMARKS AN						.80 E,	2/5594.59 N	Elevation Source	Site Surv	ey	
Borehole completed at 4ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative densit and apparent consistency based on reactions while drilling.	Borehole completed at 4ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative densit and apparent consistency based on reactions while drilling.	ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE RECOVERY (%)	SAMPLE # BLOW	RES BLO BLOV MC	SISTANCE OWS/FT	ATION 5 7 7 7 7 7 7 7 7 7					AND TESTS	
		-521521521	S1				CL	Gravel (CL); Low plasticity sand, fine angular gravel,	y, fine to coarse		Auger refusal at 4 feet. Borehole completed at 4ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on	

Project: Sweet Home WWTP Schematic Design Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-10

Boring P-10

Date(s) 04/30/2018			chnical	MaMillar	laaah	s Associa	otoo	Logged J. Irizarry		Checked	K. Elliott
Drilled 04/06/2016		Consu		Drilling				By S. H.Zuri y	Total Danth	у	K. Elliott
Rig Type 4-1/4	" Hollow stem a	uger/CME	75	Contra	ctor	Wester	n States	Soil Conservation, Inc.	of Borehole 5.5 II	i	
Hole Diameter 4.25	in			Hamm	er Weig	ht/Drop (It	o/in.)/Typ	ne e	Ground Surface Elevation/Datum	530.1 ft	
Location Survey				Coordi	nates	761696	3.18 E, 2	275547.22 N	Elevation Source	Site Surv	ey
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE RECOVERY (%)	SAMPLE#	BLOW	RES BL	IETRA SISTAI LOWS/ DWS/FT 40 60 T T	NCE FT	GRAPHIC LOG	SOSO	MATERIAL DESCR		BACKFILL INFORMATION	REMARKS AND TESTS
-526 -5								Very loose to loose, moist, bro GRAVEL with Sand (GM); Fingravel, fine and coarse sand, (Fill) Very soft to soft, wet, gray-bro with Sand (CL); Low plasticity, angular fine angular gravel. (F	e to coarse low plasticity. wn, LEAN CLAY fine sand, trace		Auger refusal at 5.5 feet. Borehole completed at 5.5ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on reactions while drilling.
	•										

Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-11

Boring P-11

Date(s) 04/3	0/2018 - 0	4/30/2018	Geo	technical sultant	McMillen	Jacob	s Associa	ates	Logged By J. Irizarry		Checked By	K. Elliott
Drilling Method Rig Type	l/ 4-1/4'	' Hollow s	tem auger/CM		Drilling Contract	tor	Wester	n States	Soil Conservation, Inc.	Total Depth of Borehole 5.5 f		
Hole Diameter		n					ht/Drop (It	o/in.)/Typ	pe	Cround Surface	529.8 ft	
Location Su	ırvey				Coordin	ates	761700	7.94 E,	275561.88 N		Site Surv	еу
ELEV. (FT) WATER LEVEL DEPTH (FT)	SAMPLE TYPE RECOVERY (%)	SAMPLE#	BLOW	PENETRATION RESISTANCE BLOWS/FT BLOWS/FT 20 40 60 80 H 1 1 1 MC LL/PL			GRAPHIC LOG	SOSO	MATERIAL DESC		BACKFILL INFORMATION	REMARKS AND TESTS
-520 10 - -515 15 - -550 20 - -500 30 - -500 30 -		S1						GC	Very loose to loose, moist to brown, Clayey GRAVEL with to coarse gravel, fine to coarse	Sand (GC); Fine		Auger refusal at 5.5 feet. Borehole completed at 5.5ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on reactions while drilling.

Project: Sweet Home WWTP Schematic Design Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-12

Boring P-12

Date(s) 04/30/2	2018	Geotechnic	al McMillen Jacobs	s Associa	ates	Logged J. Irizarry		Checked	K. Elliott		
Drilling Method/		Consultant stem auger/CME 75	Drilling Contractor			By J. Irizarry	Total Depth of Borehole 5.0 ft	, y			
Rig Type		atem augen/CIVIC /5					Ground Surface				
Hole Diameter	4.25 in		Hammer Weigh				Elevation/Datum	26.4 ft	REMARKS AND TESTS Auger refusal at 5 feet. Borehole completed at 5ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration.		
Location Surv	<u> </u>	<u> </u>	Coordinates	761707	0.84 E, 2	275609.56 N	Elevation Source \$	Site Surv	ey		
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE	RECOVERY (%)	STNUC SUOW		GRAPHIC LOG	sosn	MATERIAL DESCR		BACKFILL INFORMATION	AND TESTS		
-522	, S1				CL	Very soft to soft, moist, brown, CLAY with Sand (CL); Low pla coarse angular gravel, fine to	Gravelly LEAN sticity, fine to coarse sand (Fill)		Borehole completed at 5ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on		
-512											
-502											

Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-13

Boring P-13

Date(s) Drilled 04/30/2018 - 04/3	Geotechnical Consultant	AcMillen Jacobs	Associates	Logged J. Irizarry		Checked	K. Elliott		
Drilling Mathod/	ollow stem auger/CME 75	Drilling Contractor	By 3. IIIZarry s Soil Conservation, Inc.	Total Donth	By 5 ft				
Hole Diameter 4.25 in		Hammer Weigh	pe	Ground Surface Elevation/Datum	523.6 ft				
Location Survey		Coordinates	7617121.16 E,	275620.34 N	Elevation Source	Site Surv	Se ft Survey REMARKS AND TESTS		
——————————————————————————————————————	PENE	TRATION	(7)			Z			
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE RECOVERY (%)	# RES MDI BLOW	ISTANCE OWS/FT VS/FT 40 60 80	GRAPHIC LOG USCS	MATERIAL DESC			AND TESTS		
-519519514			CL	Very soft to soft, moist to wet gray brown, Sandy LEAN CL (CL); Low plasticity, fine to co angular gravel (Fill)	AY with Gravel		Auger refusal at 8.5 feet. Borehole completed at 8.5ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on		

Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-14

Boring P-14

Date(s) Drilled 05/01/2	018	Geo	otechnical nsultant M	cMillen Jacobs	Associa	ites	Logged J. Irizarry		Checked By	K. Elliott		
Drilling Method/ Rig Type	4-1/4" Hollow		AE 75	Drilling Contractor	Western	n States	s Soil Conservation, Inc.	Total Depth of Borehole 5.5 f				
Hole Diameter	4.25 in			Hammer Weigh	ıt/Drop (lb	/in.)/Typ	pe	Cround Surface	521.7 ft			
Location Surve	еу			Coordinates	7617149	9.97 E, 2	275571.01 N	Elevation Source	Site Surv	еу		
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE	RECOVERY (%) SAMPLE #	BLOW	RESI BLO	TRATION ISTANCE DWS/FT /S/FT 10 60 80	GRAPHIC LOG	nscs	MATERIAL DESCR		BACKFILL INFORMATION	REMARKS AND TESTS		
-517 5	S1					CL	Very soft to soft, wet, gray-bro CLAY with Gravel (CL); Low p coarse sand, fine angular grav	lasticity, fine to	_	Observed groundwater at approximately 4.0 feet water present, difficult to measure due to gravel. Auger refusal at 5.5 feet. Borehole completed at 5.5ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on reactions while drilling.		

Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-15

Boring P-15

Date(s) Drilled O4/30/2018 Geotechnical Consultant Co											Checked	K. Elliott			
Drilled Drilling Metho	od/ 4	I-1/4" H	ollow st	tem auger/CME	ullarit	Drilling			s Soil Conservation, Inc.	Total Depth 2.	By 5 ft				
Rig Type Hole Diamete		1.25 in		- -		Contractor Hammer Weigh				Ground Surface	519.1 ft				
	Survey					Coordinates			275628.88 N	Elevation/Datum Elevation Source		REMARKS AND TESTS Auger refusal at 2.5 feet. Borehole completed at 2.5ft. below ground surface (bgs). Grab samples obtained from auger cuttings during exploration. Reported relative density and apparent consistency based on			
								4.34 E,	273020.00 N	Elevation Source					
ELEV. (FT) WATER LEVEL DEPTH (FT)	SAMPLE TYPE	RECOVERY (%)	SAMPLE#	BLOW	RES BLO	ETRATION ISTANCE OWS/FT VS/FT 40 60 80	GRAPHIC LOG	nscs	MATERIAL DESCF			AND TESTS			
- · · · · · · · · · · · · · · · · · · ·	- - -							GC	Very loose to loose, moist to v CLAYEY GRAVEL with Sand angular gravel, fine to coarse	(GC); Fine		Auger refusal at 2.5 feet.			
\times 1507/5018 \times 5	- - - -											2.5ft. below ground			
	- - - - -						-					from auger cuttings during exploration. Reported relative density and apparent			
-510 - - 10 - 	- - - - -														
	-														
	-														
	- - - - - - -														
- · · · · · · · · · · · · · · · · · · ·	- - - - - - -						-								
-490 - - 30 - 	- - - - -														

Project: Sweet Home WWTP Schematic Design Log of Boring P-16 Project Location: Sweet Home, OR Project Number: 5834.0 Geotechnical Checked J. Quinn Date(s) Logged L. Ferguson 10/29/2019 McMillen Jacobs Associates Drilled Total Depth of Borehole Drilling Method/ Drilling Air Track Probe/Furukawa HCR900 McCallum Rock Drilling 26.0 ft Rig Type Contractor Ground Surface Hole Diameter Hammer Weight/Drop (lb/in.)/Type N/A 517.1 ft 3.00 in Elevation/Datum Sweet Home WWTP 7617309.79 E, 275352.81 N Elevation Source Site Survey PENETRATION BACKFILL INFORMATION SAMPLE TYPE **GRAPHIC LOG** ELEV. (FT) WATER LEVEL DEPTH (FT) RECOVERY (%) **RESISTANCE** SAMPLE# BLOW COUNTS USCS **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS** 20 ⊃ мс LL/PL Asphalt (Pavement) Soft, moist, brown, CLAY (CL) (Fill) -513 CL -508 10 BASALT, hard, gray (Little Butte Volcanic Series - Tholeiitic Basalt) 503 15 498 20 493 25 Borehole completed at 26ft. below ground surface (bgs). 488 30 **Boring P-16**

Project: Sweet Home WWTP Schematic Design Log of Boring P-17 Project Location: Sweet Home, OR Project Number: 5834.0 Geotechnical Checked J. Quinn Date(s) Drilled Logged L. Ferguson 10/29/2019 McMillen Jacobs Associates Total Depth of Borehole Drilling Method/ Drilling Air Track Probe/Furukawa HCR900 McCallum Rock Drilling 11.0 ft Rig Type Contractor Ground Surface Hole Diameter Hammer Weight/Drop (lb/in.)/Type N/A 3.00 in 516.4 ft Elevation/Datum Sweet Home WWTP 7617358.56 E, 275414.92 N Elevation Source Site Survey PENETRATION ELEV. (FT) WATER LEVEL DEPTH (FT) BACKFILL INFORMATION SAMPLE TYPE **GRAPHIC LOG** RECOVERY (%) **RESISTANCE** SAMPLE# BLOW COUNTS USCS **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS** ⊃ мс LL/PL Asphalt (Pavement) Soft, moist, brown, CLAY (CL) (Fill) CL -512 Basalt, hard, gray (Little Butte Volcanic Series Tholeiitic Basalt) 507 Borehole completed at 11ft. below ground surface (bgs). 15 497 20 492 25



487

30

Boring P-17

Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-18

Boring P-18

Date(s) Drilled 10/29/2019 Geotechnical Consultant McMillen Jacobs Associates Logged By L. Ferguson By J. Quinn											
Drilling Mothod/	ck Probe/Furukawa H		Drilling Contractor	McCallu	ım Roc	k Drilling	Total Depth of Borehole	.0 ft			
Hole Diameter 3.00 in			Hammer Weigh				Ground Surface Elevation/Datum	518.5 ft			
Location Sweet Home	WWTP		Coordinates			275451.86 N	Elevation Source	Site Surv	ey		
		PENE	TRATION					7			
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE RECOVERY (%)	SAMPLE # BLOW COUNTS	RES BLO	ISTANCE DWS/FT	GRAPHIC LOG	nscs	MATERIAL DESCR		BACKFILL INFORMATION	REMARKS AND TESTS		
-504504					CL	BASALT, hard, gray (Little But Series - Tholeiitic Basalt)		_			
- 15									Borehole completed at 15ft. below ground surface (bgs).		

