## QUASIUS CONSTRUCTION

## EB FLO COFFEEHOUSE - ADDITION

340 SOUTH PIER DRIVE SHEBOYGAN, WI 53081

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INTERIOR ELEVATIONS, SCHEDULES, DOOR/WINDOW TYPES, AND DETAILS GREF

275 West Wisconsin Avenue, Milwaukee, WI 53203

www.graef-usa.com

414 / 259 1500

CLIENT:



PROJECT TITLE: EB FLO COFFEEHOUSE - ADDITION

2 340 SOUTH PIER DRIVE SHEBOYGAN, WI 53081

PROJECT NUMBER: 2025-0141.00

CHECKED BY: APPROVED BY:

AS NOTED SCALE:

TITLE SHEET

SHEET TITLE:

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INFORMATION PERTAINING TO EXISTING PROJECT CONDITIONS, SUCH AS LOCATIONS OF ARCHITECTURAL AND STRUCTURAL BUILDING COMPONENTS, MECHANICAL AND ELECTRICAL EQUIPMENT. PIPING. DUCTWORK. ROUGH-INS AND OTHER MISCELLANEOUS CONSTRUCTION. APPEARS ON PROJECT DRAWINGS. THIS INFORMATION IS BASED ON AVAILABLE RECORDS AS WELL AS INFORMATION COLLECTED WITH REASONABLE CARE AT THE PROJECT SITE. CONTRACTORS SHALL BE SOLELY RESPONSIBLE FOR VERIFYING DIMENSIONS AND RELATED INFORMATION AT THE PROJECT SITE PRIOR TO

PROCURING ANY MATERIALS, PRODUCTS OR EQUIPMENT TO PERFORM THEIR WORK.

STRUCTURAL

CIVIL

ARCHITECTURAL

275 West Wisconsin Avenue Suite 300 BENCHMARK X IN CONCRETE EL. = 586.48 EB FLO COFFEE HOUSE 340 S PIER DRIVE SHEBOYGAN, WI 53081 BENCHMARK X IN CONCRETE EL. = 586.48 SPIRAL STAIRCASE POST NW. COR. LOT 5 CONDUIT WITH OUTLET GAS METER -CONDUIT WITH OUTLET -- ELECTRIC METER LEGEND O = Iron Stake Found

O = 0.75" Iron Stake Set

O = Power Pole = Light Pole CONDUIT WITH OUTLET 👉 = Hydrant — E — = Electric Line — T — = Telecom Line TOPOGRAPHIC SURVEY FOR: QUASIUS CONSTRUCTION 1202A NORTH 8TH STREET SHEBOYGAN, WI 53082 PART OF THE SW 1/4 OF THE SE 1/4, SECTION 23, T15N, R23E, CITY OF SHEBOYGAN, SHEBOYGAN COUNTY, WISCONSIN NOTES:

1. PARCEL MAY BE SUBJECT TO EASEMENTS AND RIGHTS NOT SHOWN THAT A COMPLETE TITLE SEARCH MAY DISCLOSE.

2. COORDINATES ARE SHEBOYGAN COUNTY COORDINATE SYSTEM. UNDERGROUND UTILITY CAUTION STATEMENT: UTILITY STRUCTURES VISIBLE ON THE GROUND SURFACE HAVE BEEN SHOWN PER ACTUAL MEASUREMENTS. UNDERGROUND UTILITY LINES HAVE BEEN Oostburg, WI 53070 PER ACTUAL MEASUREMENTS. UNDERGROUND UTILITY LINES HAVE BEEN SHOWN PER AVAILABLE RECORDS AND MARKINGS BY DIGGERS HOTLINE AND SHOULD NOT BE INTERPRETED AS THE EXACT LOCATION NOR THE ONLY UTILITIES IN THIS AREA. THE SURVEYOR MAKES NO WARRANTY OR GUARANTEE, EXPRESSED OR IMPLIED, REGARDING THE ACCURACY, COMPLETENESS, OR RELIABILITY OF THE UNDERGROUND UTILITY DATA DEPICTED HEREON. IT IS THE RESPONSIBILITY OF THE USER TO VERIFY THE LOCATION OF UNDERGROUND UTILITIES THROUGH APPROPRIATE MEANS PRIOR TO ANY EXCAVATION OR CONSTRUCTION ACTIVITIES. 3. ELEVATIONS ARE NAVD88(GEOID12B). CEDAR CREEK SURVEYING, LLC ENGINEERS • SURVEYORS • DRAFTERS www.cedarcreeksurveying.com FILE No.: 2025111S DATE: 6/27/2025 PAGE: 1 OF 1 CHECKED BY: APPROVED BY: SJF **EXISTING CONDITIONS** In accordance with Wisconsin statute 182.0175, damage to transmission facilities, excavator shall be solely responsible to provide advance notice to the designated "ONE CALL SYSTEM" not less than three working days prior to commencement of any excavation required to perform work contained on this drawing, and further, excavator shall comply with all other requirements of this statute relative to excavator's work. DISCLAIMER: The underground utilities shown have been located from field survey information and existing drawings. GRAEF makes no guarantees that the underground utilities shown comprise all such utilities in the area, either in service or abandoned. GRAEF further does not warrant that the underground utilities shown are in the exact location indicated. GRAEF has not physically located the underground utilities.

GRaEF

Milwaukee, WI 53203-3318 414 / 259 1500

CONSULTANTS:

PROJECT TITLE:

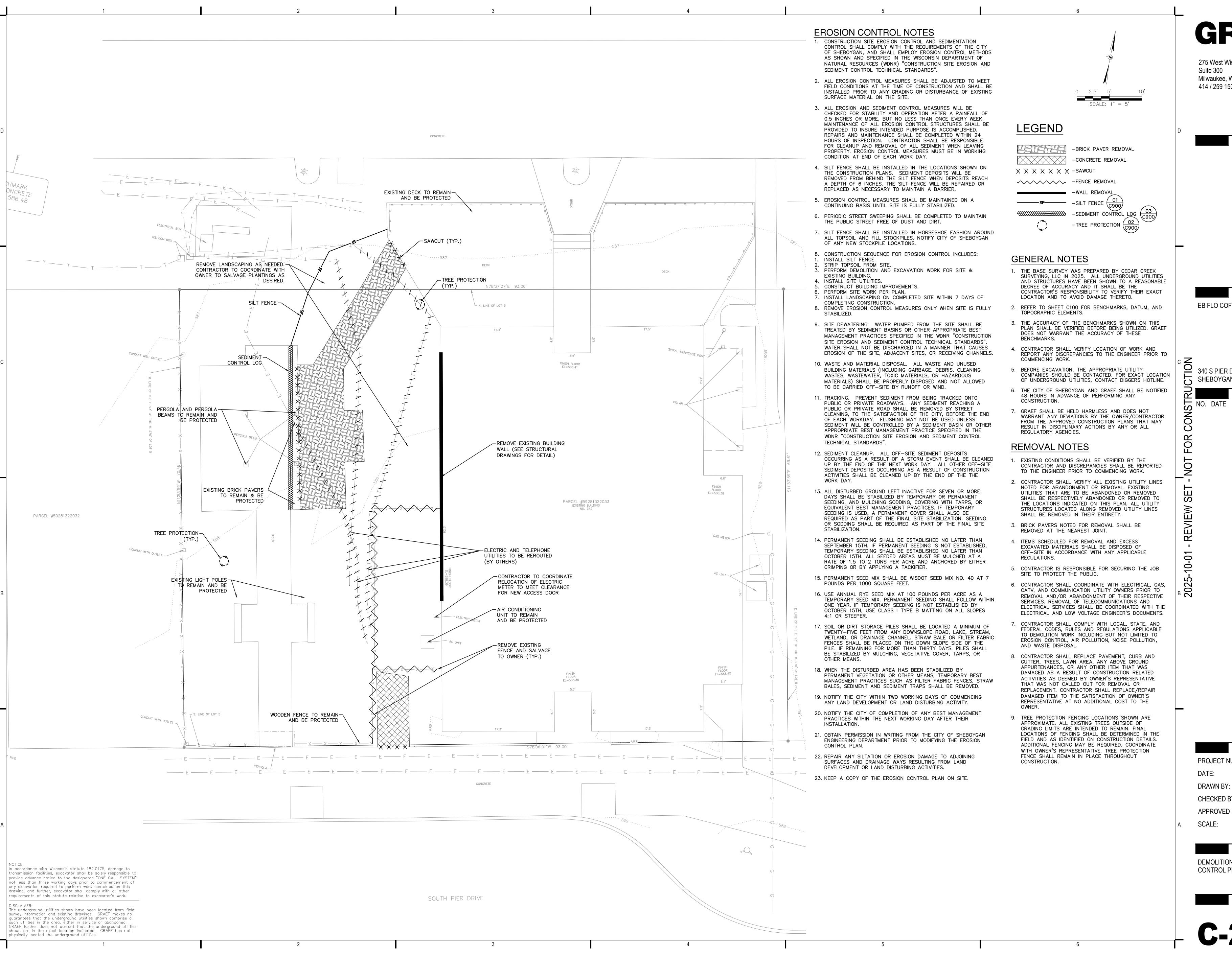
NO. DATE REVISIONS BY

PROJECT INFORMATION:

PROJECT NUMBER: 2025-0141

AS SHOWN

SHEET TITLE:



275 West Wisconsin Avenue Suite 300 Milwaukee, WI 53203-3318 414 / 259 1500

**CONSULTANTS:** 

REVISIONS BY

PROJECT TITLE:

EB FLO COFFEE HOUSE

340 S PIER DRIVE SHEBOYGAN, WI 53081

PROJECT INFORMATION:

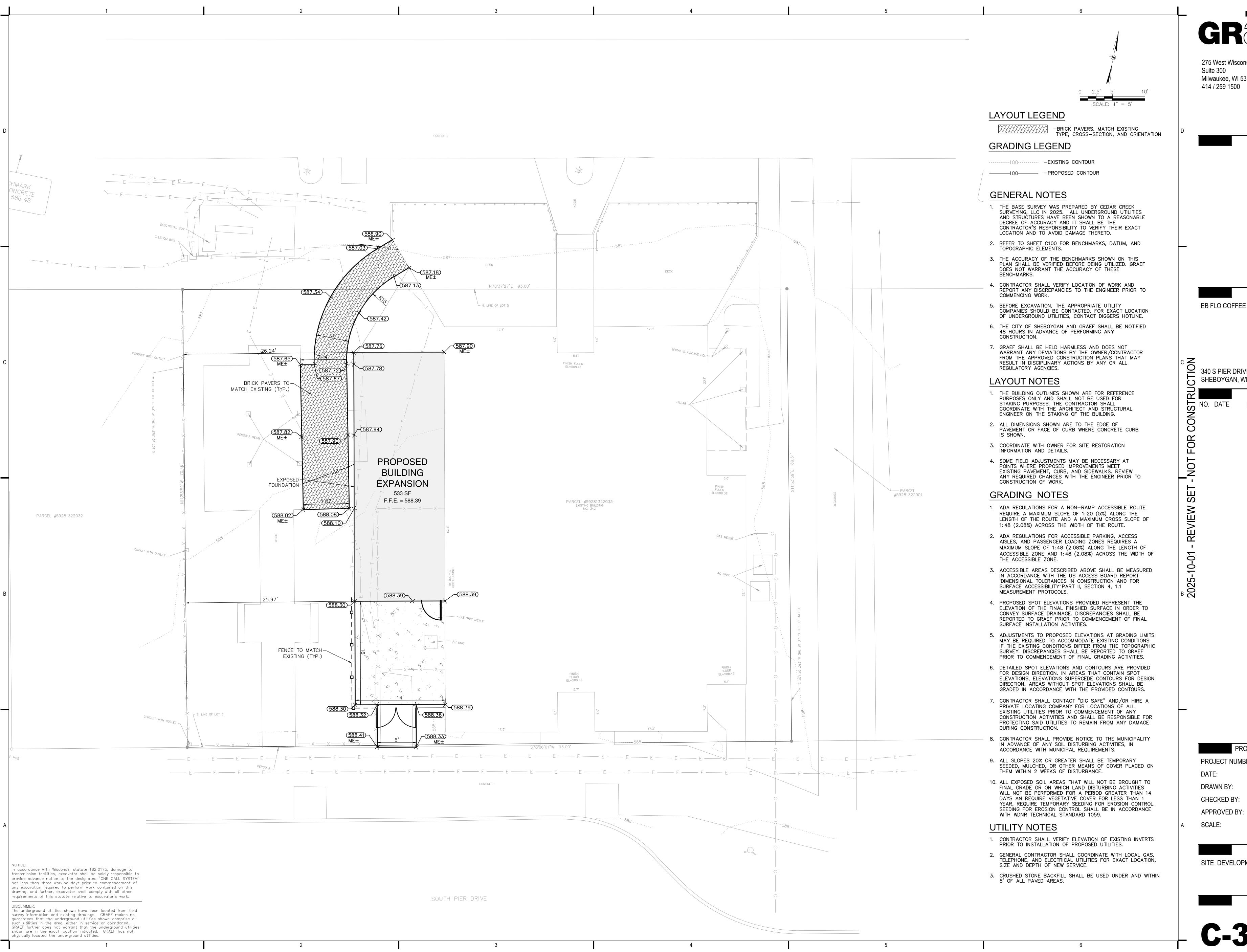
PROJECT NUMBER: 2025-0141 10/01/2025

**CHECKED BY:** 

APPROVED BY:

AS SHOWN

SHEET TITLE: **DEMOLITION AND EROSION** CONTROL PLAN



GREF

275 West Wisconsin Avenue Suite 300 Milwaukee, WI 53203-3318 414 / 259 1500

CONSULTANTS:

PROJECT TITLE:

EB FLO COFFEE HOUSE

340 S PIER DRIVE SHEBOYGAN, WI 53

SHEBOYGAN, WI 53081

NO. DATE

REVISIONS BY

PROJECT INFORMATION:

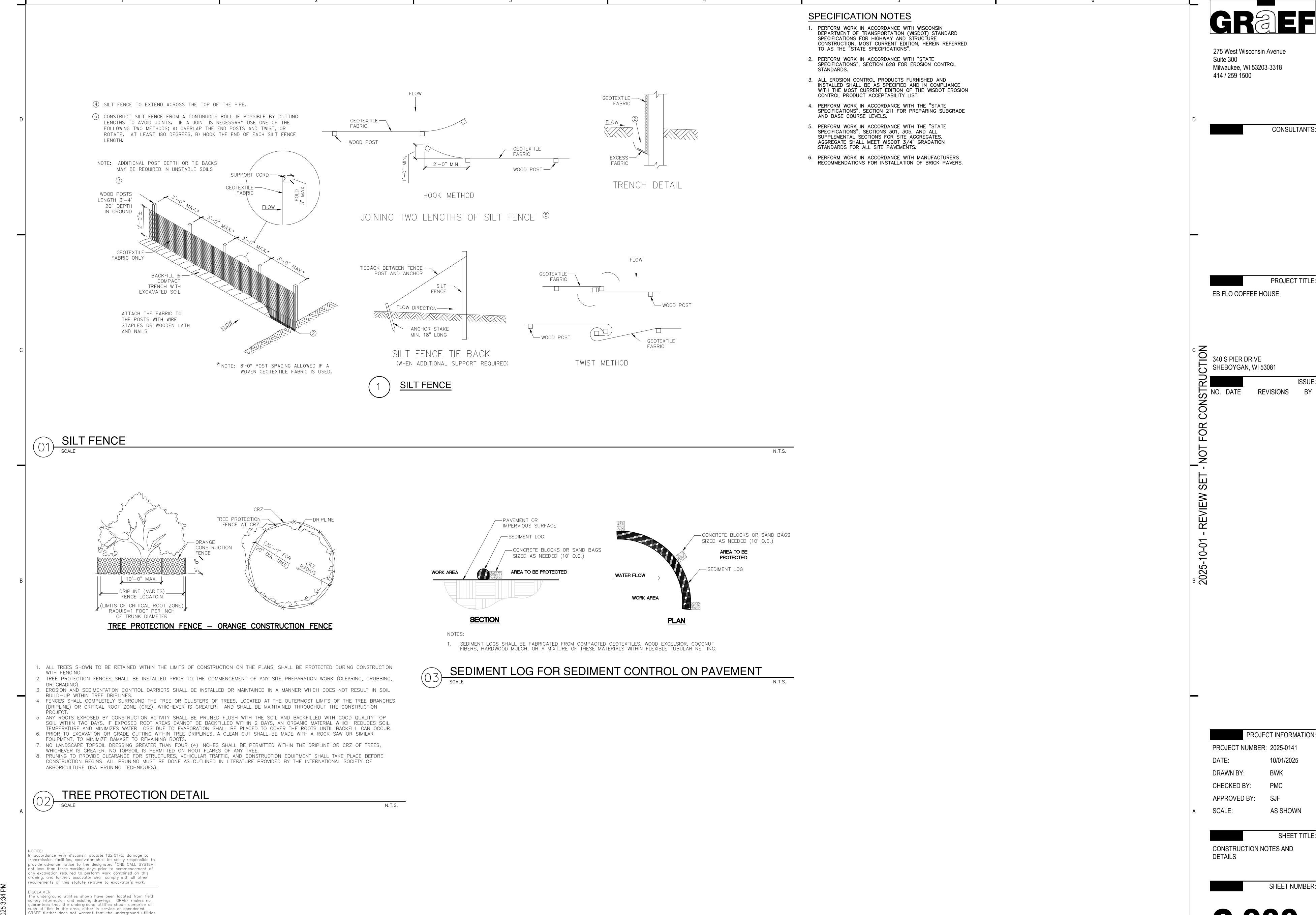
PROJECT NUMBER: 2025-0141 10/01/2025 DRAWN BY: CHECKED BY:

SCALE:

SHEET TITLE:

SITE DEVELOPMENT PLAN

AS SHOWN



CONSULTANTS:

PROJECT TITLE:

10/01/2025

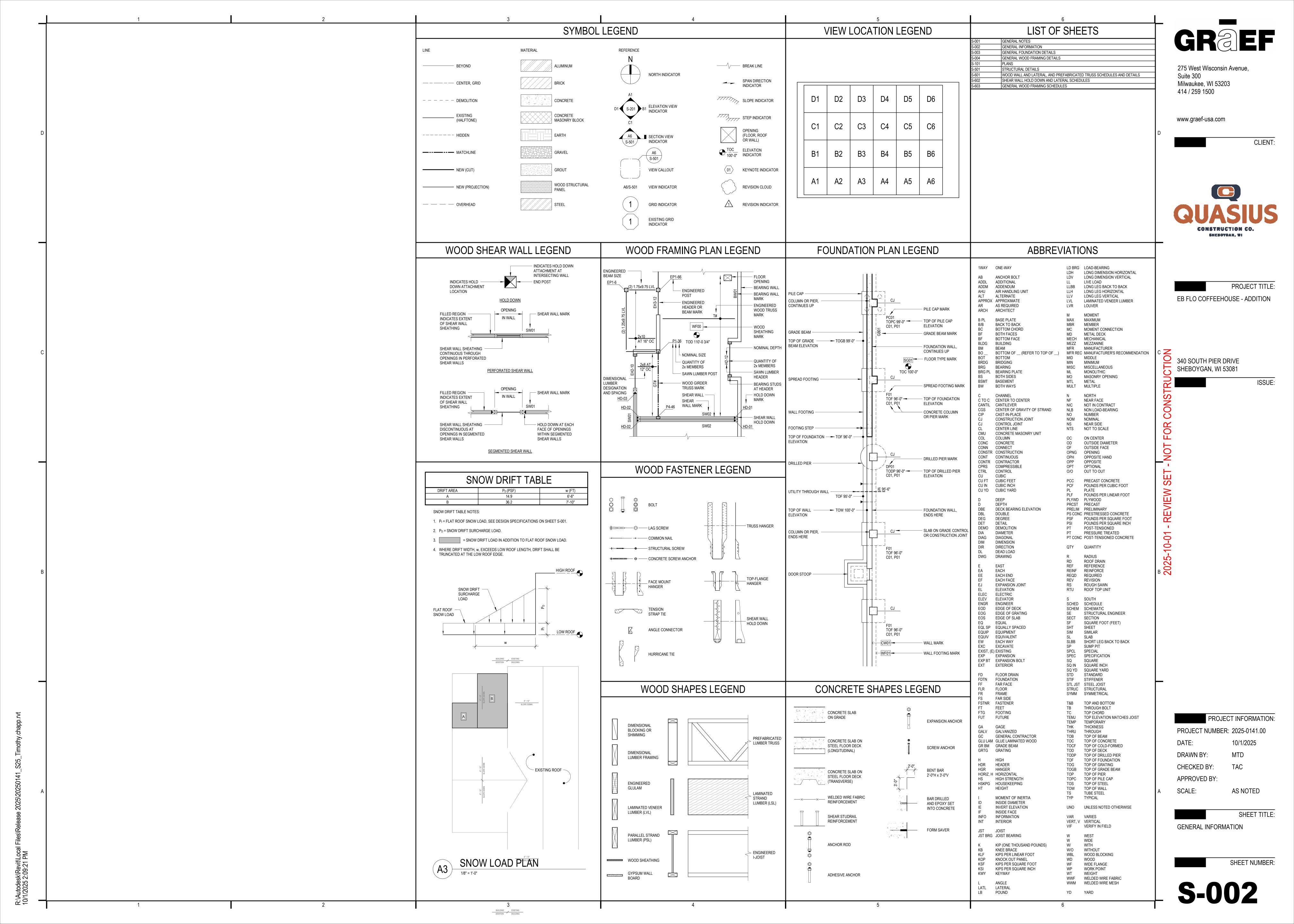
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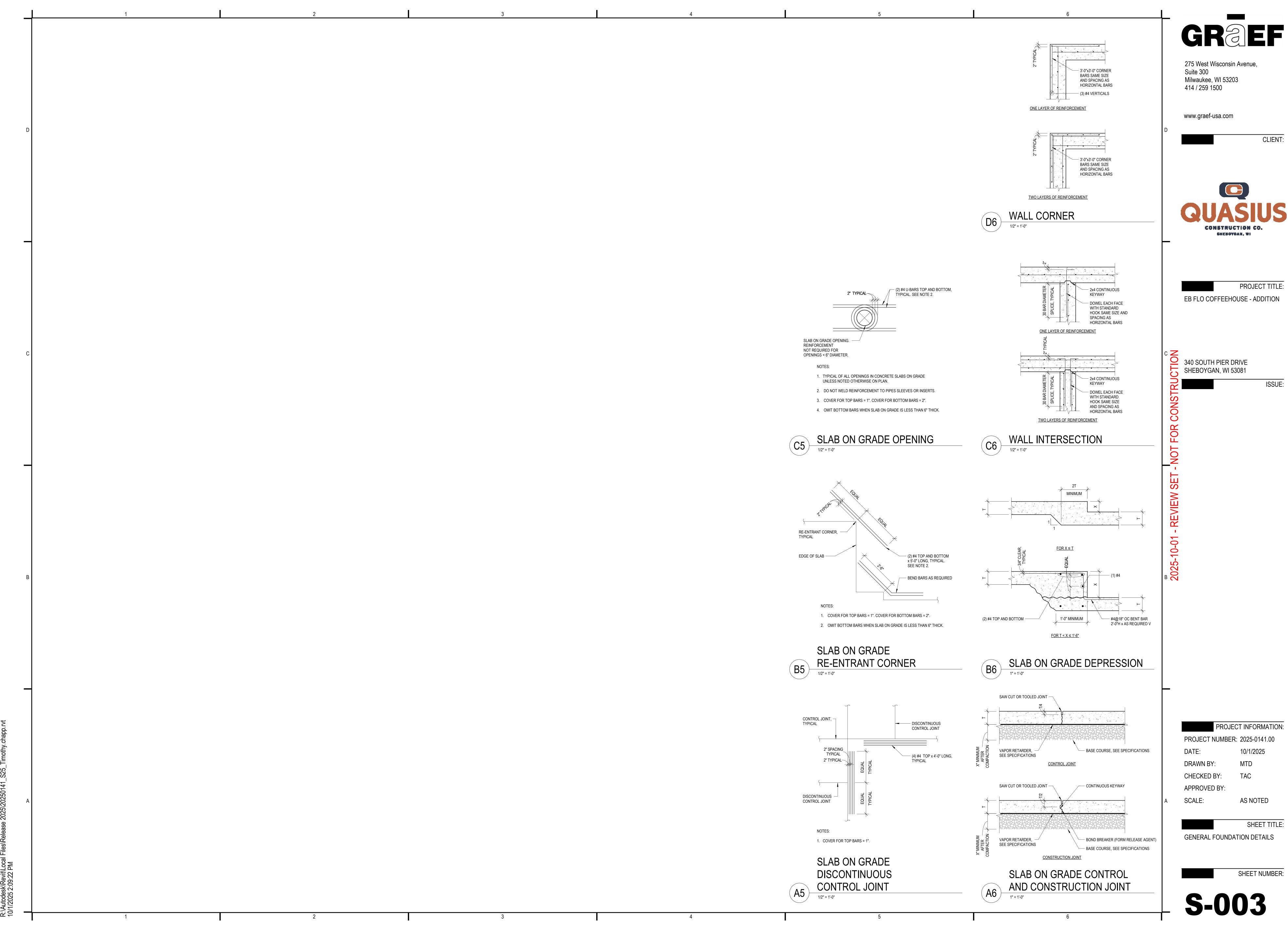
SHEET TITLE:

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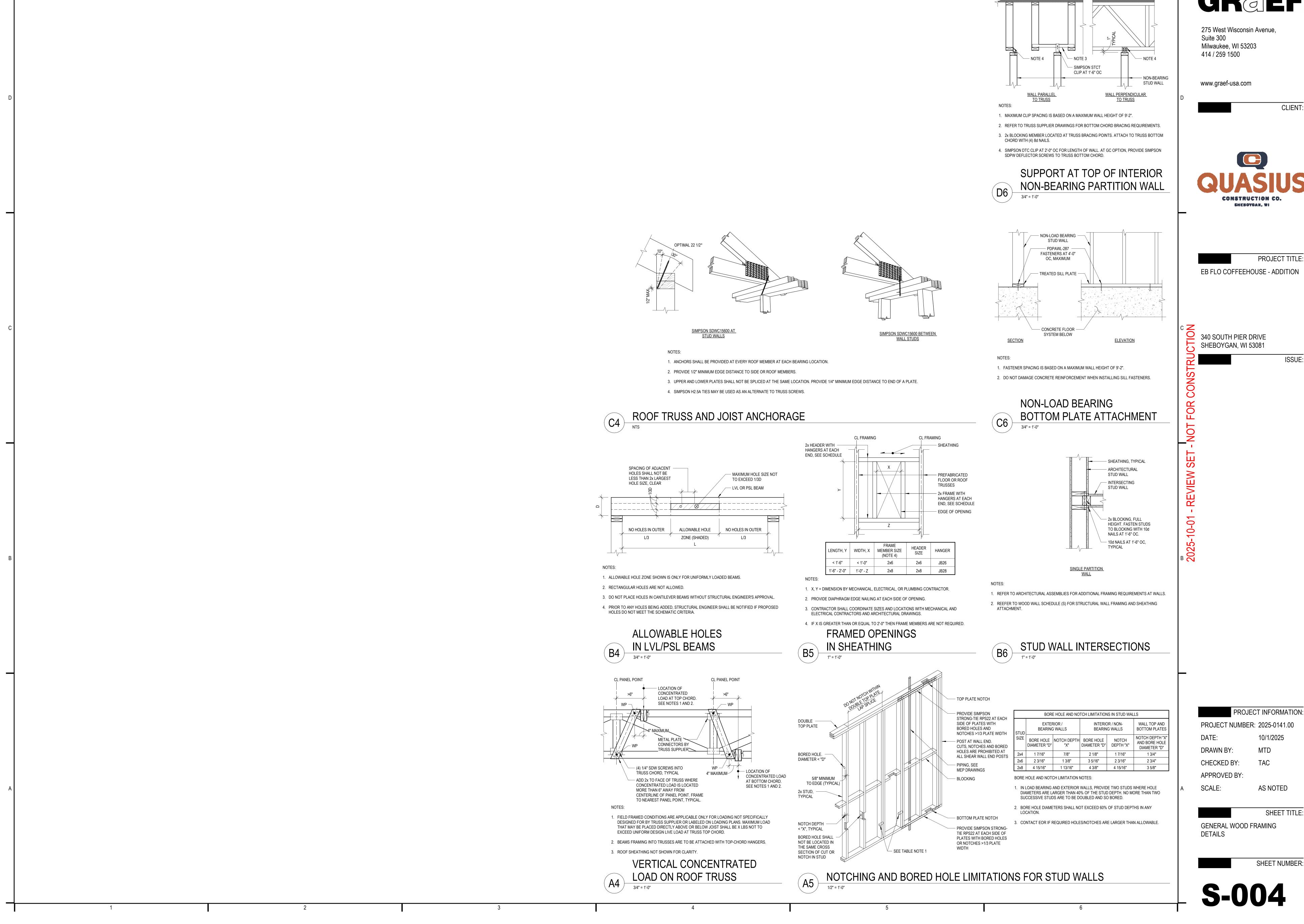
physically located the underground utilities.

