

January 9, 2024

City of Sidney Parks and Recreation Committee

RE: Sidney Tiger Sharks Svarre Municipal Pool Permanent Bulkhead Engineering Commentary from 8-29-23 Meeting ie# WR22-04-097

All,

Interstate Engineering's Pool, Aquatics and Recreation (PAR) Group and Structural Group have reviewed the general statements and questions posed during the City of Sidney's Park and Recreation Committee Meeting held on August 29, 2023 at 5:15pm relating to a future permanent bulkhead at the Svarre Municipal Pool. The preliminary plan sheet for a permanent bulkhead that was presented to the committee along with notes from the aforementioned committee meeting are enclosed as Attachment #1 and Attachment #2, respectively.

For clarification, those statements and questions from the committee meeting requiring input from Interstate Engineering are restated in **bold**. Our responses follow in *italics*.

1. Will lap swimming still be available with a permanent bulkhead?

Yes, lap swimming will still be available with a permanent bulkhead. Depending on the how the Pool is operated, there will essentially be two (2) 25-meter lap swim area. One 25-meter lap swim area on the north side of the permanent bulkhead and one \sim 25-meter lap swim area on the south side of the permanent bulkhead. Please refer to Attachment #1.

2. The Committee has concerns with the weight of the permanent Bulkhead.

a. Can a structural analysis be performed on the permanent design with the existing slab and subgrade ahead of a council decision?

Yes. Please see Attachment #3.

b. The Committee does not want to take an unnecessary risk with the permanent design that could lead to substantial repairs in the future that the City would have to pay to fix.

There are always risks when working with unknowns; however, the structural analysis was completed using the soil classifications published within the geotechnical investigation completed on January 17, 1991 by Braun Engineering. Please refer to Attachment #4. The investigation was performed for the design of the original pool facilities.

Without destructive testing/exploration, the material properties of the soil below the pool floor are unknown. Destructive testing/exploration can be done if requested by the Council; however, this style of testing/exploration may generate additional risks that can compromise the integrity of the existing pool floor.

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c. Is there a chance that any gravel under the slab is gone now?

The pool floor was constructed on unexcavated/undisturbed earth. Refer to Pool Cross Section 1/P3 within Attachment #5.

There is no piping under the pool floor that could have broken and cavitated the slab.

Without destructive testing/exploration, the material properties of the soil below the pool floor are unknown. Destructive testing/exploration can be done if requested by the Council; however, this style of testing/exploration may generate additional risks that can compromise the integrity of the existing pool floor.

d. Can actions such as mud jacking be performed ahead of the permanent bulkhead to increase slab performance? What about after the permanent bulkhead is constructed and the slab begins to show signs of distress?

If structural issues are not present in the existing pool tank, we do not recommend performing any preventative maintenance such as mud jacking.

Depending on the type of structural issue present in the pool tank after a permanent bulkhead is constructed, mud jacking is one option that can be performed to mitigate settlement. If issues arise in the existing pool tank, we recommend every issue be analyzed and the available remedies be vetted before a method is chosen.

3. If the permanent bulkhead results in cracks in the floor or wall, what are the options for repair?

Any new cracks formed after a permanent bulkhead was installed would be treated the same as if cracks formed without a permanent bulkhead. We recommend cracks be filled with Sikaflex. If a crack is leaking a substantial amount of water, then a PVC liner may be necessary to make the pool watertight again.

a. What would a catastrophic failure look like that would cause the pool to be shut down temporarily or permanently? And what is the risk?

The bulkhead walls fall apart and crumbles to the bottom of the pool. Risk = very low

The bulkhead breaks the concrete floor, and the pool loses a substantial amount of water. Risk = very low

4. Is the current pool leaking? What is causing the leak and what is the best practice to fix / mitigate the leak if one exists or is discovered in the future?

We have not heard of any leak. But if one exists, we recommend taking steps to isolate the problem. Steps:

- 1. Plug the main drainpipes, fill the tank to below the transition from concrete wall to gutters, and observe water levels. If water levels remain steady, move on to the next step. If losing water, then there's likely cracks that are leaking.
- 2. Pull the plugs in the main drainpipe and observe water levels. If water levels remain steady, move on to the next step. If losing water, then there's likely a hydrostatic relief vale leaking or a pipe leaking.
- 3. Plug the stainless-steel gutter outlet pipe(s), fill the gutter full of water, and observe water levels. If water levels remain steady, move on to the next step. If losing water, then there's likely a failing weld in the gutter.

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4. Fill the pool water level up to normal water level and observe water levels. If losing water, then it's likely leaking at the transition from the wall to the gutter.

a. Would a permanent bulkhead increase this possibility?

The only increase probability would be cracking in the floor. See Attachment #3 for results of the structural analysis. The existing floor slab of the pool is structurally adequate to support a permanent bulkhead and associated loading.

If you have any questions, please don't hesitate to call me at (406) 433.5617 or email me at <u>Jordan.Mayer@InterstateEng.com</u>. Interstate Engineering and the Sidney Swim Team respectfully request another Parks and Recreation Committee Meeting in late January / early February to discuss these findings and address any other comments, questions, or concerns from the Committee. We are hopeful to identify a solution that benefits both the City and Swim Team for the next 20+ years.

Respectfully, INTERSTATE ENGINEERING, INC.

Jordan Mayer, P.E. Sidney Office Manager

Kaden Bedwell, El PAR Group Leader

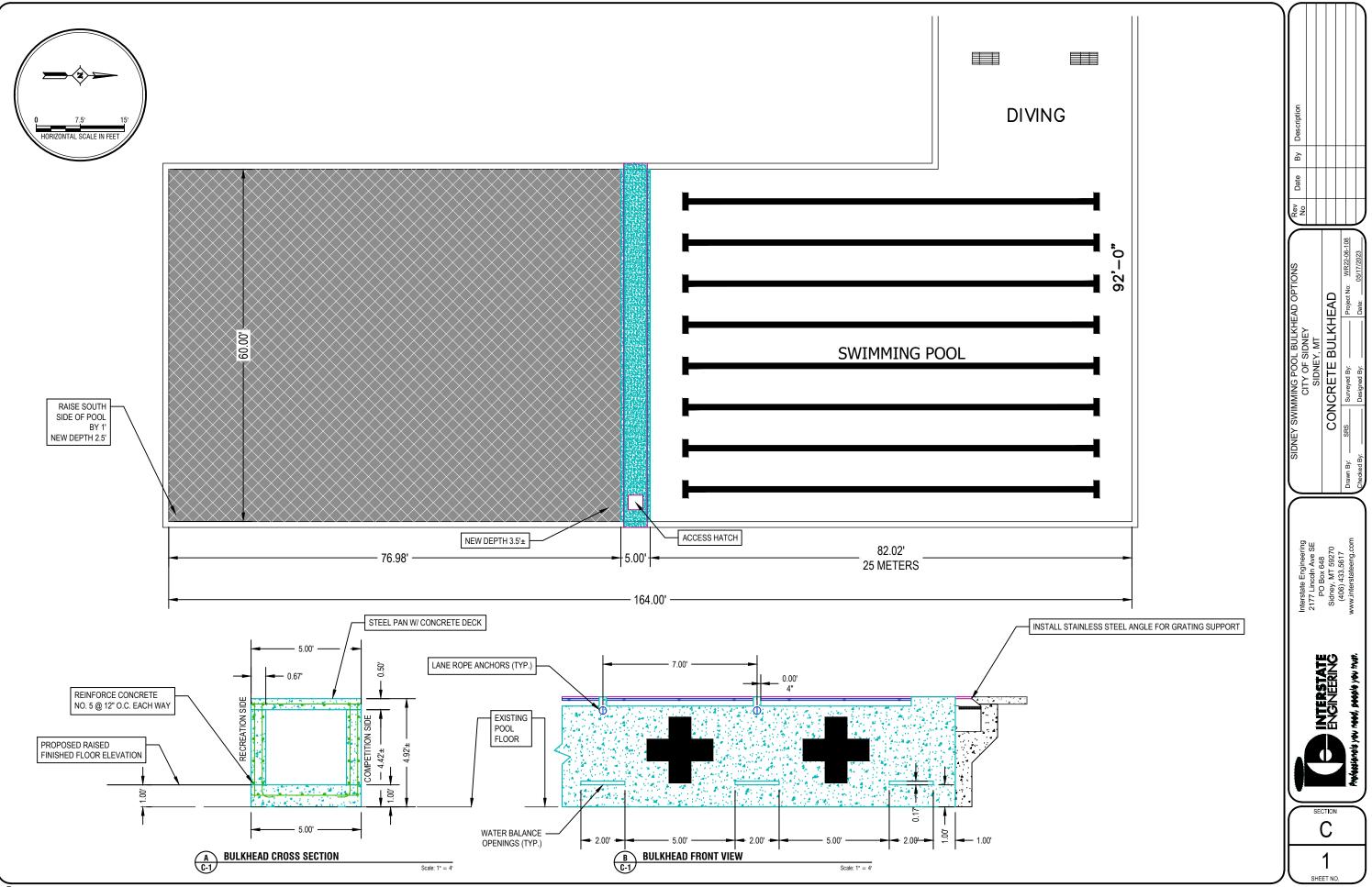
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Teaguean Knudsen, P.E. Structural Group Leader

Enclosures:

Attachment #1: SIDNEY_POOL_CONCRETE_BULKHEAD-07_31_23 Attachment #2: Jordan Mayer Email 8/30/2023 @ 11:10AM; RE: Park & Rec Committee 8-29-23 at 5:15pm Attachment #3: WR2204097 Sidney Pool Bulkhead-Pool Floor Analysis Attachment #4: Braun Engineering Testing Geotechnical Investigation 1/17/1991 Attachment #5: Svarre Municipal Pool Plans 4/15/1991

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Attachment #1

Jordan Mayer

From: Sent:	Jordan Mayer Wednesday, August 30, 2023 11:10 AM
To:	Jessie Redfield; Tami Christensen (tricntyadm@midrivers.com); Tami Christensen; Kali
	Godfrey; Frank DiFonzo; Rick Norby; Jeff Hintz
Cc:	Nick Kallem; Brian Milne; Kaden Bedwell; Peter Erickson
Subject:	RE: Park & Rec Committee 8-29-23 at 5:15pm

Attachment #2

All,

Below are my notes from the meeting yesterday. <u>Please let me know if you have anything to revise, add or delete by</u> the end of the day Friday, September 1st.

The blue text I feel is best suited to be answered by the City. The black text is general statements. IEI will provide comment on the green text.

- There was a general consensus that the pool and bath house is in good shape and will last another 20+ years.
- Committee wants to make sure that the decision of temporary v. permanent takes into account all users of the pool and not just the swim team.
- Will lap swimming still be available with a permanent bulkhead?
- Is there any data that shows the amount and type of current pool users within the last 1-3 years?
 - o Adult lap swimmers
 - Adult exercise
 - Swim team
 - Youth swim
 - o Swimming lessons
 - o Other?
- Committee has concern with the weight of the permanent bulkhead.
 - Can a structural analysis be performed on the permanent design with the existing slab & subgrade ahead of a council decision?
 - Committee does not want to take an unnecessary risk with the permanent design that could lead to substantial repairs in the future that the City would have to pay to fix.
 - Is there a chance that any gravel under the slab is gone now?
 - Can actions such as mud jacking be performed ahead of the permanent bulkhead to increase slab performance
 - What about after the permanent bulkhead is constructed and the slab begins to show signs of distress?
- If the permanent bulkhead results in cracks in the floor or wall, what are the options for repair?
 - What would a catastrophic failure look like that would cause the pool to be shut down temporarily or permanently? And what is the risk?
- Is the current pool leaking? What is causing the leak and what is the best practice to fix / mitigate the leak if existing or if one is discovered in the future?
 - Would a permanent bulkhead increase this possibility?
- Are there any other costs or responsibilities that the City will incur as a result of the permanent bulkhead?
 - Additional life guard chair
 - o New blankets
 - o Other?
- The committee would like to take the raising of the pool out of the decision

- There was a general consensus that construction of a permanent bulkhead would be best to start after swimming lessons are completed (generally around fair time)
 - There is uncertainty every year that the pool is able to start-up without a hitch without taking any construction into consideration.

The committee recommended to the council on September 5th for the swim team to use the existing temporary bulkhead for the 2024 swim season and state meet, to increase the funding from the Oil and Gas revenue to the swimming pool CIP this budget year from \$75,000 to \$145,000 to cover the cost of painting the pool and raising the pool, and put the decision on hold between temporary and permanent bulkhead until more information is presented by IEI.

Thanks!

Jordan Mayer

Project Engineer | Office Manager Interstate Engineering

From: Jessie Redfield <clerktreasurer@cityofsidneymt.com>

Sent: Monday, August 28, 2023 9:27 AM

To: Tami Christensen (tricntyadm@midrivers.com) <tricntyadm@midrivers.com>; Tami Christensen <council2ward3@cityofsidneymt.com>; Kali Godfrey <council2ward2@cityofsidneymt.com>; Frank DiFonzo <council1ward3@cityofsidneymt.com>; Rick Norby <mayor@cityofsidneymt.com>; Jeff Hintz <publicworks@cityofsidneymt.com>

Cc: Nick Kallem <poolmanager@cityofsidneymt.com>; Jordan Mayer <Jordan.Mayer@interstateeng.com> **Subject:** Park & Rec Committee 8-29-23 at 5:15pm

CAUTION: This email originated from outside your organization. Exercise caution when opening attachments or clicking links, especially from unknown senders.

Here is your reminder that we do have a Park & Rec Committee meeting tomorrow night pertaining to the pool dock at 5:15pm.

You can find the agenda and packet at the address below (same agenda packet items as was in the council packet).

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https://cityofsidneymt.com/meetings

Jessica Chamberlin City Clerk/Treasurer 115 2nd St SE Sidney, MT 59270

(406)433-2809



Attachment #3

01/07/2024

Sidney Bulkhead Structure Analysis

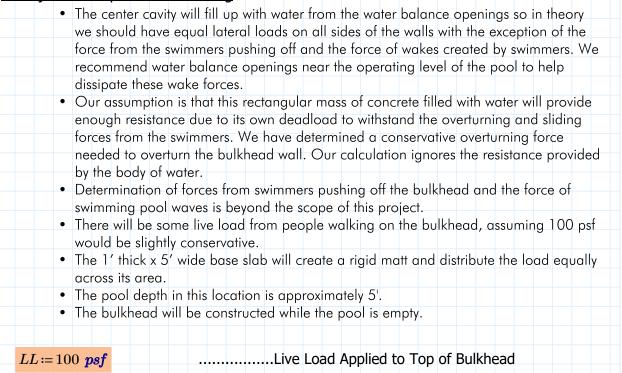
City of Sidney, MT

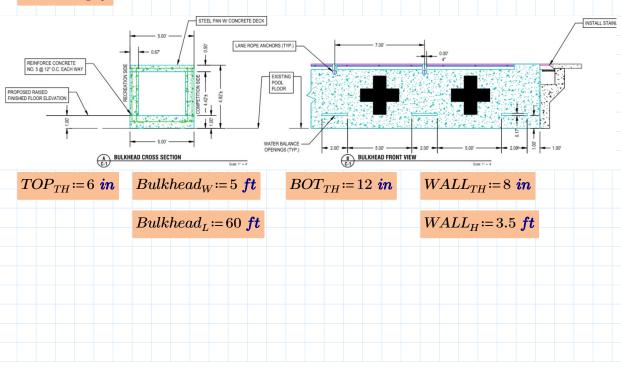
Prepared For: Jordan Mayer, PE

Prepared By: <u>Teaguean Knudsen, PE</u>

Project #: WR22-04-097

1. Project Assumptions and Loading





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$WALLS_{DL} := WALL_{TH} \cdot W_{A}$	$ALL_H \cdot Bulkhead_L \cdot 2 \cdot 150 \ pcf = 42000 \ lbf$	
$BOT_{DL} \coloneqq 1 \; \mathbf{ft} \cdot Bulkhead_W$	$_{V} \cdot Bulkhead_{L} \cdot 150 \ pcf = 45000 \ lbf$	
$AREA_{Base} \coloneqq 5 \ ft \cdot 60 \ ft$		
$DL \coloneqq \frac{TOP_{DL} + WALLS_{DL}}{AREA_{Base}}$	$+BOT_{DL} = 365 \ psf$ Dead Load of Concrete	
$Density_{Water} \coloneqq 62.4 \ pcf$	$Water_W \coloneqq Bulkhead_W - (2 \cdot WALL_{TH}) = 3.667 \ ft$	
	$Water_{H} \coloneqq WALL_{H} = 3.5 \; ft$	
$HL \coloneqq Density_{Water} \cdot Water_{H} =$	= 218.4 <i>psf</i> Hydrostatic Load of Water	
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01/07/2024

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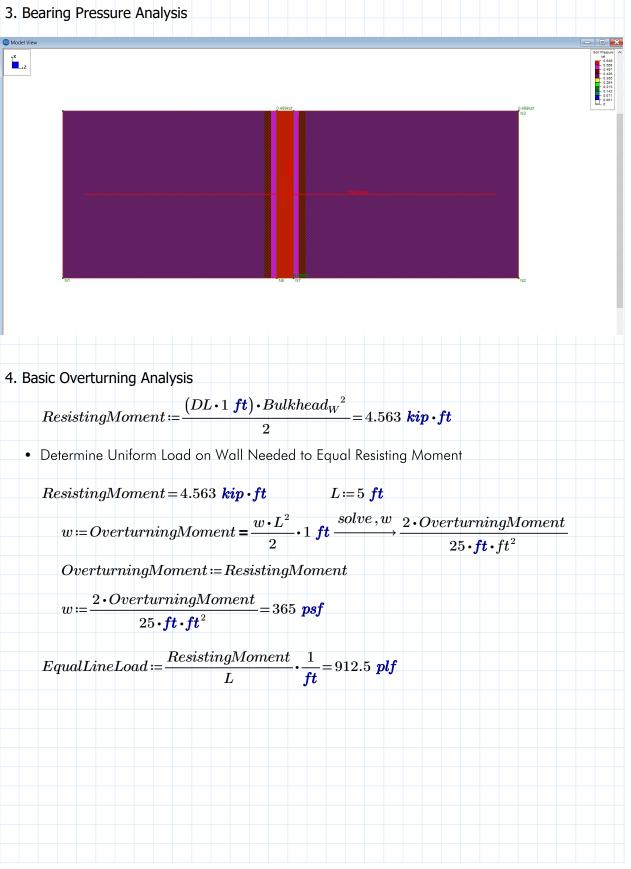
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01/07/2024

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, ((t)	
	The above bearing pressure graphic displays the bearing pressure results from an
	The above bearing pressure graphic displays the bearing pressure results from an applied vertical live load of 100 psf and a lateral live load of 900 plf applied
	along the top of the bulkhead wall. The moment created by the 900 plf lateral
	live load at the top of the wall equates to a 360 psf uniform load applied along
	live load at the top of the wall equates to a 360 psf uniform load applied along
	the full height of the wall. Fluid pressure from the pool filled to a depth of 5' is
	the full height of the wall. Fluid pressure from the pool filled to a depth of 5' is also included in the analysis.
Su	the full height of the wall. Fluid pressure from the pool filled to a depth of 5' is
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Attachment #4

BRAU

B. <u>RESULTS</u>:

B.1. Logs: Log of Boring sheets indicating the depths and identifications of the various soil strata, the penetration resistances, laboratory test data and water level information are attached. It should be noted that the depths shown as boundaries between the strata are only approximate. The actual changes may be transitions and the depths of the changes likely vary horizontally.