Project: Sweet Home WWTP Schematic Design Log of Boring P-19 Project Location: Sweet Home, OR Project Number: 5834.0 Geotechnical Checked J. Quinn Date(s) Logged L. Ferguson 10/29/2019 McMillen Jacobs Associates Drilled Total Depth of Borehole Drilling Method/ Drilling Air Track Probe/Furukawa HCR900 McCallum Rock Drilling 20.0 ft Rig Type Contractor Ground Surface Hole Diameter Hammer Weight/Drop (lb/in.)/Type N/A 3.00 in 518.0 ft Elevation/Datum Sweet Home WWTP 7617228.61 E, 275499.48 N Elevation Source Site Survey PENETRATION BACKFILL INFORMATION SAMPLE TYPE **GRAPHIC LOG** ELEV. (FT) WATER LEVEL DEPTH (FT) RECOVERY (%) **RESISTANCE** SAMPLE# BLOW COUNTS USCS **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS** 20 ⊃ мс LL/PL Asphalt (Pavement) GM Medium dense, gray, silty GRAVEL (GM) (Fill) Soft, moist, brown, CLAY (CL) (Fill) CL -514 BASALT, hard, gray (Little Butte Volcanic Series - Tholeiitic Basalt) -509 10 -504 15 499 20 Borehole completed at 20ft. below ground surface (bgs). 494 25 489 30



Project: Sweet Home WWTP Schematic Design Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-20

Boring P-20

Deling Methods Air Track Probe/Furukawa HCR900 Contractor McCallum Rock Drilling For Expension of Elevation Source Site Survey Hole Diameter 3.00 in Hammer Weight/Drop (blin)Type NA Graund Surface Elevation Source Site Survey Coordinates 7617111.38 E, 275430.15 N Elevation Source Site Survey Coordinates	Date(s) 10/29/2019 Drilled		Geotechnical Consultant	/IcMillen Jacobs	Associat	es	Logged L. Ferguso	n	Checked By	J. Quinn
Hammer WeightDrop (blink)Type N/A Ground Starter SEAL R Elevation Statum SEAL R		ack Probe/Furuka	awa HCR900	Drilling Contractor	McCallur	n Rock	- T	Total Depth of Borehole 20		
PENETRATION PESISTANCE SUMMER STANDS PENETRATION PESISTANCE SUMMER STANDS PENETRATION PESISTANCE PENETRATION PESISTANCE PENETRATION PESISTANCE PENETRATION PESISTANCE PENETRATION PENETRAT		1			ıt/Drop (lb/i	in.)/Typ	pe N/A	Ground Surface	525.4 ft	
Soft, moist, brown, CLAY (CL) (Fill) CL BASALT, hard, gray (Little Butte Volcanic Series - Tholeitic Basalt) Basalt, hard, gray (Little Butte Volcanic Series - Tholeitic Basalt) Basalt, hard, gray (Little Butte Volcanic Series - Tholeitic Basalt) Basalt, hard, gray (Little Butte Volcanic Series - Tholeitic Basalt) Basalt, hard, gray (Little Butte Volcanic Series - Tholeitic Basalt) Basalt, hard, gray (Little Butte Volcanic Series - Tholeitic Basalt) Basalt, hard, gray (Little Butte Volcanic Series - Tholeitic Basalt) Basalt, hard, gray (Little Butte Volcanic Series - Tholeitic Basalt) Basalt, hard, gray (Little Butte Volcanic Series - Tholeitic Basalt)	Location Sweet Home WWTP Coordinates				7617111.	38 E, 2	75430.15 N	Elevation Source	Site Surv	әу
621 - 621 -	ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE RECOVERY (%)	SAMPLE #	STANDOD BROWN MC	ISTANCE OWS/FT	GRAPHIC LOG				BACKFILL INFORMATION	AND
-496 - 30	-5					CL	BASALT, hard, gray (Little But			

Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-21

Boring P-21

Date(s) Drilled 10/29/2019 Geotechnical Consultant McMillen Jacobs Associates Logged By L. Ferguson By Checked By J. Quinn										
Drilling Method/ Rig Type Air Track Probe/Furukawa HCR900	Drilling Contractor	McCallum Rock I		Total Depth of Borehole 23.	0 ft					
Hole Diameter 3.00 in		/Drop (lb/in.)/Type		Ground Surface Elevation/Datum	521.8 ft					
Location Sweet Home WWTP	Coordinates	7617111.78 E, 27	5548.31 N	Elevation Source	Site Survey					
EV. (FT) ER. LEVE TH (FT) PLE TYP NVERY (% SLOW DUNTS RES	ETRATION SISTANCE OWS/FT WS/FT 40 60 80	GRAPHIC LOG USCS	MATERIAL DESCR		BACKFILL INFORMATION CASAL CAS					
-517 5		GM	BASALT, hard, gray (Little Butt Series - Tholeiitic Basalt)		Borehole completed at 23ft. below ground surface (bgs).					