1	DESIGN SPECIFICATIONS	4 CONCRETE	WOOD TRUSSES	DESIGN WIND PRESSURE TABLE, PSF	
	DESIGN IS IN ACCORDANCE WITH THE STATE OF WISCONSIN AND THE 2015 INTERNATIONAL BUILDING CODE.	FORMWORK SHALL BE DESIGNED IN ACCORDANCE WITH THE ACI "MANUAL OF CONCRETE PRACTICE", LATEST EDITION.	METAL PLATE CONNECTED TRUSSES SHALL BE DESIGNED AND MANUFACTURED IN     ACCORDANCE WITH "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD	EFFECTIVE WIND AREA, SF	GREF
	CONCRETE MATERIAL STRENGTHS:  A. MINIMUM 28 DAY CONCRETE CYLINDER STRENGTH SHALL BE:	2. REINFORCING STEEL SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH THE ACI "MANUAL OF CONCRETE PRACTICE", LATEST EDITION, UNLESS OTHERWISE NOTED.	TRUSS CONSTRUCTION", ANSI/TPI 1 2015 EDITION.  2. METAL PLATE CONNECTED WOOD TRUSSES SHALL BE ERECTED IN ACCORDANCE WITH THE STRUCTURAL BUILDING COMPONENTS ASSOCIATION (SBCA) AND THE TRUSS PLATE INSTITUTE	1 -28.5 -28.5 -27.0 -25.1 -23.6 -23.6 ROOF INTERIOR ZONE 2 -33.3 -33.3 -31.8 -29.9 -28.5 -28.5 -28.5 END ZONE REGION OF THE ROOF	275 West Wisconsin Avenue,
	FOOTINGS 3000 PSI FOUNDATION WALLS 4000 PSI SLABS ON GRADE 4000 PSI	3. LAP ALL WALL BARS 30 DIAMETERS UNLESS OTHERWISE DETAILED. LAP WELDED WIRE MESH 6 INCHES.  4. PROVIDE WALL DOWELS OF THE SAME SIZE AND NUMBER AS THE RESPECTIVE AND WALL	<ul><li>(TPI).</li><li>3. SHOP DRAWINGS AND OTHER ITEMS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW</li></ul>	3 -33.3 -33.3 -31.8 -29.9 -28.5 -28.5 CORNER ZONE REGION OF THE ROOF	Suite 300 Milwaukee, WI 53203
	B. REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60.	REINFORCEMENT UNLESS OTHERWISE DETAILED.  5. CONCRETE PROTECTION FOR REINFORCING BARS SHALL BE IN ACCORDANCE WITH THE	PRIOR TO FABRICATION. ALL SHOP DRAWINGS SHALL BE REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR BEFORE SUBMITTAL TO THE ENGINEER. THE ENGINEER'S REVIEW WILL BE BASED ON THE CONTRACT DOCUMENTS. THE ENGINEER'S REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW, CHECK, AND COORDINATE THE	ROOF OVERHANG (ZONES 1 AND 2)       -48.3       -48.3       -46.8       -44.9       -43.4       -43.4       -43.4         ROOF OVERHANG (ZONE 3)       -48.3       -48.3       -46.8       -44.9       -43.4       -43.4       -43.4	414 / 259 1500
	C. EPOXY COATED REINFORCING STEEL SHALL CONFORM TO ASTM A775 GRADE 60.  D. SYNTHETIC FIBER REINFORCEMENT SHALL CONFORM TO ASTM C1116.	"BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", ACI 318-14.  6. SLABS ON GRADE SHALL BE CAST ALLOWING A SUFFICIENT NUMBER OF JOINTS TO ADEQUATELY CONTROL SHRINKAGE CRACKING, SAWCUTTING SHALL BE DONE AS SOON AS	SHOP DRAWINGS PRIOR TO SUBMISSION. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, DIMENSIONS, ETC.	4 (+) 28.5 28.5 27.2 25.5 25.5 22.9 21.2 4 (-) -30.9 -30.9 -29.6 -27.9 -27.9 -25.3 -23.6 WALL INTERIOR ZONE	www.graef-usa.com
D	3. POST-INSTALLED ANCHORS MATERIAL STRENGTHS:  A. EXPANSION ANCHORS SHALL BE HILTI KWIK BOLT 3.	SAWCUT WILL NOT RAVEL CONCRETE OR WITHIN 24 HOURS MAXIMUM OF INITIAL POURING OPERATION. MAXIMUM SIZE OF PANELS SHALL BE 12 FEET BY 12 FEET FOR 4-INCH SLAB ON GRADE. GENERALLY, JOINTS SHALL OCCUR ON COLUMN CENTERLINES.	<ol> <li>TRUSS MANUFACTURER SHALL DESIGN AND PROVIDE ALL TRUSS-TO-TRUSS CONNECTIONS.</li> <li>TRUSS MANUFACTURER SHALL REFER TO MECHANICAL DRAWINGS FOR DUCT LAYOUT. THE</li> </ol>	5 (+) 28.5 28.5 27.2 25.5 25.5 22.9 21.2 END ZONE REGION OF	D
	<ul><li>B. ADHESIVE ANCHORS SHALL BE HILTI HIT-HY 200.</li><li>C. SLEEVE ANCHORS SHALL BE HILTI HLC.</li></ul>	<ol> <li>EXTERIOR SLABS ON GRADE SHALL BE 5 INCHES THICK AND REINFORCED WITH 4 LB PER CUBIC YARD POLYPROPYLENE MACRO FIBERS.</li> </ol>	CONTRACTOR SHALL CONFIGURE TRUSS WEBS TO AVOID DUCT WORK AND NOTE ALL DUCT SIZES AND AREAS OF POTENTIAL CONFLICT ON SHOP AND ERECTION DRAWING SUBMITTALS.  6. MAXIMUM VERTICAL TRUSS DEFLECTION SHALL BE LIMITED TO THE SPAN/240 (INCLUDING	5 (-)   -38.1   -35.6   -32.2   -29.6   -27.0   -23.6   THE WALL DESIGN WIND PRESSURE TABLE NOTES:	CLIENT:
	D. SCREW ANCHORS SHALL BE HILTI KWIK HUS.  4. WOOD MATERIAL STRENGTHS:	<ol> <li>ALLOW AT LEAST 24 HOURS BEFORE POURING ADJACENT WALL SECTIONS BETWEEN CONSTRUCTION JOINTS. MAXIMUM LENGTH OF POUR TO BE 40 FEET, UNLESS CRACK INDUCERS ARE USED AS DETAILED ON THE DRAWINGS.</li> </ol>	CALCULATED CREEP) FOR DEAD LOAD PLUS SNOW LOAD, UNLESS OTHERWISE NOTED.  7. MAXIMUM HORIZONTAL TRUSS DEFLECTION SHALL BE LIMITED TO 1/4" EACH SIDE.	<ol> <li>NEGATIVE WIND PRESSURES ACT AWAY FROM COMPONENT SURFACE. POSITIVE WIND PRESSURES ACT TOWARD COMPONENT SURFACE.</li> <li>WIND PRESSURES SHOWN IN THIS TABLE ARE SERVICE PRESSURES (0.6W).</li> </ol>	
	A. STRUCTURAL WOOD FRAMING SHALL CONFORM TO NFPA NATIONAL DESIGN SPECIFICATIONS (OR MEET ALL THE MINIMUM PUBLISHED VALUES) AS FOLLOWS:	<ol> <li>CONTRACTOR SHALL NOTIFY THE ENGINEER AT LEAST 24 HOURS PRIOR TO PLACING CONCRETE.</li> </ol>	<ol> <li>ALL ROOF TRUSSES SHALL BE SECURED TO THE TOP PLATES WITH SIMPSON UPLIFT CONNECTORS (OR APPROVED EQUAL) PROVIDED BY THE TRUSS SUPPLIER. REFER TO COMPONENTS AND CLADDING WIND PRESSURES FOR UPLIFT REQUIREMENTS.</li> </ol>	3. FOR NET UPLIFT TO ROOF TRUSSES, SUBTRACT A ROOF DEAD LOAD OF 10 PSF (NOT INCLUDING SELF WEIGHTS) FROM THE WIND PRESSURES SHOWN.	
	LOCATION: SPECIES: GRADE:  TOP AND BOTTOM PLATES: SPF No. 2 OR BETTER  STUDS, CAPS AND SILLS: SPF No. 2 OR BETTER	<ul><li>10. DO NOT PLACE OR CUT HOLES IN CONCRETE SLABS WITHOUT PRIOR APPROVAL OF THE ENGINEER.</li><li>11. EXTERIOR EXPOSED CONCRETE SHALL BE AIR-ENTRAINED. AIR CONTENT SHALL BE 6 PERCENT</li></ul>	<ol> <li>DOUBLE 2x4 CONTINUOUS BLOCKING SHALL BE PROVIDED AT THE PEAK OF ROOF RIDGES WHERE PRESENT.</li> </ol>	GABLE ROOF BUILDING (7°< $\theta \le 45^{\circ}$ ) (h $\le 60$ FT)	
	BEAMS AND HEADERS: SPF No. 2 OR BETTER  5. LAMINATED VENEER LUMBER (LVL) SHALL MEET THE FOLLOWING MINIMUM DESIGN VALUES:	(+/-1 1/2 PERCENT).  12. PIPES AND CONDUITS EMBEDDED IN OR PASSING THROUGH STRUCTURAL MEMBERS MUST BE	10. WOOD ROOF TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT THE FOLLOWING MINIMUM LOADS UNLESS OTHERWISE NOTED ON PLAN. REFER TO PREFABRICATED TRUSS SCHEDULE(S) FOR ADDITIONAL ROOF LOADS:	a = 3'-0"	QUASIUS
	$E = 2.0 \times 10^{6} \text{ PSI}$ $F_b = 2,600 \text{ PSI}$ $F_{cz} = 2,510 \text{ PSI}$	APPROVED BY THE STRUCTURAL ENGINEER. PIPE AND CONDUITS EMBEDDED IN CONCRETE SHALL NOT BE LARGER THAN 2 INCHES IN OUTSIDE DIAMETER AT THEIR WIDEST POINT OR FITTING OR 1/3 OF THE THICKNESS OF THE SLAB, BEAM OR WALL.	TOP CHORD DEAD LOAD 10 PSF (ON THE SURFACE AREA) BOTTOM CHORD DEAD LOAD 10 PSF (ON THE SURFACE AREA)		CONSTRUCTION CO. SHEBOYGAN, WI
	$F_{\nu} = 285  PSI$ 6. PARALLEL STRAND LUMBER (PSL) SHALL MEET THE FOLLOWING MINIMUM DESIGN VALUES:	<ul> <li>13. ELECTRICAL CONDUIT OR PIPES EMBEDDED IN OR PASSING THROUGH SLABS, BEAMS OR WALLS         SHALL BE LOCATED AND PLACED SO THAT:</li> <li>A. THEY ARE NOT CLOSER THAN THREE DIAMETERS ON CENTER.</li> </ul>	ADDITIONAL 5 PSF AT ROOF TRUSSES SUPPORTING INFILL FRAMING AT EXTENSION OF ADJOINING ROOFS  11. ROOF TRUSS TOP CHORDS SHALL BE DESIGNED AS INDICATED BY LOADING ON THIS SHEET		
	COLUMNS AND POSTS: $E = 1.8 \times 10^{6} \text{ PSI}$ $F_b = 2,400 \text{ PSI}$	<ul><li>B. THE CONCRETE COVER IS NOT LESS THAN 2 INCHES.</li><li>C. THEY RUN BETWEEN REINFORCING AND DO NOT DISPLACE IT IN ANY MANNER.</li></ul>	OR SNOW LOAD, WHICHEVER IS GREATER. SNOW LOADING SHALL TAKE DRIFTING SNOW INTO CONSIDERATION WHERE INDICATED ON SNOW DRIFT PLANS.		
	$F_c = 2,500  \text{PSI}$ $F_{cz} = 545  \text{PSI}$ $F_v = 190  \text{PSI}$	<ul><li>14. ALUMINUM CONDUITS SHALL NOT BE PLACED IN CONCRETE.</li><li>15. CHAMFER ALL EXPOSED CONCRETE CORNERS. SEE ARCHITECTURAL/STRUCTURAL DRAWINGS FOR REQUIREMENTS.</li></ul>	12. TRUSS DESIGNER TO DESIGN FOR UPLIFT AT TRUSSES SUPPORTING INFILL FRAMING. REFER TO "COMPONENTS AND CLADDING" WIND LOAD TABLE FOR DESIGN LOADS. TRUSS DESIGNER SHALL DESIGN AND SUPPLY UPLIFT CONNECTIONS (HURRICANE TIES) AS REQUIRED.		PROJECT TITLE:
	7. ASSUMED BEARING CAPACITY FOR SPREAD FOOTINGS IS 1500 PSF. DESIGN LOADS:	16. CONCRETE SHALL BE TESTED BY THE OWNER'S TESTING LAB. REFER TO SPECIFICATIONS FOR REQUIREMENTS.	13. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL TEMPORARY AND PERMANENT BRACING AS REQUIRED FOR SAFE ERECTION AND PERFORMANCE OF THE TRUSSES, THE GUIDELINES SET FORTH BY THE TRUSS PLACE INSTITUTE PUBLICATION "HIB-91 COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND REACHING METAL PLATE CONNECTED.		EB FLO COFFEEHOUSE - ADDITION
	FLOOR LIVE LOAD PUBLIC ROOMS AND CORRIDORS SERVING THEM 100 PSF	<ul><li>17. PROPER CURING PROCEDURES SHALL BE USED FOR SLAB ON GRADE TO PREVENT CURLING.</li><li>18. CALCIUM CHLORIDE SHALL NOT BE USED IN CONCRETE MIXES.</li></ul>	RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" SHALL BE A MINIMUM REQUIREMENT.  14. TRUSS MEMBERS AND COMPONENTS SHALL NOT BE CUT, NOTCHED, DRILLED OR OTHERWISE	h B	
	FOR FLOOR LIVE LOAD < 80 PSF, A 15 PSF PROVISION FOR PARTITIONS HAS BEEN INCLUDED.  ROOF LIVE LOAD	19. PROVIDE WATERSTOPS AT ALL CONSTRUCTION JOINTS BELOW THE WATER TABLE AND AS SHOWN ON DRAWINGS. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION.	ALTERED IN ANY WAY WITHOUT THE WRITTEN APPROVAL OF THE TRUSS MANUFACTURER.  15. STRUCTURAL DRAWINGS DO NOT SHOW ALL BRACING AND BRIDGING. PROVIDING BRACING AND BRIDGING BOTH TEMPORARY AND PERMANENT FOR ALL WOOD TRUSSES AS		
C	MINIMUM ROOF LIVE LOAD 20 PSF  LIVE LOAD REDUCTION  LIVE LOAD REDUCTION PER IBC 2015 SECTION 1607.10 IS INCLUDED.	20. EPOXY FOR DRILLED AND EPOXIED REBAR SHALL CONFORM TO HILTI HIT-HY 200.  WOOD FRAMING	PERMANENT TRUSS BRACING		C 340 SOUTH PIER DRIVE
	ROOF SNOW LOAD  RISK CATEGORY  IMPORTANCE FACTOR  RISK OF SNOW LOAD  ROOF SNOW LOAD  RISK CATEGORY  II  ROOF SNOW LOAD  ROOF S	ERECTION OF ALL WOOD FRAMING SHALL CONFORM TO THE "NATIONAL DESIGN	PERMANENT BRACING SHALL COMPLY WITH TRUSS PLATE INSTITUTE "NATIONAL DESIGN		SHEBOYGAN, WI 53081 ISSUE
