

#### **GENERAL NOTES**

FABRICATION SHALL BE IN ACCORDANCE WITH METAL BUILDING SUPPLIER, STANDARD PRACTICES IN COMPLIANCE WITH THE APPLICABLE SECTIONS, RELATING TO DESIGN REQUIREMENTS AND ALLOWABLE STRESSES OF THE LATEST EDITION OF THE "AWS STRUCTURAL WELDING CODE D1.1" AND D1.3".

2.6 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR OVERALL PROJECT COORDINATION. ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS CONCERNING ANY MATERIALS NOT FURNISHED BY MIS.S. AND M.B.S. AND M.B

ASTM DESIGNATION MIN. YIELD STRENGTH

	HOT ROLLED STEEL SHAPES (W, & C)	A5/2	Fy = 50 KSI
	HOT ROLLED STEEL ANGLES (L)	A36	Fy = 36  KSI
	STEEL PIPES	A500	Fy = 42  KSI
	STRUCTURAL TUBING	A500	Fy = 42  KSI
	STRUCTURAL STEEL WEB PLATE	A572/A1011	Fy = 50 KSI
	STRUCTURAL STEEL FLANGE PLATES/BARS	A529/A572	Fy = 55  KSI
	COLD FORMED LIGHT GAGE	A653/A1011	Fy = 55  KSI
	ROOF & WALL SHEETS	A792/A653	$F_{y} = 50, 80 \text{ KSI}$
	CABLE BRACE	A475 - TYPE 1	EXTRA HIGH STRENGTH
	ROD BRACE	A36	Fy = 36 KSI
			MIN. TENSILE STRENGTH
	MACHINE BOLTS & NUTS	A307	Fu = 60 KSI
	HIGH STRENGTH BOLTS (1" & LESS)	A325-TYPE 1	Fu = 120 KSI
	HIGH STRENGTH BOLTS (>1"ø TO 1 1/2"ø)	A325-TYPE 1	Fu = 105 KSI
ĺ	HIGH STRENGTH BOLTS (>1"ø TO 1 1/2"ø) ANCHOR BOLTS (NOT SUPPLIED BY M.B.S.)	A36/A307/F1554	Fu = 60 KSI

PRIMER
SHOP PRIMER PAINT IS A RUST INHIBITIVE PRIMER WHICH MEETS THE END PERFORMANCE OF
FEDERAL SPECIFICATION SSPC NO. 15 AND IS GRAY OXIDE IN COLOR. THIS PAINT IS NOT
INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS. METAL BUILDING SUPPLIER IS NOT
RESPONSIBLE FOR ANY DETERIORATION OF THE SHOP PRIMER PAINT AS A RESULT OF
IMPROPER HANDLING AND/OR JOBSITE STORAGE. METAL BUILDING SUPPLIER SHALL NOT BE
RESPONSIBLE FOR ANY FIELD APPLIED PAINT AND/OR COATINGS.

(AISC CODE OF STANDARD PRACTICE, LATEST EDITION).
NOMINAL THICKNESS OF PRIMER WILL BE 1 MIL UNLESS OTHERWISE SPECIFIED IN CONTRACT
TORS LIBERTY.

1.4 GALVANIZED OR SPECIAL COATINGS: SEE CONTRACT DOCUMENTS

.5 ALL BOLTS ARE 1/2" x 0'-1 1/4" A307 EXCEPT :

A) ENDWALL RAFTER SPLICE  $-5/8"\phi \times 0"-13/4"$  A325-N B) ENDWALL COLUMN TO RAFTER CONNECTION - (SEE WALL ELEVATION)

C) MAIN FRAME CONNECTIONS — SEE CROSS SECTION
D) FLANGE BRACE CONNECTIONS — 1/2" ø x 0"-1 1/4" A325

NOTE: WASHERS ARE NOT SUPPLIED UNLESS NOTED OTHERWISE ON DRAWING

.6 A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTS ARE A325-N UNLESS SPECIFICALLY NOTED OTHERWISE. HOLES ARE NOT SLOTTED AND DESIGN IS BEARING CONNECTION.
STRUCTURAL BOLTS SHALL BE TIGHTENED BY THE "TURN-OF-THE-NUT" METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS" USING ASTM A325-OR A490 BOLTS, WHEN SPECIFICALLY REQUIRED. A325-N BOLTS ARE SUPPLIED WITHOUT WASHER UNLESS OTHERWISE NOTED ON THE DRAWINGS.

ALL BOLTED CONNECTIONS UNLESS NOTED ARE DESIGNED AS BEARING TYPE CONNECTIONS WITH BOLT THREADS NOT EXCLUDED FROM THE SHEAR PLANE.

BUILDINGS IN SEISMIC DESIGN CATEGORY C OR LOWER AND/OR WITH CRANE SYSTEMS 10 TONS OR LESS DO NOT REQUIRE TURN OF THE NUT PRE TENSIONING

CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:

INSIDE- UNDER ROOF PANELS & BASE OF WALL PANELS OUTSIDE - BETWEEN ROOF PANELS & RIDGE CAP - BETWEEN WALL PANELS & EAVE/GABLE TRIM

ERECTION NOTE:
ALL BRACING, STRAPPING, & BRIDGING SHOWN AND PROVIDED BY M.B.S. FOR THIS BUILDING IS
REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE
STRUCTURE. IF ADDITIONAL BRACING IS REQUIRED FOR STABILITY DURING ERECTION, IT SHALL
BE THE ERECTOR'S RESPONSIBILITY TO DETERMINE THE AMOUNT OF SUCH BRACING AND TO

PROCURE AND INSTALL AS NEEDED. ERECTION AND UNLOADING NOT BY G.W.B.

1.10 SHORTAGES
ANY CLAIMS OR SHORTAGES BY BUYER MUST BE MADE TO M.B.S. WITHIN FIVE (5) WORKING
DAYS AFTER DELIVERY, OR SUCH CLAIMS WILL BE CONSIDERED TO HAVE BEEN WAIVED BY THE CUSTOMER AND DISALLOWED.

CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)
CLAIMS FOR CORRECTION OF ALLEGED MISTITS WILL BE DISALLOWED UNLESS M.B.S. SHALL
HAVE RECEIVED PRIOR NOTICE THEREOF AND ALLOWED REASONABLE INSPECTION OF SUCH
MISTITS. THE CORRECTION OF MINOR MISTITS BY THE USE OF DRIFT PINS TO DRAW THE
COMPONENTS INTO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE
REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND
ARE NOT SUBJECT TO CLAIM. NO PART OF THE BUILDING MAY BE RETURNED FOR ALLEGED
MISFITS WITHOUT THE PRIOR APPROVAL OF M.B.S.

#### BUYER/END USE CUSTOMER RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO OBTAIN APPROPRIATE APPROVALS AND SECURE NECESSARY PERMITS FROM CITY, COUNTY, STATE, OR FEDERAL AGENCIES AS REQUIRED, AND TO ADVISE/RELEASE M.B.S. TO FABRICATE UPON RECEIVING

SUCH.

METAL BUILDING SUPPLIER (HEREAFTER REFERRED TO AS M.B.S.)

STANDARD SPECIFICATIONS APPLY UNLESS STIPULATED OTHERWISE IN THE CONTRACT
DOCUMENTS. M.B.S. DESIGN, FABRICATION, QUALITY CRITERIA, STANDARDS, PRACTICE,
METHODS AND TOLERANCES SHALL GOVERN THE WORK WITH ANY OTHER INTERPRETATIONS
TO THE CONTRARY NOTWITHSTANDING. IT IS UNDERSTOOD BY BOTH PARTIES THAT THE
BUYER/END USE CUSTOMER IS RESPONSIBLE FOR CLARIFICATION OF INCLUSIONS OR
EXCLUSIONS FROM THE ARCHITECTURAL PLANS AND/OR SPECIFICATIONS.

IN CASE OF DISCREPANCIES BETWEEN M.B.S. STRUCTURAL STEEL PLANS AND PLANS FOR
OTHER TRADES, M.B.S. PLANS SHALL GOVERN. (SECTION 3 AISC CODE OF STANDARD
PRACTICES. LATEST EDITION)

APPROVAL OF M.B.S. DRAWINGS AND CALCULATIONS INDICATE THE M.B.S. HAS CORRECTLY INTERPRETED AND APPLIED THE CONTRACT DOCUMENTS. THIS APPROVAL CONSTITUTES THE CONTRACTOR/OWNERS ACCEPTANCE OF THE M.B.S. DESIGN CONCEPTS, ASSUMPTIONS, AND LOADING. (SECTION 4 AISC CODE AND MBMA 3.3.3)

ONCE THE BUYER/END USE CUSTOMER HAS SIGNED M.B.S. APPROVAL PACKAGE AND THE PROJECT IS RELEASED FOR FABRICATION, CHANGES SHALL BE BILLED TO THE BUYER/
END USE CUSTOMER INCLUDING MATERIAL, ENGINEERING AND OTHER COSTS. AN ADDITIONAL FEE MAY BE CHARGED IF THE PROJECT MUST BE MOVED FROM THE FABRICATION AND

THE BUTER/END USE OUSTIMENT'S RESPONSIBLE FOR OVERALL PROJECT COORDINATION.
ALL INTERFACE, COMPATIBILITY, AND DESIGN CONSIDERATIONS
CONCERNING ANY MATERIALS NOT FURNISHED BY M.B.S. AND M.B.S. STEEL SYSTEM ARE TO BE
CONSIDERED AND COORDINATED BY THE BUYER/END USE CUSTOMER. SPECIFIC DESIGN CRITERIA
CONCERNING THIS INTERFACE BETWEEN MATERIALS MUST BE FURNISHED BEFORE RELEASE FOR FABRICATION OR M.B.S. ASSUMPTIONS WILL GOVERN (AISC CODE OF STANDARD PRACTICE,

2.7 IT IS THE RESPONSIBILITY OF THE BUYER/END USE CUSTOMER TO INSURE THAT M.B.S. PLANS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT M.B.S. OR ITS DESIGN ENGINEERS ARE ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THESE DRAWINGS ARE SEALED ONLY TO CERTIFY THE DESIGN OF THE STRUCTURAL COMONICATE CIRCUMSTANCES.

2.8 THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL IN ACCORDANCE WITH M.B.S. "FOR ERECTION" DRAWINGS ONLY. TEMPORARY SUPPORTS SUCH AS GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION SHALL BE DETERMINED, PURISHED AND INSTALLED BY THE ERECTOR. NO ITEMS SHOULD BE PURCHASED FROM A PRELIMINARY SET OF DRAWINGS, INCLUDING ANCHOR BOLTS. USE ONLY FINAL "FOR ERECTION" DRAWINGS FOR THIS USE. (AISC CODE OF STANDARD PRACTICE, LATEST EDITION.)

METAL BUILDING SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE ANCHOR BOLTS TO PERMIT THE TRANSFER OF FORCES BETWEEN THE BASE PLATE AND THE ANCHOR BOLT IN SHEAR, BEARING AND TENSION, BUT IT IS NOT RESPONSIBLE FOR THE TRANSFER OF ANCHOR BOLT FORCES TO THE CONCRETE OR THE ADEQUACY OF THE ANCHOR BOLT IN RELATION TO THE

CONCRETE.

UNLESS OTHERWISE NOTED PROVIDED IN THE ORDER DOCUMENTS, M.B.S. DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND CONSTRUCTION OF THE FOUNDATION OR FOUNDATION EMBEDMENTS. THE END USE CUSTOMER SHOULD BE ASSURE HIMSELF THAT ADEQUATE PROVISIONS ARE MADE IN THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER EXPERIENCED IN THE DESIGN OF SUCH STRUCTURES. (LATEST MBMA LOW RISE BUILDING SYSTEMS MANUAL) SYSTEMS MANUAL)

2.10 NORMAL ERECTION OPERATIONS INCLUDE THE CORRECTIONS OF MINOR MISFITS BY MODERATE AMOUNTS OF REAMING, CHIPPING, WELDING OR CUTTING, AND THE DRAWING OF ELEMENTS INTO LINE THROUGH THE USE OF DRIFT PINS. ERRORS WHICH CANNOT BE CORRECTED BY THE FOREGOING MEANS OR WHICH REQUIRE MAJOR CHANGES IN MEMBER CONFIGURATION ARE TO BI REPORTED IMMEDIATELY TO M.B.S. BY THE BUYER/END USE CUSTOMER, TO ENABLE WHOEVER IS RESPONSIBLE EITHER TO CORRECT THE ERROR OR TO APPROVE THE MOST EFFICIENT AND ECONOMIC METHOD OF CORRECTION TO BE USED BY OTHERS. (AISC CODE OF STANDARD DRACTICE LATEST ENTION). PRACTICE LATEST EDITION)

2.11 NEITHER THE FABRICATOR NOR THE BUYER/END USE CUSTOMER WILL CUT, DRILL OR OTHERWISE ALTER HIS WORK, OR THE WORK OF OTHER TRADES, TO ACCOMMODATE OTHER TRADES, UNLESS SUCH WORK IS CLEARLY SPECIFIED IN THE CONTRACT DOCUMENTS. WHICHEVER SUCH WORK IS SPECIFIED, THE BUYER/END USE CUSTOMER IS RESPONSIBLE FOR FURNISHING COMPLETE INFORMATION AS TO MATERIALS, SIZE, LOCATION AND NUMBER OF ALTERATIONS PRIOR TO PREPARATION OF SHOP DRAWINGS. (AISC CODE OF STANDARD PRACTICE LATEST EDITION)

2.12 <u>Warning</u> in no case should galvalume steel panels be used in conjunction with lead or copper both lead and copper have harmful corrosive effects on the galvalume alloy coating when they are in contact with galvalume steel panels. Even run-off from copper flashing, wiring, or tubing onto galvalume should be

2.13 SAFETY COMMITMENT. METAL BUILDING SUPPLIER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF M.B.S. IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE. LOCAL, STATE, AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKERS AFETY. MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED. WHERE APPLICABLE, ARE RECOMMENDED.

2.14 ROOF DRAINAGE SYSTEMS (GUTTER, DOWNSPOUTS, ETC.) MUST BE FREE OF ANY OBSTRUCTION TO ENSURE SMOOTH OPERATION AT ANY GIVEN TIME.

2.15 IT IS RECOMMENDED BY FACTORY MUTUAL (REFERENCE B2.44) THAT ROOFS BE CLEARED OF SNOW WHEN HALF OF THE MAXIMUM SNOW DEPTH IS REACHED. THE MAXIMUM SNOW DEPTH CAN BE ESTIMATED BASED ON THE DESIGN SNOW LOAD AND THE DENSITY OF SNOW AND/OR ICE BUILDUP. SEE TABLE BELOW.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

FOR SNOW/ICE REMOVAL PROCEDURE, REFER TO METAL BUILDING SYSTEM MANUAL 2002 EDITION, SECTION A8.4, PAGE XI-A8-2

## **BUILDING LOADS** THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:	(94918A)	(94918B) (LEAN-TO))	DWN.
DESIGN CODE / WIND CODE	: IBC-18	: IBC-18	
OCCUPANCY / RISK CATEGORY	: II—Normal	: II—Normal	DATE 05/31/24 06/07/24
ENCLOSURE	: Enclosed	:Partially Enclosed	2 8 8
ROOF DEAD LOAD (D) (PSF)	: 2.00	: 2.42	
ROOF COLLATERAL LOAD (C) (PSF)	: 2.00	: 2.00	
WIND LOAD			ISSUE APPROVAL PERMIT
ULTIMATE WIND SPEED, (VULT) (MPH)	: 115.00	: 115.00	ISSUE PPROVA PERMIT
WIND EXPOSURE CATEGORY	:C	: C	[종[윤[윤]
INTERNAL PRESSURE COEFFICIENT, (GCpi)	: 0.18/-0.18	: 0.55/-0.55	
WALL PANEL DESIGN WIND PRESSURE (PSF)	: 23.48/-25.43	: 31.60/-33.57	
WIND ENCLOSURE CLASSIFICATION (	: Enclosed	: Partially Enclosed	
LIVE LOAD		•	
PRIMARY FRAMING (PSF)	: 20.00	: 20.00	
TRIB. AREA REDUCTION	: No	: No	
SECONDARY FRAMING (PSF)	: 20.00	: 20.00	
SNOW LOAD	. 20.00	. 20.00	
	. 21.00	: 31.00	
GROUND SNOW LOAD, (Pg) (PSF)		: 21.00	
ROOF SNOW LOAD, (Pf) (PSF)	: 21.00	: 21.00	
SNOW EXPOSURE FACTOR, (Ce)	:1.00	:1.00	/
SNOW IMPORTANCE FACTOR, (Is)	:1.00	:1.00	-1/A
THERMAL FACTOR, (Ct)	: 1.00	: 1.20	$\sqcup \longleftarrow$
SEISMIC LOAD			
SEISMIC IMPORTANCE FACTOR, (Ie)	: 1.00		/ '
SITE CLASSIFICATION SPECTRAL RESPONSE ACCELERATION SPECTRAL RESPONSE COEFFICIENTS SEISMIC DESIGN CATEGORY	:D		
SPECTRAL RESPONSE ACCELERATION	: Ss = 0.186 : S1 = 0.056		\
SPECTRAL RESPONSE COEFFICIENTS	: Sds = 0.198 : Sd1 = 0.090		
BASIC SEISMIC FORCE RESISTING SYSTEM			178
	DETAILED FOR RESISTANCE		
	:RIGID FRAMES (OMF)		
	:BRACED FRAMES (OCBF/OMF)		
	(94918A) (9	94918B)	
TOTAL DESIGN BASE SHEAR, (V) (KIPS)	:LONGITUDINAL = 2.41 =	0.20	
	:TRANSVERSE = 2.27 =	0.14	🗐
			7
RESPONSE MODIFICATION FACTORS, (R)	:RIGID FRAMES = 3.00 $\Omega$ = 3.	00	%
	:SW WIND BENT = 3.00 $\Omega$ = 3.	00 (ONLY FOR 94918A)	TUMEH TUMEH
		·	၂립의필
SEISMIC RESPONSE COEFFICIENTS, (Cs)	:RIGID FRAMES = 0.0662		기기기롱
	:SW WIND BENT = 0.0662 (ON	NLY FOR 94918A)	AMER AMER 2716 CH
			4 4 5
ANALYSIS PROCEDURE USED	:EQUIVALENT LATERAL FORCE PROC	EDURE	
OTHER LOADS/REQUIREMENTS			NAMI NA
•			CT CT
			CUSTOWER NAME: AMER TUMEH PROJECT NAME: AMER TUMEH PROJECT COATION: 2716 CHIMNEY PARK RD, BLUFF, UT 84512