Project: Sweet Home WWTP Schematic Design Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-22

Boring P-22

Committee Comm	ate(s) 10/29/2019 Geotechnical Consultant	McMillen Jacobs	s Associates	Logged By L. Ferguson	Checked By	J. Quinn
Name		Drilling Contractor	McCallum Rock Drilling			
			nt/Drop (lb/in.)/Type N/A	Grour	nd Surface 519 5 ft	
Soft, most, brown, slightly sandy CLAY (CL) (Fil) BASALT, hard, gray (Little Butte Volcanic Series - Tholeitito Basalt) Soft series - Tholeitito Basalt) Borehole completed at 20th below ground surface (bgs).	ocation Sweet Home WWTP	Coordinates	7617179.82 E, 275578.51 N			ey
514 5- 509 10- 509 10- 150 15- 499 20- 494 25- 498 1-	SAMPLE TYP SAMPLE TYP SAMPLE TYP SAMPLE TYP SAMPLE TYP CCOUNTS COUNTS MATER LEVE DEPTH (FT)	SISTANCE LOWS/FT				AND
	514 5		Soft, moist, (Fill) CL BASALT, ha Series - Tho	rd, gray (Little Butte Vo	DICANIC	20ft. below ground

Project: Sweet Home WWTP Schematic Design Log of Boring P-23 Project Location: Sweet Home, OR Project Number: 5834.0 Geotechnical Checked J. Quinn Date(s) Logged L. Ferguson 10/29/2019 McMillen Jacobs Associates Drilled Total Depth of Borehole Drilling Method/ Drilling Air Track Probe/Furukawa HCR900 McCallum Rock Drilling 23.0 ft Rig Type Contractor Ground Surface Hole Diameter Hammer Weight/Drop (lb/in.)/Type N/A 3.00 in 530.4 ft Elevation/Datum Sweet Home WWTP 7617036.68 E, 275432.95 N Elevation Source Site Survey PENETRATION BACKFILL INFORMATION SAMPLE TYPE **GRAPHIC LOG** ELEV. (FT) WATER LEVEL DEPTH (FT) RECOVERY (%) **RESISTANCE** SAMPLE# BLOW COUNTS USCS **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS** 20 ⊃ мс LL/PL Asphalt (Pavement) GM Medium dense, gray, slightly silty GRAVEL (GM) (Fill) Soft, moist, brown, CLAY (CL) (Fill) 526 CL -521 -516 15 BASALT, hard, gray (Little Butte Volcanic Series - Tholeiitic Basalt) -511 20 There was no return of rock chips until 22 feet bgs. Driller commented that the hole was being plugged by dirt and preventing return of rock fragments. Borehole completed at 23ft. below ground 25 surface (bgs).



501

30

Boring P-23

Project: Sweet Home WWTP Schematic Design Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-24

Boring P-24

Date(s) Drilled Drilled Doublets Doublets Doublets Date(s) Drilled Dri							ates	Logged L. Fergus	son	Checked By	J. Quinn
Drilling Method/ Rig Type	Air Track	Probe/	Furukawa HCF	R900	Drilling Contractor	McCall	um Roc	k Drilling	Total Depth of Borehole	.0 ft	
Hole Diameter	3.00 in				Hammer Weigh	nt/Drop (It	o/in.)/Ty	pe N/A	Ground Surface Elevation/Datum	529.7 ft	
Location Swe	et Home W	WTP			Coordinates	761700	8.03 E,	275578.53 N	Elevation Source	Site Surv	ey
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE	RECOVERY (%)	SAMPLE#	BLOW COUNTS	RES BLOV BLOV 20 4	ETRATION ISTANCE OWS/FT VS/FT 40 60 80	GRAPHIC LOG	nscs	MATERIAL DESC	RIPTION	BACKFILL INFORMATION	REMARKS AND TESTS
525 5	RE			MC LL/PL			GM	Loose, gray, silty GRAVEL (d. BASALT, hard, gray (Little Biseries - Tholeiitic Basalt)			Driller commented that it got wet around 5 feet bgs. Borehole completed at 15ft. below ground surface (bgs).
	MILL	FNI									

Project: Sweet Home WWTP Schematic Design Project Location: Sweet Home, OR

Project Location: Sweet Home, OK Project Number: 5834.0

Log of Boring P-25

Boring P-25

Sheet 1 of 1

Geotechnical Checked J. Quinn Date(s) Logged L. Ferguson 10/29/2019 McMillen Jacobs Associates Drilled Total Depth of Borehole Drilling Method/ Drilling Air Track Probe/Furukawa HCR900 McCallum Rock Drilling 15.0 ft Rig Type Contractor Ground Surface Hole Diameter Hammer Weight/Drop (lb/in.)/Type N/A 3.00 in 506.4 ft Elevation/Datum 7617287.27 E, 275672.91 N Pleasant Valley Boat Ramp Elevation Source Site Survey PENETRATION BACKFILL INFORMATION SAMPLE TYPE **GRAPHIC LOG** ELEV. (FT) WATER LEVEL DEPTH (FT) RECOVERY (%) **RESISTANCE** SAMPLE# BLOW USCS **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS** ⊃ мс LL/PL Asphalt (Pavement) Medium dense, gray, silty GRAVEL (GM) (Fill) BASALT, hard, gray (Little Butte Volcanic Series - Tholeiitic Basalt) -502 497 492 15 Borehole completed at 15ft. below ground surface (bgs). 487 20 482 25 30

Project Location: Sweet Home, OR Project Number: 5834.0

Log of Boring P-26

Boring P-26

Date(s) Drilled 10/29/2019 Drilling Method/ Rig Type Air Track Probe/Furuka Hole Diameter 3.00 in Location Pleasant Valley Boat Ramp	Consultant	Drilling Contractor		By L. Ferguso	Total Donth	Ву	J. Quinn		
Hole Diameter 3.00 in		Contractor			23 () ft			
		Hammer Mei-	tractor of Borehole 23.0 ft Ground Surface 507.2 ft						
Location Blackant Valley Boot Bown					Elevation/Datum 507.2 π				
Location Fleasant Valley Boat Ramp		Coordinates	7617279.85 E, 2	275716.52 N	Elevation Source	Site Surv	ey		
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE RECOVERY (%) SAMPLE #		ETRATION SISTANCE OWS/FT WS/FT 40 60 80	GRAPHIC LOG USCS	MATERIAL DESCR		BACKFILL INFORMATION	REMARKS AND TESTS		
			GM	Medium dense, gray, silty GR. BASALT, fractured, gray (Little Series - Tholeiltic Basalt) BASALT, hard, gray (Little But Series - Tholeiltic Basalt)	Butte Volcanic		Driller comments that between 5 and 12 feet bgs seemed like highly fractured rock. Borehole completed at 23ft. below ground surface (bgs).		