(B-1)

B.2. <u>Site Conditions</u>: The site is located in Sidney, Montana, in the southwest quandrant of the intersection of 7th Avenue and 6th Street. The site is relatively flat and is partially covered with concrete and asphalt. It was previously a maintenance yard for the city.

B.3. <u>Soils Encountered</u>: Up to 1 foot of asphaltic concrete and gravel base or Portland cement concrete was encountered at all of the borings except boring ST-1W. Beneath the surfacing material, borings ST-1W, ST-2, ST-5 and ST-6 encountered medium to stiff silty clay or clayey gravel fill to depths of 3 1/2 to 5 1/2 feet underlain by soft to medium clayey sand to sandy/silty clay fine alluvium. In borings ST-3 and ST-4, natural sandy/silty clay was encountered immediately beneath the asphalt and gravel base. Fuel odors were noticed in borings ST-3 and ST-4. These strata are described in more detail below. B.3.a. <u>Fill</u>: About one foot of 1-minus sandy gravel/gravel base was encountered in borings ST-4 and ST-6. Silty clay fill was encountered to depths of 3 1/2 to 5 1/2 feet in borings ST-1W, ST-2 and ST-6. Clayey gravel fill was encountered to a depth of 3 1/2 feet in boring ST-5. Penetration resistances ranged from 7 to 15 blows per foot (BPF). Pocket penetrometer strengths ranged from 1 1/2 to over 4 1/2 tons per square foot (tsf). These values indicated the silty clay/clayey gravel fill was medium to stiff in consistency.

B.3.b. <u>Fine Alluvium</u>: The fine alluvium consisted of clayey silt to sandy/silty clay. Penetration resistances ranged from 3 to 8 BPF and penetrometer strengths ranged from a 3 1/2 down to 1/3 tsf. These values indicated the fine alluvium was medium to soft in consistency. It was generally wetter and softer below depths of about 8 to 9 feet than it was above those depths.

B.4. <u>Groundwater Measurements</u>: Groundwater was observed at a depth of 18 feet in Boring ST-3. This depth corresponds to a elevation of 28 1/2 feet on the assumed datum. It should be noted, however, that the time of observation was very short and that several days may be required for groundwater to stabilize in a bore . hole in clay soils. Groundwater was not observed to a depth of 15



feet in the well installed in boring ST-1W when checked 32 days after completion.

Seasonal and annual fluctuations of the groundwater table occur due to variations in rainfall, irrigation, snow melt and other factors not evident at the time of the investigation. It appears that these fluctuations may affect the design, construction and performance of the proposed swimming pool.

B.5. <u>Laboratory Tests</u>: The results of the laboratory tests are presented on the boring logs and the attached consolidation test graphs. The results are summarized below.

B.5.a. <u>Moisture Contents</u>: The moisture contents of the samples tested ranged from 22.2% to 22.6%. The average value was 22.4%. These are relatively high moisture contents for silty clays.

B.5.b. <u>Dry Densities</u>: The dry densities of the samples tested ranged from 81.5 pounds per cubic foot (pcf) to 97.7 pcf. The average value was 89.6 pcf. These are low to average values for silty clays.

B.5.c. <u>Consolidation/Collapse</u>: The result of the consolidation test performed on the moist thin-walled tube sample from boring



ST-4 is shown on the attached graph. The sample collapsed about 4% when it was inundated under a load of 1000 psf. This is a moderate to high value. Compression under a load increase of 1000 psf was about 9%. This is a high value.

The result of the consolidation test performed on the wet thinwalled tube sample from boring ST-5 is shown on the attached graph. The sample collapsed only about 1% when it was inundated under a load of 1000 psf. This is a relatively small value. Compression under a load increase of 1000 psf was about 3%. This is a moderate value.



D. CONSTRUCTION:

D.1. <u>Excavation</u>: It is our opinion that the soils encountered by the borings can be excavated with a backhoe or front end loader. However, rubber-tired equipment likely will not be able to work in the bottoms of the footing and pool subexcavations due to the soft and wet nature of the silty clay. The borings indicate that the silty clay soils in the sidewalls of the excavations will be Type B soils under Department of Labor Occupational Safety and Health Administration (OSHA) guidelines.

D.2. <u>Observations</u>: We recommend that footing, slab and pool subexcavation subgrades be observed by a geotechnical engineer or an engineering technician under the direction of a geotechnical engineer to evaluate if the subgrade soils are similar to those encountered by the borings.

D.3. <u>Moisture Conditioning</u>: Site soils which will be excavated and reused as backfills and fills appeared to be at or wet of optimum. We anticipate it will be necessary to dry some of these soils to achieve a moisture content near or slightly above optimum.

D.4. <u>Testing</u>: We recommend density tests of fills and backfills placed beneath footings, slabs and pools. Samples of proposed



(D-1)

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backfill and fill materials should be submitted to our testing laboratory at least 3 days prior to placement on the site for evaluation and determination of the optimum moisture content and maximum dry density.

We recommend slump, air content and strength tests on Portland cement concrete.

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E. GENERAL REMARKS:

E.1. <u>Basis of Recommendations</u>: The analysis and recommendations submitted in this report are based upon six soil borings performed at the locations indicated on the attached sketch. Variations likely occur between these borings, the nature and extent of which may not become evident until construction. If variations are encountered, it may be necessary to make a re-evaluation of the recommendations of this report after performing on-site observations during the construction and noting the characteristics of any variations. Such variations may result in additional foundation costs and it is suggested that a contingency be provided for this purpose.

E.2. <u>Review of Plans and Specifications</u>: This report is based on the preliminary design of the proposed structure as submitted to us for the preparation of this report. Because only limited amount of information was available, a number of assumptions were necessary to permit us to make recommendations. It is recommended that we be retained to review the final design and specifications to determine whether those assumptions were correct and whether any change in concept may have had any effect on the validity of our recommendations, and whether our recommendations have been implemented in the design and specifications. If we are not per-



mitted to make this recommended review, we will not be liable for losses arising out of such design changes, or misinterpretation or misapplication of our recommendations.

E.3. Observation and Testing: It is recommended that we be retained to develop and perform the necessary observation and testing program for the excavation and foundations phases of the project to: (1) permit correlation of the soil data used in this report with actual soil conditions encountered during construction, (2) provide continuing professional responsibility for the concepts contemplated in this report and (3) promote conformance to the plans and specifications.

If others perform the recommended observations and/or testing of construction, professional responsibility becomes divided since in doing so, they assume responsibility for verifying that the soil conditions throughout the construction areas are similar to those encountered in the borings or recognizing variations which would require a change in recommendations.

E.4. <u>Groundwater</u>: Water level readings have been made in the borings at the times and under conditions stated on the boring logs. This data has been reviewed and interpretations made in the



text of this report. However, it must be noted that the period of observation was relatively short and that fluctuations in the level of the groundwater may occur due to variations in rainfall, temperature, irrigation, snow melt, pumping and other factors not evident at the time measurements were made and reported herein. Design drawings and specifications and construction planning should recognize the possibilities of variations.

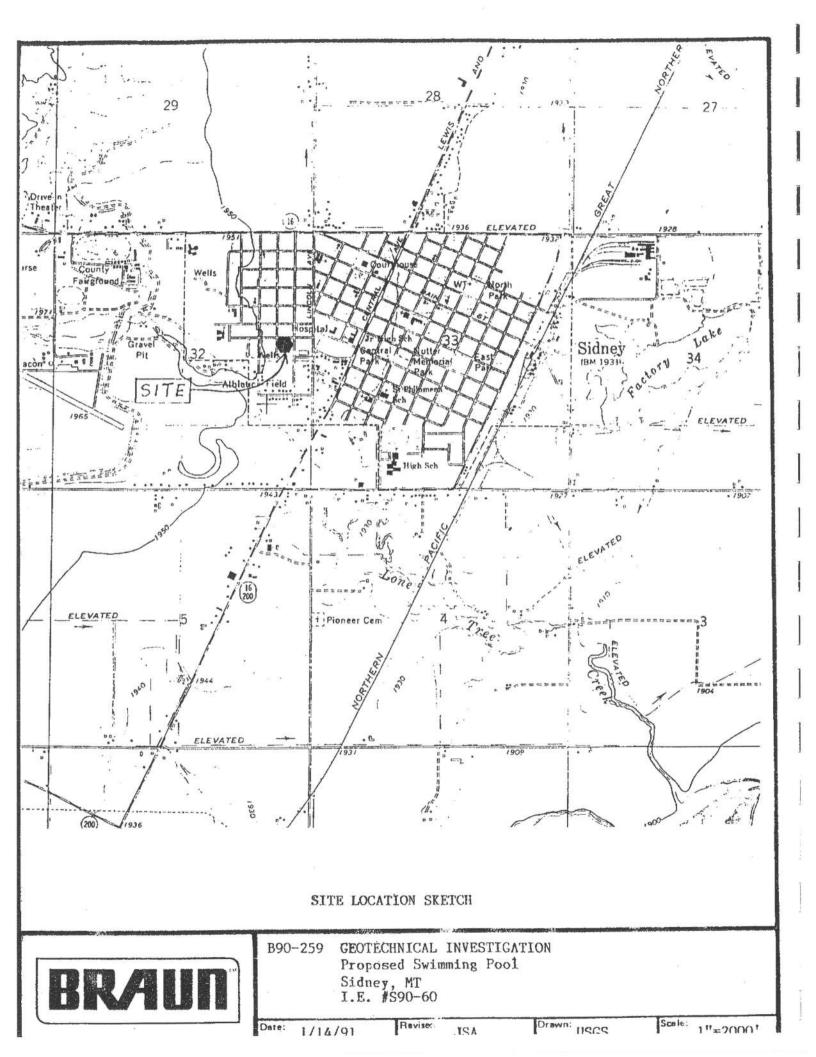
E.5. <u>Standard of Care</u>: Services performed by the geotechnical and material engineers for this project have been conducted in a manner consistent with that level of care and skill ordinarily exercised by members of the profession currently practicing in this area under similar budget and time restraints. This is our professional responsibility. No warranty, expressed or implied, is made.

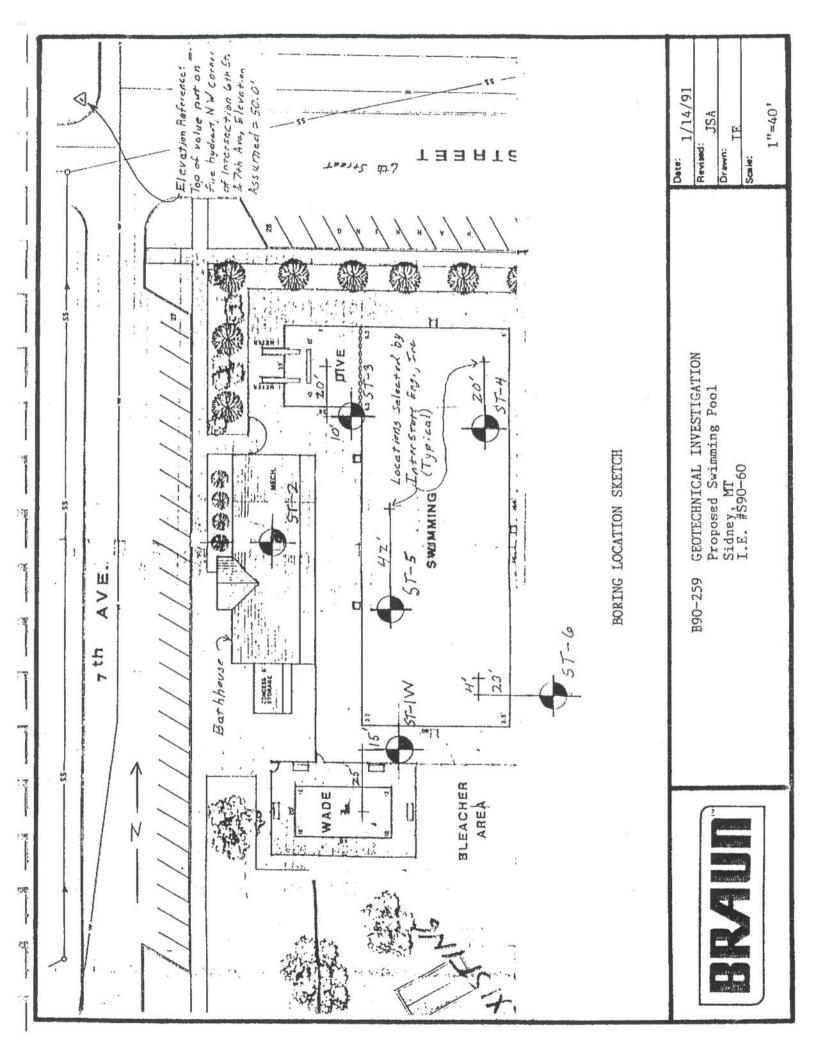
E.6. <u>Professional Certification</u>: I hereby certify that this report was prepared by me or under my direct supervision and that I am a duly Registered Professional Engineer under the laws of the state of Montana.

mansha

John S. Anevski, P.E. Registration Number 10068PE Date: January 17, 1991









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PROJE	ECT: B	90-259		BORING	G:	S	T-1	W			
	C	FOTECI	INICAL INVESTIGATION	LOCAT	ION:		3				
	P	oposed S	Swimming Pool	Wading	Pool						
	SI	dney, M	Г								
				DATE:	12/1	4/90	S	CALE: $1'' = 3'$			
Elev. 46.1	Depth 0.0	ASTM Symbol	Description of Materials (ASTM D2488)		BPF	WL	qp*	Tests or Notes			
		CL V	FILL: Silty Clay, low to medium pla								
	-		little sand, brown, slightly moist, me rather stiff.	dium to	7		2 1/2	Elevation Reference: Top			
	×				ñ			of valve nut on			
	-				10		1 1/2-				
42.6	3.5				8			corner of Intersection			
72.0	J.J	ML	CLAYEY SILT, slightly plastic, pale	brown,	11			6th Street & 7th			
	-		moist, medium. (fine alluvium)					Avenue,			
-	_				86	-		Elevation assumed =			
40.1	6.0				R			50.0'			
		CL	SILTY CLAY, medium plasticity, pa	le brown,	1						
	-		moist, rather soft. (fine alluvium)		\$ 5		1/2				
					, second		1/2				
	-				ñ						
	_		-wet below 9'								
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30.6	15.3				84		1/2				
30.0	15.5		END OF BORING		8						
	-										
	-		Water level not encountered with 14'	of				*qp=pocket			
			hollow-stem auger in the ground.	100000				penetrometer			
	1		A 1 1/4" diameter PVC monitoring w	all with	11			estimate of			
	_		a screen from 10 to 15 feet was left					unconfined compressive			
1		× .	boring.					strength, tons p			
_			Water not observed to a depth of 15	feet in	1			square foot.			
			well on January 15, 1991.	ioot in							
			Construction and Discontraction of the Construction of the Cons								
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PRO	ECT: B	90-259)	· · · · · · · · · · · · · · · · · · ·	Boring:			ST	-2		
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	2000			wimming Pool	Bathhou						
	S	dney, l	MI								
					DATE:	12/1	4/9	0	SCALE:	1" :	= 3'
Elev. 47.3	Depth 0.0	ASTN Symbo		Description of Materials (ASTM D2488)		BPF	WL	qp	Tests	or	Notes
	1.0	-		Portland Cement Concrete		1					
46.3 46.3	1.0	CL		FILL: Silty Clay, medium plasticity, t sand and wood, olive gray, moist, med stiff.	race lium to	8		4 1/2			
41.8	-	CL		SILTY CLAY, medium plasticity, trac	e sand,	TW-	1	1 1/	1/2		
#tion				olive brown, with some gray zones, we rather soft to medium. (fine alluvium	et,	Ŋ					
valuat 1	-			Tathor Sore to mourant. (time anaviant	,	8		1/2	2		
10-											
36.3	11.0					× 4		1/2	2		
Standard	_	CL		SILTY CLAY, low plasticity, trace san to brown, wet, rather soft. (fine alluv	id, gray ium)	4		1			
	-				τ.	8					
31.8	15.5					4		1/2	2		
11 - 11 - 11 - 11 - 11 - 11 - 11 - 11	-			END OF BORING							
(Se	-			Water level not encountered with 14' o hollow-stem auger in the ground.	f						
_	-			Water level not encountered to dry cav depth of 8' immediately after withdray auger.	ve-in wal of						
-	_		e								
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B90-259	L			BRAUN		11	L	L	1		