BUILDING DESCRIPTION: WIDTH (FT) LENGTH (FT) EAVE HEIGHT AT BSW (FT) EAVE HEIGHT AT FSW (FT) ROOF SLOPE AT BSW ROOF SLOPE AT FSW BAY SPACING (FT)		(94918A) : 60.00 : 70.00 : 19.75 : 26.00 : 1.25:12 : N/A : 3 AT 23.33	(94918B) (LEAN-TO)) :10.00 :20.33 :20.00 :21.00 :1.25:12 :N/A :1 AT 20.33
COVERING AND TRIMS: ROOF PANELS & TRIMS			
PANEL TYPE PANEL COLOR TRIM COLORS		:26 GA. PBR :ASH GRAY	:26 GA. PBR :ASH GRAY
GABLE/EAVE		: COAL BLACK	: COAL BLACK
EAVE GUTTER		:N/A	: COAL BLACK
WALL PANELS & TRIMS			
PANEL TYPE		:26 GA. PBR	: N/A
PANEL COLOR		:CHARCOAL GRAY	: N/A
TRIM COLORS CORNER		: COAL BLACK	: N/A
FRAMED OPENING		: COAL BLACK	: N/A
DOWNSPOUTS		:N/A	: COAL BLACK
BASE		: COAL BLACK	: N/A
		ROOF SOFFIT PANE	LS & TRIMS (94918A ONLY)
ROOF INSULATION	. 0 1 /0" (D 70) CKY	AUGO PANEL TYPE	: 26 GA. PBR
WALL INSULATION	: 9 1/2" (R-30) SKY : 6" (R-19) WMP-VR	FAINLL COLOR	
WAINSCOT PANELS & TRIMS	` '	TI COLOTT	
PANEL TYPE	: 26 GA. PBR	PANEL TYPE	: 26 GA. PBR
PANEL COLOR	: COAL BLACK	PANEL COLOR	: POLAR WHITE
TRIM COLOR	: COAL BLACK	TRIM COLOR	: POLAR WHITE



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GRAND JUNCTION, CO 8150 PHONE: (800)-497-2135 WWW.GREATWESTERNBUILDINGS.(

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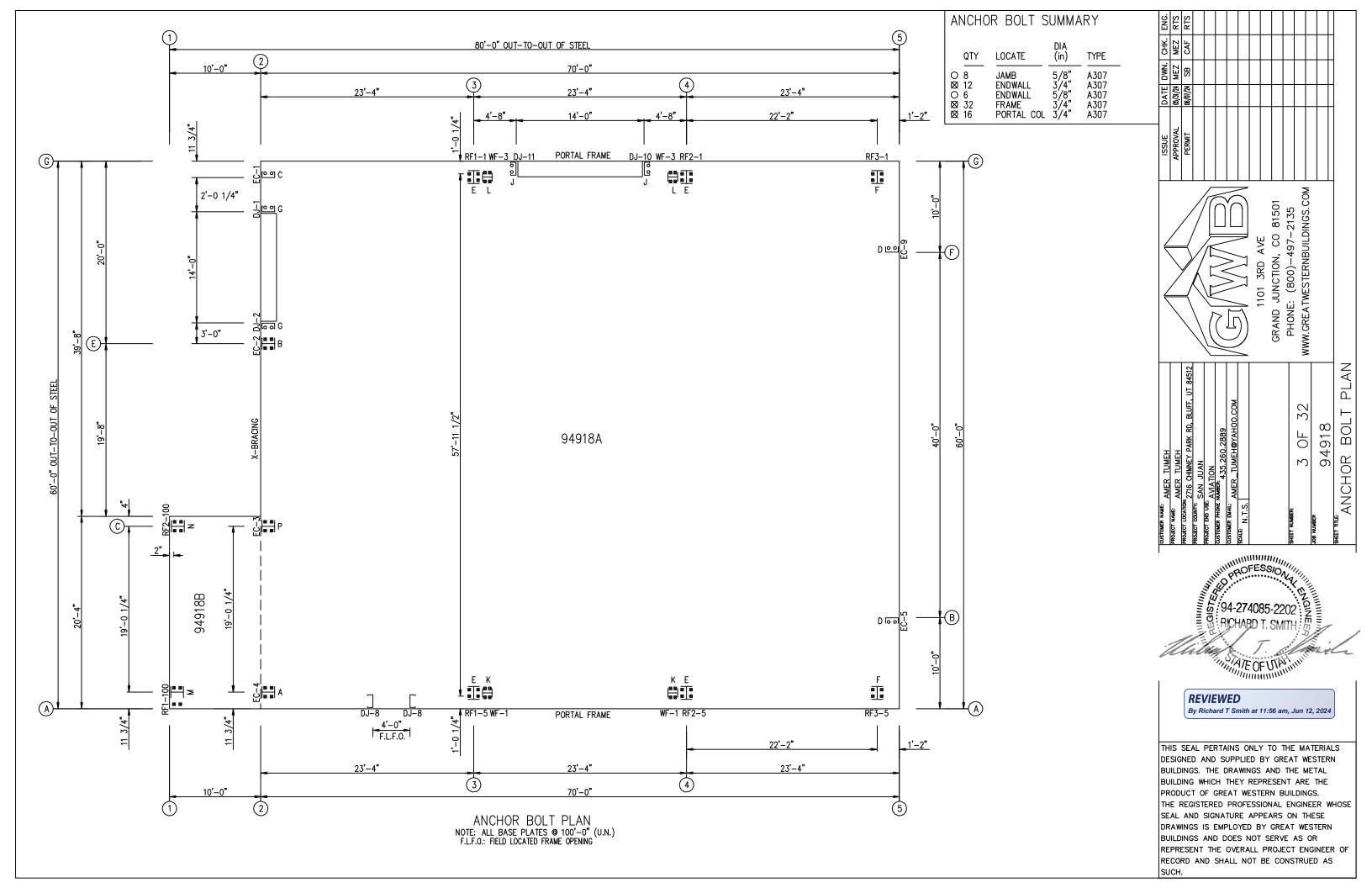
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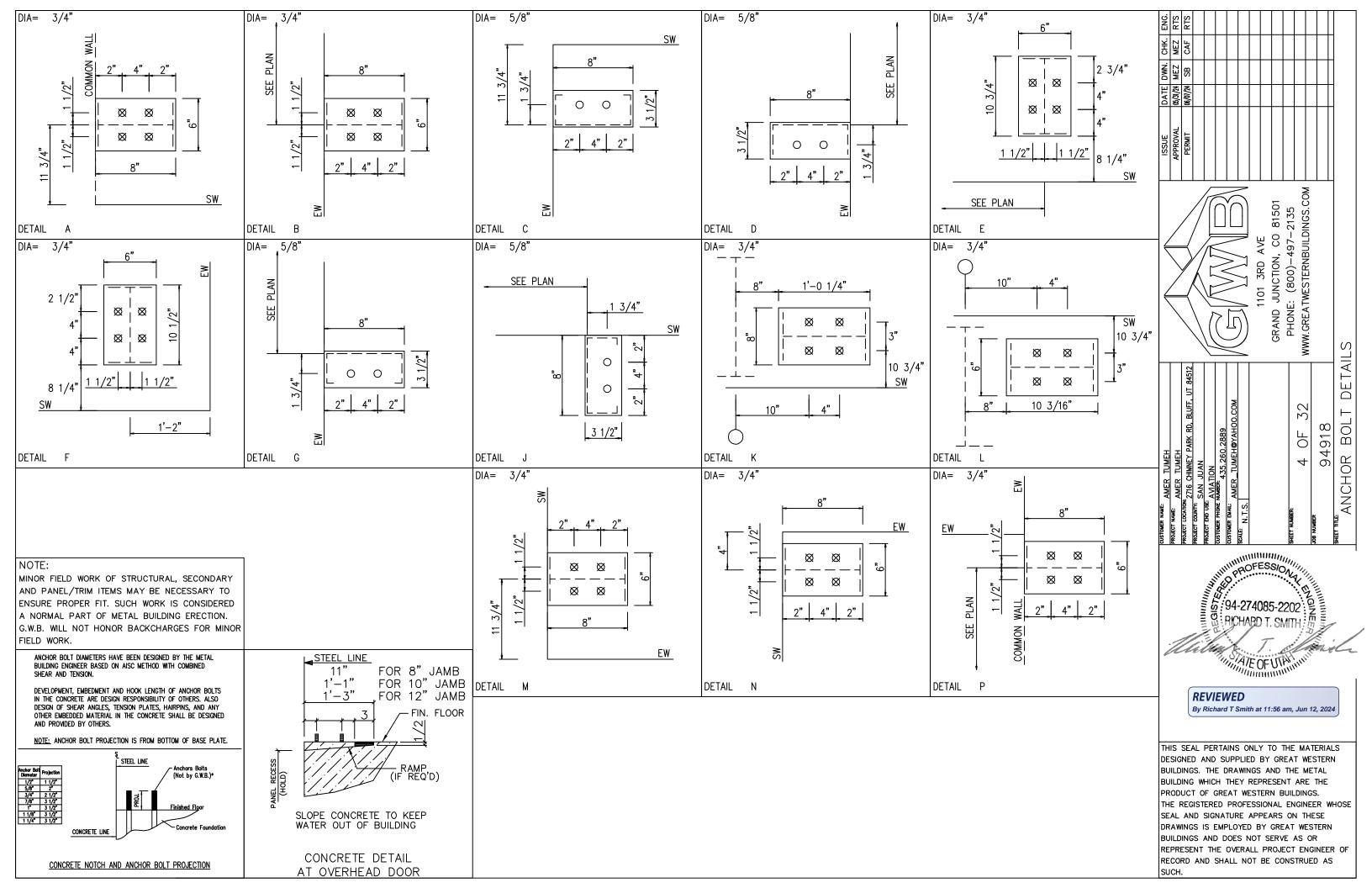
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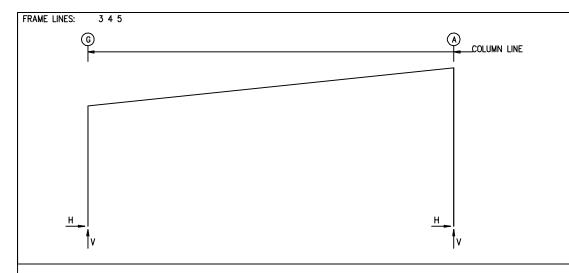
REVIEWED By Richard T Smith at 11:56 am, Jun 12, 2024

THIS SEAL PERTAINS ONLY TO THE MATERIALS DESIGNED AND SUPPLIED BY GREAT WESTERN BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE PRODUCT OF GREAT WESTERN BUILDINGS. THE REGISTERED PROFESSIONAL ENGINEER WHOSE SEAL AND SIGNATURE APPEARS ON THESE DRAWINGS IS EMPLOYED BY GREAT WESTERN BUILDINGS AND DOES NOT SERVE AS OR REPRESENT THE OVERALL PROJECT ENGINEER OF

RECORD AND SHALL NOT BE CONSTRUED AS



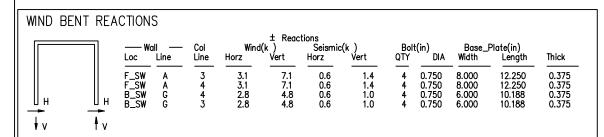




	DIOID FDANE												
RIGID	FRAME:		MAXIMUM	REACTION	IS, ANCI	HOR BOLT	S, & BASE	PLATE	:S				
				_									
Frm	Col	Load	Coli Hmax	umn_Reac V	tions(k Load	) Hmin		Bolt	(in)	Rase	e_Plate(in)		Grout
Line	Line	ld	H	Vmax	ld	Η	Vmin	QTY	DIA	Width	Length	Thick	(in)
		_											
3	G	2	8.1	19.5	5	-5.0	-4.7	4	0.750	6.000	10.75	0.375	0.0
					3	-5.0 -5.0	-4.7 -8.1						
3	Α	4	4.1 -8.1	-5.8 20.0	2	-8.1	20.0	4	0.750	6.000	10.75	0.375	0.0
		1	-8.1	20.0	6	1.3	-6.5						

RIGID I	RAME:		MAXIMUM	REACTION	IS, ANCI	HOR BOLT	TS, & BASE	PLATE	:S				
Frm Line	Col Line	Load Id	Colu Hmax H	ımn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bolf QTY	(in) DIA	Base Width	:_Plate(in) Length	Thick	Grout (in)
4	G	2	8.1	19.5	5 3	-5.0 -5.0	-4.7 -8.1	4	0.750	6.000	10.75	0.375	0.0
4	A	4 1	4.1 -8.1	-5.8 20.0	2 6	-8.1 1.3	20.0 -6.5	4	0.750	6.000	10.75	0.375	0.0

RIGID I	FRAME:		MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES									
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bolt(in) QTY DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
5	G	2	5.0	12.4	5 3	-2.9 -2.9	-2.6 -4.7	4 0.750	6.000	10.50	0.375	0.0
5	A	4 2	2.4 -5.0	-3.2 12.7	2 6	-5.0 0.6	12.7 -3.7	4 0.750	6.000	10.50	0.375	0.0

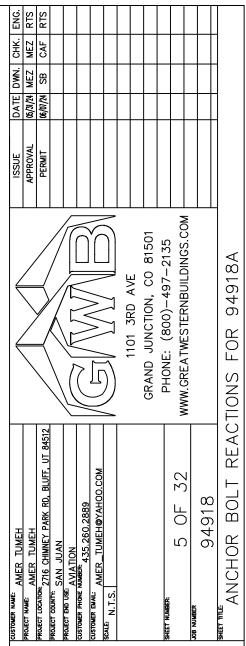


RIGID	FRAN	ΛE:	BAS	C COLUM	N REACT	10NS (k )	)						
FRAME Line 3 3	Column Line G A	Horz	-Dead Vert 2.5 2.7	Collo Horz 0.6 -0.6	Vert 1.5	Horz 6.2 -6.2	Vert	Horz 6.5 -6.5		Wind Horz -9.2 0.9	_Left1- Vert -15.9 -12.4	-Wind_ Horz 2.0 7.8	Right1- Vert -6.6 -12.3
FRAME Line 3	Column Line G A	Wind Horz -9.3 0.9	Vert	-Wind_ Horz 2.6 7.2	Vert		Vert	Wind Horz -0.6 0.4	-8.5			Seismid Horz 0.4 0.3	
	Column Line G A	-MIN_S Horz 6.2 -6.2	NOW Vert 14.9 15.0										
FRAME Line 4 4	Line	Horz 0.9 -0.9	-Dead Vert 2.5 2.7	Horz 0.6	Vert 1.5	Horz 6.2 -6.2	Vert	Horz 6.5 -6.5	-Snow Vert 15.6 15.8		Vert -15.9	-Wind_ Horz 2.0 7.8	Vert −6.6
FRAME Line 4 4	Line	Horz -9.3	Vert	Horz	Vert	Wind Horz -2.3 3.0	Vert	Horz -0.6	I_Long2- Vert -8.5 -8.3	Horz -0.4	Vert -0.2	Seismid Horz 0.4 0.3	Vert 0.2
FRAME Line 4	Column Line G A	Horz 6.2											
	Line	 Horz 0.7 -0.7	1.9		Vert 0.9	Horz	Vert 9.1		-Snow Vert 9.5 9.6		_Left1- Vert -9.7 -7.5	-Wind_ Horz 1.2 4.8	Vert −4.0
FRAME Line 5	Line G			Horz	Vert −0.7	Horz	Vert −7.9	Horz		-Seism Horz -0.3 -0.2	ic_Left Vert -0.2 0.2	Seismid Horz 0.3 0.2	_Right Vert 0.2 -0.2
FRAME Line 5	Column Line G A	-MIN_S Horz 3.7 -3.7											
l													

NOTES FOR REACTIONS	
Building reactions are based on the following building data: Width (ft) Length (ft) Eave Height (ft) Roof Slope (rise/12) Dead Load (psf) Collateral Load (psf) Live Load (psf) Snow Load (psf) Ultimate Wind Speed (mph) Wind Code Exposure Closed/Open Importance Wind Importance Seismic Seismic Coeff (Fa*Ss)	= 60.00 = 70.00 = 19.75/26.00 = 1.25:120/ = 2.00 = 2.00 = 20.00 = 21.00 = 115.00 = IBC-18 = C = Enclosed = 1.00 = 1.00 = B = 0.30
ID Description	
1 Dead+Collateral+Snow 2 Dead+Collateral+Snow+Slide_Sno 3 0.6Dead+0.6Wind_Left1 4 0.6Dead+0.6Wind_Left1 5 0.6Dead+0.6Wind_Left2 6 0.6Dead+0.6Wind_Long1R 7 0.6Dead+0.6Wind_Left1+0.6Wind_8  0.6Dead+0.6Wind_Pressure+0.6Wind_Dead+0.6Wind_Suction+0.6Wind_Dead+0.6Wind_Suction	Suction ind_Long1L .SL_3 id_Long1E snow_Drift+0.75Floor_Live d_Suction ind_Long2L