Project: Sweet Home WWTP Schematic Design Log of Boring P-27 Project Location: Sweet Home, OR Project Number: 5834.0 Geotechnical Checked J. Quinn Date(s) Logged L. Ferguson 10/29/2019 McMillen Jacobs Associates Drilled Total Depth of Borehole Drilling Method/ Drilling Air Track Probe/Furukawa HCR900 McCallum Rock Drilling 26.0 ft Rig Type Contractor Ground Surface Hole Diameter Hammer Weight/Drop (lb/in.)/Type N/A 3.00 in 516.6 ft Elevation/Datum Sweet Home WWTP 7617372.72 E, 275312.04 N Elevation Source Site Survey PENETRATION BACKFILL INFORMATION SAMPLE TYPE **GRAPHIC LOG** ELEV. (FT) WATER LEVEL DEPTH (FT) RECOVERY (%) **RESISTANCE** SAMPLE# BLOW COUNTS USCS **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS** 20 ⊃ мс LL/PL Concrete (Pavement) Soft, moist, brown, slightly sandy CLAY (CL) (Fill) -512 CL 507 10 BASALT, hard, gray (Little Butte Volcanic Series - Tholeiitic Basalt) 15 20 492 25 Borehole completed at 26ft. below ground surface (bgs). 487



30

Boring P-27

Project: Sweet Home WWTP Final Design Review **Project Location:** Log of Boring P-28 Project Number: 6367.0 Date(s) Drilled Geotechnical Consultant Logged 06/20/2022 - 06/20/2022 J. Quinn McMillen Jacobs Associates A. Judy Drilling Method/ Drilling Contracto Total Depth of Borehole 8.25" Hollow Stem Auger/CME 75 PLI Systems, Inc. 11.2 ft Rig Type Ground Surface Elevation/Datum Hole Diameter 8.25 in Hammer Weight/Drop (lb/in.)/Type 140 lb / 30 in / Automatic 516.5 ft See Figure 2 Site Plan 7617342.00 E. 275383.00 N Flevation Source Location Coordinates Site Survey **PENETRATION** BACKFILL INFORMATION ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE GRAPHIC LOG RESISTANCE BLOW RECOVERY SAMPLE **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS**) мс LL/PL Soft, moist, brown, SILT (ML); low plasticity, trace fine gravel, trace fine sand, trace wood fibers. (Fill) -512 67 SPT-ML Becomes gray mottled at 5.5 feet. (N=2)100 G-1 -507 10 - 11 feet: sample consists of approx. 50% 1-7-50/3" 64 SPTwood fibers by volume. (Refusal) Auger refusal at 11.25 feet on basalt bedrock Borehole completed at 11.25ft. below ground surface (bgs). -502 15 -497 20 492 25 30



Project: Sweet Home WWTP Final Design Review Log of Boring P-29 **Project Location:** Project Number: 6367.0 Geotechnical Consultant Logged Checked By 06/20/2022 - 06/20/2022 J. Quinn McMillen Jacobs Associates A. Judy Drilled Drilling Drilling Method/ Total Depth 8.25" Hollow Stem Auger/CME 75 PLI Systems, Inc. 19.0 ft Rig Type Ground Surface Hole Diameter 8.25 in Hammer Weight/Drop (lb/in.)/Type 140 lb / 30 in / Automatic 517.3 ft Elevation/Datum See Figure 2 Site Plan 7617301.00 E. 275320.00 N Flevation Source Location Coordinates Site Survey **PENETRATION** BACKFILL INFORMATION ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE GRAPHIC LOG **RESISTANCE** BLOW RECOVERY SAMPLE **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS**) мс LL/PL Asphalt - 5" thick (Pavement) Base Aggregate - 3" thick (Fill) Soft, moist, brown, SILT (ML); low plasticity, trace fine gravel, trace fine sand, trace wood fibers. (Fill) 100 G-1 -513 ₩ 1-0-2 MI 20 SPT-(N=2)1 100 G-2 -508 Auger grinding below 10 -Becomes wet at 10 feet. 0-7-26 15 feet. SPT-33 BASALT; very weak, dark brown, highly weathered (N=33)100 to decomposed (Little Butte Volcanic Series -G-3 Tholeiitic Basalt) 50/12" SPT-70 (Refusal) -503 Smooth, slow drilling below 15 26-50/5" Penetration rate decreases significantly at 15 98 SPT-15 feet. (Refusal) feet; stronger rock inferred below this depth. 4 12-23-33 87 SPT-Becomes dark blue-gray at 18 feet. (N=56)498 Borehole completed at 20 19ft. below ground surface (bgs). 493 25 488 30 **Boring P-29**

Project: Sweet Home WWTP Final Design Review **Project Location:** Log of Boring P-30 Project Number: 6367.0 Date(s) Drilled Geotechnical Consultant Checked By Logged 06/20/2022 - 06/20/2022 J. Quinn McMillen Jacobs Associates A. Judy Drilling Method/ Drilling Contractor Total Depth 8.25" Hollow Stem Auger/CME 75 PLI Systems, Inc. 11.2 ft Rig Type Ground Surface Elevation/Datum Hole Diameter 8.25 in Hammer Weight/Drop (lb/in.)/Type 140 lb / 30 in / Automatic 519.8 ft See Figure 2 Site Plan 7617195.00 E. 275427.00 N Flevation Source Location Coordinates Site Survey **PENETRATION** BACKFILL INFORMATION ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE **GRAPHIC LOG** RESISTANCE BLOW RECOVERY SAMPLE **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS**) мс LL/PL Moist, light brown, SILT with gravel (ML); low plasticity, fine to coarse angular gravel. (Fill) ML 100 G-1 Very soft, wet, light brown, Silty SAND (SM); fine to medium sand, low plasticity fines. (Fill) 1-0-1 100 SPT-(N=1)SM -510 10 -8-11-50/3" 40 SPT-(Refusal) Basalt inferred below 11 feet. Borehole completed at Auger refusal at 11.25 feet on basalt bedrock. 11.25ft. below ground surface (bgs). 505 15 -500 20 495 25 490 30