	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	SPECIFICATION FOR WOOD CONSTRUCTION", ANSI/AWC NDS-2015.  2. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE DRAWINGS OF ALL OTHER DISCIPLINES AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE	STANDARD FOR BRACING METAL PLATE CONNECTED WOOD TRUSSES", DSB 2015.  2. ALL PERMANENT AND TEMPORARY BRACING IS THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL COMPLY SBCA/BCSI 2022.		ATS.
	REFER TO TABLE ON SHEET S-002 FOR SNOW DRIFT SURCHARGE LOADS ( $P_D$ ) AND WIDTHS OF SNOW DRIFTS (w)	REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, CHASES, HANGERS, INSERTS, ANCHORS, HOLES, AND OTHER ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.  3. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION, WHERE	3. PROVIDE CONTINUOUS LATERAL BRACING AND DIAGONAL BRACING FOR TRUSS BOTTOM CHORDS. TOP CHORD BRACING IS PROVIDED BY PERMANENT WOOD SHEATHING.	= INTERIOR ZONE ROOF = ZONE 1 WALLS = ZONE 4  = END ZONE ROOF = ZONE 2 ROOF = ZONE 3 WALLS = ZONE 5	
	ROOF RAIN LOAD BUILDING HAS BEEN DESIGNED FOR RAIN LOADS PER IBC 2015 SECTION 1611.	CONDITIONS ARE NOT SPECIFICALLY SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO APPROVAL BY THE ENGINEER.	<ol> <li>WEB MEMBER BRACING (STRONGBACKING) SHALL BE PLACED AT A MINIMUM OF 10 FEET ON CENTER ALIGNED WITH BOTTOM CHORD BRACING. PROVIDE CONTINUOUS LATERAL BRACING AND DIAGONAL BRACING.</li> </ol>		ON CONTRACT OF THE CONTRACT OF
	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	4. ALL STRUCTURAL SYSTEMS RELATING TO WOOD FRAMING WHICH ARE TO BE COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE, AND ERECTION IN ACCORDANCE WITH THE SUPPLIER'S INSTRUCTIONS AND REQUIREMENTS.	5. END BRACING SHALL BE PLACED CONTINUOUSLY ON ENDS OF TRUSS MEMBERS. WHERE CONTINUOUS LATERAL BRACING AND DIAGONAL BRACING ARE REQUIRED FOR WEB MEMBERS, END BRACING IS NOT REQUIRED.		T TC
	EXPOSURE  INTERNAL PRESSURE COEFFICIENT  COMPONENTS AND CLADDING  REFER TO TABLE THIS SHEET	5. LOADING APPLIED TO THE STRUCTURE DURING THE PROCESS OF CONSTRUCTION SHALL NOT EXCEED THE SAFE LOAD-CARRYING CAPACITY OF THE STRUCTURAL MEMBERS. THE	6. END BRACING AT ROOF AND FLOOR DIAPHRAGMS SHALL BE CONTINUOUS. TOP CHORD TEMPORARY LATERAL RESTRAINT (TCTLR) WITH DIAGONAL BRACING, BLOCKING PANELS,		
	SEISMIC LOAD  RISK CATEGORY  IMPORTANCE FACTOR  I = 1.0	LIVE LOADS USED IN THE DESIGN OF THIS STRUCTURE ARE INDICATED IN THE "DESIGN SPECIFICATIONS". DO NOT APPLY ANY CONSTRUCTION LOADS UNTIL STRUCTURAL FRAMING IS PROPERLY CONNECTED TOGETHER AND UNTIL ALL TEMPORARY BRACING IS IN PLACE.	RIBBON BOARDS WITH DIAGONAL BRACING, AND RIM BOARDS ARE ALL ACCEPTABLE END BRACES WHERE CONTINUOUS LATERAL BRACING AT TOP AND BOTTOM CHORDS ARE NOT REQUIRED. PROVIDE BLOCKING PANELS AT ALL EXTERIOR SHEAR WALLS UNLESS OTHERWISE DETAILED WITHIN THE DESIGN DOCUMENTS.		SET
	MAPPED SPECTRAL RESPONSE ACCELERATION $S_S = 0.066 \text{ g}$ PARAMETERS $S_1 = 0.039 \text{ g}$ DESIGN SPECTRAL RESPONSE ACCELERATION $S_{DS} = 0.070 \text{ g}$	ROOF DECK SHALL BE 5/8-INCH APA RATED SHEATHING, EXPOSURE 1. SEE SHEATHING LAYOUT AND FASTENING SCHEDULE.  7. ROOF SHEATHING TO BE LAID IN STAGGERED PATTERN WITH LENGTH/2 OVERLAP (MINIMUM)	GENERAL REQUIREMENTS		
	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	AT EACH EDGE. PROVIDE SIMPSON PSCL (OR APPROVED EQUAL) PANEL SHEATHING CLIPS AT ALL UNSUPPORTED PANEL EDGES. PROVIDE 2 CLIPS FOR ALL SHEATHING SPANS GREATER THAN OR EQUAL TO 28" BUT LESS THAN 36".	CONTRACTOR IS RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION AND JOB SITE SAFETY.		ZEV
	SEISMIC DESIGN CATEGORY A SEISMIC FORCE-RESISTING SYSTEM LIGHT FRAME WOOD WALLS WITH STRUCTURAL WOOD SHEAR PANELS	<ul><li>8. ROOF SHEATHING SHALL NOT SPAN 24" OR GREATER IN ANY CASE.</li><li>9. WALL AND SOFFIT SHEATHING SHALL BE 1/2-INCH APA RATED SHEATHING, EXPOSURE 2.</li></ul>	2. DIMENSIONS OF EXISTING CONSTRUCTION OR CONSTRUCTION IN PROGRESS SHALL BE VERIFIED AND COORDINATED PRIOR TO FABRICATION OF STRUCTURAL COMPONENTS.		<u></u>
	ANALYSIS PROCEDURE INDEX FORCE ANALYSISR DESIGN BASE SHEAR 0.010W KIPS	10. NAILING OF WALL SHEATHING AND SOFFIT SHEATHING SHALL BE:	3. THE INTENT OF THESE DESIGN SPECIFICATIONS AND GENERAL NOTES IS TO INDICATE INFORMATION THAT IS ROUTINELY UTILIZED. ADDITIONAL PROJECT REQUIREMENTS ARE LOCATED WITHIN THE PROJECT SPECIFICATIONS, ALSO KNOWN AS THE PROJECT MANUAL		10-0-
D	GENERAL NOTES	8d AT 6 INCHES ON CENTER AT PANEL EDGES  8d AT 12 INCHES ON CENTER AT INTERMEDIATE FRAMING MEMBERS	CONFLICTING INFORMATION BETWEEN THESE DRAWINGS AND THE PROJECT SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER.		025
	EARTHWORK	11. INSTALL ALL SHEATHING WITH THE LONG DIMENSIONS OF THE PANEL ACROSS SUPPORTS AND WITH PANEL CONTINUOUS OVER TWO OR MORE SPANS. STAGGER PANEL END JOINTS. ALLOW 1/8-INCH SPACING AT PANEL ENDS AND EDGES UNLESS OTHERWISE RECOMMENDED BY THE SHEATHING MANUEL COLUMN.	4. WHILE THE DESIGN DOCUMENTS MAY REFERENCE OSHA, THEY ARE NOT INTENDED TO SPECIFICALLY IDENTIFY ALL APPLICABLE OSHA REQUIREMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO IDENTIFY AND COMPLY WITH ALL APPLICABLE OSHA REQUIREMENTS.		
	<ol> <li>FOOTINGS SHALL BE CAST ON UNDISTURBED SUBSOIL. IF DESIGN CAPACITY IS NOT ENCOUNTERED AT THE ELEVATIONS SHOWN, FOOTINGS MUST BE LOWERED. CONSULT ENGINEER BEFORE PROCEEDING.</li> </ol>	BY THE SHEATHING MANUFACTURER.  12. ALL SHEATHING SHOWN ON THE STRUCTURAL DRAWINGS SHALL MEET THE REQUIREMENTS OF THE U.S. PRODUCT STANDARD PS-1 FOR STRUCTURAL 1 GRADE MATERIAL.	<ol> <li>RESISTANCE TO LATERAL LOADS ON STRUCTURE IS PROVIDED BY WOOD SHEAR WALLS AND ROOF DIAPHRAGMS. CONTRACTOR SHALL PROVIDE SUFFICIENT TEMPORARY BRACING UNTIL ALL LATERAL SUPPORT SYSTEMS ARE IN PLACE AND FUNCTIONAL.</li> </ol>		
	<ol> <li>NO HOLES, TRENCHES OR DISTURBANCES OF THE SOIL SHALL BE ALLOWED WITHIN THE VOLUME DESCRIBED BY 45 DEGREE LINES SLOPING FROM THE BOTTOM EDGE OF THE FOOTING. IF SUCH ARE REQUIRED, FOOTINGS MUST BE LOWERED.</li> </ol>	13. ALL NAILING SHALL BE CAREFULLY DRIVEN AND NOT OVERDRIVEN. THE USE OF STAPLES IS PROHIBITED.	ALL STRUCTURAL FRAMING AND CONNECTIONS HAVE BEEN DESIGNED FOR THE FINAL COMPLETED CONDITION AND HAVE NOT BEEN INVESTIGATED FOR POTENTIAL LOADINGS ENCOUNTERED DURING ERECTION AND CONSTRUCTION. ANY INVESTIGATION OF THE		
	3. BACKFILL EVENLY ON EACH SIDE OF FOUNDATION WALLS AND RETAINING WALLS.  4. TOPSOIL AND FILL BELOW SLABS ON GROUND SHALL BE REMOVED. AGGREGATE BASE COURSE	14. ALL EXTERIOR WALL AND ROOF SHEATHING NAILS SHALL BE HOT-DIPPED GALVANIZED UNLESS NOTED OTHERWISE.  15. CONTRACTOR SHALL VERIEY AND COORDINATE THE LOCATION OF ALL SHEAR WALL	STRUCTURAL FRAMING AND CONNECTIONS FOR ADEQUACY DURING THE ERECTION AND CONSTRUCTION PROCESS IS THE RESPONSIBILITY OF THE CONTRACTOR.  7. VERIFY AND COORDINATE, WITH ALL CONTRACTORS, THE LOCATION OF ALL ARCHITECTURAL		
	UNDER SLABS ON GROUND SHALL BE AS SPECIFIED COMPACTED TO 6-INCH LAYERS (EXCEPT WHERE LOOSE FILL IS INDICATED ON DRAWINGS).	15. CONTRACTOR SHALL VERIFY AND COORDINATE THE LOCATION OF ALL SHEAR WALL ANCHORS WITH CONCRETE AND FRAMING CONTRACTORS PRIOR TO FABRICATION AND ERECTION.	SHOP DRAWINGS AND SUBMITTALS		
	<ul> <li>5. BACKFILL AGAINST INTERIOR FOUNDATION WALLS SHALL BE AS SPECIFIED COMPACTED TO MAXIMUM 6-INCH LAYERS.</li> <li>6. BACKFILL AGAINST EXTERIOR FOUNDATION WALLS SHALL BE AS SPECIFIED COMPACTED TO</li> </ul>	<ul><li>16. HANGER CONNECTIONS SHALL DEVELOP THE SHEAR STRENGTH OF THE TRUSS, BEAM OR JOIST.</li><li>17. NAILING OF WOOD FRAMING MEMBERS SHALL CONFORM TO THE FASTENER REQUIREMENTS</li></ul>	SHOP DRAWINGS AND CALCULATIONS BEARING A REGISTERED ENGINEER'S CERTIFICATION		
	MAXIMUM 6-INCH LAYERS.  7. PROVIDE MINIMUM 24 INCHES OF FREE DRAINING AGGREGATE AS SPECIFIED OVER ALL DRAIN TILES AND 4 INCHES BELOW.	OF IBC 2018 2304.10.1.  18. ALL FRAMING EXPOSED TO THE WEATHER OR IN CONTACT WITH CONCRETE SHALL BE PRESSURE-TREATED IN ACCORDANCE WITH THE AMERICAN WOOD PRESERVERS	SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW AND SHALL CONTAIN THE FOLLOWING INFORMATION FOR EACH TYPE AND SIZE TRUSS ASSEMBLY TO BE PROVIDED:  A. DETAIL OF TRUSS SHOWING SIZE OF MEMBERS.		
	TILLO / INCITLO BELOTT.	ASSOCIATION SPECIFICATIONS. WHERE POSSIBLE, ALL CUTS AND HOLES SHOULD BE COMPLETED BEFORE TREATMENT. CUTS AND HOLES DUE TO THE ON-SITE FABRICATION SHALL BE BRUSHED WITH 2 COATS OF COPPER NAPHTHENATE SOLUTION CONTAINING A	<ul><li>B. SPECIES AND WORKING STRESS OF LUMBER USED</li><li>C. LOADING CONDITIONS USED IN DESIGN.</li><li>D. CALCULATED FORCES FOR EACH MEMBER.</li></ul>		
		MINIMUM OF 2% METALLIC COPPER IN SOLUTION (PER AWPA STANDARD M4).  19. PREFABRICATED METAL JOIST HANGERS, HURRICANE CLIPS, HOLD-DOWN ANCHORS, AND OTHER ACCESSORIES SHALL BE AS MANUFACTURED BY "SIMPSON STRONG-TIE COMPANY"	<ul> <li>E. CONNECTOR SIZES AND LOCATION.</li> <li>2. INFORMATION PROVIDED ON SHOP DRAWINGS SHALL ALSO TAKE INTO ACCOUNT AND SHOW ALL SPECIAL DESIGN, FRAMING, AND CONNECTION REQUIREMENTS FOR TRUSSES, SUCH AS</li> </ul>		PROJECT INFORMATION:
		OR APPROVED EQUAL. INSTALL ALL ACCESSORIES PER THE MANUFACTURER'S REQUIREMENTS. ALL STEEL SHALL HAVE A MINIMUM THICKNESS OF 0.04 INCHES (PER ASTM A653) AND BE GALVANIZED (G60 COATING).	AT CONCENTRATED LOADS, UNBALANCED OR UNSYMMETRICAL LOAD CONDITIONS, AND OTHER NON TYPICAL FRAMING DETAILS.  3. AN ERECTION PLAN OF THE TRUSS FRAMING, CONCRETE, REBAR AND WOOD WALL PANELS		PROJECT NUMBER: 2025-0141.00  DATE: 10/01/2025
		20. REFER TO ARCHITECTURAL DRAWINGS FOR WALL ASSEMBLIES. COORDINATE SHEATHING LAYER REQUIREMENTS AND PLACEMENT TO MAINTAIN SPECIFIED STC RATINGS.	3. AN ERECTION PLAN OF THE TRUSS FRAMING, CONCRETE, REBAR AND WOOD WALL PANELS SHALL BE PROVIDED INDICATING ALL TRUSS LOCATIONS AND FRAMING CONDITIONS.		DRAWN BY: MTD
					CHECKED BY: TAC APPROVED BY:
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					ALIEET TITLE
					GENERAL NOTES
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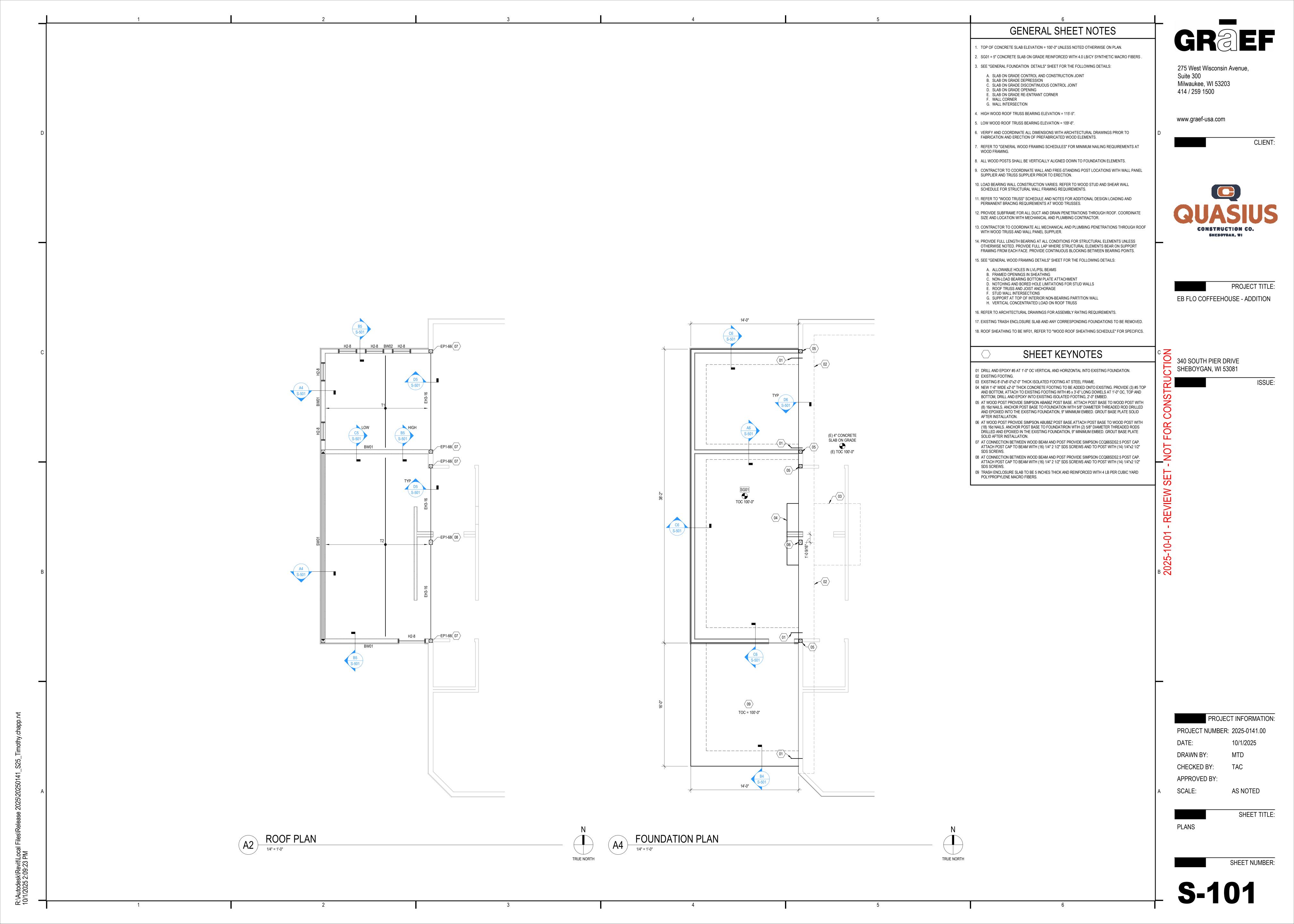