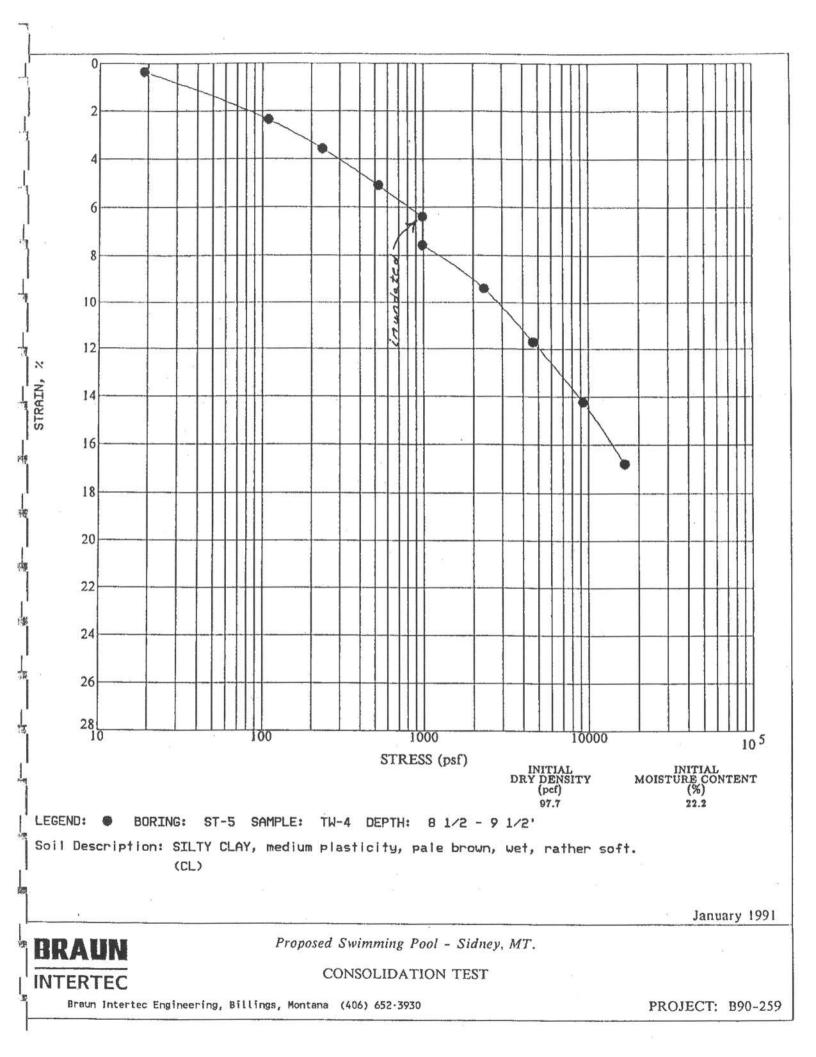
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				NICAL INVESTIGATION	LOCAT	ION:				5		
				wimming Pool	NE Corr	ner of	Poo	1, 2	0'S	of Ma	rked	
		dney,			Location							
					DATE:	12/1	4/9	0	SC	ALE:	1"	= 3'
	T	<u> </u>			.					Tests	or	Notes
Elev.	Depth	AST		Description of Materials	9	BPF	WL	qp				
Elev. 46.3 46.1 45.3		Sym	bol	(ASTM D2488)						-		
46.1				FILL: 1" minus Sandy Gravel Base	/							
		CL	1	SILTY CLAY, low plasticity, fuel odd	r gray							
<u></u>	-			moist, medium. (fine alluvium)	, gruy,	88		3 1	h			
						8°		51	1			
42.8	3.5					Π						
42.8		CL		SILTY CLAY, medium plasticity, fue sand lenses (fine grained), gray to oliv			1					
				brown, moist, rather soft to medium.	(fine	8		3 1	12			2.52
		1		alluvium)		8						
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5-	-			-wet below 9'								
						6		1,	12			
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						5	1	1,	/2			
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	15 7					4		1,	/2			
30.8	15.5		111	END OF BORING	-	fi -						
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	-			Water level not encountered with 14'	of							
				hollow-stem auger in the ground.	01							
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1				depth of 11 1/2' immediately after withdrawal of auger.	s			1				
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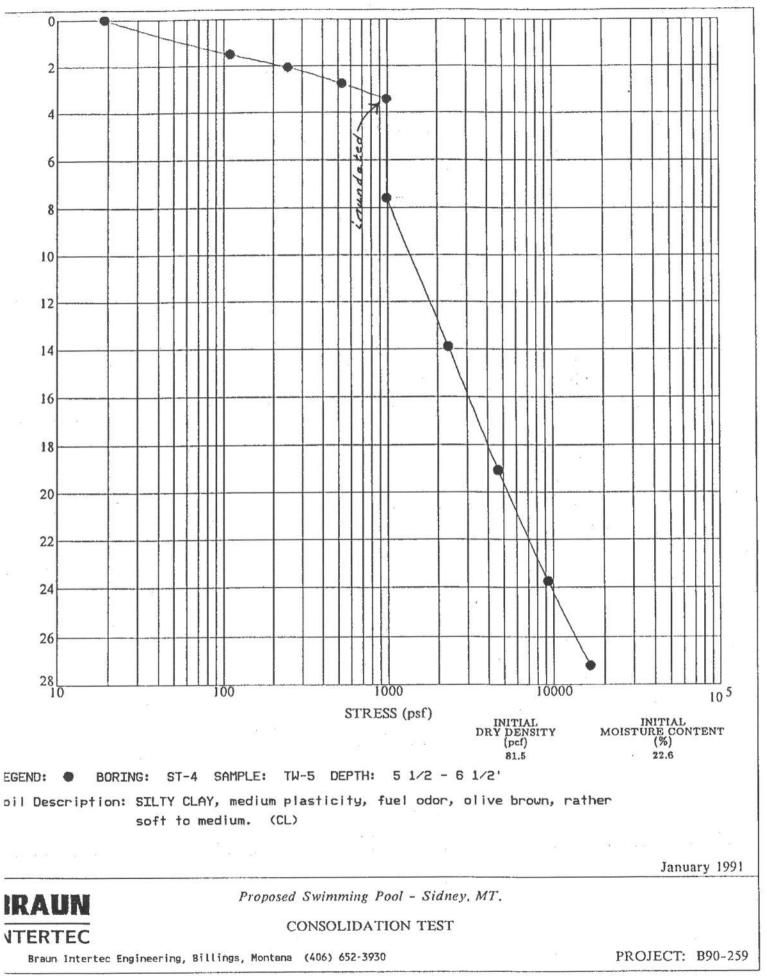


PROJI	ECT: B	90-259		Boring:			ST	-5		
	G	EOTECH	INICAL INVESTIGATION	LOCAT	ION:					
	P	roposed S	Swimming Pool	Pool						
	SI	dney, M								
			r	DATE:	12/1	4/9	0	SCALE:	1	' = 3'
Elev. 46.5 46.3	Depth 0.0	ASTM Symbol	Description of Materials (ASTM D2488)		BPF	WL	qp	Tests	or	Notes
46.3	0.2	GC	Asphaltic Concrete	/	1					4
-	-		FILL: Clayey Gravel, low plasticity, brick, brown, rather dry, stiff.	trace						
-	_				15		4 1/3	2+		÷
43.0	3.5				1					
43.0	-	ML	CLAYEY SILT, slightly plastic, pale moist, rather soft to medium. (fine a		84		1 1/	/2		×
					8					
- 40.0	6.5				TW.	3	1 1,	12		
-	-	CL	SILTY CLAY, medium plasticity, pal- moist to wet, rather soft to medium.		\$5		1 1,	12		
-	-		alluvium)		NTW					
					NTW.	4	1/2			
					6		1 1,	/2		
-	-				8					
-	-		a		5		1/:	2		
-	-				×					
- 21.0	15.5		2 ⁹		5		1			
31.0	15.5		END OF BORING		M					
_			Water level not encountered with 14' hollow-stem auger in the ground.	of						
-	_		Water level not encountered to dry ca depth of 11.7' immediately after with	ve-in drawal						
	_		of auger.						87	
-	-									
-	-									
			2							
B90-259	1		BRAUN	······	11	1	1	1	T-5	nore I of



ſ	PROJI	ECT: B	90-25	9		Boring:		1	ST	-6									
					NICAL INVESTIGATION	LOCATI	ION:	an and a second											
		Pr	opose	d S	wimming Pool	SW Corr	ner of	Poo											
		Si	dney,	MI				·											
						DATE:	12/1	4/90		SCALE:		= 3'							
(.76	Elev. 46.6	Depth 0.0	AST Syml		Description of Materials (ASTM D2488)	w Ś	BPF	WL	qp	Tests	or	Notes							
erminalagu	46.4	0.2			Asphaltic Concrete		1												
LII	45.6	1.0	01		FILL: 1" Minus Gravel Base														
4	43.1	3.5	CL		FILL: Silty Clay, medium plasticity, t sand, brown, slightly moist, stiff.	race	814		2 1/	2									
d descriptive	41.1	5.5	ML	410	CLAYEY SILT, slightly plastic, dark i slightly moist, rather soft. (fine alluvi		84		3 1/	/2		2 M.)							
on and	- -		CL		SILTY CLAY, medium plasticity, pale moist, soft to medium. (fine alluvium		TW.	2	2	10 									
evaluati		_					5		1			а.							
stes for							5		1/2	2									
rd Plat		_																	
d Standa		-			5 5 5	8.8	7		I										
art and		-			-wet below 14'		3		1/2	2									
Report	31.1	15.5			END OF BORING		Ĩ												
alas)		-			Water level not encountered with 14' of hollow-stem auger in the ground.	of			2. 2										
		_			Water level not encountered to dry car depth of 10 1/2' immediately after withdrawal of auger.	ve-in													
					e) K	t,													
		-			54						17								
ļ	90-259				BRÁUN					s	T-6	page 1 of							





45 Designation D 2487 - 83 Standard Test Method for CLASSIFICATION OF SOILS FOR ENGINEERING PURPOSES

SE-GRAINED SOILS Than Sol, retained an Na. 200 sieve	LAI	76 903 51937	STENING GRO	UP SYNBOLS MAD	GROUP	DIL CLASSIFICATION
Sort		ROUP NAMES U	SING LABORA	TORY TESTS A	SYMBOL	CHOMP NAME D
D SOIL etoned	GRAVELS	CLEAN G	RAVELS	$C_{u} \ge 4$ and $1 \le C_{c} \le 3$?	GW	Nell-graded grave) f
022	More than 50% of coarse fraction	Less than	5% fines C	Cu (4 and/or 1 > Cc > 3 *	GP	Poorly graded gravel f
WYE	relained on No. 4 sieve	GRAVELS WI	TH FINES	Fines classify as HL or MH	GM	Stilly graves 1,g,h
100		Hore than 1		fines classify as GL or CH	CC .	Clayey gravel 1.g.h
5 5 5	SANDS 50% or more of	CLEAN Less than	SAMUS 5% fines d	$C_{u} \geq 5$ and $1 \leq C_{c} \leq 3^{e}$	54	Well-graded sand 1
	coarse fraction passes No. 4			Cy < 6 and/or 1 > Cc > 3 C	\$P	Poorly graded sand 1
more more	steve	SANDS WI More than 1	TH FINES 2% fines d	Fines classify as ML or Mi Fines classify as CL or CH	SC SK	Silly sand 9.h.l Clayey sand 9.h.l
0		Inorganic		d plats on or above	CL CL	Lean clay k.l.m
501L5	SILIS AND CLAYS Liquid limit Jess than 500	snorganic		plots being "A" line J	ML	Stit 1,1,m
		organic	Liguid 1	mit - oven dried < 0,75	oL	Organic clay k, 1, m, m Organic silt k, 1, m, o
FINE -GRAINED SC more passed the sieve	SILTS MIL CLAYS		PI plats	nn or above "A" line	Сн	Fat clay k.1,m
" 5	Liquid limit 50% ar more	linorgan ir	Pl plots	below "A" line	MH	Elastic silt k,1,m
202		organic	Liquid 1 Liquid 1	mit - oven dried < 0.75	OH	Organic clay k. l. m. s Organic sile k. l. m. s
	hly organic solls	Primarily o organic o	rganic mati	ler, dark in color, and	PI	Peat
	SP-SH poerty gr SP-SC paerty gr Cu * D ₆₀ /D ₁₀	In the sand with $\frac{(D_{30})^2}{(D_{10} + D_{10})^2}$	silt clay th silt th clay		el" which ogroup n ly" to gr	ever is predominant, ame, oup name.
р. q.	Pi plots on er abo Pi plots below "A" For clossificatio		grained s			

PARTICLE SIZE IDENTIFICATION

Boulders	over 12"
Cobbles	3" to 12"
Gravel	
Coarse	
Fine	No. 4 14"
Sand	
Coarse	No. 4 No. 10
Medium	No. 10 No. 40
Fine	No. 40 - No. 200
Silt	No. 200005 mm
Clay	less than .005 mm

RELATIVE DENSITY OF COHESIONLESS SOILS

very loose		13		22	2	2	2				4				20	22					•		0	-		4	BPF
loose				2																			5			10	BPF
medium de	r	15	56	2									*									1	1	-		30	BPF
dense																						3	1		-	50	BPF
very dense																											

CONSISTENCY OF COHESIVE SOILS

very soft 0	1 BPF
soft	
rather soft 4	5 BPF
medium	
rather stilf	
stiff	16 BPF
very sliff	
hard	30+ BPF

DRILLING NOTES

Standard penetration test borings were advanced by 314" or 614" I.D. hollow-stem augers unless noted otherwise. Jelting water was used to clean out auger prior to sampling only where indicated on logs. Standard penetration test borings are designated by the prefix "ST" (Split Tube).

Power auger borings were advanced by 4" or 6" diameter, continuous-flite, solid stem augers. Soil classification and strain depths are inferred from disturbed samples augered to the surface and are therefore somewhat approximate. Power auger borings are designated by the prefix "B".

Hand probings were advanced manually with a 115" diameter probe and are limited to the depth from which the probe can be manually withdrawn. Hand probings are indicated by the prefix "H".

SAMPLING - All samples are taken with the standard 2" O.D. split tube sampler, except where noted. TW indicates thin-wall (undisturbed) sample.

BPF - Numbers indicate blows per foot recorded in standard penetration test, also known as "N" value. The sampler is set 6" into undisturbed soll below the hollow-stem auger. Driving resistances are then counted for second and third 6" increments and added to get BPF. Where they differ significantly, they are reported in the following form - 2/12 for the second and third 6" increments respectively.

WH -- WH indicates that sampler penetrated soil under weight of hammer and rods alone, driving not required.

NOTE -- All lests run in accordance with applicable ASTM standards.



DD	Dry Density, pcf
WD	Wet Density, pcf
MC	Natural Moisture Content, %
LL .	Liquid Limit, %
PL	Plastic Limit, %
PI	Plasticity Index, %

LABORATORY TESTS

LIQUID LIMIT (LL)

R c

ML

OR OL

OC Organic Content, % S

MH OR OH

- Percent of Saturation, %
- SG Specific Gravity
- Cohesion C
- Ø
 - Angle of Internal Friction

00

Пo

qu Unconfined Compressive Strength

PLASTI

20

10

PLAN INDEX

SITE PLAN
BATHHOUSE PLANS
SYMBOLS – LEGENDS – GENERAL NOTES –
FLOOR PLAN AND FOUNDATION PLAN
BUILDING SECTIONS
ELEVATIONS (Exterior & Interior) AND DETAILS
WALL SECTIONS AND DETAILS
MECHANICAL
LIGHTING PLAN
POWER AND SYSTEMS PLAN
SITE PLAN (Exterior Lighting & Receptacles) —
POOL PLANS
POOL AND DECK PLAN
POOL AND DECK EQUIPMENT PLAN
POOL SECTIONS - UNDERDRAIN PLAN - DETAI
POOL SECTIONS AND DETAILS
WADING POOL - FILTER - EQUIPMENT RM. PLA
DETAILS AND SCHEDULES

CONSTRUCTION PLANS

FOR

SIDNEY PUBLIC SWIMMING POOL

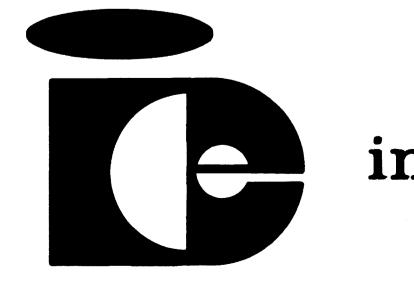
SIDNEY, MONTANA

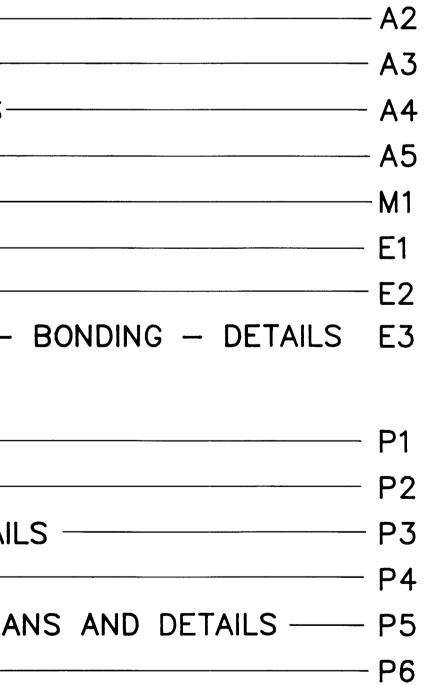
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MAYOR COUNCILM COUNCILM COUNCILM COUNCILM COUNCILM COUNCILM CITY CLEF DIRECTOR PUBLIC V