Project: Sweet Home WWTP Final Design Review Log of Boring P-31 **Project Location:** Project Number: 6367.0 Date(s) Drilled Geotechnical Consultant Logged 06/20/2022 - 06/20/2022 J. Quinn McMillen Jacobs Associates A. Judy Drilling Drilling Method/ Total Depth 8.25" Hollow Stem Auger/CME 75 PLI Systems, Inc. 12.7 ft Rig Type Ground Surface Hole Diameter 8.25 in Hammer Weight/Drop (lb/in.)/Type 140 lb / 30 in / Automatic 520.8 ft Elevation/Datum See Figure 2 Site Plan 7617173.00 E. 275391.00 N Flevation Source Location Coordinates Site Survey **PENETRATION** BACKFILL INFORMATION ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE GRAPHIC LOG **RESISTANCE** BLOW RECOVERY SAMPLE **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND _ BLOWS/FT **TESTS**) мс LL/PL Asphalt - 5" thick (Pavement) Base Aggregate - 1" thick (Fill) Moist, light brown, SILT with gravel (ML); low plasticity, fine to coarse angular gravel. (Fill) ML Very soft, moist to wet, Sandy SILT (ML); fine to medium sand, low plasticity fines. (Fill) 0-0-1 33 SPT-(N=1)ML Rod chatter begins at 9 feet and increases significantly below 10 feet. Basalt inferred below 10 feet. 30-38-16 40 SPT-(N=54)2 50/2" SPT 147 (Refusal) Auger refusal at 12.67 feet on basalt bedrock. Borehole completed at 12.67ft. below ground surface (bgs). -506 15 --501 20 496 25 491 30 **Boring P-31**

Project: Sweet Home WWTP Final Design Review Log of Boring P-32 **Project Location:** Project Number: 6367.0 Date(s) Drilled Geotechnical Consultant Logged 06/20/2022 - 06/20/2022 J. Quinn McMillen Jacobs Associates A. Judy Drilling Method/ Drilling Contractor Total Depth 8.25" Hollow Stem Auger/CME 75 PLI Systems, Inc. 10.2 ft Rig Type Ground Surface Hole Diameter 8.25 in Hammer Weight/Drop (lb/in.)/Type 140 lb / 30 in / Automatic 530.4 ft Elevation/Datum See Figure 2 Site Plan 7617027.00 E. 275515.00 N Flevation Source Location Coordinates Site Survey **PENETRATION** BACKFILL INFORMATION ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE GRAPHIC LOG RESISTANCE BLOW RECOVERY SAMPLE **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS**) мс LL/PL Asphalt - 6" thick (Pavement) Base Aggregate - 2" thick (Fill) Moist, dark brown, SILT with gravel (ML); low plasticity, fine to coarse gravel. (Fill) 100 G-1 Soft, moist, gray, SILT (ML); low plasticity, trace fine -526 Becomes gray at 4.5 feet. 0-1-1 100 SPT-(N=2)ML Rod chatter below 9.5 feet 50/2" ШШШШ 10 -SPT-47 (Refusal) Auger refusal at 10.17 feet on basalt bedrock. 2 Borehole completed at 10.17ft. below ground surface (bgs). 15 --511 20 -506 25 -501 30 **Boring P-32**

Project: Sweet Home WWTP Final Design Review **Project Location:** Log of Boring P-33 Project Number: 6367.0 Date(s) Drilled Geotechnical Consultant Checked By Logged 06/20/2022 - 06/20/2022 J. Quinn McMillen Jacobs Associates A. Judy Drilling Method/ Drilling Contracto Total Depth of Borehole 8.25" Hollow Stem Auger/CME 75 PLI Systems, Inc. 3.7 ft Rig Type Ground Surface Elevation/Datum Hole Diameter 8.25 in Hammer Weight/Drop (lb/in.)/Type 140 lb / 30 in / Automatic 529.6 ft See Figure 2 Site Plan 7616945.00 E. 275701.00 N Flevation Source Location Coordinates Site Survey **PENETRATION** BACKFILL INFORMATION ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE **GRAPHIC LOG** RESISTANCE BLOW RECOVERY SAMPLE **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS**) мс LL/PL Moist, brown, Silty GRAVEL (GM); fine to coarse Strong rod chatter from gravel, non-plastic fines. (Fill) ground surface to 3.5 feet ₩ 100 G-1 GM Auger bit grinding at 3.5 Basalt inferred below 3.5 feet. 50/2" SPT-0 Auger refusal at 3.67 feet on basalt bedrock. (Refusal) Borehole completed at -525 3.67ft. below ground surface (bgs). 10 -15 -20 --505 25 -500 30