Wo Loc	Line	- Col Line	Horz	ind — Vert	− —Sei Horz	smic - Vert	- (lb	/ft) Seis	Note
L_EW F_SW R_EW	2 A 5 G	E,C 3,4	4.2	4.8	0.5	0.5			(a) (h)
B_SW	Ğ	3,4							(")
(a)Wind (h)Rigio	bent frame	in bay e at end	vall						.,

ANCHO	OR BOLT S	UMM	ARY	
QTY	LOCATE	DIA (in)	TYPE	
O 8 Ø 12	JAMB ENDWALL	5/8" 3/4"	A307 A307	
O 6 Ø 24 Ø 16	ENDWALL FRAME PORTAL COL	3/4" 3/4"	A307 A307 A307	





REVIEWED

By Richard T Smith at 11:56 am, Jun 12, 2024

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END'	WALL	COLUM	1N:	BASIC	COLUMN REA	CTIONS (k )							
Frm Line 2 2 2 2	Col Line G E C A	Dead Vert 0.5 1.0 2.9 2.5	Collat Vert 0.3 0.6 0.7 0.4	Live Vert 2.9 6.0 6.9 4.0	FLOOR Live Vert 0.0 0.0 5.8 5.8	Snow Vert 3.1 6.3 7.3 4.2	Snow Drift Vert 0.0 0.0 1.4 1.5	Wind_ Horz 0.0 -2.0 0.0 0.0	Left1 Vert -3.9 -9.9 -6.9 -4.3	Wind_ Horz 0.0 0.0 4.2 0.0	Right1 Vert -2.2 0.7 -10.3 -4.3	Wind_ Horz 0.0 -2.1 0.0 0.0	Left2 Vert -2.9 -7.6 -4.4 -3.6
Frm Line 2 2 2 2	Col Line G E C A	Wind_Rid Horz 0.0 0.0 3.8 0.0	Vert -1.2 2.6 -7.5	Wind Press Horz -1.8 -3.8 -4.3 -2.5	Wind Suct Horz 2.1 4.2 4.7 2.9	Wind_Long1 Horz Vert 0.0 -3.1 0.0 -5.7 1.7 -10.9 0.0 -4.1	t Horz 0.0	Long2 Vert -1.9 -3.5 -5.5 -1.8	Seis_ Horz 0.0 -0.5 -0.1	Left Vert 0.0 -0.5 0.5 0.0	Seis_F Horz 0.0 0.0 0.5 0.1	Right Vert 0.0 0.6 -0.5 0.0	Seis Long Vert 0.0 0.0 0.0
Frm Line 2 2 2 2 2	Col Line G E C A	-MIN_S Horz 0.0 0.0 0.0 0.0	NOW Vert 2.9 6.0 6.0 3.0	0.0 0.0 0.0 –	_1- E1P/ Vert Horz 1.3 0.0 1.8 0.0 0.3 0.0 0.0 0.0	AT_SL_2- vert 0.0 -0.3 1.8 1.3	0.0 1 0.0 3 0.0 1	/ert .2 3.4 .4	E1PAT_SL_ Horz Ve 0.0 -0 0.0 1. 0.0 3. 0.0 1.	ert .1 3 .4			
Frm Line 5	Col Line B F	Dead Vert 0.3 0.2	Wind Press Horz -1.3 -1.0	Wind Suct Horz 1.4 1.1	Seis Long Vert 0.0 0.0								
END'	WALL	COLUM	1N:	MAXIMU	JM REACTIONS	S, ANCHOR BO	LTS, & BAS	E PLATES	5				

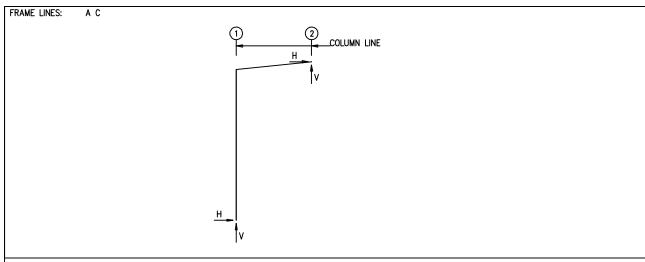
Frm Line	Col Line	Load Id	Coli Hmax H	ımn_Reac V Vmax	tions(k Load Id	) Hmin H	V Vmin	Bol QTY	t(in) DIA	Base Width	:_Plate(in) Length	Thick	Grout (in)
		- <u>-</u> 7	1.2	-2.1	- <del>-</del>	-1.1	-1.6		0.625	3.500	8.000	0.250	0.0
2	E	2	0.0 2.5	3.8 -5.3	7 8	1.2 -2.3	-2.1 -2.8	4	0.750	6.000	8.000	0.375	0.0
		9	0.0	8.2	7	2.5	-5.3	4					
2	С	10 11	2.8 0.0	-4.8 14.5	8 10	-2.6 2.8	-4.8 -4.8	4	0.750	6.000	8.000	0.375	0.0
2	Α	7 11	1.8 0.0	-1.1 11.5	8 7	-1.5 1.8	−1.0 −1.1	4	0.750	6.000	8.000	0.375	0.0
5	В	12 14	0.8 0.0	0.2 0.3	13	-0.8	0.2	2	0.625	3.500	8.000	0.250	0.0
5	F	12 15	0.7 0.5	0.1 0.2	13	-0.6	0.1	2	0.625	3.500	8.000	0.250	0.0

CUSTOMER NAME: AMER TUMEH	<b>←</b>	ISSNE	DATE DWN. CHK. ENG.	N	Ĭ.	ENG.
PROJECT NAME: AMER TUMEH		APPROVAL	05/31/24 MEZ MEZ	EZ N	EZ	RTS
		PERMIT	06/07/24	SB	CAF	RTS
PROJECT COUNTY: SAN JUAN						
PROJECT END USE: AVIATION						
CUSTOMER PHONE NUMBER: 435.260.2889						
CUSTOMER EMAIL: AMER_TUMEH@YAHOO.COM						
SCALE: N. T. S.						
	7// 702 1011					
	I OND AVE					
	GRAND JUNCTION, CO 81501					
SHEET NUMBER:	PHONE: (800)-497-2135					
6 OF 32	WWW CREATWESTERNBILLIONES COM					
	WWW.GNEALWESTERNBOILDINGS.COM					
94918						
SHETTITE ANCHOR BOLT BEACTIONS FOR 0.1018A	IONS FOR 9/9/8/					



By Richard T Smith at 11:56 am, Jun 12, 2024

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RIGID F	IGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES												
Frm Line	Col Line	Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	Bolt QTY	t(in) DIA	Base Width	e_Plate(in) Length	Thick	Grout (in)
A	1	3 4	0.5 -0.1	1.1 8.1	6	-0.5	1.2	4	0.750	6.000	8.000	0.375	0.0
A	2	8 1	1.1 -0.1	-0.4 2.8	2 5	-0.6 0.3	0.0 -0.7						

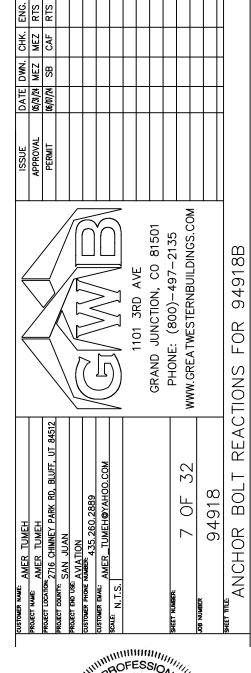
RIGID	IGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES												
Frm Line		Load Id	Hmax H	umn_Reac V Vmax	tions(k Load Id	Hmin H	V Vmin	— Bolt(in) Base_Plate(in) QTY DIA Width Length			Thick	Grout (in)	
С	1	7 4	0.5 0.0	0.3 7.6	6	-0.5	1.1	4	0.750	6.000	8.000	0.375	0.0
С	2	8 1	1.0 -0.1	-0.4 2.7	2 5	-0.5 0.3	0.0 -0.7						

NOTES	FOR REACTIONS	
	ding reactions are based on following building data: Width (ft) Length (ft) Eave Height (ft) Roof Slope (rise/12) Dead Load (psf) Collateral Load (psf) Live Load (psf) Snow Load (psf) Ultimate Wind Speed (mph) Wind Code Exposure Closed/Open Importance Wind Importance Seismic Seismic Cone Seismic Coef (Fa*Ss)	= 10.00 = 20.33 = 20.00/21.00 = 1.25:12/ = 2.42 = 2.00 = 20.00 = 21.00 = 115.00 = 18C-18 = C = Partially Enclosed = 1.00 = 1.00 = 1.00 = 1.00 = 0.30
ID	Description	
1 2 3 4 5 6 7 8	Dead+Collateral+Snow+Snow_Drift Dead+0.6Wind_Left2 Dead+0.6Wind_Long1R Dead+Collateral+0.75Snow+0.45Wind_ 0.6Dead+0.6Wind_Left1 0.6Dead+0.6Wind_Left2 0.6Dead+0.6Wind_Long1R 0.6Dead+0.6Wind_Long2R	.Right2+0.75Snow_Drift+0.75Floor_Live

BUILDING BRACING REACTIONS										
Wall Col	Note									
L_EW A F_SW 2 R_EW C B_SW 1 Torsional Bracing Used	(h) (e) (h)									
(e)Bracing loads must be applied to supporting building (h)Rigid frame at endwall										
Reactions for seismic represent shear force, Eh Reaction values shown are unfactored										

ANCH(	OR BOLT S	SUMMA	ARY	
" " " " " " " " " " " " " " " " " " "		30111117		
		ĎΙĄ	T. 65	
QTY	LOCATE	(in)	TYPE	_
<b>⊠</b> 8	FRAME	3/4"	A307	

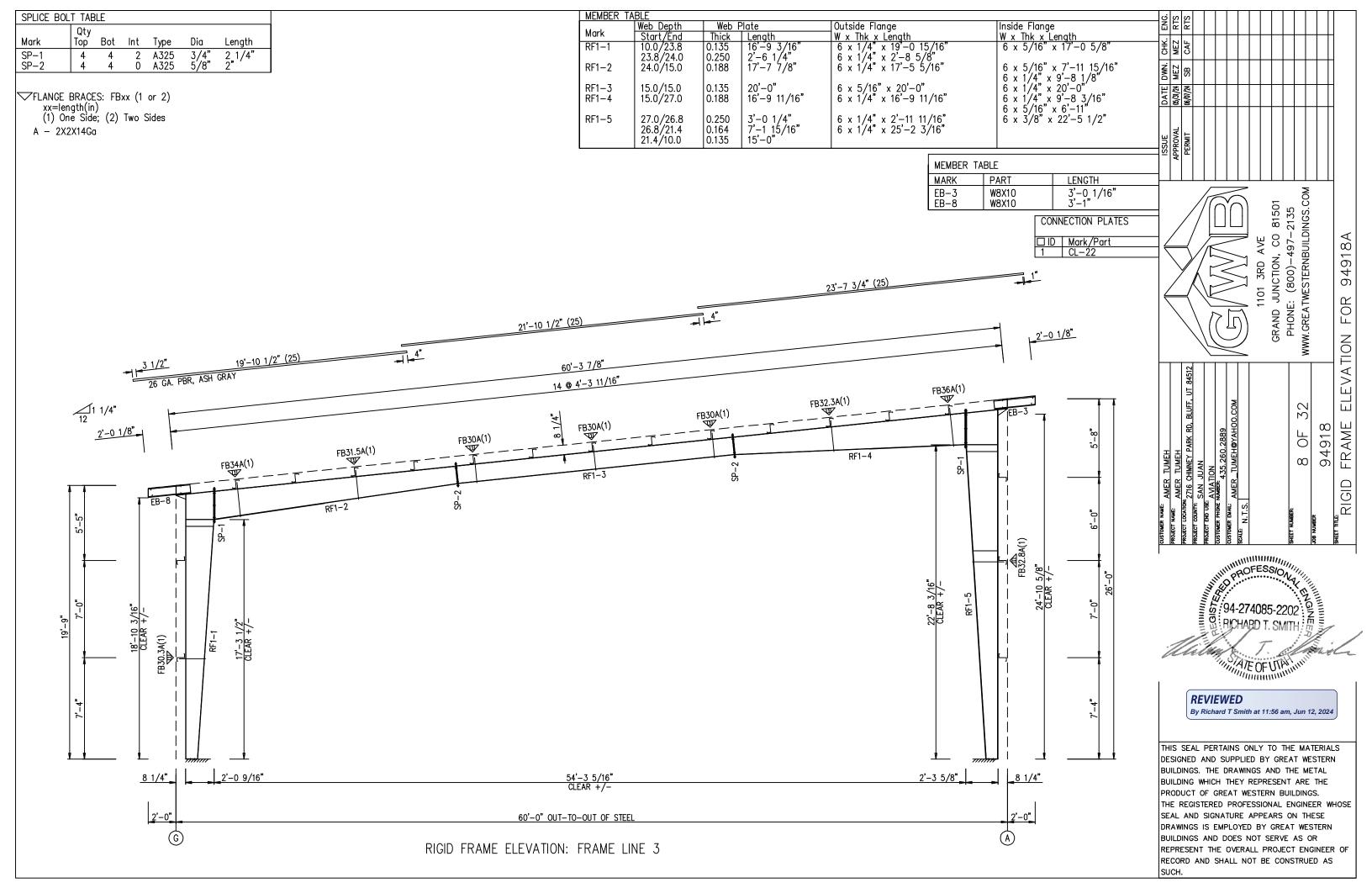
RIGIE	FRAN	ИЕ:	BASI	C COLUM	IN REACT	10NS (k )	)						
FRAME Line A A	Column Line 1 2	Horz 0.0 0.0	Dead Vert 2.0 0.2	Collo Horz 0.0 0.0	ateral— Vert 0.1 0.1	Horz 0.0 0.0	-Live Vert 1.1 1.0	F Horz 0.1 -0.1	loor Vert 5.8 0.0	Horz 0.0 0.0	-Snow Vert 1.1 1.1	Sno Horz 0.0 0.0	w_Drift- Vert 0.8 1.5
FRAME Line A A	Column Line 1 2	Wind Horz 0.1 0.5	_Left1- Vert -1.5 -1.3	-Wind_ Horz 0.8 1.1	Right1- Vert -1.2 -0.9	Wind Horz -0.8 -0.9	Left2- Vert -0.1 -0.3	-Wind_ Horz -0.2 -0.3	Right2- Vert 0.2 0.2	Wind Horz 0.9 0.8	l_Long1- Vert -1.6 -1.3	Wind Horz 0.9 1.8	i_Long2- Vert -1.2 -0.9
FRAME Line A A	Column Line 1 2	-Seismi Horz 0.0 -0.1	c_Left Vert 0.0 0.0	Seismic Horz 0.0 0.1	:_Right Vert 0.0 0.0	-Seism Horz 0.0 0.0	ic_Long Vert -0.1 0.0	-MIN_S Horz 0.0 0.0	SNOW Vert 1.1 1.0				
FRAME Line C C	Column Line 1 2	Horz 0.0 0.0	Dead Vert 1.9 0.2		ateral— Vert 0.1 0.1	 Horz 0.0 0.0	-Live Vert 1.0 0.9	F Horz 0.1 -0.1	Floor Vert 5.5 0.0	Horz 0.0 0.0	-Snow Vert 1.1 1.0	Sno Horz 0.0 0.0	w_Drift- Vert 0.7 1.4
FRAME Line C C	Column Line 1 2	Wind Horz 0.1 0.5	_Left1- Vert -1.4 -1.2	-Wind_ Horz 0.7 1.0	Right1- Vert -1.1 -0.9	Wind Horz -0.8 -0.9	_Left2- Vert -0.1 -0.2	-Wind_ Horz -0.2 -0.3	Right2- Vert 0.2 0.1	Wind Horz 0.8 0.7	l_Long1- Vert -1.5 -1.2	Wind Horz 0.8 1.7	i_Long2- Vert -1.2 -0.8
FRAME Line C C	Column Line 1 2	-Seismi Horz 0.0 -0.1	c_Left Vert 0.0 0.0	Seismic Horz 0.0 0.1	:_Right Vert 0.0 0.0	-Seism Horz 0.0 0.0	ic_Long Vert -0.1 0.0	-MIN_S Horz 0.0 0.0	SNOW Vert 1.0 0.9				