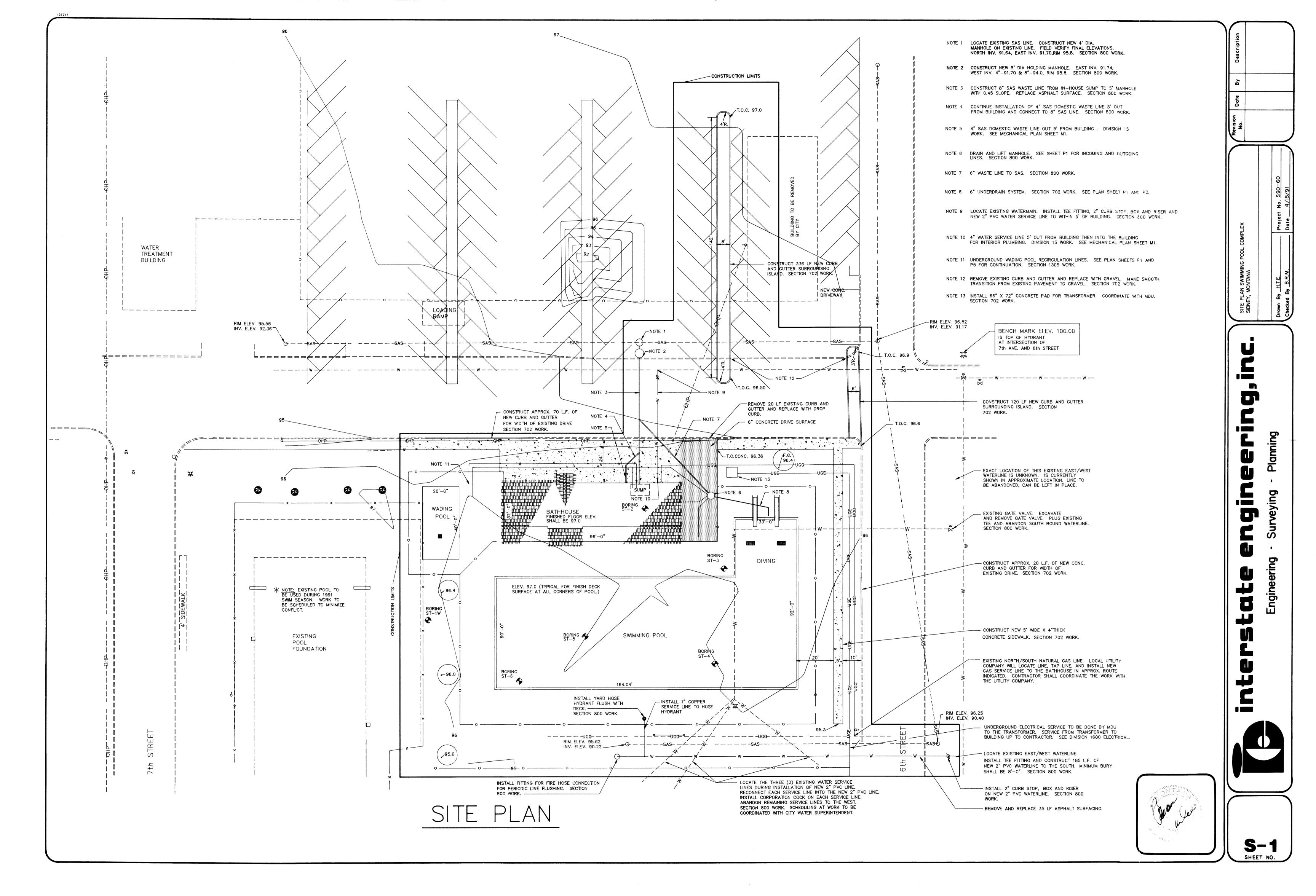
Attachment #5

CITY COUNCIL

	HAROLD MERCER
MAN	DUANE STICKNEY
MAN	CAL ORAW
MAN	RICHARD HOBBS
MAN	WILLIAM BARBER
MAN	WAYNE SWIGART
MAN	DON YADON
ERK	ETHEL SOBOLIK
R OF WORKS	TERRY MELDAHL

interstate engineering, inc.

ENGINEERING SURVEYING PLANNING



abbreviations

A.B. - ANCHOR BOLT ALT. - ALTERNATE APPROX. - APPROXIMATELY BD. - BOARD BLDG. - BUILDING CAB. - CABINET - CONTROL JOINT C.J. · CMU -- CONCRETE MASONRY UNIT CONC. - CONCRETE CONT. - CONTINUOUS CONSTR. - CONSTRUCTION D.S. - DOWN SPOUT EA. - EACH ELECT. - ELECTRICAL ELEV. - ELEVATION EXIST. - EXISTING EXP. - EXPANSION F.D. - FLOOR DRAIN FDN. - FOUNDATION F.E. - FIRE EXTINGUISHER F.R.P. - FIBERGLASS REINFORCED PANEL

F.F. - FINISHED FLOOR FIN. - FINISHED Fl.R. - Flooring Ftg. - Footing G.B. -GYPSUN BOARD HORIZ. - HORIZIONTAL H.M. - HOLLOW METAL INSUL. - INSULATION MANUF. - MANUFACTURER MAX. - MAXIMUN MECH. - MECHANICAL MIN. - MINIMUM MTL. - METAL NO. - NUMBER N.I.C. - NOT IN CONTRACT O.C. - ON CENTER 0.S.C.I. - OWNER SUPPLIED CONTR. INSTALLED PL. - PLATE PLYWD. - PLYWOOD R.O. - ROUGH OPENING REQ'D - REQUIRED SIM. - SIMILAR SQ. - SQUARE STO. - STORAGE

STL - STEEL T&G - TONGUE AND GROOVE T.O.F. - TOP OF FOOTING T.O.P. - TOP OF PLATE T.O.W. - TOP OF WALL TYP. - TYPICAL V.B. - VAPOR BARRIER VERT. - VERTICAL W.C. - WATER CLOSET W/ - WTHWGB. - WATER RESIST-ANT GYPSUM BOARD WWF. - WELDED WIRE FABRIC

<u>symbols legend:</u>

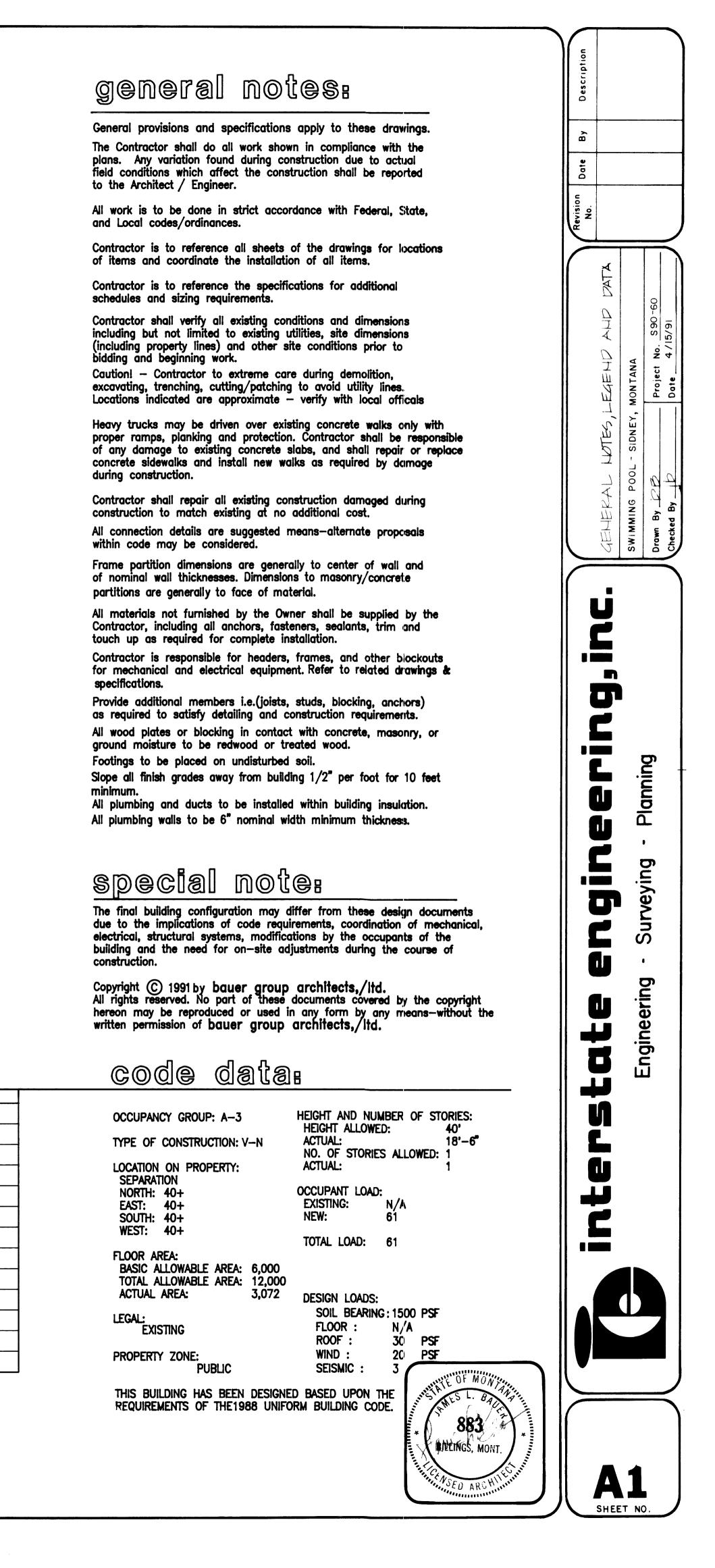
INMER SKET	DETAIL	ELEY=EXIST ELEV=NEW] SPOT Elevation
	Building Section	,)	EXISTING CONTOUR
	- WALL SECTION		NEW Contour
	ROOM NAME & NUMBER		CONTRACT LI
	DOOR NUMBER		NORTH ARRO
<u>3-4°x7-4°</u> (2)	WINDOW NUMBER	M (A) grid line
2-1 x4-1 D <u>1.0.</u> PLATE 108'-0'	REFERENCE Elevation		NEW Constructio
	center line Rated Wall		VEGETATION
		4/115	

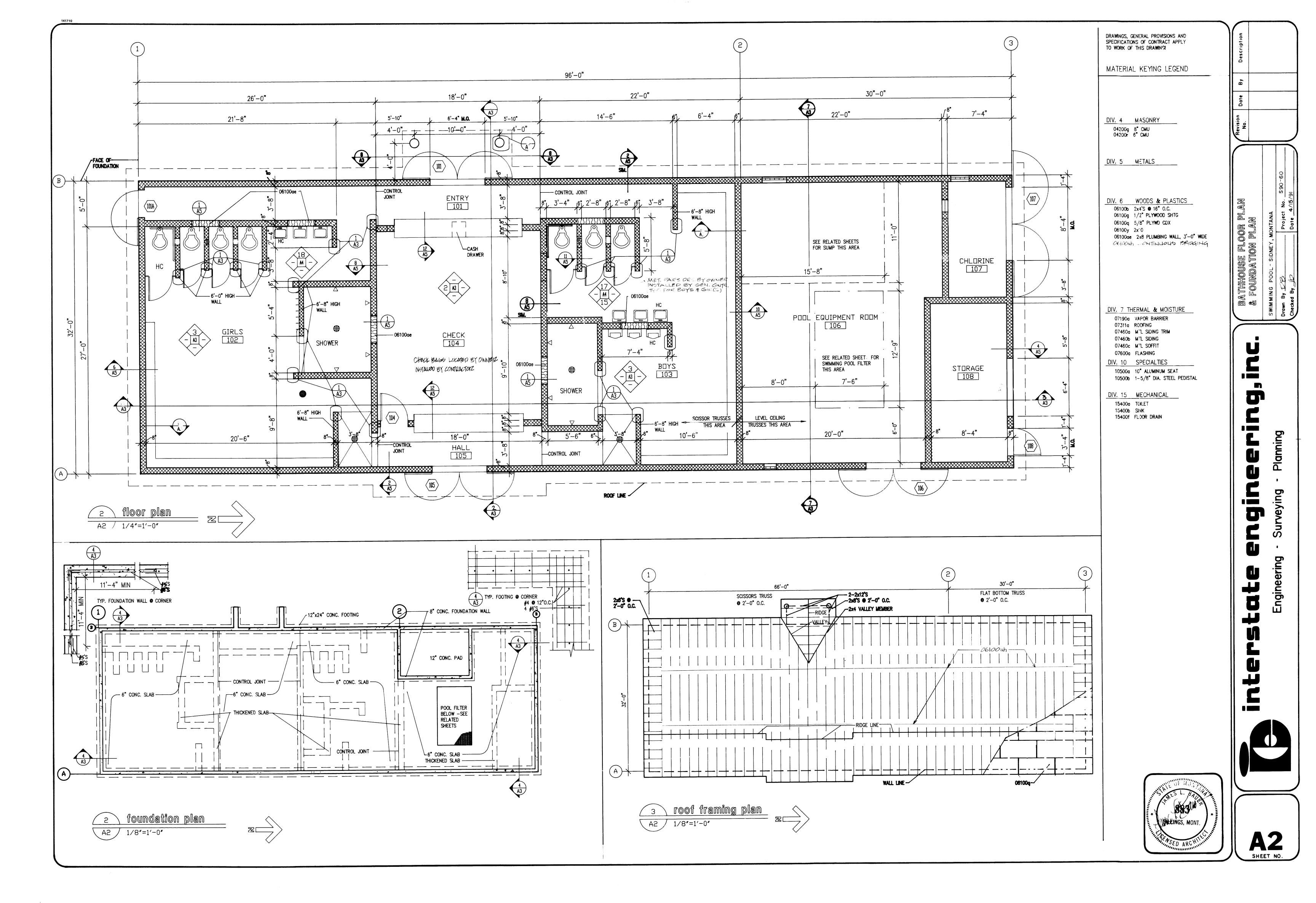
EXISTING CONSTRUCTION LEVATION - STORM SEWER existing Contour NEW Contour FI ECTRICAL ----- TELEPHONE ----- T ONTRACT LIMITS **SANITARY** WORTH ARROW SEWER ----- & ------ NATURAL GAS GRID LINE ELECTRICAL DNSTRUCTION TELEVISION

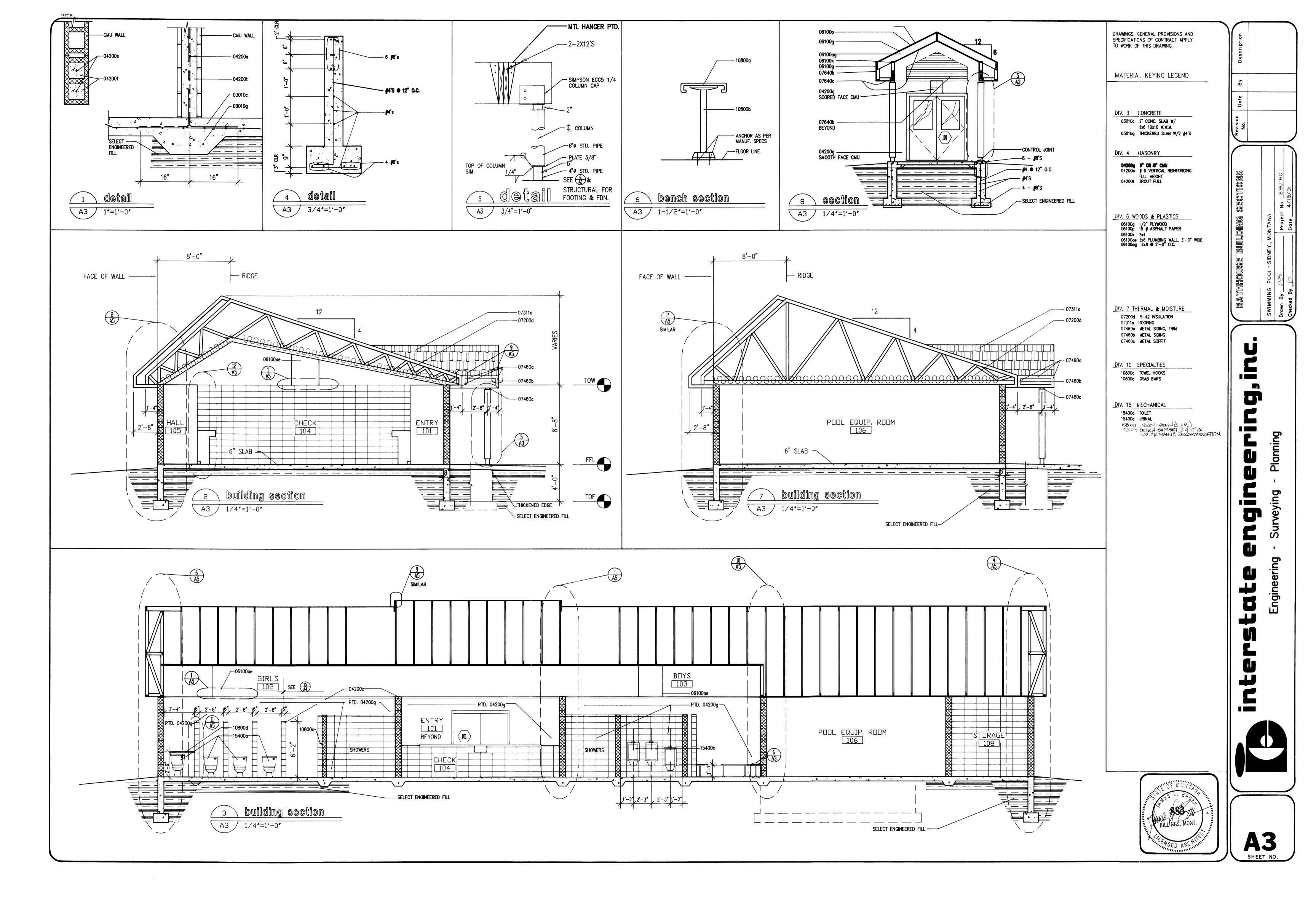
NDERGROUND

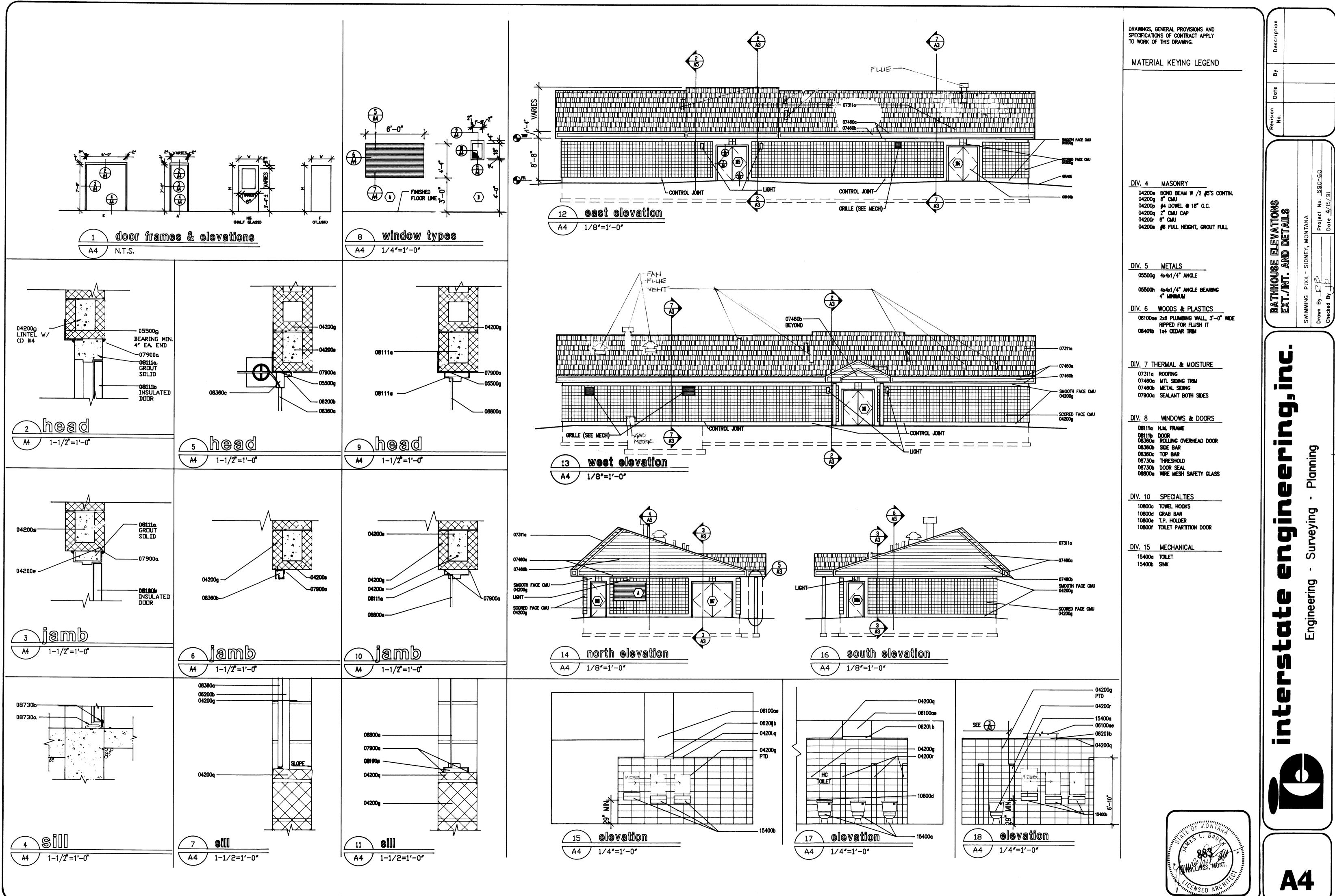
mat'l legend:

NDICATED	SYMBOL	DESCRIPTION
		CONCRETE
X	\succ	DIMENSIONAL LUMBER SECTION
		BLOCKING SECTION
		FINISH LUMBER SECTION
X	1323	BATT OR BLANKET INSULATION
X		PLYWOOD
		gypsum board
	\bigotimes	C.M.U.
X		MASONRY (BRICK)
X		COMPACTED FILL
		UNDISTURBED SOIL
		COMPACTED GRAVEL FILL
X		STEEL

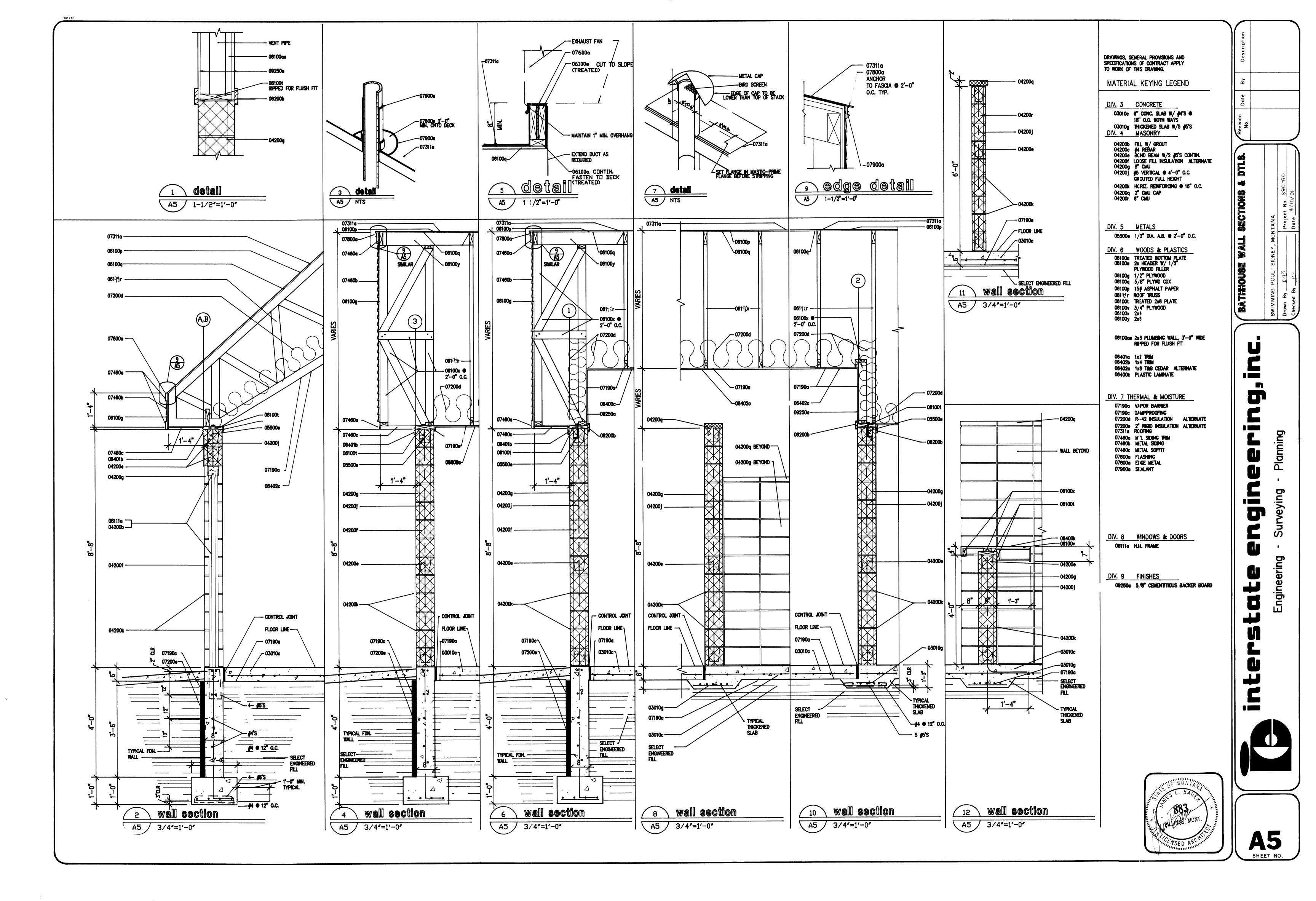


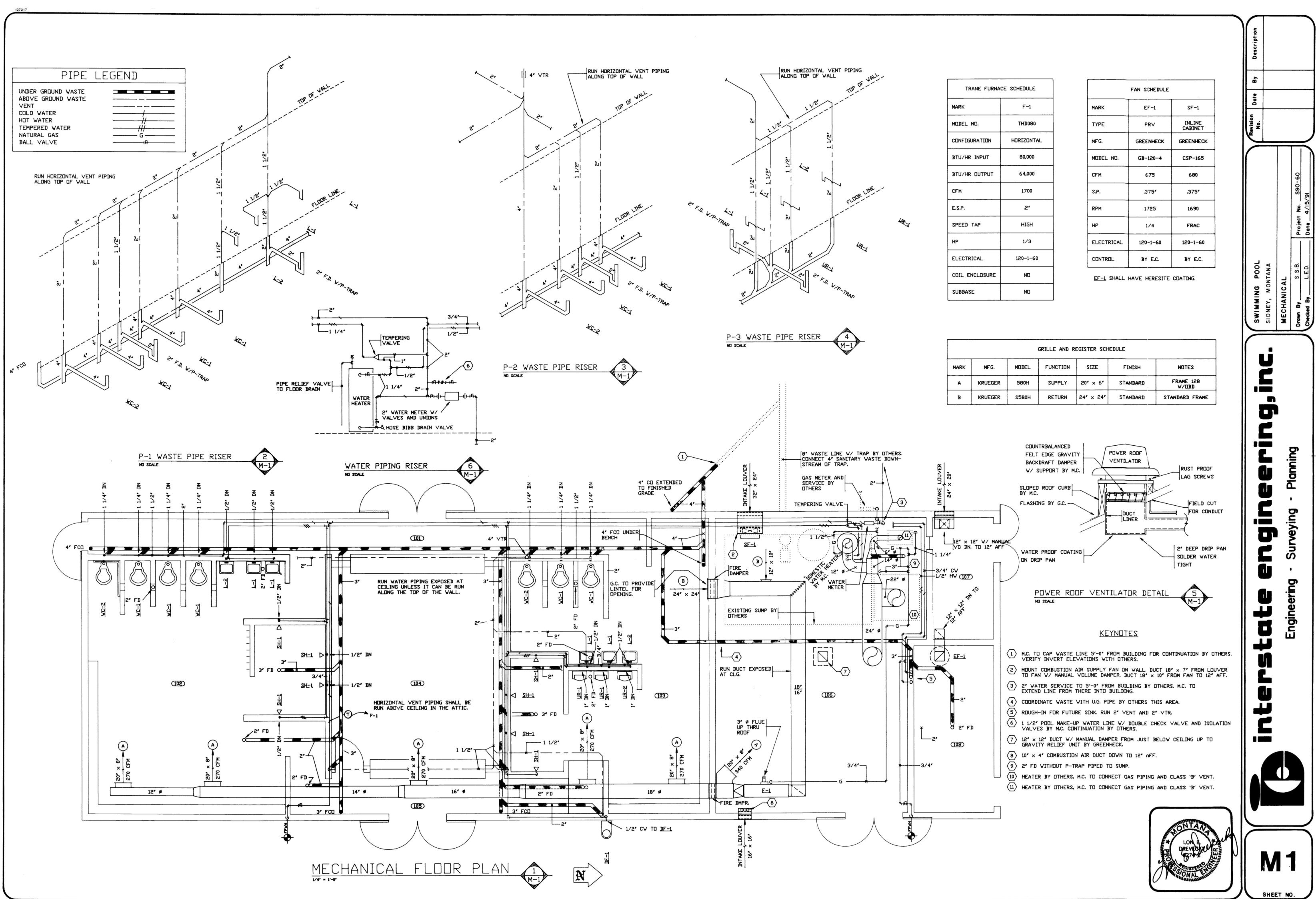






SHEET NO.

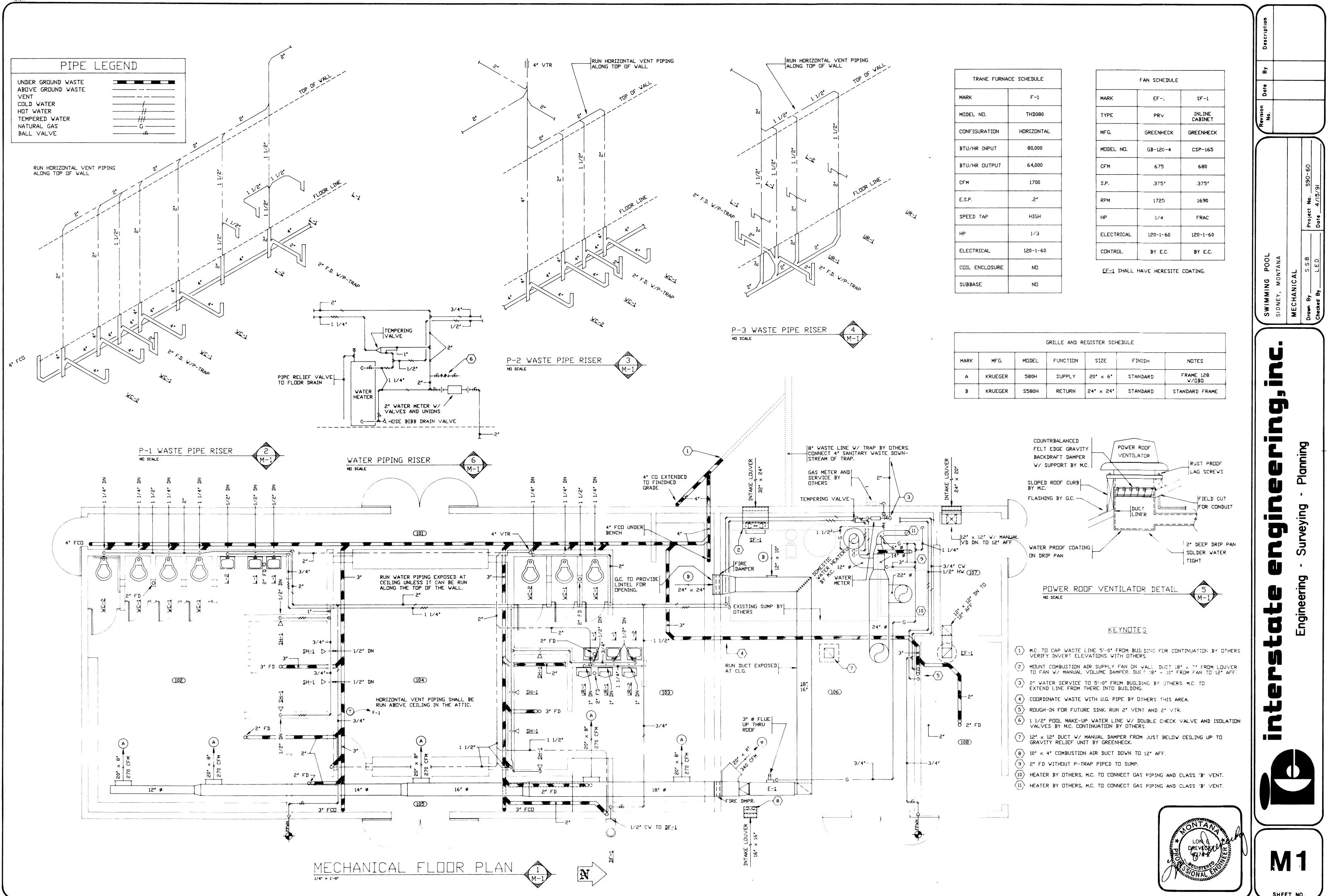




<u>-</u>
)
TAL.
I
50

	FAN SCHEDULE	
MARK	EF-1	SF-1
TYPE	PR∨	INLINE CABINET
MFG.	GREENHECK	GREENHECK
MODEL NO.	GB-120-4	CSP-165
CFM	675	680
S.P.	.375 *	.375*
RPM	1725	1690
HP	1/4	FRAC
ELECTRICAL	120-1-60	120-1-60
CONTROL	BY E.C.	BY E.C.

			GRILLE AND R	EGISTER SCHE	DULE	
MARK	MF G.	MODEL	FUNCTION	SIZE	FINISH	NOTES
A	KRUEGER	580H	SUPPLY	20" × 6"	STANDARD	FRAME 128
В	KRUEGER	S280H	RETURN	24" × 24"	STANDARD	STANDARD FRAME

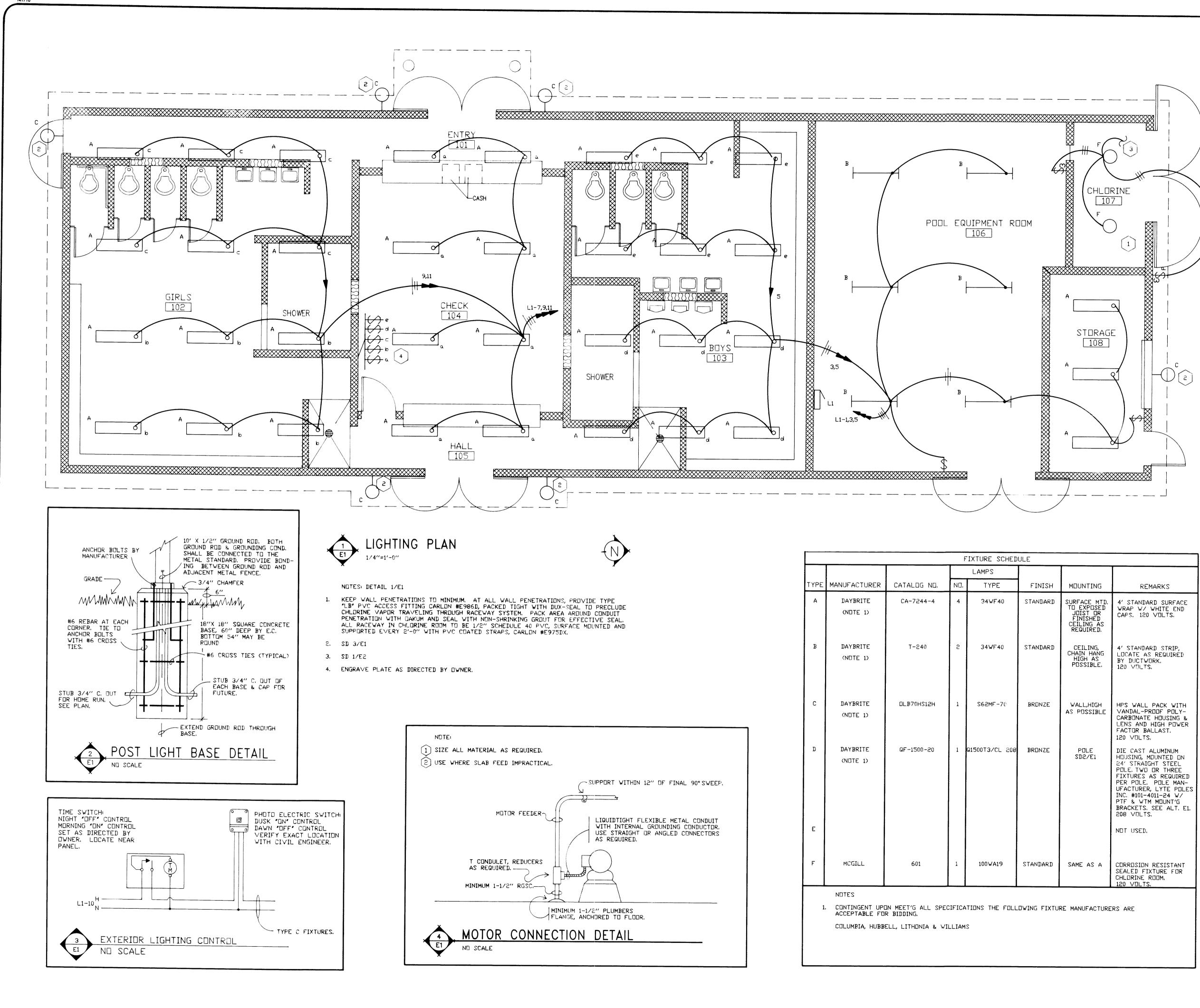


TRANE FURNA	CE SCHEDULE
MARK	F-1
MODEL NO.	THD080
CONFIGURATION	HORIZONTAL
BTU/HR INPUT	80,000
BTU/HR DUTPUT	64,000
CFM	1700
E.S.P.	.2*
SPEED TAP	HIGH
HP	1/3
ELECTRICAL	120-1-60
COIL ENCLOSURE	ND
SUBBASE	ND

	FAN SCHEDULE	
MARK	EF-1	SF-1
TYPE	PR∨	INLINE CABINET
MFG.	GREENHECK	GREENHECK
MODEL NO.	GB-120-4	CSP-165
CFM	675	680
S.P.	.375′	.375*
RPM	1725	1690
HP	1/4	FRAC
ELECTRICAL	120-1-60	120-1-60
CONTROL.	BY E.C.	BY E.C.