Project: Project L Project N	.ocatio		VTP Fir	nal Desig	n Re	eviev	V	Log	of Bo	ring P-34
Data(s)	2022 - 06/20/20	Ge	otechnical nsultant	McMillen Jacobs A	Associate	es	Logged By A. Judy		Checked By	J. Quinn
Drilling Method/ Rig Type	8.25" Hollov	v Stem Auger/CME	75	Drilling Contractor	PLI Sys	tems, Inc	p.	Total Depth of Borehole	.0 ft	
Hole Diameter	8.25 in			Hammer Weight/I	Drop (lb/i	n.)/Type	140 lb / 30 in / Automatic	Ground Surface Elevation/Datum	516.9 ft	
Location See	Figure 2 Site I	Plan		Coordinates	7617301	1.00 E, 27	75494.00 N	Elevation Source	Site Surve	е у
ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPI F TYPE	RECOVERY (%)	SAMPLE # BLOW COUNTS	RES BL	ETRATION SISTANCE .OWS/FT VS/FT 40 60 80	GRAPHIC LOG	nscs	MATERIAL DES	CRIPTION	BACKFILL INFORMATION	REMARKS AND TESTS
-512 5	33 SF	7T- 1-1-3 (N=4) 7T- 1-0-1 (N=1) 7T- (Refusal)				ML	Asphalt - 5" thick (Pavement) Base Aggregate - 2" thick (Fill Moist, brown, Silty GRAVEL (t gravel, low plasticity fines. (Fill Soft to very soft, moist, brown plasticity, trace fine sand. (Fill Becomes very soft, grades 10 feet. Auger refusal at 15 feet on	GM); fine to coarse) , SILT (ML); low) to Silt with sand at		Borehole completed at 15ft. below ground surface (bgs).
JA	MILLEN COBS GOCIATES							В	Soring	P-34 1 of 1

Project: Sweet Home WWTP Final Design Review **Project Location:** Log of Boring P-35 Project Number: 6367.0 Date(s) Drilled Geotechnical Consultant Checked By Logged 06/20/2022 - 06/20/2022 J. Quinn McMillen Jacobs Associates A. Judy Drilling Method/ Drilling Contracto Total Depth of Borehole 8.25" Hollow Stem Auger/CME 75 PLI Systems, Inc. 7.1 ft Rig Type Ground Surface Elevation/Datum Hole Diameter 8.25 in Hammer Weight/Drop (lb/in.)/Type 140 lb / 30 in / Automatic 517.5 ft See Figure 2 Site Plan 7617255.00 E. 275628.00 N Flevation Source Location Coordinates Site Survey **PENETRATION** BACKFILL INFORMATION ELEV. (FT) WATER LEVEL DEPTH (FT) SAMPLE TYPE GRAPHIC LOG RESISTANCE BLOW RECOVERY SAMPLE **REMARKS BLOWS/FT** MATERIAL DESCRIPTION AND BLOWS/FT **TESTS**) мс LL/PL Moist, dark gray and brown, Silty GRAVEL (GM); Rod chatter from ground fine to coarse gravel. (Fill) surface to 5 feet bgs. Soft, moist, brown, SILT (ML); low plasticity, trace fine sand. (Fill) ML -513 0-1-2 60 SPT-Rod chatter and difficulty (N=3)Basalt inferred below 6 feet. anvancing auger below 6 1 50/1" 0 SPT-Auger refusal at 7.08 feet on basalt bedrock. (Refusal) Borehole completed at 7.08ft. below ground surface (bgs). -508 10 -503 15 --498 20 493 25 488 30



Appendix B Rock Core Photos



BOREHOLE B-01, 3.5 TO 13.5 FEET



BOREHOLE B-05, 9.0 TO 16.3 FEET



SWEET HOME WWTP SCHEMATIC DESIG	iN
GEOTECHNICAL DATA REPORT	

JULY 2022

CORE PHOTOGRAPHS

FIGURE B - 1



BOREHOLE B-05, 16.3 TO 23 FEET



SWEET HOME WWTP SCHEMATIC DESIGN	
GEOTECHNICAL DATA REPORT	

JULY 2022

CORE PHOTOGRAPHS

FIGURE B - 2

Appendix C Laboratory Test Results



TECHNICAL REPORT

Report To: Mr. Farid Sariosseiri **Date:** 5/14/18

McMillen Jacobs Associates

1500 SW First Avenue, Suite 750 **Lab No:** 18-108

Portland, Oregon 97201

Project: Laboratory Testing – Sweet Home WWTP 5834.0 Project No.: 2286.1.1

Report of: Atterberg limits, moisture content, and compressive strength of rock

Sample Identification

NTI completed Atterberg limits, moisture content, and compressive strength of rock testing on samples delivered to our laboratory on May 9, 2018 by a McMillen Jacobs Associates representative. Testing was performed in accordance with the standards indicated. Our laboratory test results are summarized on the following tables and attached pages.

Laboratory Test Results

Atterberg Limits (ASTM D 4318)								
Sample ID Liquid Limit Plastic Limit Plasticity Index								
B-3 S-2 @ 5 – 6.5 ft.	51	28	23					
B-5 S-1 @ 2.5 – 4 ft.	44	28	16					

Moisture Content of Soil (ASTM D 2216)						
Sample ID Moisture Content (Percent)						
B-3 S-2 @ 5 – 6.5 ft.	44.8					
B-5 S-1 @ 2.5 – 4 ft.	26.3					

Copies: Addressee

This report shall not be reproduced except in full, without written approval of Northwest Testing, Inc.

SHEET 1 of 4

REVIEWED BY: Bridgett Adame EMF



TECHNICAL REPORT

18-108

Report To: Mr. Farid Sariosseiri Date: 5/14/18

> McMillen Jacobs Associates 1500 SW First Avenue, Suite 750 Lab No:

Portland, Oregon 97201

Laboratory Testing – Sweet Home WWTP 5834.0 **Project:** Project No.: 2286.1.1

Laboratory Testing

Compressive Strength of Intact Rock Core Specimens (ASTM D 7012 Method C)								
Sample ID Diameter Height (inches) Compressive Strength (psi)								
B-1 R-1 @ 5.2 – 6.3 ft.	2.41	4.88	100	25,302				



Photo1: As received sample



Photo 2: Test sample before testing



Photo 3: Test sample after testing

This report shall not be reproduced except in full, without written approval of Northwest Testing, Inc. SHEET 2 of 4 REVIEWED BY: Bridgett Adame



TECHNICAL REPORT

Report To: Mr. Farid Sariosseiri **Date:** 5/14/18

McMillen Jacobs Associates 1500 SW First Avenue, Suite 750

1500 SW First Avenue, Suite 750 **Lab No:** 18-108

Portland, Oregon 97201

Project: Laboratory Testing – Sweet Home WWTP 5834.0 Project No.: 2286.1.1

Laboratory Testing

Compressive Strength of Intact Rock Core Specimens (ASTM D 7012 Method C)								
Sample ID Diameter (inches) Diameter (inches) Height (lbs/s) Rate of Loading (lbs/s) Strength (psi)								
B-5 R-1 @ 11.7 – 12.4 ft.	2.41	4.85	100	25,919				