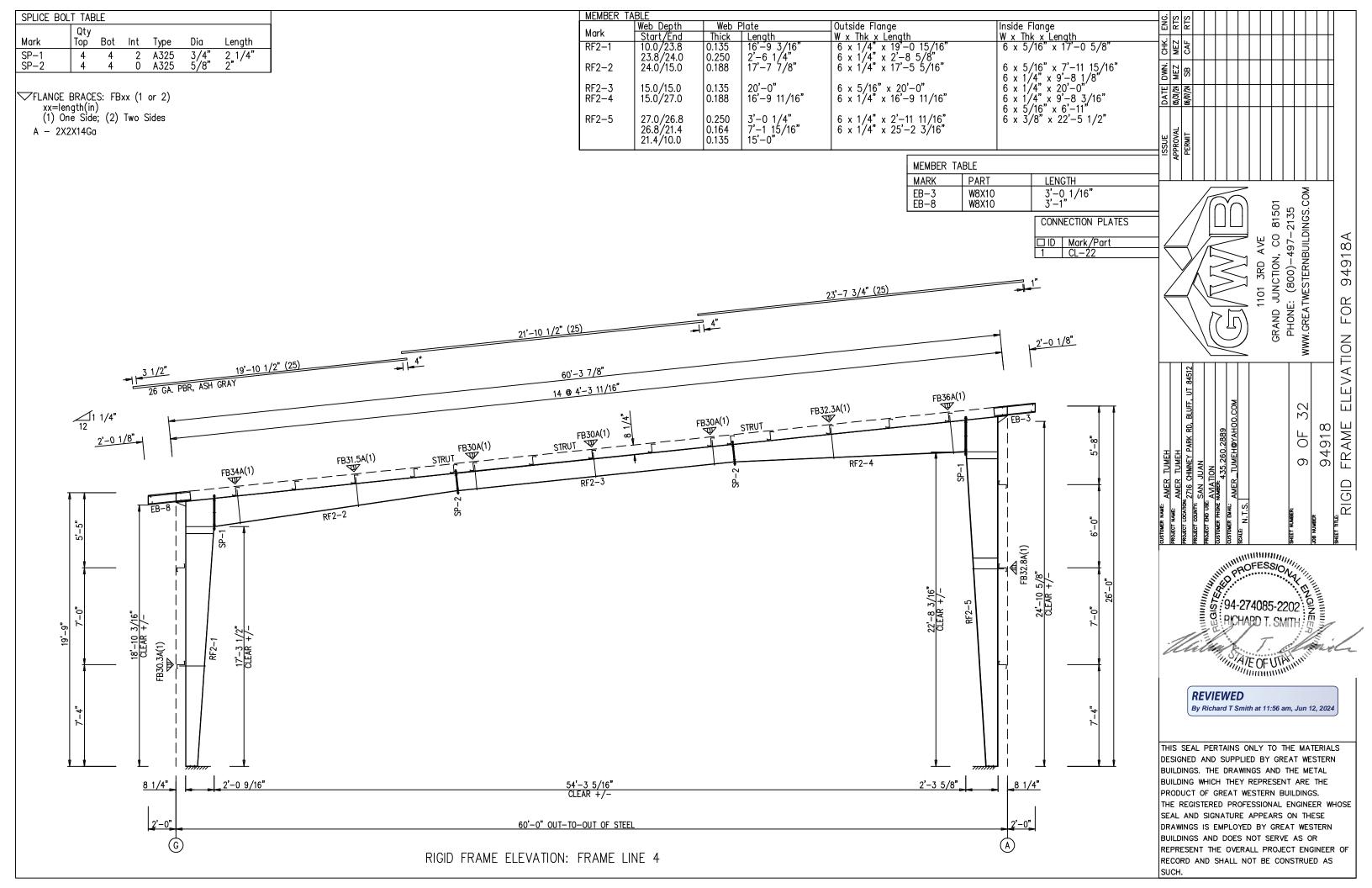


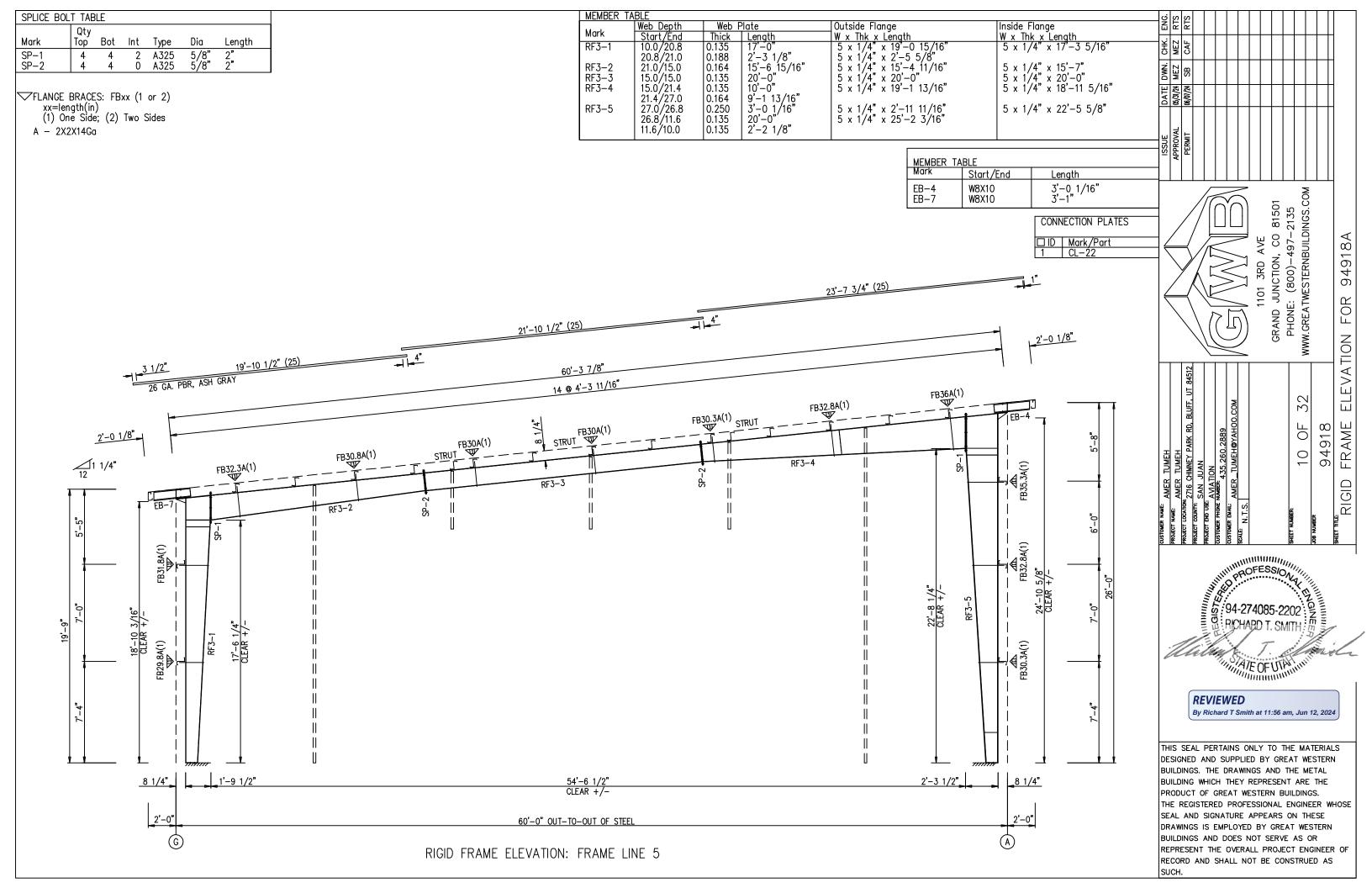


By Richard T Smith at 11:56 am, Jun 12, 2024

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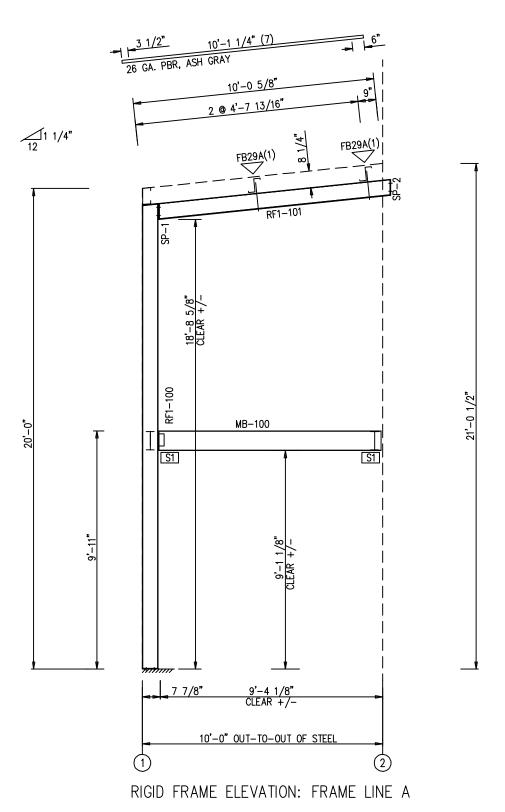




SPLICE BOL	T TAB	LE				
Mark	Qty Top	Bot	Int	Туре	Dia	Length
SP-1 SP-2	4 4	0 0	0 0	A325 A325	5/8" 5/8"	1 3/4" 1 1/2"

SUPPO	ORT BE	AM BOLT	TABLE	
	Qty	Туре	Dia	Length
S1	2	A325	1/2"	1 1/2"

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A - 2X2X14Ga



ISSUE DATE DWN. CHK. ENG.	APPROVAL  05/31/24   MEZ   MEZ   RTS	PERMIT 06/07/24 SB CAF RTS								01		700					
<b>←</b>	<u>/</u> _\							7. CO. F.	IIOI JRD AVE	GRAND JUNCTION, CO 81501	HONE: (800)-497-2135	WWW GREATWESTERNBIII DINGS COM				SID FRAME ELEVATION FOR 94918B	
CUSTOMER NAME: AMER TUMEH	PROJECT NAME: AMER TUMEH	ا ښا		PROJECT END USE: AVIATION	CUSTOMER PHONE NUMBER: 435,260,2889	CUSTOWER EMAIL: AMER_TUMEH@YAHOO.COM	SCALE: N.T.S.				SHEET NUMBER:	11 OF 32	JOB NUMBER	94918	SHEET TITE:	$\tilde{\Xi}$	
Ä			Тинининий в	PEGISTEDIII	9 R	4-12-12-1-13-1-13-1-13-1-13-1-13-1-13-1-	27-HAF	400 D	SS T. I	22 SM	202 ITh	The state of the s	O Z TO			/	/

MEMBER SIZE TABLE

BEAM TABLE

Mark Part

MB-100 W10X12

MARK

RF1-101

MEMBER

W8X10

W8X10

LENGTH 19'-3 11/16 9'-8 9/16"

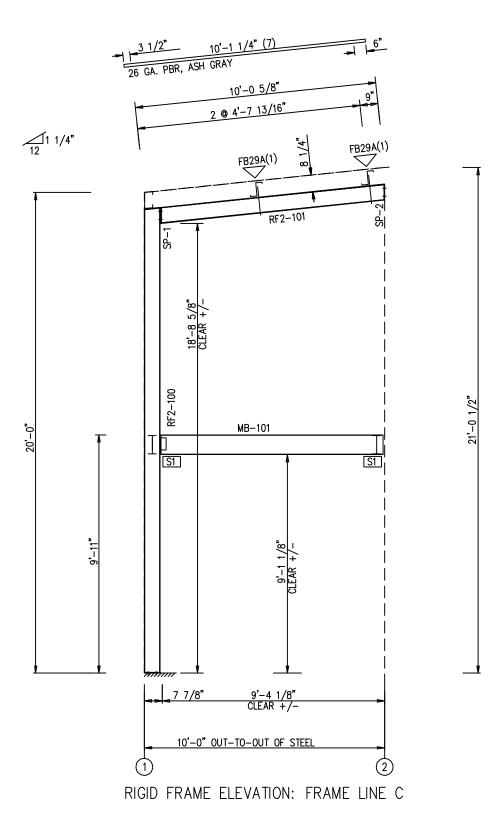
## REVIEWED By Richard T Smith at 11:56 am, Jun 12, 2024

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SPLICE BOL	SPLICE BOLT TABLE												
Mark	Qty Top	Bot	Int	Туре	Dia	Length							
SP-1 SP-2	4 4	0 0	0 0	A325 A325	5/8" 5/8"	1 3/4" 1 1/2"							

SUPPO	ort be	AM BOLT	TABLE		
	Qty	Туре	Dia	Length	
S1	2	A325	1/2"	1 1/2"	

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A - 2X2X14Ga



	Length 9'-2 7/8"	9′-4 9/16″	LENGTH 19'-3 11/16"	LENCTH
ISSNE	DATE	¥.		ENG.
APPROVAL	05/31/24 M	_	MEZ	RTS
PERMIT	S 47/10/90	Н	CAF	RTS
		$\dagger\dagger$	$\prod$	
	_			
	ISSUE APPROVAL PERMIT			19'-3 11/16"

MEMBER SIZE TABLE

MARK RF2-100

RF2-101

 BEAM TABLE

 Mark
 Part

 MB-101
 W10X12

**MEMBER** 

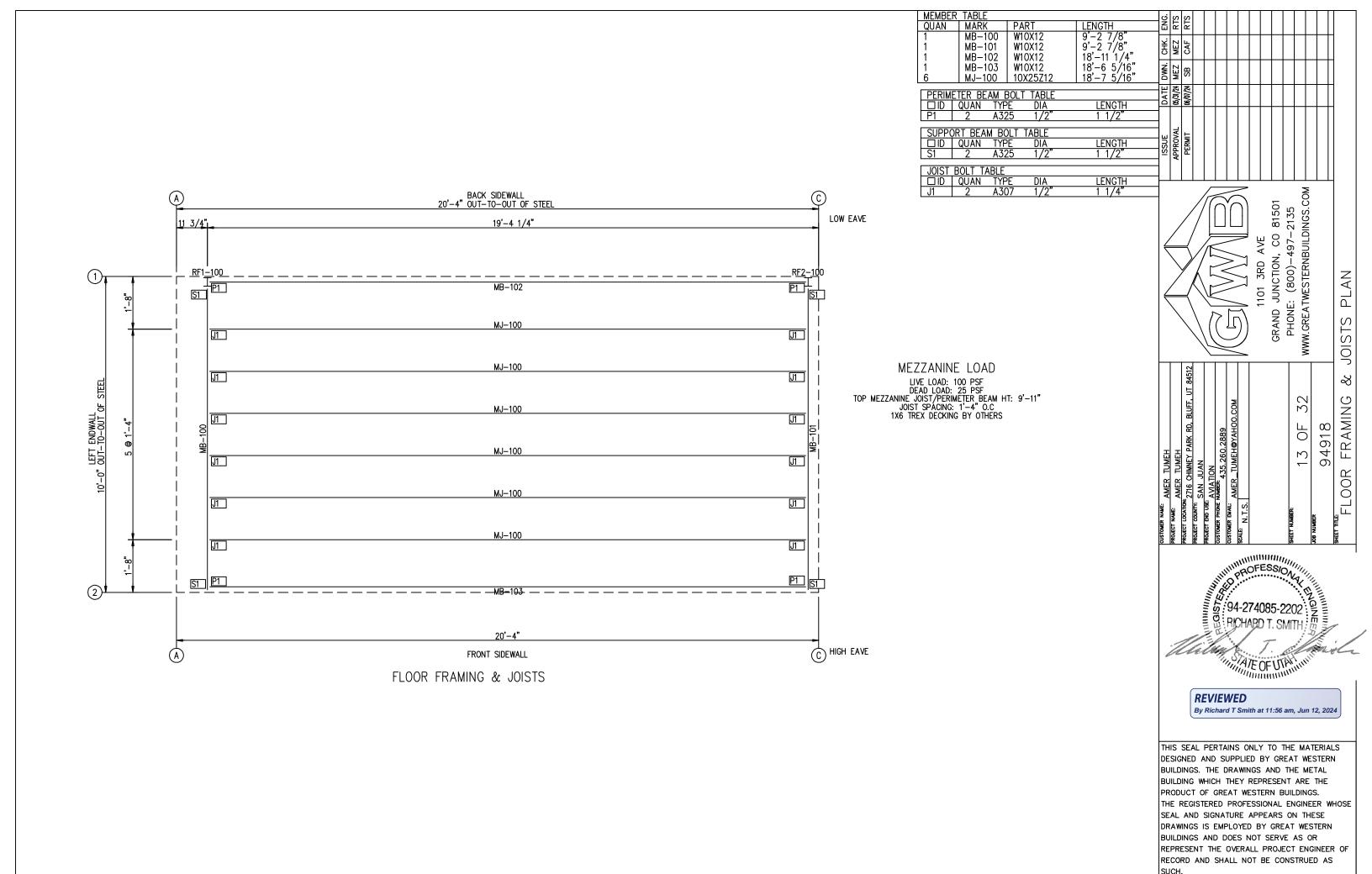
W8X10

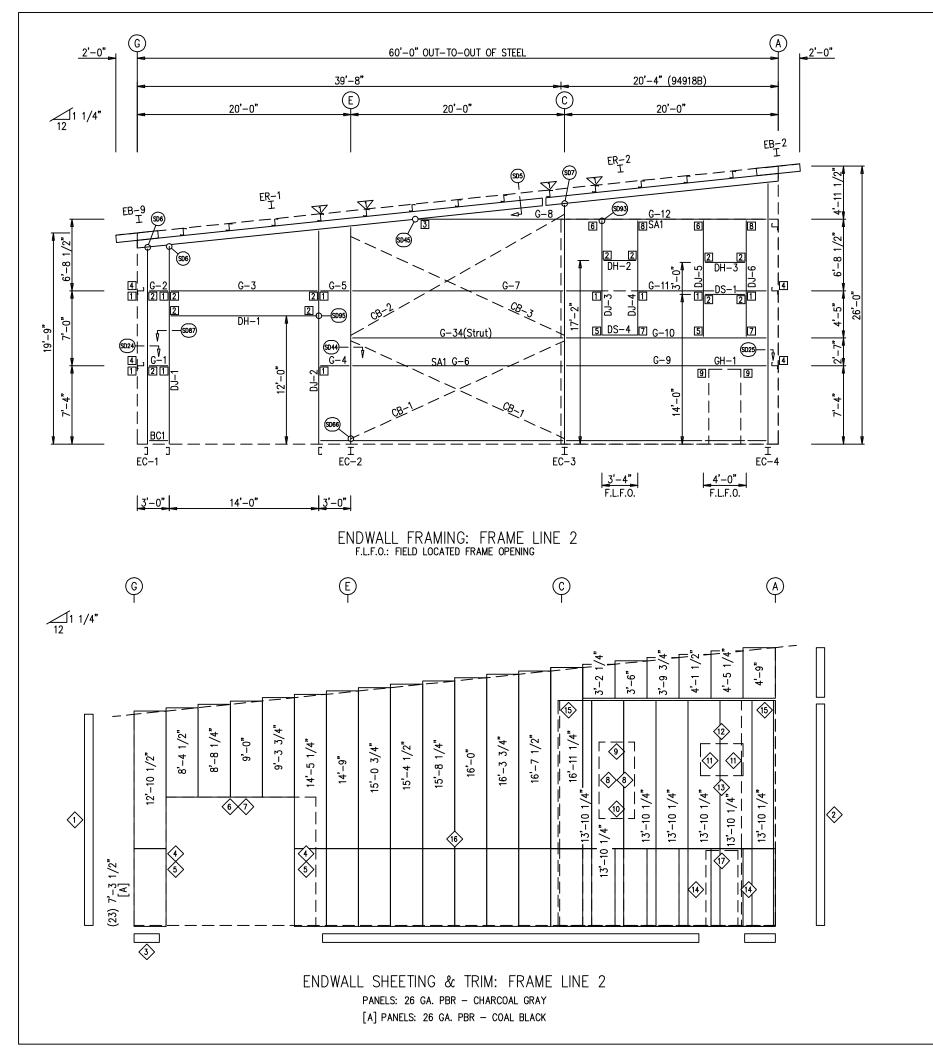
W8X10



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GRAND JUNCTION, CO 81501 PHONE: (800)-497-2135 WWW.GREATWESTERNBUILDINGS.COM BOLT TABLE FRAME LINE 2 AVE 3RD LOCATION TYPE LENGTH QUAN DIA ER-1/ER-2 COLUMNS/RAFTER A325 A325 A325 5/8" 5/8" 5/8" 1 1/2" 1 1/2" 1 1/2" 1101 JAMBS/RÁFTER SHEETING MEMBER TABLE FRAME LINE 2 QUAN MARK PART LENGTH LENGTH