			GRILLE AND R	EGISTER SCHE	IDULE	
MARK	MF'G.	MODEL	FUNCTION	SIZE	FINISH	NOTES
A	KRUEGER	580H	SUPPLY	20 . × 9.	STANDARD	FRAME 128 V/DBD
В	KRUEGER	S280H	RETURN	24" × 24"	STANDARD	STANDARD FRAME

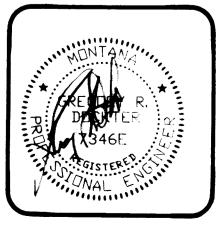
SHEET NO.

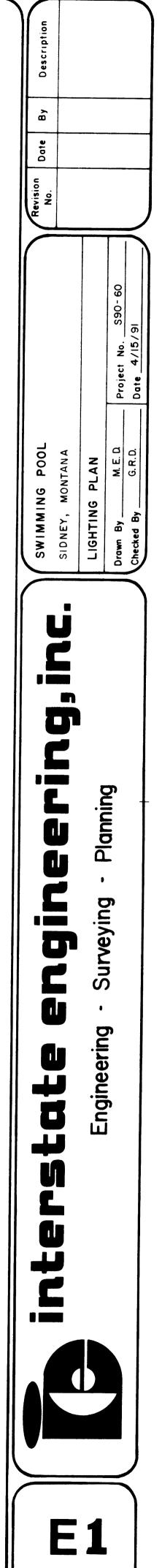


			F	IXTURE SCHED	ULE		
				LAMPS			
TYPE	MANUFACTURER	CATALOG NO.	NO,	TYPE	FINISH	MOUNTING	REMARKS
A	DAYBRITE (NOTE 1)	CA-7244-4	4	34WF40	STANDARD	SURFACE MTD. TO EXPOSED JOIST OR FINISHED CEILING AS REQUIRED.	4' STANDARD SURFACE WRAP W/ WHITE END CAPS, 120 VOLTS,
В	DAYBRITE (NDTE 1)	T-240	2	34WF40	STANDARD	CEILING, CHAIN HANG HIGH AS POSSIBLE.	4' STANDARD STRIP, LOCATE AS REQUIRED BY DUCTWORK, 120 VOLTS,
С	DAYBRITE (NDTE 1)	DLB70HS12H	1	S62MF-70	BRÜNZE	WALL,HIGH AS POSSIBLE	HPS WALL PACK WITH VANDAL-PROOF POLY- CARBONATE HOUSING & LENS AND HIGH POWER FACTOR BALLAST. 120 VOLTS.
ם	DAYBRITE (NOTE 1)	QF-1500-20	1	Q1500T3/CL 208	BRDNZE	POLE SD2/E1	DIE CAST ALUMINUM HOUSING, MOUNTED ON 24' STRAIGHT STEEL POLE, TWO OR THREE FIXTURES AS REQUIRED PER POLE, POLE MAN- UFACTURER, LYTE POLES INC. #101-4011-24 W/ PTF & WTM MOUNT'G BRACKETS, SEE ALT, E1, 208 VOLTS.
E							NDT USED.
F	MCGILL	601	1	100WA19	STANDARD	SAME AS A	CORROSION RESISTANT SEALED FIXTURE FOR CHLORINE ROOM, 120 VOLTS.
	NDTES						

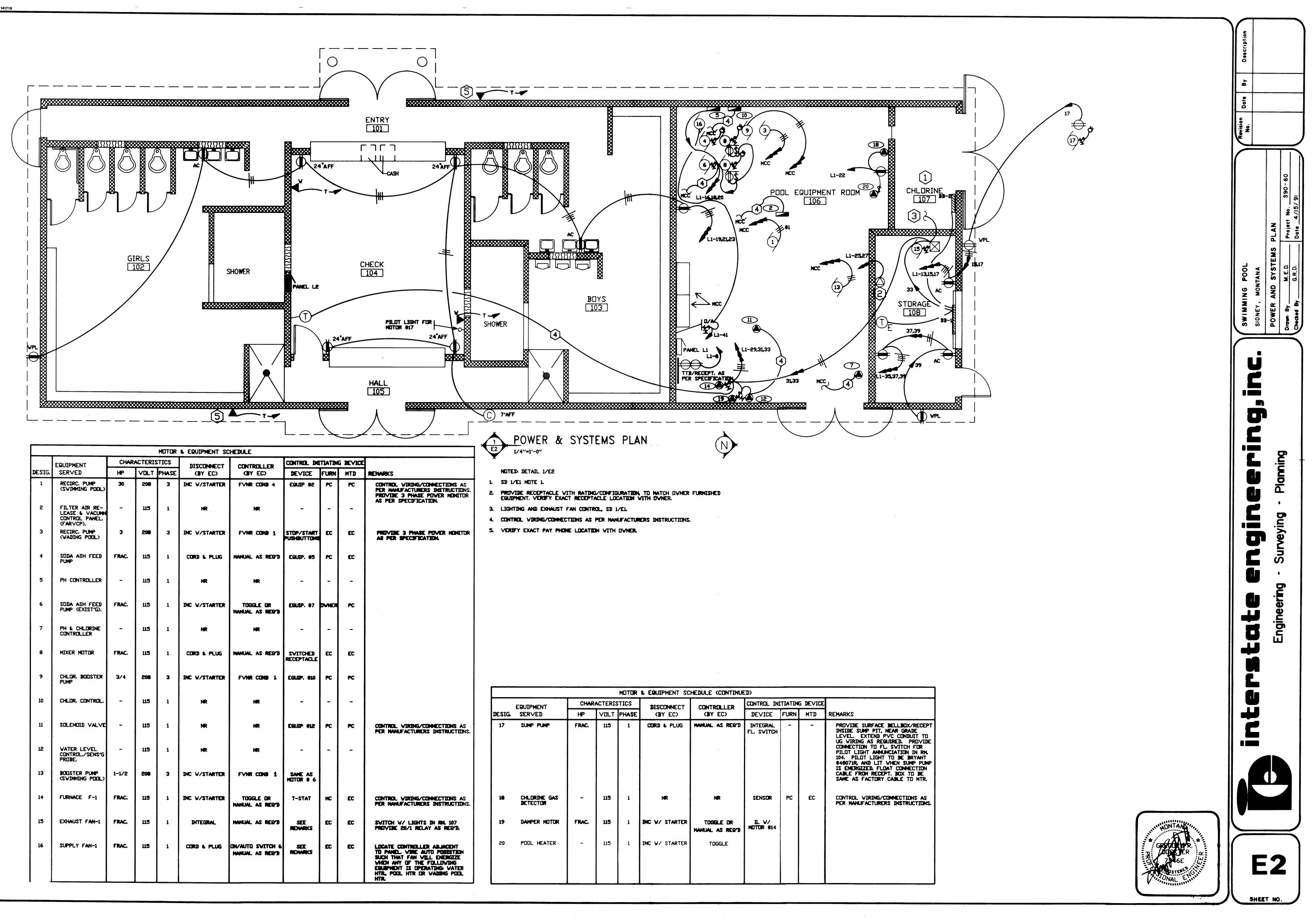
OVALIDATI	MOUNTING HEIGHT MEASUREMENTS SHALL DE MADE FROM FINISH FLOOR TO CENTER LINE OF OUTLET	
SYMBOL	DESCRIPTION LIGHTING DUTLETS	MTG. HT
*O:	CEILING INCANDESCENT DR HID. TYPE & CKT 1, SV b.	
	WALL INCANDESCENT OR HID. FLUDRESCENT, SLASH INDICATES INBOARD UNSVITCHED EN CKT.	SCHEDULE
	BARE LAMP FLUDRESCENT STRIP VITH VIRE GUARD.	
Ð	EXIT SIGNAGE	
	EMERGENCY BATTERY UNIT REMOTE SEALED BEAM	SCHEDULE SCHEDULE
	PORCELAIN LAMPHOLDER. 1004 LAMP	SCHEDOLE
J	JUNCTION BOX	
Θ^1	RECEPTACLE DUTLETS	18"
Ð	DUPLEX	18"
⊕ ≜	DUPLEX - SPLIT VIRED DUPLEX - GROUND FAULT CIRCUIT INTERRUPTER	18"
⊕ ^	SPECIAL CONFIGURATION, DESIGNATION REFERS TO SCHEDULE	18" SCHEDULE
t_∯ ¹⁸ •	MULTI-OUTLET ASSEMBLY, ARRIV'S EXTEND TO LIMIT OF INSTALLATION, SUBSCRIPT INDICATES SPACING OF DUTLETS	
 -©	CLOCK RECEPTACLE	82''
\odot	DUPLEX RECEPTACIE - FLOOR BOX	
\$	SWITCH BUTLETS	
\$2 \$2	DOUBLE POLE	50" 50"
\$3	THREE-WAY	50"
\$ ₄ \$κ	FOUR-WAY	50"
φĸ	KEY DPERATED MOTOR - PROVIDE OVERLOAD UNIT AS REG'D. TOGGLE	
\$n	ACCEPTABLE IF INTERNAL THERMAL PROTECTION INCLUDED, SWITCH NOT REQUIRED IF NOTOR ASSEMBLY	
¢	HAS INTEGRAL DISCONNECTING MEANS.	
\$р \$т	PILOT HANDLE TIME DELAY	50" 50"
Þ	DIMMER - 1000W UNLESS OTHERVISE INDICATED	3 6"
\$ \$ 100	GANGED SVITCHES - ARROV INTICATES MULTI-LEVEL SVITCHING	50"
	COMMUNICATION/DATA SYSTEM DUTLET	
	TELEPHONE DUTLET	1 5 11
⊴⊲∲∲∲∲∲∲⊚⊘	TELEPHONE DUTLET OR COMPUTER/VDT - FLOOR BOX	15''
<u>(</u> B)	BELL	
٩	BUZZER	
	INTERCOM STATION	58″
Ŵ		18"
\sim	TELEVISION DUTLET	18"
×	PUSH BUTTON	58''
š	SPEAKER/BAFFLE/BACKBOX COMBINATION	CETLING
\odot	CLICK	84"
0	MISCELLANEDUS	
ě	SPECIAL PURPOSE CONNECTION - AS REQUIRED BY EQUIPMENT	
-	MANUFACTURER, CO-ORDINATE ROUGH-IN VITH SHOP DVG.	
	BRANCH CIRCUIT PANELBOARD SHADED INDICATE FLUSH MOUNT CONTROL PANEL	TOP 75"
	EXTERNALLY OPERATED DISCONNECT SVITCH	
	CONTROLLER OR RELAY	
×	COMBINATION CONTROLLER AND DISCONNECTION MEANS	
(1)	MOTOR, DESIGNATION REFERS TO SCHEDULE.	
2 BB-1	EQUIPMENT DESIGNATION, SEE SCHEDULE.	
I DIN BB-1	ELECTRIC HEAT TO SCALE. DESUGNATION REFER TO SCHEDULE. 'T' INDICATES INTEGRAL THERMOSTAT.	
٦E	THERMOSTAT-PROVIDED BY DIV. 16	58"
T B	THERMOSTAT-FURNISHED BY DIV. 15, INSTALLED BY DIV. 16 HUMIDISTAT-FURNISHED BY DIV. 15, INSTALLED BY DIV. 16	58" 58"
P	POTENTIONETER-FURNISHED BY DIV. 15, INSTALLED BY DIV. 16 POTENTIONETER-FURNISHED BY DIV.15, INSTALLED BY DIV.16	58″ 58″
	TIME SWITCH	
Ps		
#	CIRCUITING HOME RUN, MIN 3/4" C. ARROVS AND SUBSCRIPTS INDICATE	
	NUMBER AND IDENTIFICATION OF CIRCUITS. EMERGENCY. MIN 1/2" C-010 AVG.	
~!~	TELEPHONE MIN 3 (AND HOLE ONLY TO TELEVISION	
	TELEPHONE, MIN 3/4"C. STUB INTO CELLING SPACE	
	LOW VOLTAGE. MIN 1/2"C-014 AWG AS RED'D.	
	AS REQ'D BY MANUFACTURER. SUBSCRIPT INDICATES	
	SYSTEM. SEE STANDARD ABBREVIATIONS.	
	STANDARD ABBRE VIATIONS	
ABOVE FINIS		
ABOVE FINIS	UNIT MCA MINIMUM CIRCUIT A	
BOTTOM OF F	The second s	D MUSIC
CIRCUIT	EL PA PUBLIC ADDRESS	
CONDENSING CABINET UNIT	UNIT RF RELIEF FAN	
	CUNTRACTOR RT RAIN-TIGHT	
DUST TIGHT	S SECURITY	
ELECTRICAL (EXHAUST FAN EMERGENCY	SD SEE DETAIL	
ELECTRICAL & EXHAUST FAN EMERGENCY EXPLOSION PI ELECTRIC VA	RODF SS SUPPRESSION	N
ELECTRICAL & EXHAUST FAN EMERGENCY EXPLOSION PI ELECTRIC WA ELECTRIC WA FUSED	RODF SS SUPPRESSID TER COOLER SV SVITCH TER HANDLER TOP TOP OF PANEL T TELEPHONE	N
ELECTRICAL & EXHAUST FAN EMERGENCY EXPLOSION PI ELECTRIC VA ELECTRIC VA FUSED FULL LUAD A FIRE ALARM	RODF SS SUPPRESSID TER CODLER SV SVITCH TER HANDLER TOP TOP OF PANEL T TELEPHONE TERMIN WPERES TTB TELEPHONE TERMIN VDT VLVED DISPLAY TEL	n l Boa rd
ELECTRICAL & EXHAUST FAN EMERGENCY EXPLOSION PI ELECTRIC VA ELECTRIC VA FUSED FULL LUAD A FIRE ALARM FIRE ALARM	RODF SS SUPPRESSID TER CODLER SV SVITCH TER HANDLER TOP TOP OF PANEL T TELEPHONE TERMIN MPERES TTB TELEPHONE TERMIN VDT VIDED DISPLAY TEL CONTROL PANEL VT VAPOR TIGHT E NON REVERSING V VALL MTG. CBM AR	nl Board Rhinal