Photo1: As received sample



Photo 2: Test sample before testing



Photo 3: Test sample after testing

Laboratory Testing

This report shall not be reproduced except in full, without written approval of Northwest Testing, Inc. SHEET 3 of 4 REVIEWED BY: Bridgett Adame



TECHNICAL REPORT

Report To: Mr. Farid Sariosseiri **Date:** 5/14/18

McMillen Jacobs Associates 1500 SW First Avenue, Suite 750

1500 SW First Avenue, Suite 750 **Lab No:** 18-108

Portland, Oregon 97201

Project: Laboratory Testing – Sweet Home WWTP 5834.0 Project No.: 2286.1.1

Compressive Strength of Intact Rock Core Specimens (ASTM D 7012 Method C)								
Sample ID Diameter Height (inches) Compressive Strength (psi)								
B-5 R-2 @ 18 – 19 ft.	2.41	4.86	100	6932				



B5-R2@18.0'-19.0' U.C. Rock Cores

Photo1: As received sample



Photo 2: Test sample before testing

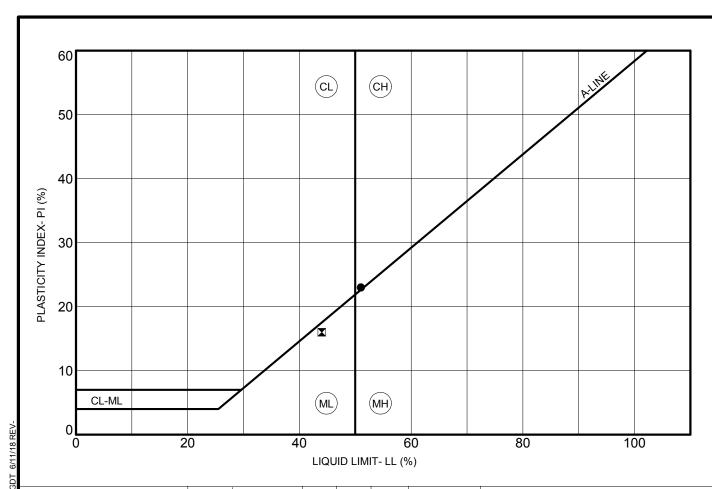


Photo 3: Test sample after testing

This report shall not be reproduced except in full, without written approval of Northwest Testing, Inc.

SHEET 4 of 4

REVIEWED BY: Bridgett Adame



3 REV-		0		2	0		40		-	60	8	<u> </u>	10	00
3DT 6/11/18		U		2	O		-	JID LIN	MIT- LL (%			O .	,	00
GENERAL SOIL PROJECTS.GDT 6/11/18 REV	Sample ID			Depth (ft)	Moisture Content %		PL	PI	% Pass #200	Class	Classification			
SOIL	•	B-03			5.0	(44.8)	51	28	23		СН			
NERAI		B-05			2.5	(26.3)	44	28	16		ML			
JACOBS DATA TEMPLATE														
ATA TE														
OBS D														
J JAC														
ME.GP														
ET HO														
O SWE														
5834														
DRESS														
CE AD						T								
ATTERBERG PORTLAND OFFICE ADDRESS 5834.0 SWEET HOME.GPJ				McMILLI	EN .					ATTER	BERG	LIMITS	3	
PORTL	JACUBS ASSOCIATES							t: W	aste Wa	ater Treatr	ment Plan	ıt - Schen	natic Desi	gn
1500 SW 1ST AVE STE 750							Number: 5834.0							
JA_ATTERE								Sample Location: Sweet Home Testing Agency: Northwest Testing, Inc.						



ATTERBERG LIMITS

Point Load Strength Index Test Results ASTM D-5731

Sweet Home WWTP **PROJECT:** LAB SAMPLE NO.: 5834.0 5834.0 - B-5 **PROJECT NO.:** SAMPLE NO.: **PROJECT LOCATION:** Sweet Home, OR **SAMPLE DESCRIP:** Basalt **SAMPLED BY:** Julia Irizarry **DATE REPORTED:** 5/11/2018 **DATE SAMPLED:** 5/2/2018 **REPORTED BY:** Devin Roth

Sample No.	Test Number	Test Type*	Dock Type	Width, W	Depth or Diameter, D	Failure Load, P	De ²	Point Load Strength Index, I _{s(50)}	Uniaxial Compressive Strength, UCS
				(in)	(in)	(lbs)	(in ²)	(psi)	(psi)
1	1	d	Basalt	5.31	2.36	5959	10313	513	12563
2	2	d	Basalt	5.91	2.36	3828	11459	304	7437
2B	3	d	Basalt	3.23	2.36	5158	6264	653	13717
3	4	d	Basalt	10.04	2.36	5104	19481	268	5098
3B	5	d	Basalt	5.59	2.36	6882	10848	569	13951
5	6	d	Basalt	6.69	2.36	8044	12987	579	13315
6	7	d	Basalt	5.12	2.36	4598	9931	407	9778
7	8	d	Basalt	4.13	2.36	6830	8021	714	17138
8	9	d	Basalt	11.42	2.36	8044	22154	383	9185
9	10	d	Basalt	5.20	2.36	7882	10084	690	14493
							Min	268	5098
							Max	714	17138
							Avg	508	11667

Size Corrected Point Load Index, Is(50)

 $I_{s(50)} =$ 508 psi or 73,159 psf or 3.5 MPa Mean Uniaxial Compressive Strength, σ_c $\sigma_c =$ 11,667 psi or 1,680,111 psf or 80 MPa

*Test Type

d = diametral

a = axial

b = block

l= lump

McMillen Jacobs Associates Rev0/August 2018