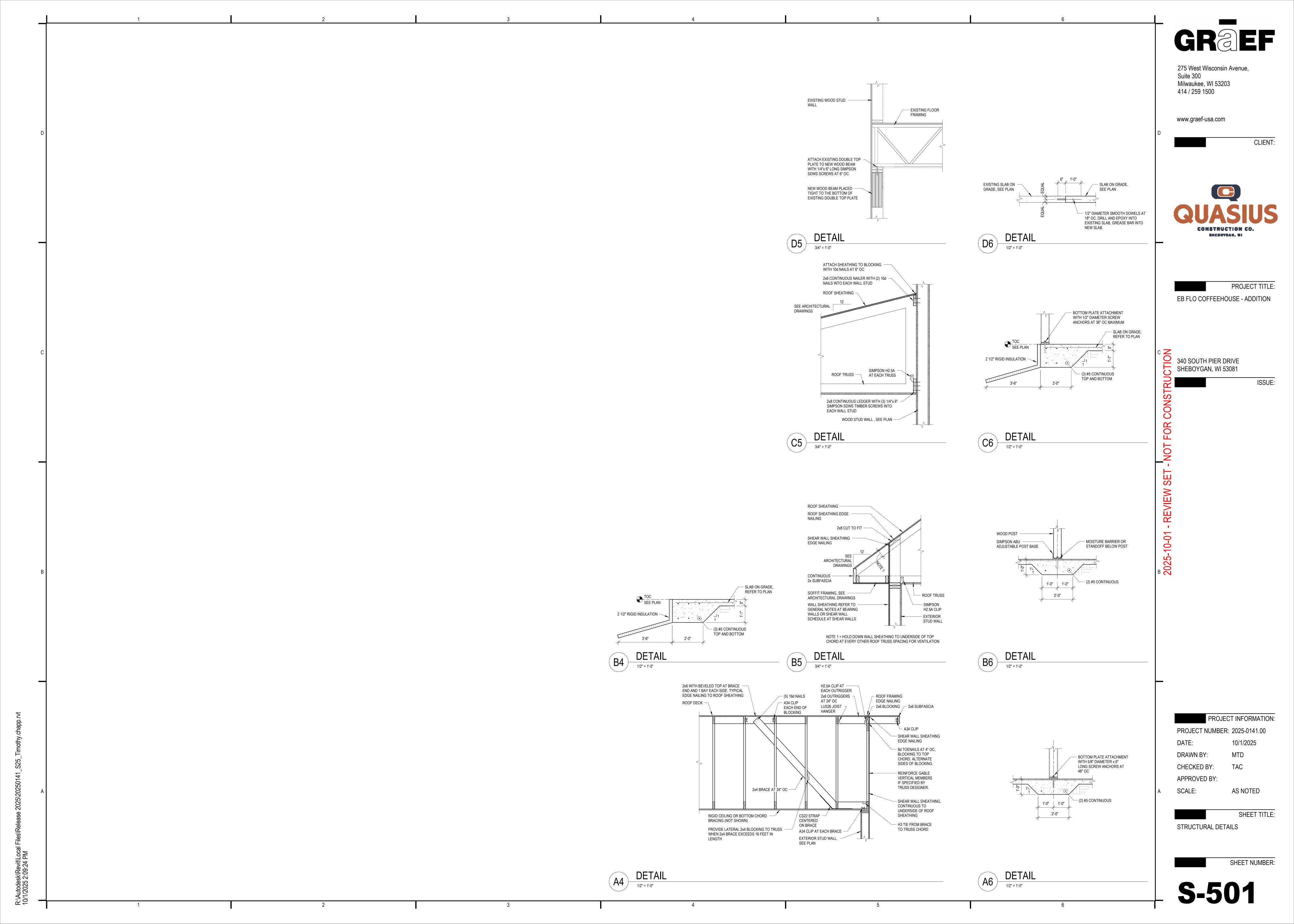


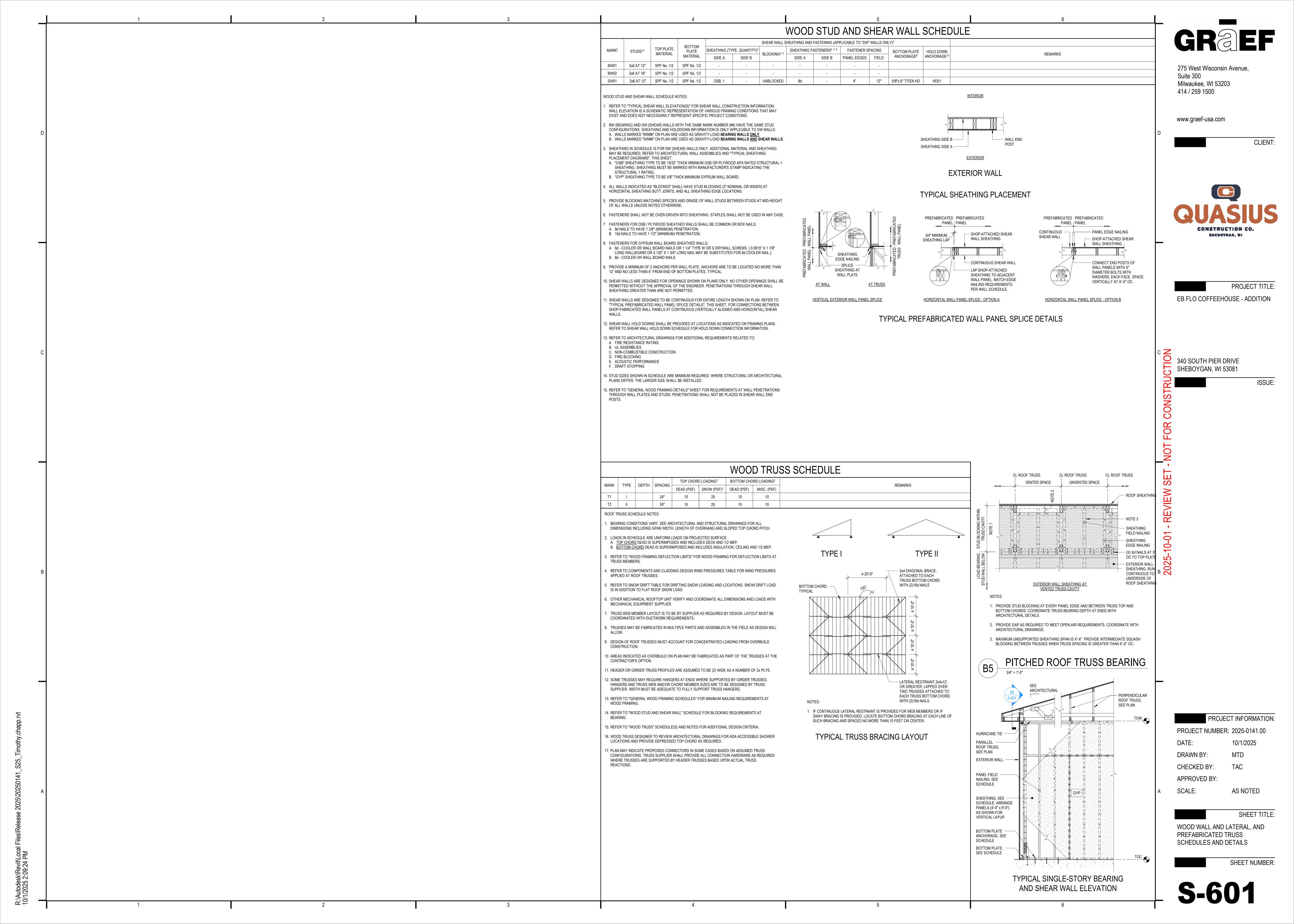
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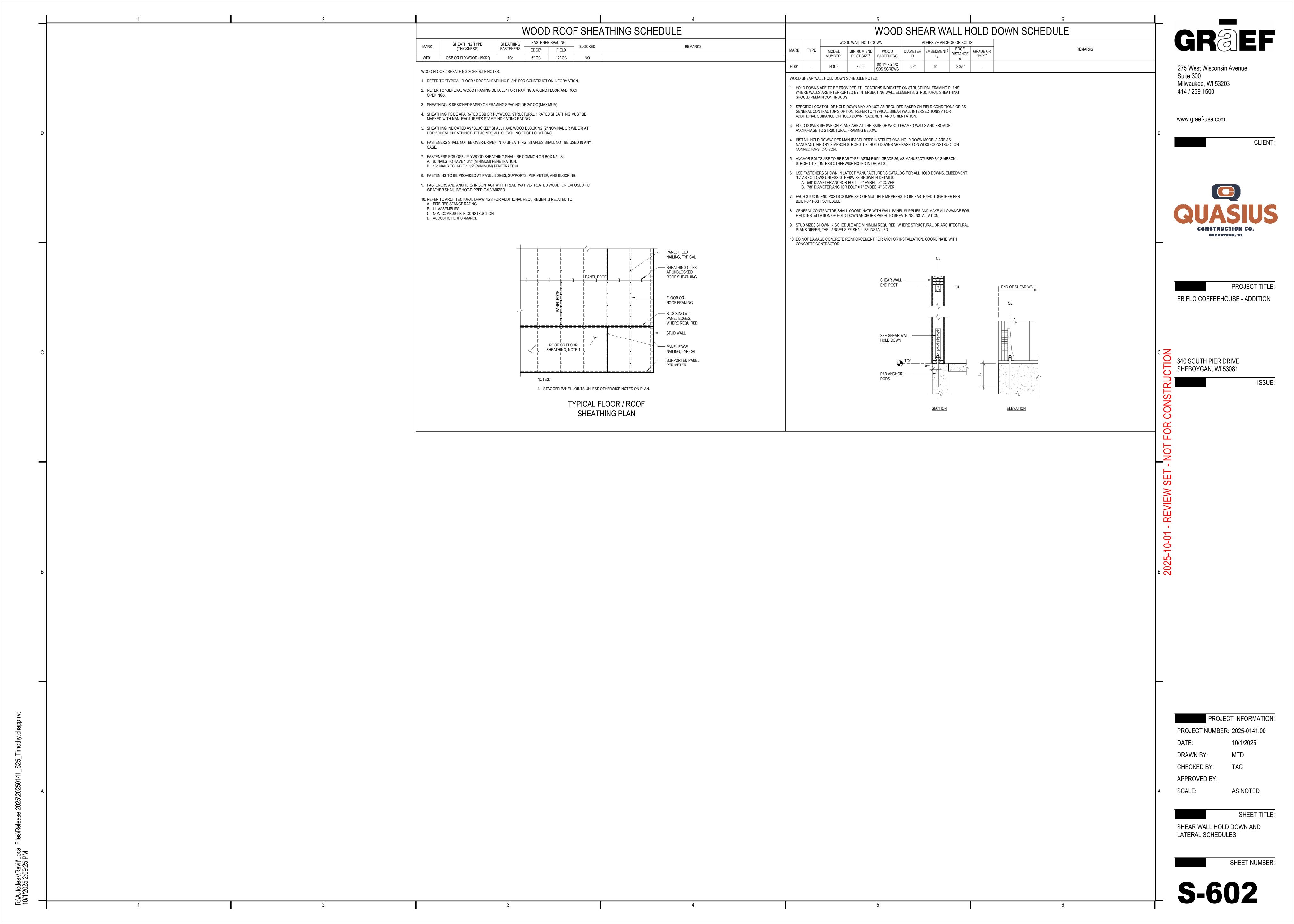


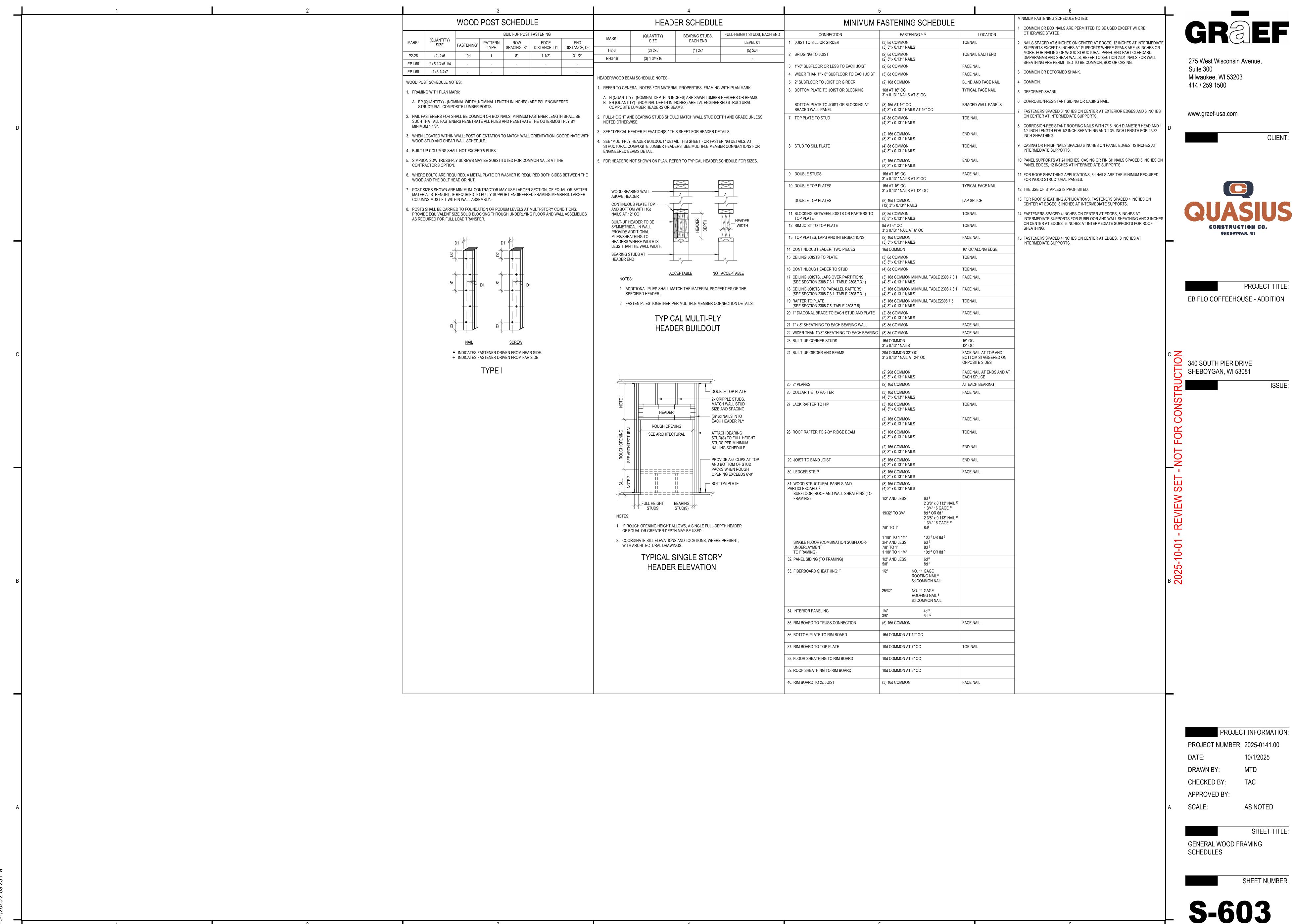
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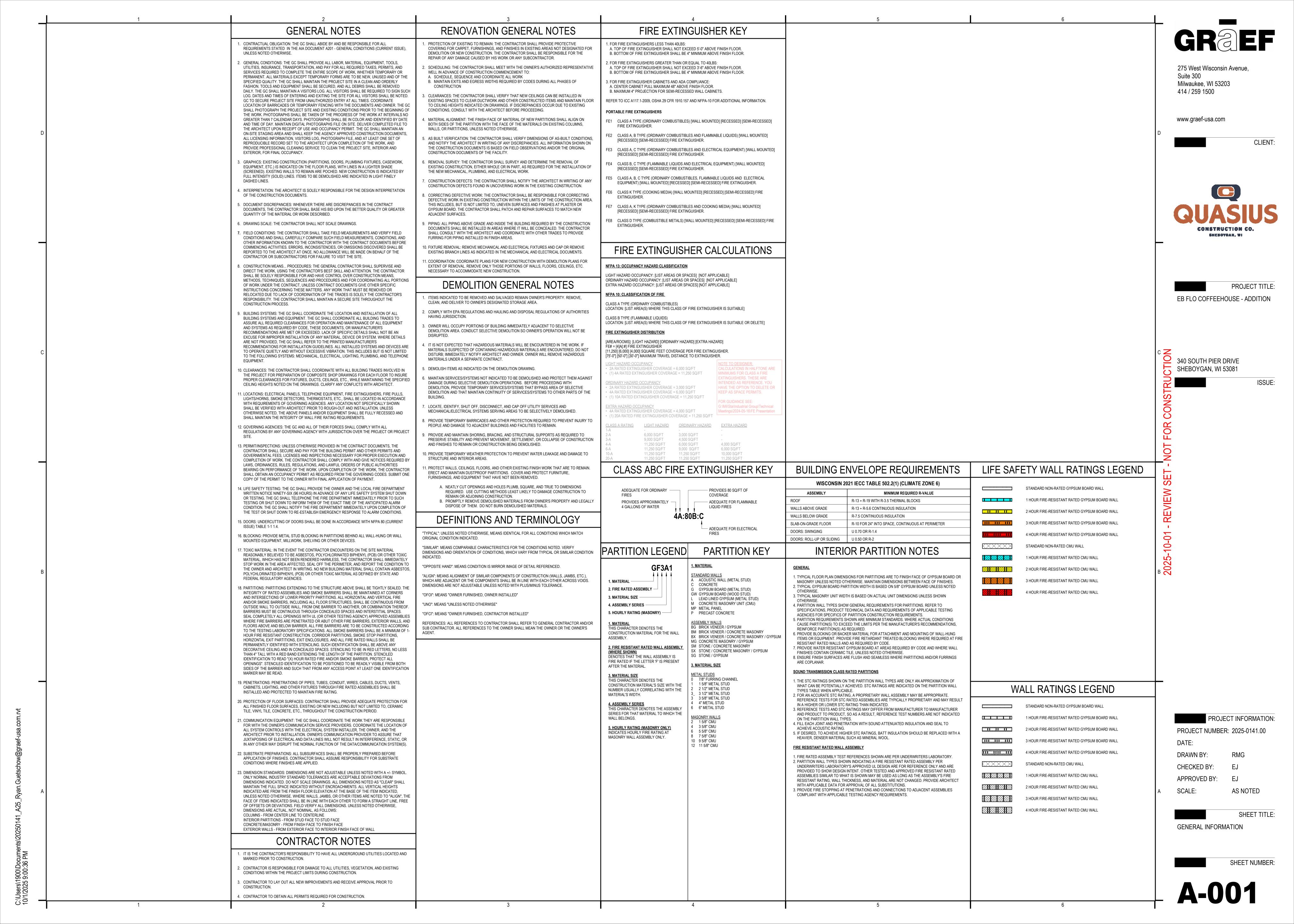