3'-0 1/16"
3'-1"
18'-5 1/4"
20'-5"
22'-6"
24'-5 13/16"
38'-1 1/2"
22'-1 15/16"
18'-7 3/4"
20'-1 1/4"
10'-5 1/2"
10'-5 1/2"
10'-5 1/2"
10'-5 1/2"
13'-11 1/2"
3'-3 1/2"
3'-11 1/2"
3'-11 1/2"
3'-11 1/2" W8X10 W8X10 EB-2 EB-9 EC-1 EC-2 EC-3 EC-4 ER-1 DJ-3 DJ-4 DJ-5 DJ-6 DH-1 DH-3 DS-1 DS-1 G-7 G-7 G-7 G-7 G-11 G-12 G-3 CB-1 CB-3 8x25C12 W8X10 ઝ W8X13 W8X13 FRAMING W8X10 W8X10 94918 9 8X35C14 8X35C14 10x25C16 10x25C16 10x25C16 10x25C16 WALI 8x25C16 10x25C16 10x25C16 10x25C16 3'-11 1/2"
3'-4"
1'-7 15/16"
13'-11 1/2"
2'-3 7/8"
2'-3 7/8"
19'-3 13/16"
14'-0 1/16"
18'-4 1/16"
18'-4 1/16"
18'-4 1/16"
18'-4 1/16"
18'-4 1/16"
18'-4 1/16"
19'-3 13/16"
22'-4 1/2"
23'-7"
22'-7 1/2" GH-1 8X35Z16 HILLIAM PROFESSIO. PROFESSION 8X25Z16 8X25Z16 8X35Z16 8X25Z16 8X25Z16 8X35Z14 8X25Z14 94-274085-2202 8X25Z16 8X35Z16 W10X12 8X25Z16 ATE OF UTAL 8X35Z14 8X25Z16 RD0500 RD0500 **REVIEWED** By Richard T Smith at 11:56 am, Jun 12, 2024 RD0500 CONNECTION PLATES FRAME LINE 2 FLANGE BRACE TABLE FRAME LINE 2 THIS SEAL PERTAINS ONLY TO THE MATERIALS QUAN MARK ▽ID QUAN MARK ESIGNED AND SUPPLIED BY GREAT WESTERN CL-103 CL-100 4 FB28.3 10 12 BUILDINGS. THE DRAWINGS AND THE METAL BUILDING WHICH THEY REPRESENT ARE THE b1 CL-5 2 2 2 2 SEAL AND SIGNATURE APPEARS ON THESE 6 DRAWINGS IS EMPLOYED BY GREAT WESTERN f2 j2 BUILDINGS AND DOES NOT SERVE AS OR 8 REPRESENT THE OVERALL PROJECT ENGINEER OF CL-200 RECORD AND SHALL NOT BE CONSTRUED AS

TRIM TABLE FRAME LINE 2 ♦ID QUAN PART

5 6 7

8 9

2 6

FL-502 FL-502 FL-60 FL-65 FL-48 FL-55 FL-52 FL-52 FL-50 FL-48

FL-52 FL-50 FL-48 FL-22 FL-237 FL-52

**LENGTH** 

19'-9"
13'-2"
10'-2"
12'-2"
14'-7"
14'-4"
7'-4"
3'-8"
3'-0"
4'-4"
4''-4"
8"

**DETAIL** 

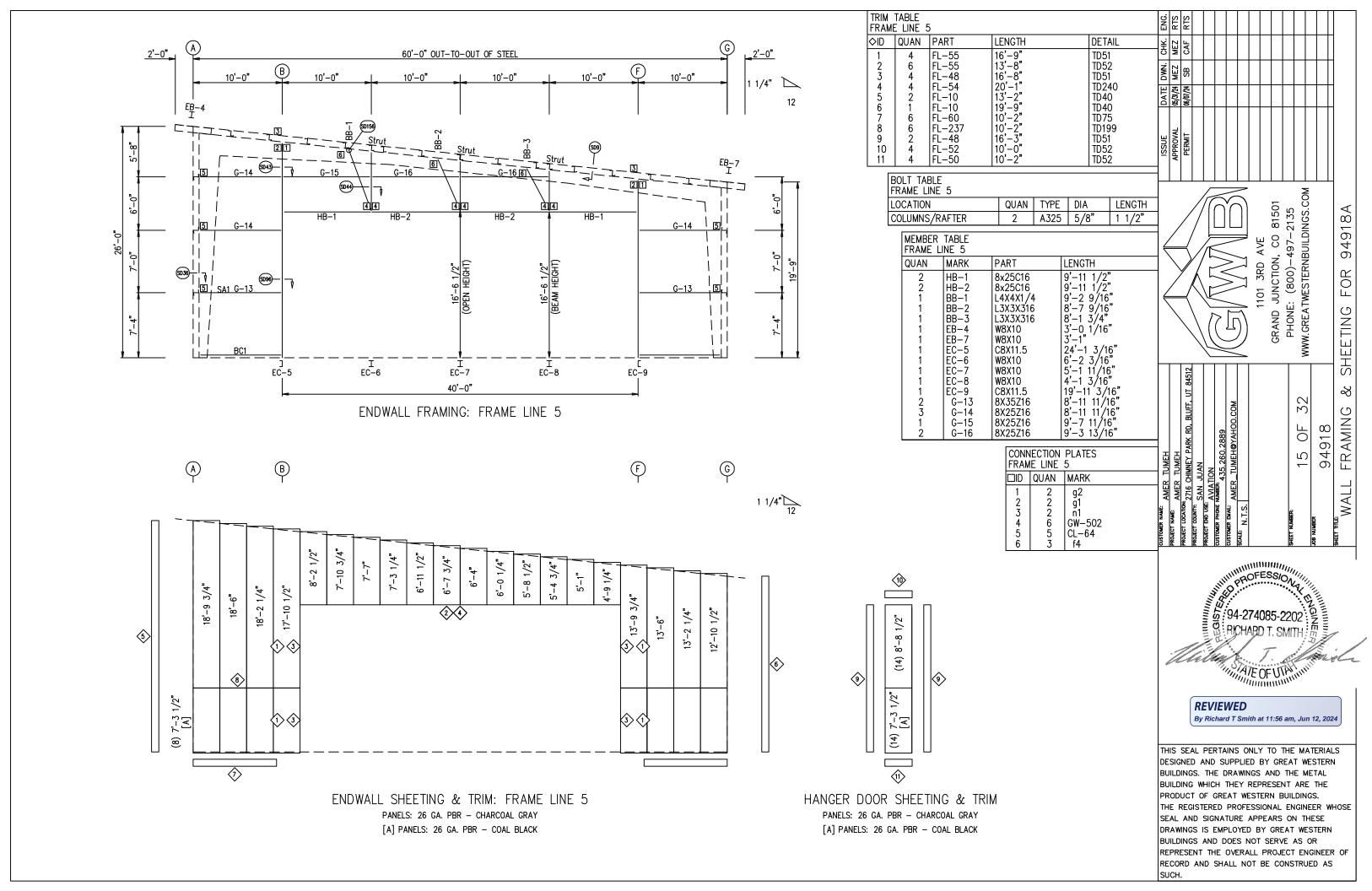
TD40 TD40 TD75

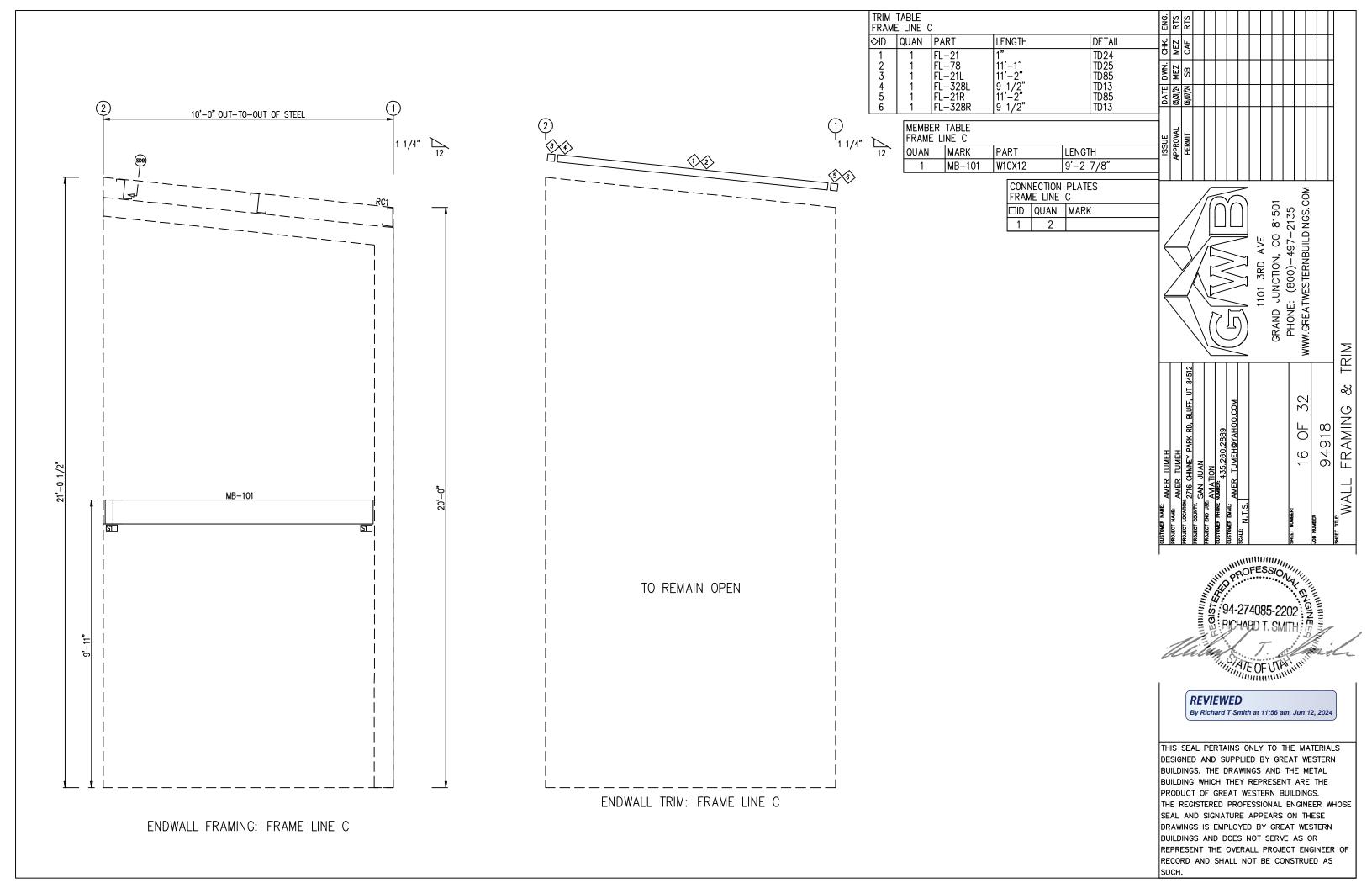
TD51 TD51

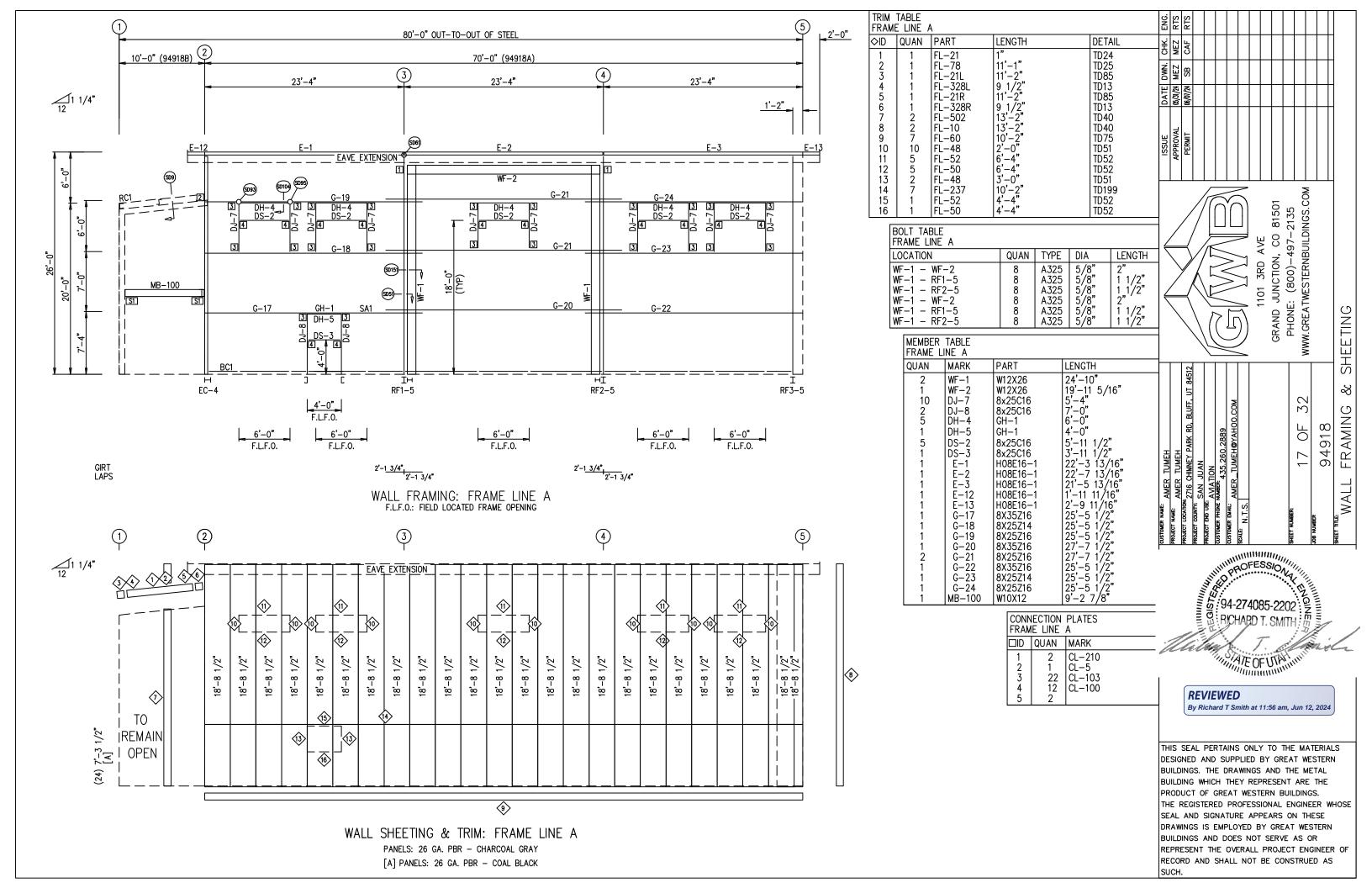
TD52 TD52 TD51 TD51 TD52 TD52 TD51

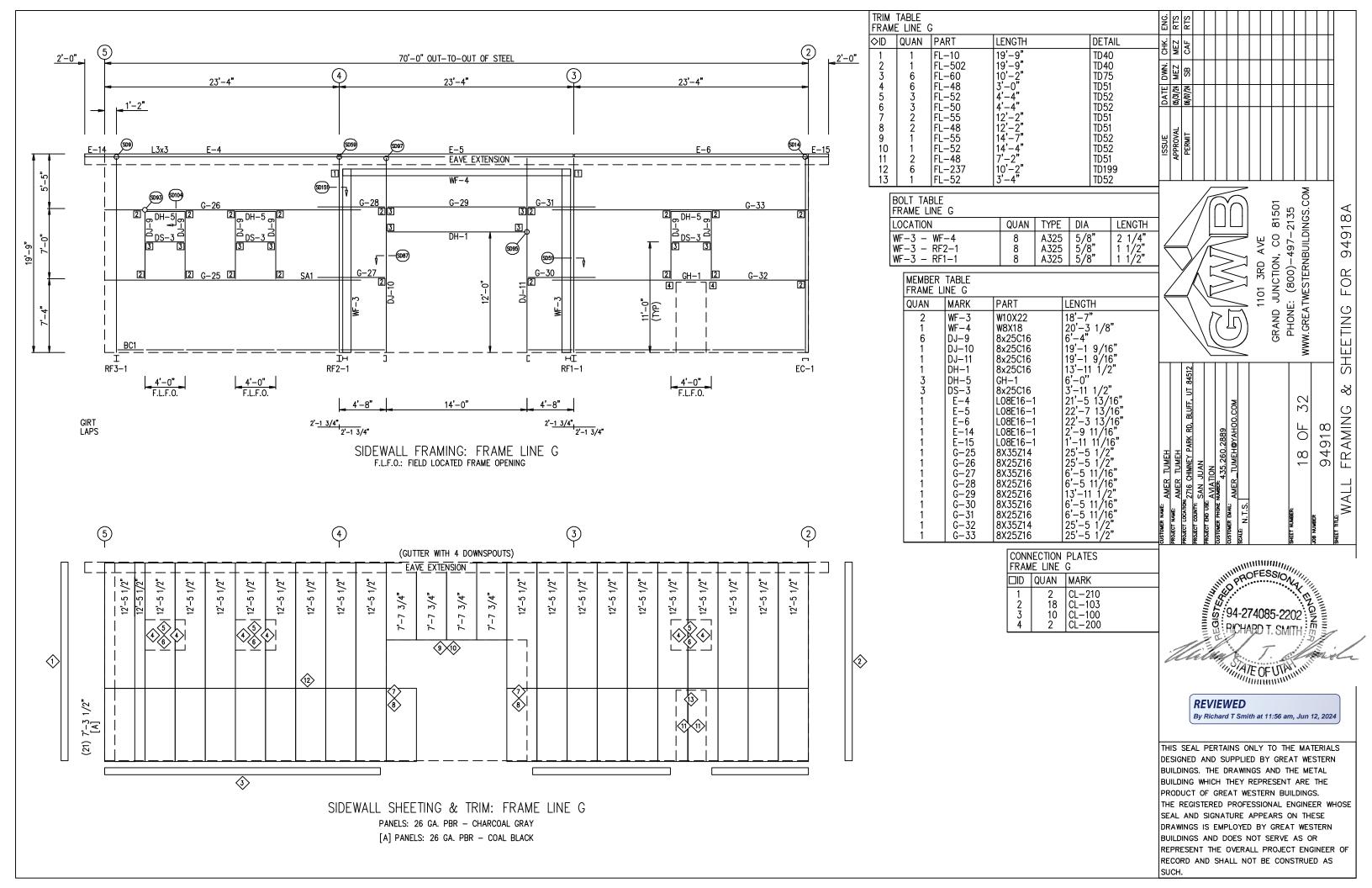
TD52 TD52 TD51

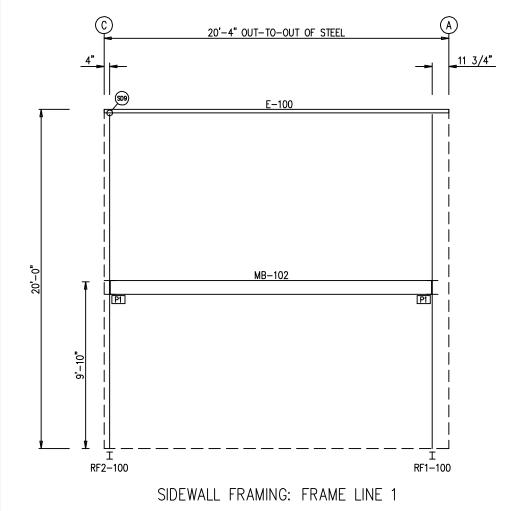
TD199 TD52



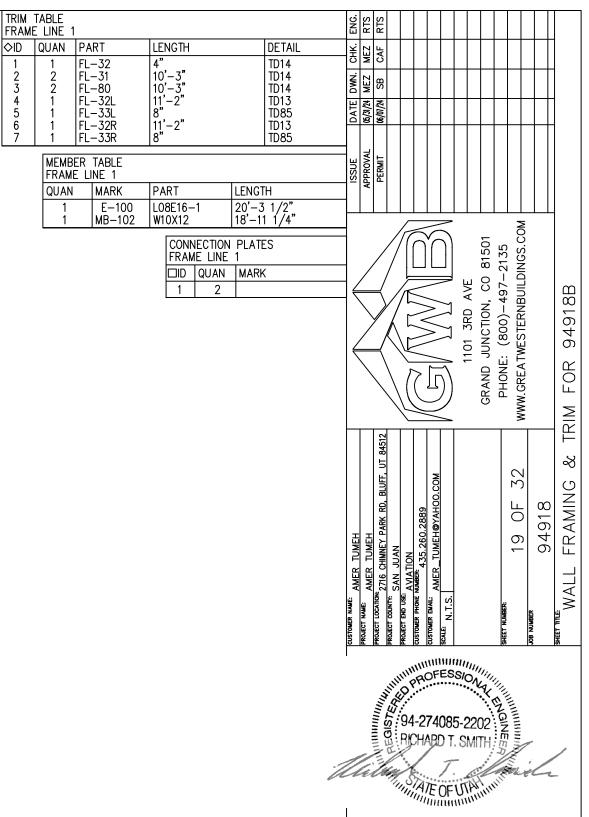








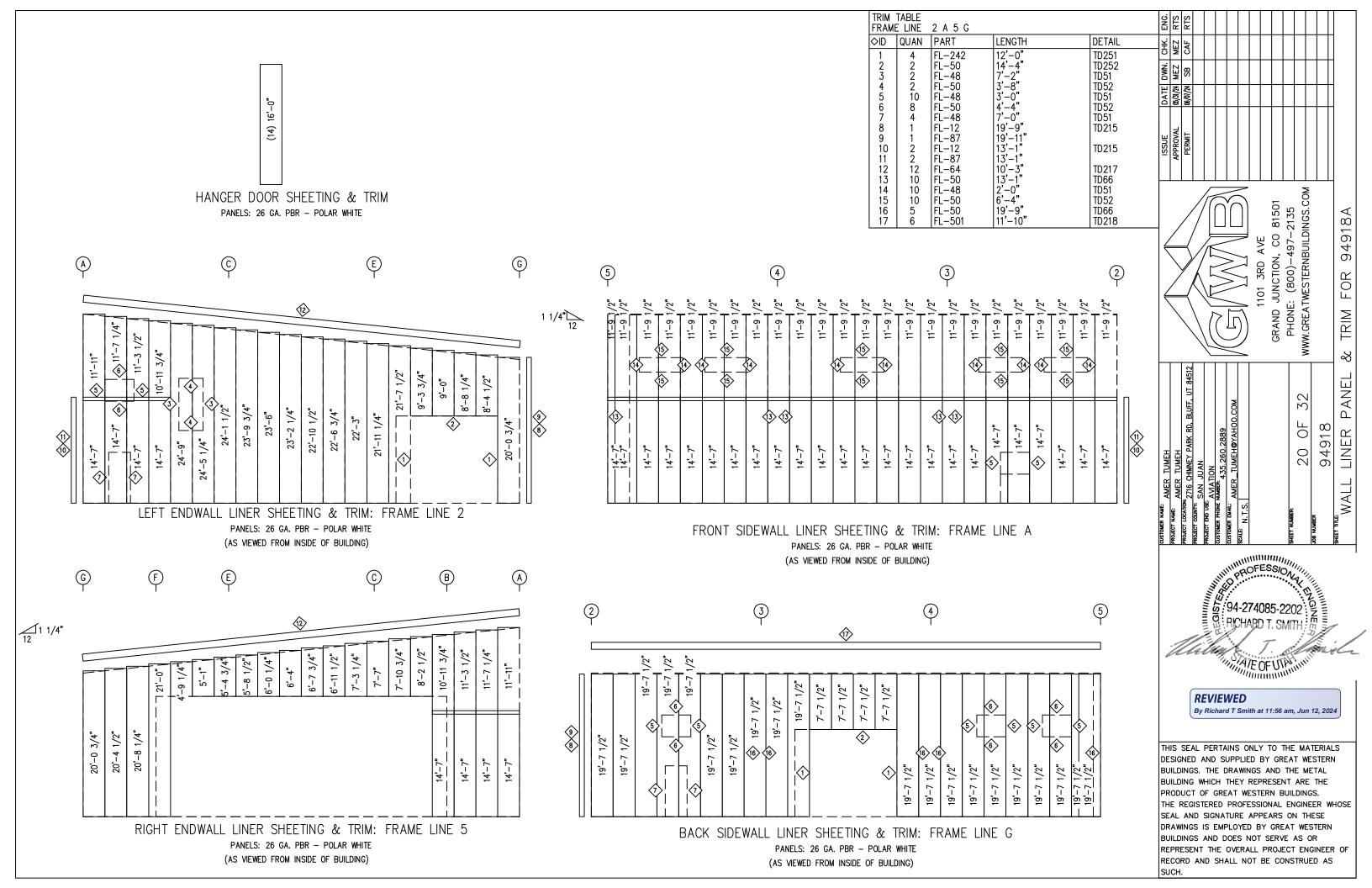


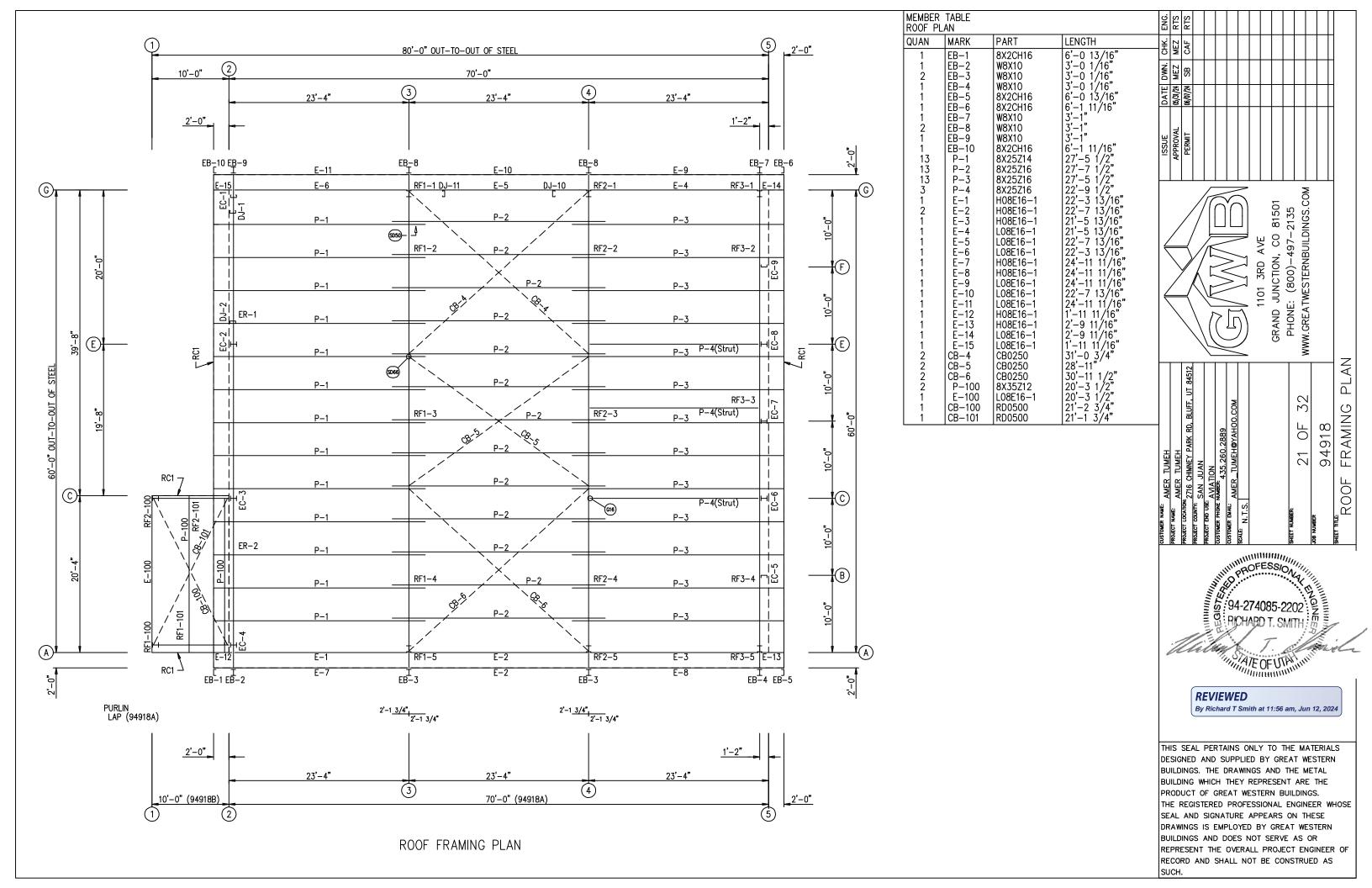


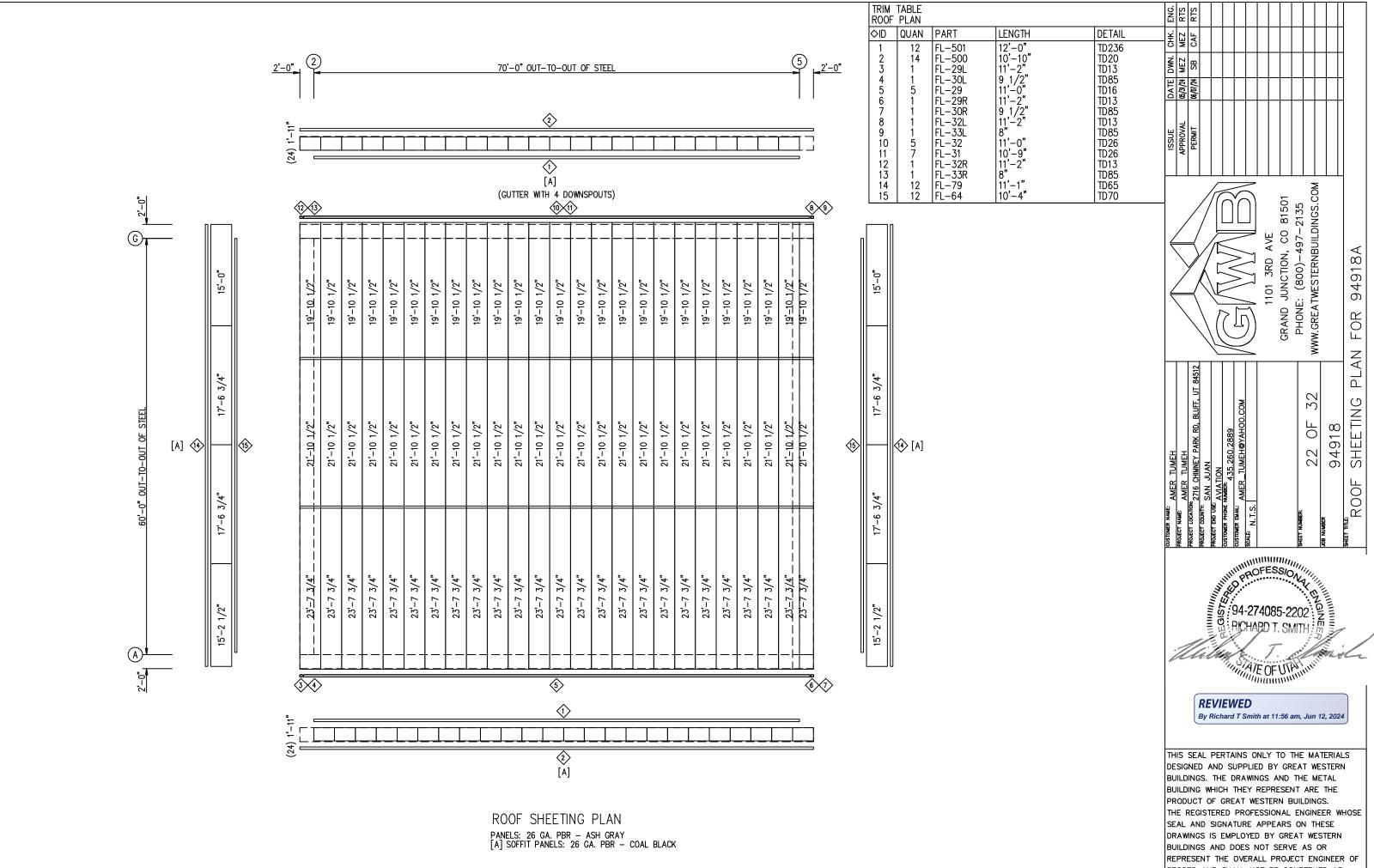
REVIEWED

By Richard T Smith at 11:56 am, Jun 12, 2024

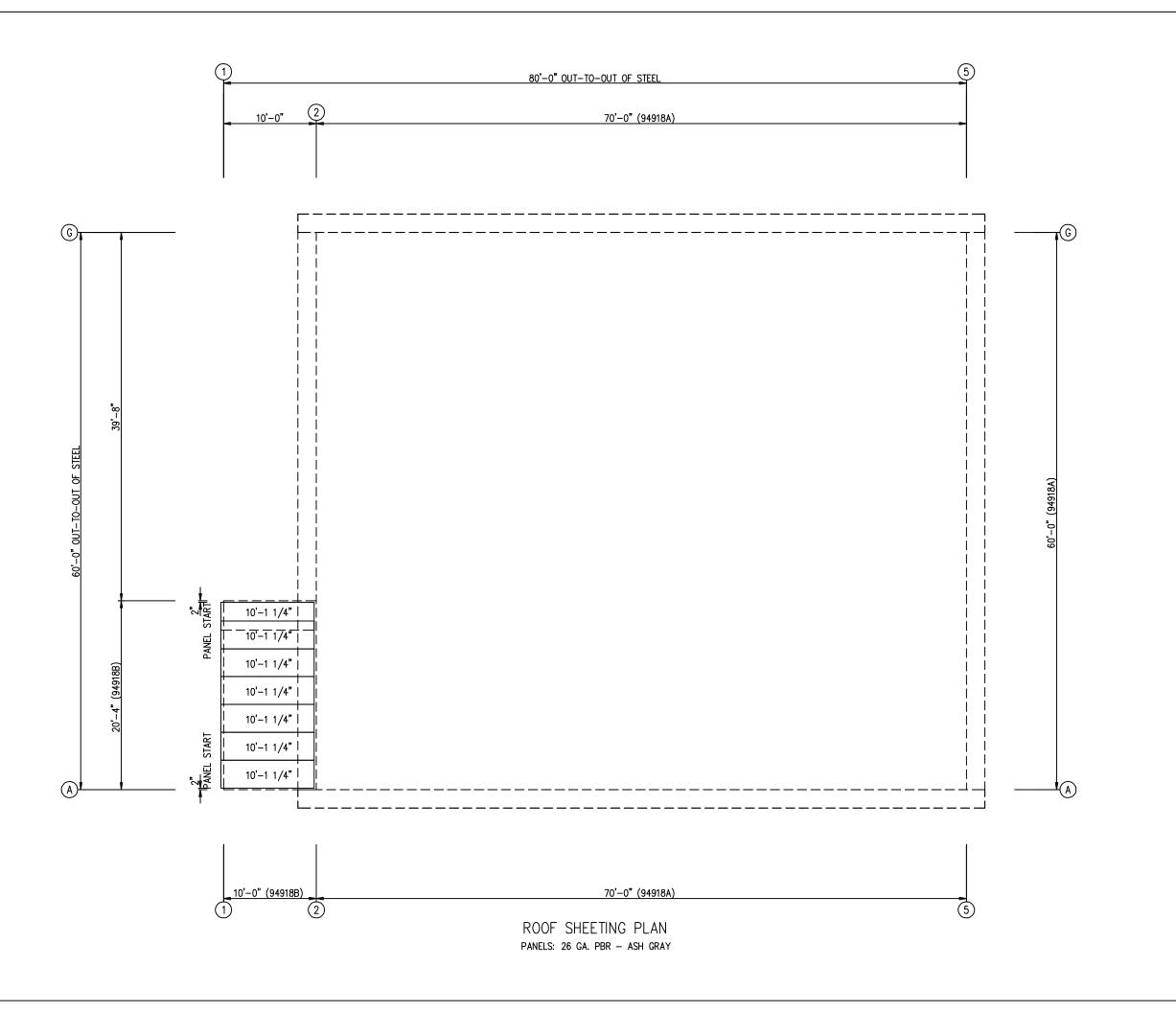
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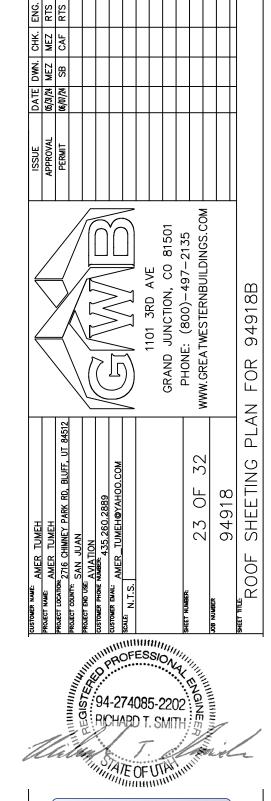






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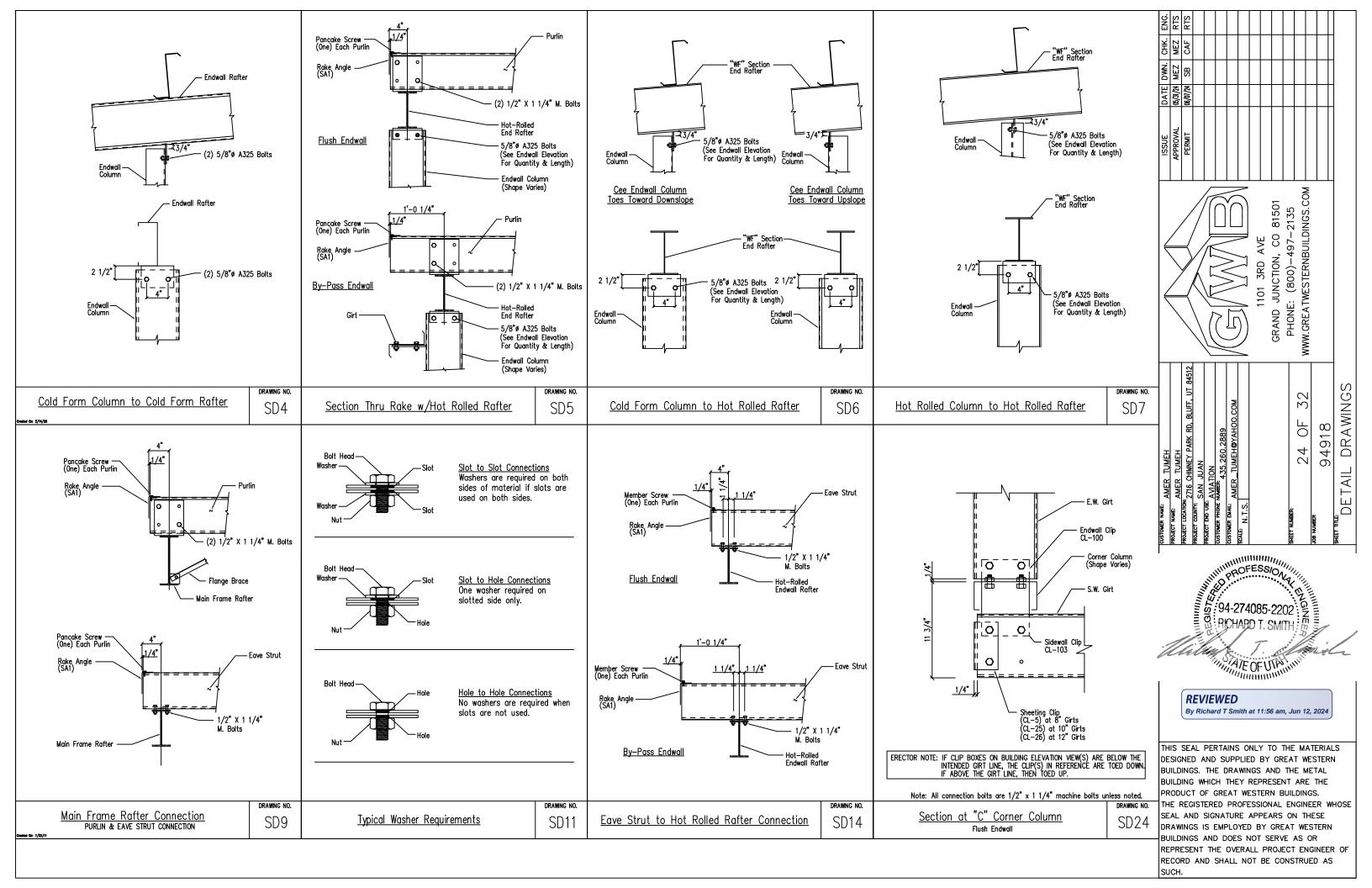


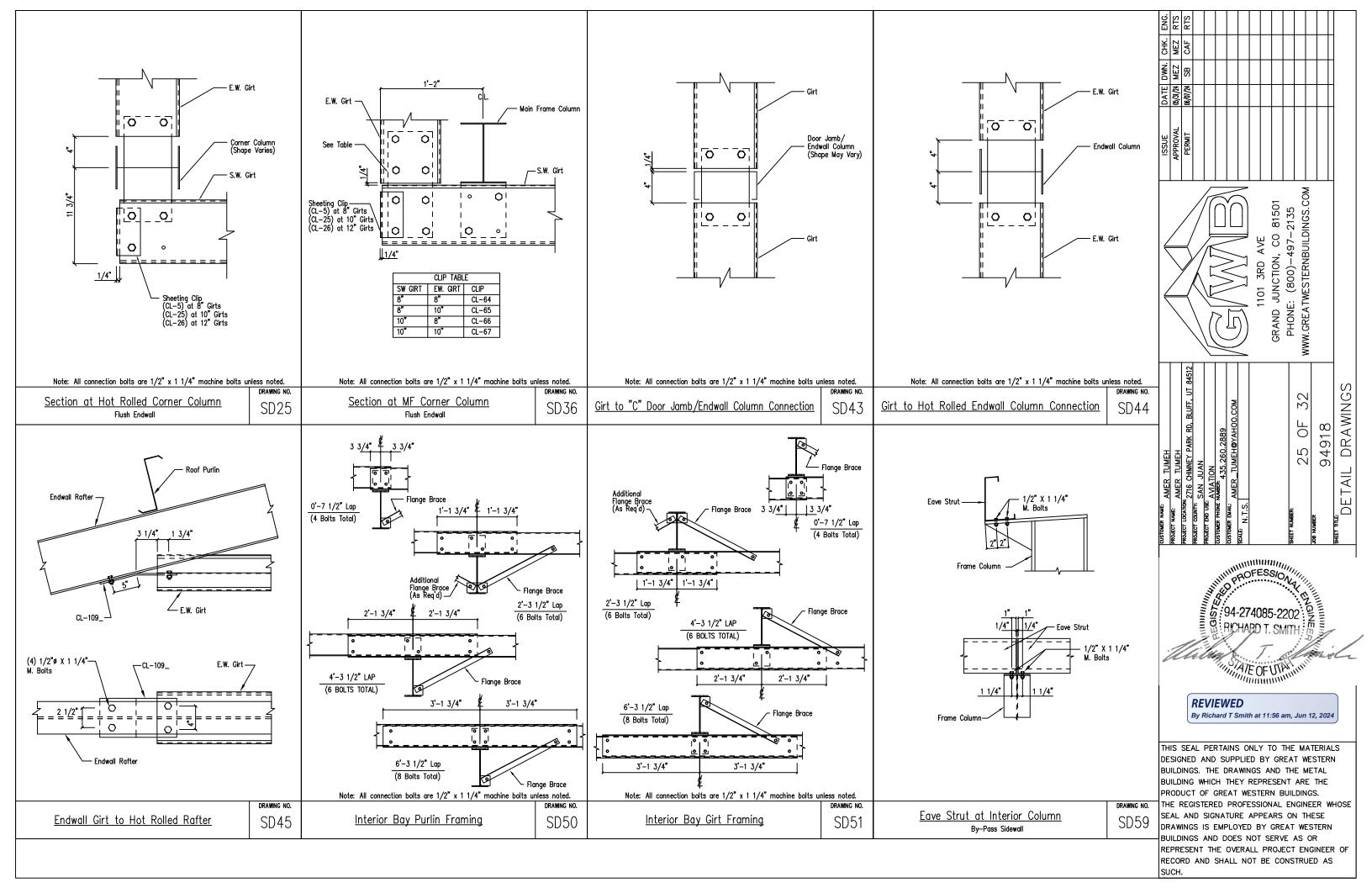


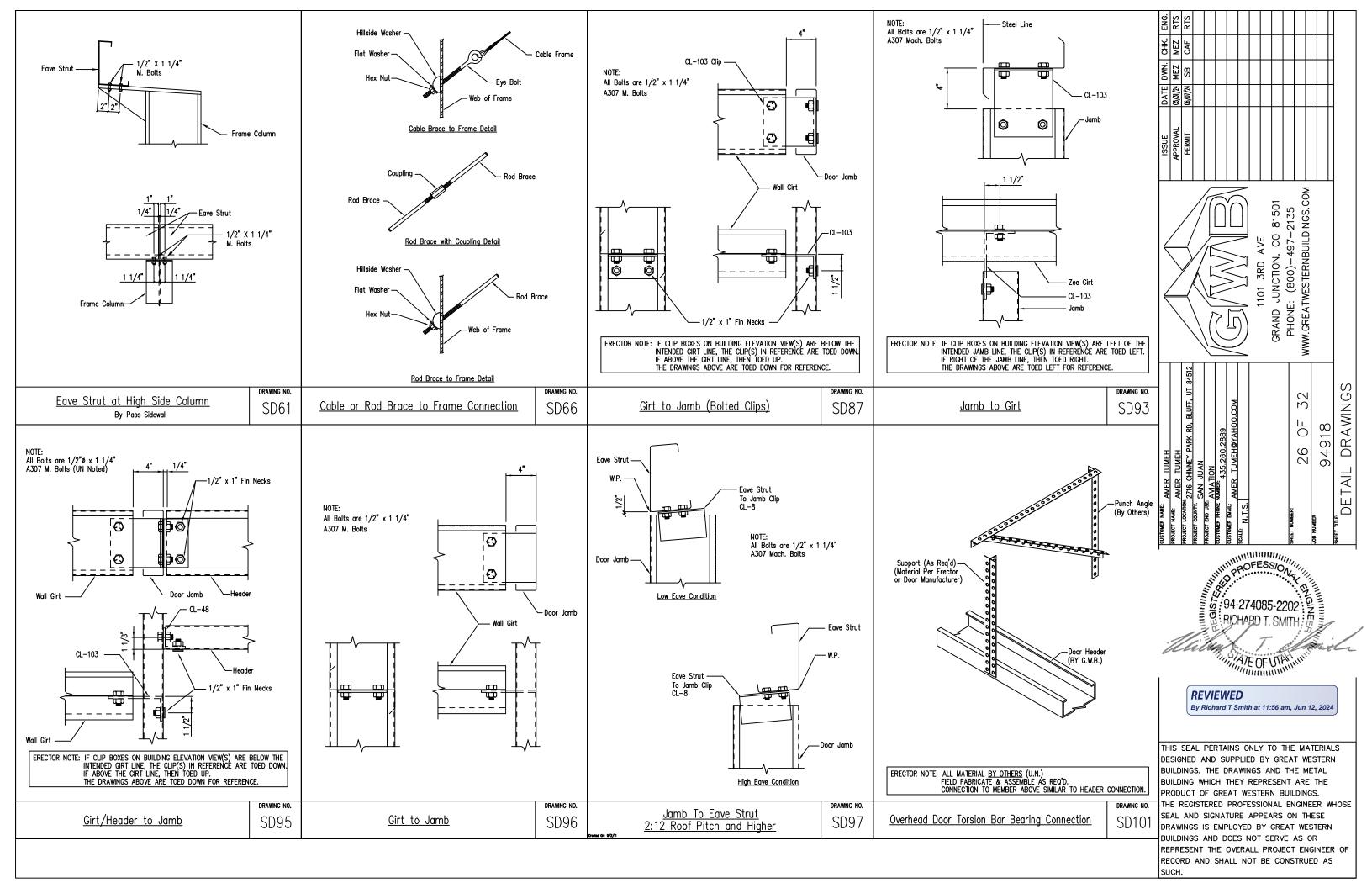
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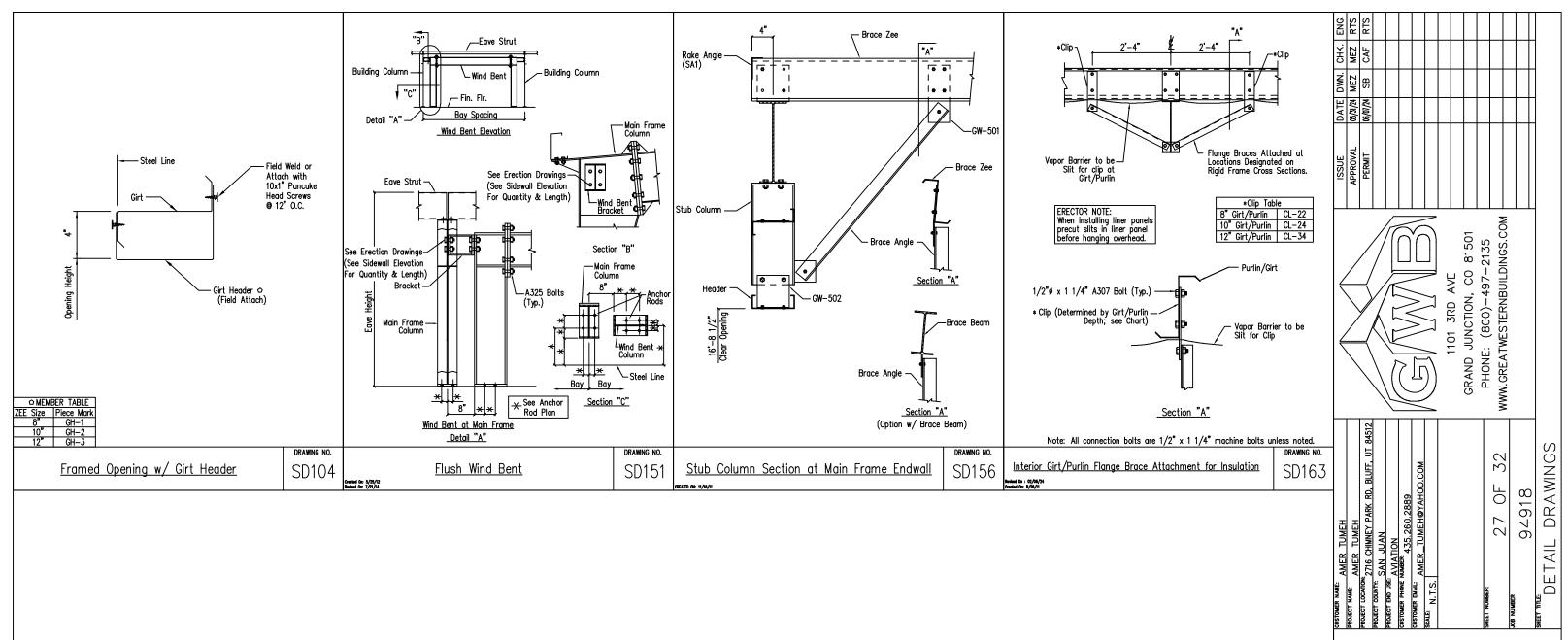
By Richard T Smith at 11:56 am, Jun 12, 2024

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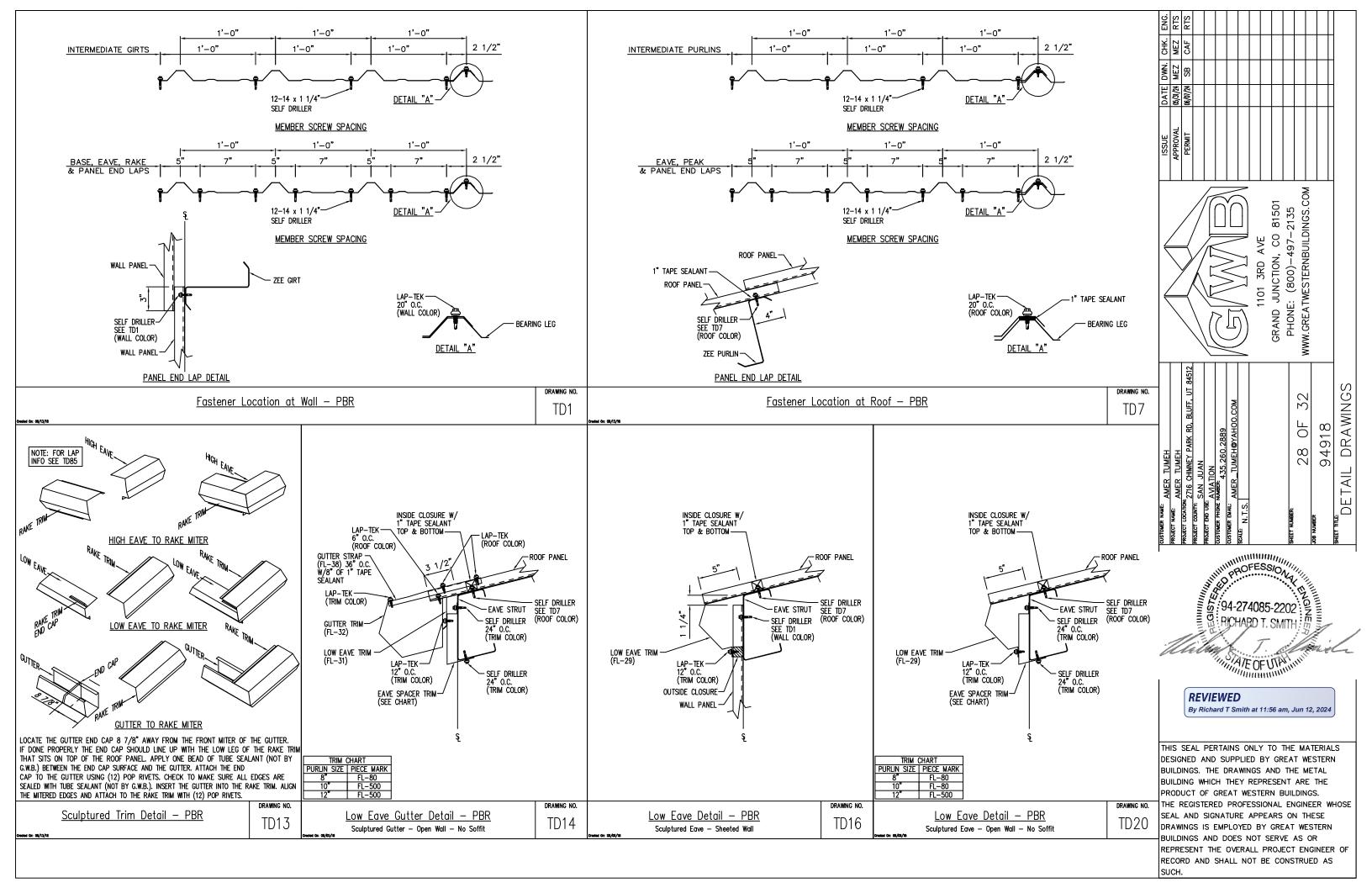


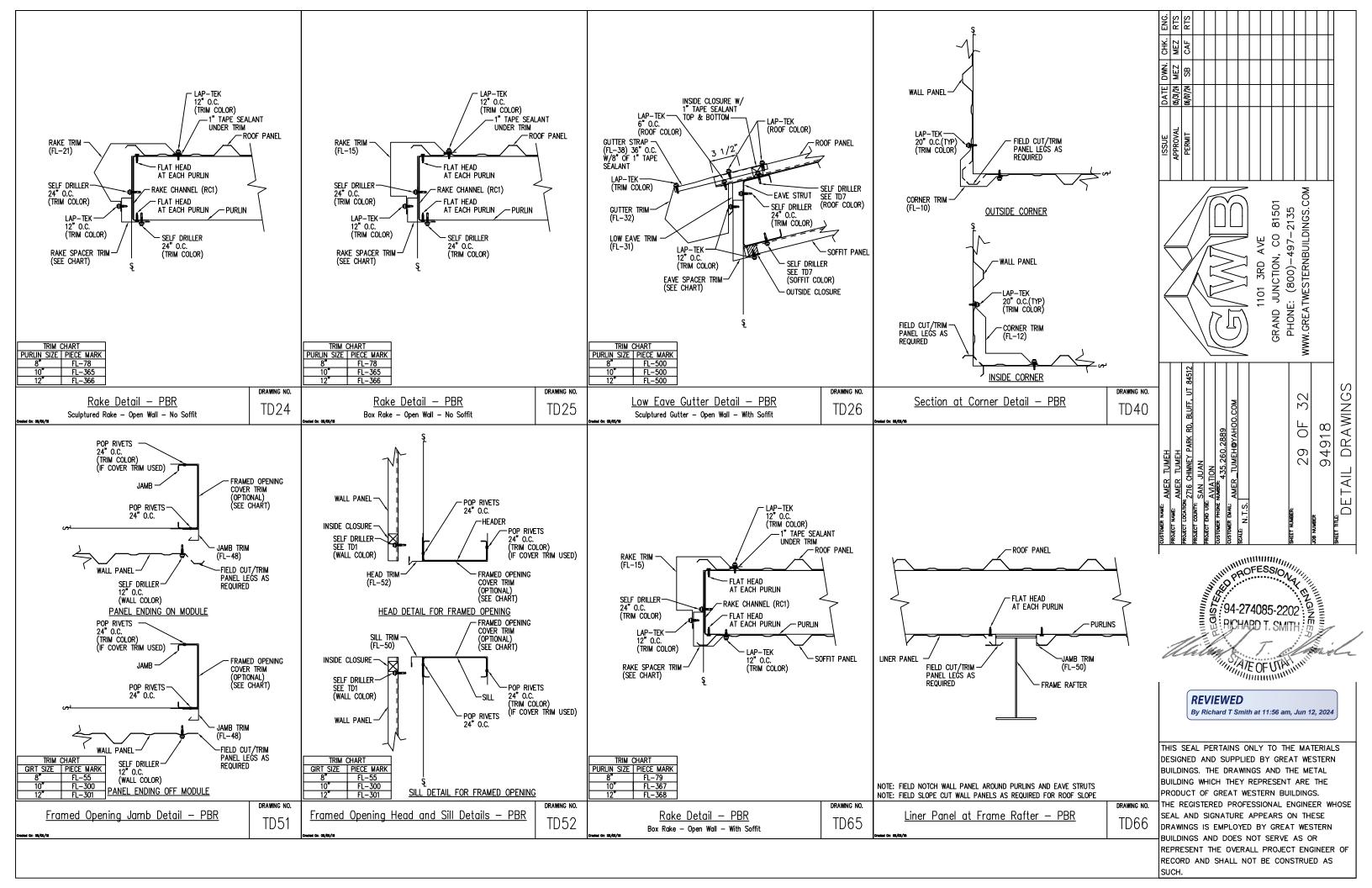


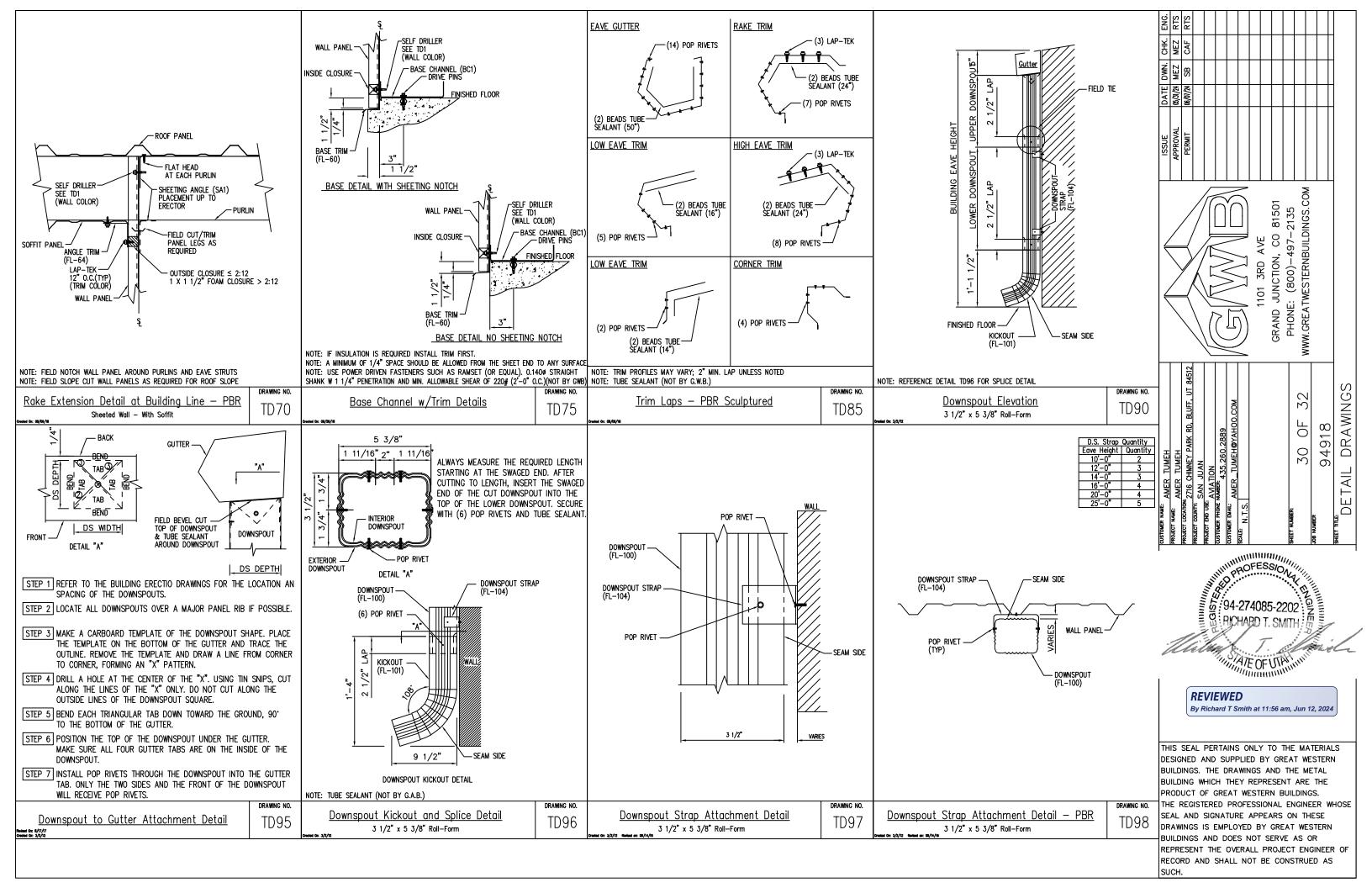


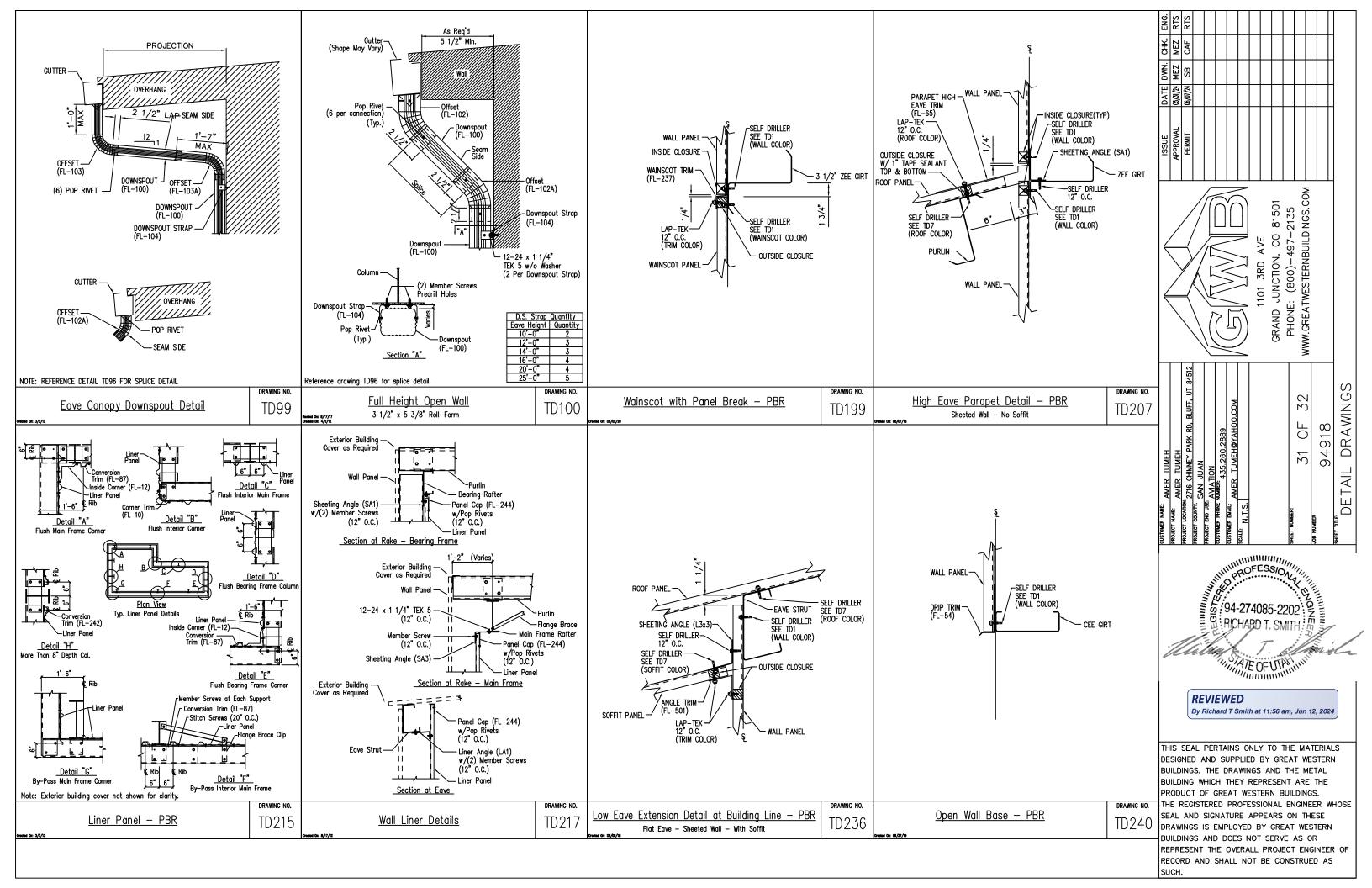
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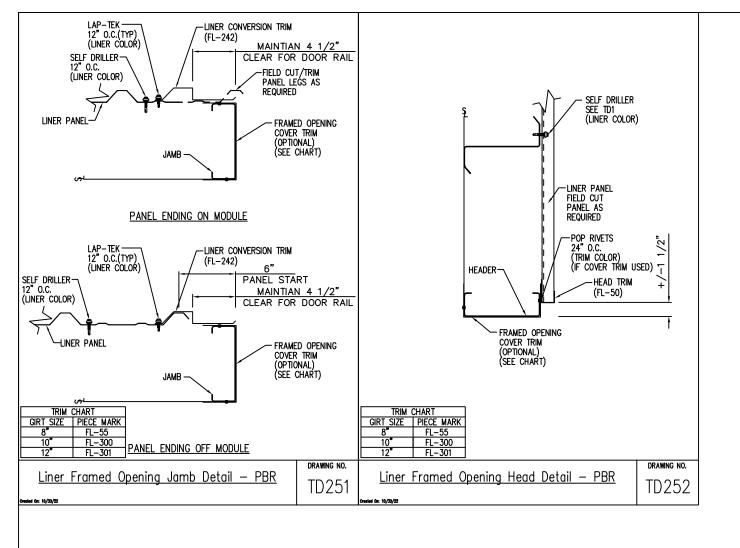
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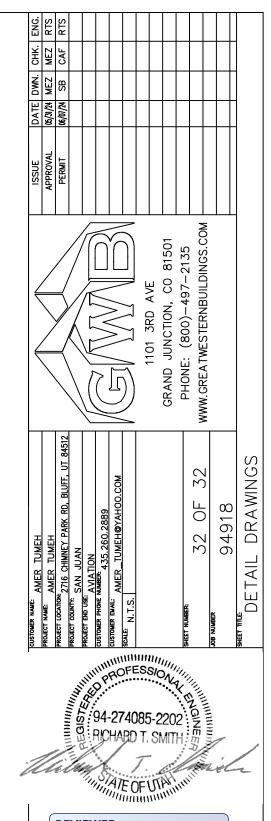












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