SPECIAL ABBREVIATIONS PECILIAR TO THIS PROJECT PC POOL CONTRACTOR VPL VEATHERPROOF LOCKING



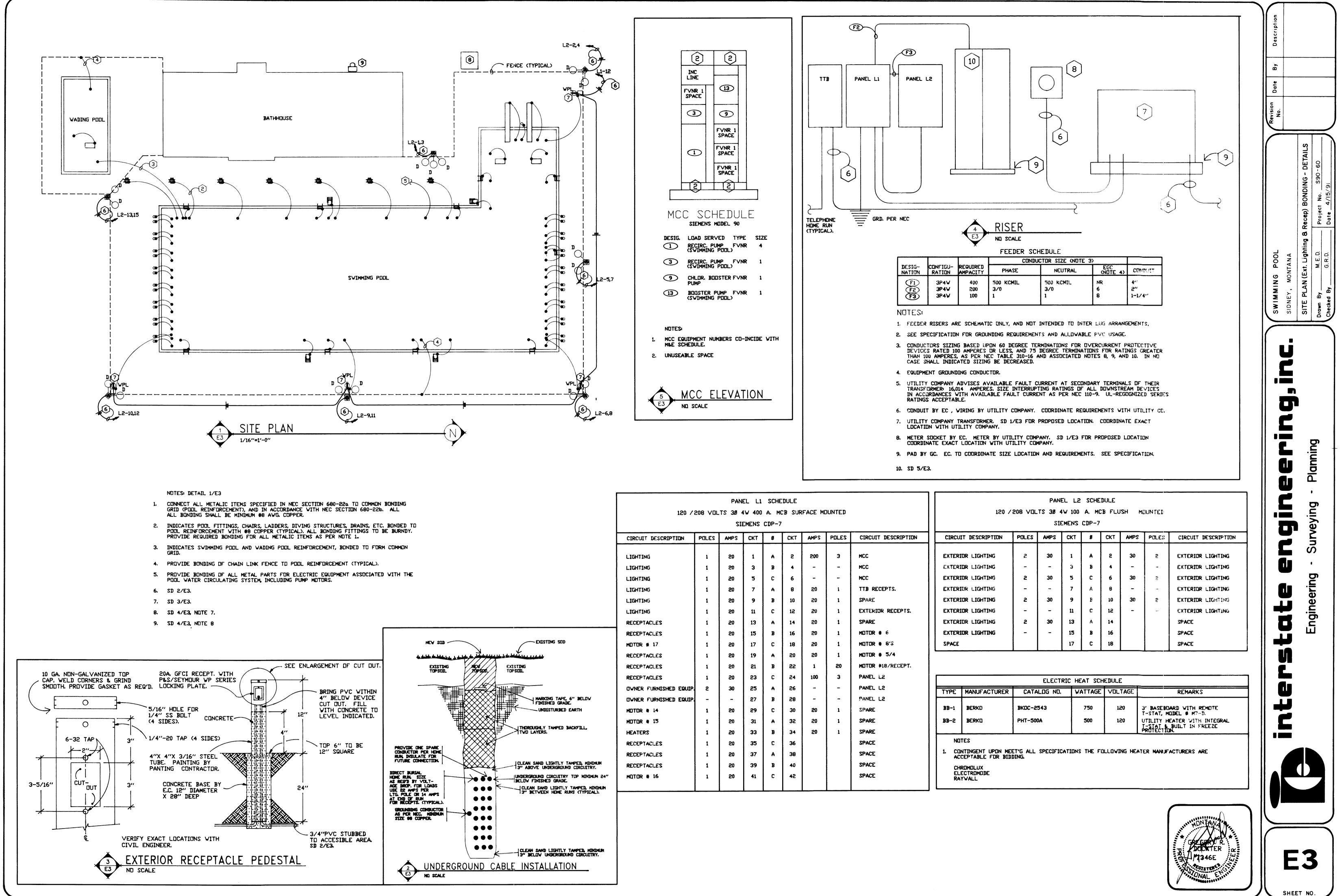


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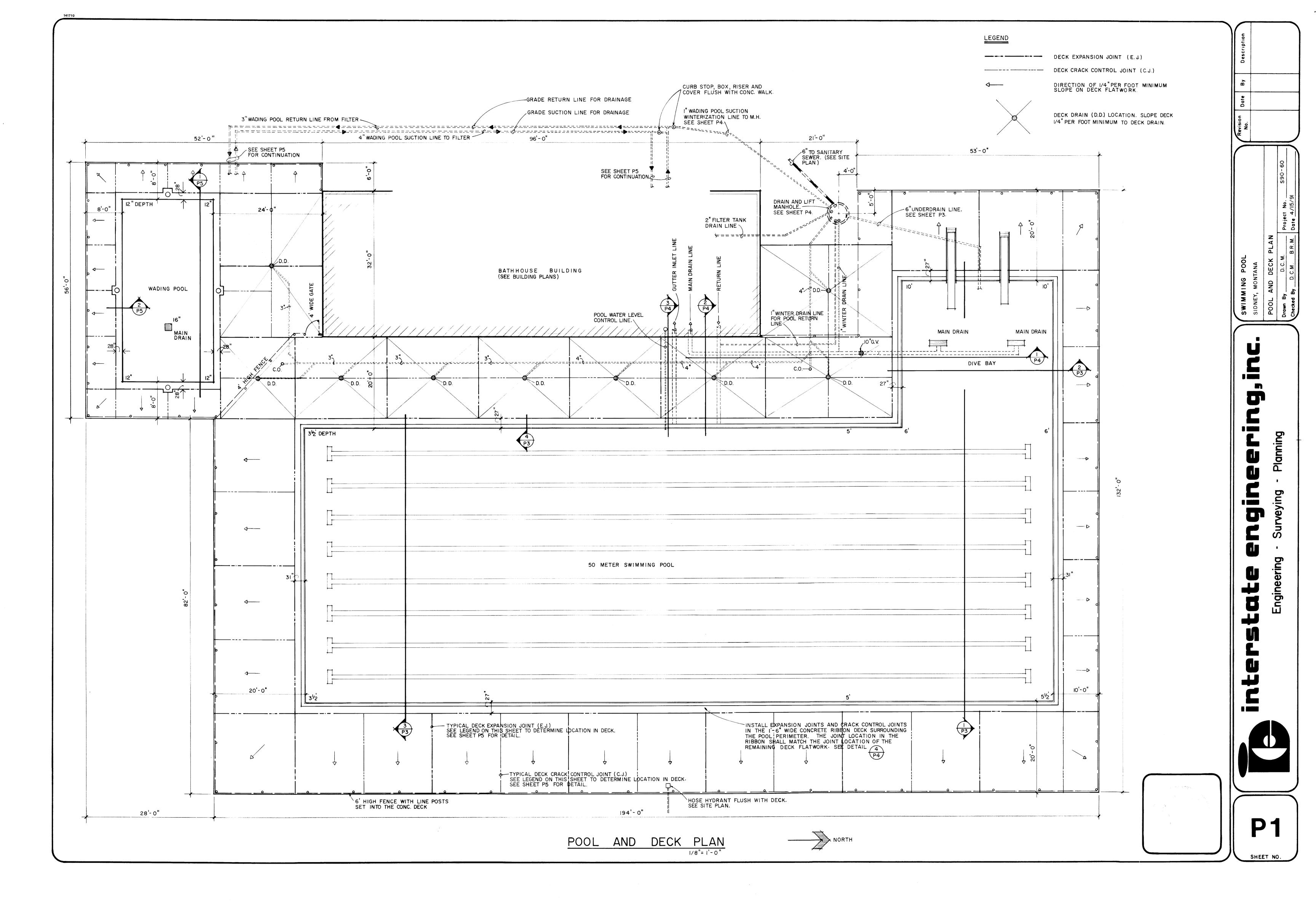


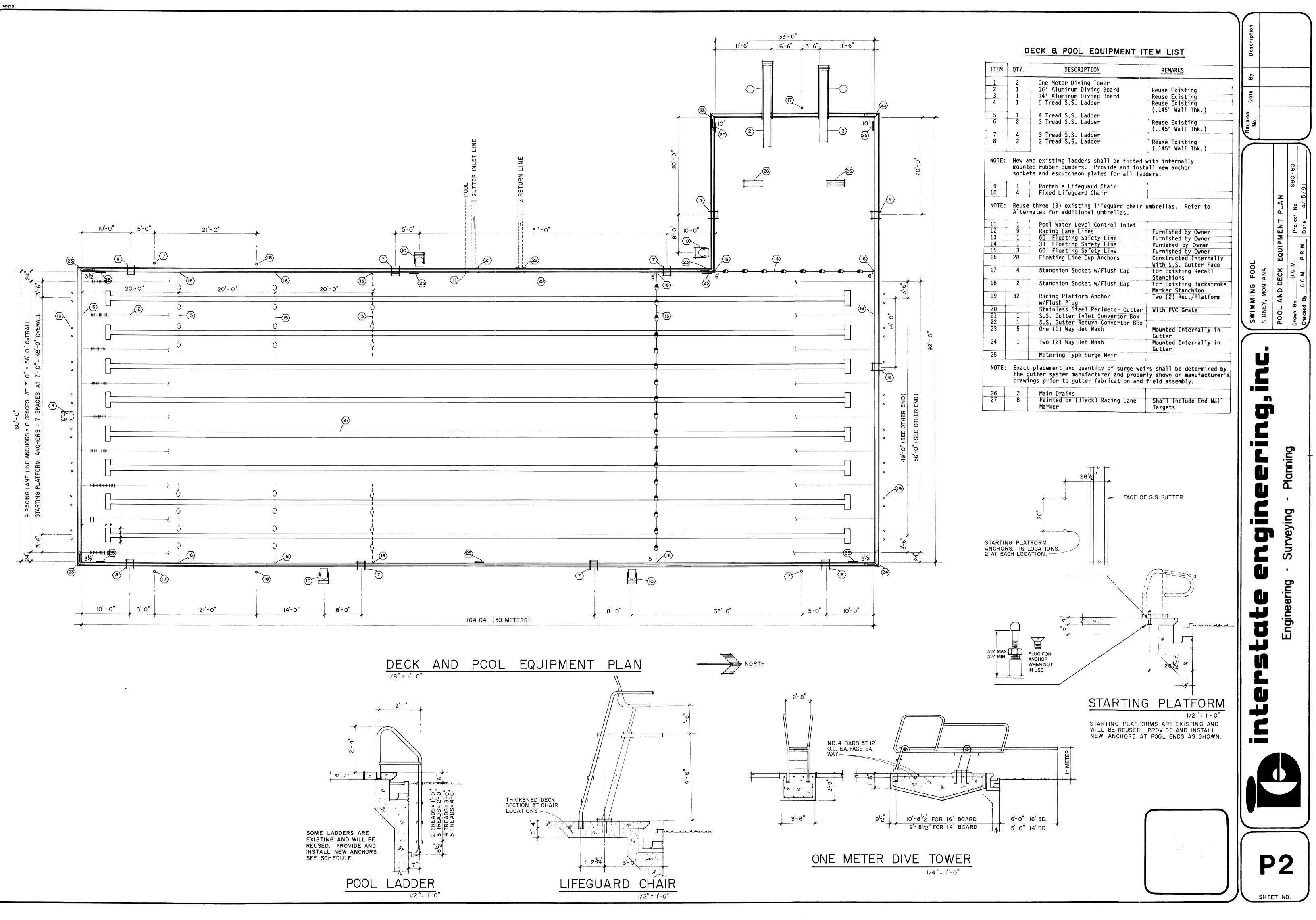
				TICS		T				
DESIG.	EQUIPMENT SERVED	HP	ACTERIS	PHASE	DISCONNECT	CONTROLLER (BY EC)	CONTROL IN			
1	RECIRC. PUMP (SWIMMING PDDL)	30	208	3	(BY EC) INC V/STARTER	FVNR COHB 4	DEVICE	FURN PC	MTD PC	REHARKS CONTROL VIRD PER HANUFACT PROVIDE 3 PH
2	FILTER AIR RE- LEASE & VACUMN CONTROL PANEL. (FARVCP).	-	115	1	NR	NR	-	-	-	AS PER SPECI
3	RECIRC, PUMP (WADING POOL)	3	208	3	INC V/STARTER	FVNR CONB 1	STOP/START PUSHBUTTONS	æ	33	AS PER SPECE
4	SODA ASH FEED Pump	FRAC.	115	1	Cord & Plug	HANUAL AS RED'D	EQUIP. #5	PC	£C	
5	PH CONTROLLER	-	115	1	NR	NR	-	-	-	
6	SODA ASH FEED Pump (exist'g).	FRAC.	115	1	INC V/STARTER	toggle or Manual as req19	EQUIP. \$7	DVNER	PC	
7	PH & CHLORINE Controller	-	115	1	NR	NR	-	-	-	
8	MIXER MOTOR	FRAC.	115	1	cord & plug	HANUAL AS REPTD	SVITCHED RECEPTACLE	£C	23	
9	Chlor. Booster Pump	3/4	208	3	INC W/STARTER	FVNR CONS 1	EQUIP. 8 10	PC	PC	
10	CHLOR. CONTROL.	-	115	1	NR	NR	-	-	-	
11	SOLENOID VALVE	-	115	1	NR	NR	EQUIP #12	PC	PC	CONTROL VIRD PER HANUFACTI
12	VATER LEVEL CONTROL./SENS'G PROBE.	-	115	1	NR	NR	-	-	-	
13	BOOSTER PUMP (Svimming Pool)	1-1/2	208	3	INC V/STARTER	FVNR CONB 1	SAME AS HILTOR # 6			
14	FURNACE F-1	FRAC.	115	1	INC V/STARTER	toggle dr Manual as regyd	T-STAT	нс	£C	control vird Per Hanufacti
15	exhaust f an- 1	FRAC.	115	1	INTEGRAL	HANUAL AS RED'D	SEE REIVARKS	EC	£C	SVITCH V/ LI PROVIDE 20/1
16	SUPPLY FAN-1	FRAC.	115	1	cord & plug	on/auto svitch & Hanual as repyd	SEE Reivarks	£	EC	LOCATE CONTRI TO PANEL. VIR SUCH THAT FAI VHEN ANY OF EQUIPMENT IS HTR, POOL HTF HTR,

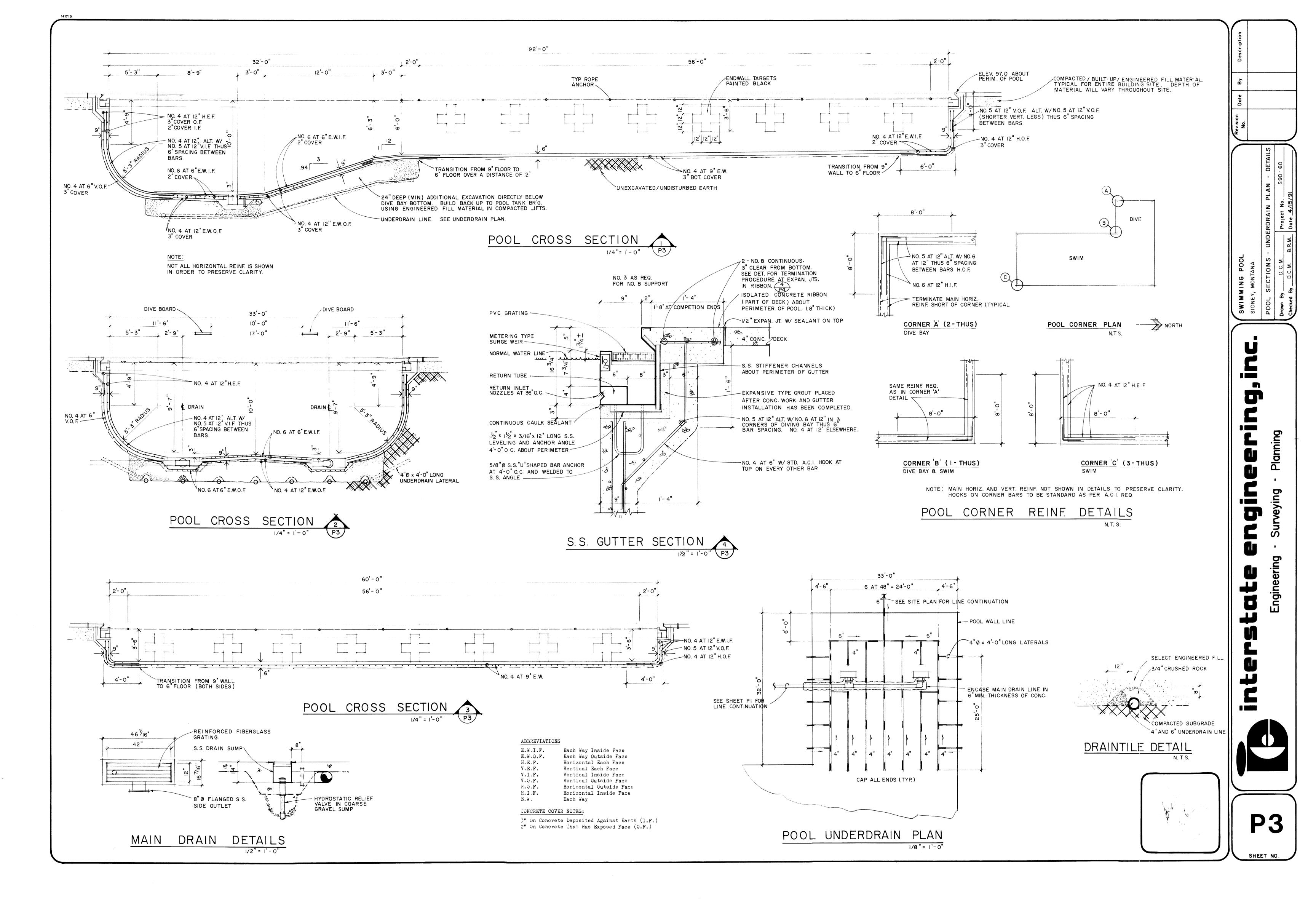
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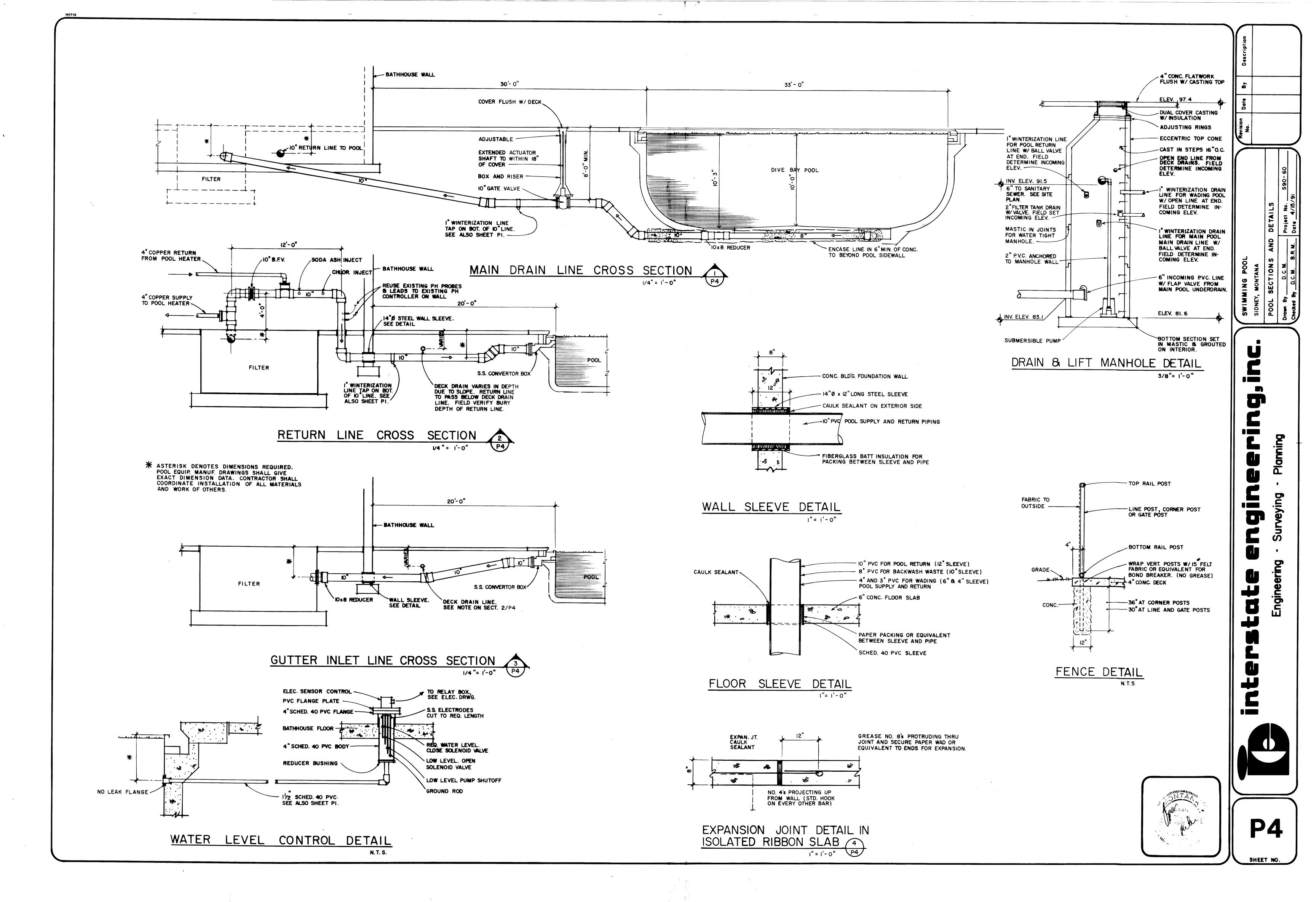


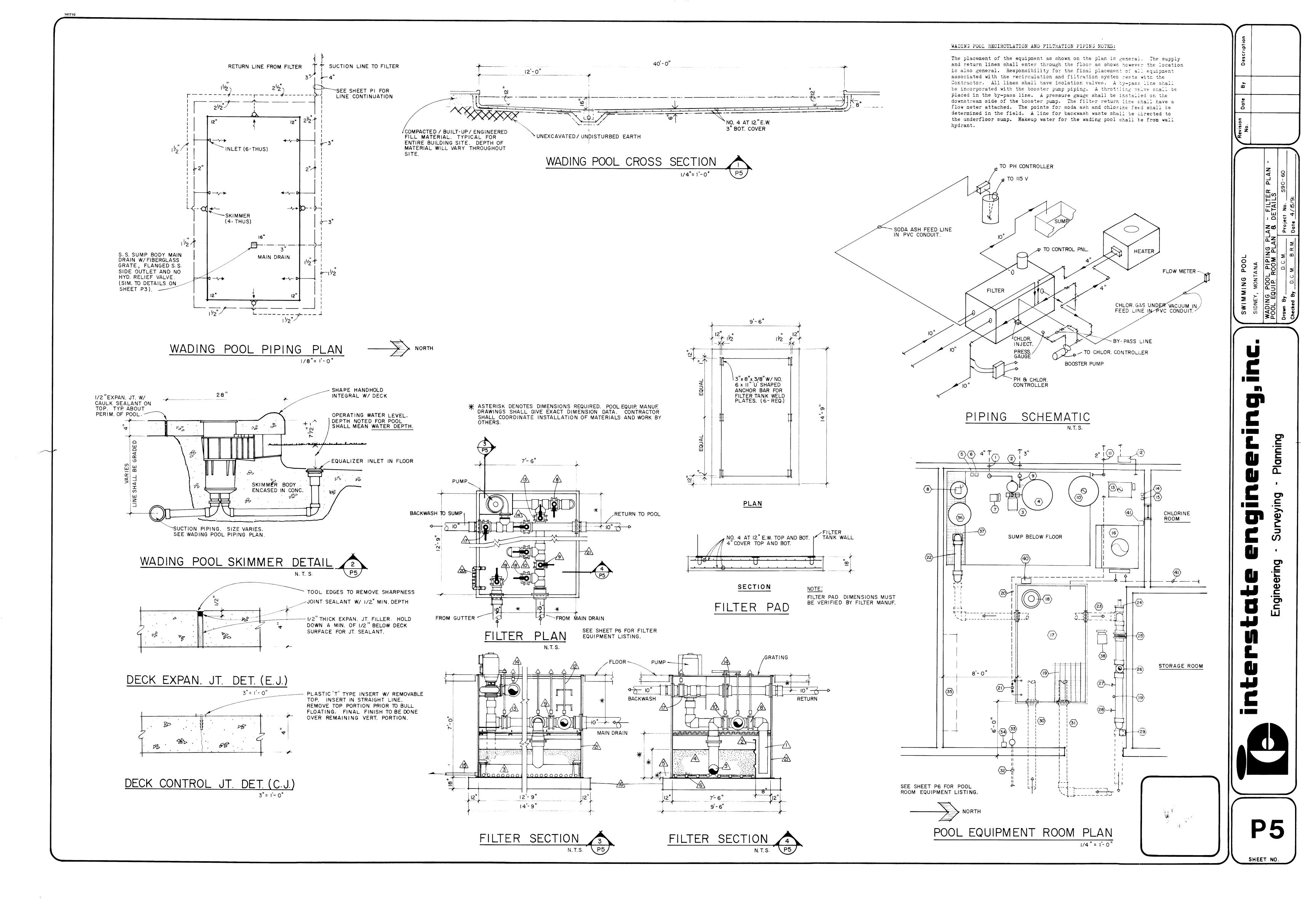
		PAN	EL L1	SCHE	DULE			
120 / 3	208 VOL	TS 30 4	ŧ₩ 400	A. MC	B SUR	FACE M	UNTED	
		SI	EMENS	CDP-7				
CIRCUIT DESCRIPTION	POLES	AMPS	СКТ	Ø	СКТ	AMPS	POLES	CIRCUIT DESCRIPTION
LIGHTING		20	1	A	2	200	3	MCC
	1	50	3	B	4	-	-	MCC
	1		5	с С				MCC
	1	20			6	-		TTB RECEPTS.
	1	20	7	A	8	20		
LIGHTING	1	20	9	B	10	20		SPARE
LIGHTING	1	50	11	С	12	20	1	EXTERIOR RECEPTS.
RECEPTACLES	1	20	13	A	14	20	1	SPARE
RECEPTACLES	1	50	15	B	16	50	1	MOTOR # 6
MOTOR # 17	1	20	17	С	18	20	1	MOTOR # 8'S
RECEPTACLES	1	20	19	A	20	20	1	MOTOR # 5/4
RECEPTACLES	1	50	21	В	55	1	20	MOTOR #18/RECEPT.
RECEPTACLES	1	20	23	С	24	100	3	PANEL L2
OWNER FURNISHED EQUIP.	z	30	25	A	26	-	-	PANEL L2
OWNER FURNISHED EQUIP.	-	-	27	В	28	-	-	PANEL L2
MOTOR # 14	1	50	29	С	30	20	1	SPARE
MOTOR # 15	1	20	31	A	32	20	1	SPARE
HEATERS	1	20	33	В	34	20	1	SPARE
RECEPTACLES	1	50	35	С	36			SPACE
RECEPTACLES	1	20	37	A	38			SPACE
RECEPTACLES	1	20	39	В	40			SPACE
MOTOR # 16	1	20	41	с	42			SPACE

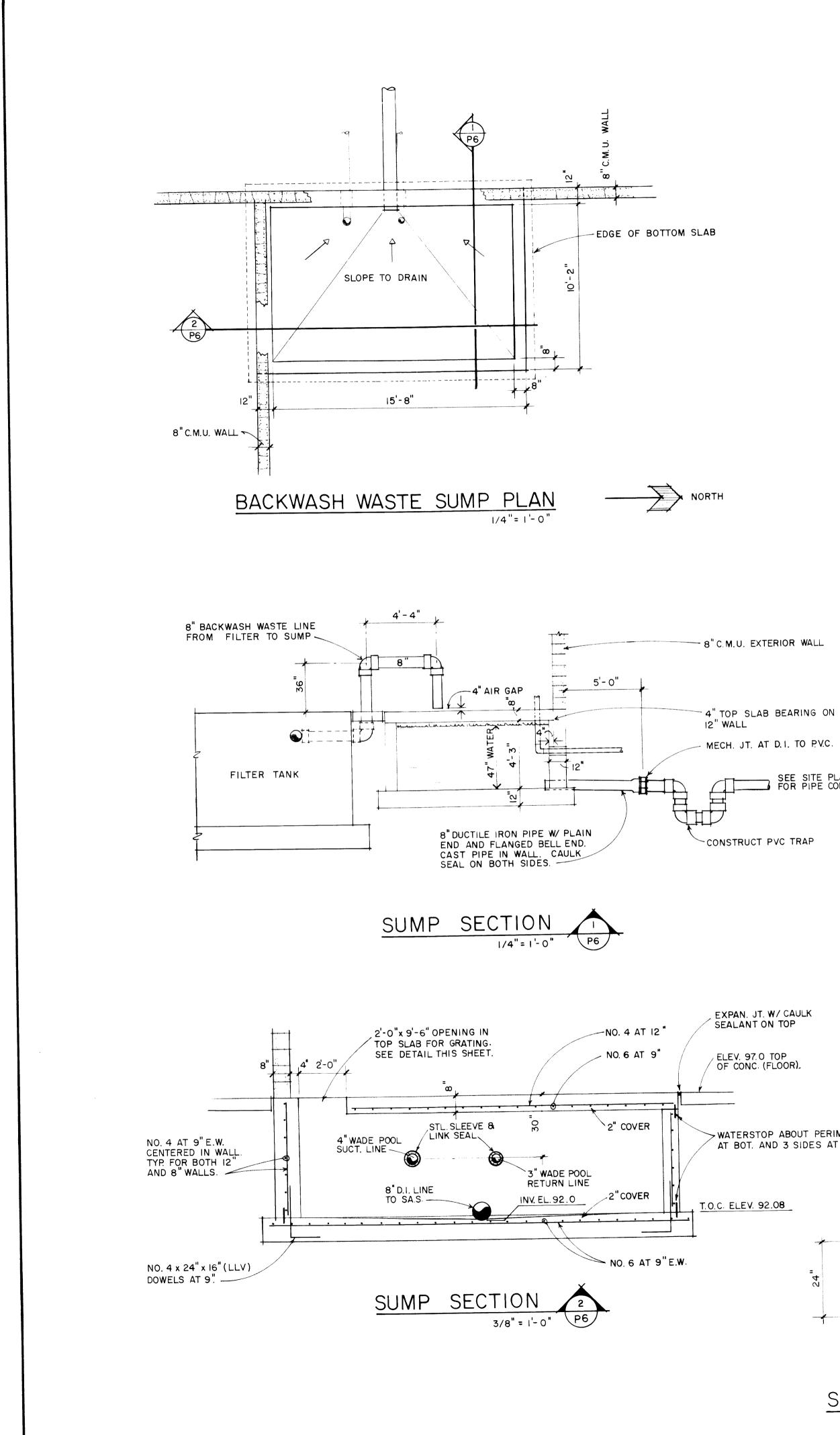


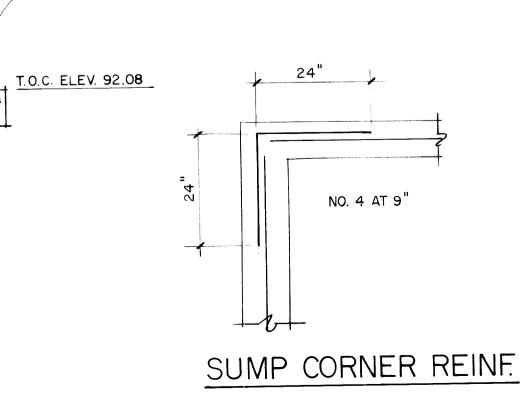










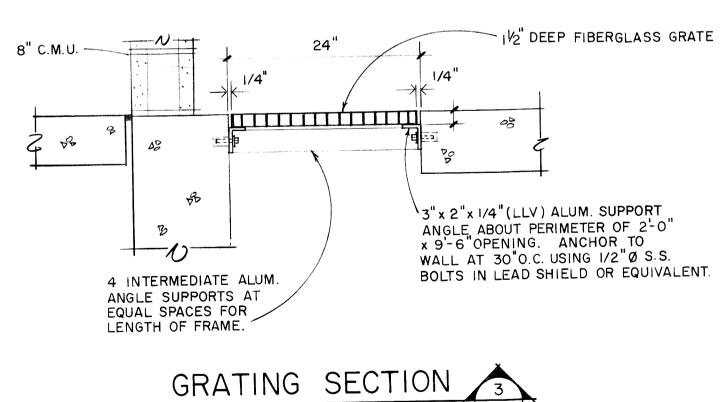


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WATERSTOP ABOUT PERIM. AT BOT. AND 3 SIDES AT TOP

ELEV. 97.0 TOP OF CONC. (FLOOR).

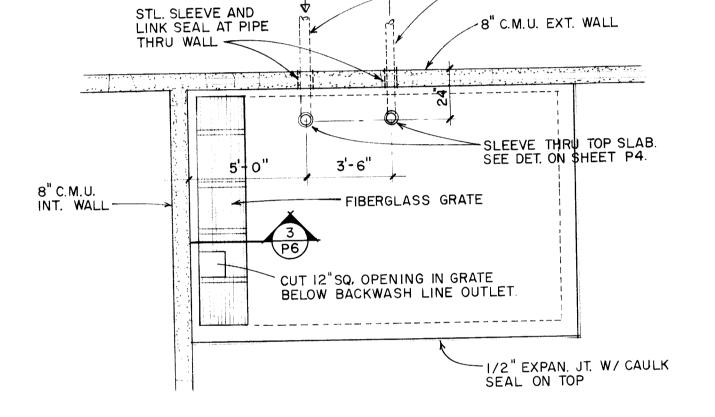
EXPAN. JT. W/ CAULK SEALANT ON TOP



1/4"= 1'- 0"

P6

|"= |'-0"



BACKWASH SUMP TOP VIEW

- 8" C.M.U. EXTERIOR WALL

SEE SITE PLAN FOR PIPE CONTINUATION

TEM	DESCRIPTION	REMARKS
$\overline{\mathbb{A}}$	10" Backwash Suction	
2	Backwash Trough	
3	10" Suction Header	Filter Cycle
4	Sand Media	
5	Gravel Media	
6	2" Underdrain Laterals	
7	Fiberglass Equalization Screen	
8	Backwash Trough Valve	
9	Backwash Influent Valve	
10	Underdrain Control Valve	
11	Pump Suction Connection	
12	Extended Valve Actuators and Handles	Typical for all Valves
13	Suction Header Valve	
14	Return to Pool Valve	
15	Filter Tank Drain	Route to Drain and Lift Manhole
16	Concrete Filter Pad	
17	Pump Backwash to Waste Valve	
18	Main Drain Valve	
20	Perimeter (Gutter) Overflow Valve	
21	Pool Water Make-up Supply Manifold	With Solenoid Valve and Isolation Valves
- 22	S.S. Filter Tank	

- 4" SUCTION & 3" RETURN FOR WADING POOL. SEE SHEET PI.

SWIMMING POOL FILTER ITEM LIST

EM	DESCRIPTION	REMARKS
1	4" Suction Line From Wading Pool	
2	3" Return Line to Wading Pool	
3	3 HP, 208v/3rh/60 hz Wading Pool Recirculation Pump w/Integral Strainer	2" Suction/2" Discharge
4	Floor Mounted High Rate Sand Wading Pool Filter	2" Bulkhead Fittings
5	Eye Level, Wall Mounted, PH Controller for Wading Pool	Controls Soda Ash Feed Pump Start/Stop
6	Eye Level, Wall Mounted, Chlorine	Controls Chlorine Booster Pump Start/Stop
7	Controller for Wading Pool 3/4 HP, 208V, 3 ph/60 hz Chlorine	11" Suction/1" Discharge
8	Booster Pump for Wading Pool Soda Ash Container, Feed Pump and	(Construct Integral By-Pass Line)
9	Mixer for Wading Pool Flow Rate Indicator for Wading Pool	On Filter Return Line to Pool
10	Domestic Use Hot Water Heater	Refer to Mechanical Plans for
11	Water Service Line into Building	Gas Piping and Venting See Site Plan and Mechanical Plan
		for In-House Continuation See Site Plan and Mechanical Plan
12	Gas Meter by Utility Company	for In-House Continuation
13	Water Heater for Wading Pool	2" Bulkhead Fittings for Copper Pipe Plumbing. See Mechanical Plan for Gas Piping and Venting.
14	Eye Level, Wall Mount, Chlorine Flow Meter for Wading Pool	
15	Eye Level, Wall Mount, Chlorine Flow Meter for Swimming Pool	Reuse Existing
16	Water Heater for Swimming Pool	4" Bulkhead Fittings for Copper Pipe Plumbing. See Mechanical
17	Recessed, Vacuum Sand Compact (VSC) Swimming Pool Filter	Plan for Gas Piping and Venting.
18	30 HP 208V /3 ph/60 hz	Furnished w/Filter
19	Swimming Pool Recirculation Pump PVC Filter Grating at Floor Level	
20	11" Swimming Pool Make-up Water	Tap for Supply Shall be Made on
21	Supply Line Below Floor Make-up Water Supply Manifold w/Elec. Solenoid and Isolation	Downstream Side of Water Meter Solenoid Open/Close Controlled by Water Level Controller
22	Ball Valves 8" Backwash Waste Line to Sump	Terminate 4" Above Grate for
23	10" Swimming Pool Return Line	Required Air Gap
24	Fitting in 10" Line to Allow	
25	Copper Pool Heater Supply Piping 10" Butterfly Valve w/ Handwheel Operator	
26	Fitting in 10" Line to Allow Copper	
27	Pool Heater Return Piping Soda Ash Feed Point	
28	Chlorine Feed Point	Reuse Existing Injector
29	Eye Level, Wall Mounted, Chemical Controller for PH and Chlorine	Reuse Existing Controller for Start/Stop of Soda Ash Feed Pump and Chlorine Booster Pump
30	10" Inlet Line from Gutter to Filter	Gravity Flow
31	10" Inlet Line from Swimming Pool Main Drains	Under Suction
32	1 ¹ / ₂ " Water Level Control Line	Beneath Deck
33	Swimming Pool Water Level Sensor	Recessed in Floor
34	Wall Mounted Relay Box for Level Controller	Controls Solenoid Open/Close on Make-up Water Supply Manifold
35	Space Reserved for Electrical Switchgear	See Electrical Drawings
36	Soda Ash Container, Feed Pump and	Reuse Existing
37	Mixer for Swimming Pool Grating Over Sump Opening	Flush w/ Floor
37	11 HP, 208/230V/3 ph/60 hz	Reuse Existing. Provide all
	Chlorinator Booster Pump	New Piping.
39 40	Flow Rate Indicator Filter Air Release and Vacuum Control Panel	On 10" Return Line 115V Elec. Required. Interlock to Recirculation Pump Motor
41	2" Filter Tank Drain Line	Starter. See Electrical Drawings Slope to Drain and Lift Manhole.
41		See Sheet P4. Wall Mounted at Eye Level with



SHEET NO.