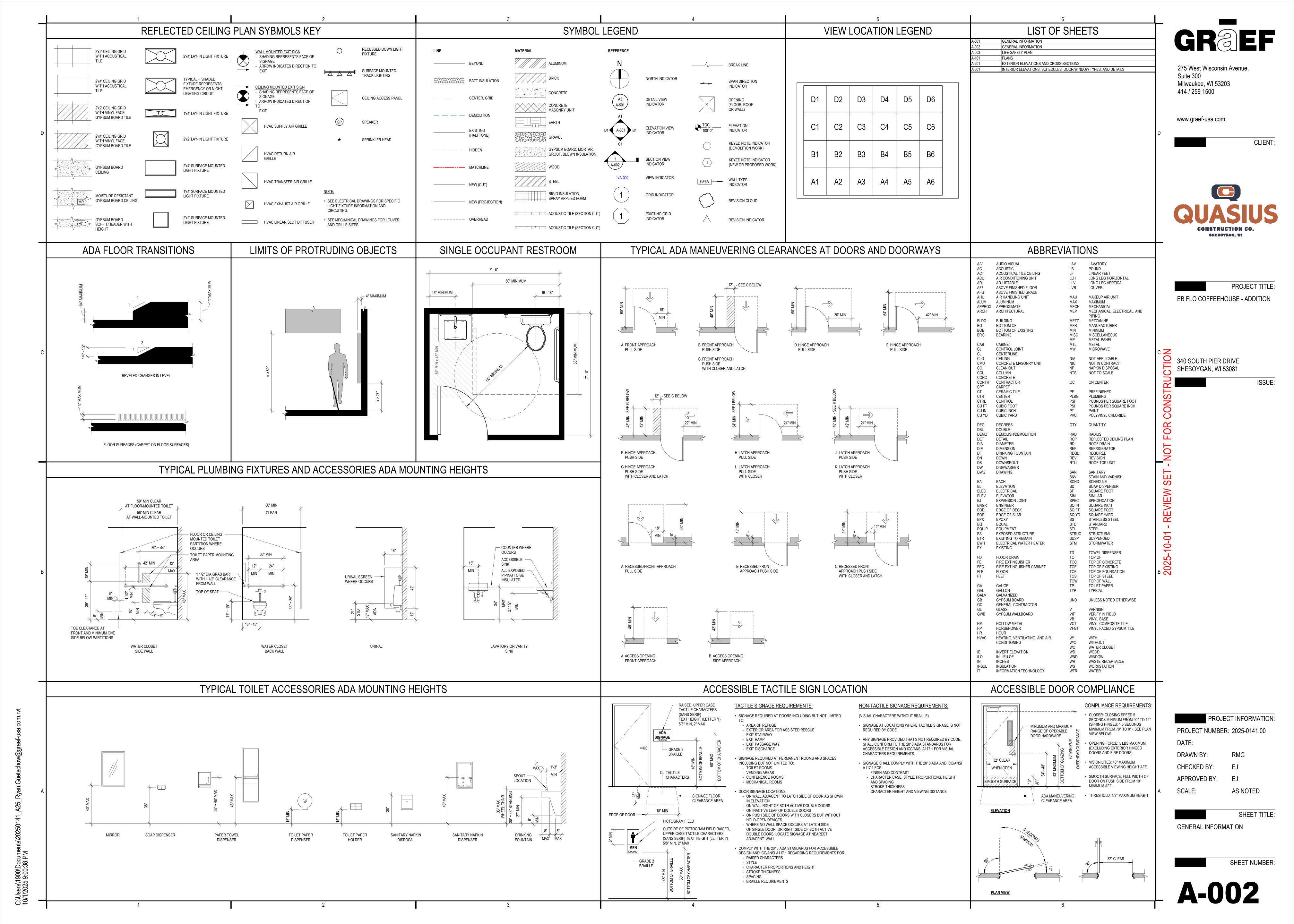


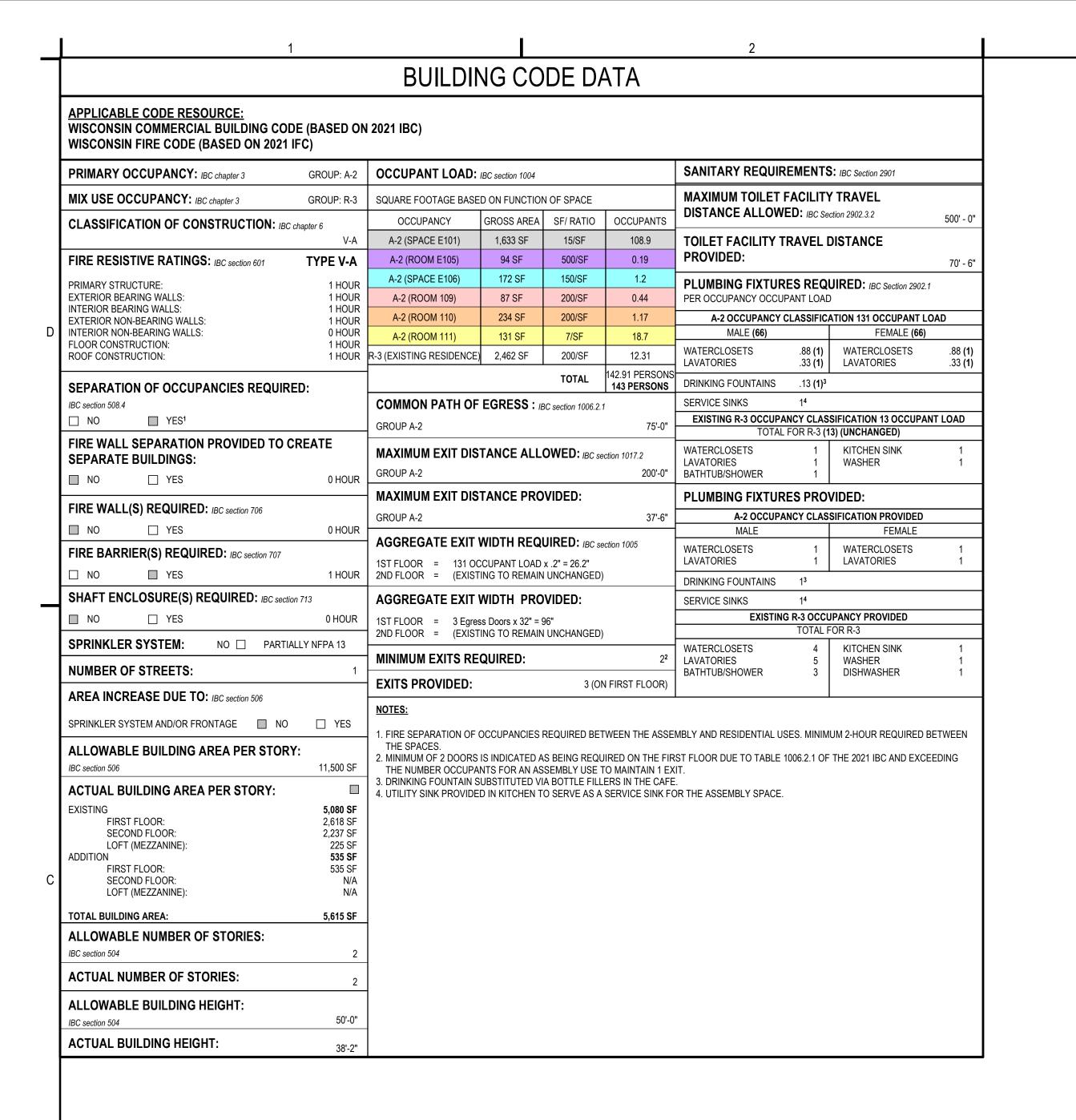


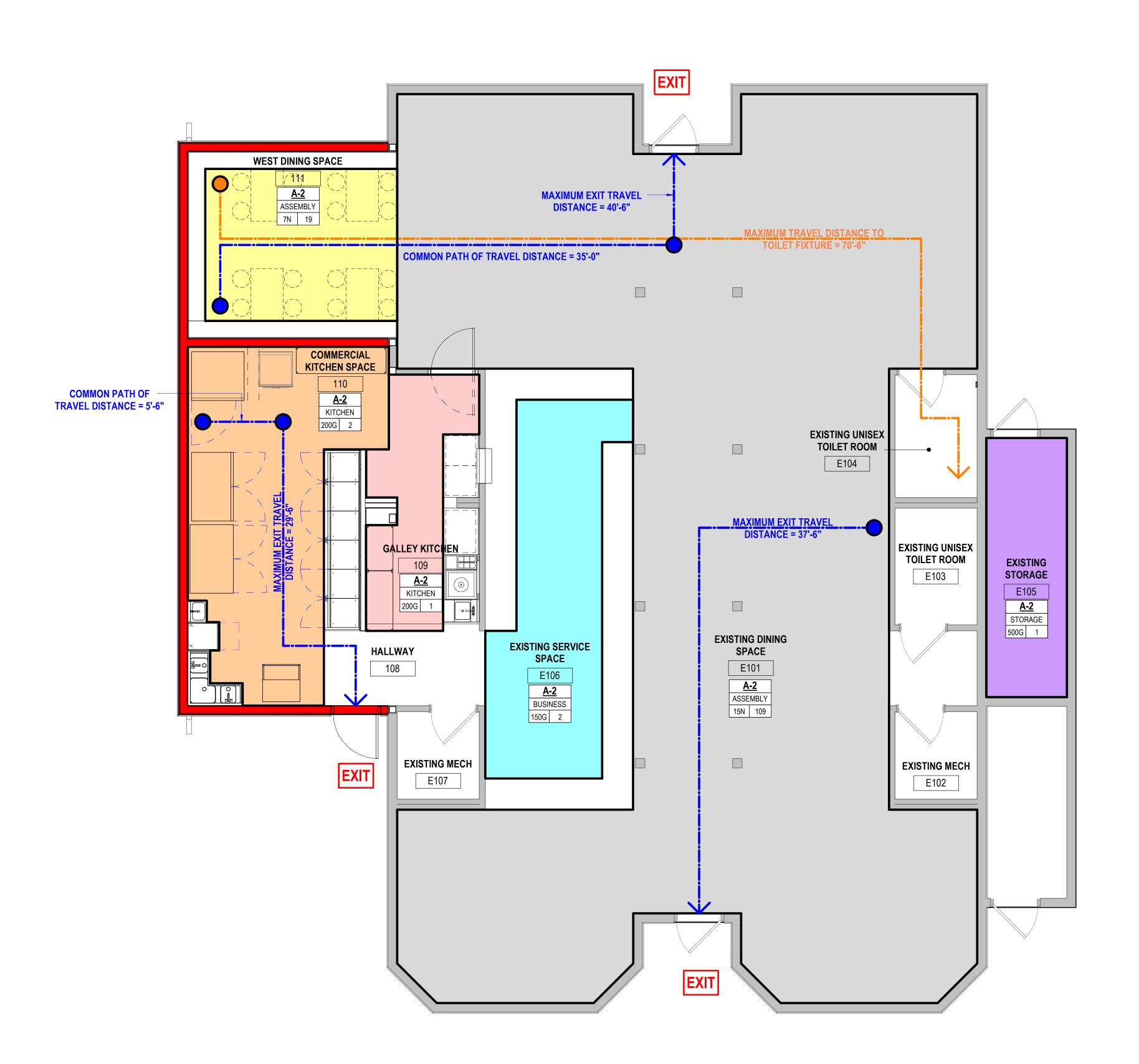












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CLIENT:

PROJECT TITLE: EB FLO COFFEEHOUSE - ADDITION

2 340 SOUTH PIER DRIVE SHEBOYGAN, WI 53081

PROJECT INFORMATION: PROJECT NUMBER: 2025-0141.00

DATE:

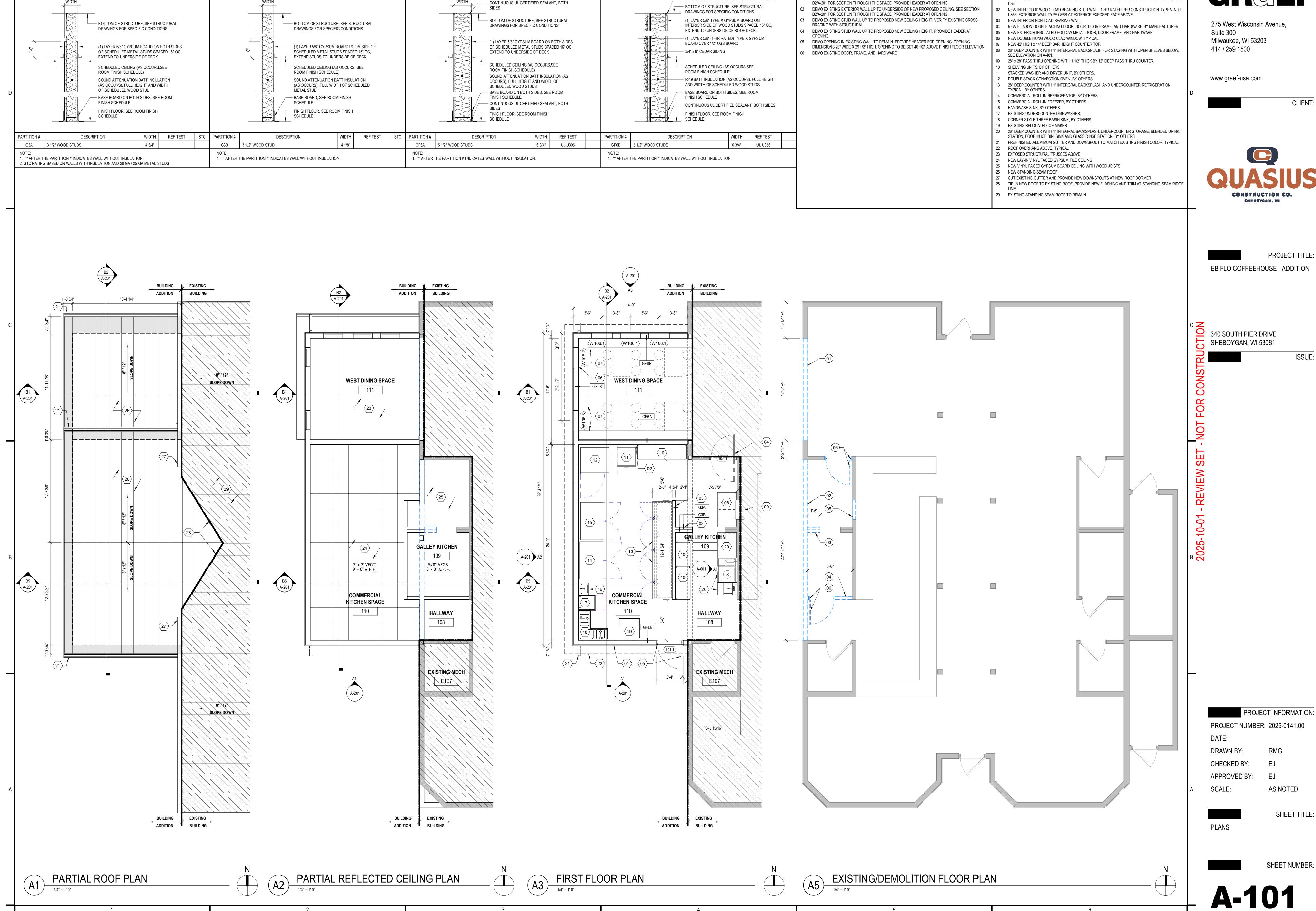
CHECKED BY: APPROVED BY:

SCALE: AS NOTED

LIFE SAFETY PLAN

SHEET TITLE:

FIRST FLOOR PLAN



1-HOUR FIRE RATED ASSEMBLY

INTERIOR LOAD BEARING

— CONTINUOUS UL CERTIFIED SEALANT, BOTH SIDES

**DEMO SHEET KEYNOTES** 

SHEET KEYNOTES

DEMO EXISTING EXTERIOR WALL UP TO UNDERSIDE OF NEW PROPOSED ROOF DECK. SEE SECTION 01 NEW EXTERIOR 6" WOOD LOAD BEARING STUD WALL. 1-HR RATED PER CONSTRUCTION TYPE V-A. U

NON-RATED PARTITION: (1) SIDE 5/8" GYPSUM BOARD

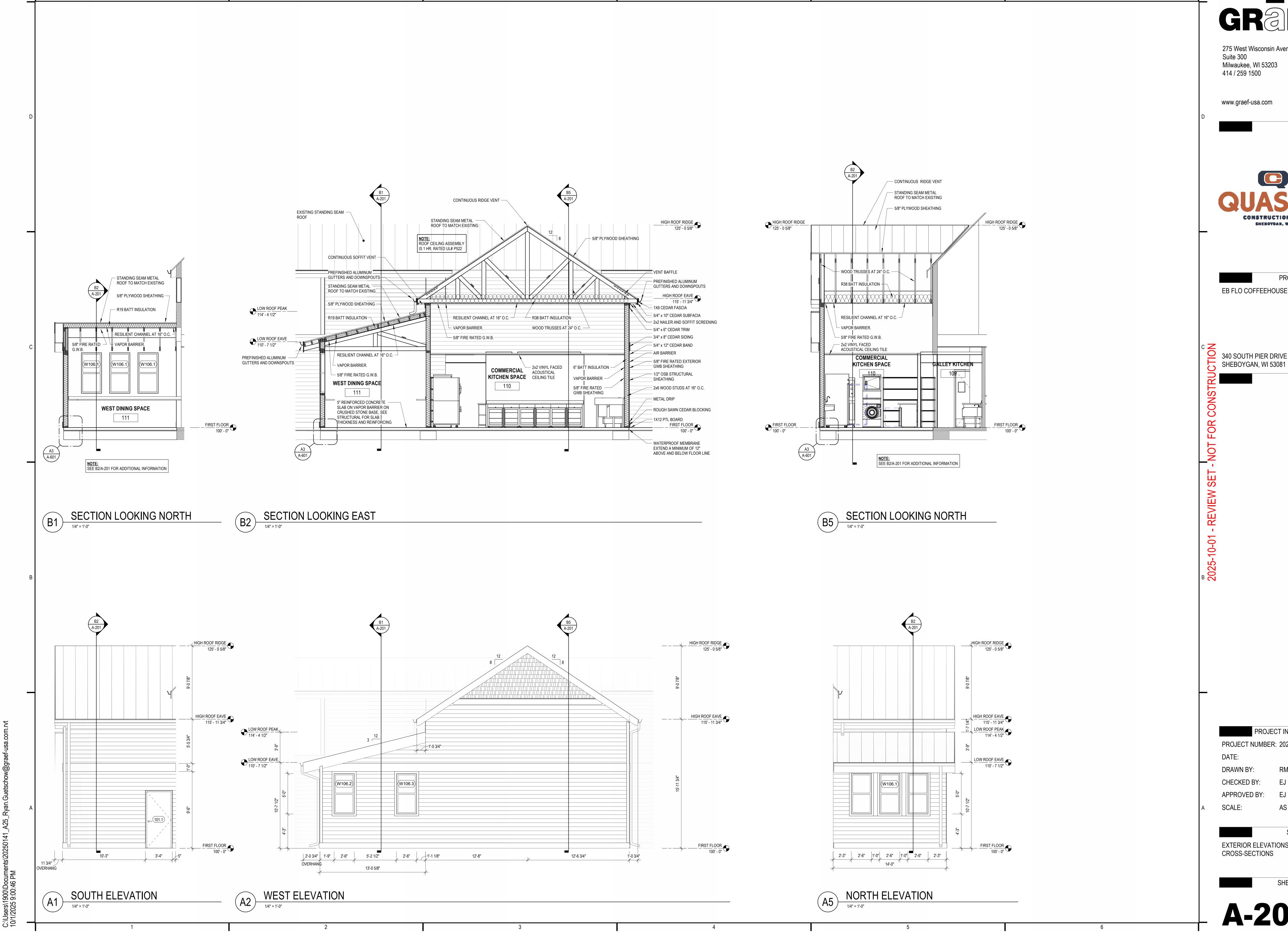
PARTIAL HEIGHT

1-HOUR FIRE RATED ASSEMBLY

INTERIOR LOAD BEARING

NON-RATED PARTITION: (2) SIDES 5/8" GYPSUM BOARD

PARTIAL HEIGHT



275 West Wisconsin Avenue, Milwaukee, WI 53203

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CLIENT:

PROJECT TITLE: EB FLO COFFEEHOUSE - ADDITION

2 340 SOUTH PIER DRIVE

PROJECT NUMBER: 2025-0141.00

APPROVED BY: EJ

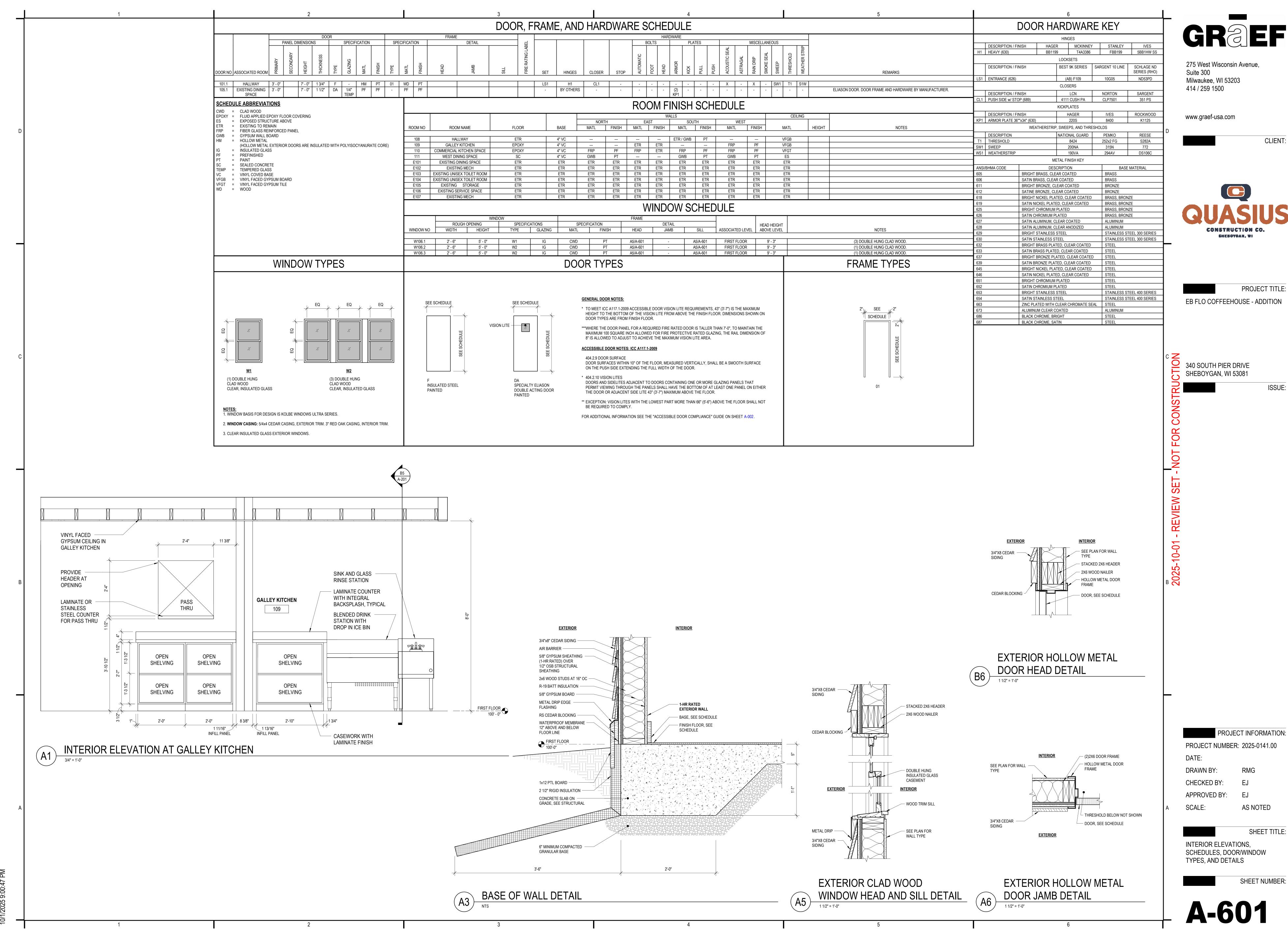
AS NOTED

SHEET TITLE:

EXTERIOR ELEVATIONS AND CROSS-SECTIONS

SHEET NUMBER:

**A-201** 





PROJECT TITLE: