## CITY OF ROLLINGWOOD, TEXAS

## CITY OF ROLLINGWOOD PROPOSED DRAINAGE IMPROVEMENTS FOR PLEASANT DRIVE AND NIXON DRIVE



#### PREPARED & SUBMITTED FOR APPROVAL BY:

#### **PRELIMINARY**

THIS DOCUMENT IS RELEASED FOR THE PURPOSE OF INTERIM REVIEW UNDER THE AUTHORITY OF ABE A. SALINAS III, P.E. LIC. # 105144

IT IS NOT TO BE USED FOR CONSTRUCTION, BIDDING OR PERMIT PURPOSES.

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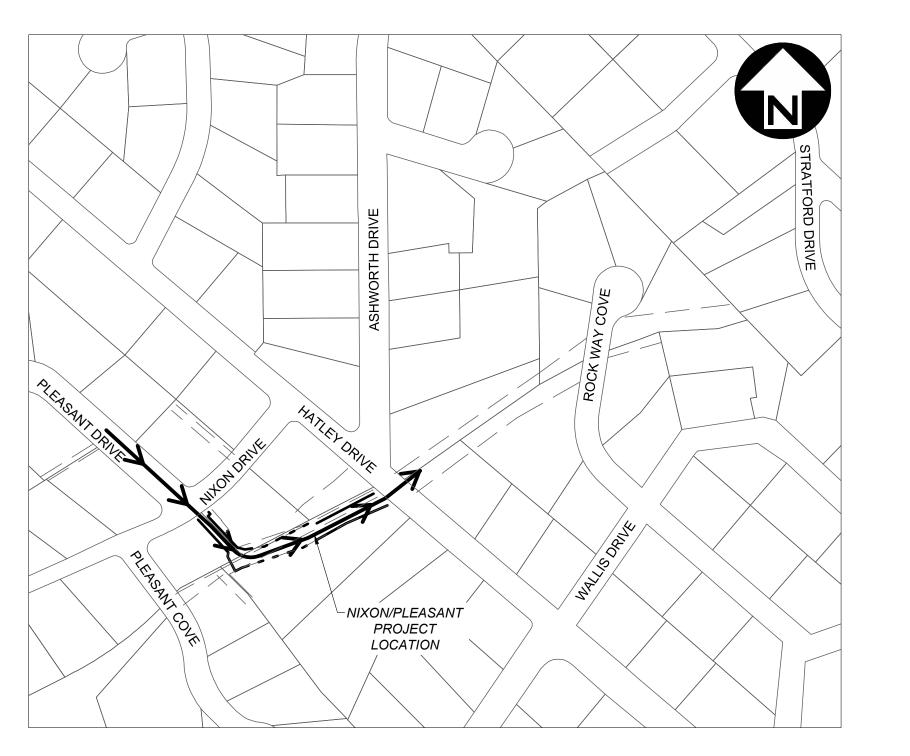
MAYOR GAVIN MASSINGILL - CITY OF ROLLINGWOOD

DATE

ASHLEY WAYMAN - CITY ADMINISTRATOR

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#### **MAYOR** GAVIN MASSINGILL

#### **COUNCIL MEMBERS**

SARA HUTSON **ALEC ROBINSON BROOK BROWN** PHIL McDUFFEE KEVIN GLASHEEN

**CITY ADMINISTRATOR ASHLEY WAYMAN** 



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#### **GENERAL NOTES:**

- 1. THE CONTRACTOR WILL NOTIFY THE OWNER'S REPRESENTATIVE FORTY-EIGHT (48) HOURS IN ADVANCE OF BEGINNING ANY CONSTRUCTION IN THE RIGHT OF WAY OR EASEMENTS.
- 2. THE INFORMATION SHOWN ON THESE DRAWINGS INDICATING TYPE AND LOCATION OF SURFACE, SUBSURFACE, AND AERIAL UTILITIES IS NOT GUARANTEED TO BE EXACT OR COMPLETE. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE EXACT TYPE AND LOCATION OF ALL UTILITIES AFFECTED BY THE CONSTRUCTION IN ORDER TO AVOID DAMAGING THOSE UTILITIES.
- 3. THE CONTRACTOR SHALL COORDINATE WITH OTHER CONTRACTORS AND UTILITIES IN THE VICINITY OF THIS PROJECT. THIS INCLUDES, BUT IS NOT LIMITED TO, GAS, WATER. WASTEWATER, ELECTRIC, TELEPHONE, CABLE TELEVISION, PETROLEUM PIPELINES. FIBER OPTIC, STREET, DRAINAGE, AND ANY OTHER WORK OCCURRING IN OR NEAR THE PROJECT SITE. ONCE THE CONTRACTOR BECOMES AWARE OF A POSSIBLE CONFLICT, IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE OWNER'S REPRESENTATIVE IMMEDIATELY, BUT NO LATER THAN TWENTY-FOUR (24) HOURS AFTER DISCOVERY.
- 4. SHOULD THE CONTRACTOR DAMAGE A UTILITY DURING THE COURSE OF THE WORK, THE CONTRACTOR SHALL IMMEDIATELY ARRANGE FOR REPAIR AND RESTORATION OF THE DAMAGED UTILITY. THE EXPENSE FOR THESE REPAIRS WILL BE AT THE CONTRACTOR'S SOLE EXPENSE.
- 5. ALL EXISTING STRUCTURES, FACILITIES, AND UTILITIES DAMAGED BY CONSTRUCTION SHALL BE REMOVED AND RESTORED WITH MATERIALS EQUAL TO OR BETTER THAN THE ORIGINAL. UNLESS OTHERWISE NOTED IN THE PLANS, THIS WILL NOT BE MEASURED AND PAID FOR DIRECTLY, BUT SHALL BE AT THE CONTRACTOR'S SOLE EXPENSE.
- 6. THE CONTRACTOR SHALL UNCOVER AND VERIFY THE DEPTHS AND HORIZONTAL LOCATION OF ALL EXISITNG WATER, WASTEWATER, AND GAS MAINS TO BE ALTERED OR SUBJECT TO DAMAGE OR INCONVENIENCE BY THIS PROJECT PRIOR TO COMMENCING CONSTRUCTION. NO SEPERATE PAY ITEM.
- SLOPES OF ROADWAY CUTS AND EMBANKMENTS DAMAGED BY ANY OPERATION OF THE CONTRACTOR DURING THE EXECUTION OF THIS PROJECT SHALL BE REPAIRED AND RESTORED TO THE ORIGINAL PRE-CONSTRUCTION CONDITION. BACKFILL AND FILL PLACED DURING REMEDIAL GRADING SHALL BE COMPACTED TO AT LEAST 95% COMPACTION AND TO THE SATISFACTION OF THE ENGINEER AND GOVERNING AUTHORITIES.
- 8. THE SITE IS LOCATED IN THE EDWARD'S AQUIFER RECHARGE ZONE.
- 9. THE CONTRACTOR SHALL NOTIFY ALL RESIDENTS WITHIN THE CONSTRUCTION AREAS 48 HOURS PRIOR TO BEGINNING CONSTRUCTION OF THE PROJECT VIA DOOR FLYERS. THE FLYER IS TO CONSIST OF, BUT IS NOT LIMITED TO:
- A. CONSTRUCTION START DATE AND ESTIMATED COMPLETION DATE.
- DESCRIPTION OF CONSTRUCTION.
- TIME FRAME THE RESIDENT WILL BE WITHOUT WATER IF TEMPORARY SHUTDOWNS ARE REQUIRED, PROVIDED 48 HOURS IN ADVANCE OF WORK.
- CONTRACTOR'S CONTACT INFORMATION.
- CITY'S CONTACT INFORMATION.
- 10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING ANY NECESSARY OFF--SITE LOCATIONS FOR STORAGE OF ALL EQUIPMENT AND MATERIALS REQUIRED FOR THE CONSTRUCTION OF THE PROJECT.
- 11. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVAL OF ALL WASTE MATERIALS DURING CONSTRUCTION AND UPON COMPLETION. THIS WORK WILL BE DONE IN A TIMELY MANNER AS APPROVED BY THE ENGINEER. THIS WORK WILL NOT BE PAID FOR DIRECTLY, BUT CONSIDERED SUBSIDIARY TO THE VARIOUS BID ITEMS.
- 12. BLASTING WITHIN THE PROJECT AREA WILL NOT BE ALLOWED.
- 13. THE CONTRACTOR SHALL BE PREPARED WITH ROCK EXCAVATION EQUIPMENT CAPABLE OF RIPPING THROUGH VERY HARD LIMESTONE SHOULD IT BE ENCOUNTERED FOR THE CONSTRUCTION SITE. BORING LOGS ARE PROVIDED IN THE GEOTECHNICAL REPORT FOR INFORMATIONAL PURPOSES ONLY. CONTRACTOR RESPONSIBLE FOR PERFORMING THEIR OWN TESTING IN THE FORM OF TEST PITS TO DETERMINE THE QUANTITIES OF THE DIFFERENT MATERIALS TO BE EXCAVATED. AS WELL AS THE PREFERRED METHODS AND EQUIPMENT FOR THIS SITE.
- 14. CONTRACTOR WILL MINIMIZE USE OF STREET PARKING BY THEIR EMPLOYEES AND SUBCONTRACTORS IN THE VICINITY OF THE CONSTRUCTION AREA.
- 15. ALL LOCATIONS USED FOR STORING CONSTRUCTION EQUIPMENT, MATERIALS, AND STOCKPILES OF ANY TYPE WITHIN THE CONSTRUCTION LIMITS SHALL BE APPROVED IN ADVANCE BY THE OWNER'S REPRESENTATIVE. USE OF THE AREA WITHIN THE CONSTRUCTION LIMITS FOR THESE PURPOSES WILL BE RESTRICTED TO THOSE LOCATIONS WHERE DRIVER SIGHT DISTANCE TO BUSINESSES AND SIDE STREET INTERSECTIONS IS NOT OBSTRUCTED AND AT OTHER LOCATIONS WHERE AN UNSIGHTLY APPEARANCE AS DETERMINED BY THE OWNER'S REPRESENTATIVE WILL NOT EXIST.
- 16. ALL SITE WORK MUST COMPLY WITH ENVIRONMENTAL REQUIREMENTS INCLUDING TCEQ, TPDES STANDARDS, CLEANWATER ACT, TPDES GENERAL PERMIT TXR150000 (MS4), AND CITY OF ROLLINGWOOD REQUIREMENTS.
- 17. IF CULTURAL RESOURCES ARE ENCOUNTERED DURING CONSTRUCTION (ARCHAEOLOGICAL FINDS UNEARTHED) CONTRACTOR SHALL STOP WORK IN THAT AREA AND IMMEDIATELY CONTACT THE TEXAS HISTORICAL COMMISSION AT (512)463--6100.
- 18. THE CONTRACTOR SHALL FURNISH, INSTALL AND MAINTAIN BARRICADES, WARNING SIGNS, FLASHERS AND OTHER DEVICES OF THE TYPE AND SIZE AS INDICATED IN THE LATEST EDITION OF THE 'TEXAS MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES" OR AS DIRECTED BY THE ENGINEER.
- 19. THE CONTRACTOR SHALL ENSURE THAT ADEQUATE SAFETY PRECAUTIONS ARE MAINTAINED AT ALL TIMES REGARDING AREAS OF OPEN PIPE TRENCH. ALL PIPE TRENCHES SHALL BE COVERED AT ALL TIMES WHEN CONSTRUCTION IS NOT IN PROGRESS. THE TRENCH COVERING SHALL BE CAPABLE OF SUPPORTING TRAFFIC LOADS.

- 20. ALL TRENCH SAFETY CONSTRUCTION OPERATIONS SHALL BE ACCOMPLISHED IN ACCORDANCE WITH OSHA SPECIFICATIONS, STATE OF TEXAS REQUIREMENTS, AND CONTRACT DOCUMENTS WHICH INCLUDE A TRENCH SAFETY PLAN AND A PAY ITEM FOR TRENCH SAFETY MEASURES.
- 21. THE CONTRACTOR SHALL ARRANGE THE OPERATION IN SUCH A MANNER AS TO AVOID UNNECESSARY INCONVENIENCE TO THE PUBLIC IN CONSTRUCTION AREAS.
- 22. CONTRACTOR SHALL NOTIFY THE CITY OF ROLLINGWOOD POLICE DEPARTMENT (512-328-1900) AND THE WESTLAKE FIRE DEPARTMENT (512-539-3400) OF THE CONSTRUCTION SCHEDULES AT LEAST TWO WEEKS IN ADVANCE OF PROPOSED CONSTRUCTION OPERATIONS. CONTRACTOR SHALL PROVIDE PERTINENT INFORMATION ABOUT LANE CLOSURES AND DETOURS AND ANY OTHER CONSTRUCTION RELATED ACTIVITY WHICH MAY INTERFERE WITH NORMAL SERVICES.
- 23. CONTRACTOR SHALL MAINTAIN THE JOB SITE IN A SAFE, NEAT AND WORKMAN-LIKE MANNER AT ALL TIMES. JOB SITE SAFETY SHALL NOT BE COMPROMISED ANY UNATTRACTIVE NUISANCE SHALL BE REMOVED OR CAMOUFLAGED BY CONTRACTOR WHEN DIRECTED BY THE OWNER OR ENGINEER. CONTRACTOR SHALL REMOVE OR CAMOUFLAGE ANY CHILD ATTRACTIVE NUISANCE.
- 24. ALL CONSTRUCTION EQUIPMENT INVOLVED IN ROADWAY WORK SHALL BE EQUIPPED WITH A PERMANENTLY- MOUNTED 360-DEGREE REVOLVING OR STROBE WARNING LIGHT AMBER LENS IN WORKING ORDER. THIS LIGHT SHALL HAVE A MINIMUM LENS HEIGHT OF 5" AND A DIAMETER OF 5". THIS LIGHT SHALL HAVE A MOUNTING HEIGHT OF NOT LESS THAN 6 FEET ABOVE ROADWAY SURFACE AND SHALL BE VISIBLE FROM ALL SIDES. THIS EQUIPMENT SHALL ALSO HAVE ATTACHED AT EACH SIDE OF THE REAR END OF THE VEHICLE AN APPROVED ORANGE WARNING FLAT MOUNTED. NOT LESS THAN 6 FEET ABOVE THE ROADWAY SURFACE.
- 25. EXCAVATION EXCEEDING THE STANDARD PLATING DETAIL SHALL HAVE MATERIAL ONSITE TO BACKFILL OR CONTRACTOR TO PROVIDE STRUCTURAL ENGINEERED PLATING PLANS TO THE CITY OF ROLLINGWOOD PUBLIC WORKS DEPARTMENT FOR APPROVAL PRIOR TO STARTING WORK.
- 26. OVERNIGHT PROTECTION OF WORK ZONE IN CITY OF ROLLINGWOOD R.O.W., REFER TO CITY OF AUSTIN STANDARD DETAIL 804S-4, 1 THRU 4 OF 9. IF PLATING IS NEEDED, REFER TO STANDARD DETAIL 804S-4, 7 OF 9.
- 27. CONTRACTOR SHALL PERFORM WORK ONLY DURING HOURS ALLOWED PER THE CURRENT ORDINANCES.
- 28. THE CONTRACTOR SHALL RE-ESTABLISH ANY PROPERTY MARKER, BENCHMARK, ETC. DISTURBED DURING CONSTRUCTION TO ITS ORIGINAL LOCATION AND ELEVATION. NO ADDITIONAL COMPENSATION SHALL BE ALLOWED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE VERTICAL AND HORIZONTAL CONTROL SHOWN ON THE PLANS THROUGHOUT THE PROJECT, RE-ESTABLISH DISTURBED OR DESTROYED ITEMS BY REGISTERED LAND SURVEYOR IN THE STATE OF TEXAS AT NO ADDITIONAL COST TO THE CITY.
- 29. FENCES, GATES, GROUND SURFACES, CURBS, DRIVEWAYS, MAILBOXES, ETC. SHALL BE LEFT IN A CONDITION EQUAL TO OR BETTER THAN THAT FOUND.
- 30. LANDSCAPED AREAS SHALL BE LEFT UNDISTURBED AS MUCH AS POSSIBLE DURING CONSTRUCTION. ALL AREAS THAT HAVE BEEN DISTURBED DURING CONSTRUCTION SHALL BE RE-SODDED, RE-VEGETATED AND RESTORED TO ORIGINAL OR BETTER CONDITIONS. ALL NEW VEGETATION MUST BE OF THE SAME SPECIES AS ORIGINAL CONDITIONS.
- 31. ACCESS TO ALL SIDE STREETS AND DRIVEWAYS SHALL BE MAINTAINED AT ALL TIMES AT THE SOLE EXPENSES OF THE CONTRACTOR UNLESS OTHERWISE DIRECTED BY THE OWNER'S REPRESENTATIVE.
- 32. CONTRACTOR IS RESPONSIBLE FOR COORDINATING WITH ALL PROPERTY OWNERS PRIOR TO ANY FENCE OR GATE REMOVAL FOR PET MANAGEMENT PURPOSES.
- 33. CONTRACTOR IS RESPONSIBLE FOR VERIFYING CONDITION OF ANY IRRIGATION LINES PRIOR TO CONSTRUCTION.
- 34. CONTRACTOR IS RESPONSIBLE FOR COORDINATING RELOCATION OF PRIVATE IRRIGATION LINES WITH ALL PROPERTY OWNERS PRIOR TO CONSTRUCTION.
- 35. SHOULD CONSTRUCTION IMPACT RESIDENTS RECEIVING SOLID WASTE COLLECTION SERVICES (GARBAGE COLLECTION, RECYCLING COLLECTION, ORGANICS COLLECTION, BRUSH COLLECTION, BULK-TRASH COLLECTION), CONTRACTOR SHALL NOTIFY PROGRESSIVE WASTE SERVICES AT 512-282-3508 TO JOINTLY COORDINATE A SERVICE DELIVERY PLAN IN ORDER TO MINIMIZE DISRUPTION OF ROUTINE SOLID WASTE COLLECTION SERVICES DURING CONSTRUCTION.
- 36. CONTRACTOR AGREES TO ABIDE BY THE LANDOWNERS WRITTEN CONDITIONS IN LANDOWNER AGREEMENTS INCLUDING THOSE RELATED TO PLACEMENT, COMPACTION. RESTORATION, AND EROSION CONTROL MEASURES OF THE SITE(S). IT IS THE CONTRACTORS RESPONSIBILITY TO OBTAIN WRITTEN APPROVAL FROM THE LANDOWNER(S) PRIOR TO PLACEMENT, AND THAT ANY COSTS THE CITY INCURS TO ADDRESS LEGITIMATE LANDOWNER CONCERNS WILL BE CONSIDERED AND MAY BE DEDUCTED FROM THE CONTRACTOR'S FINAL PAYMENT AS DETERMINED BY THE CITY. THE CITY RESERVES THE RIGHT TO REQUEST A COPY OF THE AGREEMENT BETWEEN THE LANDOWNER AND THE CONTRACTOR.
- 37. CONTRACTOR TO REPAIR OR REPLACE IN KIND, AT ITS OWN EXPENCE ANY STRUCTURES DAMAGED IN THE COURSE OF EXECUTING THE WORK.

#### STREET CONSTRUCTION SPECIAL NOTE:

ALL DAMAGE CAUSED DIRECTLY OR INDIRECTLY TO THE STREET SURFACE, SIDEWALK, DRIVEWAY, CURB & GUTTER, OR SUBSURFACE OUTSIDE OF THE PAVEMENT CUT AREA SHALL BE REGARDED AS A PART OF THE STREET CUT REPAIR. THIS INCLUDES ANY SCRAPES, GOUGES, CUTS, CRACKING, DEPRESSIONS, AND/OR ANY OTHER DAMAGE CAUSED BY THE CONTRACTOR DURING THE EXECUTION OF THE WORK. THESE REPAIR AREAS WILL BE INCLUDED IN THE TOTAL AREA OF RESTORATION. THESE AREAS SHALL BE SAW CUT IN STRAIGHT. NEAT LINES PARALLEL TO THE EXCAVATION OR UTILITY TRENCH AND TO THE NEXT EXISTING JOINT FOR SIDEWALKS AND CURB & GUTTER. ALL SUCH REPAIRS SHALL BE AT THE CONTRACTOR'S EXPENSE AND SHALL MEET ALL STANDARDS, AND SPECIFICATIONS.

#### **PLAN NOTES:**

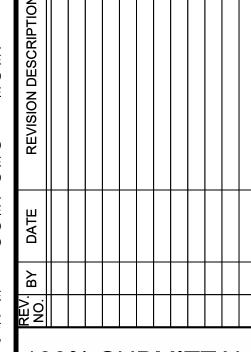
- 1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING AND INSPECTING, ON A REGULAR BASIS, ALL EROSION AND SEDIMENT CONTROL BEST MANAGEMENT PRACTICES, INCLUDING SILT FENCES, CONSTRUCTION ENTRANCES, ROCK FILTER DAMS, ETC., DURING CONSTRUCTION/DEMOLITION AND INCLUDING THE REMOVAL AND PROPER DISPOSAL OF ANY ACCUMULATED SILT AND DEBRIS.
- 2. THE CONTRACTOR SHALL NOT BEGIN ANY WORK UNTIL TREE PROTECTION AND THE EROSION AND SEDIMENT CONTROL BEST MANAGEMENT PRACTICES SUCH AS SILT FENCE, CONSTRUCTION ENTRANCES, ROCK FILTER DAMS, ETC., HAVE BEEN INSTALLED.
- 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR KEEPING THE STREETS FREE OF MUD, DIRT, DEBRIS AND MATERIAL AT ALL TIMES AND SHALL CLEAN/SWEEP THE STREETS ON A REGULAR BASIS AND AT THE DIRECTION OF THE CITY.
- 4. INCREASED STORMWATER PEAK FLOWS DURING CONSTRUCTION MUST BE MITIGATED WITH TEMPORARY BEST MANAGEMENT PRACTICES TO PREVENT HARM TO NEIGHBORING

- 1. CONTRACTOR TO PROVIDE ENGINEER WITH AN UPDATED SCHEDULE WEEKLY. IF NO CHANGES ARE MADE TO THE SCHEDULE FROM THE LAST SUBMITTAL, THE CONTRACTOR IS TO NOTIFY THE ENGINEER OF NO CHANGES.
- 2. THE CONTRACTOR SHALL SUBMIT A DETAILED SCHEDULE OF CONSTRUCTION WHICH COMPLIES WITH THE FOLLOWING SEQUENCE:
- INSTALL TEMPORARY EROSION AND SEDIMENTATION CONTROLS IMMEDIATELY PRIOR TO CONSTRUCTION.
- SET UP TEMPORARY TRAFFIC CONTROL AREAS.
- INSTALL UTILITIES, STRUCTURES, AND PERFORM GRADING AS INDICATED ON CONSTRUCTION PLANS.
- PERFORM STREET RECONSTRUCTION IN AREAS AS NOTED. CONTRACTOR SHALL EXCAVATE AND INSTALL SECTIONS OF FLEXIBLE BASE MATERIAL AND HMAC UP TO THE TOP OF PROPOSED GRADE IN ONE DAY.
- REPAIR CURB AND GUTTER, SIDEWALK CURB RAMP AND OTHER FEATURES AS
- COMMENCE RESTORATION AND REVEGETATION IMMEDIATELY UPON COMPLETION OF EACH PHASE OF THE PROJECT.

- 1. AT LEAST 48 HOURS BEFORE BEGINNING ANY CONSTRUCTION IN PUBLIC R.O.W. OR PUBLIC EASEMENT, THE CONTRACTOR SHALL NOTIFY PUBLIC WORKS.
- 2. THE CONTRACTOR SHALL CONTACT THE ROLLINGWOOD AREA "ONE" CALL SYSTEM AT 1-800-344-8377 FOR EXISTING UTILITY LOCATIONS PRIOR TO ANY EXCAVATION IN ADVANCE OF CONSTRUCTION. THE CONTRACTOR SHALL VERIFY THE LOCATIONS OF ALL UTILITIES TO BE EXTENDED, TIED TO, OR ALTERED OR SUBJECT TO DAMAGE/INCONVENIENCE BY THE CONSTRUCTION OPERATIONS. THE CITY OF ROLLINGWOOD WATER AND WASTEWATER MAINTENANCE RESPONSIBILITY ENDS AT R.O.W./EASEMENT LINES.
- 3. ALL MATERIALS TESTS, INCLUDING SOIL DENSITY TESTS AND DETAILED SOIL ANALYSES, SHALL BE CONDUCTED BY AN INDEPENDENT LABORATORY AND FUNDED BY THE OWNER IN ACCORDANCE WITH CITY OF AUSTIN STANDARD SPECIFICATION ITEM 18045.04.
- 4. ALL MATERIAL USED ON THIS PROJECT MUST BE LISTED ON THE CITY OF AUSTIN STANDARD PRODUCTS LISTING.
- 5. SEWER SERVICES BROKEN BY CONTRACTOR DURING CONSTRUCTION SHALL BE REPLACED BY CONTRACTOR. REPLACEMENT LENGTH IS DEPENDENT ON EXTENT OF DAMAGE. REPLACEMENT PIPE SHALL BE 4" PVC (OR LARGER) SDR 26 AND ATTACHED TO EXISTING SERVICE WITH FLEXIBLE FERNCO CONNECTORS WITH STAINLESS STEEL CLAMPS OR APPROVED EQUAL. WITH NO SEPARATE PAY.
- 6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL COORDINATION BETWEEN THEMSELF AND OTHER CONTRACTORS AND UTILITIES IN THE VICINITY OF THIS PROJECT. THIS INCLUDES, BUT IS NOT LIMITED TO GAS, WATER, WASTEWATER, ELECTRICAL, TELEPHONE, COMMUNICATIONS NETWORKS, CABLE TELEVISION, PETROLEUM PIPELINES, AND STREET AND POSSIBLE CONFLICT. IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY THE CONSTRUCTION INSPECTOR WITHIN TWENTY- FOUR (24) HOURS.
- 7. CONTRACTOR TO ACQUIRE ALL REQUIRED PERMITS.

#### **CONSTRUCTION NOTES**

- WHERE REMOVAL OF BASE AND PAVEMENT IS NECESSARY FOR THE PROJECT, ALL BASE AND PAVEMENT SHALL BE REPLACED IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS, CITY OF AUSTIN, STANDARD SPECIFICATIONS AND STANDARD DETAILS FOR CUT IN PUBLIC RIGHT-OF-WAY. ALL PAVEMENT CUTS SHALL BE SAW-CUT PRIOR TO PLACEMENT OF HMAC.
- 2. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO REMOVE, PRESERVE AND RESET STREET MARKERS AND TRAFFIC CONTROL SIGNS THAT ARE WITHIN THE CONSTRUCTION LIMITS, AS NECESSARY, TO THE LINE AND HEIGHT AS DESCRIBED IN THE LATEST EDITION OF THE TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES BEFORE AND DURING ALL CONSTRUCTION PHASES AND UPON THE COMPLETION OF CONSTRUCTION. SIGNS SHALL NOT BE LAID ON THE GROUND. NO PAYMENT WILL BE MADE FOR THIS WORK, BUT IT WILL BE CONSIDERED SUBSIDIARY TO OTHER BID ITEMS.
- THE CONTRACTOR SHALL SCHEDULE HIS WORK TO MINIMIZE EXPOSURE OF SUBGRADE TO RAIN. IF SUBGRADE IS EXPOSED, CONTRACTOR SHALL UNDERTAKE EXTRA MEASURES TO ACCELERATE DRYING OF THE SUBGRADE INCLUDING PUMPING OF EXCESS WATER AND REWORKING OF THE SUBGRADE AT HIS OWN EXPENSE TO ALLOW THE WORK TO CONTINUE.
- ALL RECONSTRUCTION PREPARATION WORK AND PAVING SHALL BE COMPLETED IN A MANNER SO AS TO PROVIDE A SMOOTH RIDING SURFACE FREE OF BUMPS, DIPS, AND RIPPLES AND A SMOOTH UNIFORM APPEARANCE. THE FINISHED SURFACE SHALL APPROXIMATE THE EXISTING PROFILE.
- 5. CONCRETE SHALL BE REPLACED NO LATER THAN FOUR (4) WORKING DAYS AFTER EXCAVATION OF THE SITE.
- EXPANSION JOINTS SHALL BE PROVIDED AT THE TIE-IN OF NEW CURB AND GUTTER TO EXISTING CURB AND GUTTER AND AT OTHER LOCATIONS AS SHOWN ON THE PLANS OR AS INSTRUCTED BY THE ENGINEER.
- 7. CONTRACTOR SHALL TRIM SHRUBS AND TREES TO PROVIDE CONSTRUCTION CLEARANCE. ALL PRUNING PROPOSED TO BE APPROVED IN ADVANCE BY CITY OF ROLLINGWOOD.
- 8. SODDING FOR EROSION CONTROL SHALL BE APPLIED AS SPECIFIED IN THE PERMANENT EROSION CONTROL NOTES OVER AREAS DISTURBED BY CONSTRUCTION ACTIVITIES AS DESIGNATED BY THE ENGINEER. SODDING SHALL BE WATERED UNTIL A UNIFORM 1 /2" GROWTH IS ESTABLISHED. AT WHICH TIME THE PAYMENT WILL BE MADE, SUBJECT TO APPROVAL BY GENERAL PERMIT PROGRAM OFFICE. WATERING IS INCLUDED IN PAYMENT FOR SODDING.
- 9. CONTRACTOR'S EQUIPMENT SHALL NOT BE LEFT RUNNING WHEN LEFT UNATTENDED OR LEFT IN ONE LOCATION FOR MORE THAN FIVE (5) MINUTES WHILE ATTENDED.



#### 100% SUBMITTAL

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# 0 CIT

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DESIGNED BY	LWM	08/21
CHECKED BY	GE	08/21
REVIEWED BY		_

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#### CONSTRUCTION ACCESS AND SEQUENCING NOTES:

- 1. CHANNEL DEMOLITION AND CONSTRUCTION SHALL BE SEQUENCED FROM DOWNSTREAM TO UPSTREAM. EACH SEGMENT MUST BE ACCEPTED BY THE CITY PRIOR TO COMMENCING NEXT SEGMENT. THE CONTRACTOR SHALL SUBMIT AN ALTERNATE SEQUENCING PLAN FOR REVIEW AND ACCEPTANCE BY THE CITY IN WRITING.
- 2. EROSION CONTROL MEASURES MUST BE IN PLACE PRIOR TO COMMENCING CONSTRUCTION ACTIVITIES.
- 3. NO MORE THAN (10) WORKING DAYS SHALL PASS BETWEEN COMPLETION OF DEMOLITION AND COMMENCEMENT OF PROPOSED CHANNEL ACTIVITIES.
- 4. CONTRACTOR SHALL REQUEST WRITTEN AUTHORIZATION FROM CITY PRIOR TO MOVING TO A NEW SEGMENT OF CONSTRUCTION. ALLOW FOR 3 DAYS FROM AUTHORIZATION REQUEST TO RECEIVE AUTHORIZATION TO PROCEED.
- 5. CARE OF WATER SHALL BE PROVIDED AT ALL TIMES SO AS NOT TO IMPEDE THE FLOW OF STORMWATER.
- 6. CONTRACTOR SHALL MAINTAIN DRAINAGE BETWEEN THE EXISTING CULVERT AND PROPOSED CULVERT AT 300 PLEASANT DURING ALL PHASES.
- 7. SUGGESTED POINTS OF ACCESS ARE SHOWN TO ASSIST THE CONTRACTOR WITH DEMOLITION AND MATERIAL ENTRY. LETTERS OF PERMISSION FOR THESE LOCATIONS ARE NOT INCLUDED WITH THE BID CONTRACT DOCUMENTS AND WILL BE COORDINATED WITH THE CITY.
- 8. CONTRACTOR SHALL NOTIFY PROPERTY OWNERS 48 HOURS PRIOR TO BEGINNING ANY CONSTRUCTION RELATED ACTIVITIES ON THEIR PROPERTY.
- 9. CONTRACTOR SHALL MAINTAIN UP-TO-DATE COPIES OF ALL RIGHT-OF-ENTRY FORMS ON THE PROJECT SITE.
- 10. UPON COMPLETION OF WORK ALL STAGING AREAS SHALL BE RESTORED TO THE ORIGINAL LINES, GRADES, CLEARED OF ALL BRUSH AND DEBRIS, AND REVEGETATED PER SPECIFICATION 609S UNLESS OTHERWISE SPECIFIED IN THE PLANS.
- 11. ALL TREES, SIGNS, WALKWAYS, UTILITIES AND OTHER PHYSICAL FEATURES (WHETHER SHOWN OR NOT SHOWN ON THE PLANS) SHALL BE PROTECTED DURING CONSTRUCTION UNLESS OTHERWISE DIRECTED BY THE CITY OR IN THESE PLANS.
- 12. CONTRACTOR IS RESPONSIBLE FOR PROTECTING PRIVATE PROPERTY FROM DAMAGES, ALL PRIVATE PROPERTY DAMAGED BY CONSTRUCTION ACTIVITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.
- 13. CONTRACTOR MAY NEGOTIATE ADDITIONAL ACCESS AND/OR STORAGE WITH INDIVIDUAL PROPERTY OWNERS AT THEIR EXPENSE.
- 14. CONTRACTOR IS RESPONSIBLE FOR EXPENSES DUE TO NEGLIGENCE.
- 15. CONTRACTOR SHALL OBTAIN APPROVAL FROM THE CITY TO REMOVE TREES NOT IDENTIFIED FOR REMOVAL ON THE DEMOLITION PLAN.
- 16. NO CONSTRUCTION STORAGE OR STAGING SHALL OCCUR WITHIN THE FEMA FLOODPLAIN.

#### **CONSTRUCTION SEQUENCING NOTES:**

- 1. PHASE 1 CONSTRUCTION ACTIVITIES SHALL INCLUDE CHANNEL CONSTRUCTION PROGRESSING FROM STATION 1+00 TO 4+75 DOWNSTREAM TO UPSTREAM.
- PHASE 2 CONSTRUCTION ACTIVITIES SHALL INCLUDE CONSTRUCTION OF THE STORM DRAIN IMPROVEMENTS PROGRESSING FROM STATION 10+00.00 TO 13+80.00 DOWNSTREAM TO UPSTREAM.
- 3. CONSTRUCTION SHALL BE SEQUENCED IN A MANNER THAT WILL NOT DISTURB OR DAMAGE PREVIOUSLY CONSTRUCTED WORK.

#### PHASE 1: CHANNEL IMPROVEMENTS

- 1. PHASE 1 CONSTRUCTION ACTIVITIES SHALL INCLUDE CHANNEL CONSTRUCTION PROGRESSING FROM STATION 1+00 TO 4+50 DOWNSTREAM TO UPSTREAM.
- 2. INSTALL TEMPORARY FENCING AT 303 NIXON DRIVE TO ENCLOSE BACKYARD PERIMETER AREA PRIOR TO BEGINNING CONSTRUCTION. TEMPORARY FENCING SHALL REMAIN IN PLACE THRU DURATION OF CONSTRUCTION UNTIL NEW FENCING IS IN PLACE AND BACKYARD PERIMETER IS SECURED.
- 3. COORDINATE AND RELOCATE EXISTING TREE HOUSE AND SHED LOCATED AT 303 NIXON DRIVE. RELOCATION ON PROPERTY TO BE COORDINATED WITH PROPERTY OWNER.
- PERFORM DEMOLITION AND REMOVAL OF LARGE ROCK, CONCRETE RIPRAP AND OTHER DELETERIOUS MATERIALS FROM THE EXISTING CHANNEL LOCATED ALONG 303 NIXON DRIVE AND REPLACE WITH SELECT FILL COMPACTED TO 95% STANDARD PROCTOR DENSITY.
- 5. CONSTRUCT CHANNEL IMPROVEMENTS INCLUDING EXCAVATION, GRADING, ACTIVITIES AND ROCK BOULDER PLACEMENT. CONTRACTOR TO PROVIDE MATERIAL SAMPLES PRIOR TO SELECTION AND DELIVERY OF ROCK BOULDERS.
- 6. CONTRACTOR SHALL PROVIDE 48 HOURS NOTICE TO THE CITY PRIOR TO PLACEMENT OF ROCK BOULDERS IN THE CHANNEL.
- 7. PLACEMENT OF TOP SOIL AND LANDSCAPE PLANTINGS SHALL BE PERFORMED AS PART OF PHASE 3 FINAL WORK.
- 8. COMPLETE ALL WORK IN THIS PHASE BEFORE STARTING PHASE 2.

#### PHASE 2: PLEASANT DRIVE IMPROVEMENTS

- CONSTRUCT ALL STORM DRAIN AND ROADWAY IMPROVEMENTS WITHIN THE WORK ZONE PER THE PLANS. ADJUST ANY VALVES AND MANHOLES TO MEET PROPOSED GROUND ELEVATIONS FOR UTILITIES WITHIN THE WORK ZONE PER THE UTILITY PLANS.
- 2. CONSTRUCT THE STORM DRAIN BOX IN PHASES TO MAINTAIN ACCESS TO ALL RESIDENTIAL PROPERTIES WITHIN THE WORK ZONE. COORDINATE ANY REQUIRED DRIVEWAY CLOSURES WITH THE RESIDENTS OF THE PROPERTY AT LEAST 48 HOURS IN ADVANCE.
- 3. CONSTRUCTION OF STORM DRAIN BOX SHALL PROGRESS FROM DOWNSTREAM TO UPSTREAM. IT IS WITH THE INTENTION OF AIDING WITH CONSTRUCTABILITY AND ACCESS THAT CAST-IN-PLACE BOX CULVERT IS PROPOSED FROM STATIONS 10+32 TO 11+23. ALTERNATIVELY, THE CONTRACTOR MAY SUBMIT A PLAN FOR THE USE OF A PRECAST BOX FOR THIS SEGMENT, SUBJECT TO APPROVAL BY THE ENGINEER. NO ADDITIONAL WORK DAYS WILL BE AWARDED FOR THIS SUBSTITUTION.
- 4. DUE TO THE OVERALL SIZE OF THE STORM DRAIN WITHIN PLEASANT DRIVE, SAFETY IS OF THE UTMOST IMPORTANCE. MAINTAIN BARRIERS AND SAFETY FENCING AROUND ANY OPEN EXCAVATIONS. MAKE SURE ALL TRENCHES ARE FILLED IN AT THE END OF THE WORK DAY.
- 5. MAINTAIN 3:1 MAX SIDE SLOPES AT THE END OF EACH WORK DAY FOR PAVEMENT DROP-OFFS GREATER THAN 2'.

#### PHASE 3: FINAL WORK

- 1. CONSTRUCT FINAL 2" LIFT OF THE TYPE D HOT MIX ASPHALTIC PAVEMENT SURFACE LAYER USING TXDOT TRAFFIC CONTROL DETAILS FOR SURFACING OPERATIONS STANDARD (7-1) AND TXDOT TCP MOBILE OPERATIONS STANDARD (3-1).
- 2. PERFORM ANY REMAINING FINAL GRADING AND PLACE TOPSOIL AND SEEDING AND/OR SODDING. AND LANDSCAPE PLANTINGS.
- 3. REMOVE ALL TEMPORARY SW3P DEVICES AND TREE PROTECTION, AS DIRECTED.
- 4. PERFORM FINAL CLEANUP.

### Texas Commission on Environmental Quality Water Pollution Abatement Plan General Construction Notes

#### Edwards Aquifer Protection Program Construction Notes – Legal Disclaimer

The following/listed "construction notes" are intended to be advisory in nature only and do not constitute an approval or conditional approval by the Executive Director (ED), nor do they constitute a comprehensive listing of rules or conditions to be followed during construction. Further actions may be required to achieve compliance with TCEQ regulations found in Title 30, Texas Administrative Code (TAC), Chapters 213 and 217, as well as local ordinances and regulations providing for the protection of water quality. Additionally, nothing contained in the following/listed "construction notes" restricts the powers of the ED, the commission or any other governmental entity to prevent, correct, or curtail activities that result or may result in pollution of the Edwards Aquifer or hydrologically connected surface waters. The holder of any Edwards Aquifer Protection Plan containing "construction notes" is still responsible for compliance with Title 30, TAC, Chapters 213 or any other applicable TCEQ regulation, as well as all conditions of an Edwards Aquifer Protection Plan through all phases of plan implementation. Failure to comply with any condition of the ED's approval, whether or not in contradiction of any "construction notes," is a violation of TCEQ regulations and any violation is subject to administrative rules, orders, and penalties as provided under Title 30, TAC § 213.10 (relating to Enforcement). Such violations may also be subject to civil penalties and injunction. The following/listed "construction notes" in no way represent an approved exception by the ED to any part of Title 30 TAC, Chapters 213 and 217, or any other TCEQ applicable regulation

- 1. A written notice of construction must be submitted to the TCEQ regional office at least 48 hours prior to the start of any regulated activities. This notice must include:
- nours prior to the start of any regulated activition the name of the approved project;
  - the activity start date; and
  - the contact information of the prime contractor.
- 2. All contractors conducting regulated activities associated with this project must be provided with complete copies of the approved Water Pollution Abatement Plan (WPAP) and the TCEQ letter indicating the specific conditions of its approval. During the course of these regulated activities, the contractors are required to keep on-site copies of the approved plan and approval letter.
- If any sensitive feature(s) (caves, solution cavity, sink hole, etc.) is discovered during construction, all regulated activities near the sensitive feature must be suspended immediately. The appropriate TCEQ regional office must be immediately notified of any sensitive features encountered during construction. Construction activities may not be resumed until the TCEQ has reviewed and approved the appropriate protective measures in order to protect any sensitive feature and the Edwards Aquifer from potentially adverse impacts to water quality.
- 4. No temporary or permanent hazardous substance storage tank shall be installed within 150 feet of a water supply source, distribution system, well, or sensitive feature.
- 5. Prior to beginning any construction activity, all temporary erosion and sedimentation (E&S) control measures must be properly installed and maintained in accordance with the approved plans and manufacturers specifications. If inspections indicate a control has been used inappropriately, or incorrectly, the applicant must replace or modify the control for site situations. These controls must remain in place until the disturbed areas have been permanently stabilized.
- Any sediment that escapes the construction site must be collected and properly disposed of before the next rain event to ensure it is not washed into surface streams, sensitive features, etc.

Page 1 of 2

Page 2 of 2

7. Sediment must be removed from the sediment traps or sedimentation basins not later than

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when it occupies 50% of the basin's design capacity.

- Litter, construction debris, and construction chemicals exposed to stormwater shall be prevented from being discharged offsite.
- All spoils (excavated material) generated from the project site must be stored on-site with proper E&S controls. For storage or disposal of spoils at another site on the Edwards Aquifer Recharge Zone, the owner of the site must receive approval of a water pollution abatement plan for the placement of fill material or mass grading prior to the placement of spoils at the other site.
- 10. If portions of the site will have a temporary or permanent cease in construction activity lasting longer than 14 days, soil stabilization in those areas shall be initiated as soon as possible prior to the 14<sup>th</sup> day of inactivity. If activity will resume prior to the 21<sup>st</sup> day, stabilization measures are not required. If drought conditions or inclement weather prevent action by the 14<sup>th</sup> day, stabilization measures shall be initiated as soon as possible.
- 11. The following records shall be maintained and made available to the TCEQ upon request:
  - the dates when major grading activities occur;
  - the dates when construction activities temporarily or permanently cease on a portion of the site; and
  - of the site; and
     the dates when stabilization measures are initiated.
- 12. The holder of any approved Edward Aquifer protection plan must notify the appropriate regional office in writing and obtain approval from the executive director prior to initiating any of the following:
  - A. any physical or operational modification of any water pollution abatement structure(s), including but not limited to ponds, dams, berms, sewage treatment plants, and diversionary structures;
  - B. any change in the nature or character of the regulated activity from that which was originally approved or a change which would significantly impact the ability of the plan to prevent pollution of the Edwards Aquifer;
  - C. any development of land previously identified as undeveloped in the original water pollution abatement plan.

Austin Regional Office
12100 Park 35 Circle, Building A
Austin, Texas 78753-1808
Phone (512) 339-2929
Fax (512) 339-3795

San Antonio Regional Office
14250 Judson Road
San Antonio, Texas 78233-4480
Phone (210) 490-3096
Fax (210) 545-4329

THESE GENERAL CONSTRUCTION NOTES MUST BE INCLUDED ON THE CONSTRUCTION PLANS PROVIDED TO THE CONTRACTOR AND ALL SUBCONTRACTORS.

REV. BY DATE REVISION DESCRIPTION

#### 100% SUBMITTAL

THIS DOCUMENT IS RELEASED FOR THE PURPOSE OF INTERIM REVIEW UNDER THE AUTHORITY OF ABE A. SALINAS III, P.E. LIC. # 105144 4/12/2023

IT IS NOT TO BE USED FOR CONSTRUCTION, BIDDING OR PERMIT PURPOSES.

CITY OF ROLLINGWOOD
PROPOSED DRAINAGE IMPROVEMENTS
GENERAL NOTES (2 OF 2)

R@LLINGWOOD TEXAS K•FRIESE + ASSOCIATES

PUBLIC PROJECT ENGINEERING

NOTES NAME DATE

SURVEY BY

DRAWN BY AH 08/21

DESIGNED BY LWM 08/21

CHECKED BY GE 08/21

1120 S. Capital of Texas Highway

P - 512.338.1704 F - 512.338.1784

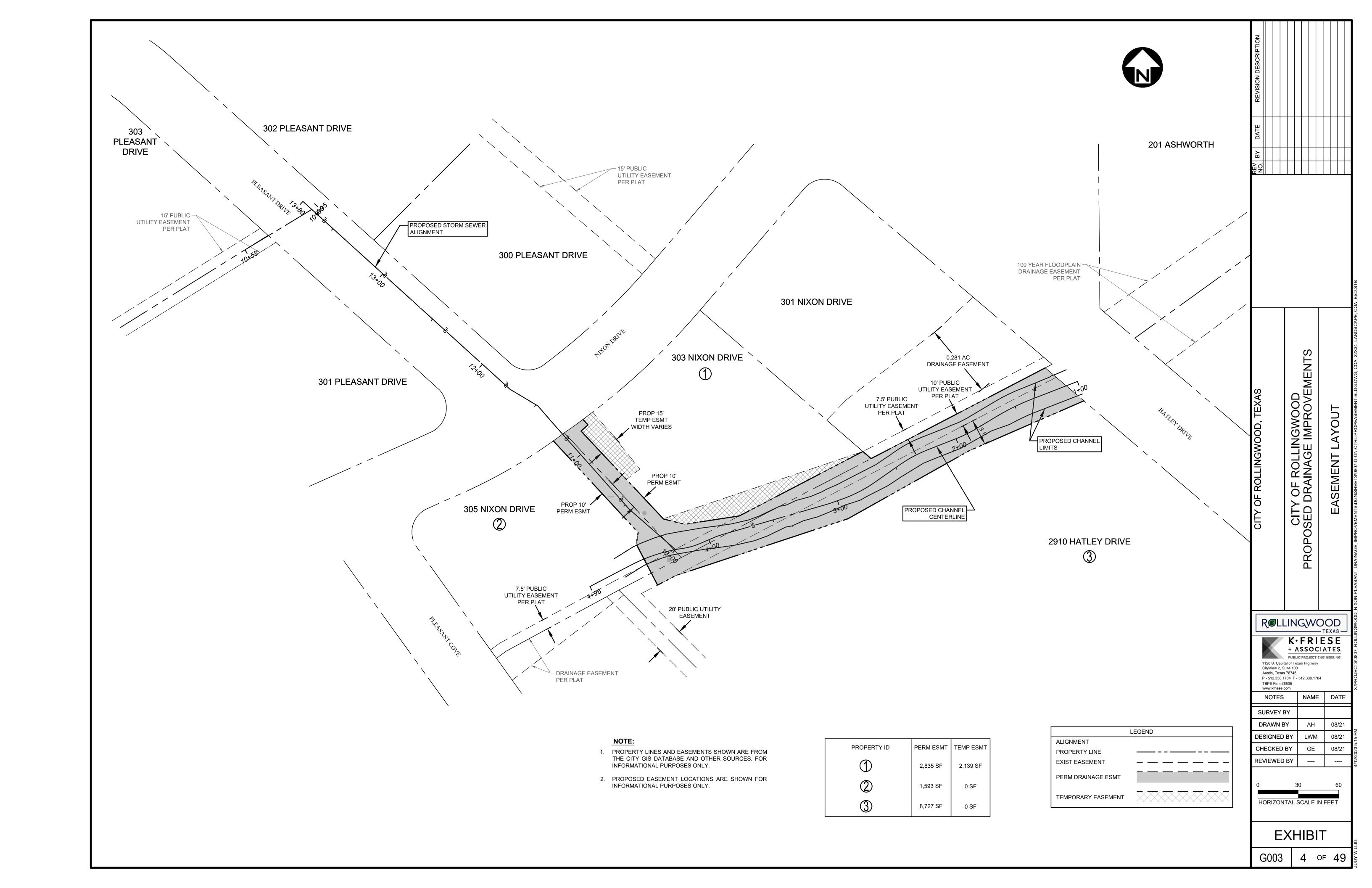
CityView 2, Suite 100

TBPE Firm #6535

REVIEWED BY

G003 3 OF 49

TCEQ-0592 (Rev. July 15, 2015)



			TRAFFIC CONTROL								UTILITY							
	CITY OF AUSTIN				ION SHEETS			INAGE SHE	I		SHEETS	SWPPP SI				PE SHEETS	T	
ITEM NO.	BID ITEM DESCRIPTION	UNIT	8 (CT101)	9 (DP101)	10 (DP102)	15 (PLPR01)	16 (PLPR02)	17 (PLPR03)	19 (GD101)	20 GD(102)	21 (CV01)	37 (EV101)	38 (EV102)	40 (TP01)	41 (TP02)	42 (LL01)	43 (LL02)	TOTAL
1	MOBILIZATION INSURANCE & BOND	LS LS																1
101S-C	PREPARING RIGHT-OF-WAY	LS																1
	BARRICADES, SIGNS, AND TRAFFIC HANDLING	LS LF	1	110											-			1 116
*(1)	REMOVE CHAIN LINK FENCE REMOVE 18" RCP AND CAP	LF		116 25														116 25
*(1)	REMOVE CINDERBLOCK WALL	LF		86														86
*(1)	REMOVE FOOT BRIDGE AND STAIRS REMOVE EXT WOODEN FENCE	EA LF																0
*(1)	REMOVE, SALVAGE, RELOCATE SHED AT 303 NIXON DRIVE	EA		1														1
*(1)	REMOVE, SALVAGE, RELOCATE PLAYHOUSE AT 303 NIXON DRIVE REMOVE MISCELLANEOUS CONCRETE, MASONRY AND ROCKWALLS	EA		303	18													321
*(1)	REMOVE ASPHALT PAVEMENT DRIVEWAYS	SY		36	10													36
*(1)	REMOVE CONCRETE RIPRAP	SY LF		15 312														15
104S-A 104S-C	REMOVE CONCRETE CURB REMOVE CONCRETE SIDEWALKS & DRIVEWAYS	SY		55														312 55
	STREET EXCAVATION (12" DEPTH)	CY		177														177
315S-A 120S-B	SURFACE MILLING ASPHALTIC PAVEMENT (1.5" TO 2" DEPTH)  CHANNEL EXCAVATION	SY CY		562			612											562 612
132S-A	EMBANKMENT (FINAL) (ORD COMP) (TY A)	СҮ				0	0											0
160-6003 TxDOT 210S-A	FURNISHING AND PLACING TOPSOIL (4")  FLEXIBLE BASE (8" COMP. DEPTH)	SY CY					944											944
340S-B-1	HOT MIX ASPHALTIC PAVEMENT, TYPE D (NO RAP, NO RAS, PG-64-22) (2" COMP. DEPTH)	SY					1092											1092
340S-B-2 340S-B-3	HOT MIX ASPHALTIC PAVEMENT, TYPE C OR D LEVEL UP (20% MAX RAP, NO RAS) (2" COMP. DEPTH)  ASPHALTIC CONCRETE DRIVEWAY	SY SY					530 24											530 24
3405-B-3 402-A	FLOWABLE FILL (LOW STRENGTH)	CY					4	8	X									12
4115	BOX CULVERT RESURFACING & REPAIR (HATLEY CULVERTS)	EA				1												1
401S-A 414S-C	BOX CULVERT STRUCTURAL EXCAVATION & BACKFILL  CONCRETE RETAINING WALL, INCLUDING REINFORCEMENT (CHANNEL)	CY				651												651
430S-A	P.C. CONCRETE CURB AND GUTTER (EXCAVATION) (6 IN TO 8 IN HEIGHT)	LF					197											197
430S-C 433S-A	P.C. CONCRETE CURB (EXCAVATION) (6 IN TO 8 IN HEIGHT)  PORTLAND CEMENT CONCRETE DRIVEWAYS (FLARED TYPE I)	LF SF					657		6									6 657
	CAST-IN-PLACE REINFORCED CONCRETE BOX CULVERT (5'X4)	LF					91											91
462-6006	PRECAST REINFORCED CONCRETE BOX (5'X2')	LF					250	54										54
462-6008	PRECAST REINFORCED CONCRETE BOX (5'X4')  REINFORCED CONCRETE BOX CONNECTION (5'X2')	LF EA					250											250
464-6005	REINFORCED CONCRETE PIPE (CL III) (24" DIA)	LF						76										76
506S-J2 506S-4	JUNCTION BOX 8'X8'X6' W/ MH RISER MINOR MANHOLE HEIGHT ADJUSTMENT	EA EA					1			1								1
508S-I10R	INLET TYPE I (COMPLETE) (10 FT)	EA					1			1								1
508S-120R	INLET TYPE I (COMPLETE) (20 FT)	EA					3											3
508S-E 508S-9	ENERGY DISSIPATORS, 48 IN. DIA. (AT OUTFALL OF 5'X4' RCB)  4-SIDED AREA INLET (4'X4')	EA EA				3	1											3
	T CONC HEADWALL WITH FLARED WINGS, HW=X FT (CH-FW-0)	EA					2											2
466 466	CONC HEADWALL WITH FLARED WINGS, HW=X FT (SW-0)  CONC HEADWALL WITH FLARED WINGS, HW=X FT (CH-FW-0)	EA EA					1											0
591S-B	DRY ROCK RIPRAP (D50 18")	CY				57												57
591S-F	CONCRETE RIPRAP (6" THICK)	SY				4												4
?	METAL FENCE (5' HIGH)  METAL FENCE PEDESTIRAN GATE, 5 FOOT X 3 FOOT	LF EA				170 2												170
?	WOODEN FRAMED HOGWIRE FENCE	LF				21												21
?	CONCRETE CINDERBLOCK (CMU) WALL (2' HIGH)  ROCK RIPRAP (18-INCH D50)	SF SY				7												7
	STONE VENEER (LIMESTONE)	SY							78									78
SS-1 504S-32	GAS UTILITY ADJUSTMENT ADJUST WATER VALVE BOXES TO GRADE	EA EA		-			2											2
101S-C1	6" WATER LINE REMOVAL	LF									232							232
101S-C2	WATER VALVES, REMOVE AND SALVAGE	EA									3							3
510-AR-1 510-BR	8" WATERLINE, C-900 TYPE WATER TIE-IN CONNECTION	LF EA									254 3							254 3
510-KR	FITTINGS, WATER	TON																0
*(2)	TREE REMOVAL REMOVAL, TREE	LS IN																0
\—/	LANDSCAPE WALL (LESS THAN 4 FEET TALL)	TON																0
	LANDSCAPE WALL, BOULDERS (4-6 FEET TALL)  LANDSCAPE WALL, STRUCTURAL CONCRETE	TON CY					-											0
	METAL FENCE (5' HIGH) W/ PEDESTRIAN GATES	LF																0
	PRIVACY WALL	FF									——————————————————————————————————————							0
	DRAINAGE WALL VENEER  GRASS, NATIVE GRASS SEED	FF SF																0
	GRASS, NATIVE BUFFALO GRASS SOD	SF																0
	SHRUB, 1 GALLON SHRUB, 5 GALLON	CY EA.																0
	SHRUB, 10 GALLON	EA.																0
	TREE PLANTINGS, X GALLON	TON																0
701S-T	MULCH, CRUSHED LIMESTONE (600 SQUARE FEET AT 4" THICK)  TEMPORARY FENCE, 4' HIGH, CHAIN LINK WIRE	TON LF										165						165
	CURB INLET GRAVEL FILTERS	EA																0
639S-0	ROCK FILTER DAMS (INSTALL/REMOVE) (TYPE 3)  CONSTRUCTION EXITS (INSTALL/REMOVE)	LF SY					-					20	77					97
	TEMPORARY SEDIMENT CONTROL FENCE	LF										177						177
	GRASS SODDING FOR EROSION CONTROL, OTHER APPROVED SODDING (BERMUDA, ST. AUGUSTINE, ETC.)	1										192 LF (MS)						0
610S-0 999-0	TREE AND VEGETATION PROTECTION  MATERIAL ADJUSTMENT ALLOWANCE	LS LS																0
	TRENCH EXCAVATION SAFETY PROTECTION	LF																0
	REMOVE AND RELOCATING IRRIGATION SYSTEM	LF									-							0
	NEW OF EACH OF THE CONTROL OF THE CO			,	1								,			, '	1	0
	TXDOT BID ITEMS											 						0
																		0 0

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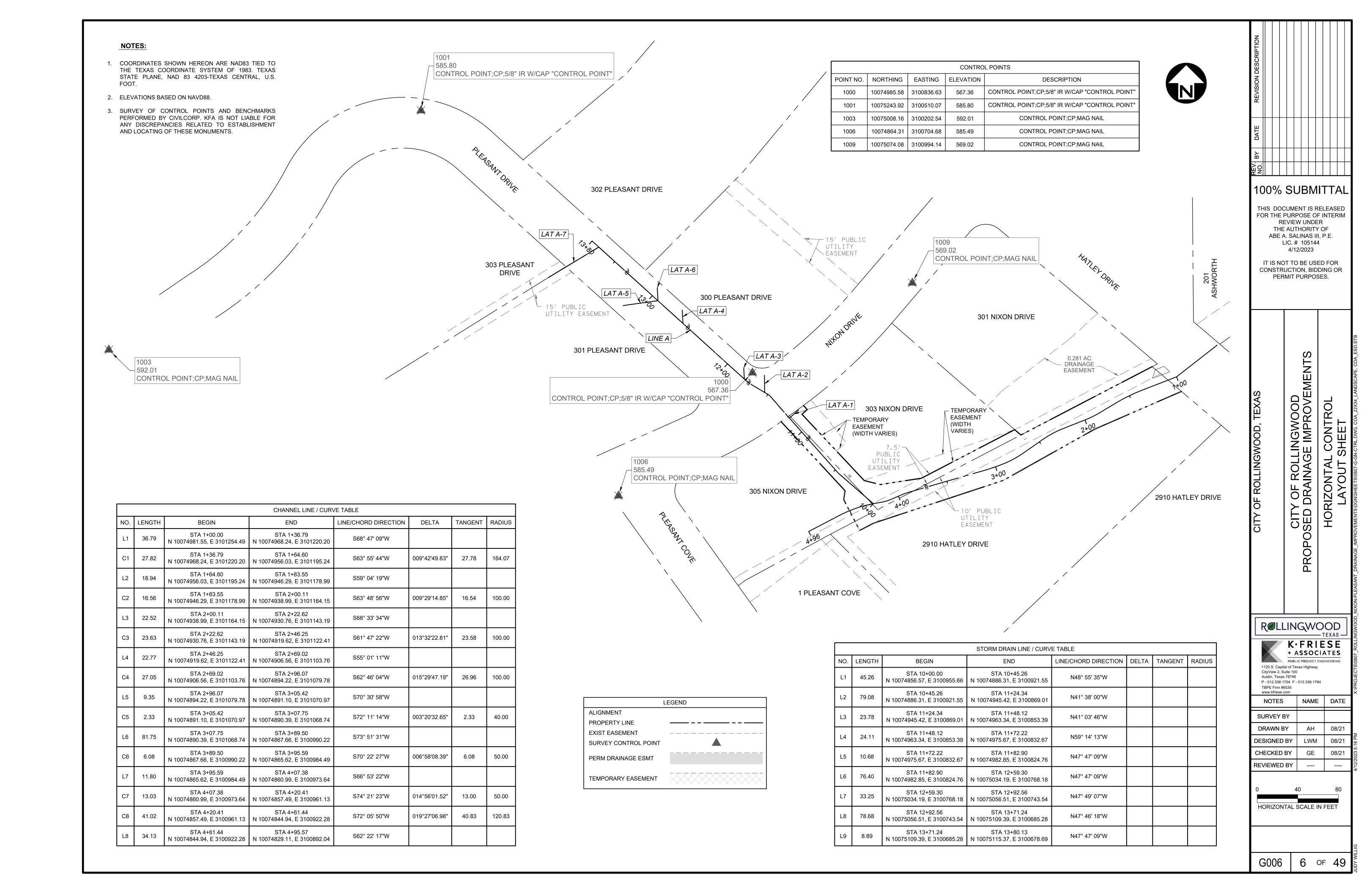
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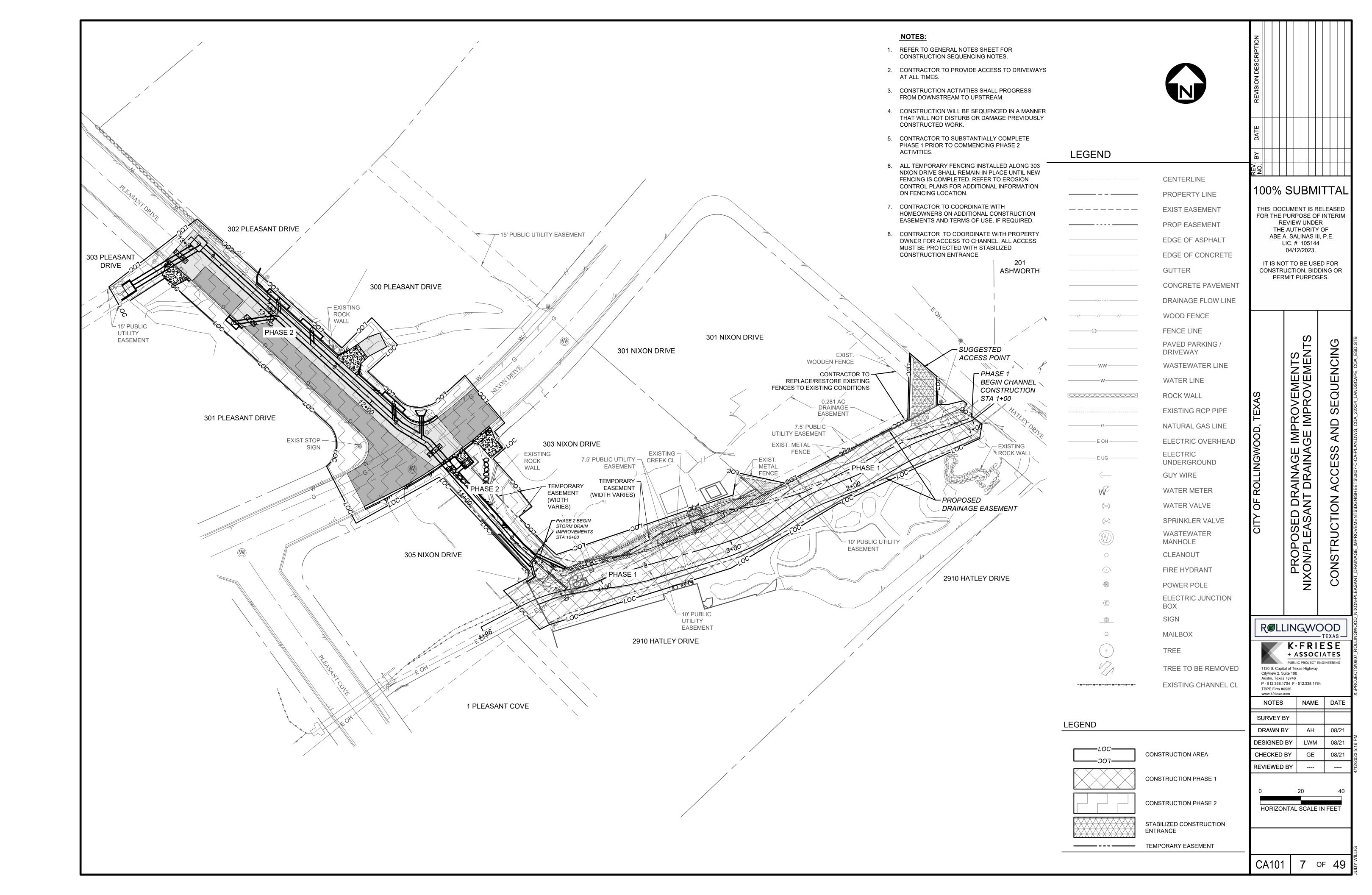
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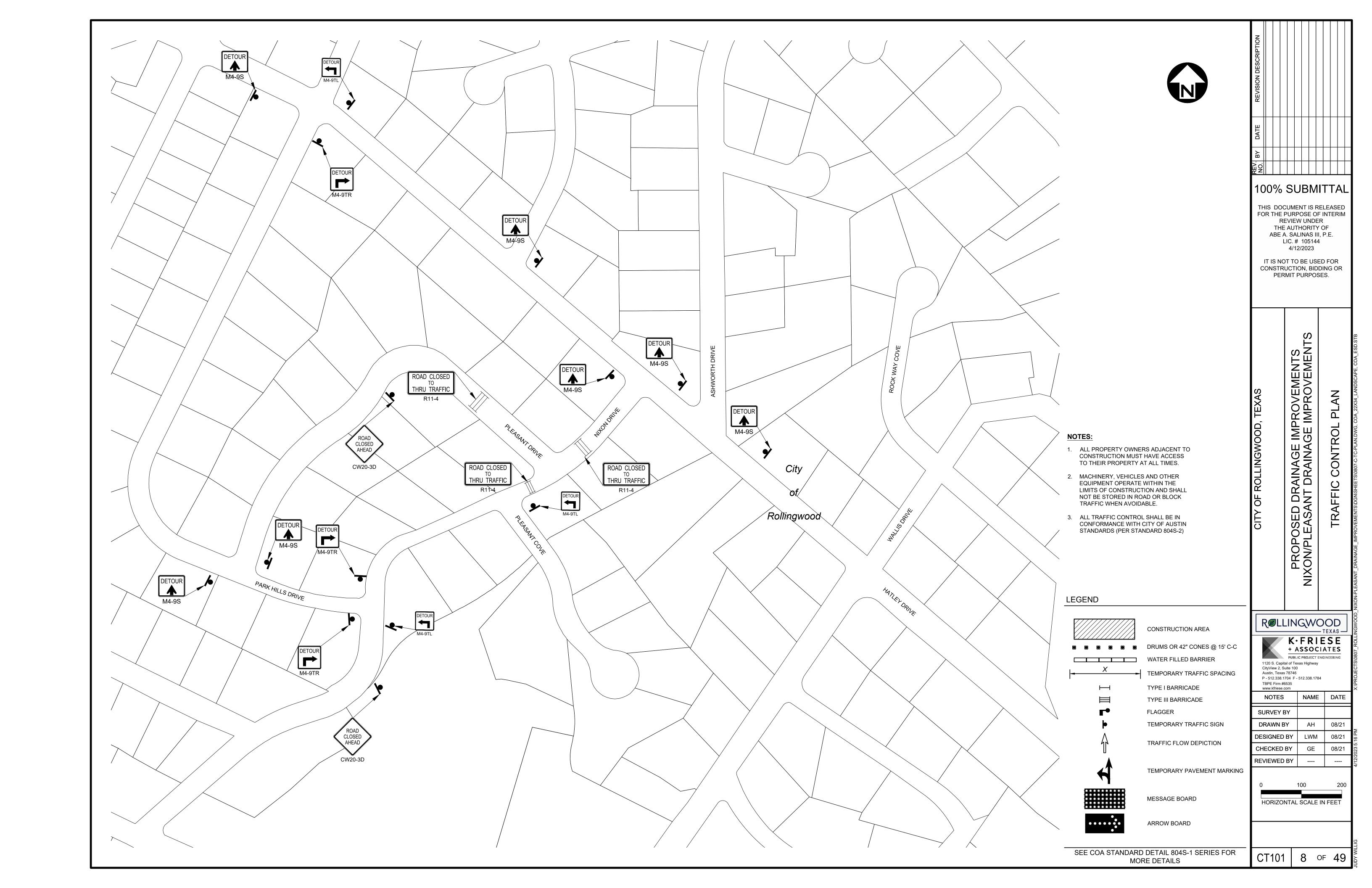
NOTES	NAME	DATE
SURVEY BY		
DRAWN BY	АН	08/21
DESIGNED BY	LWM	08/21
CHECKED BY	GE	08/21
REVIEWED BY		

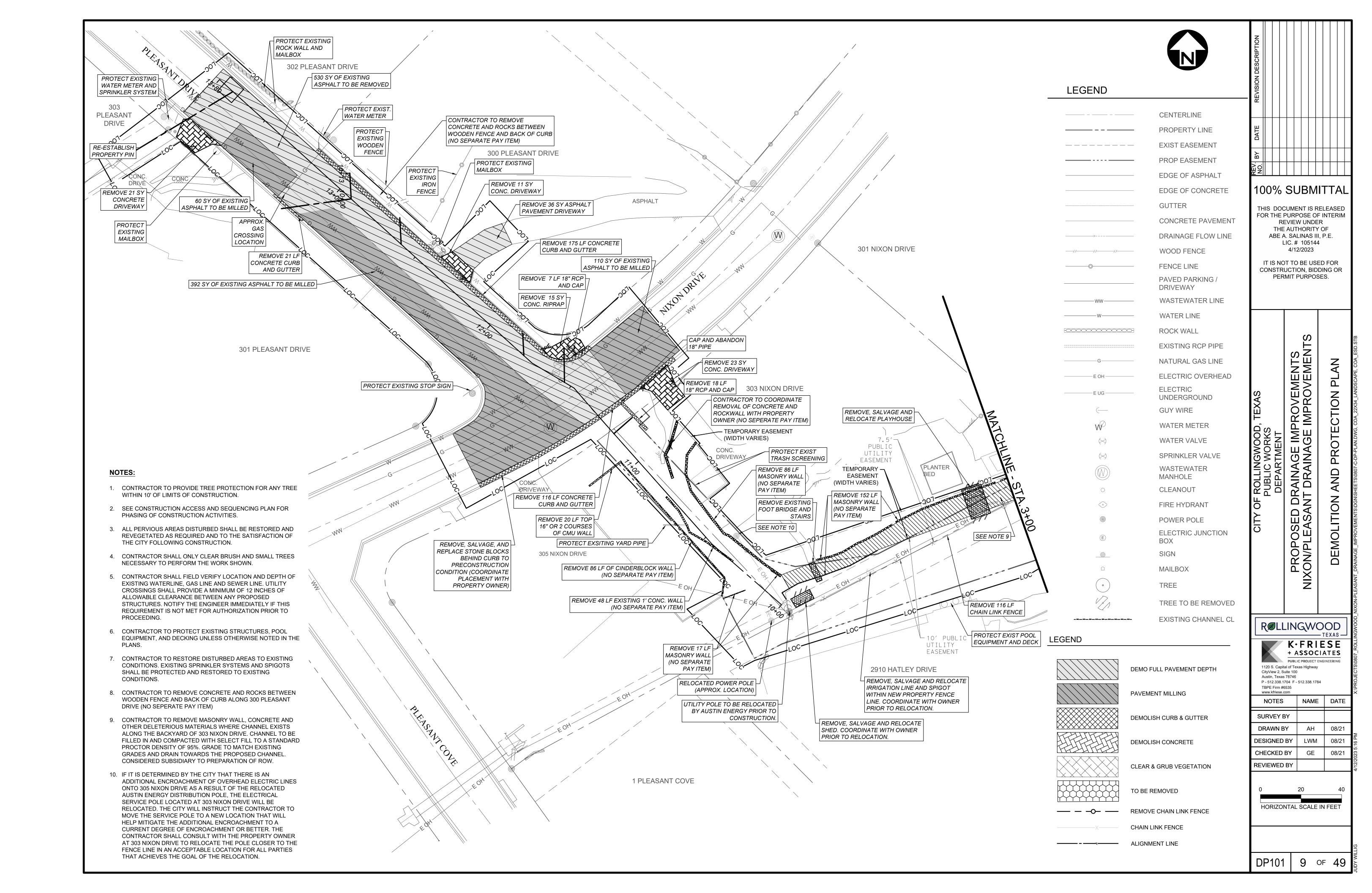
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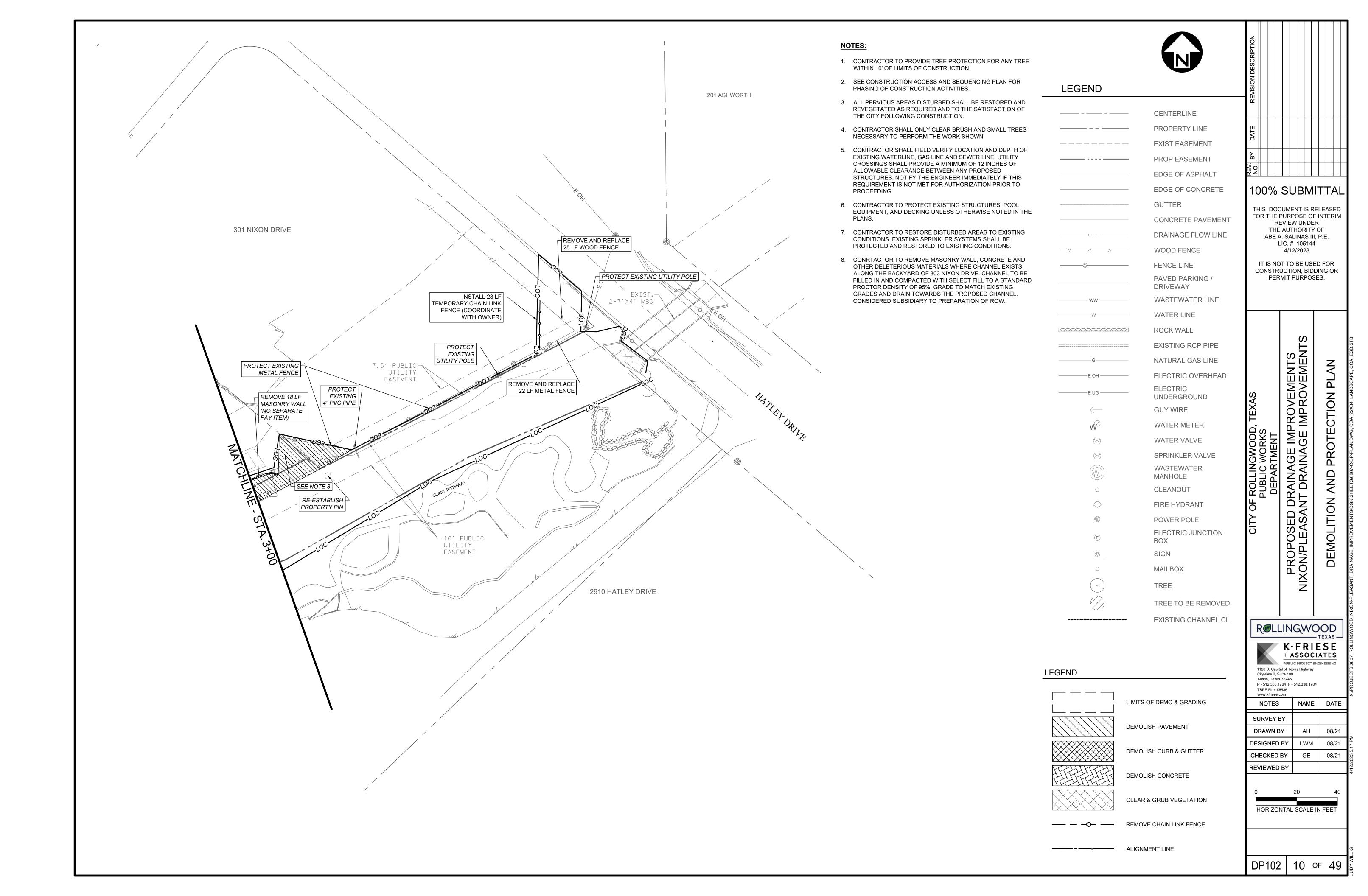
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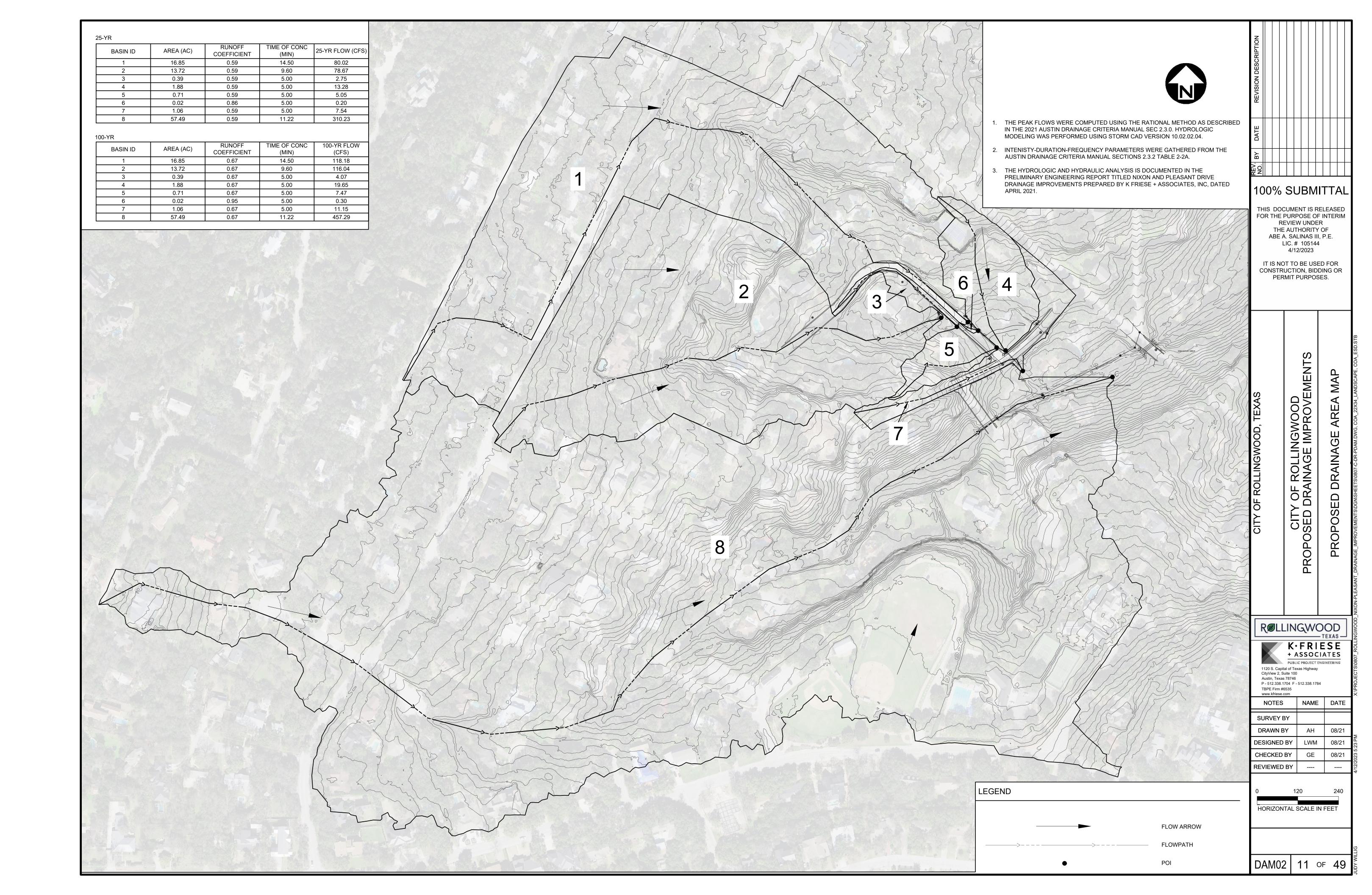








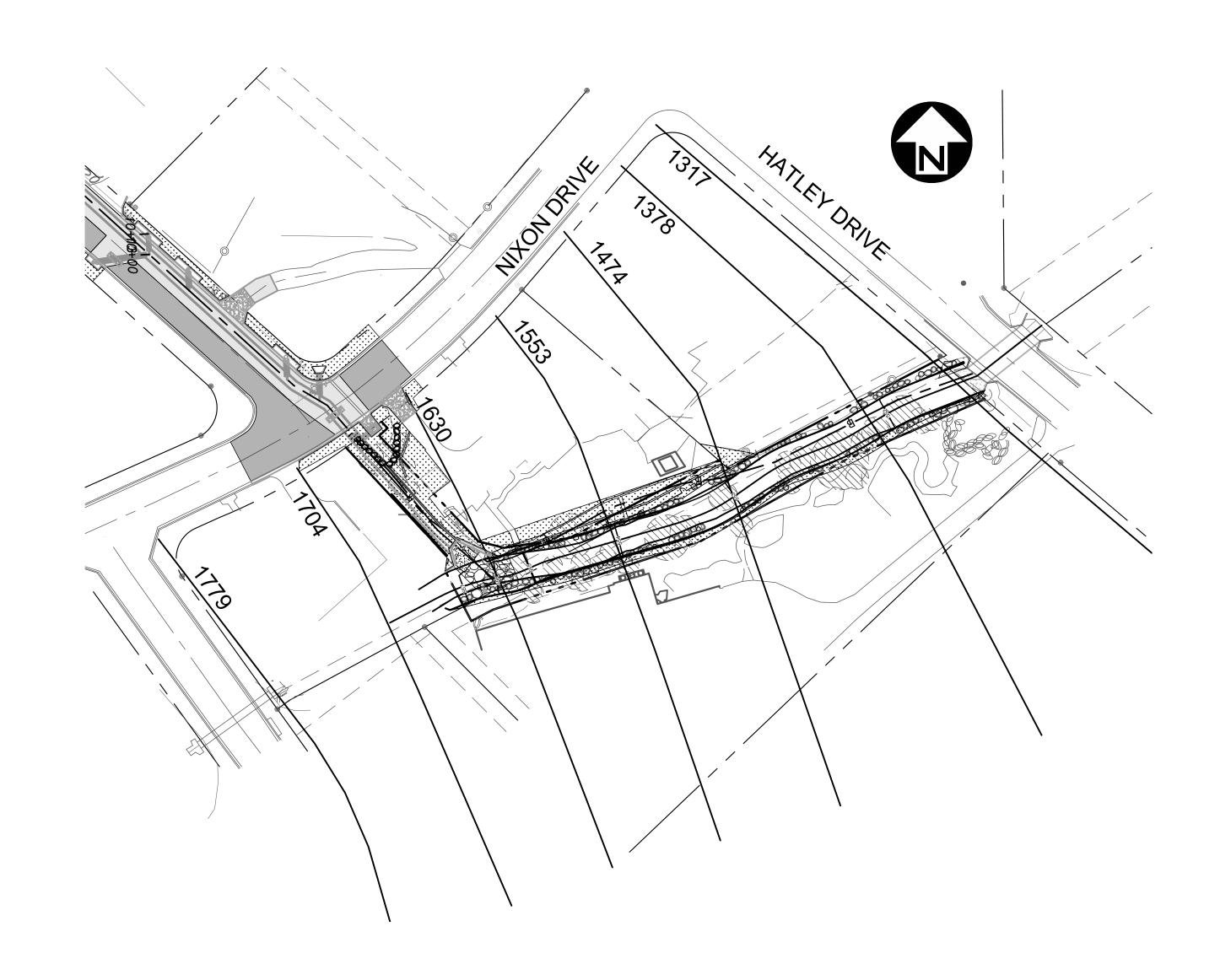




#### HEC-RAS CALCULATIONS

						EXISTING	G CONDITIC	ONS												PROPOSED	CONDITIO	NS					
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Ch Depth	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area T	Γop Width	Froude # Chl	Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Ch Depth	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # C
			(cfs)	(ft)	(ft)		(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	ft)					(cfs)	(ft)	(ft)		(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Reach 1	1779	25-year	444	561.95	565.37	3.42	565.37	566.27	0.011482	8.53	62.92 4	12.46	0.91	Reach 1	1779	25-year	444	561.95	565.37	3.42	565.37	566.27	0.011482	8.53	62.92	42.46	0.91
Reach 1	1779	100-year	600	561.95	565.91	3.96	565.91	566.73	0.008598	8.34	89.24 4	19.95	0.82	Reach 1	1779	100-year	600	561.95	565.91	3.96	565.91	566.73	0.008598	8.34	89.24	49.95	0.82
Reach 1	1704	25-year	444	559.06	562.27	3.21	562.27	563.24	0.000145	8.01	60.06 3	36.5	1	Reach 1	1704	25-year	444	559.06	562.27	3.21	562.27	563.24	0.000145	8.01	60.06	36.5	1
Reach 1	1704	100-year	600	559.06	562.70	3.64	562.7	563.87	0.000137	8.85	77.27 4	43.72	1	Reach 1	1704	100-year	600	559.06	562.7	3.64	562.7	563.87	0.000137	8.85	77.27	43.72	1
Reach 1	1630	25-year	783	556.62	560.04	3.42	560.04	560.86	0.013948	9.12	145.69 9	95.54	1.02	Reach 1	1630	25-year	783	555.64	559.45	3.81	559.45	560.39	0.007517	8.49	138.34	83.76	0.81
Reach 1	1630	100-year	1031	556.62	560.46	3.84	560.46	561.31	0.012422	9.54	187.56 1	103.63	0.99	Reach 1	1630	100-year	1031	555.64	559.88	4.24	559.88	560.93	0.007558	9.23	176.56	92.46	0.83
Reach 1	1553	25-year	783	554.39	559.09	4.7		559.36	0.002295	4.73	244.99 1	107.61	0.44	Reach 1	1553	25-year	783	554.15	559.06	4.91		559.33	0.001762	5.02	269.81	106.61	0.4
Reach 1	1553	100-year	1031	554.39	559.78	5.39		560.06	0.001989	4.95	326.82 1	128.31	0.42	Reach 1	1553	100-year	1031	554.15	559.75	5.60		560.03	0.0017	5.3	351.03	125.44	0.4
Reach 1	1474	25-year	783	552.93	559.12	6.19		559.21	0.000767	3.49	392.52 1	104.87	0.27	Reach 1	1474	25-year	783	553.03	559.1	6.07		559.21	0.000514	3.45	403.65	104.74	0.25
Reach 1	1474	100-year	1031	552.93	559.80	6.87		559.92	0.00081	3.89	465.37 1	108.75	0.28	Reach 1	1474	100-year	1031	553.03	559.77	6.74		559.92	0.000566	3.89	476	108.62	0.27
Reach 1	1378	25-year	783	552.24	559.13	6.89		559.17	0.000155	1.8	671.8 1	L40.45	0.13	Reach 1	1378	25-year	783	552.06	559.13	7.07		559.16	0.000154	2.13	682.52	140.36	0.14
Reach 1	1378	100-year	1031	552.24	559.82	7.58		559.86	0.000183	2.09	770.13 1	L46.84	0.14	Reach 1	1378	100-year	1031	552.06	559.81	7.75		559.86	0.000184	2.49	780.58	146.74	0.16
Reach 1	1317	25-year	783	551.58	559.00	7.42	555.82	559.14	0.000853	3.16	304.36 1	108.77	0.27	Reach 1	1317	25-year	783	551.75	559.02	7.27	555.14	559.14	0.000505	2.84	330.96	108.99	0.24
Reach 1	1317	100-year	1031	551.58	559.66	8.08	556.54	559.83	0.000842	3.46	380.2 1	118.31	0.28	Reach 1	1317	100-year	1031	551.75	559.69	7.94	555.79	559.83	0.000519	3.17	406.94	118.58	0.25
Reach 1	1284		Culvert											Reach 1	1284		Culvert										

NOTE: THE EXISTING 2-7'x4' MULTIPLE BOX CULVERT AT HATLEY DRIVE IS CURRENTLY UNDERSIZED RESULTING IN A BACKWATER CONDITION THROUGH THE CHANNEL. THEREFORE, CHANNEL IMPROVEMENTS CONTAINED IN THIS PLAN SET PRODUCE A NEGLIGIBLE IMPACT AT THE UPSTREAM FACE OF CULVERT DUE TO THE EXISTING CULVERT CAPACITY. A FUTURE PROJECT MAY IMPROVE THE DOWNSTREAM CULVERT SIZE WHICH WOULD IMPROVE THE CHANNEL TAILWATER CONDITION AND LOWER WSELs WITHIN THE CHANNEL. SEE THE PRELIMINARY ENGINEERING REPORT FOR NIXON AND PLEASANT DRIVE DRAINAGE IMPROVEMENTS PREPARED BY K. FRIESE + ASSOCIATES FOR MORE INFORMATION.



REVISION DESCRIPTION						
DATE						
ВУ						
REV. BY NO.						

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# IMPROVEMENTS GE IMPROVEMENT

## CALCULATIONS DRAINAGE INT DRAINAG PROPOSED I

R@LLINGWOOD TEXAS —

K.FRIESE + ASSOCIATES 1120 S. Capital of Texas Highway
CityView 2, Suite 100
Austin, Texas 78746
P - 512.338.1704 F - 512.338.1784
TBPE Firm #6535
www.kfriese.com

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NOTES	NAME	DATE
SURVEY BY		
DRAWN BY	АН	08/21
DESIGNED BY	LWM	08/21
CHECKED BY	GE	08/21
REVIEWED BY	PS	09/21

HORIZONTAL SCALE IN FEET

CH01 12 OF 49

#### 25-YR CALCULATIONS

Label	Туре	Profile Type	Curb Opening Length (ft)	Longitudinal Slope (Inlet) (ft/ft)	Road Cross Slope (ft/ft)	Spread Manning's N	Depression (ft)		Flow (Total Bypass to Inlet) (cfs)	Flow (Captured) (cfs)	Flow (Bypass) (cfs)	Bypass To	Spread / Top Width (ft)	Depth (Gutter) (in)
Area Inlet A1	Area	On Grade	4	0.017	0.02	0.013	0	7.5	26.6	34.1	0	Outfall	22.7	5.4
Curb Inlet A2	Curb	On Grade	20	0.012	0.015	0.013	0	5.1	29.1	19.6	14.6	A1	29	5.2
Curb Inlet A3	Curb	On Grade	20	0.021	0.02	0.013	0	0.2	53.7	24.8	29.1	A2	25.9	6.2
Curb Inlet A4	Curb	On Grade	10	0.03	0.019	0.013	0	2.8	16.3	7.4	11.7	A1	16.9	3.9
Curb Inlet A5	Curb	On Grade	20	0.019	0.015	0.013	0	80	0	27.7	53.7	A3	36.8	6.6

	Elevation (Invert) (ft)	Rise (ft)	Span (ft)	Flow (Captured) (cfs)	Flow (Bypass) (cfs)	Inlet Depth (ft)
Headwall A2	563.5	2	2	13.3	0	2
Headwall A3	571	2	5	62.3	16.3	3

#### 25-YR CALCULATIONS

Link Start Node	Invert (Start) (ft)	Link Stop Node	Invert (Stop) (ft)	Depth (In) (ft)	Hydraulic Grade Line (In) (ft)	Loss (ft)	Depth (Out) (ft)	Hydraulic Grade Line (Out) (ft)	Velocity (ft/s)	Diameter (in)	Rise (ft)	Span (ft)	Length (ft)	Slope (Calculated) (ft/ft)	Friction Slope (ft/ft)	Manning's N Value	Flow (cfs)	Capacity (Design) (cfs)
Headwall A-3	571	JPB	568.7	1.7	572.7	0.91	0.88	569.5	16.8		2	5	52.3	0.045	0.017	0.013	62.3	194.5
Inlet A5	564.4	Junction A5	563.2	1.8	566.2	0.83	1.34	564.6	14.9	24			29.9	0.038	0.028	0.013	27.7	44.4
Inlet A4	566.8	Junction A5	564.1	1	567.7	0.89	1.56	565.6	13.2	24			37.3	0.074	0.026	0.013	7.4	61.3
Inlet A3	563.9	Junction A4	563.2	1.8	565.7	0.66	1.3	564.5	16.3	24			13.2	0.052	0.05	0.013	24.8	51.7
Inlet A2	562	Junction A3	561.3	2	564	0.1	2.63	563.9	15.7	24			14	0.056	0.007	0.013	19.6	53.5
Headwall A2	561.3	Junction A2	560.7	2.4	563.7	0.07	2.95	563.6	4.2	24			20.3	0.031	0.003	0.013	13.3	40.1
Area Inlet	562	Junction A1	561.3	2	564	0.36	1.69	563	18.5	24			15	0.054	0.029	0.013	40.6	52.8
JPB	563.7	Junction STA. 11+72.22	560.3	1.7	565.4	0.08	2.95	563.5	16.9		4	5	200	0.017	0.001	0.013	127	377.3
Junction STA. 11+72.22	560.3	Junction STA. 11+48.12	559.9	2.7	564.4	0.05	3.43	562.7	16.9		4	5	24.11	0.017	0.003	0.013	136	377.3
Junction STA. 11+48.12	559.9	Junction STA 11+11.85	558.8	2.8	562.7	0.22	3.59	562.3	19.3		4	5	36.27	0.017	0.006	0.013	136	442.1
Junction STA 11+11.85	558.8	Junction STA 10+45.26	556.5	3.4	562.1	2.17	3.97	560.4	21.2		4	5	66.51	0.042	0.033	0.013	174.9	455.8
Junction STA 10+45.26	556.5	Headwall A1	556.4	3.4	559.8	0.07	3.13	559.5	12.3		4	5	12.72	0.008	0.006	0.013	174.8	217.4

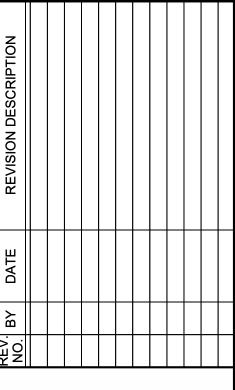
#### 100-YR CALCULATIONS

Label	Туре	Profile Type	Curb Opening Length (ft)	Longitudinal Slope (Inlet) (ft/ft)	Road Cross Slope (ft/ft)	Spread Manning's N	Depression (ft)	100-Yr Discharge (cfs)	Flow (Total Bypassed to Inlet) (cfs)	Flow (Captured) (cfs)	Flow (Bypass) (cfs)	Bypass To	Spread / Top Width (ft)	Depth (Gutter) (in)
Area Inlet A1	Area	On Grade	4	0.017	0.02	0.013	0	11.2	74.1	50	35.2	Outfall	32	7.7
Curb Inlet A2	Curb	On Grade	20	0.012	0.015	0.013	0	7.5	55.6	26.8	36.2	A1	36.5	6.6
Curb Inlet A3	Curb	On Grade	20	0.021	0.02	0.013	0	0.3	86.8	31.5	55.6	A2	31	7.4
Curb Inlet A4	Curb	On Grade	10	0.03	0.019	0.013	0	4.1	44.6	11.5	37.2	A1	24.1	5.5
Curb Inlet A5	Curb	On Grade	20	0.019	0.015	0.013	0	120.2	0	33.4	86.8	А3	42.6	7.7

	Elevation (Invert) (ft)	Rise (ft)	Span (ft)	Flow (Captured) (cfs)	Flow (Bypass) (cfs)	Inlet Depth (ft)
Headwall A2	563.5	2	2	19.65	0	3.3
Headwall A3	571	2	5	71.4	44.59	3.53

#### 100-YR CALCULATIONS

Link Start Node	Invert (Start) (ft)	Link Stop Node	Invert (Stop) (ft)	Depth (In) (ft)	Hydraulic Grade Line (In) (ft)	Loss (ft)	Depth (Out) (ft)	Hydraulic Grade Line (Out) (ft)	Velocity (ft/s)	Diameter (in)	Rise (ft)	Span (ft)	Length (ft)	Slope (Calculated) (ft/ft)	Friction Slope (ft/ft)	Manning's N Value	Flow (cfs)	Capacity (Design) (cfs)
Headwall A-3	571	JPB	568.7	1.9	572.9	0.87	0.98	569.6	17.6		2	5	52.3	0.045	0.017	0.013	71.4	194.5
Inlet A5	564.4	Junction A5	563.2	1.9	566.3	0.91	1.51	564.7	15.5	24			29.9	0.038	0.03	0.013	33.4	44.4
Inlet A4	566.8	Junction A5	564.1	1.2	568	0.75	1.85	566	15	24			37.3	0.074	0.022	0.013	11.5	61.3
Inlet A3	563.9	Junction A4	563.2	1.9	656.8	0.27	2.08	565.3	17.3	24			13.2	0.052	0.02	0.013	31.5	51.7
Inlet A2	562	Junction A3	561.3	2.2	564.2	0.2	2.74	564	8.5	24			14	0.056	0.014	0.013	26.8	53.5
Headwall A2	561.3	Junction A2	560.7	2.2	563.5	0.15	2.65	563.3	6.3	24			20.3	0.031	0.008	0.013	19.7	40.1
Area Inlet	562	Junction A1	561.3	2.2	564.2	0.85	1.99	563.3	18.9	24			15	0.054	0.069	0.013	59.5	52.8
JPB	563.7	Junction STA. 11+72.22	560.3	1.9	565.6	0.06	3.67	564	10.6		4	5	200	0.017	0.001	0.013	155.4	377.3
Junction STA. 11+72.22	560.3	Junction STA. 11+48.12	559.9	3.1	563.4	0.07	3.32	563.2	15.8		4	5	24.1	0.017	0.004	0.013	168.6	377.3
Junction STA. 11+48.12	559.9	Junction STA 11+11.85	558.8	3.3	563.2	0.16	4.09	563.5	17.7		4	5	36.3	0.017	0.005	0.013	168.6	359.1
Junction STA 11+11.85	558.8	Junction STA 10+45.26	556.5	4	562.7	2.14	4.16	560.6	22.7		4	5	66.5	0.042	0.032	0.013	225.7	455.8
Junction STA 10+45.26	556.5	Headwall A1	556.4	4	560.4	0.1	3.98	560.3	11.3		4	5	12.7	0.008	0.008	0.013	225.5	217.4



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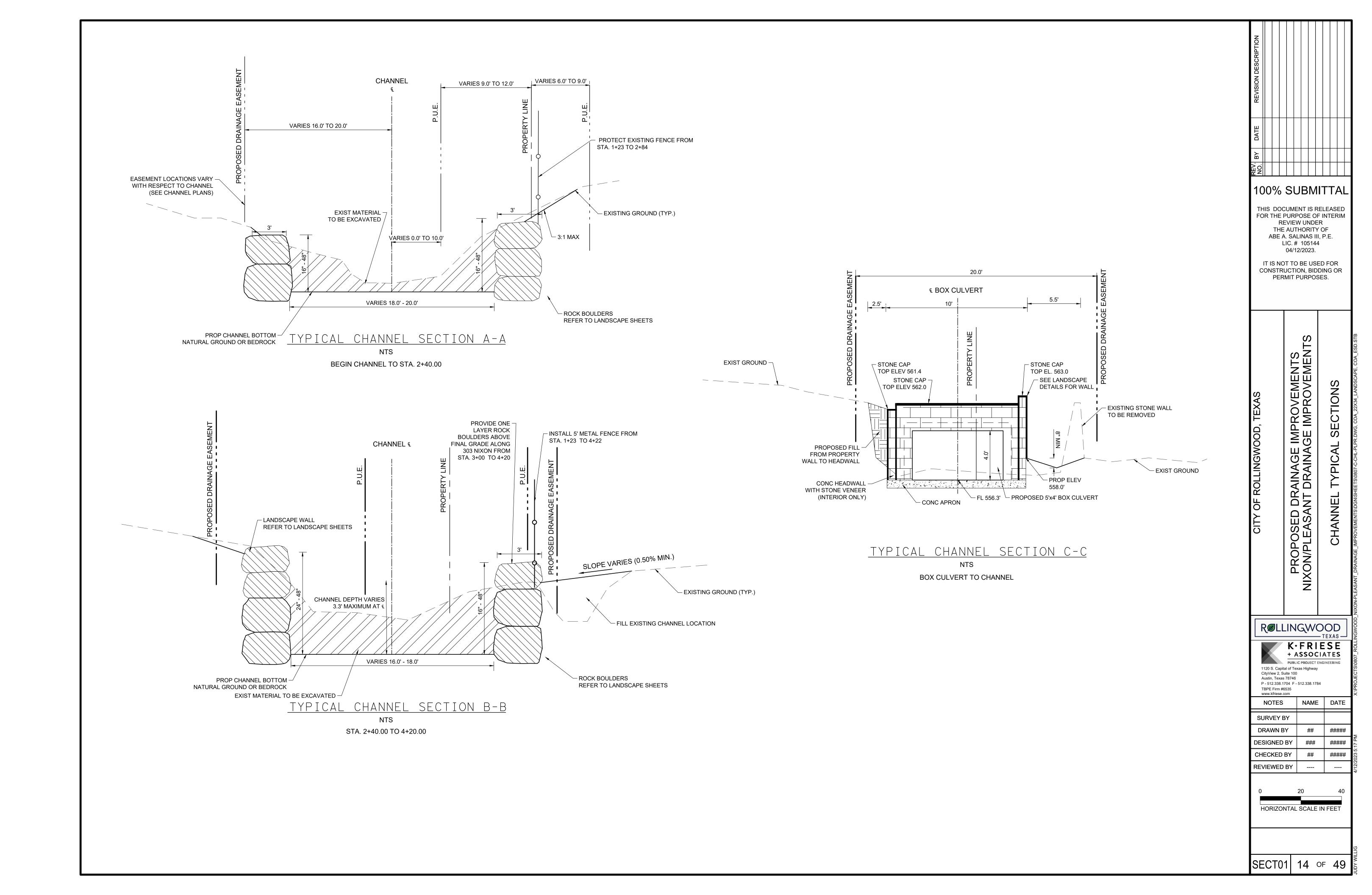
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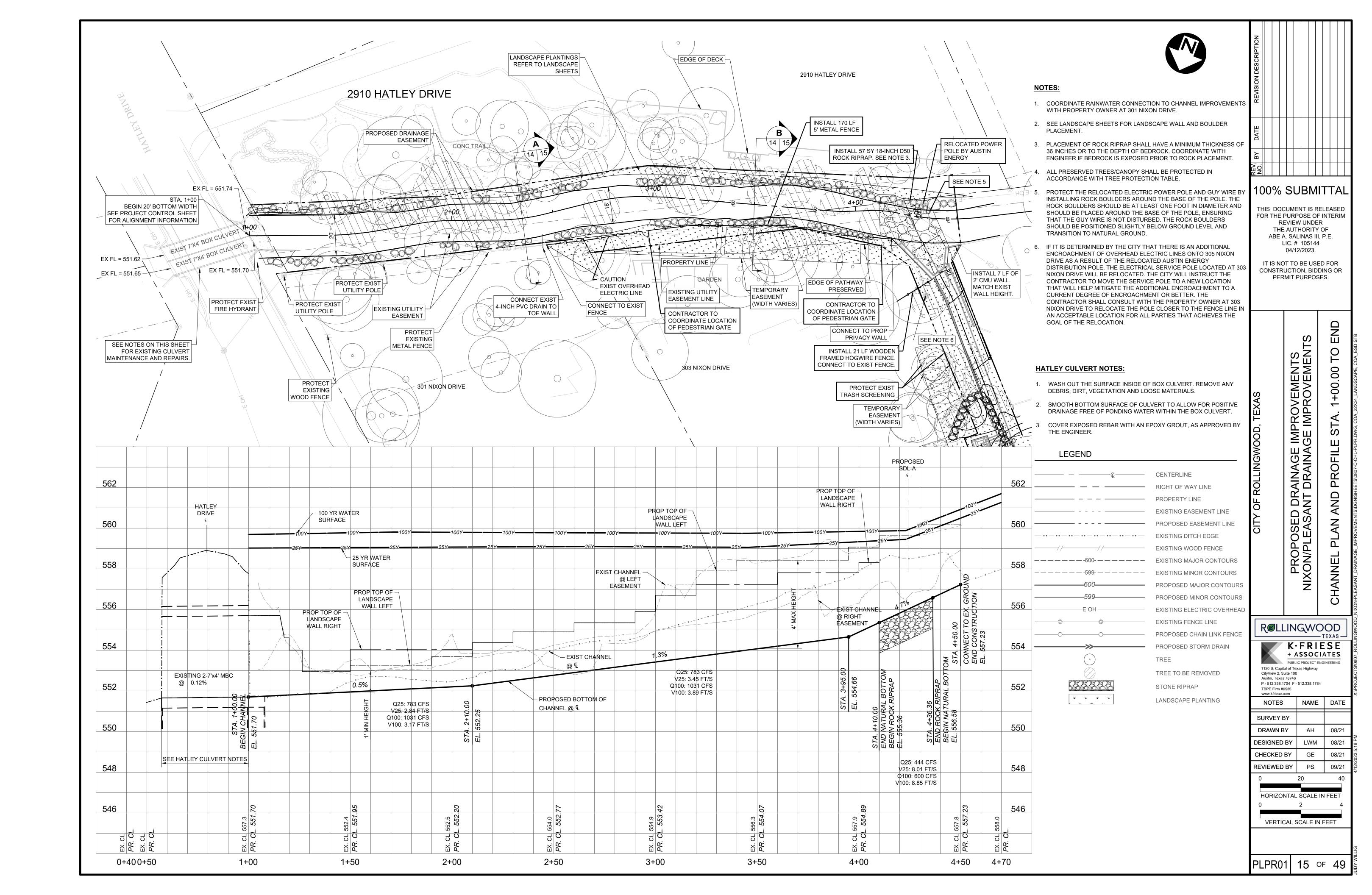
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+ ASSOCIATES
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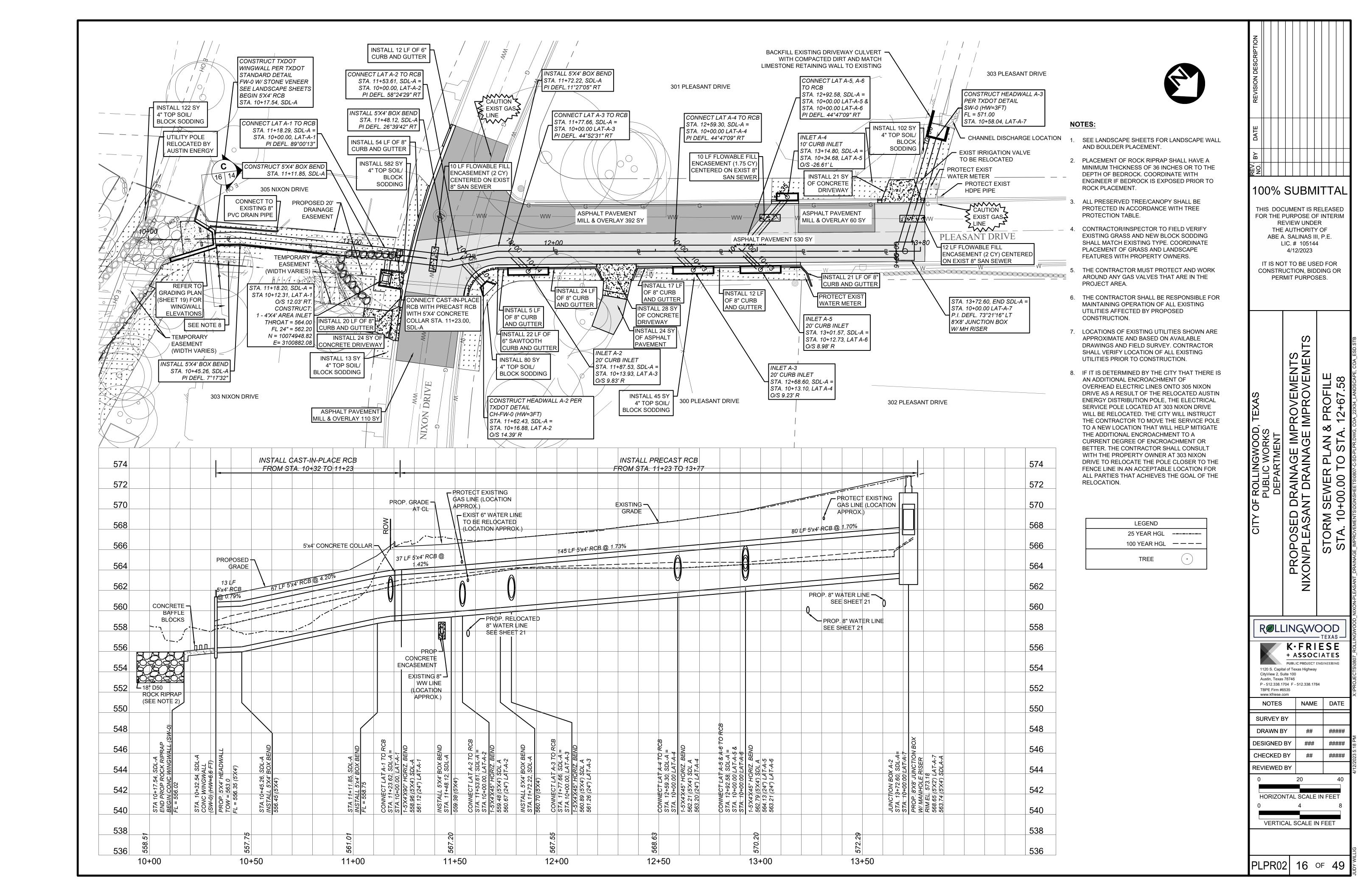
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CityView 2, Suite 100
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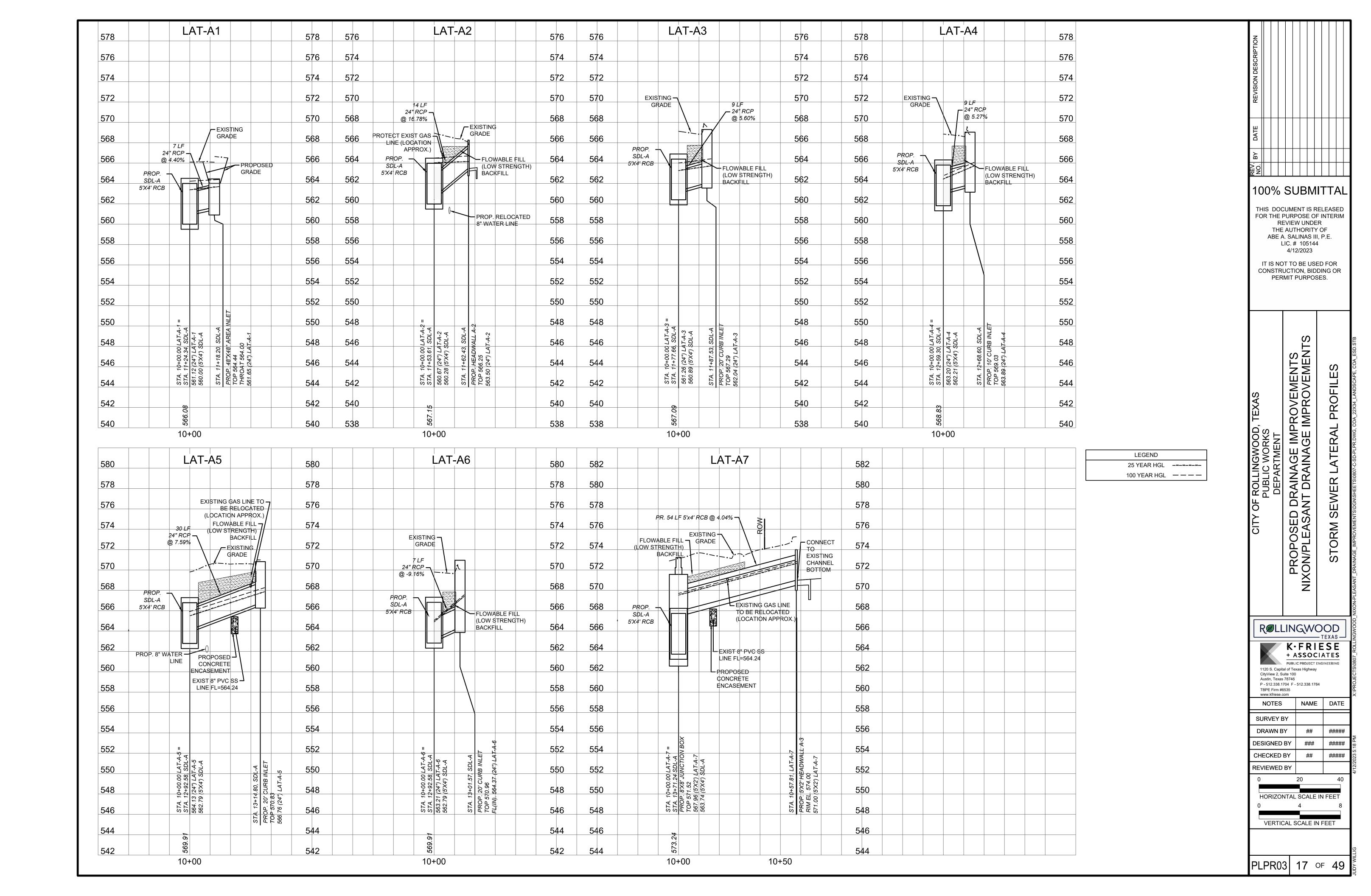
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NOTES	NAME	DA				
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DRAWN BY	AH	08				

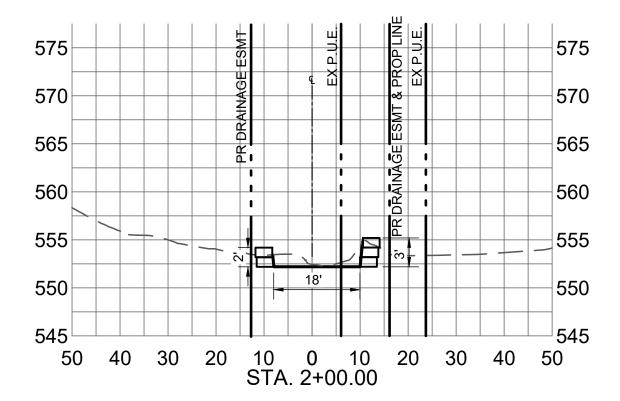
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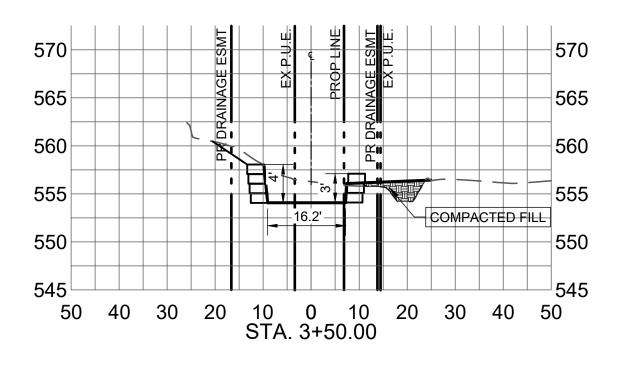


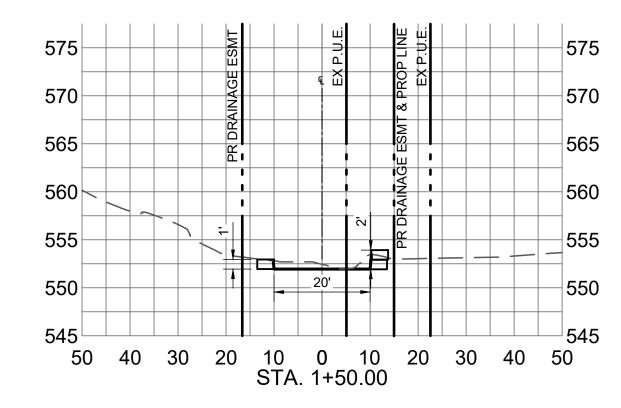


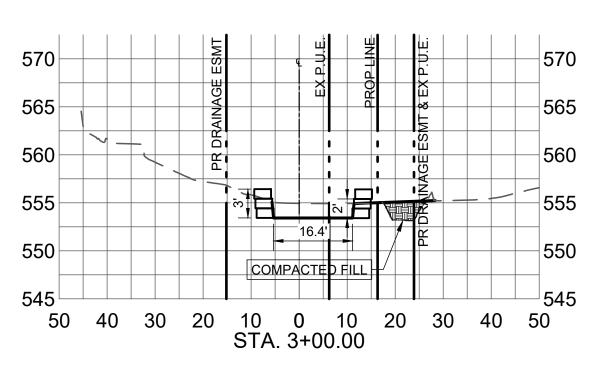


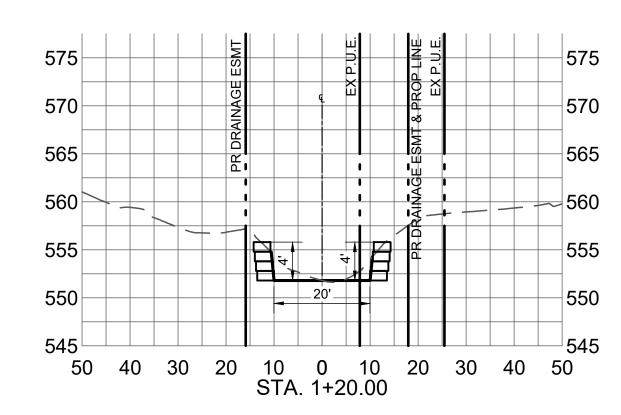


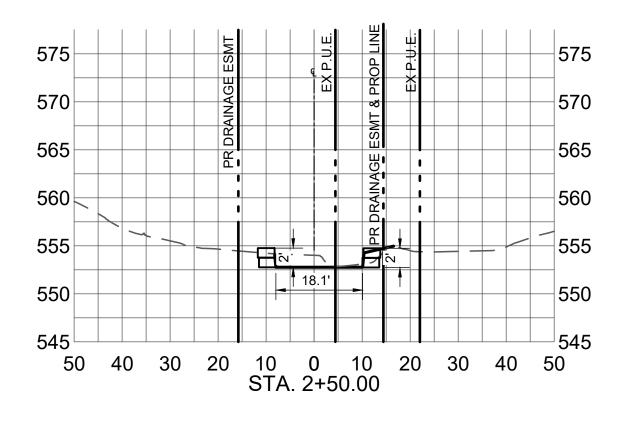






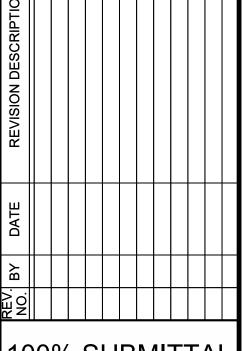






#### NOTES:

- 1. CROSS-SECTIONS ARE SHOWN ALONG THE BASELINE LOOKING UPSTREAM.
- 2. EXISTING GROUND FOR CROSS-SECTIONS WERE GENERATED FROM CIVILCORP SURVEY DATA.
- 3. ELEVATION CALLOUTS AT THE LIMITS OF THE EXCAVATION ARE TO THE ACCURACY OF THE SURVEY DATA. CONTRACTOR TO HOLD THE OFFSET IN THE EVENT OF A DISCREPANCY.
- 4. APPARENT ROW OR EASEMENT LIMITS ARE SHOWN FOR INFORMATIONAL PURPOSES AND SHALL BE FIELD VERIFIED BY THE CONTRACTOR.



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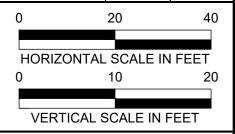
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## R@LLINGWOOD\_

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NOTES	NAME	DATE
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DRAWN BY	##	#####
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REVIEWED BY		



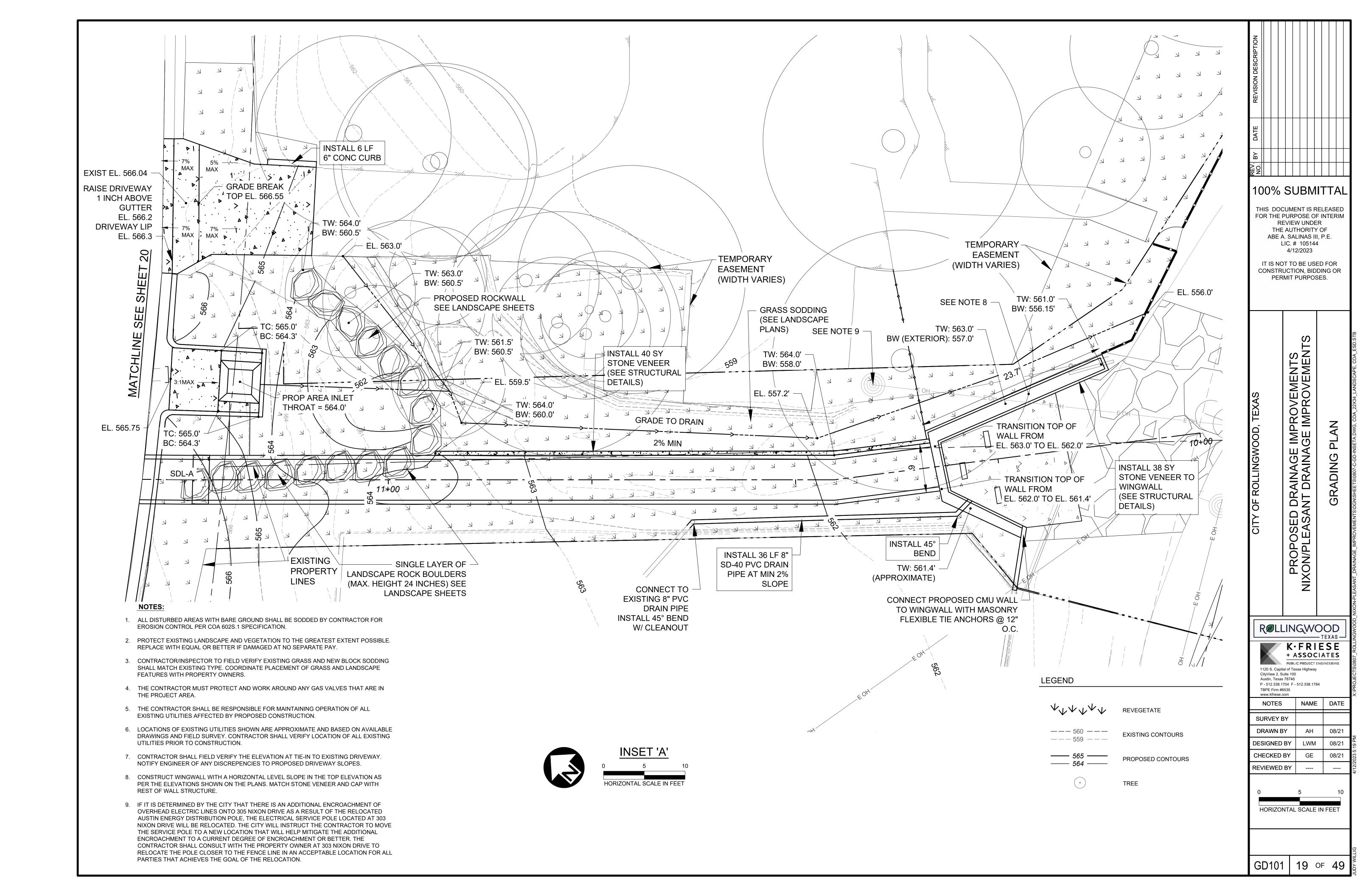
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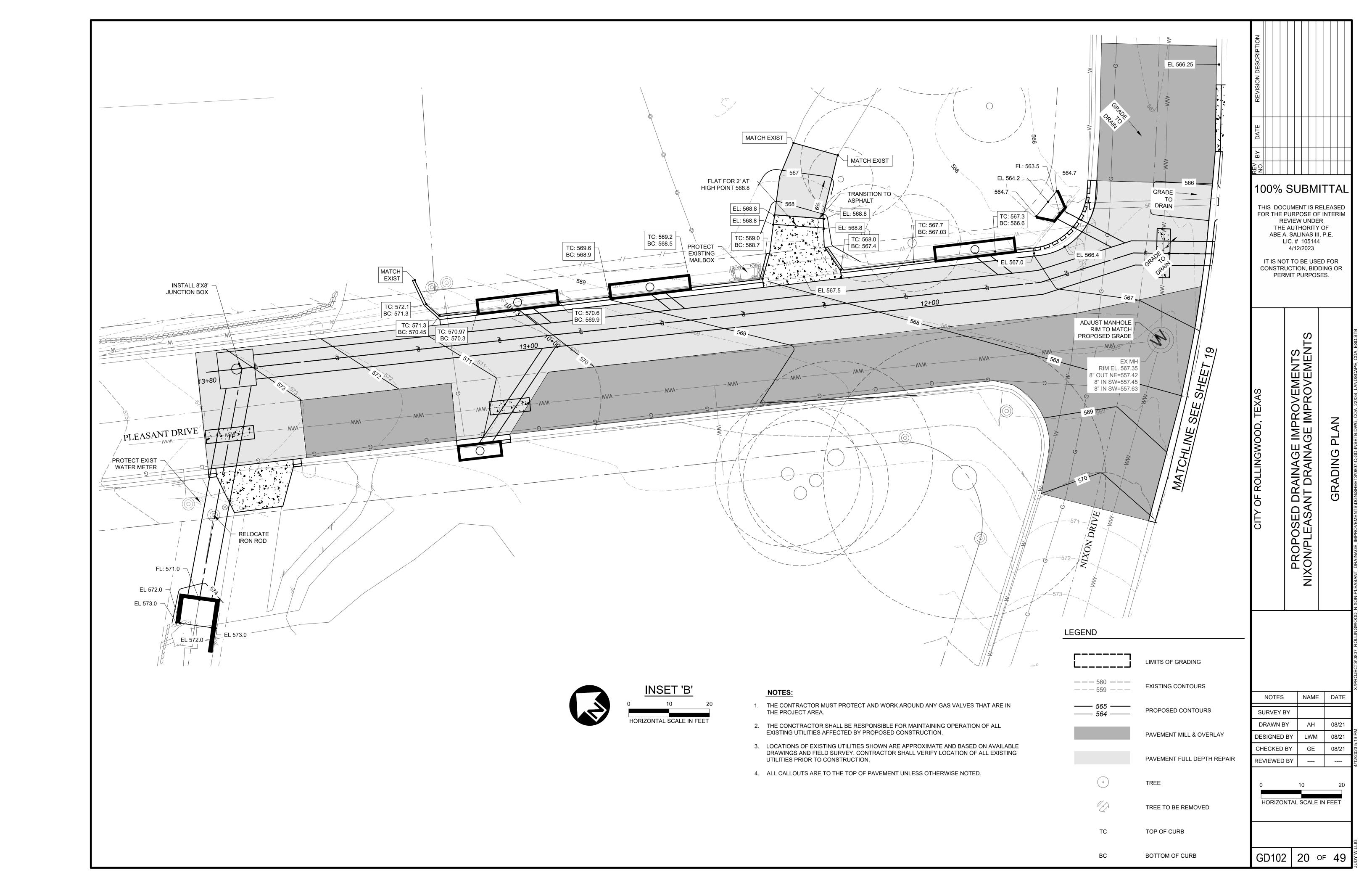
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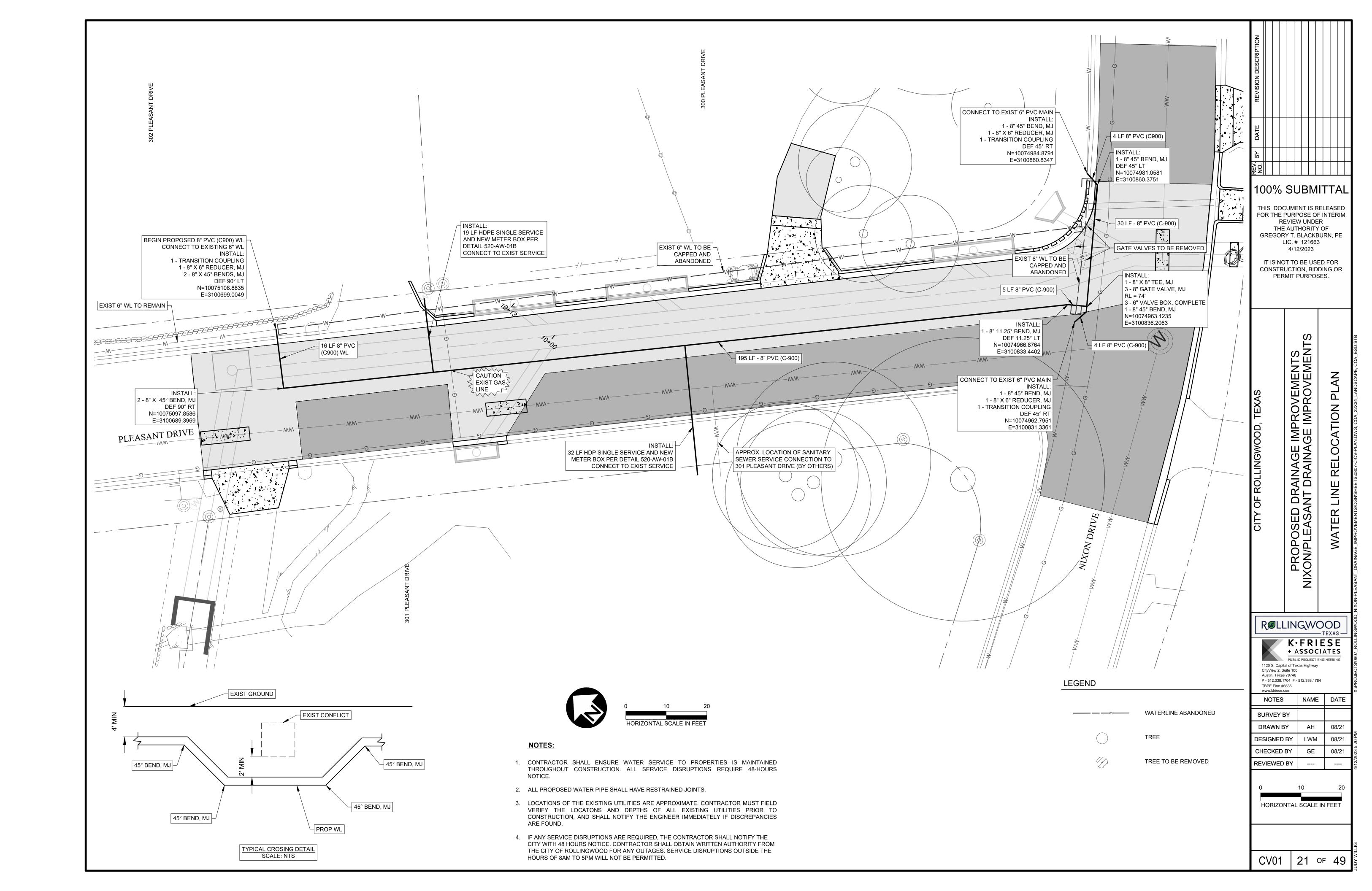
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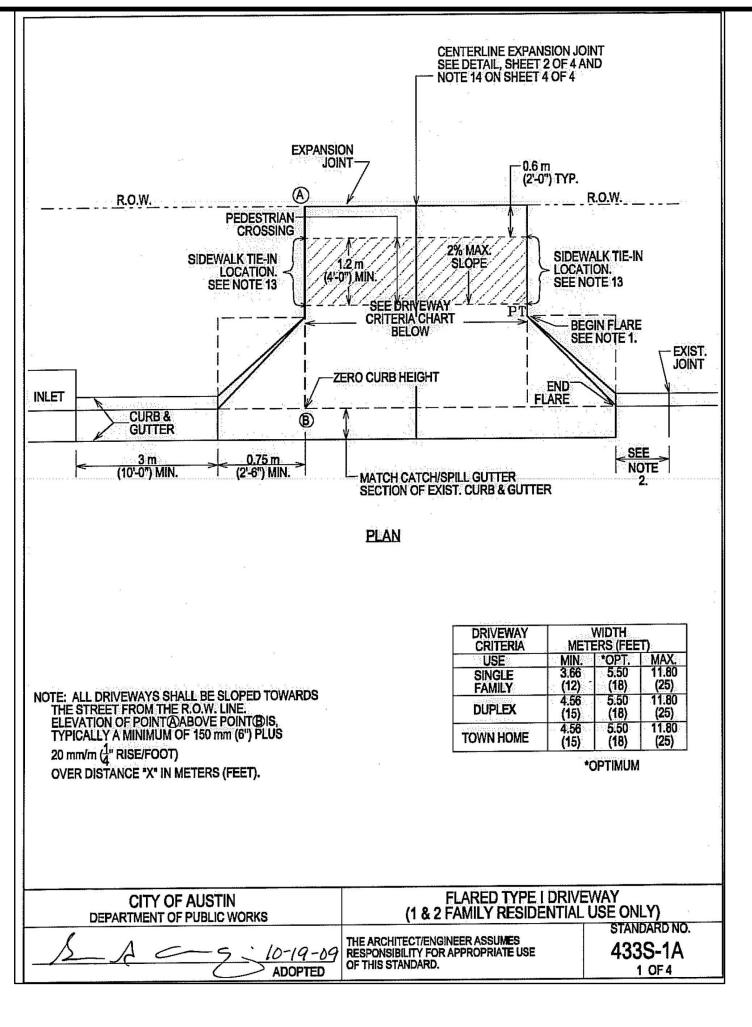
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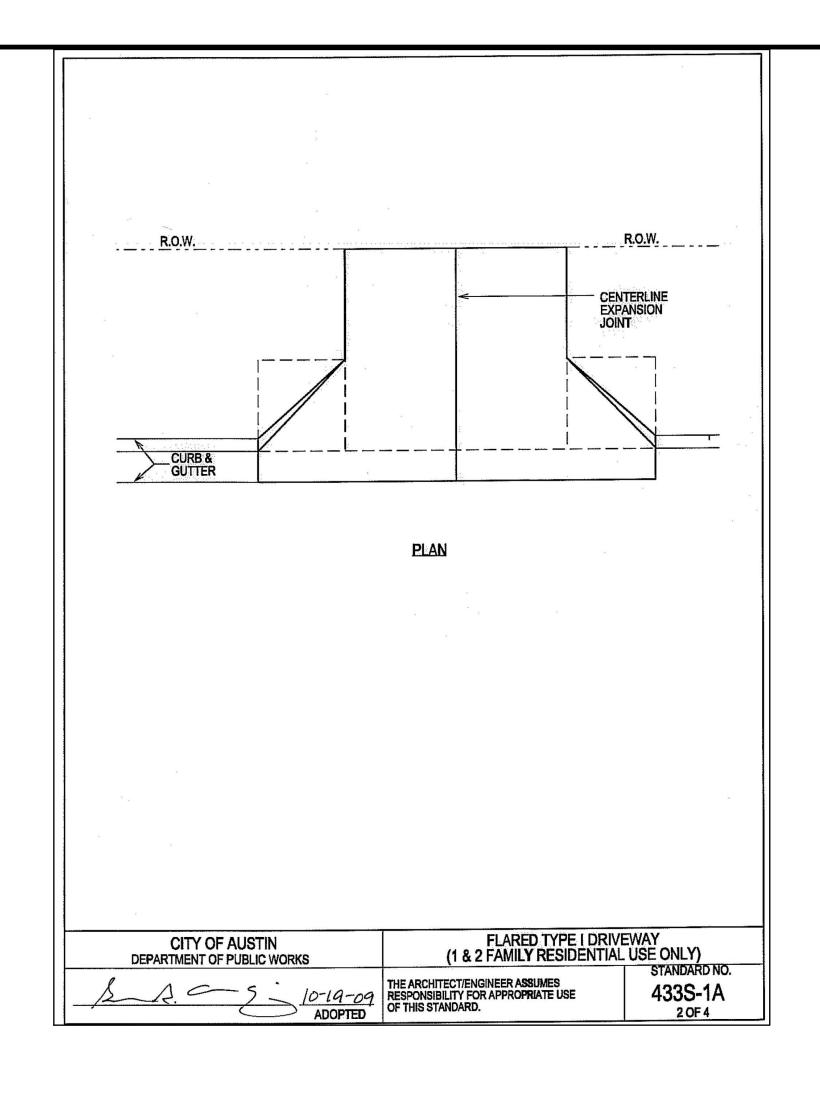
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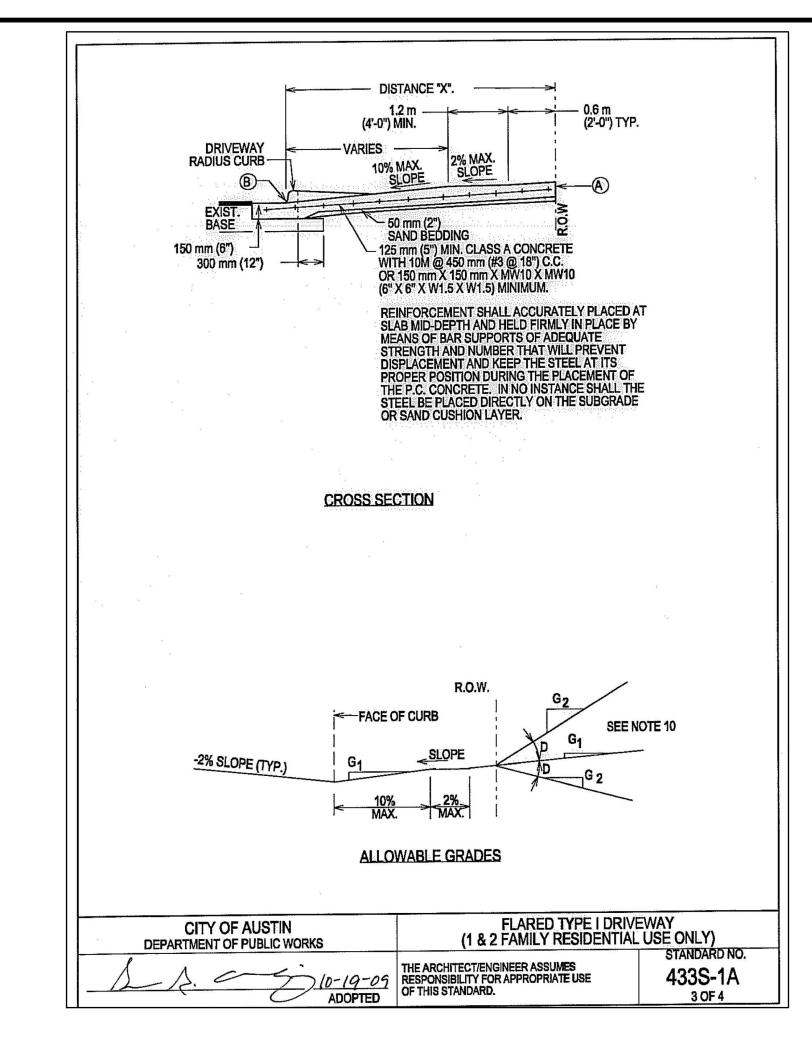


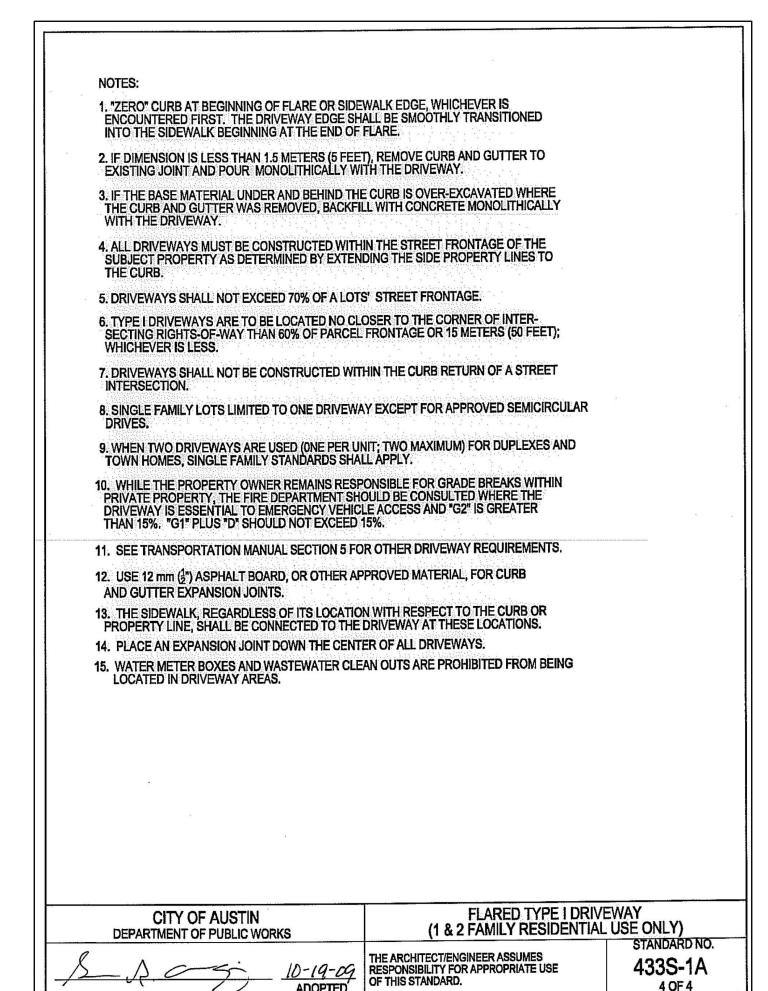






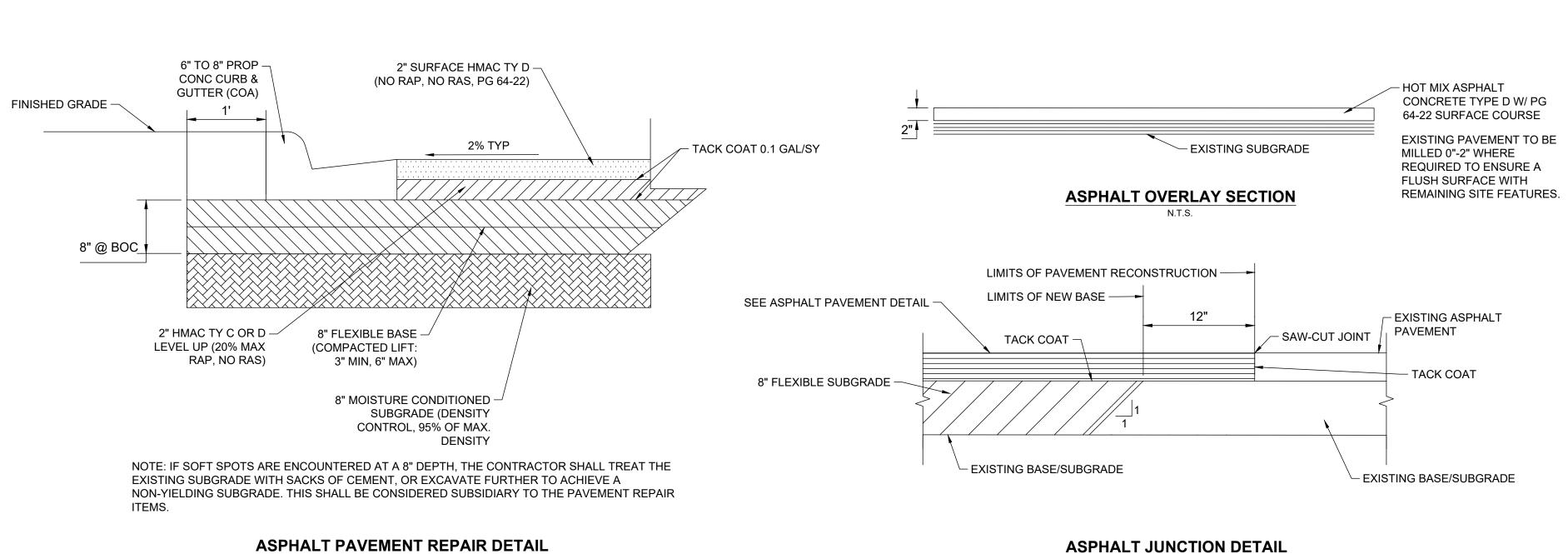






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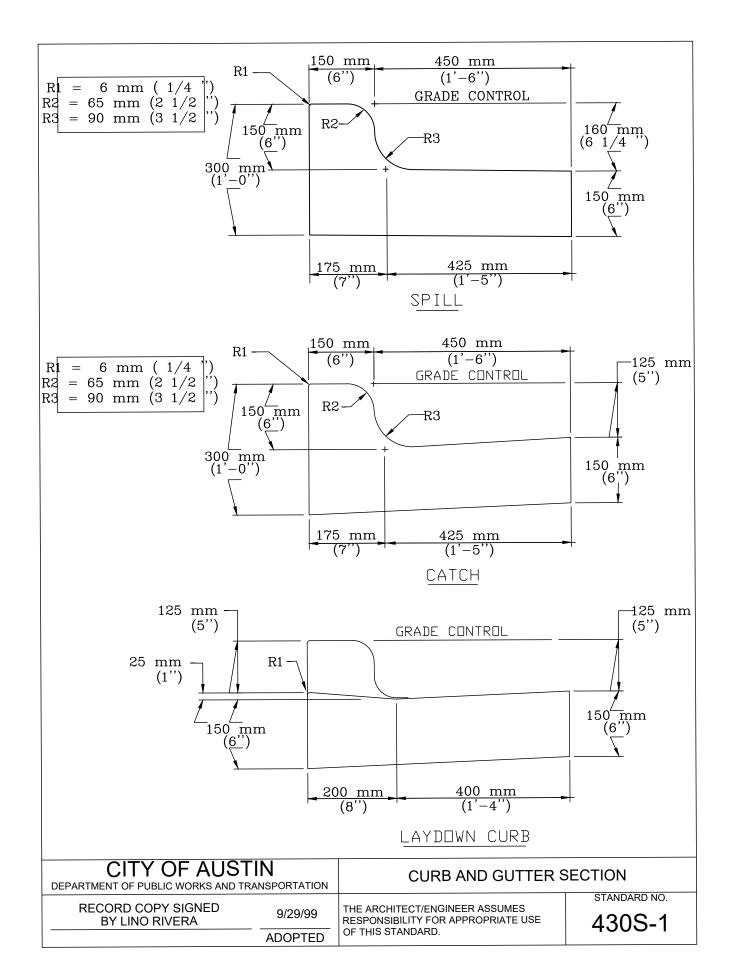
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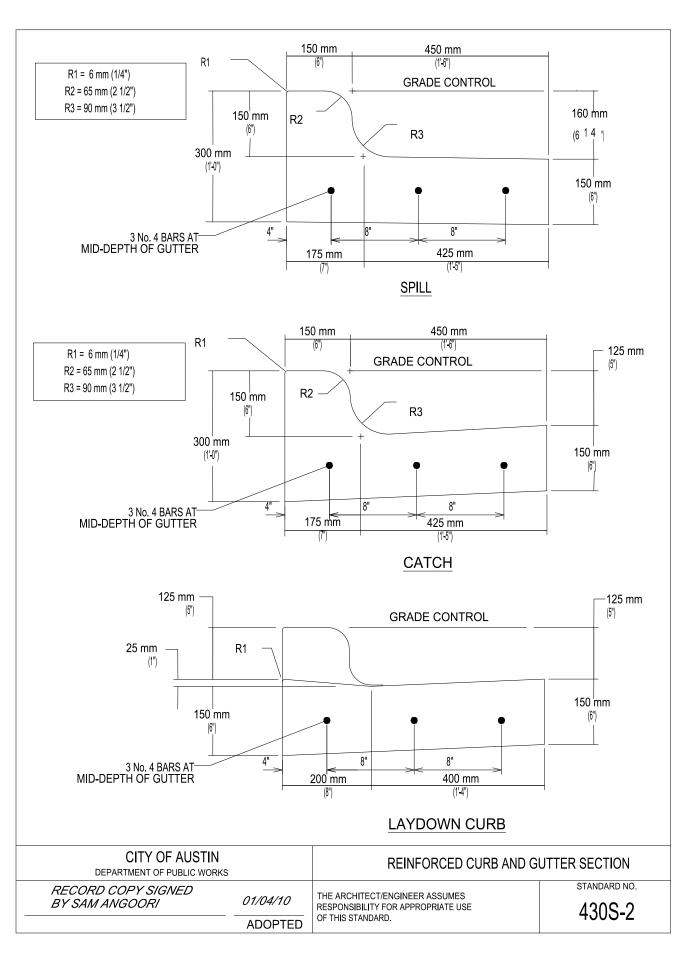
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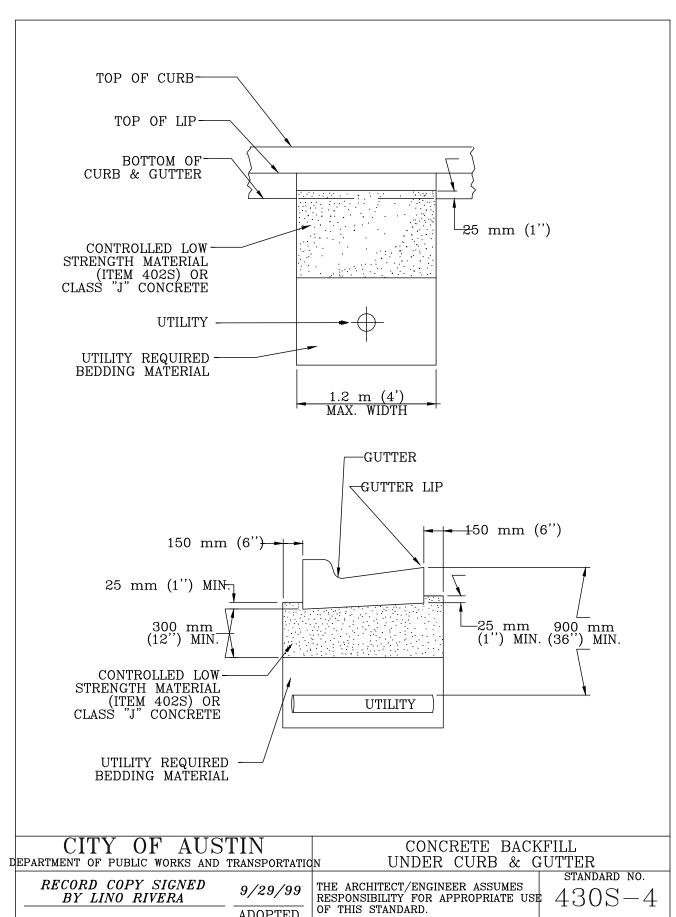
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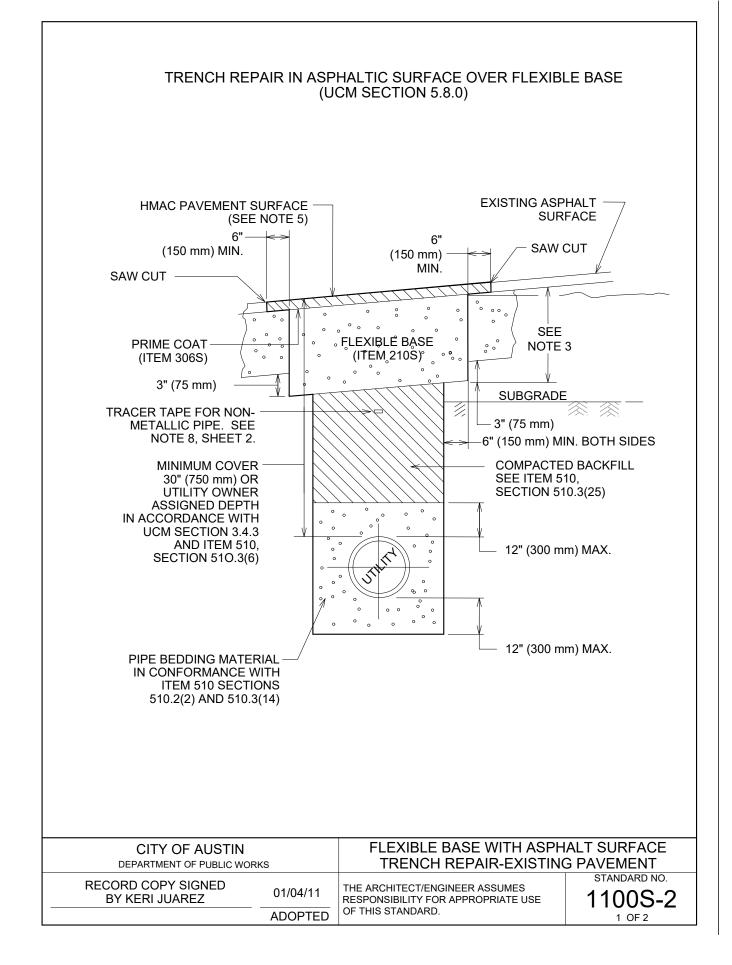
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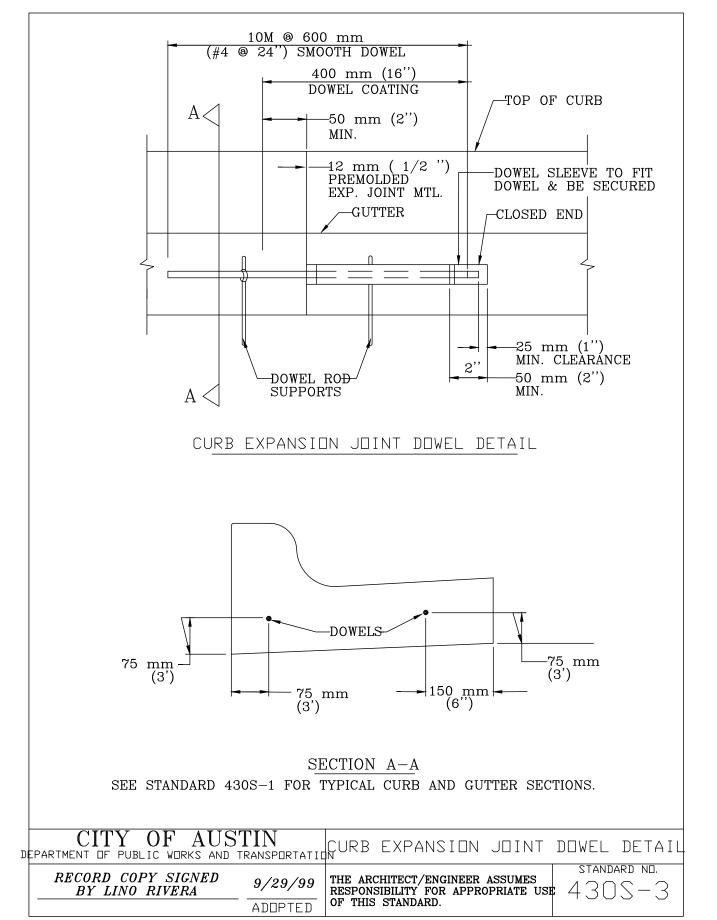


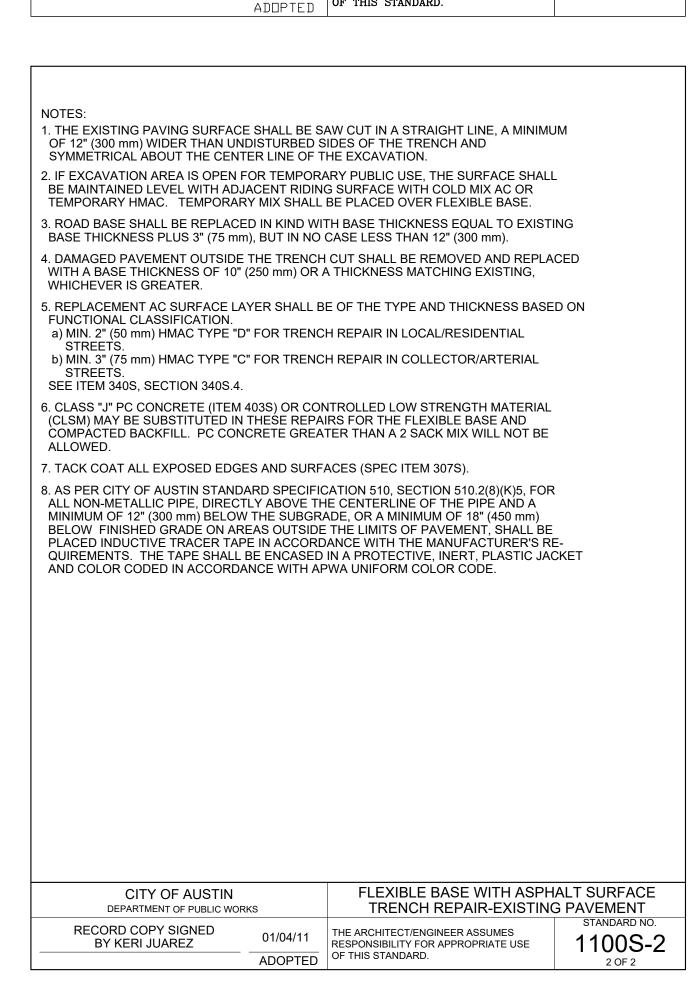


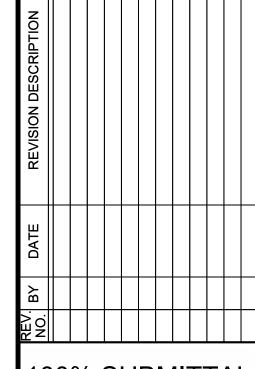


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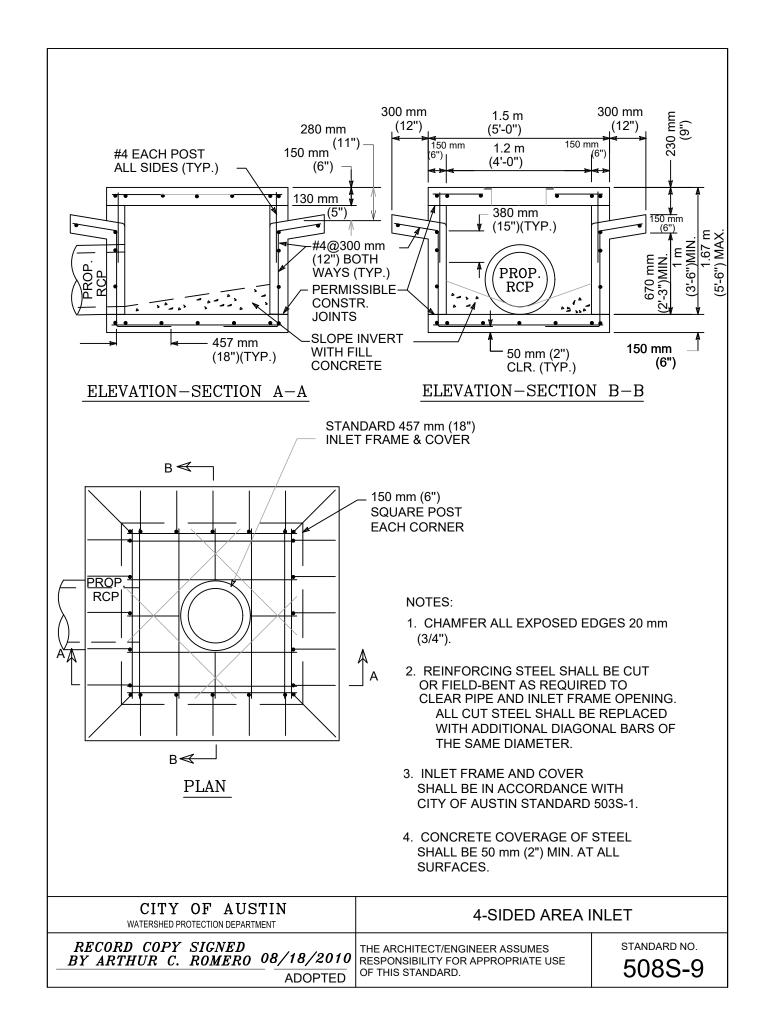
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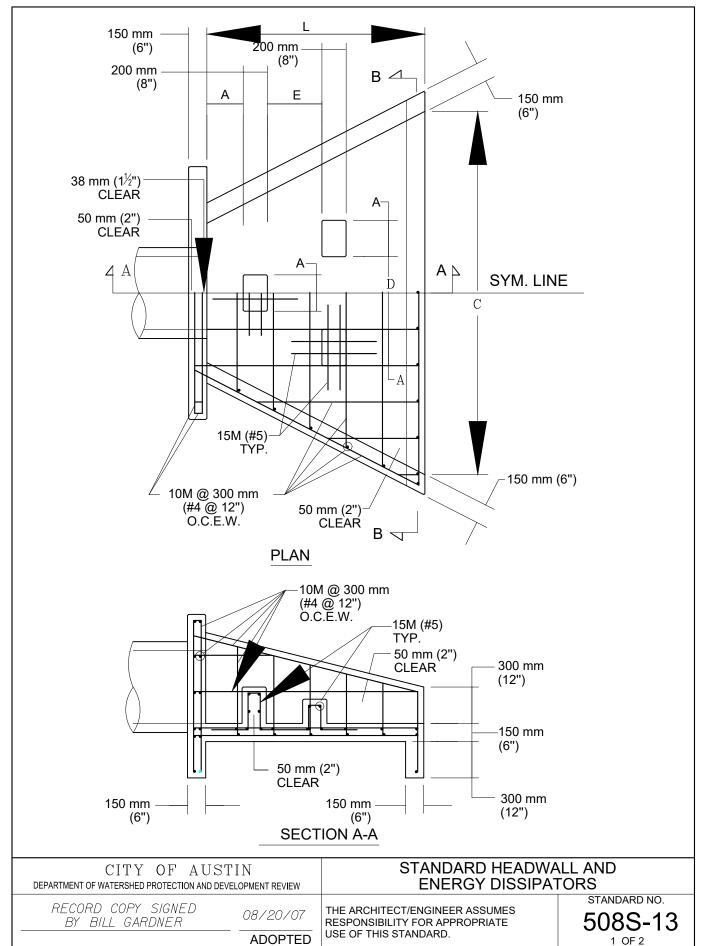
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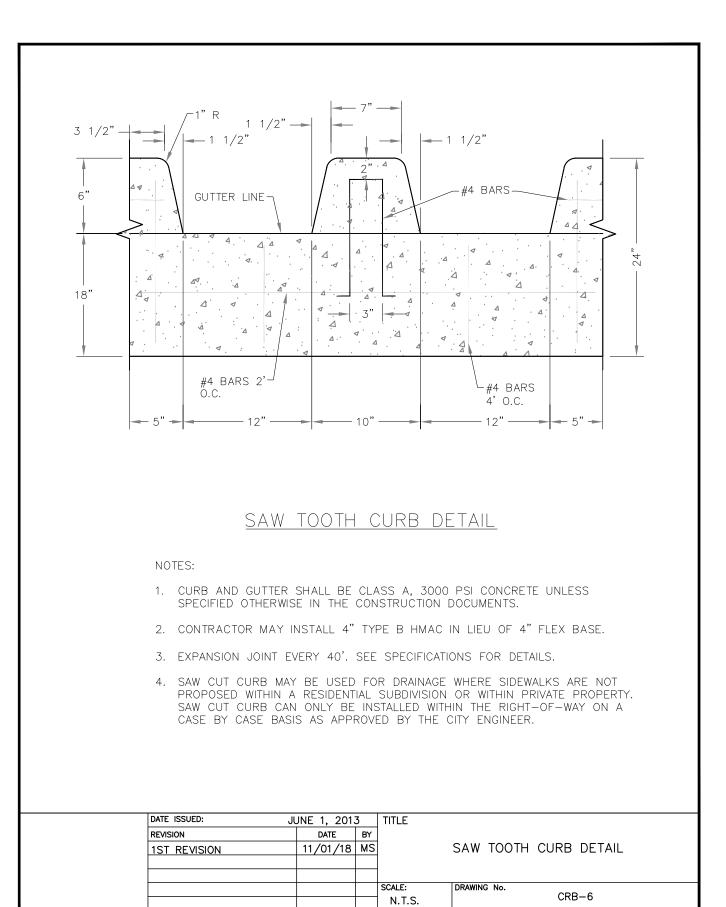
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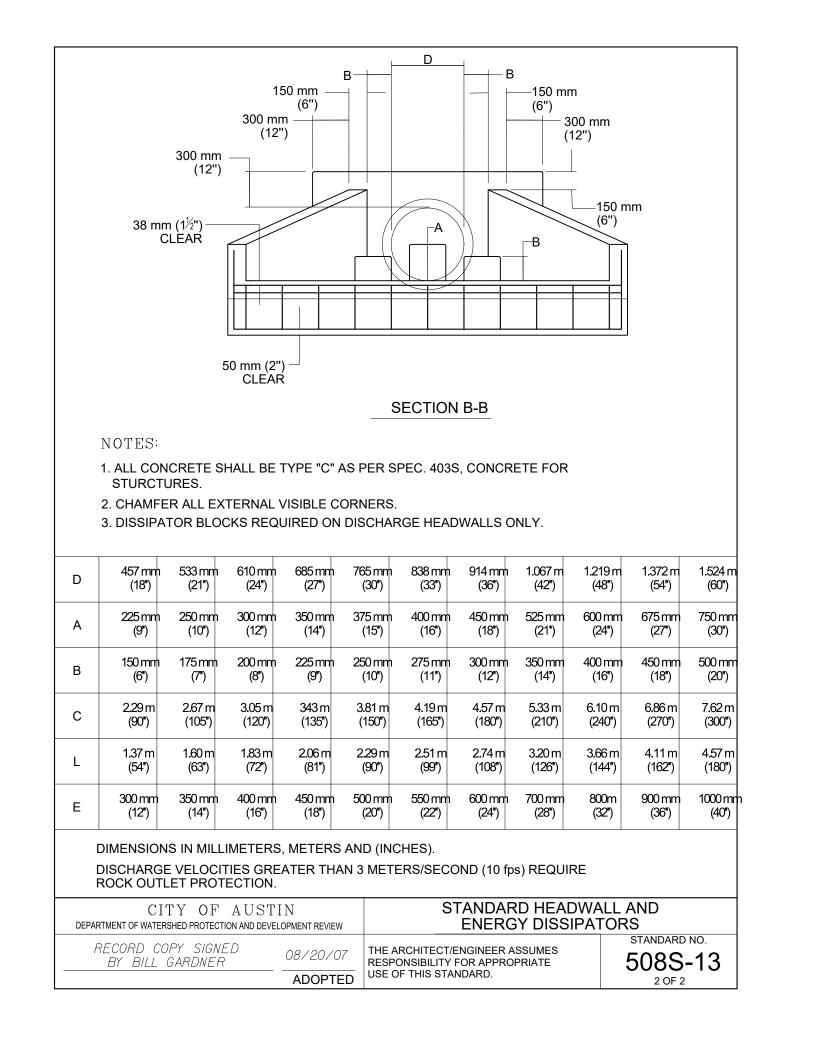
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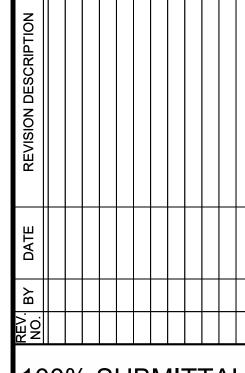
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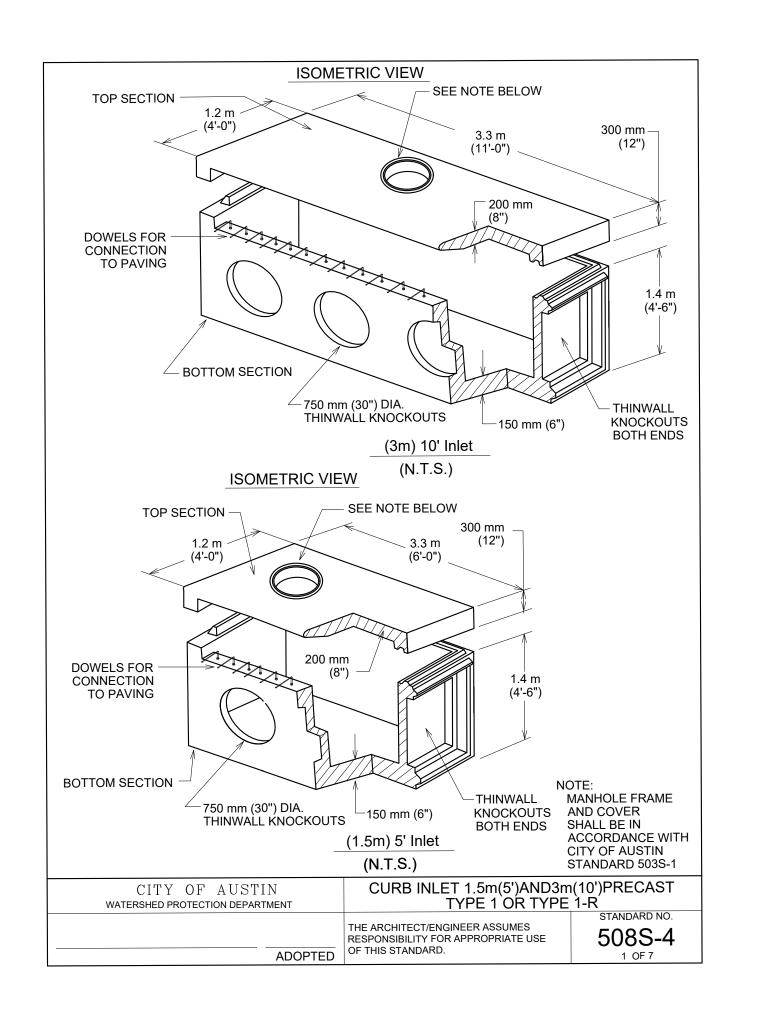
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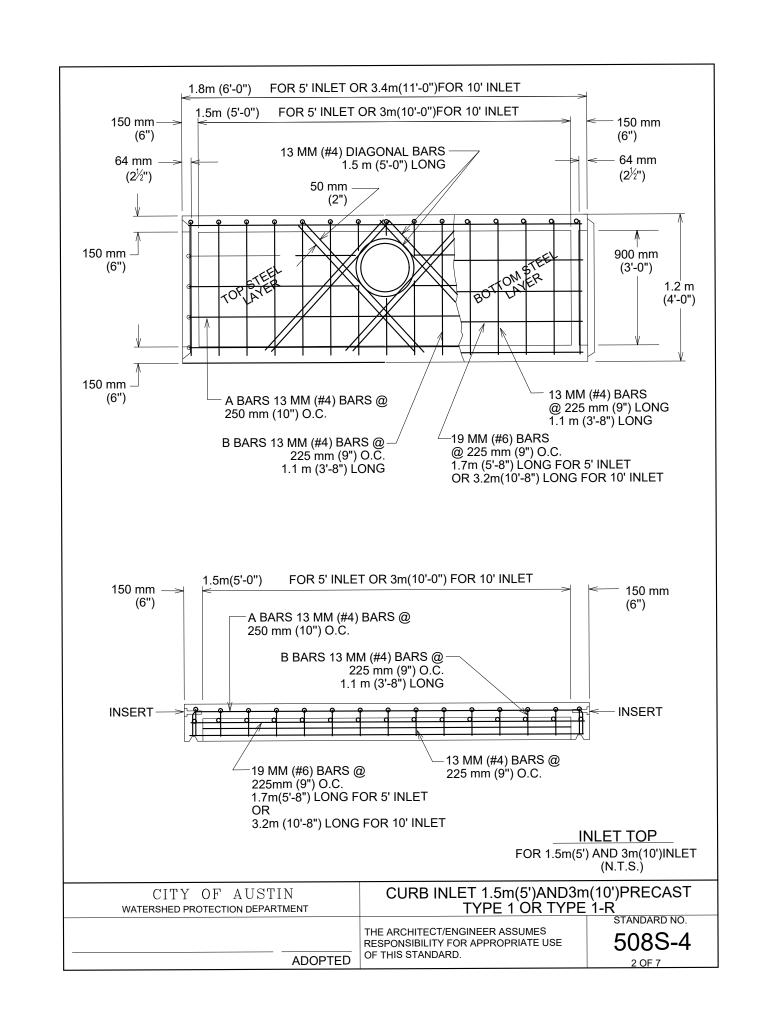
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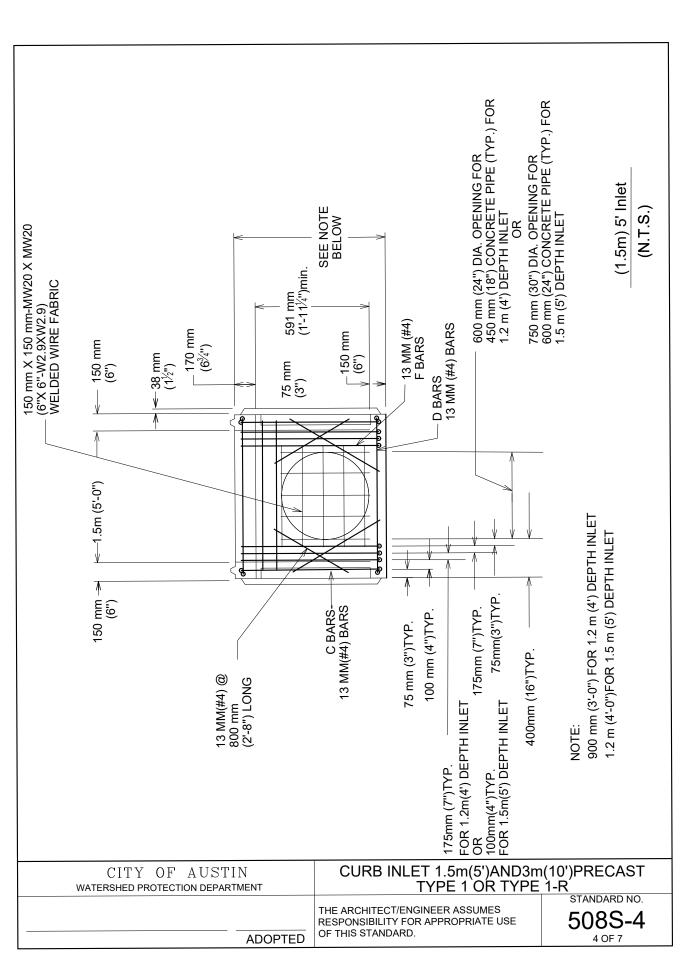
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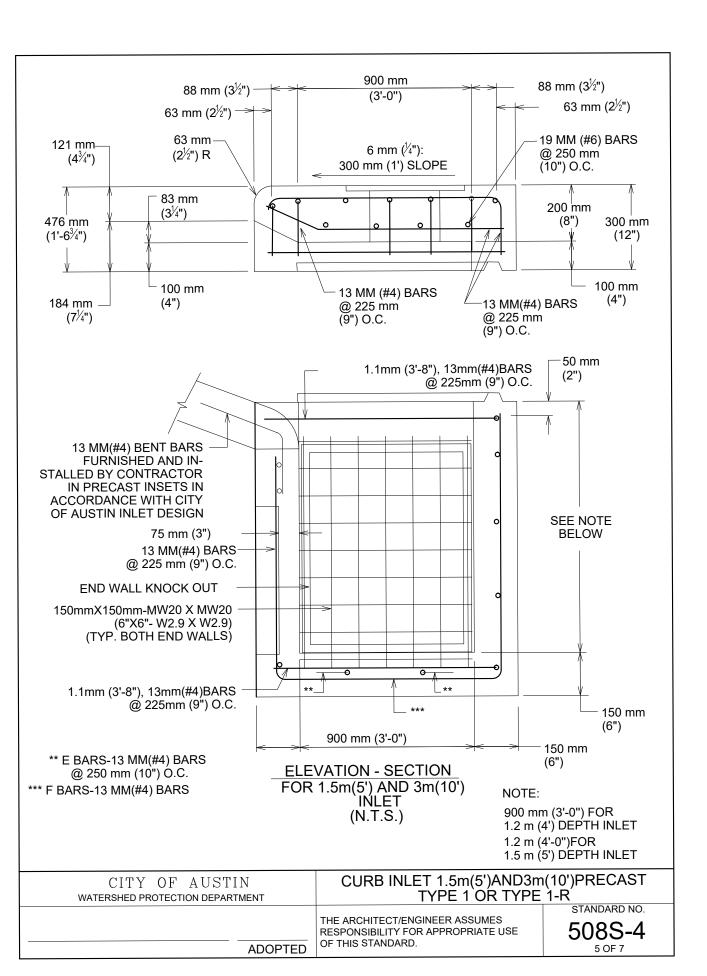
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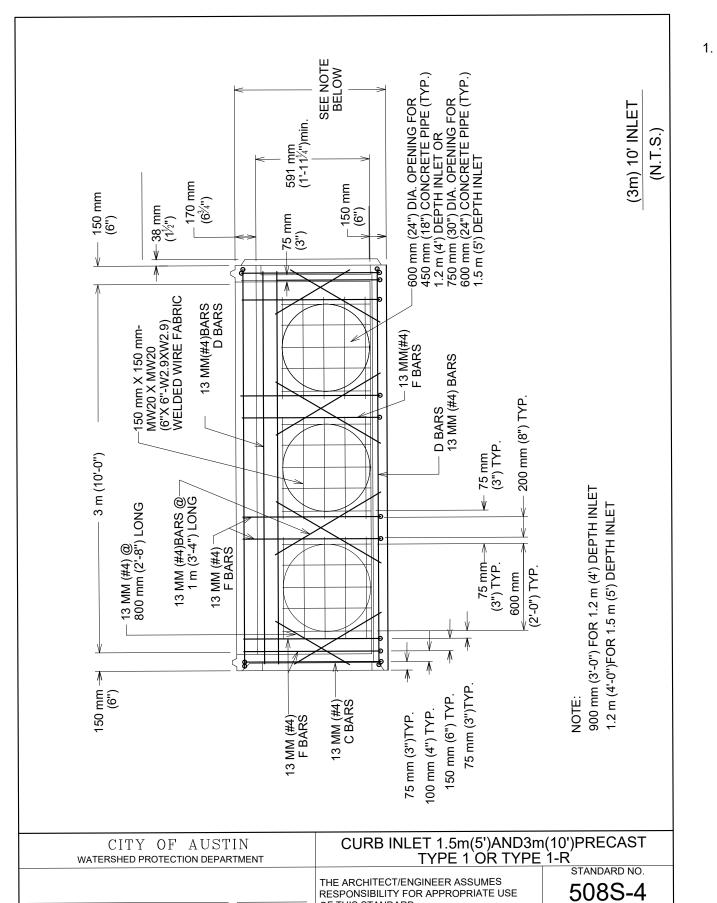
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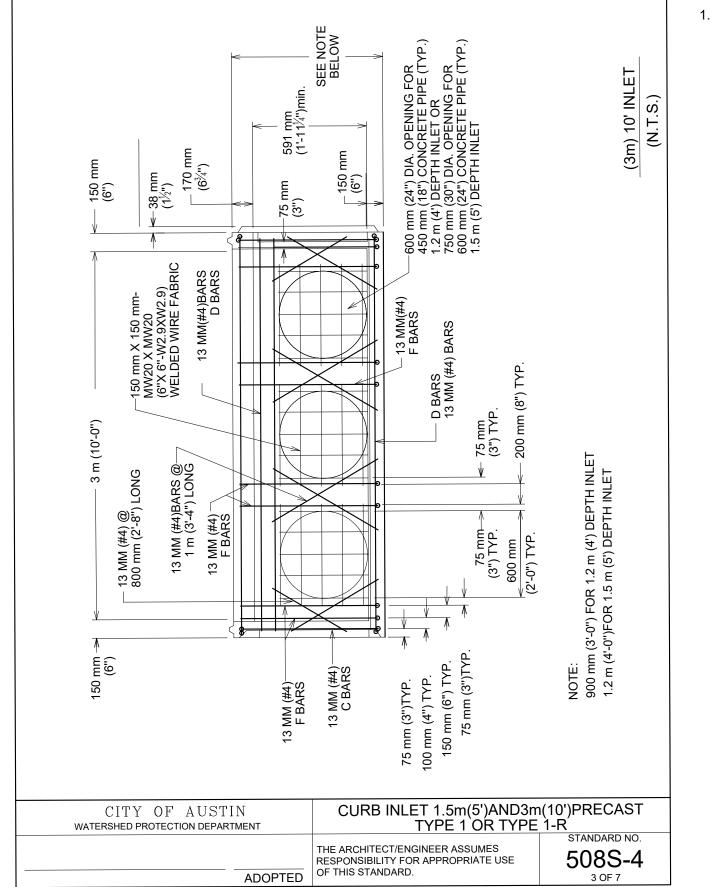






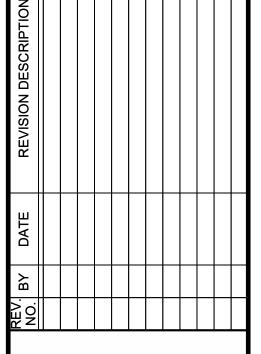






#### NOTES:

- CONCRETE CURB AND GUTTER SECTION (DETAIL 430S-2)
- a. REFER TO GRADING PLAN SHEET FOR LOCATIONS WITH 8-INCH CURB HEIGHTS.
- b. GUTTER WIDTH TO MATCH EXISTING WIDTHS.



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**RELLINGWOOD K**·FRIESE + ASSOCIATES PUBLIC PROJECT ENGINEERING

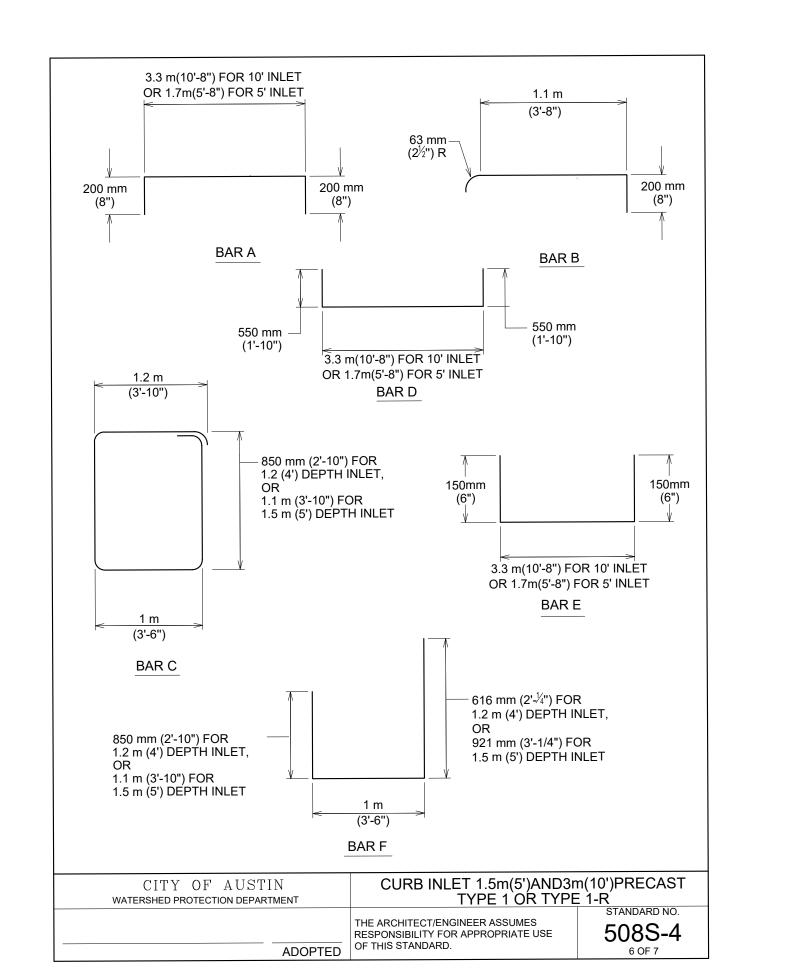
1120 S. Capital of Texas Highway CityView 2, Suite 100

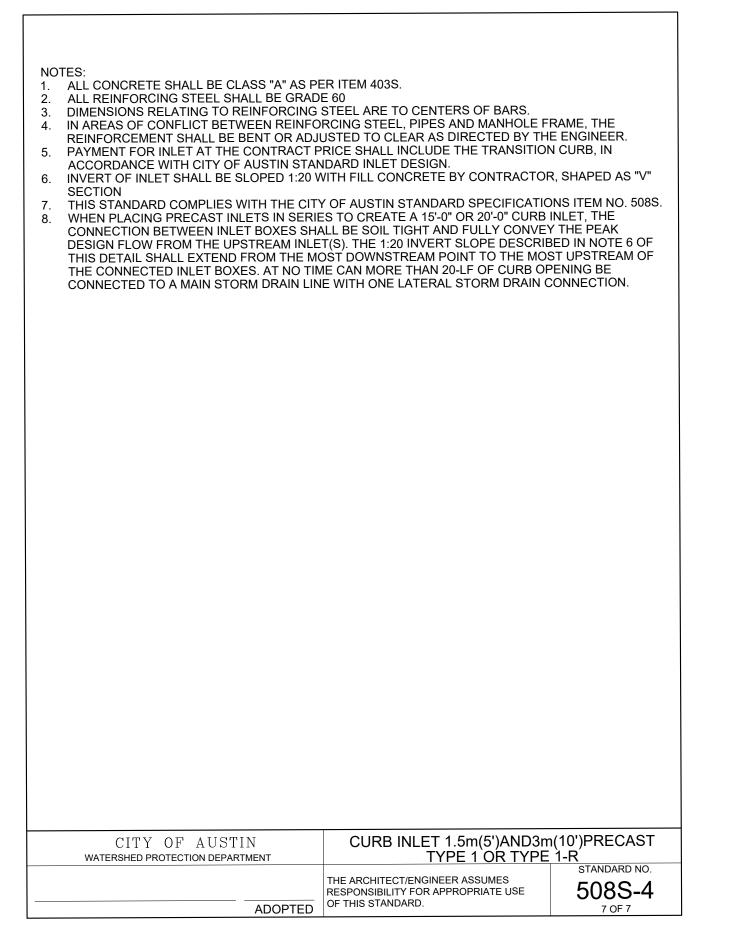
Austin, Texas 78746

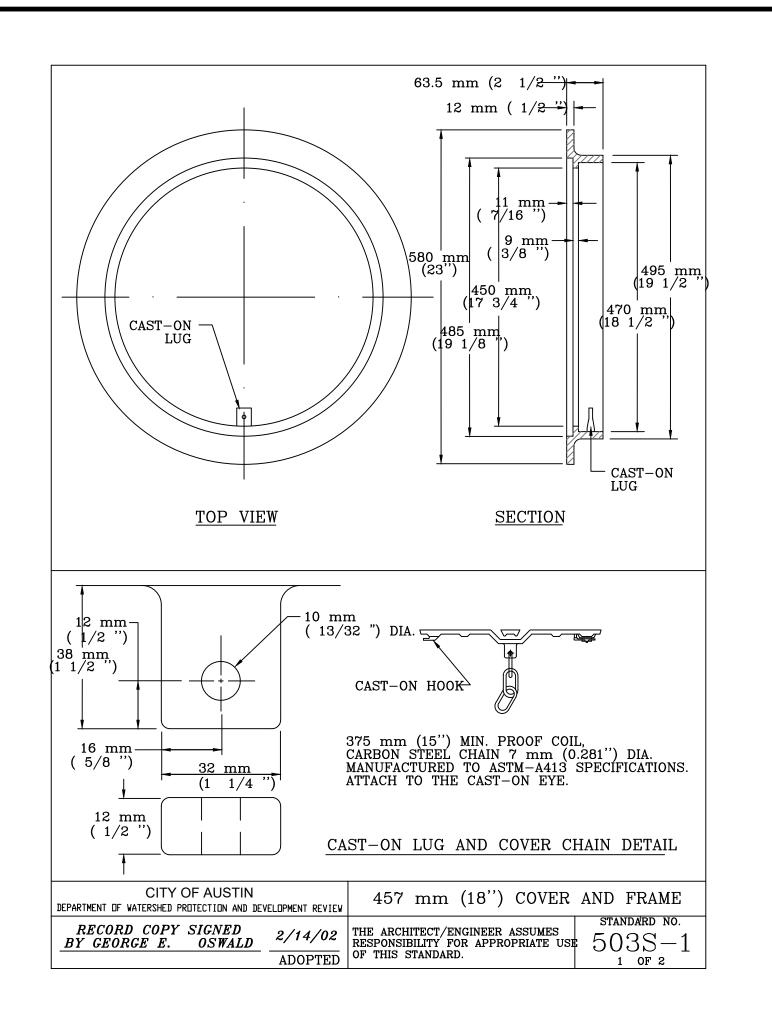
P - 512.338.1704 F - 512.338.1784 TBPE Firm #6535 www.kfriese.com NOTES NAME DATE SURVEY BY DRAWN BY 08/21 AΗ LWM

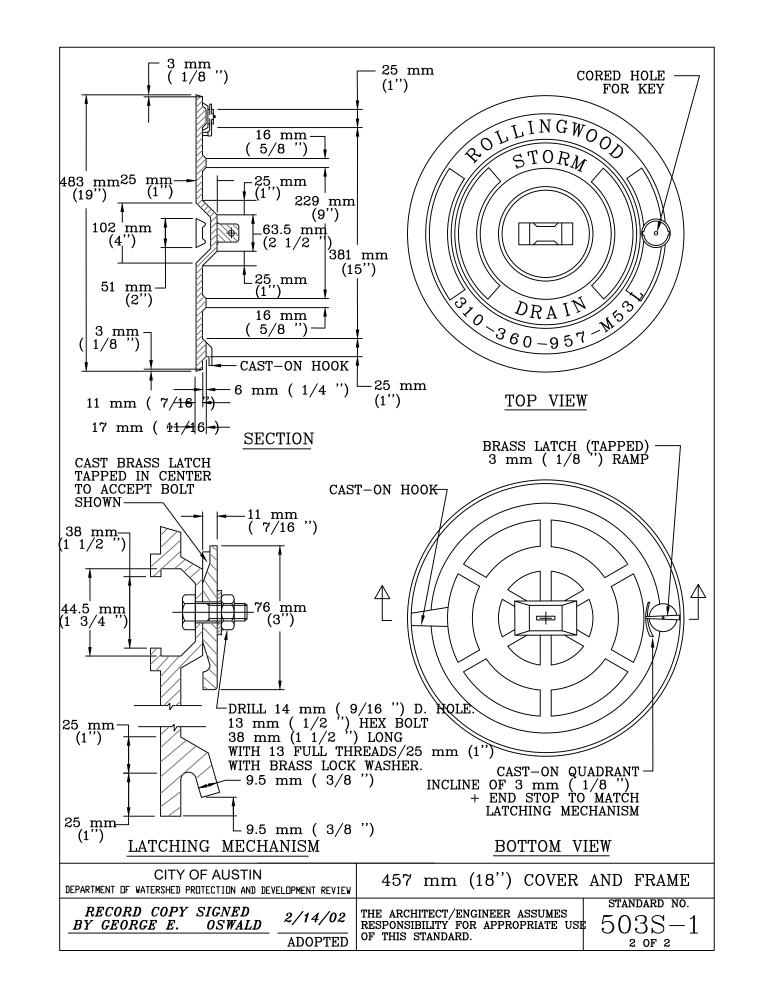
DESIGNED BY CHECKED BY GE 08/21 REVIEWED BY PS 09/21

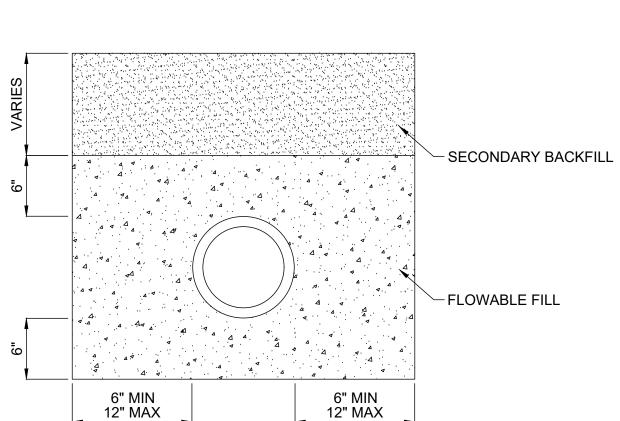
SD502 25 of 49





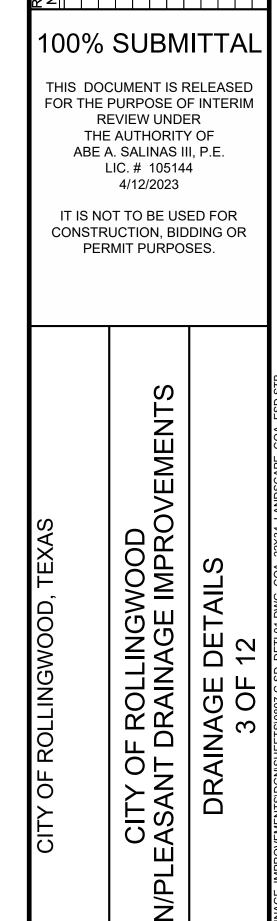






#### FLOWABLE FILL ENCASEMENT DETAIL N.T.S.

APPLICABLE AS INDICATED BY CALLOUTS ON THE PLAN SET



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TBPE Firm #6535

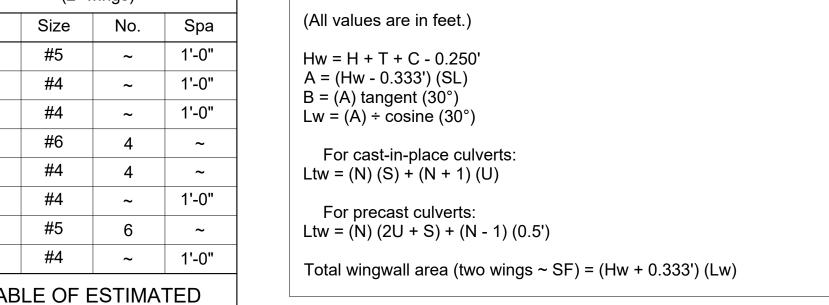
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NOTES	NAME	DATE	
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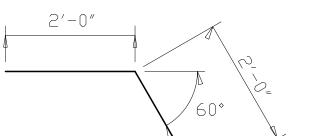
SD503 26 OF 49

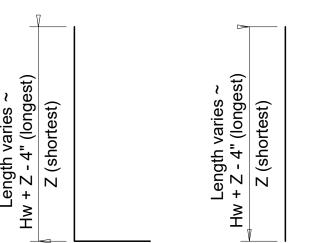
## TABLE OF WINGWALL REINFORCING (2~wings)

Q

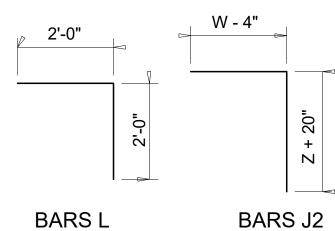
(Z**Willigs)							
Bar	Size	No.	Spa				
D	#5	~	1'-0"				
E	#4	~	1'-0"				
F	#4	~	1'-0"				
G	#6	4	~				
М	#4	4	~				
Р	#4	~	1'-0"				
R	#5	6	~				
V	#4	~	1'-0"				
TABLE OF ESTIMATED							







BARS V



**GENERAL NOTES:** 

noted otherwise.

with finished grade.

MATERIAL NOTES:

elsewhere in the plans.

(1) Extend Bars P 3'-0" minimum into bottom slab of

(3) Quantities shown are based on an average wing height

for two wings (one structure end). To determine total

quantities for two wings, multiply the tabulated values

(4) Recommended values of side slope are: 2:1, 3:1, 4:1, and 6:1.

2 Adjust as necessary to maintain 1 1#2" clear

(5) When shown elsewhere on the plans, construct

provide a 6" wide by 1'-6" deep reinforced

concrete toewall along all edges of the riprap

extend construction joints or grooved joints

oriented in the direction of flow across the full

shown in SECTION B-B will not be required.

When such riprap is provided, the culvert toewall

(6) At Contractor's option, culvert toewall may be ended

(7) 0" Min to 5'-0" Max. Estimated curb heights are shown

elsewhere in the plans. For structures with pedestrian

with T631 or T631LS bridge rail, refer to the Mounting

8 For vehicle safety, the following requirements must be met: For structures without bridge rail, construct curbs

No changes will be made in quantities and no additional

Provide Class C concrete (f'c=3,600 psi).

Provide galvanized reinforcing steel if required

In riprap concrete synthetic fibers listed on the

"Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing unless

no more than 3" above finished grade.

compensation will be allowed for this work.

Provide Grade 60 reinforcing steel.

Details for T631 & T631LS Rails (T631-CM) standard sheet.

Refer to the Box Culvert Rail Mounting Details (RAC) standard sheet for structures with bridge rail other than T631 or T631LS.

For structures with bridge rail, construct curbs flush

Reduce curb heights, if necessary, to meet the above requirements.

rail or curbs taller than 1'-0, refer to the Extended Curb

flush with wingwall toewall. Adjust reinforcing

Details (ECD) standard sheet. For structures

5" deep concrete riprap. Payment for riprap is

shown on the plans or directed by the Engineer,

as required by Item 432, "Riprap". Unless otherwise

adjacent to natural ground; reinforce the toewall by

extending typical riprap reinforcing into the toewall; and

distance of the riprap at intervals of approximately 20'.

cover and 4" minimum between bars.

box culvert.

by Lw.

as needed.

Designed according to AASHTO LRFD Bridge Design Specifications.

When structure is founded on solid rock, depth of toewalls for culverts and wingwalls may be reduced or eliminated as directed by the Engineer. See Box Culvert Supplement (BCS) standard sheet for additional dimensions and information.

The quantities for concrete and reinforcing steel resulting from the formulas given on this sheet are for Contractor's information only.

Reinforcing dimensions are out-to-out of bars.



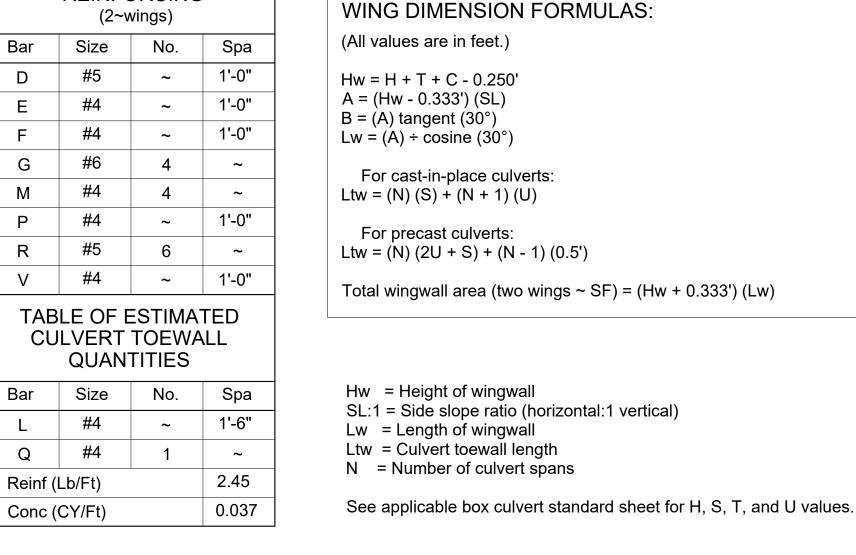
**CONCRETE WINGWALLS** WITH FLARED WINGS FOR 0° SKEW BOX CULVERTS

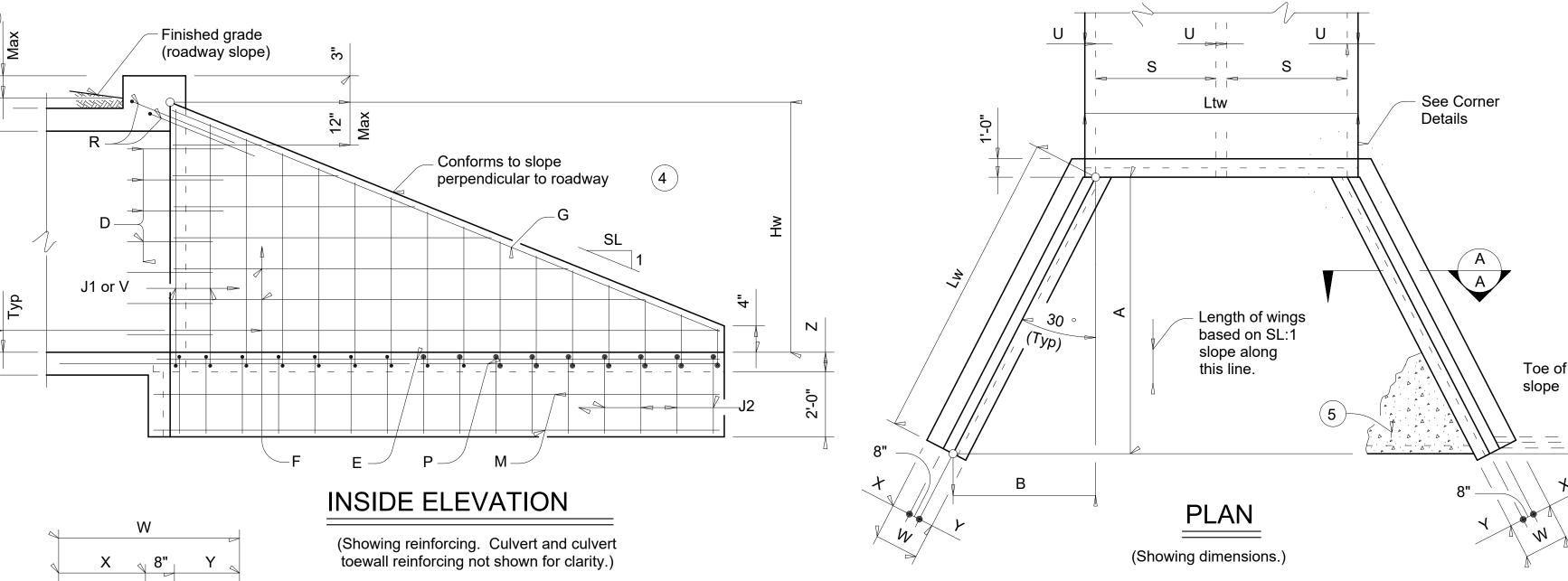
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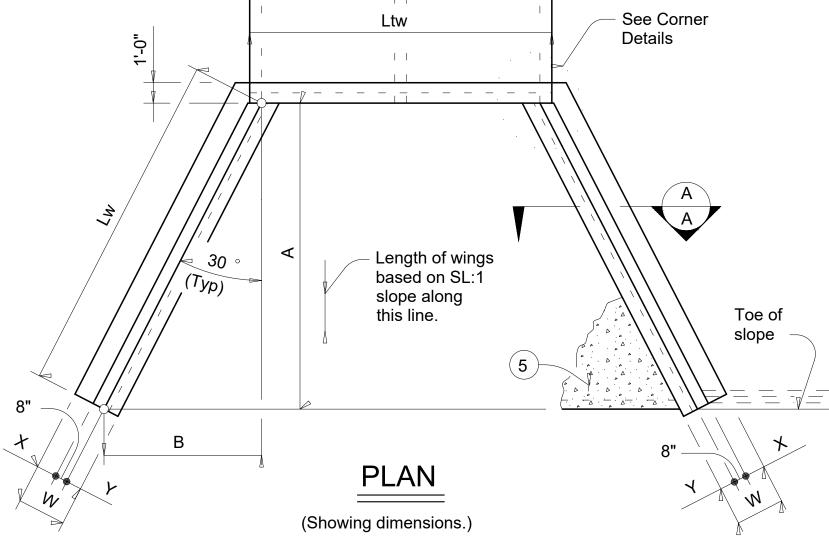
Bridge Division

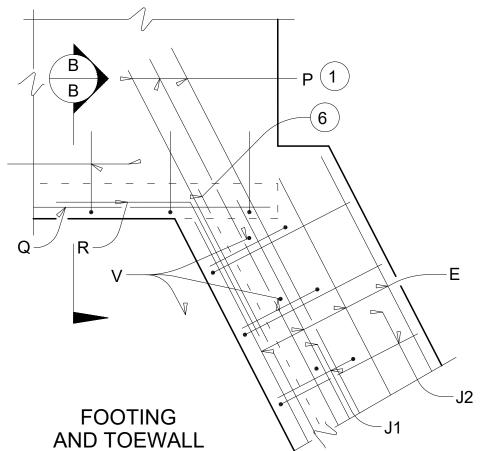
Standard

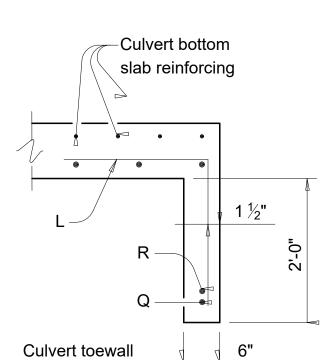
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©TxDOT February 2020	CONT	SECT		JOB			HIGH	IWAY								
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**CORNER DETAILS** 

reinforcing not shown for clarity.)

F or G

WINGWALL

(Culvert and culvert toewall

const joint

Const joint

Wingwall toewall

SECTION A-A

BARS D

BARS R

Y + 4" BARS J1

Cover dimensions are clear dimensions, unless noted otherwise.

SCL, nd is this

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#### TABLE OF DIMENSIONS AND REINFORCING STEEL (Wings for one structure end) Estimated Variable Reinforcing **Dimensions** Quantities per ft of wing length Bars J1 Bars J2 Maximum (2~wings) Wingwall W Χ HeightHw Reinf Conc Spa Spa Size Size (Lb/Ft) (CY/Ft) 2'-6" 2'-5" 1'-0" 1'-0" 1'-0" 33.73 0.248 3'-0" 1'-0" 0.261 2'-5" 1'-0" 9" 1'-0" 37.07 3'-6" 2'-5" 1'-0" 1'-0" #4 1'-0" 37.74 0.273 0.285 4'-0" 2'-5" 1'-0" 1'-0" 38.41 4'-6" 1'-6" 1'-0" 1'-0" 41.75 0.330 3'-2" 1'-0" 5'-0" 3'-2" 1'-6" 1'-0" 1'-0" 1'-0" 45.09 0.343 5'-6" 1'-0" 45.75 0.355 3'-2" 1'-6" 3'-2" 1'-6" 1'-0" 1'-0" 1'-0" 46.42 0.367 6'-0" #4 7'-0" 1'-0" 52.77 3'-8" 1'-0" 0.414 1'-9" 1'-3" 8'-0" 4'-2" 2'-0" 1'-6" 8" 1'-0" 1'-0" 60.19 0.486 #5 9'-0" 4'-8" 2'-3" 1'-9" 8" #4 6" 81.49 0.535 10'-0" 5'-2" 8" #5 6" 97.25 0.584 11'-0" 5'-8" 2'-9" 8" 6" 0.634 2'-3" #6 6" #5 133.65 12'-0" 6'-2" 9" 162.29 0.721 3'-0" 2'-6" #7 6" #5 6" 13'-0" 6'-8" 3'-3" 2'-9" 178.80 0.856 11" #7 6" #5 6" 14'-0" 7'-2" 3'-0" 1'-0" #8 6" #5 6" 216.78 0.959 15'-0" 1.068 7'-8" 3'-0" 1'-1" 283.06 16'-0" 1'-3" 1.234 8'-2" 4'-6" 3'-0" #9 6" #6 297.02 6"

RE	.E OF W INFORC 2~wings		LL								
Bar	Size	No.	Spa								
D	#5	~	1'-0"								
Е	#4	~	1'-0"								
F											
F #4 1'-0" G #6 4 ~											
М	#4	4	~								
Р	#4	~	1'-0"								
R	#5	6	~								
V	#4	~	1'-0"								
TABLE OF ESTIMATED											

'	z wiiige	· )	
3ar	Size	No.	Spa
D	#5	~	1'-0"
E	#4	~	1'-0"
F	#4		1'-0"
G	#6	4	~
М	#4	4	~
Р	#4	~	1'-0"
R	#5	6	~
V	#4	~	1'-0"
ГАВЬ	E OF ES	STIMATI	=D

TABL	E OF E	STIMATI	ΞD
CULV	ERT TO	DEWALL	ı
QUAN	ITITIES		

Bar	Size	No.	Spa
L	#4	~	1'-6"
Q	#4	1	~
Reinf (	Lb/Ft)		2.45
Conc (	CY/Ft)		0.037

#### WING DIMENSION FORMULAS: (All values are in feet.) Hw = H + T + C - 0.250'

Lw = (Hw - 0.333') (SL)	
For cast-in-place culverts: Ltw = (N) (S) + (N + 1) (U)	
For precast culverts:	

Ltw = (N) (2U + S) + (N - 1) (0.5')

Hw = Height of wingwall

See Corner

Details.

Length of wings

Toe of

based on SL:1 slope along

this line.

SL:1 = Side slope ratio (horizontal:1 vertical)

Total Wingwall Area (two wings ~ SF) = (Hw + 0.333') (Lw)

Lw = Length of wingwall Ltw = Culvert toewall length

N = Number of culvert spans

See applicable box culvert standard sheet for H, S, T, and U values.

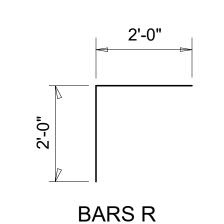
Ltw

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**PLAN** 

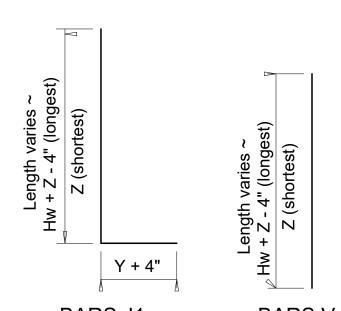
(Showing dimensions.)

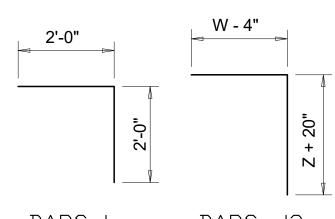
В

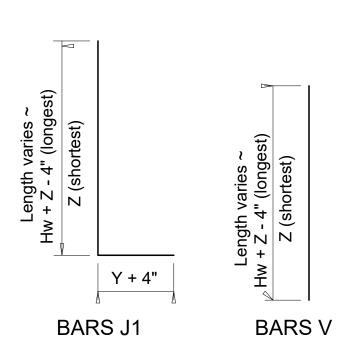


4'-0"

BARS D







BARS L BARS J2

- (1) Extend Bars P 3'-0" minimum into bottom slab of box culvert.
- Adjust as necessary to maintain 1 1#2" clear cover and 4" minimum between bars.
- Quantities shown are based on an average wing height for two wings (one structure end). To determine total quantities for two wings, multiply the tabulated values by Lw.
- Recommended values of side slope are: 2:1, 3:1, 4:1, and 6:1.
- √ When shown elsewhere on the plans, construct 5" deep concrete riprap. Payment for riprap is as required by Item 432, "Riprap". Unless otherwise shown on the plans or directed by the Engineer, provide a 6" wide by 1'-6" deep reinforced concrete toewall along all edges of the riprap adjacent to natural ground; reinforce the toewall by extending typical riprap reinforcing into the toewall; and extend construction joints or grooved joints oriented in the direction of flow across the full distance of the riprap at intervals of approximately 20'. When such riprap is provided, the culvert toewall shown in SECTION B-B will not be required.
- At Contractor's option, culvert toewall may be ended flush with wingwall toewall. Adjust reinforcing as needed.
- 7 0" Min to 5'-0" Max. Estimated curb heights are shown elsewhere in the plans. For structures with pedestrian rail or curbs taller than 1'-0, refer to the Extended Curb Details (ECD) standard sheet. For structures with T631 or T631LS bridge rail, refer to the Mounting Details for T631 & T631LS Rails (T631-CM) standard sheet. Refer to the Box Culvert Rail Mounting Details (RAC) standard sheet for structures with bridge rail other than T631 or T631LS.
- 8 For vehicle safety, the following requirements must be met:
  - For structures without bridge rail, construct curbs
  - no more than 3" above finished grade.
  - · For structures with bridge rail, construct curbs flush with finished grade.

Reduce curb heights, if necessary, to meet the above requirements. No changes will be made in quantities and no additional compensation will be allowed for this work.

#### **MATERIAL NOTES:**

Provide Class C concrete (f'c=3,600 psi). Provide Grade 60 reinforcing steel. Provide galvanized reinforcing steel if required elsewhere in the plans.

In riprap concrete, synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing unless noted

#### **GENERAL NOTES:**

for Contractor's information only.

Designed according to AASHTO LRFD Bridge Design Specifications.

When structure is founded on solid rock, depth of toewalls for culverts and wingwalls may be reduced or eliminated as directed by the Engineer. See Box Culvert Supplement (BCS) standard sheet for additional dimensions and information. The quantities for concrete and reinforcing steel resulting from the formulas given on this sheet are

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing dimensions are out-to-out of bars.



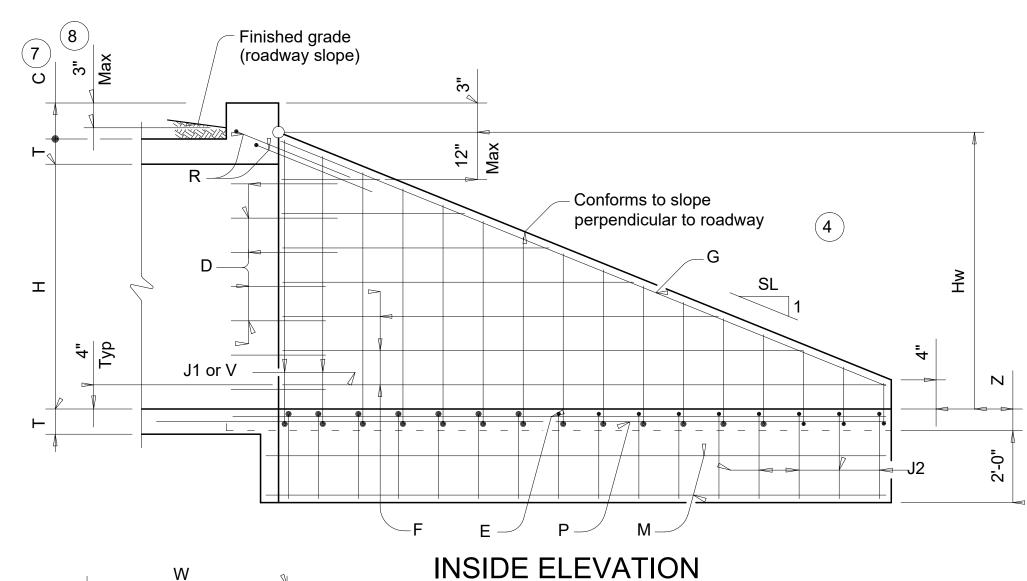
CONCRETE WINGWALLS WITH STRAIGHT WINGS FOR 0° SKEW BOX CULVERTS

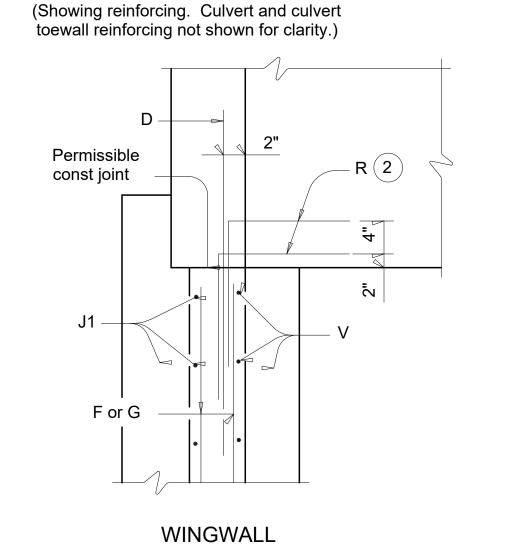
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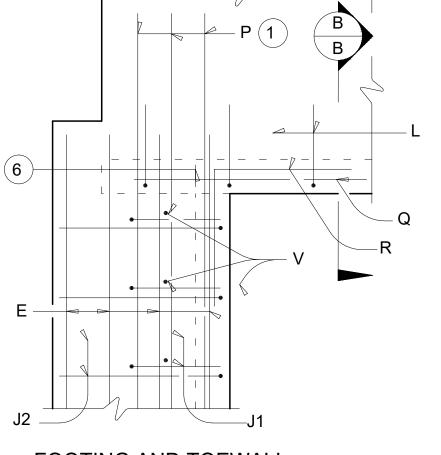
Bridge Division

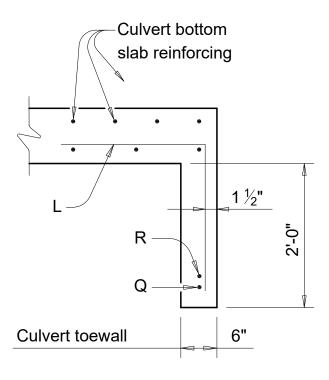
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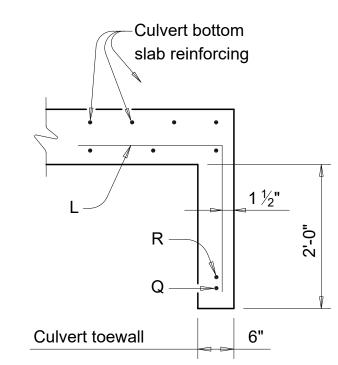
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SECTION B-B

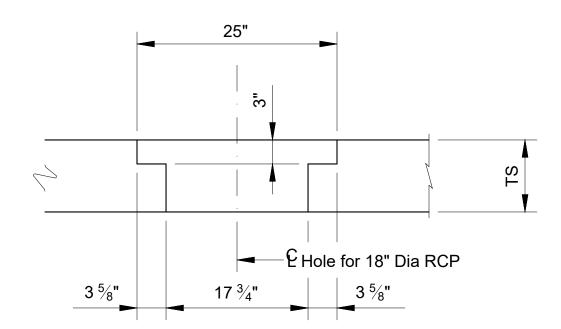
FOOTING AND TOEWALL **CORNER DETAILS** 

**SECTION A-A** 

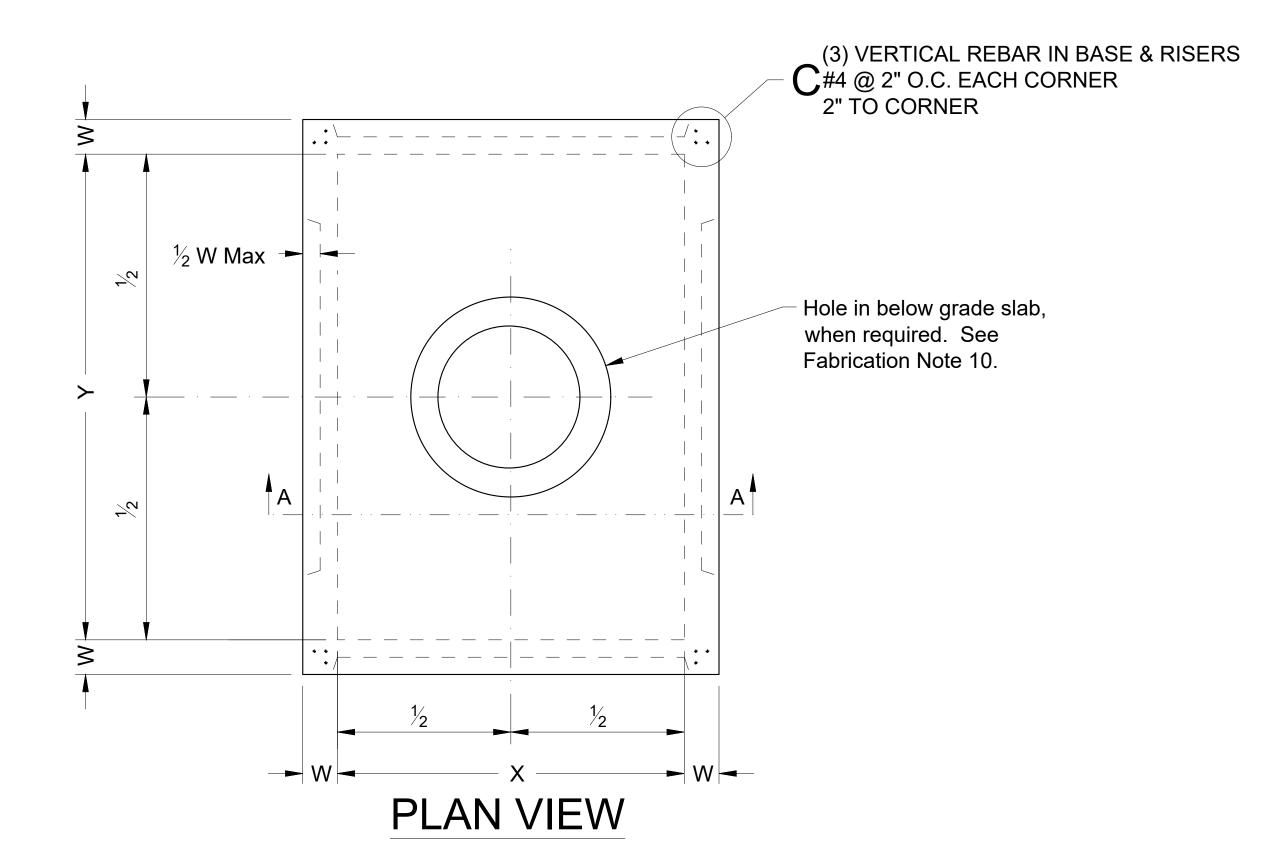
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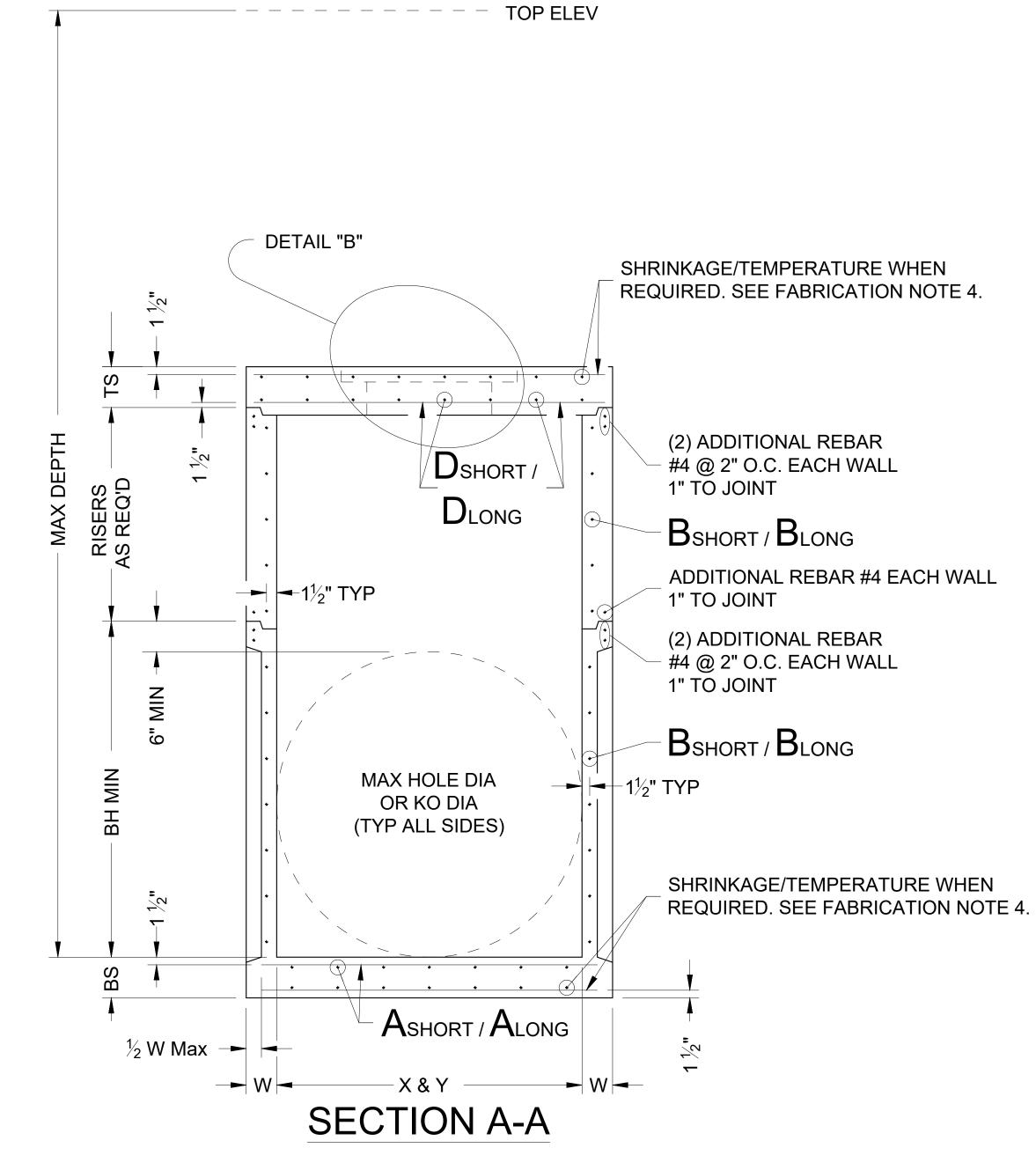
Const joint

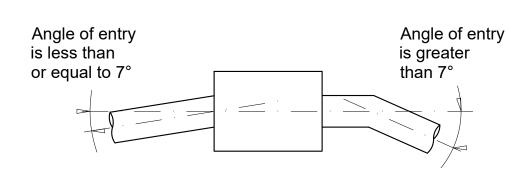
Wingwall toewall



### DETAIL "B"







### PIPE CONNECTION DETAIL

Connect pipes within 7° of normal to PJB wall. If necessary, use pipe elbow or curved approach alignment to stay within this limit.

#### **FABRICATION NOTES:**

- 1. Provide Class "H" concrete in accordance with Item 421 and having a minimum compressive strength of 5,000 psi.
- 2. Provide Grade 60 reinforcing steel or equivalent area of WWR.
- 3. Provide typical clear cover of 1 ½" to reinforcing steel at interior or exterior walls.
- 4. Walls or slabs with a thickness of 8" or greater require shrinkage and temperature reinforcing steel. Provide steel area =  $0.11 \text{ in}^2/\text{ft}$  each way.
- 5. No substitution is allowed for vertical and horizontal #4 bars in corners.
- 6. Manufacture base and risers to nearest 3" increment.
- 7. Design tongue and groove joints for full closure on both shoulders. Minimum spigot depth is 3/4".
- 8. Provide lifting devices in conformance with Manufacturer's recommendations.
- 9. See sheet PDD for sizes, dimensions, and reinforcing steel not shown. 10. Provide hole in below grade slab only when PJB is installed with inlet type POD.

#### **INSTALLATION NOTES:**

- 1. Inverts (benching) to be provided by Contractor. Concrete or mortar used for invert is subsidiary
- 2. Seal tongue and groove joints with preformed or bulk mastic in conformance with Manufacturer's recommendations. Tongue and groove joints may be grouted no more than 1" between each section, or ½ the joint depth, whichever is greater.
- 3. Do not grout rubber gasket joints without Manufacturer's recommendation.
- 4. For rigid pipe, cut hole in thin wall panel (KO) 4" Max, 2" Min larger than pipe OD.
- 5. For flexible pipe, consult boot/seal Manufacturer's specification for placement tolerance and hole size. Center pipe in hole and install boot/seal per Manufacturer's specification.

#### **GENERAL NOTES:**

- 1. Precast Junction Box consists of base slab, base unit, risers (as required), and below grade slab. See sheet PDD for sizes
- 2. Designed according to ASTM C913.
- 3. Payment for junction box is per Item 465 "Junction Boxes, Manholes, and Inlets" by type and size.

Cover dimensions are clear dimensions, unless noted otherwise.

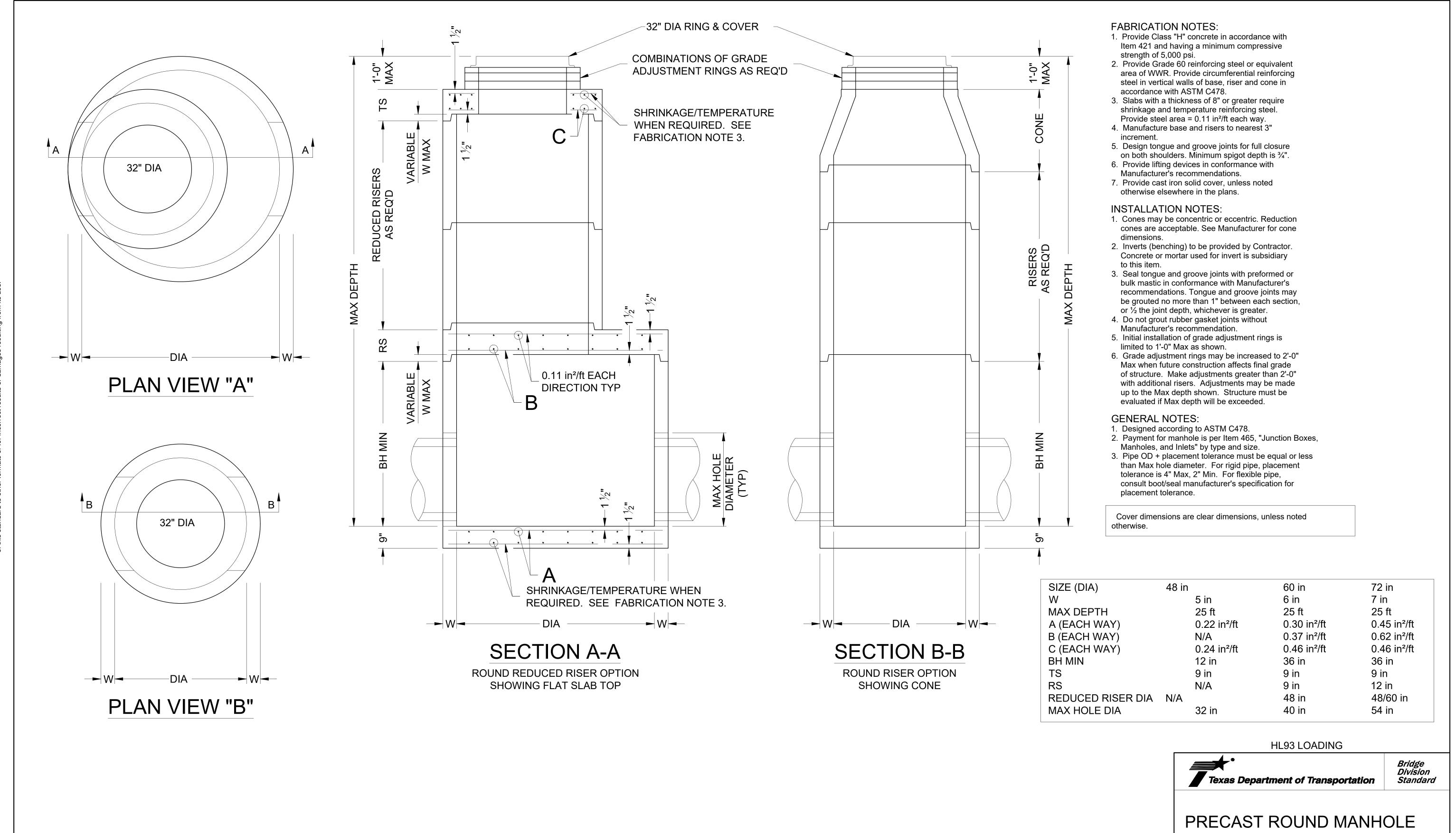


Bridge Division Texas Department of Transportation Standard

PRECAST JUNCTION BOX

PJB DN: TxDOT CK: TxDOT DW: TxDOT CONT SECT

prestd09-20.dgn ck: TxDOT C)TxDOT February 2020 HIGHWAY REVISIONS COUNTY SHEET NO. 29 OF 49



PRM

TILE: prestd02-20.dgn

DN: TxDOT

CK: TxDOT

DW: TxDOT

CK: TxDOT

1) For skewed box culverts with less than 2'-0" of fill, break back the top slab to provide a 1'-10" minimum lap of the existing longitudinal bars with the longitudinal bars in the extension.

For non-skewed box culverts with less than 2'-0" of fill and for skewed or non-skewed culverts with a fill depth of 2'-0" or greater, break back the top slab to provide a 1'-10" minimum lap of the existing longitudinal bars with the longitudinal bars in the extension. Alternatively, if the box is non-skewed, embed #6 anchor bars with a Type III, C, D, E, or F anchor adhesive into the existing walls, top and bottom slab at 1'-6" center-to-center spacing. Minimum embedment depth is 8". Anchor adhesive chosen must be able to achieve a basic bond strength in tension, Nba, of 26.4 kips. Submit signed and sealed calculations or the manufacturer's published literature showing the proposed anchor adhesive's ability to develop this load to the Engineer for approval prior to use. Anchor installation, including hole size, drilling, and clean out, must be in accordance with Item 450, "Railing." Test adhesive anchors in accordance with Item 450.3.3, "Tests." Test 3 anchors per 100 anchors installed.

Break back wings and apron as necessary to install the extension. Clean and extend the exposed wingwall and apron reinforcing into the extension. When lengthening existing box culverts with dimensions different than current standard dimensions, form horizontal and vertical transitions as directed by the Engineer. Match bottom slabs to maintain an uninterrupted flow line. Field bend existing and new reinforcing into transitions and maintain specified cover requirements. For top slabs of culverts with overlay, with 1-to-2 course surface treatment, or with the top slab as the final riding surface, adjust the "H" dimension to provide a smooth riding surface.

- 2 When the spacing between Bars B becomes less than half of the normal spacing, cut bars to avoid conflict.
- 3 The length of Bars B vary in the skewed end sections.

- (Typ)(6)

- € Culvert

- (4) [One half of overall width] x [tangent of the skew angle]
- 5 Place Bars F1 and F2 continuously through the angle section. Bend Bars F1 and F2 to remain parallel to the walls of the box culvert.
- 6 When necessary to avoid conflict in acute corners, shorten the slab extension leg of Bars C and Bars D to a minimum of 1'-6" for skews of 30° thru 45°.
- At the Contractor's option, for skews of 15° or less, place Bars B, C, and D parallel to the skewed end while maintaining spacing along centerline of box. Increase lengths of Bars B shown on the Single Box Culverts Cast-In-Place (SCC) standards sheets to accommodate the skew.

#### **CONSTRUCTION NOTES:**

Do not use permanent forms.

When required, lap Bars H 1'-8" for uncoated or galvanized bars. Provide a minimum of  $1\frac{1}{2}$ " clear cover.

#### MATERIAL NOTES:

Provide Grade 60 reinforcing steel.

Provide galvanized reinforcing steel, if required elsewhere in the plans.

Provide Class C concrete (f'c = 3,600 psi) with these exceptions: provide Class S concrete (f'c = 4,000 psi) for top slabs of culverts with overlay,

with 1-to-2 course surface treatment, or with the top slab as the final riding

#### **GENERAL NOTES:**

Designed according to AASHTO LRFD Bridge Design Specifications.

Refer to Single Box Culverts Cast-in-Place (SCC) standard sheets for details of straight sections of culvert.

For skewed sections and angle sections, refer to Single Box Culverts Cast-in-Place (SCC) standard sheets for slab and wall dimensions, bar sizes, maximum bar spacing, and any other details not shown.

For skewed ends with curbs, adjust length of Bars H, number of Bars K, curb concrete volume, and reinforcing steel weight by dividing the values shown on the culvert Single Box Culverts Cast-In-Place (SCC) standard sheets by the cosine of the skew

Cover dimensions are clear dimensions, unless noted otherwise.

**HL93 LOADING** 



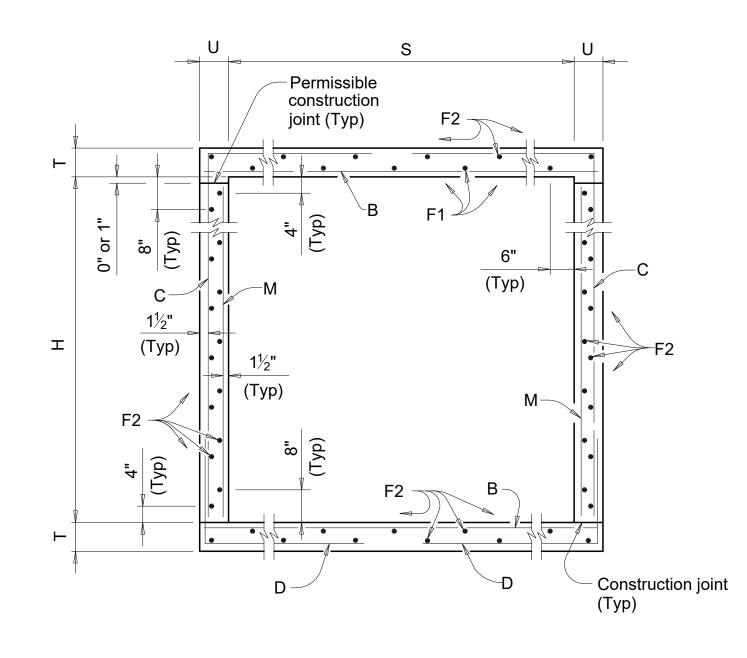
## CAST-IN-PLACE MISCELLANEOUS DETAILS

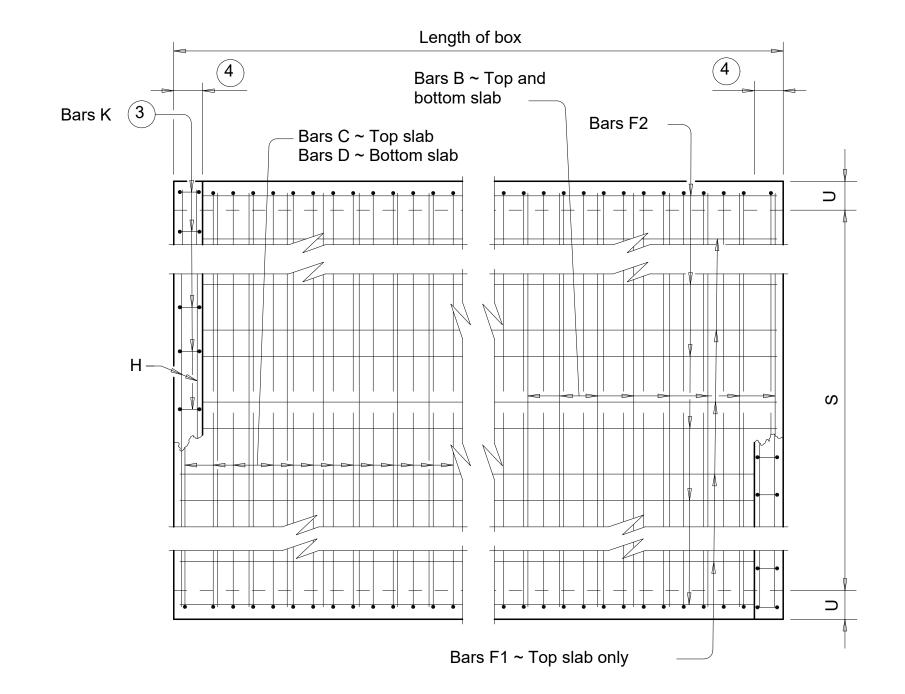
SCC-MD

Bridge Division

Standard

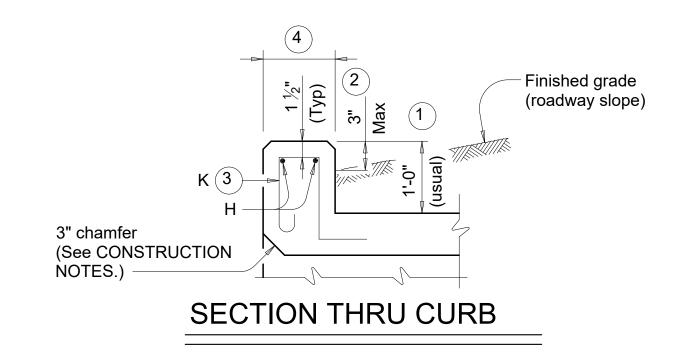
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	DIST		COUNTY			SHEET NO.
						31 OF 49





#### TYPICAL SECTION





#### PLAN OF REINF STEEL

0" Min to 5'-0" Max. Estimated curb heights are shown elsewhere in the plans. For structures with pedestrian rail or curbs taller than 1'-0", refer to the Extended Curb Details (ECD) standard sheet. For structures with T631 or T631LS bridge rail, refer to the Mounting Details for T631 & T631LS Rails (T631-CM) standard sheet. Refer to the Rail Anchorage Curb (RAC) standard sheet for structures with bridge rail other than T631 or T631LS.

2 For vehicle safety, the following requirements must be met: · For structures without bridge rail, construct curbs no more than 3" above

· For structures with bridge rail, construct curbs flush with finished grade. Reduce curb heights, if necessary, to meet the above requirements. No changes will be made in quantities and no additional compensation will be allowed for this work.

For curbs less than 1'-0" high, tilt Bars K or reduce bar height as necessary to maintain cover. For curbs less than 3" high, Bars K may be omitted.

4 1'-0" typical. 2'-3" when the Rail Anchorage Curb (RAC) standard sheet is referred to elsewhere in the plans.

The Contractor may replace Bars B, C, D, E, F1, F2, M, Y, and/or Z with deformed welded wire reinforcement (WWR) meeting the requirements of ASTM A1064. The area of required reinforcement may be reduced by the ratio of 60 ksi / 70 ksi.
Spacing of WWR is limited to 4" Min and 18" Max. When required, provide lap splices in the WWR of the same length required for the equivalent bar size, rounded up for wire sizes between conventional bar sizes. The lap length required for WWR is never less than the lap length required for uncoated #4 bars.

Example conversion: Replacing No. 6 Gr 60 at 6" Spacing with WWR. Required WWR = (0.44 sq. in. per 0.5 ft.) x (60 ksi / 70 ksi) = 0.755 sq. in. per ft. If D30.6 wire is used to meet the 0.755 sq. in. per ft. requirement in this example, the required spacing =  $(0.306 \text{ sq. in.}) / (0.755 \text{ sq. in. per ft.}) \times (12 \text{ in. per ft.}) = 4.86$ " Max spacing. Required lap length for the provided D30.6 wire is 2'-1" (the same minimum lap length required for uncoated #5 bars, as listed under MATERIAL NOTES).

#### **CONSTRUCTION NOTES:**

Do not use permanent forms.

Chamfer the bottom edge of the top slab 3" at the entrance.

Optionally, raise construction joints shown at the flow line by a maximum of 6". If this option is taken, Bars M may be cut off or raised, Bars C and D may be reversed.

#### MATERIAL NOTES:

Provide Grade 60 reinforcing steel.

Provide galvanized reinforcing steel if required elsewhere in the plans. Provide Class C concrete (f'c = 3,600 psi) for culvert barrel and curb, with the following exceptions: provide Class S concrete (f'c = 4,000 psi) for top slabs of:

· culverts with overlay,

· culverts with 1-to-2 course surface treatment, or

· culverts with the top slab as the final riding surface.

Provide bar laps, where required, as follows:

· Uncoated or galvanized ~ #4 = 1'-8" Min

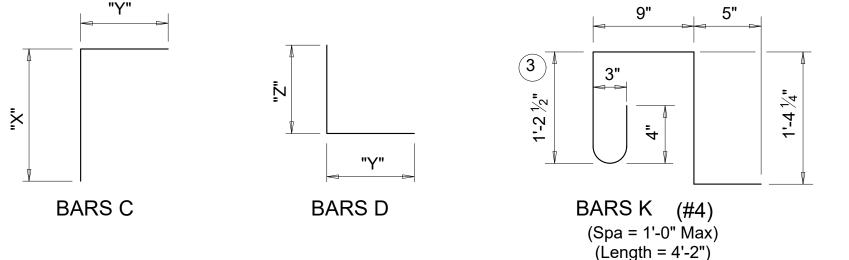
· Uncoated or galvanized ~ #5 = 2'-1" Min · Uncoated or galvanized ~ #6 = 2'-6" Min

#### **GENERAL NOTES:**

Designed according to AASHTO LRFD Bridge Design Specifications for the range of fill heights shown.

See the Single Box Culverts Cast-In-Place Miscellaneous Detail (SCC-MD) standard sheet for details pertaining to skewed ends, angle sections, and lengthening.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.





SHEET 1 OF 2 Bridge Division

Texas Department of Transportation Standard SINGLE BOX CULVERTS

CAST-IN-PLACE 0' TO 30' FILL

SCC-5 & 6

	_	_									
FILE: scc56ste-21.dgn	DN: TBE		ск: ВМР	DW: Tx	DOT		CK: TxDOT				
©TxDOT February 2020	CONT SECT JOB						HIGHWAY				
REVISIONS											
04/2021 Updated X values.	DIST		COUN	ΤΥ		S	SHEET NO.				
						3	2 OF 49				

DISCLAIMER:	The use of this standard is governed by the "Texas Engineering Practice Act". No warranty of any
kind is made by	TxDOT for any purpose whatsoever. TxDOT assumes no responsibility for the conversion
of this standard	to other formats or for incorrect results or damages resulting from its use.

				5		BILLS OF REINFORCING STEEL (For Box Length = 40 feet)																	Q	QUANTI	ITIES																
	ECTIO			HEIGHT			Bai	rs B					Bars	s C						В	ars D				Ва	rs M ~ #4		Bars F1 ~ at 18" S	#4 pa		rs F2 ~ #4 at 18" Spa		Bars I 4 ~ #4	Н 1	Bars K	Pe of l	r Foot Barrel	Curk	b	Total	ıl
S	Н	Т	U	FILL	No.	Size	Spa	Length	Weight	No.	Size	nd Le	ength	Weight	"X"	"Y"	No.	Size	Spa	Length	Weigh	"Y"	" Z '	"	No. ed	Length	Weight	No. Lengtl	n Wt	No.	Length	Weight	Length	Wt	No.	Wt Cond (CY)	Reinf (Lb)	Conc (CY)		Conc (CY)	Reinf
5' - 0"	2' - 0"	8"	7"	26'	108	#6	9"	5' - 11"	960	108	#5 9	9" 6	6' - 3"	704	2' - 6"	3' - 9"	108	#5	9"	6' - 5"	723	3' - 9'	' 2' -	8"	108 9"	2' - 0"	144	4 39' - 9	)" 106	22	39' - 9"	584	5' - 11"	16	14	39 0.391	80.5		55 1		3,276
5' - 0"	2' - 0"	9"	7"	30'	108	#6	9"	5' - 11"	960	108	#5 9	9" 6	6' - 4"	713	2' - 7"	3' - 9"	108	#5	9"	6' - 6"	732	3' - 9'	' 2' -	9"	108 9"	2' - 0"	144	4 39' - 9	)" 106	22	39' - 9"	584	5' - 11"	16	14 3	39 0.429	81.0	0.5	55 17	17.6	3,294
5' - 0"	3' - 0"	8"	7"	26'	108	#6	9"	5' - 11"	960	108	#5 9	9" 7	7' - 3"	817	3' - 6"	3' - 9"	108	#5	9"	6' - 5"	723	3' - 9'	' 2' -	8"	108 9"	3' - 0"	216	4 39' - 9	)" 106	26	39' - 9"	690	5' - 11"	16	14 3	39 0.434	87.8	3 0.5	55 17	17.8	3,567
5' - 0"	3' - 0"	9"	7"	30'	108	#6	9"	5' - 11"	960	108	#5 9	9" 7	7' - 4"	826	3' - 7"	3' - 9"	108	#5	9"	6' - 6"	732	3' - 9'	' 2' -	9"	108 9"	3' - 0"	216	4 39' - 9	)" 106	26	39' - 9"	690	5' - 11"	16	14 3	39 0.472	88.3	3 0.5	55 19	19.3	3,585
5' - 0"	4' - 0"	8"	7"	26'	108	#6	9"	5' - 11"	960	108	#5 9	9" 8	8' - 3"	929	4' - 6"	3' - 9"	108	#5	9"	6' - 5"	723	3' - 9'	' 2' -	8"	108 9"	4' - 0"	289	4 39' - 9	)" 106	26	39' - 9"	690	5' - 11"	16	14 3	39 0.477	92.4	4 0.5	55 19	19.5	3,752
5' - 0"	4' - 0"	9"	7"	30'	108	#6	9"	5' - 11"	960	108	#5 9	9" 8	8' - 4"	939	4' - 7''	3' - 9"	108	#5	9"	6' - 6"	732	3' - 9'	' 2' -	9"	108 9"	4' - 0"	289	4 39' - 9	)" 106	26	39' - 9"	690	5' - 11"	16	14 3	39 0.515	92.9	9 0.5	55 2°	21.1	3,771
5' - 0"	5' - 0"	8"	7"	26'	108	#6	9"	5' - 11"	960	108	#5 9	9" 9	9' - 3"	1,042	5' - 6"	3' - 9"	108	#5	9"	6' - 5"	723	3' - 9'	' 2' -	8"	108 9"	5' - 0"	361	4 39' - 9	)" 106	30	39' - 9"	797	5' - 11"	16	14 3	39 0.521	99.7	7 0.5	55 2°	21.3	4,044
5' - 0"	5' - 0"	9"	7"	30'	108	#6	9"	5' - 11"	960	108	#5 9	9" 9	9' - 4"	1,051	5' - 7"	3' - 9"	108	#5	9"	6' - 6"	732	3' - 9'	' 2' -	9"	108 9"	5' - 0"	361	4 39' - 9	)" 106	30	39' - 9"	797	5' - 11"	16	14 (	39 0.559	100.2	2 0.5	55 22	22.8	4,062
6' - 0"	2' - 0"	8"	7"	20'	108	#6	9"	6' - 11"	1,122	108	#5 9	9" 6	6' - 7"	742	2' - 6"	4' - 1"	108	#5	9"	6' - 9"	760	4' - 1"	' 2' -	8"	108 9"	2' - 0"	144	5 39' - 9	)" 133	25	39' - 9"	664	6' - 11"	18	16 4	45 0.440	89.1	1 0.5	63 18	18.1	3,628
6' - 0"	2' - 0"	9"	7"	26'	108	#6	9"	6' - 11"	1,122	162	#5 6	6" 6	6' - 8"	1,126	2' - 7"	4' - 1"	162	#5	6"	6' - 10'	1,155	4' - 1"	' 2' -	9"	108 9"	2' - 0"	144	5 39' - 9	)" 133	25	39' - 9"	664	6' - 11"	18	16 4	45 0.485	108.6	0.5	63 19	19.9	4,407
6' - 0"	2' - 0"	10"	8"	30'	108	#6	9"	7' - 1"	1,149	162	#5 6	6" 6	6' - 10"	1,155	2' - 8"	4' - 2"	162	#5	6"	7' - 0"	1,183	4' - 2'	' 2' -	10"	82 12"	2' - 0"	110	5 39' - 9	)" 133	25	39' - 9"	664	7' - 1"	19	18 5	50 0.551	109.9	0.5	69 22	22.6	4,463
6' - 0"	3' - 0"	8"	7"	20'	108	#6	9"	6' - 11"	1,122	108	#5 9	9" 7	7' - 7"	854	3' - 6"	4' - 1"	108	#5	9"	6' - 9"	760	4' - 1"	' 2' -	8"	108 9"	3' - 0"	216	5 39' - 9	)" 133	29	39' - 9"	770	6' - 11"	18	16 4	45 0.484	96.4	4 0.5	63 19	19.9	3,918
6' - 0"	3' - 0"	9"	7"	26'	108	#6	9"	6' - 11"	1,122	162	#5 6	5" 7	7' - 8"	1,295	3' - 7"	4' - 1"	162	#5	6"	6' - 10'	1,155	4' - 1"	' 2' -	9"	108 9"	3' - 0"	216	5 39' - 9	)" 133	29	39' - 9"	770	6' - 11"	18	16 4	45 0.528	117.3	0.5	63 2	21.6	4,754
6' - 0"	3' - 0"	10"	8"	30'	108	#6	9"	7' - 1"	1,149	162	#5 6	5" 7	7' - 10"	1,324	3' - 8"	4' - 2"	162	#5	6"	7' - 0"	1,183	4' - 2'	' 2' -	10"	82 12"	3' - 0"	164	5 39' - 9	)" 133	29	39' - 9"	770	7' - 1"	19	18	50 0.601	118.1	0.5	69 2	24.6	4,792
6' - 0"	4' - 0"	8"	7"	20'	108	#6	9"	6' - 11"	1,122	108	#5 9	9" 8	8' - 7"	967	4' - 6"	4' - 1"	108	#5	9"	6' - 9"	760	4' - 1"	' 2' -	8"	108 9"	4' - 0"	289	5 39' - 9	)" 133	29	39' - 9"	770	6' - 11"	18	16 4	45 0.527	101.0	0.5	63 2	21.6	4,104
6' - 0"	4' - 0"	9"	7"	26'	108	#6	9"	6' - 11"	1,122	162	#5 6	8" 8	8' - 8"	1,464	4' - 7''	4' - 1"	162	#5	6"	6' - 10'	1,155	4' - 1"	' 2' -	9"	108 9"	4' - 0"	289	5 39' - 9	)" 133	29	39' - 9"	770	6' - 11"	18	16	45 0.571	123.3	, 0.5	63 2	23.4	4,996
6' - 0"	4' - 0"	10"	8"	30'	108	#6	9"	7' - 1"	1,149	162	#5 6	8" 8	8' - 10"	1,493	4' - 8"	4' - 2"	162	#5	6"	7' - 0''	1,183	4' - 2'	' 2' -	10"	82 12"	4' - 0"	219	5 39' - 9	)" 133	29	39' - 9"	770	7' - 1"	19	18	50 0.650	123.7	0.5	69 2	26.5	5,016
6' - 0"	5' - 0"	8"	7"	20'	108	#6	9"	6' - 11"	1,122	108	#5 9	9" 9	9' - 7"	1,080	5' - 6"	4' - 1"	108	#5	9"	6' - 9"	760	4' - 1"	' 2' -	8"	108 9"	5' - 0"	361	5 39' - 9	)" 133	33	39' - 9"	876	6' - 11"	18	16	45 0.570	108.3	, 0.5	63 2	23.3	4,395
6' - 0"	5' - 0"	9"	7"	26'	108	#6	9"	6' - 11"	1,122	162	#5 6	6" 9	9' - 8"	1,633	5' - 7"	4' - 1"	162	#5	6"	6' - 10'	' 1,155	4' - 1"	' 2' -	9"	108 9"	5' - 0"	361	5 39' - 9	)" 133	33	39' - 9"	876	6' - 11"	18	16 4	45 0.614	132.0	0.5	63 2	25.1	5,343
6' - 0"	5' - 0"	10"	8"	30'	108	#6	9"	7' - 1"	1,149	162	#5 6	6" 9	9' - 10"	1,661	5' - 8"	4' - 2"	162	#5	6"	7' - 0"	1,183	4' - 2'	' 2' -	10"	82 12"	5' - 0"	274	5 39' - 9	)" 133	33	39' - 9"	876	7' - 1"	19	18	50 0.700	131.9	0.5	69 2	28.5	5,345
6' - 0"	6' - 0"	8"	7"	20'	108	#6	9"	6' - 11"	1,122	108	#5 9	9" 10'	0' - 7"	1,192	6' - 6"	4' - 1"	108	#5	9"	6' - 9"	760	4' - 1"	' 2' -	8"	108 9"	6' - 0''	433	5 39' - 9	)" 133	37	39' - 9"	982	6' - 11"	18	16	45 0.613	115.6	0.5	63 2	25.0	4,685
6' - 0"	6' - 0"	9"	7"	26'	108	#6	9"	6' - 11"	1,122	162	#5 6	5" 10	)' - 8"	1,802	6' - 7"	4' - 1"	162	#5	6"	6' - 10'	1,155	4' - 1"	' 2' -	9"	108 9"	6' - 0"	433	5 39' - 9	)" 133	37	39' - 9"	982	6' - 11"	18	16 4	45 0.657	140.7	0.5	63 2	26.8	5,690
6' - 0"	6' - 0"	10"	8"	30'	108	#6	9"	7' - 1"	1,149	162	#5 6	5" 10	0' - 10"	1,830	6' - 8"	4' - 2"	162	#5	6"	7' - 0"	1,183	4' - 2'	' 2' -	10"	82 12"	6' - 0''	329	5 39' - 9	)" 133	37	39' - 9"	982	7' - 1"	19	18 5	50 0.749	140.2	2 0.5	69 3	30.5	5,675

5 For direct traffic culverts (fill height ≤ 2 ft.), identify the required box size and select the option with the minimum fill height.

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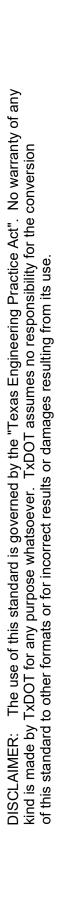
SHEET 2 OF 2

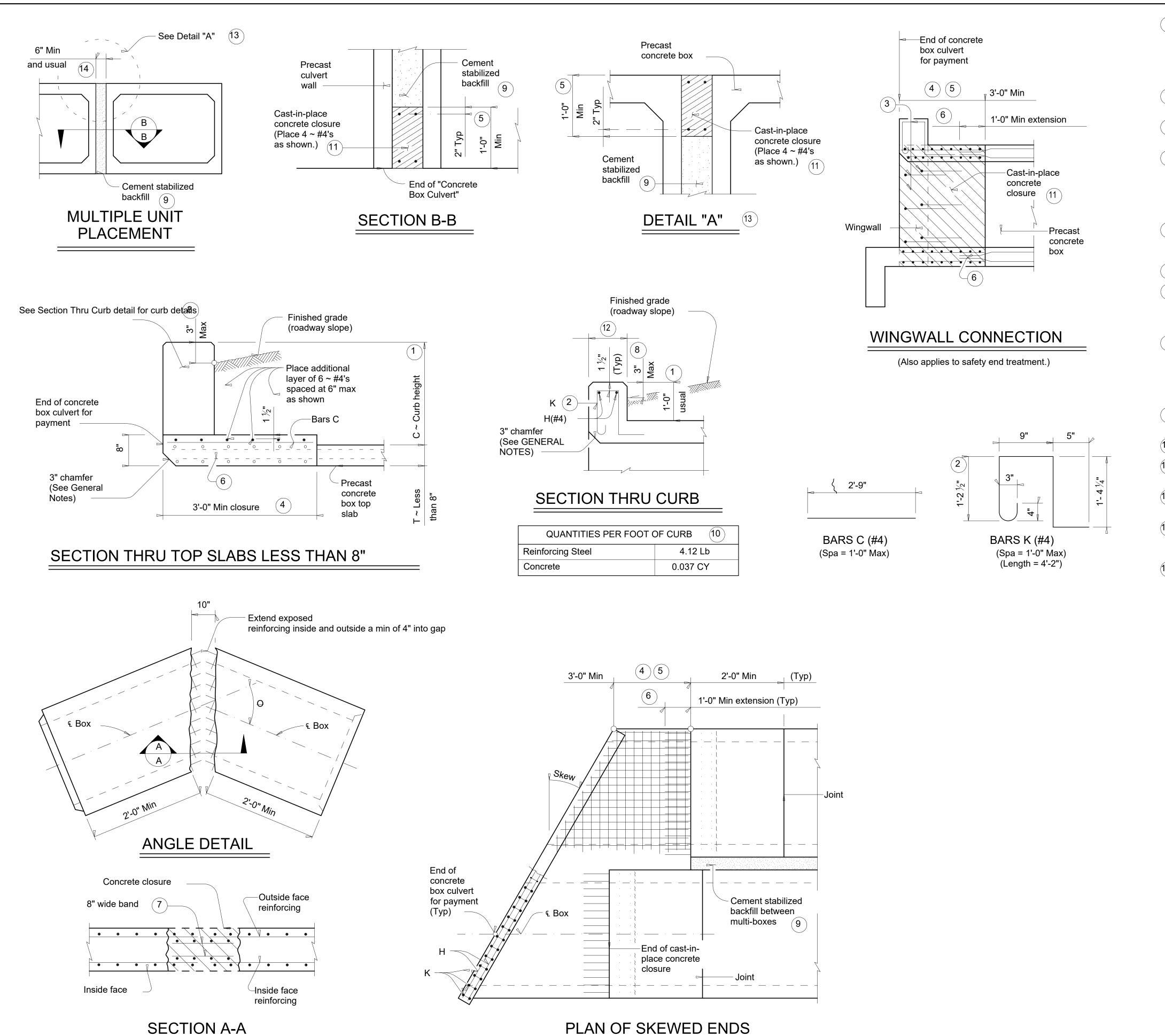
Texas Department of Transportation

SINGLE BOX CULVERTS
CAST-IN-PLACE
0' TO 30' FILL

SCC-5 & 6

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TxDOT February 2020	CONT	SECT	JOB		HIG	HWAY
REVISIONS						
2021 Updated X values.	DIST		COUN	ГΥ	:	SHEET NO.
					3	33 OF 49





(Showing multi-box placement.)

- 0" Min to 5'-0" Max. Estimated curb heights are shown elsewhere in the plans. For structures with pedestrian rail, bicycle rail, or curbs taller than 1'-0, refer to the Extended Curb Details (ECD) standard sheet. For structures with T631 or T631LS bridge rail, refer to the Mounting Details for T631 & T631LS Rails (T631-CM) standard sheet. Refer to the Box Culvert Rail Mounting Details (RAC) standard sheet for structures with bridge rail other than T631 or T631LS.
- For curbs less than 1'-0" high, tilt Bars K or reduce bar height as necessary to maintain cover. For curbs less than 3" high, Bars K may be omitted.
- Extend curb, wingwall, or safety end treatment reinforcing into concrete closure. Bend or trim, as necessary, any reinforcing that does not fit into closure area.
- Provide a 3'-0" Min cast-in-place concrete closure. Break back boxes in the field or cast boxes short. Provide bands of reinforcing in the closure that are the same size and spacing as in the precast box section. Provide #4 longitudinal reinforcement spaced at 12 inches Max within the closure. Except where shown otherwise, construct the cast-in-place closure flush with the inside and outside faces of the precast box section.
- For multiple unit placements, adjust the length of the closure for the interior walls as necessary. Provide a 3'-0" Min cast-in-place closure in the top slab, bottom slab, and exterior wall. See Section B-B detail when interior walls are cast full length.
- $\binom{6}{}$  Extend precast box reinforcing a minimum of 1'-0" into concrete closure (Typ).
- Place bands of reinforcing matching the inside and outside face reinforcing in the gaps of the top and bottom slabs. Place a band matching the outside face reinforcing of the wall in the gaps of the walls (placed in the outside face only). Tack weld the bands to the exposed reinforcing at each point of contact.
- For vehicle safety, the following requirements must be met:

   For structures without bridge rail, construct curbs no more than 3" above finished grade
  - · For structures with bridge rail, construct curbs flush with finished grade.

    Reduce curb heights, if necessary, to meet the above requirements. No changes will be made in quantities and no additional compensation will be allowed for this work.
- 9 Cement stabilized backfill between boxes is considered part of the box culvert for payment.
- O All curb concrete and reinforcing is considered part of the box culvert for payment.
- Any additional concrete and reinforcing required for the closures will be considered subsidiary to the box culvert for payment.
- 1'-0" typical. 2'-3" when the Box Culvert Rail Mounting Details (RAC) standard sheet is referred to elsewhere in the plans.
- For multiple unit placement with overlay, with 1 to 2 course surface treatment, or with the top slab as the final riding surface, provide wall closure as shown in Detail "A".
- This dimension may be increased with approval of the Engineer to allow the precast boxes to be tunneled or jacked in accordance with Item 476, "Jacking, Boring, or Tunneling Pipe or Box". No payment will be made for any additional material in the gap between adjacent boxes.

#### **MATERIAL NOTES:**

Provide Grade 60 reinforcing steel.

Provide ASTM A1064 welded wire reinforcement.

Provide Class C concrete (f'c = 3,600 psi) for the closures.

Provide cement stabilized backfill meeting the requirements of Item 400.

"Excavation and Backfill for Structures."

Any additional concrete required for the closures will be considered

subsidiary to the box culvert.

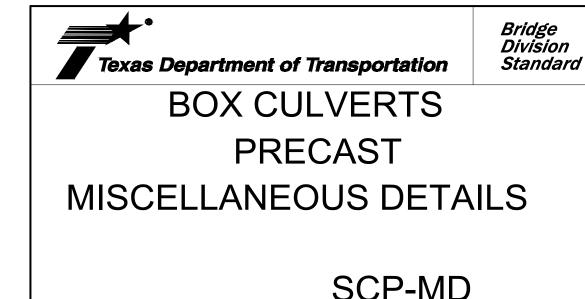
#### **GENERAL NOTES:**

Designed according to AASHTO LRFD Bridge Design Specifications. Refer to the Single Box Culverts Precast (SCP) standard sheets for details and notes not shown.

Chamfer the bottom edge of the top slab closure 3 inches at culvert closure ends.

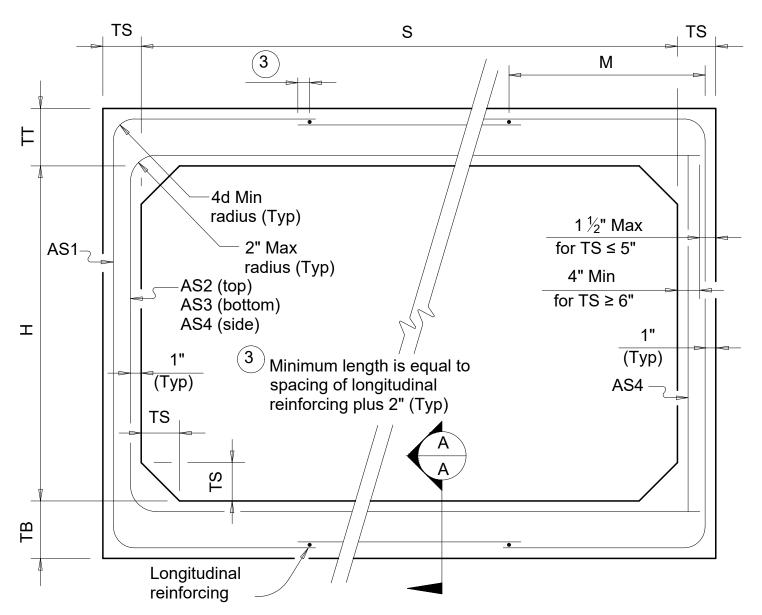
Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bars dimensions are out-to-out of bars.

#### HL93 LOADING

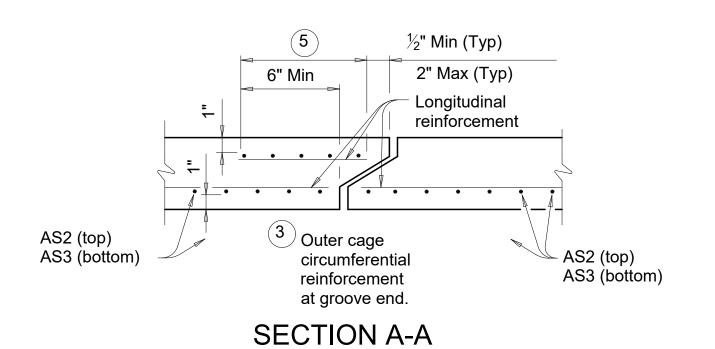


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©TxDOT	February 2020	CONT	SECT	JOB		HIGHWAY		<b>′</b>
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						ВС	X DA	TA						
SECTION DIMENSIONS				Fill	M		RE	REINFORCING (sq. in. / ft.)					1 Lift	
S	Н	TT	ТВ	TS	Height	(Min)	AS1	AS2	AS3	AS4	AS5	AS7	AS8	Weigh
(ft.)	(ft.)	(in.)	(in.)	(in.)	(ft.)	(in.)								(tons)
5	2	8	7	6	< 2	_	0.19	0.27	0.18	0.14	0.19	0.19	0.17	6.0
5	2	6	6	6	2 < 3	44	0.22	0.20	0.16	0.14	-	-	-	5.1
5	2	6	6	6	3 - 5	44	0.16	0.14	0.14	0.14	-	-	-	5.1
5	2	6	6	6	10	36	0.15	0.14	0.14	0.14	-	-	-	5.1
5	2	6	6	6	15	36	0.20	0.18	0.18	0.14	-	-	-	5.1
5	2	6	6	6	20	36	0.26	0.23	0.24	0.14	-	-	-	5.1
5	2	6	6	6	25	36	0.33	0.29	0.29	0.14	-	-	-	5.1
5	2	6	6	6	30	36	0.39	0.34	0.35	0.14	-	-	-	5.1
5	3	8	7	6	< 2	-	0.19	0.31	0.21	0.14	0.19	0.19	0.17	6.6
5	3	6	6	6	2 < 3	45	0.18	0.24	0.19	0.14	-	-	-	5.7
5	3	6	6	6	3 - 5	36	0.14	0.17	0.16	0.14	-	-	_	5.7
5	3	6	6	6	10	36	0.14	0.16	0.17	0.14	-	-	-	5.7
5	3	6	6	6	15	35	0.16	0.21	0.22	0.14	-	-	_	5.7
5	3	6	6	6	20	35	0.21	0.27	0.28	0.14	-	-	_	5.7
5	3	6	6	6	25	35	0.26	0.34	0.34	0.14	-	-	-	5.7
5	3	6	6	6	30	35	0.31	0.41	0.41	0.14	-	-	-	5.7
5	4	8	7	6	< 2	-	0.19	0.33	0.24	0.14	0.19	0.19	0.17	7.2
5	4	6	6	6	2 < 3	45	0.16	0.27	0.22	0.14	-	-	_	6.3
5	4	6	6	6	3 - 5	45	0.14	0.19	0.18	0.14	-	-	_	6.3
5	4	6	6	6	10	36	0.14	0.18	0.18	0.14	-	-	-	6.3
5	4	6	6	6	15	35	0.14	0.23	0.24	0.14	-	-	-	6.3
5	4	6	6	6	20	35	0.17	0.30	0.31	0.14	-	-	-	6.3
5	4	6	6	6	25	35	0.21	0.37	0.38	0.14	-	-	-	6.3
5	4	6	6	6	30	35	0.25	0.44	0.45	0.14	-	-	-	6.3
5	5	8	7	6	< 2	-	0.19	0.35	0.26	0.14	0.19	0.19	0.17	7.8
5	5	6	6	6	2 < 3	45	0.14	0.29	0.24	0.14	-	-	-	6.9
5	5	6	6	6	3 - 5	45	0.14	0.21	0.20	0.14	-	-	-	6.9
5	5	6	6	6	10	45	0.14	0.19	0.20	0.14	-	-	_	6.9
5	5	6	6	6	15	36	0.14	0.24	0.25	0.14	-	-	_	6.9
5	5	6	6	6	20	35	0.15	0.31	0.32	0.14	-	-	-	6.9
5	5	6	6	6	25	35	0.18	0.38	0.39	0.14	-	-	_	6.9
5	5	6	6	6	30	35	0.21	0.46	0.47	0.14	-	-	_	6.9
								1		1		1		



#### FILL HEIGHT 2 FT AND GREATER



(Showing top and bottom slab joint reinforcement.)

MATERIAL NOTES:
Provide 0.03 sq. in./ft. minimum longitudinal reinforcement at each face in slabs and walls. This minimum requirement may be met by the transverse wires when wire mesh reinforcement is used.

Provide Class H concrete (f`c = 5,000 psi).

#### **GENERAL NOTES:**

Designs shown conform to ASTM C1577. Refer to ASTM C1577 for information or details not shown.

See Box Culverts Precast Miscellaneous Details (SCP-MD) standard sheet for details and notes not shown.

In lieu of furnishing the designs shown on this sheet, the contractor may furnish an alternate design that is equal to or exceeds the box design for the design fill height in the table. Submit shop plans for alternate designs in accordance with Item "Precast Concrete Structural Members (Fabrication)".

#### **HL93 LOADING**



**PRECAST** 

5'-0" SPAN

SCP-5

Bridge Division

Standard

FILE:	scp05sts-20.dgn	DN: TxD	TC	ck: TxDOT	DW: Tx[	TOC	CK:	TxDOT
C)TxDOT	February 2020	CONT	SECT	JOB	JOB			Y
	REVISIONS							
		DIST		COUNTY			SHEET NO.	
							35 (	OF 49

1 For box length = 8'-0"

AS1 thru AS4, AS7 and AS8 are minimum required areas of reinforcement per linear foot of box length. AS5 is minimum required area of reinforcement per linear foot of box width.



CORNER OPTION "A"

CORNER OPTION "B"

**CORNER OPTION "A"** 

4d Min

1" (Typ unless

<sup>⊸</sup>(Typ)

noted otherwise)

radius (Typ)

2" Max

radius

(Typ)

CORNER OPTION "B"

\_AS3

TS

AS1

1" Max

for TS ≤ 5"

4" Min

for TS ≥ 6"

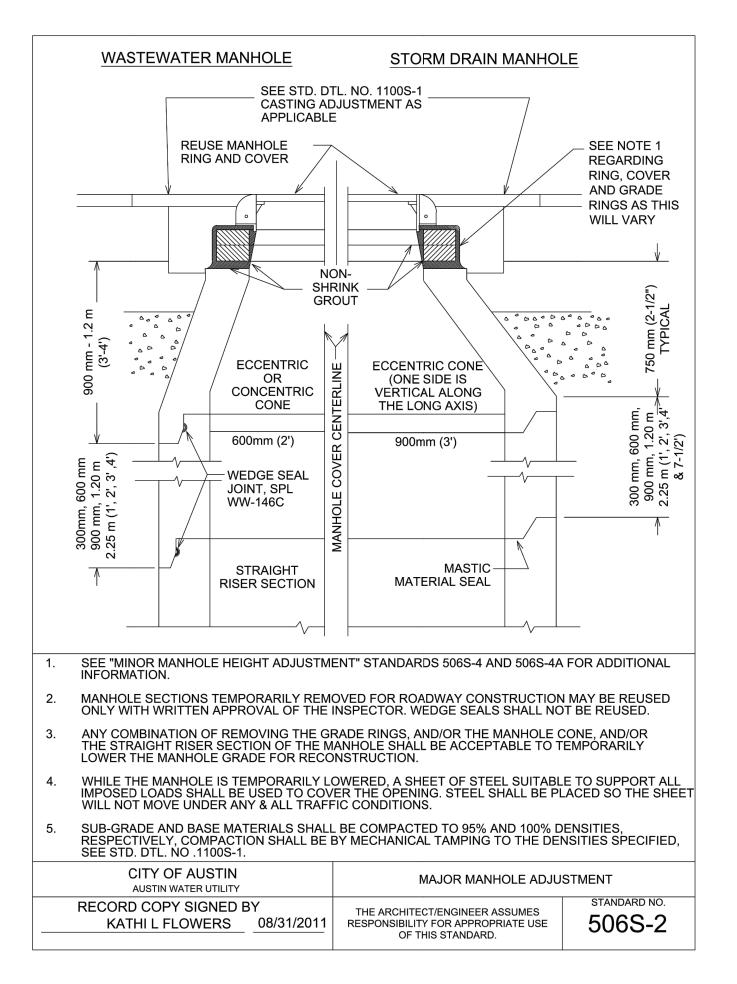
4

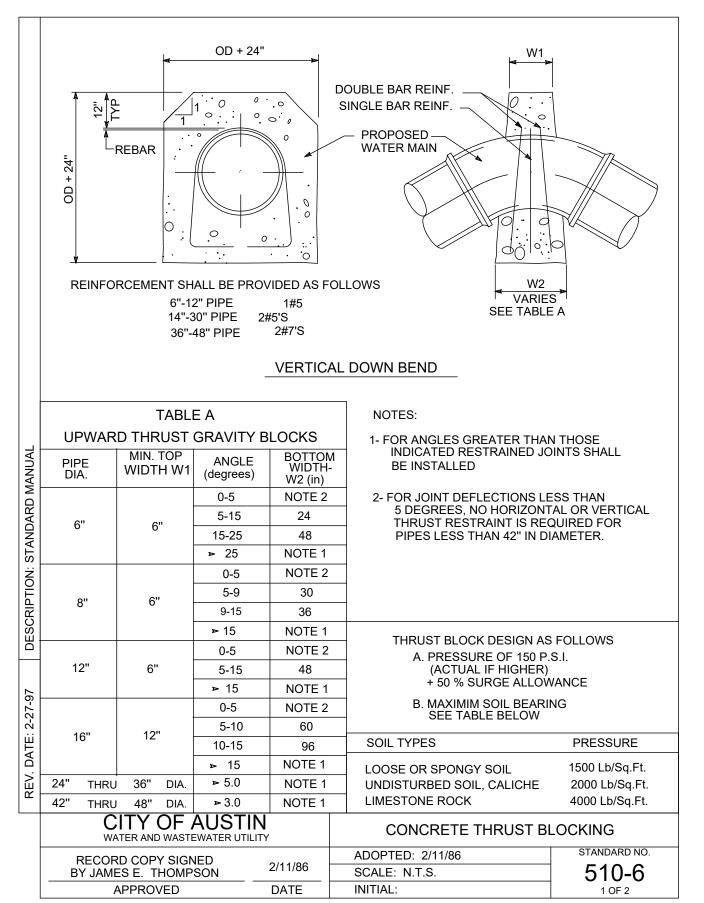
#### FILL HEIGHT LESS THAN 2 FT

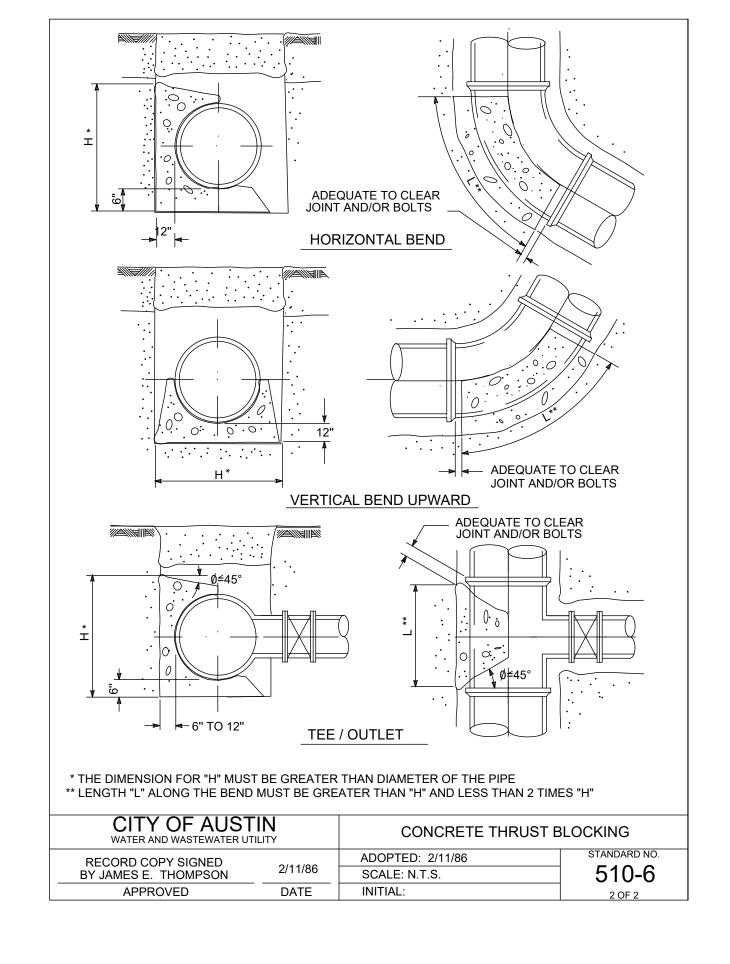
Length is equal to spacing of longitudinal reinforcing plus 2". (10" Min) (Typ)

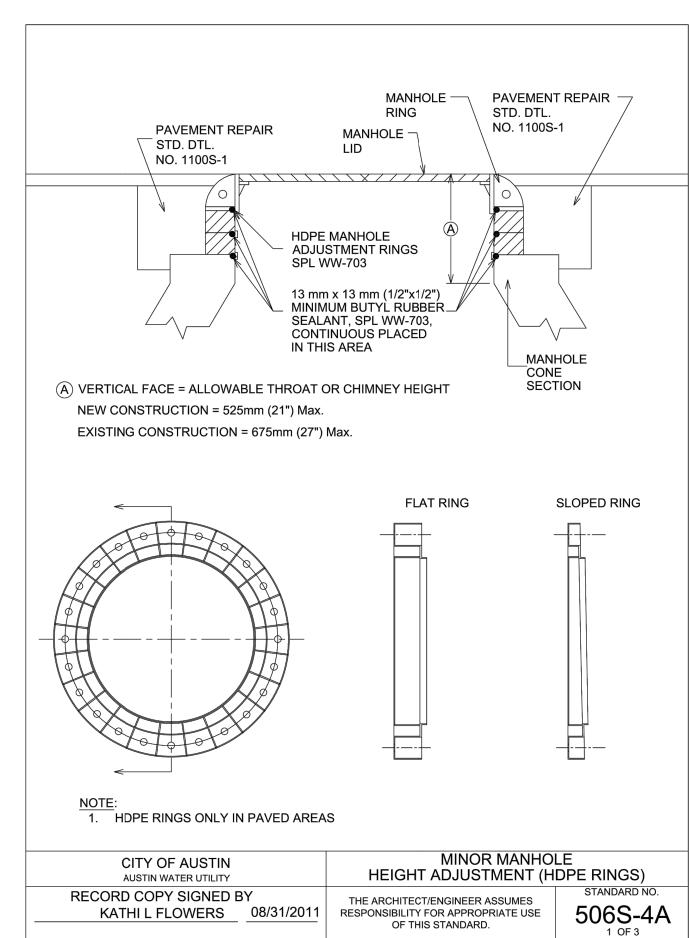
S

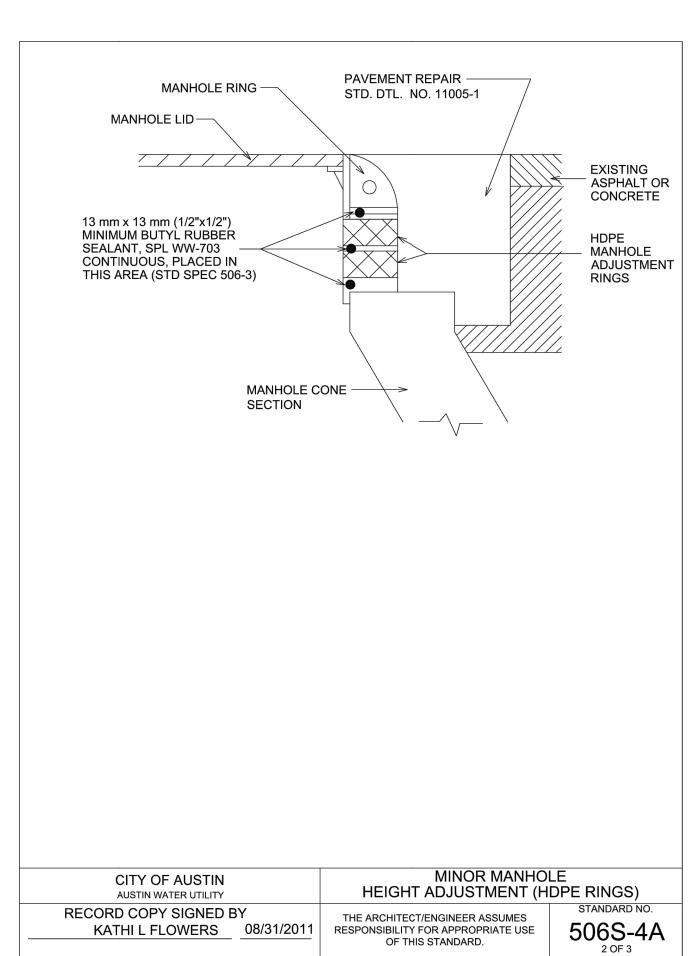
\_AS2 \_\_AS7











#### **INSTALLATION INSTRUCTIONS** HDPE MANHOLE ADJUSTMENT RINGS (FOR NEW CONSTRUCTION AND REHABILITATION)

- REMOVE EXISTING CASTING AND RINGS OR COVER PLATE TO EXPOSE CONCRETE. MANHOLE SURFACE MUST BE CLEAN AND LEVEL ±6 mm (1/4"). NOTE: IF TOP OF CONCRETE STRUCTURE IS IN VERY POOR CONDITION OR IRREGULAR, THE FIRST RING CAN BE PRESSED FULLY INTO A HEAVY BED OF MORTAR ALLOWING IT AND SUBSEQUENT RINGS TO BE POSITIONED WITHOUT WOBBLE OR POINT-LOADING.
- POSITION STRAIGHT AND LEVEL TO DETERMINE HEIGHT ADJUSTMENT REQUIRED. CONFIRM BEFORE PLACING RINGS.
- COMBINE VARIOUS RING HEIGHTS, IN BOTH FLAT AND SLOPE, TO REACH DESIRED ADJUSTMENT. PLACE RINGS WITH INNER POCKET OPENING UP (OUTER POCKET OPENING WITH HOLES DOWN). THE INTERLOCKING LIP (MALE TONGUE) EXTENDS DOWN INTO THE STRUCTURE OF LOWER RING. MULTIPLE SLOPE RINGS CAN BE ADDED AND ROTATED TO ACHIEVE DESIRED GRADE MATCH WITHIN 6 mm (1/4"), NOTE: SLOPE RINGS ARE TAPERED TO PROVIDE 2% GRADE COMPENSATION, IF REQUIRED, HDPE SHIMS (MAX. 6 mm (1/2") PER RING MAY BE USED TO MATCH GRADE EXACTLY.
- DRY STACK RINGS WITHOUT SEALANT TO ACHIEVE DESIRED VERTICAL AND SLOPE ADJUSTMENT. IF SLOPE RINGS ARE USED, MARK INNER FACE OF STOCK WITH PAINT OR MARKER TO MAINTAIN ALIGNMENT WHEN RE-STACKING.
- REMOVE THE RINGS AND PLACE NEARBY IN UPSIDE-DOWN ORDER (THIS ALLOWS FOR EASY RE-ASSEMBLY IN THE CORRECT ORDER). APPLY A CONTINUOUS BEAD OF APPROVED RUBBER BUTYL SEALANT (SPL WW-703) TO THE FLAT SURFACE OF THE FIRST RING ON THE SIDE OF THE MALE TONGUE THAT WILL REST AGAINST THE CONE OPENING (MALE TONGUE DOWN). APPLY SEALANT TO AFFECT A SEAL BETWEEN THE INTERLOCKING LIP AND THE CONCRETE SURFACE IT
- APPLY 13 mm (1/2") CONTINUOUS BEAD OF APPROVED BUTYL RUBBER SEALANT (SPL WW-703) TO THE NEXT RING. THE SEALANT MUST BE APPLIED DIRECTLY INTO THE INTERLOCKING LIP (MALE TONGUE) AND THE BOTTOM OF THE RING, FAVORING THE SIDE OF THE LIP TO ASSURE A SEAL. PLACE THIS RING WITH THE INTERLOCKING LIP (MALE TONGUE) FACING DOWN ON TOP OF THE LOWER RING PREVIOUSLY INSTALLED. PRESS THE RING IN PLACE TO SET AND DISPERSE SEALANT. REMEMBER TO RE-ASSEMBLE THE RINGS USING ALIGNMENT MARKS TO POSITION SLOPE
- REPEAT #6 ON ALL PRE-ASSEMBLED RINGS TO REACH DESIRED HEIGHT AND SLOPE ADJUSTMENT.
- APPLY 13 mm (1/2") CONTINUOUS BEAD OF APPROVED BUTYL RUBBER SEALANT (SPL WW-703), TO THE TOP OF THE UPPER RING.
- POSITION CASTING ON RING(S). PRESS THE CASTING FRAME IN PLACE TO SET AND DISPERSE
- 10. BACKFILL TO RESTORE TO STD. DETAIL 1100S-1.
- 11. RING MANUFACTURER MUST BE LISTED ON SPL WW-703.
- 12. FOR USE IN PAVED AREAS ONLY.

CITY OF AUSTIN AUSTIN WATER UTILITY	MINOR MANHOLE HEIGHT ADJUSTMENT (HDPE RINGS)					
RECORD COPY SIGNED BY KATHI L FLOWERS 08/31/2011	THE ARCHITECT/ENGINEER ASSUMES RESPONSIBILITY FOR APPROPRIATE USE OF THIS STANDARD.	STANDARD NO. 506S-4A 3 OF 3				

REVISION DESCRIPTION						
DATE						
ВУ						
REV. BY NO.						

#### 100% SUBMITTAI

THIS DOCUMENT IS RELEASED FOR THE PURPOSE OF INTERIM **REVIEW UNDER** THE AUTHORITY OF ABE A. SALINAS III, P.E. LIC. # 105144 4/12/2023

IT IS NOT TO BE USED FOR CONSTRUCTION, BIDDING OR PERMIT PURPOSES.

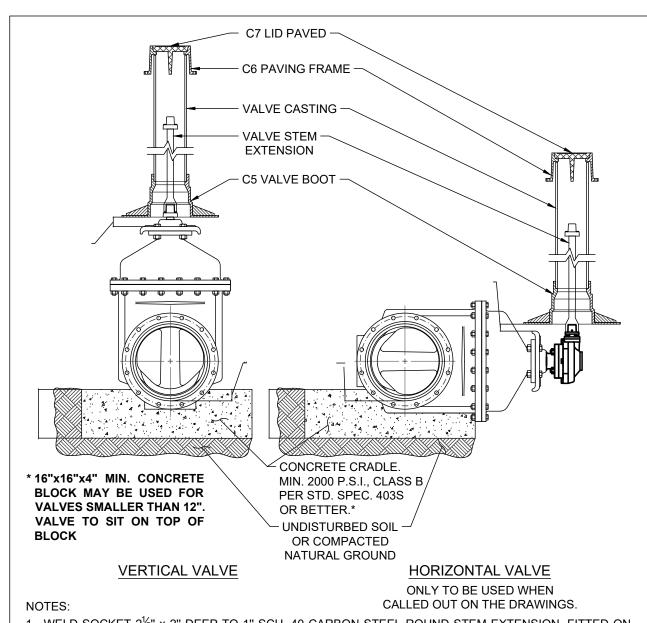
ROLLING\ RAINAGE

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**RELLINGWOOD K·FRIESE** + ASSOCIATES

PUBLIC PROJECT ENGINEERING 1120 S. Capital of Texas Highway CityView 2, Suite 100 Austin, Texas 78746 P - 512.338.1704 F - 512.338.1784 TBPE Firm #6535

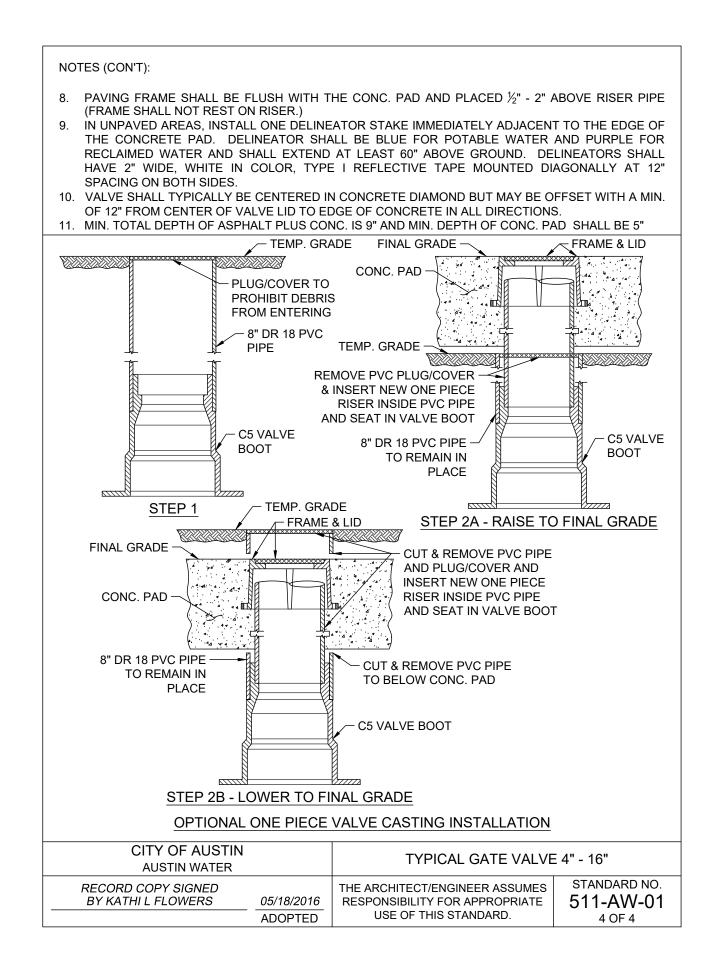
www.ktriese.com				
NOTES	NAME	DATE		
SURVEY BY				
DRAWN BY	AH	08/21		
DESIGNED BY	LWM	08/21		
CHECKED BY	GE	08/21		
REVIEWED BY	PS	09/21		
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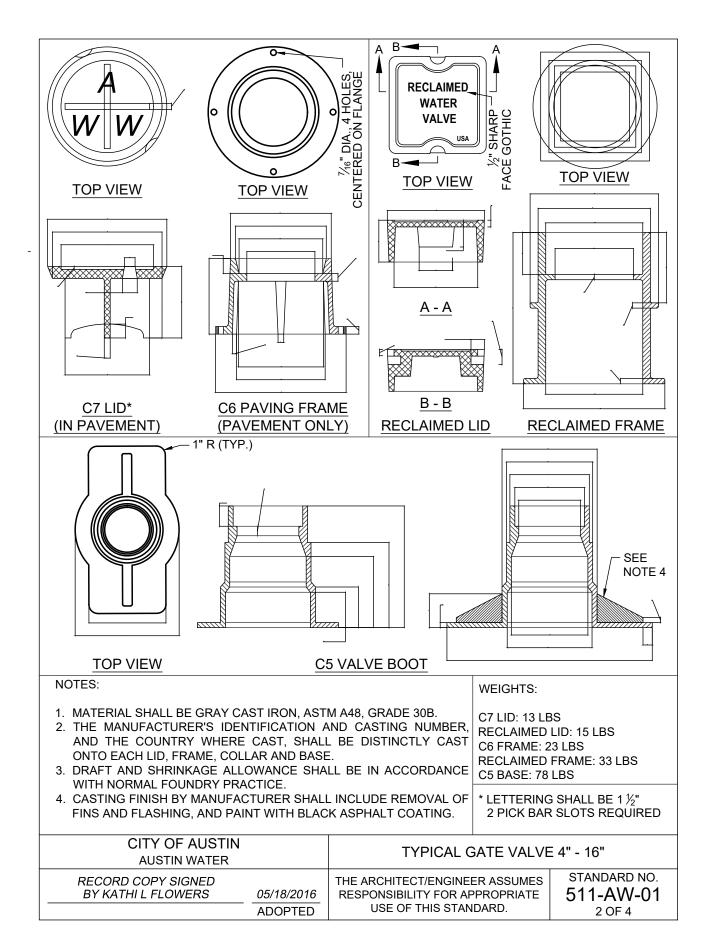


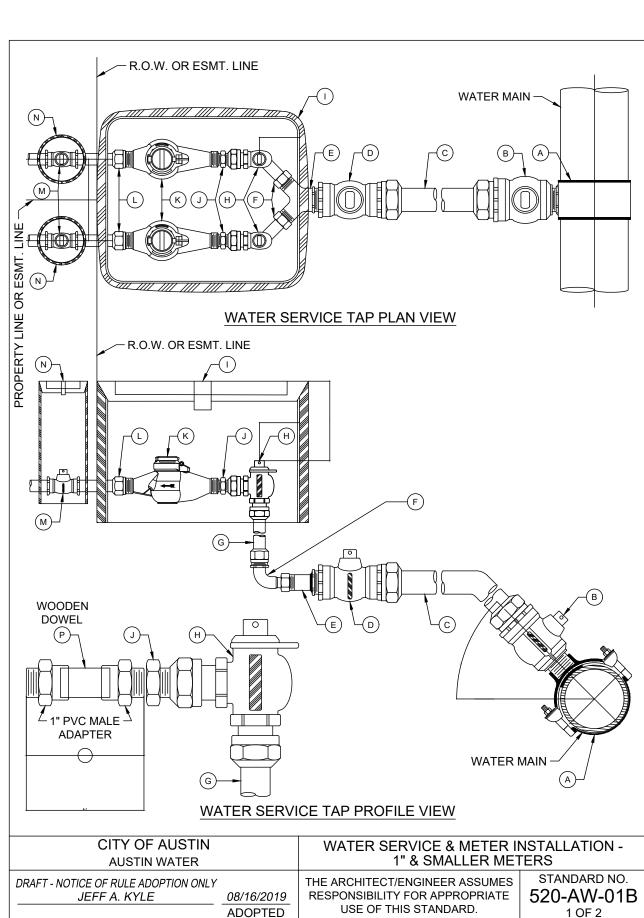
. WELD SOCKET 2½" x 2" DEEP TO 1" SCH. 40 CARBON STEEL ROUND STEM EXTENSION, FITTED ON OPERATING NUT, [SCH. 80 FOR LENGTHS OVER 10'.] 2. VALVE CASTING SHALL BE 6" DI PIPE WITH BELL OR COLLAR CENTERED OVER VALVE BOOT. 3. NUT AT TOP OF VALVE EXTENSION ROD SHALL BE SQUARE 2" LONG WELDED TO TOP OF ROD. 4. VALVE STEM EXTENSIONS ARE REQUIRED ON ALL VALVES THAT EXCEED 3' DEEP FROM FINISHED GRADE. VALVE EXTENSIONS SHALL BE PLACED SUCH THAT THE EXTENSION NUT IS BETWEEN 12" AND 18" FROM FINISHED GRADE.

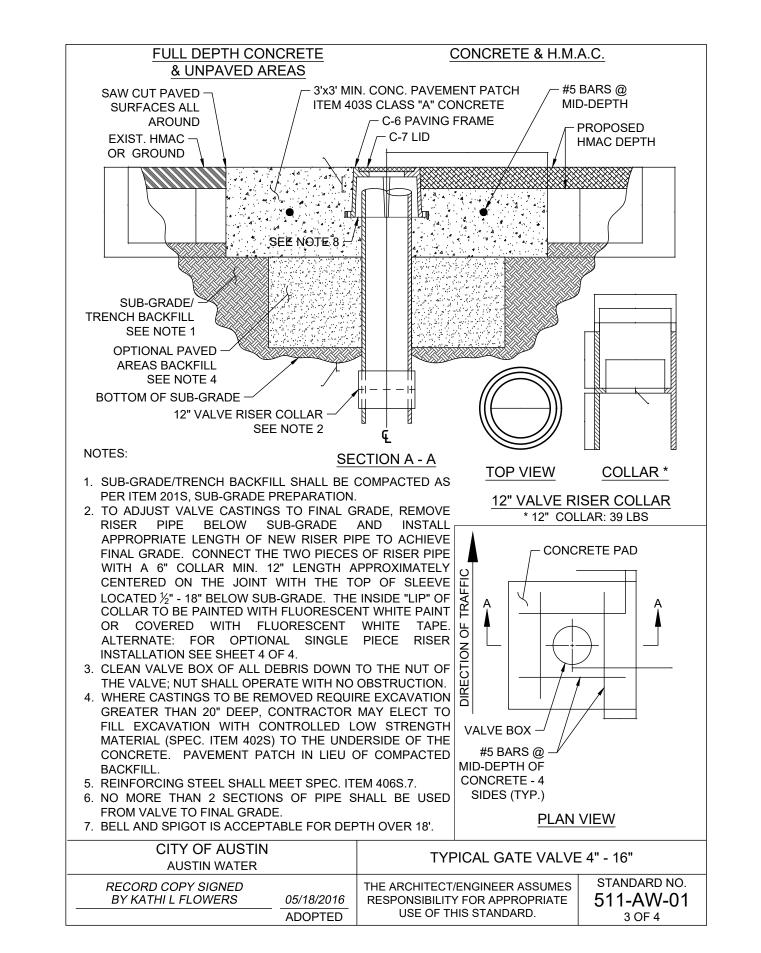
RECLAIMED WATER: ALL RECLAIMED PVC PIPE SHALL BE MANUFACTURED PURPLE PIPE. HDPE PIPE SHALL BE MANUFACTURED WITH PURPLE STRIPES. ALL OTHER PIPE AND APPURTENANCES SHALL BE MANUFACTURED PURPLE IF AVAILABLE. ALL PIPE AND FITTINGS THAT ARE NOT AVAILABLE FROM THE MANUFACTURER IN PURPLE SHALL BE PAINTED PURPLE PER SPL WW-3C. ALL BURIED DI AND CI PIPE AND FITTINGS SHALL ALSO BE WRAPPED IN PURPLE POLYETHYLENE PER SPL WW-27D. ALL COVERS SHALL HAVE "RECLAIMED WATER" CAST INTO THEM.

CITY OF AUSTIN AUSTIN WATER		TYPICAL GATE VALVE	∃ 4" - 16"
		ARCHITECT/ENGINEER ASSUMES PONSIBILITY FOR APPROPRIATE USE OF THIS STANDARD.	STANDARD NO. 511-AW-01





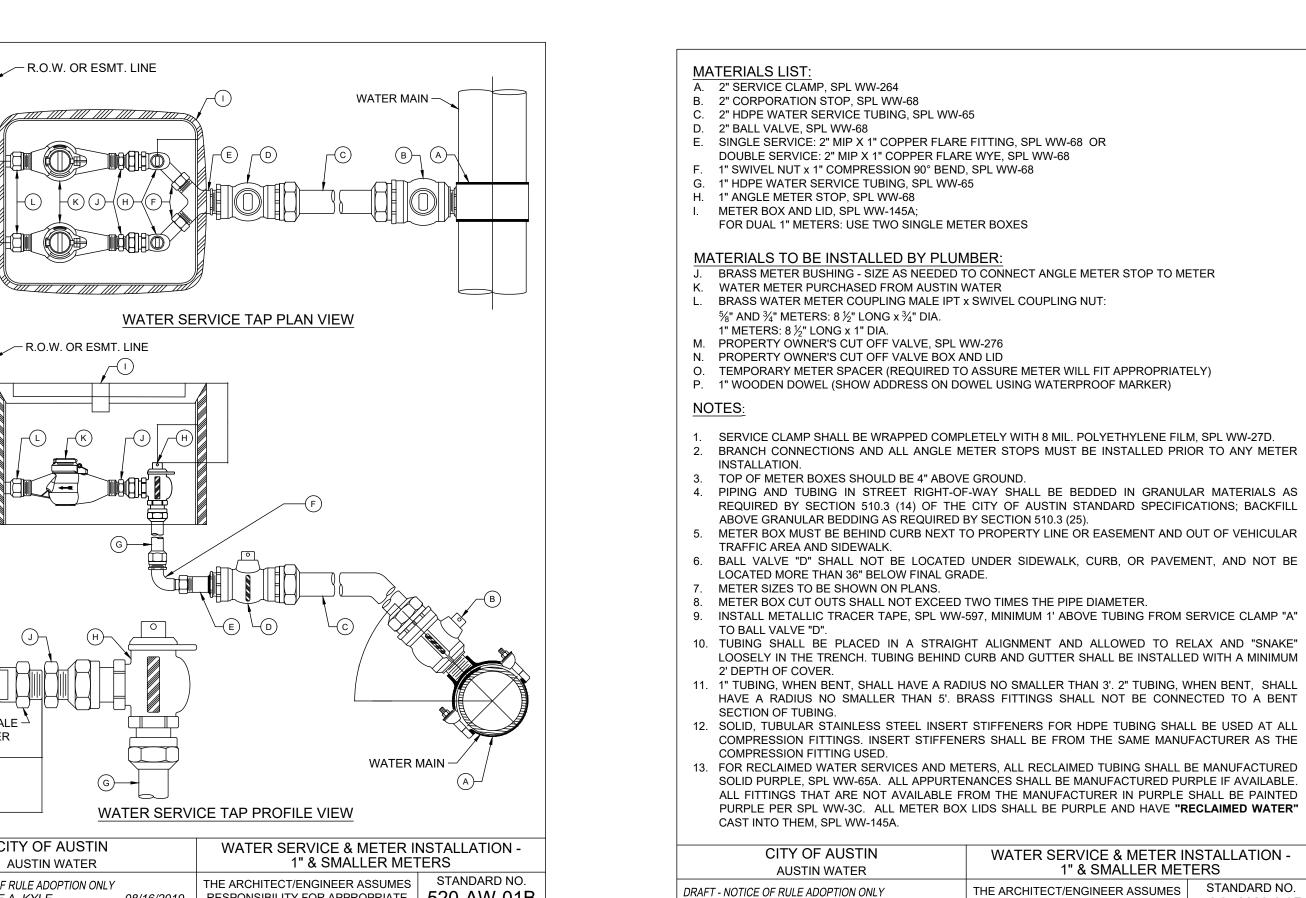




RESPONSIBILITY FOR APPROPRIATE 520-AW-01B

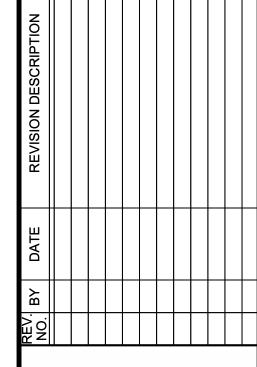
2 OF 2

USE OF THIS STANDARD.



JEFF A. KYLE

ADOPTED



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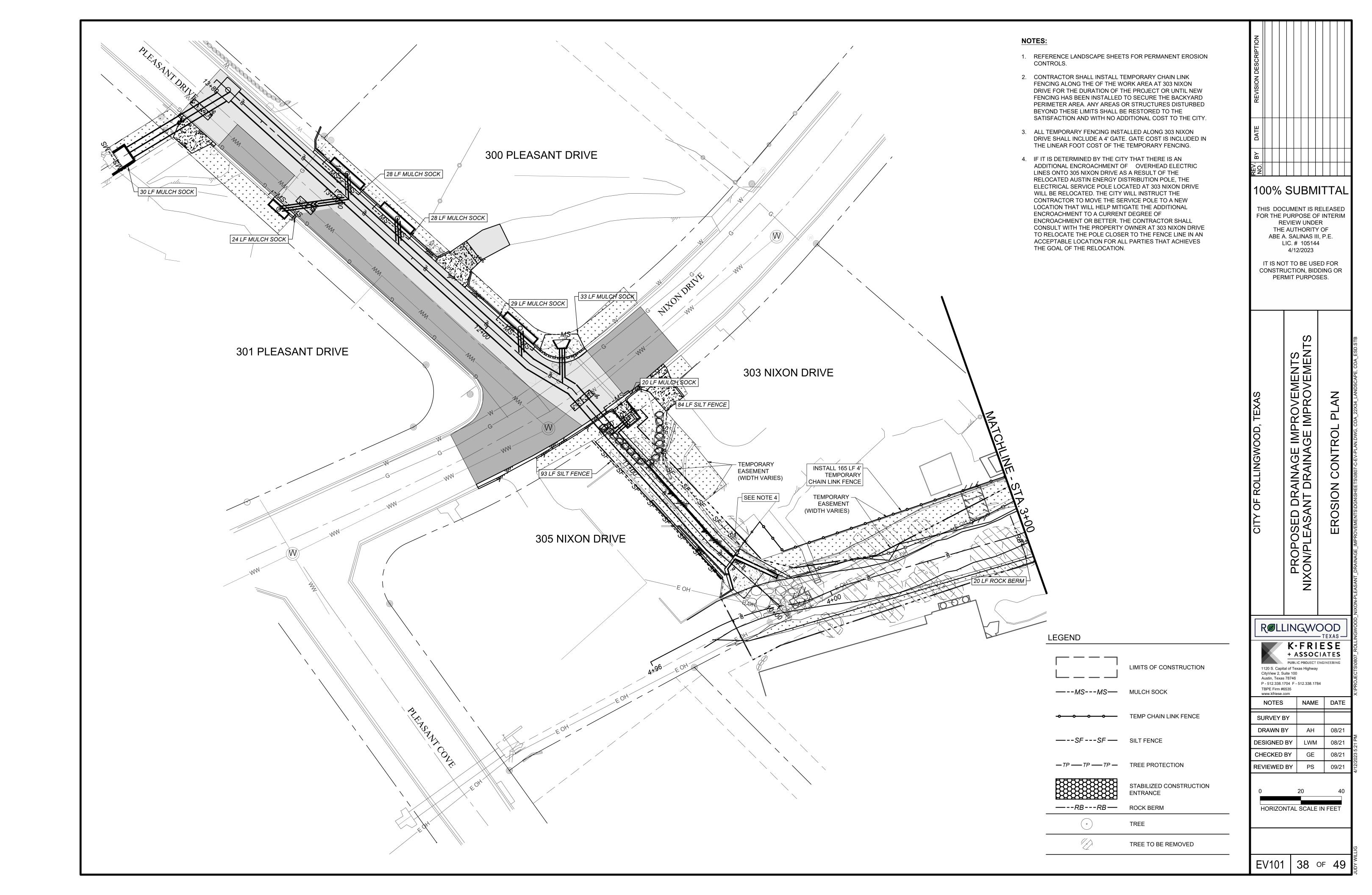
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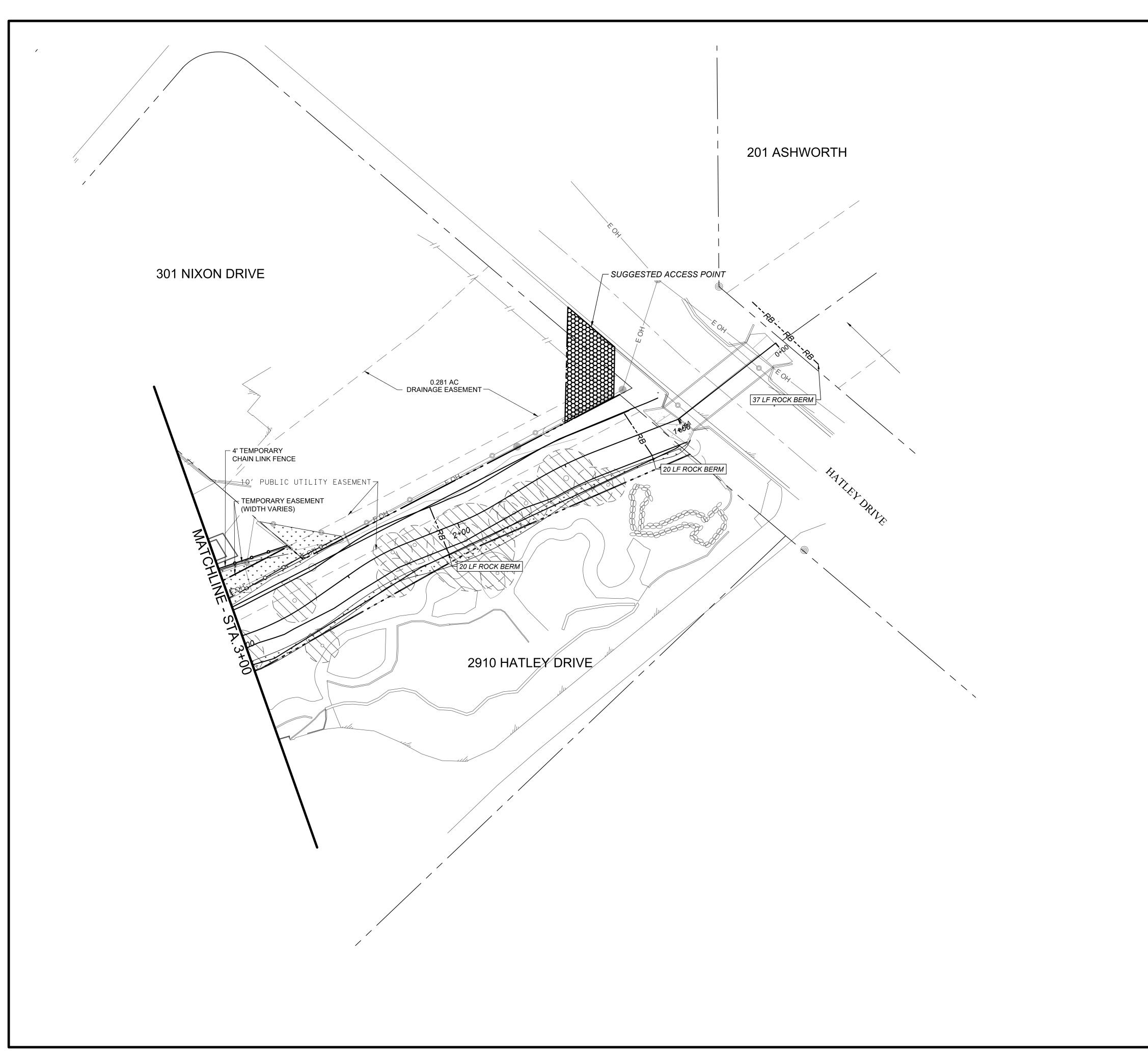
**RELLINGWOOD** 

**K·FRIESE** + ASSOCIATES PUBLIC PROJECT ENGINEERING 1120 S. Capital of Texas Highway

CityView 2, Suite 100 Austin, Texas 78746 P - 512.338.1704 F - 512.338.1784 TBPE Firm #6535 www.kfriese.com

***************************************		
NOTES	NAME	DATE
SURVEY BY		
DRAWN BY	АН	08/21
DESIGNED BY	LWM	08/21
CHECKED BY	GE	08/21
REVIEWED BY	PS	09/21





### NOTES:

- ROCK FILTER BERM SHALL BE EMBEDDED AND SECURED TO THE CHANNEL FLOOR TO PREVENT IT FROM WASHING AWAY DURING STORM EVENTS.
- 2. SIDE SLOPES OF ROCK FILTER BERM SHALL NOT EXCEED 4:1.

	REVISION DESCRIPTIO						
	DATE						
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	REV. No.						

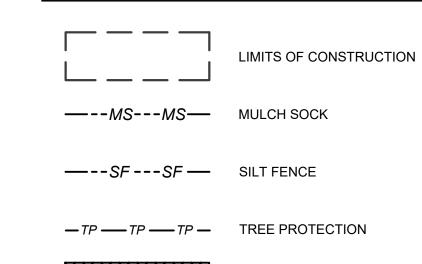
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PROPOSED DRAINAGE IMPROVEMENTS JIXON/PLEASANT DRAINAGE IMPROVEMENTS	EROSION CONTROL PLAN
PROPOSED DRAIN	EROSION C

## LEGEND



	STABILIZED CONSTRUCTION ENTRANCE
—RBRB—	ROCK BERM
•	TREE

•	TREE	
// A		

TREE TO BE REMOVE

R@LLINGWOOD TEXAS
K.FRIESE

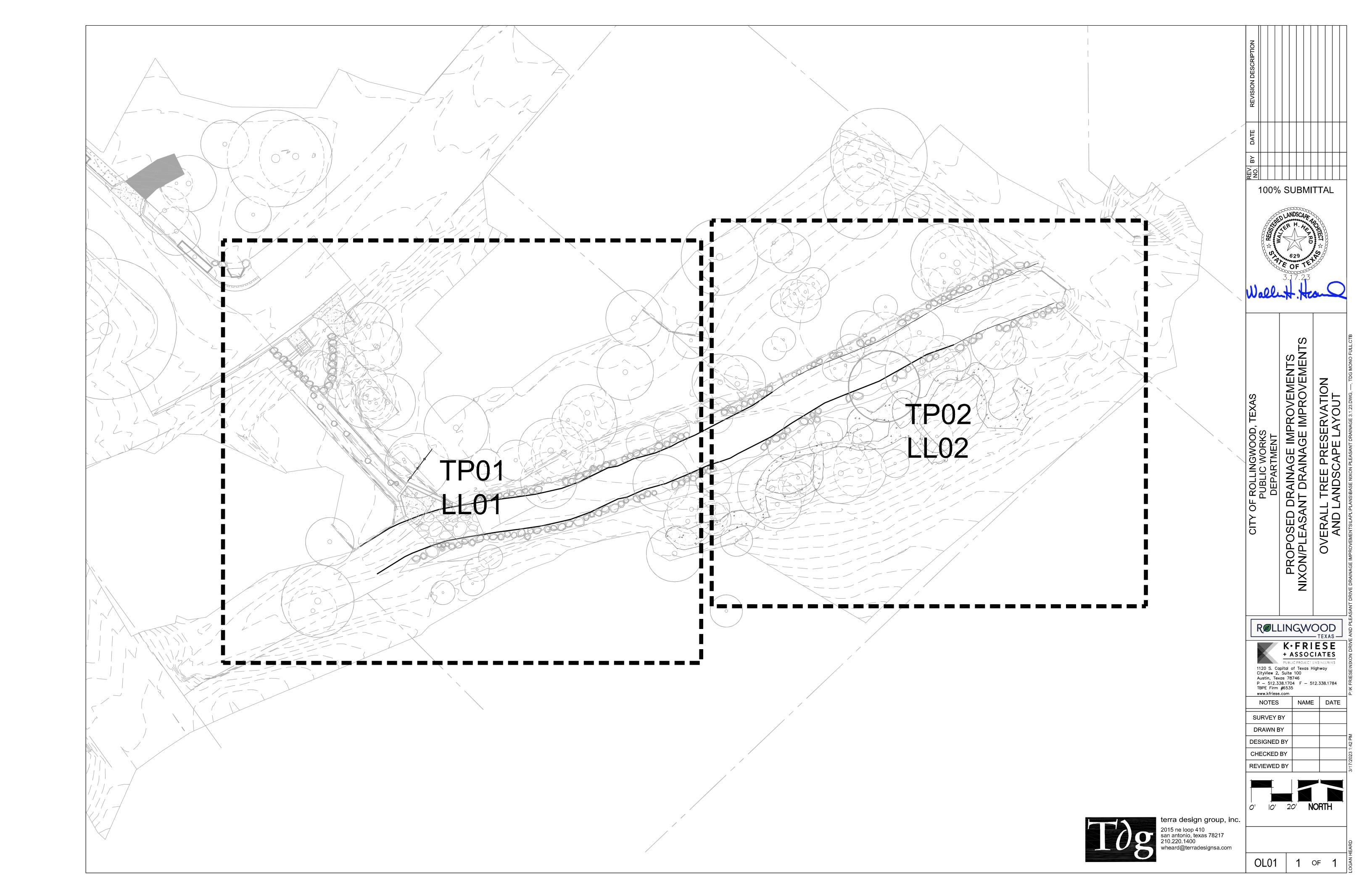
+ ASSOCIATES

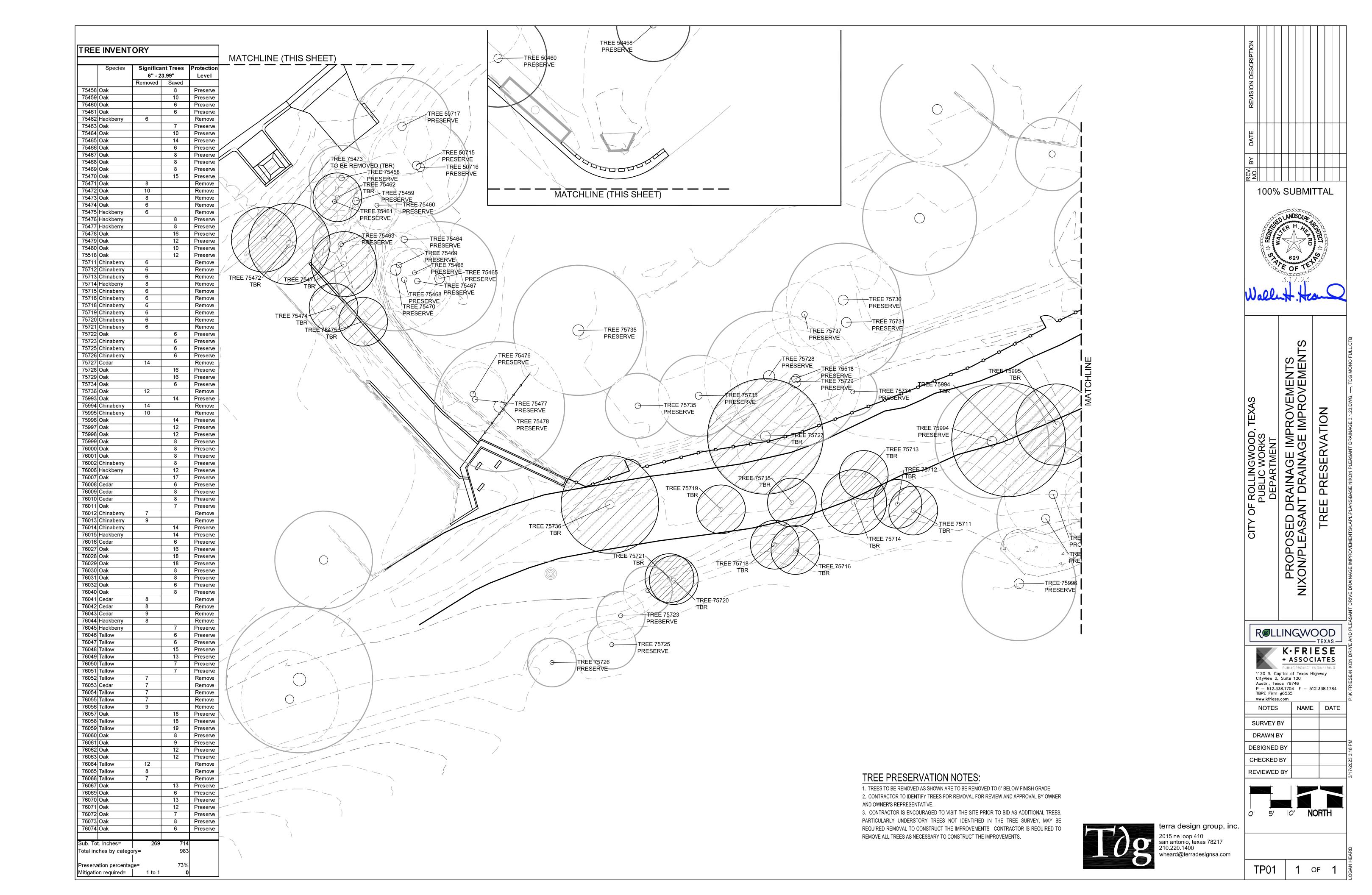
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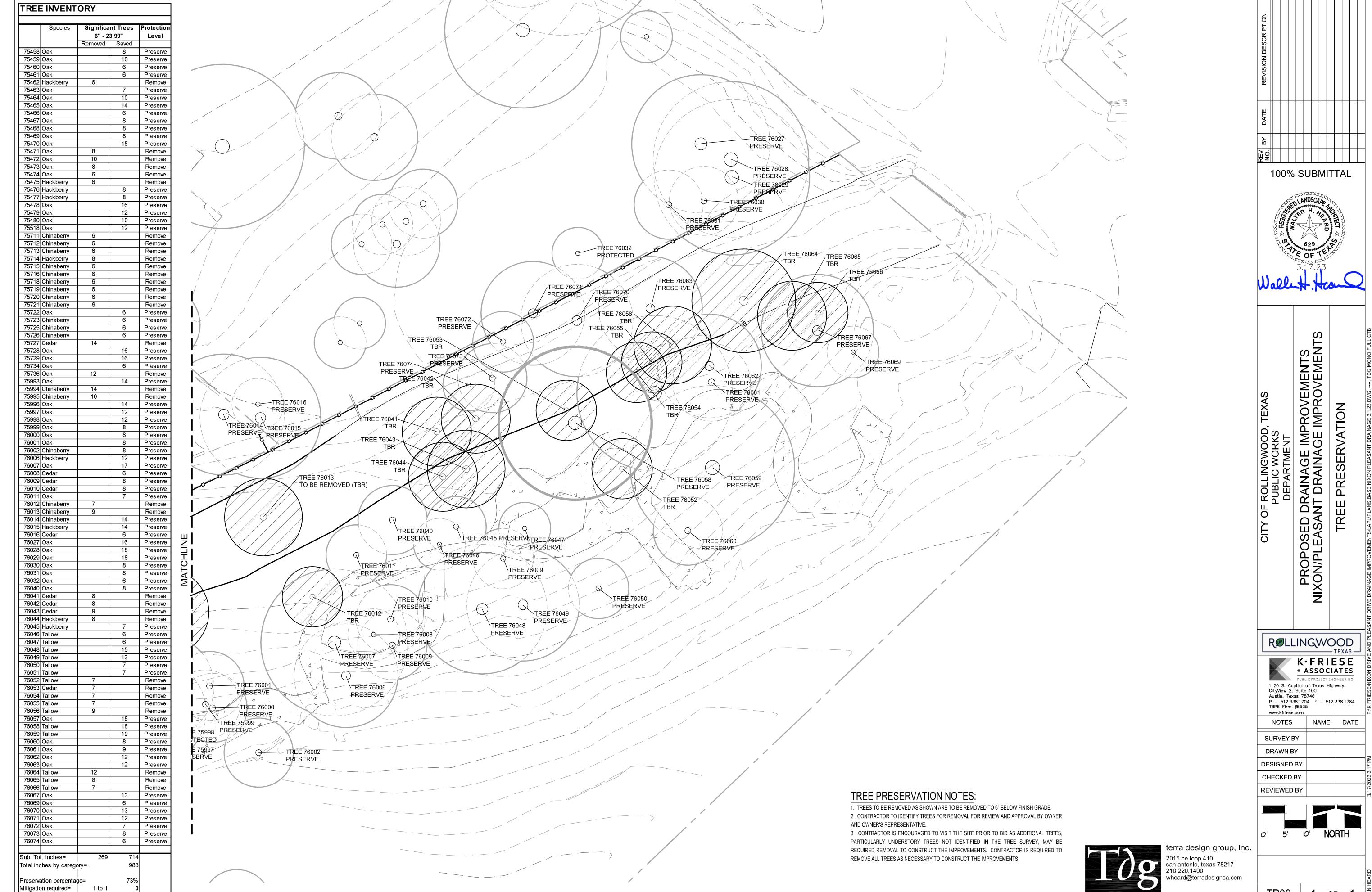
NOTES	NAME	DATE
SURVEY BY		
DRAWN BY	АН	08/21
DESIGNED BY	LWM	08/21
CHECKED BY	GE	08/21
REVIEWED BY	PS	09/21

C	) 20 40
	HORIZONTAL SCALE IN FEET

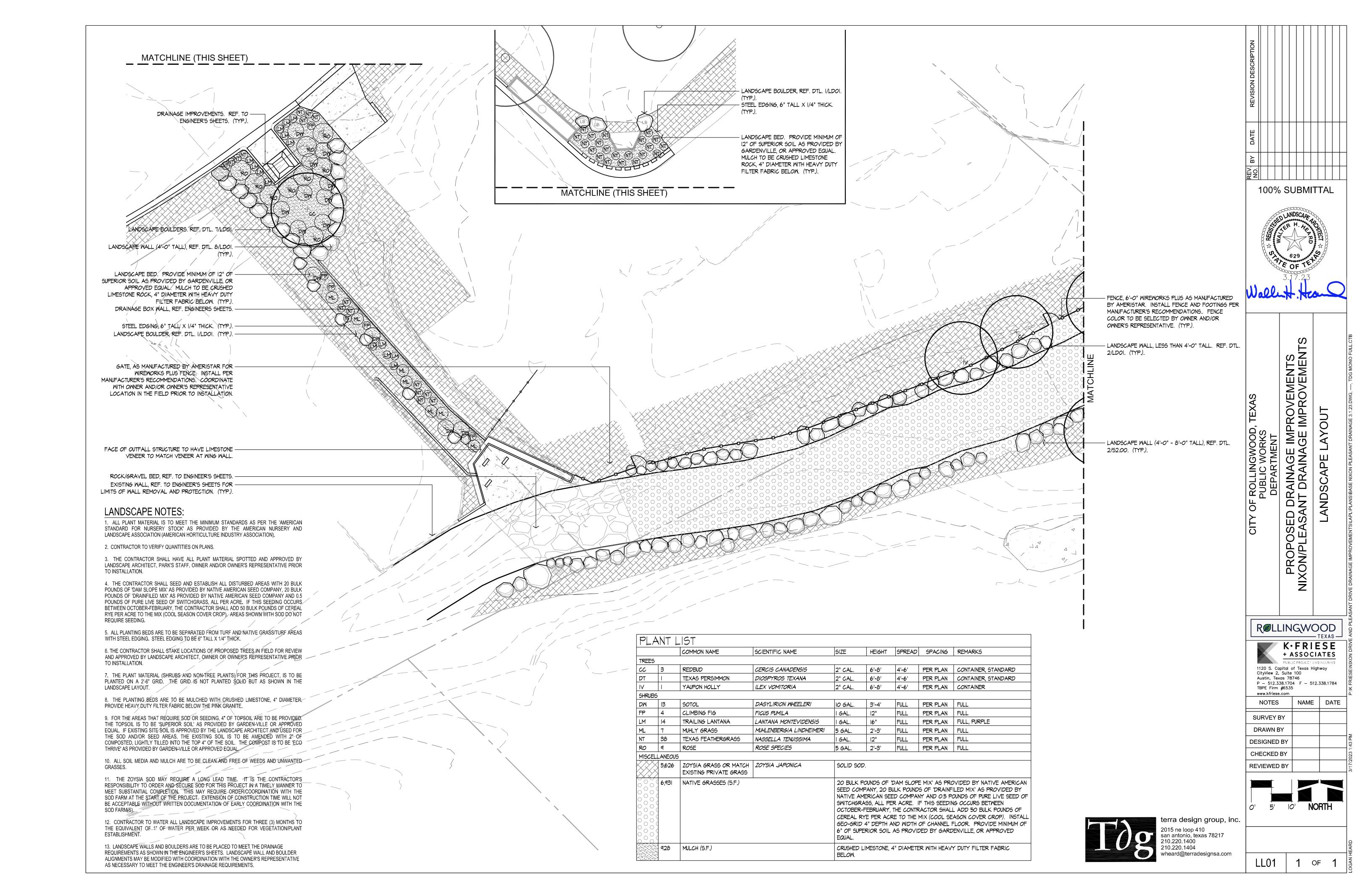
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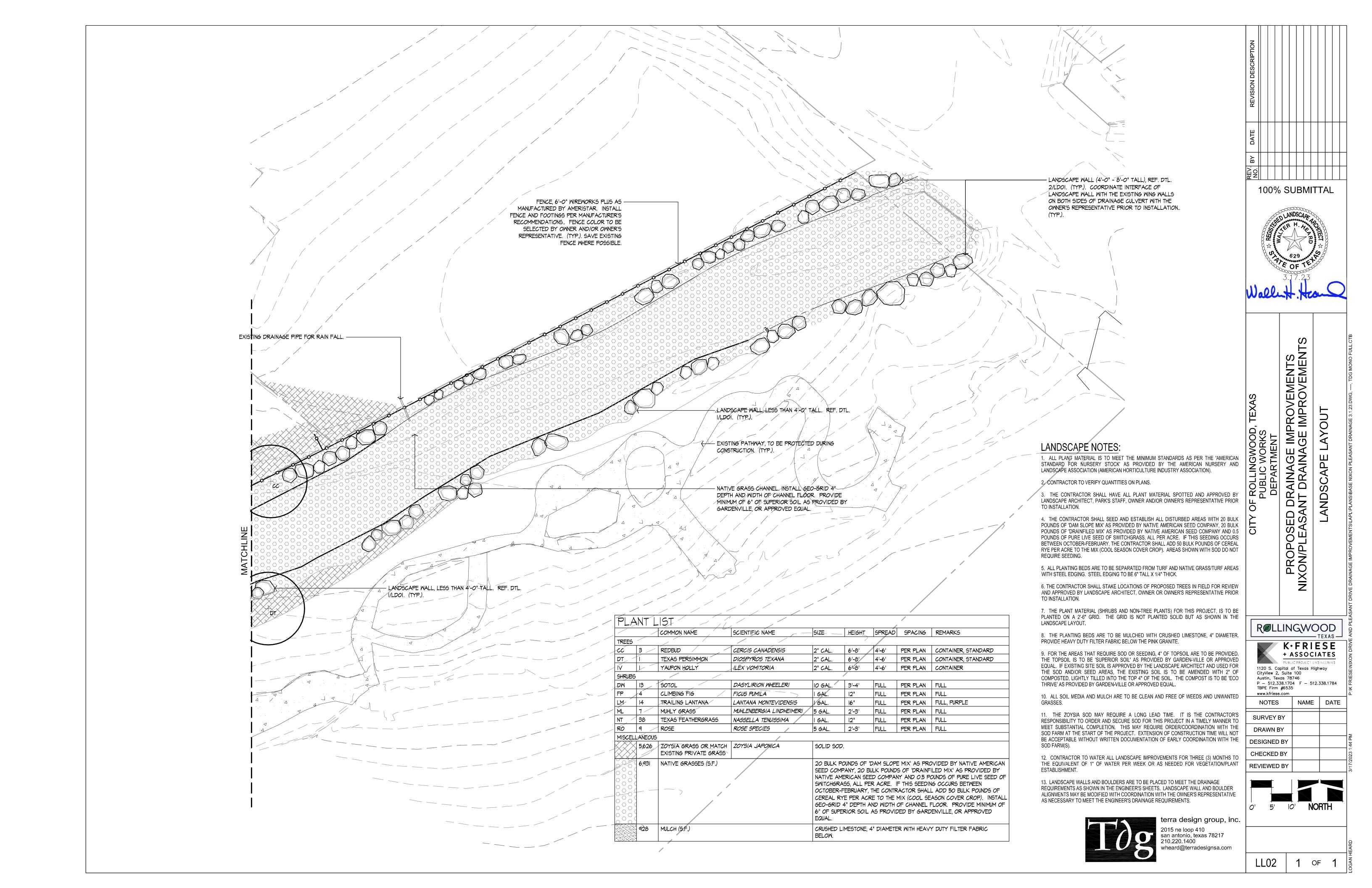


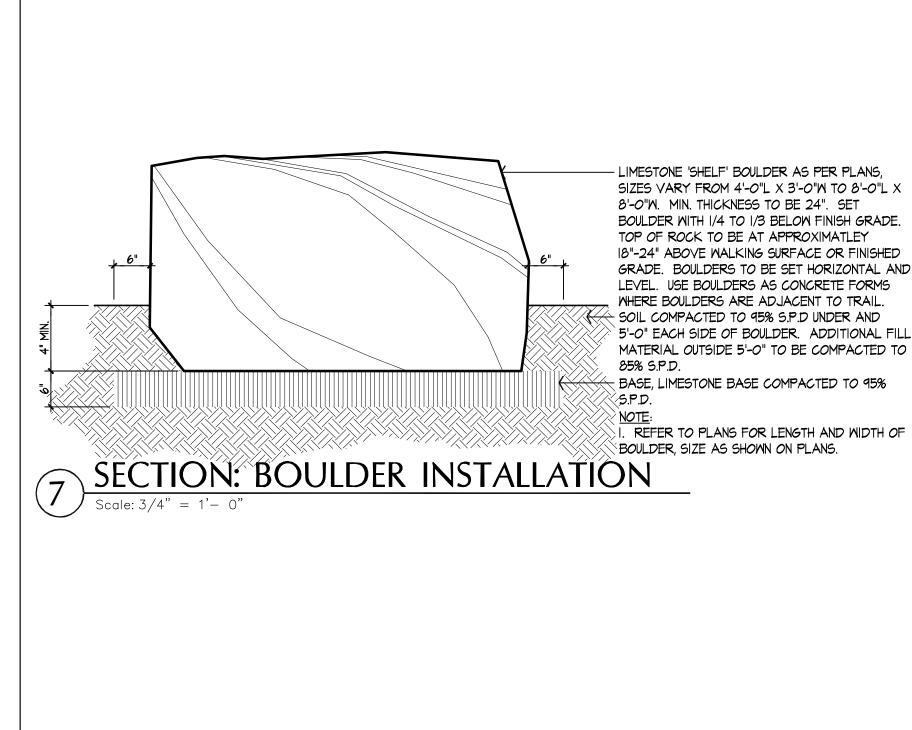




1 OF 1







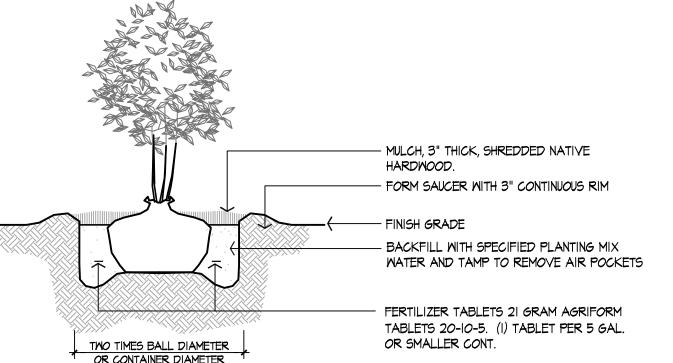
MULCH, AS PER THE PLANS.

MEED BARRIER. LANDMASTER
POLYPRO 3IO BY 'KELLER MATERIALS'
OR APPROVED EQUAL.

STEEL EDGING, 6" TALL X I/4" THICK AS
MANUFACTURED BY COLMET OR
APPROVED EQUAL. INSTALL PER
MANUFACTURER'S RECOMMENDATIONS,
UTILIZE ALL STAKE INSERTS. COR-TEN
EDGING OF THE SAME SPECIFICATIONS
IS AN ACCEPTABLE ALTERNATIVE
MATERIAL.

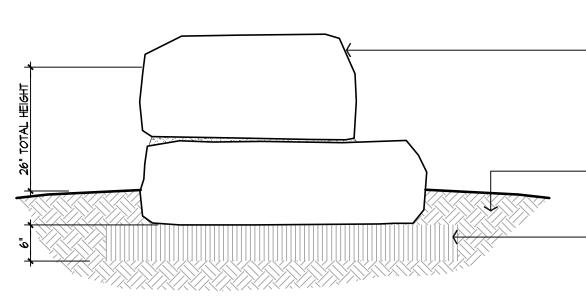
UNDISTURBED SOIL OR FILL COMPACTED
TO 45% S.P.D.

SECTION: STEEL EDGING



SECTION: SHRUB PLANTING

Scale: Not To Scale



LIMESTONE BOULDER WALL AS PER PLANS, SIZES VARY FROM 4'-0"L X 3'-0"W TO 6'-0"L X 6'-0"W. MIN. THICKNESS TO BE 24". SET BOULDER WITH 1/4 TO 1/3 BELOW FINISH GRADE. TOP OF ROCK TO BE AT APPROXIMATELY 26" TOTAL HEIGHT ABOVE SURFACE OR FINISHED GRADE. BOULDERS TO BE SET HORIZONTAL AND LEVEL. USE HIGH STRENGTH GROUT TO JOIN BOULDERS.

SOIL COMPACTED TO 95% S.P.D UNDER AND 5'-O" EACH SIDE OF BOULDER. ADDITIONAL FILL MATERIAL OUTSIDE 5'-O" TO BE COMPACTED TO 85% S.P.D.

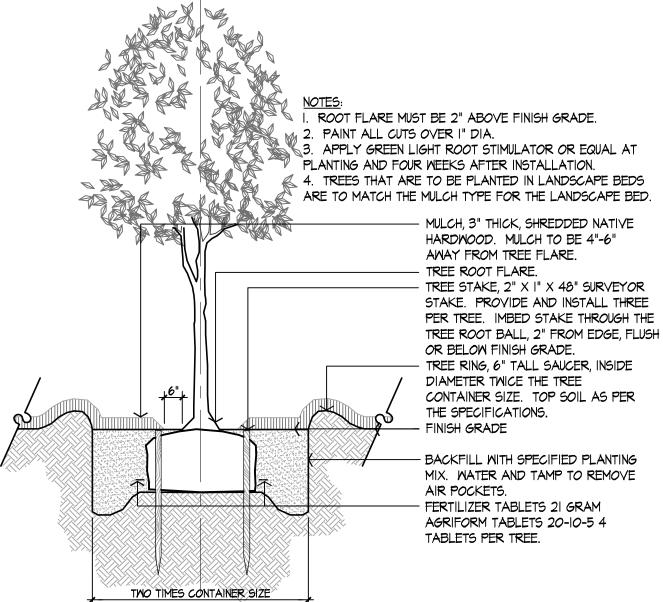
BASE, LIMESTONE BASE COMPACTED TO 95% S.P.D.

NOTE:

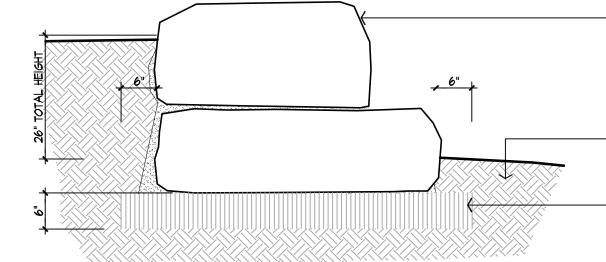
1. REFER TO PLANS FOR LENGTH AND WIDTH OF BOULDER,
SIZE AS SHOWN ON PLANS.
2. BOULDER WALL TO BE MAXIMUM TWO COURSES.
3. TOP OF BOULDER WALL TO BE LEVEL AND STAIR STEP AS
NECESSARY TO KEEP BOULDER WALL AT +/- 6" OF FINISH
TRAIL ELEVATION.
4. FILL ALL VOIDS BETWEEN BOULDERS WITH HIGH STRENGTH

SECTION: LANDSCAPE BOULDER

Scale: 3/4" = 1'- 0"



SECTION: TREE PLANTING



LIMESTONE 'SHELF' BOULDER AS PER PLANS, SIZES VARY FROM 4'-0"L X 3'-0"W TO 6'-0"L X 6'-0"W. MIN. THICKNESS TO BE 24". SET BOULDER WITH I/4 TO I/3 BELOW FINISH GRADE. TOP OF ROCK TO BE AT APPROXIMATELY 26" TOTAL HEIGHT ABOVE SURFACE OR FINISHED GRADE. BOULDERS TO BE SET HORIZONTAL AND LEVEL.

SOIL COMPACTED TO 95% S.P.D UNDER AND 5'-0" EACH SIDE OF BOULDER. ADDITIONAL FILL MATERIAL OUTSIDE 5'-0" TO BE COMPACTED TO 85% S.P.D.

BASE, LIMESTONE BASE COMPACTED TO 95% S.P.D.

NOTE:

I. REFER TO PLANS FOR LENGTH AND WIDTH OF BOULDER,

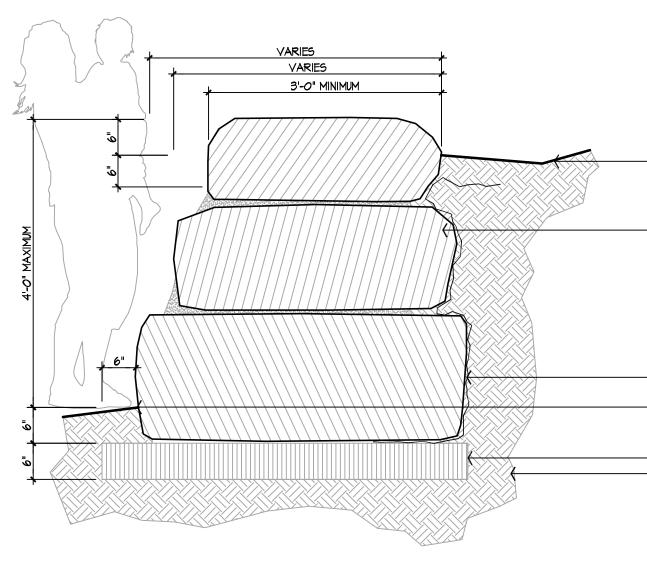
SIZE AS SHOWN ON PLANS.

2. BOULDER WALL TO BE MAXIMUM TWO COURSES.

3. TOP OF BOULDER WALL TO BE LEVEL AND STAIR STEP AS NECESSARY TO KEEP BOULDER WALL AT +/- 6" OF FINISH TRAIL ELEVATION.

4. FILL ALL VOIDS BETWEEN BOULDERS WITH HIGH STRENGTH GROUT TO PREVENT SOIL FROM PASSING THROUGH.

SECTION: LANDSCAPE BOULDER



NOTES:

I. MULTIPLE COURSES OF BOULDERS MAY BE REQUIRED.

REF. TO GRADING PLANS FOR REQUIRED WALL HEIGHTS.

2. BOULDERS TO BE SET LEVEL AND HORIZONTAL.

FINISH GRADE. SLOPE AWAY FROM WALL FOR POSITIVE DRAINAGE AWAY FROM AND AROUND WALL. COORDINATE GRADING WITH OWNER'S REPRESENTATIVE IN FIELD. ESTABLISH VEGETATION.

LIMESTONE BOULDERS. BOULDERS TO BE LUEDERS
LIMESTONE SHELF BOULDER, SIZED APPROXIMATELY 24"
TALL X 3'-O" WIDE X 6'-O" LONG. MINIMUM SIZE TO BE 18"
TALL X 3'-O" WIDE X 4'-6" LONG. CAREFULLY STACK AND
PLACE BOULDERS IN A TIGHT STAGGERED BOND. BANTER
BOULDER COURSES BACK 3"-6" EACH COURSE. MAXIMUM
NUMBER OF COURSES TO BE THREE (3), INCREASE BOULDER
THICKNESS AS REQUIRED. CHINK BETWEEN BOULDERS AS
NECESSARY AND GROUT ALL VOID SPACES.
FILTER FABRIC, HEAVY DUTY.

FINISH GRADE, REF. TO GRADING PLANS. BOTTOM BOULDER COURSE TO BE MINIMUM 6" BELOW FINISH GRADE.

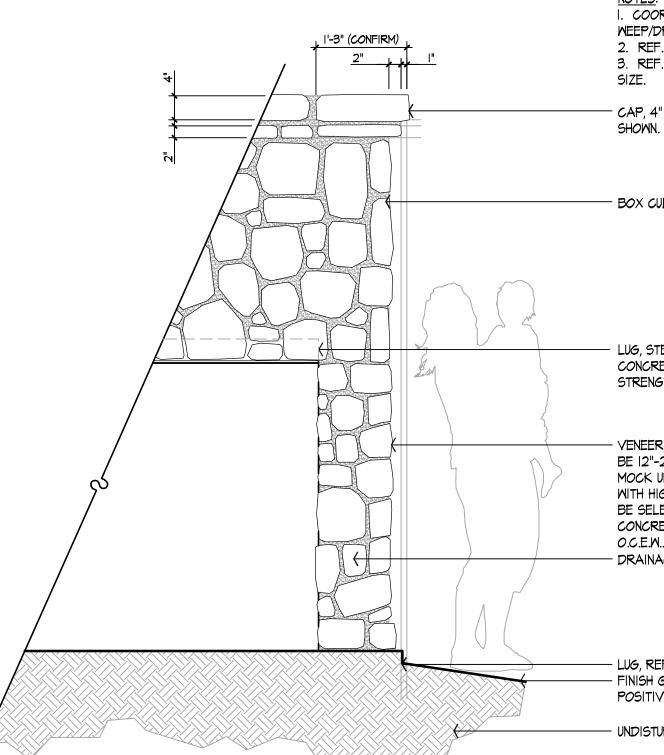
BASE, LIMESTONE BASE COMPACTED TO 95% S.P.D.
UNDISTURBED SOIL OR FILL COMPACTED TO 95% S.P.D.

NOTES:

I. REFER TO PLANS FOR LENGTH AND WIDTH OF BOULDER,
SIZE AS SHOWN ON PLANS.

SECTION: LANDSCAPE WALL (LESS THAN 4'-0")

Scale: 3/4" = 1'- 0"



NOTES:

I. COORDINATE, IN THE FIELD, WITH OWNER'S REPRESENTATIVE IF WEEP/DRAINAGE HOLES ARE NECESSARY FOR DRAINAGE.

2. REF. TO ENGINEER'S SHEETS FOR WALL HEIGHT.

3. REF. TO ENGINEER'S SHEETS FOR DRAINAGE BOX TYPE AND SIZE.

CAP, 4" AND 2" THICK LIMESTONE CAPS WITH OVERHANGS AS SHOWN.

BOX CULVERT WALL (BEHIND), REF. TO ENGINEER'S SHEETS.

LUG, STEEL ANGLE,  $4" \times 4" \times 3/8"$  STEEL ATTACHED TO FACE OF CONCRETE DRAIN BOX WITH  $1/2" \times 4"$  ANCHOR BOLTS AND HIGH STRENGTH EPOXY. STONE TO CONCEAL LUG.

VENEER, RANDOM PATTERN LIMESTONE, 3" THICK, STONE VENEER TO BE 12"-24" IN SIZE. SET IN 3/4" THICK MORTAR BED. PROVIDE MOCK UP FOR APPROVAL PRIOR TO INSTALLATION. SET STONE WITH HIGH STRENGTH MORTAR. LIMESTONE AND GROUT COLORS TO BE SELECTED BY OWNER'S REPRESENTATIVE. TIE STONE TO CONCRETE WALLS WITH GALVANIZED MASONRY ANCHORS AT 12" O.C.E.M..

DRAINAGE BOX (BEHIND), REF. TO ENGINEER'S SHEETS.

- LUG, REF. TO ENGINEER'S SHEETS. - FINISH GRADE. REF. TO PLANS FOR SITE GRADING. PROVIDE POSITIVE DRAINAGE, BOTH SIDES.

UNDISTURBED SOIL OR FILL COMPACTED TO 95% S.P.D.

3 ELEVATION: DRAINGE BOX WALL

Scale: 3/4" = 1'-0"



NO. BY DATE REVISION DESCRIPTION OF SCRIPTION OF SCRIPTIO



3.17.23 H. H. SLN

SANT DRAINAGE IMPROVEMENTS

PROPOSED DRAINAGE IN NIXON/PLEASANT DRAINAG

Røllingwood

K+FRIESE + ASSOCIATES

PUBLIC PROJECT ENGINEERING

1120 S. Capital of Texas Highway
CityView 2, Suite 100
Austin, Texas 78746
P - 512.338.1704 F - 512.338.1784
TBPE Firm #6535

www.kfriese.com		
NOTES	NAME	DATE
SURVEY BY		
DRAWN BY		
DESIGNED BY		
CHECKED BY		
REVIEWED BY		

terra design group, inc.

2015 ne loop 410
san antonio, texas 78217
210.220.1400
wheard@terradesignsa.com

LD01 1 OF 1

#### STRUCTURAL NOTES:

#### **GENERAL NOTES:**

SDS, SD1

SEISMIC DESIGN CATEGORY

#### DESIGN CODES/STANDARDS

- CONSTRUCTION SHALL CONFORM TO ALL APPLICABLE CODES, REGULATIONS AND GENERAL CONDITIONS OF THE CONTRACT DOCUMENTS. THE PROJECT HAS BEEN DESIGNED AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE FOLLOWING:
- A. INTERNATIONAL BUILDING CODE (IBC), 2018 EDITION AS ADOPTED BY THE CITY.
- B. BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-14) AND COMMENTARY (ACI 318R-14)
- DESIGN LOADS THE STRUCTURAL DESIGN WAS PREPARED USING THE FOLLOWING DESIGN CRITERIA:

LOAD COMBINATION LOAD AND RESISTANT FACTOR DESIGN REINF. CONCRETE

WIND LOADS: RISK CATEGORY BASIC WIND SPEED EXPOSURE CATEGORY ENCLOSURE CLASSIFICATION MEAN ROOF HEIGHT	II 115 MPH ULTIMATE C OPEN VARIES
GROUND SNOW LOAD	5 PSF
SEISMIC LOAD: IMPORTANCE FACTOR SITE CLASS Ss, S1	1.0 C 0.107g, 0.03g

0.086 sec, 0.034 sec

- METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR SHALL TAKE NECESSARY PRECAUTIONS TO MAINTAIN AND INSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.
- STRUCTURAL MEMBERS HAVE BEEN LOCATED AND DESIGNED TO ACCOMMODATE THE MECHANICAL EQUIPMENT AND OPENINGS SPECIFIED BY THE MECHANICAL CONSULTANT. ANY SUBSTITUTION RESULTING IN REVISIONS TO THE STRUCTURE SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE WITH UNINTECH CONSULTING ENGINEERS, INC.
- THE GENERAL CONTRACTOR AND SUB-CONTRACTORS SHALL DETERMINE THE SCOPE OF THE STRUCTURAL WORK FROM THE CONTRACT DOCUMENTS TAKEN AS A WHOLE. THE STRUCTURAL DRAWINGS SHALL NOT BE CONSIDERED SEPARATELY FOR PURPOSES OF BIDDING THE STRUCTURAL WORK. DUE CONSIDERATION SHALL BE GIVEN TO OTHER STRUCTURAL WORK OR WORK RELATED TO THE STRUCTURE, INCLUDING NECESSARY COORDINATION DESCRIBED OR IMPLIED BY THE ARCHITECTURAL AND MECHANICAL DRAWINGS.
- THE REPRODUCTIVE USE OF THE STRUCTURAL CONTRACT DOCUMENTS OR ELECTRONIC FILES AS STRUCTURAL SHOP DRAWING DOCUMENTS BY THE CONTRACTOR OR SUB-CONTRACTOR IS AT THEIR OWN RISK. UNINTECH CONSULTING ENGINEERS, INC. ASSUMES NO LIABILITY AS THE RESULT OF THE REPRODUCTIVE USE OF THE STRUCTURAL CONTRACT DOCUMENTS FOR SHOP DRAWINGS.
- SCALES NOTED ON THE DRAWINGS ARE FOR GENERAL REFERENCE ONLY. NO DIMENSIONAL INFORMATION SHALL BE OBTAINED BY DIRECT SCALING OF THE DRAWINGS.
- THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF ALL RESULTING REVISIONS TO THE STRUCTURAL SYSTEM OR OTHER TRADES AS A RESULT OF ACCEPTANCE OF CONTRACTOR PROPOSED ALTERNATIVE OR SUBSTITUTIONS.
- PRINCIPAL OPENINGS IN THE STRUCTURE ARE INDICATED ON THE CONTRACT DOCUMENTS, REFER TO ARCHITECTURAL, MECHANICAL ELECTRICAL AND PLUMBING DRAWINGS FOR SLEEVES, CURBS, INSERTS ETC. NOT HEREIN INDICATED. OPENINGS IN SLAB WITH A MAXIMUM SIDE DIMENSION OR DIAMETER OF 12 INCH OR LESS SHALL NOT REQUIRE ADDITIONAL FRAMING OR REINFORCEMENT, UNLESS NOTED OTHERWISE THE LOCATION OF SLEEVES OR OPENINGS IN STRUCTURAL MEMBERS SHALL BE SUBMITTED TO UNINTECH CONSULTING ENGINEERS, INC. FOR REVIEW.
- CONTRACTOR SHALL VERIFY ALL FIELD CONDITIONS WHICH WILL AFFECT THE FABRICATION OF COMPONENTS FOR NEW CONSTRUCTION PRIOR TO THE START OF CONSTRUCTION, THESE DOCUMENTS DO NOT INDICATE THE METHOD OF CONSTRUCTION. ANY DISCREPANCIES NOTED SHALL BE CONVEYED TO ARCHITECT AND ENGINEER PRIOR TO STARTING WORK. IGNORANCE OF CONDITIONS IS NOT A BASIS FOR A CLAIM OF ADDITIONAL COMPENSATION.
- 11. CONTRACTOR IS REQUIRED TO CORRECT AT HIS OWN EXPENSE ANY CONSTRUCTION DAMAGE, STRUCTURE DAMAGE OR OTHER OBJECTIONABLE CONDITIONS CAUSED BY HIS OPERATIONS.
- CONTRACTOR SHALL SUBMIT ALL SHOP DRAWINGS (FABRICATION, MANUFACTURING, ERECTION AND SETTING DRAWINGS), SAMPLES, TEST REPORTS AND PRODUCT DATA RELATED TO STRUCTURAL DOCUMENTS FOR ENGINEERS REVIEW. NO WORK SHALL START UNTIL APPROVAL FROM ENGINEER
- 13. A QUALIFIED INDEPENDENT TESTING LABORATORY SHALL PERFORM RETESTING WHEN WORK DOES NOT COMPLY WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS.
- 14. STRUCTURAL NOTES APPLY UNLESS OTHERWISE NOTED ON STRUCTURAL DRAWINGS. IN CASE OF CONFLICT, GREATER REQUIREMENTS GOVERN.
- 15. ALL UTILITIES CONNECTED TO BUILDING SHALL ALLOW FOR MOVEMENT DUE TO EXPANSIVE SUBGRADE. MOVEMENT CAN BE ACHIEVED BY USING SLEEVE JOINTS, BENDS, LOOPS AND OTHER SIMILAR DEVICES.
- USE THE MANUFACTURER'S CERTIFIED DRAWINGS AND SPECIFICATIONS FOR THE EQUIPMENT ANCHORAGE AND DETAILS.

#### **SUBGRADE AND BACKFILL:**

THE SUBSURFACE INFORMATION AND THE FOUNDATION DESIGN AND CONSTRUCTION GUIDELINES ARE BASED ON A REPORT PREPARED BY RABA KISTNER, DATED SEPTEMBER 26, 2021 (REPORT NO. AAA20-086-00). THE CONTRACTOR SHALL PERFORM EXCAVATIONS AND PREPARATION OF THE SUBGRADE UNDER THE FOUNDATION IN ACCORDANCE WITH THE RECOMMENDATIONS CONTAINED IN THIS SECTION. CONTRACTOR SHALL BECOME FAMILIAR WITH THE GEOTECHNICAL REPORT.

THE AREA OF PROPOSED CONSTRUCTION MAY CONTAIN VARIOUS BURIED UTILITIES OR OLD FOUNDATIONS. AS SUCH, ALL UTILITIES AND FOUNDATIONS SHOULD BE LOCATED PRIOR TO THE COMMENCEMENT OF ANY EARTHWORK. ALL FOUNDATIONS SHOULD BE REMOVED AS PART OF THE DEMOLITION ACTIVITIES ASSOCIATED WITH THE PROJECT. ANY UTILITIES NOT PLANNED TO REMAIN IN-PLACE SHOULD BE REMOVED AND BACKFILLED DURING THE EARTHWORK OPERATIONS.

BEFORE CONSTRUCTION STARTS, PERFORM ROUGH GRADING AND CUT SWALES SO THAT GROUNDS WILL DRAIN AWAY FROM THE STRUCTURE SITE. MAINTAIN DRAINAGE DURING ALL PHASES OF CONSTRUCTION SO THAT STORM WATER WILL DRAIN AWAY FROM THE SITE. KEEP EXCAVATIONS PUMPED FREE OF STORM WATER AT ALL

#### CONCRETE:

CONCRETE IN THE FOLLOWING AREAS SHALL HAVE NATURAL SAND FINE AGGREGATE AND NORMAL WEIGHT COARSE AGGREGATE CONFORMING TO ASTM 33, TYPE I PORTLAND CEMENT CONFORMING TO ASTM C150, AND SHALL HAVE THE FOLLOWING CYLINDER COMPRESSIVE STRENGTH (fc') AT 28 DAYS:

CONT. FOOTING..

- FLY ASH MAY BE USED AS A POZZOLAN TO REPLACE A PORTION OF THE PORTLAND CEMENT IN A CONCRETE MIX SUBJECT TO THE APPROVAL OF THE GENERAL CONTRACTOR AND THE STRUCTURAL ENGINEER. FLY ASH WHEN USED SHALL CONFORM TO ASTM C618, TYPE C OR F. CONCRETE MIXES USING FLY ASH SHALL BE PROPORTIONED TO ACCOUNT FOR THE PROPERTIES OF THE SPECIFIC FLY ASH USED AND TO ACCOUNT FOR THE SPECIFIC PROPERTIES OF THE FLY ASH CONCRETE THUS RESULTING. THE RATIO OF THE AMOUNT OF FLY ASH TO THE TOTAL AMOUNT OF FLY ASH AND CONCRETE IN THE MIX SHALL NOT EXCEED 25 PERCENT.
- GROUT FOR BASE PLATES SHALL BE NON-SHRINKABLE, NON-METALLIC CONFORMING TO ASTM C827, AND SHALL HAVE A SPECIFIED COMPRESSIVE STRENGTH AT 28 DAYS OF 5,000 PSI. PREGROUTING OF BASE PLATES WILL NOT BE PERMITTED.
- DETAILING OF CONCRETE REINFORCEMENT BARS AND ACCESSORIES SHALL CONFORM TO THE RECOMMENDATIONS OF ACI 315 "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT: AND ACI SP-66 " DETAILING MANUAL". PLACING OF REINFORCING BARS SHALL CONFORM TO THE RECOMMENDATIONS OF ACI315R "MANUAL OF ENGINEERING AND PLACING DRAWINGS FOR REINFORCED CONCRETE STRUCTURES: AND CRSI "MANUAL OF STANDARD PRACTICE".
- MIXING, TRANSPORTING AND PLACING OF CONCRETE SHALL CONFORM TO
- MINIMUM CONCRETE COVER PROTECTION FOR REINFORCEMENT BARS SHALL BE AS FOLLOWS: (SEE ACI 318, SECTION 20.6 FOR CONDITIONS NOT NOTED).

CONCRETE EXPOSED TO WEATHER OR IN CONTACT WITH GROUND

#5 BARS AND SMALLER. ..1 1/2 INCHES #6 BARS AND LARGER.. ....2 INCHES CONCRETE CAST AGAINST EARTH & PERMANENTLY IN CONTACT WITH GROUND......3 INCHES

CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND SLAB & WALLS.. .3/4 INCHES BEAMS.

PROVIDE STANDARD BAR CHAIRS AND SPACERS AS REQUIRED TO MAINTAIN CONCRETE PROTECTION SPECIFIED.

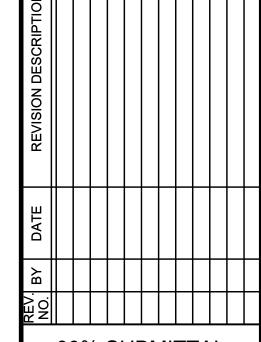
- CONCRETE REINFORCEMENT BARS SHALL CONFORM TO ASTM A615, GRADE 60.
- REINFORCEMENT BARS SHALL NOT BE TACK WELDED, HEATED OR CUT UNLESS INDICATED ON THE CONTRACT DOCUMENTS OR REVIEWED BY THE STRUCTURAL
- REINFORCEMENT DESIGNATED AS "CONTINUOUS" MAY BE SPLICED USING TYPE "B" SPLICES. REINFORCEMENT BAR SPLICE LENGTHS IN BEAMS WHICH ARE LOCATED AT THE CENTERLINE OF SUPPORT FOR BOTTOM BARS AND AT MIDSPAN FOR TOP BARS MAY BE 36 BAR DIAMETERS, UNLESS NOTED OTHERWISE. PROVIDE STANDARD ACI HOOKS FOR TOP AND BOTTOM BARS AT DISCONTINUOUS ENDS OF ALL GRADE
- REINFORCEMENT BARS INDICATED ON THE CONTRACT DOCUMENTS AS MECHANICAL SPLICES SHALL BE MADE USING A MECHANICAL CONNECTION CAPABLE OF TRANSFERRING IN TENSION OR COMPRESSION, AS REQUIRED, A MINIMUM OF 125 PERCENT OF THE SPECIFIED YIELD STRENGTH OF THE REINFORCEMENT BARS SPLICED. THE MECHANICAL CONNECTOR TO BE USED SHALL BE SUBMITTED TO UNINTECH CONSULTING ENGINEERS, INC FOR APPROVAL.
- 11. HORIZONTAL FOOTING AND HORIZONTAL WALL REINFORCEMENT SHALL BE CONTINUOUS AND SHALL HAVE 90-DEGREE BENDS AND EXTENSIONS, OR CORNER BARS OF EQUIVALENT SIZE LAPPED 36 BAR DIAMETERS, AT CORNERS AND

#### **CONCRETE MASONRY NOTES:**

- 1. MASONRY CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH ACI 530 "BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY CONSTRUCTION" AND ACI 530.1 "SPECIFICATIONS FOR MASONRY STRUCTURES". REFER TO SHEET S-5.2 FOR TYPICAL DETAIL.
- 2. CONCRETE MASONRY CONSTRUCTION SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (F'm) OF 1,500 PSI ON THE NET CROSS SECTIONAL AREA AT 28 DAYS.
- 3. MASONRY UNITS SHALL BE GRADE N, TYPE 1, MEDIUM WEIGHT OR NORMAL WEIGHT HOLLOW CONCRETE UNITS MEETING FIRE RATING REQUIREMENTS AND CONFORMING TO THE REQUIREMENTS OF ASTM C90. MASONRY UNITS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1,900 PSI ON THE NET AREA AT 28 DAYS, BUT NOT LESS THAT THE REQUIRED STRENGTH TO PRODUCE THE MINIMUM MASONRY STRENGTH (F'm). MASONRY UNITS SHALL NOT BE INSTALLED PRIOR TO ATTAINING THE REQUIRED 28 DAY STRENGTH.
- 4. MORTAR SHALL CONFORM TO THE REQUIREMENTS OF ASTM C270, TYPE M OR S. TYPE M MORTAR SHALL BE USED IN BELOW GRADE APPLICATIONS AND SHALL OBTAIN AN AVERAGE COMPRESSIVE STRENGTH OF 2,500 PSI AT 28 DAYS. TYPE S MORTAR MAY BE USED IN ABOVE GRADE APPLICATIONS AND SHALL OBTAIN AN AVERAGE COMPRESSIVE STRENGTH OF 1,800 PSI AT 28 DAYS. THE SAME MORTAR SHALL BE USED FOR WITHES OF CAVITY WALL CONSTRUCTIONS.
- 5. GROUT SHALL CONFORM TO ASTM C476 AND SHALL HAVE A COMPRESSIVE STRENGTH OF 2,000 PSI AT 28 DAYS.
- 6. REINFORCEMENT SHALL CONFORM TO THE STANDARDS SPECIFIED IN THE CONCRETE STRUCTURAL NOTES. REINFORCEMENT SHALL BE LAP SPLICED A MINIMUM OF 36 BAR DIAMETERS, UNLESS NOTED OTHERWISE.
- 7. PROVIDE 2-#5 CONTINUOUS IN BOND BEAMS UNLESS INDICATED OTHERWISE. PROVIDE CORNER BARS OF EQUIVALENT SIZE LAPPED AT CORNERS AND INTERSECTION OF WALLS. BOND BEAM REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONTROL JOINT.
- 8. MASONRY CORES CONTAINING VERTICAL REINFORCEMENT SHALL BE GROUTED SOLID.
- 9. FOR LIFTS GREATER THAT 4 FEET, HIGH LIFT GROUTING TECHNIQUES SHALL BE UTILIZED.
- 10. PROVIDE 1-#5 VERTICAL AT ENDS, CORNERS AND INTERSECTIONS OF
- 11. CELLS TO BE GROUTED SHALL BE CLEANED AND FREE OF EXCESS MORTAR AND FOREIGN MATERIALS.
- 12. IN CONCRETE SUPPORTING MASONRY WALLS, EMBED DOWELS TO MATCH SIZE AND LOCATION OF VERTICAL MASONRY REINFORCEMENT. EMBEDMENT DEPTH SHALL BE TO BOTTOM REINFORCEMENT OF CONCRETE. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL CELL TO BE REINFORCED, IT SHALL NOT BE BENT OVER, BUT SHALL BE GROUTED INTO THE CELL IN DIRECT VERTICAL ALIGNMENT, EVEN THOUGH IT IS ADJACENT TO VERTICAL WALL REINFORCING.
- 13. LINTELS SHALL BEAR ON MASONRY WALLS A MINIMUM OF 8" AT EACH END. FOR CMU WALLS, PROVIDE TWO GROUTED CORES EACH SIDE OF OPENING FULL HEIGHT OF WALL AND REINFORCE EACH OF THE TWO GROUTED CORES WITH 1-#5 VERTICAL EXTENDING FROM THE FOUNDATION TO THE POINT OF LATERAL SUPPORT FOR THE WALL ABOVE THE LINTEL LOCATION.
- 14. TEMPORARY SHORING OF LINTELS MUST BE PROVIDED UNTIL MASONRY HAS CURED. EDGES OF MASONRY OPENINGS SHALL NOT BE LOCATED CLOSER THAT 50% OF THE LINTEL SPAN FROM ENDS OR CORNERS OF WALLS OR MASONRY CONTROL JOINTS.
- 15. PROVIDE HORIZONTAL BOND BEAMS AT 4'-0" O.C., REINFORCE AS NOTED ABOVE, IN ALL MASONRY WALLS.
- 16. HORIZONTAL JOINT REINFORCEMENT SHALL BE USED IN THE MASONRY CONSTRUCTION. SUCH JOINT REINFORCEMENT SHALL BE PLACED AT 8" O.C. VERTICALLY IN WALLS BELOW GRADE AND AT 16 INCHES O.C. VERTICALLY IN WALLS THAT ARE ABOVE GRADE. HORIZONTAL JOINT REINFORCEMENT SHALL BE FABRICATED FROM GALVANIZED COLD-DRAWN STEEL WIRE CONFORMING TO ASTM A82. REINFORCEMENT SHALL CONSIST OF TWO OR MORE SMOOTH OR DEFORMED LONGITUDINAL WIRES NO. 9 GAGE OR LARGER, WELD CONNECTED WITH NO.12 GAGE OR LARGER CROSS WIRES. GALVANIZE COATING SHALL CONFORM TO ASTM A116 AND SHALL BE APPLIED AT A WEIGHT OF NOT LESS THAT 1.5 OUNCES PER SQUARE FOOT OF UNCOATED WIRE SURFACE. THE OUT TO OUT SPACING OF THE LONGITUDINAL WIRES SHOULD BE 1-5/8 INCHES LESS THAN THE WIDTH OF THE MASONRY UNIT. THE DISTANCE BETWEEN THE WELDED CONTACTS OF THE CROSS WIRES WITH EACH LONGITUDINAL WIRE SHOULD NOT EXCEED 6 INCHES AND 16 INCHES FOR SMOOTH AND DEFORMED LONGITUDINAL WIRES RESPECTIVELY. HORIZONTAL JOINT REINFORCEMENT SHALL BE DISCONTINUOUS THROUGH CONTROL JOINTS.
- 17. CONTROL JOINTS SHALL BE PLACED IN THE MASONRY CONSTRUCTION SUCH THAT THE PANEL LENGTH TO HEIGHT RATIO OF THE WALL DOES NOT EXCEED 1.5, AND THAT THE MAXIMUM PANEL LENGTH DOES NOT EXCEED 25 FEET. ADDITIONAL JOINTS SHALL BE PLACED WHERE ABRUPT CHANGES IN WALL SECTION OCCURS.
- 18. MASONRY CONSTRUCTION SHALL BE INSPECTED DURING VARIOUS WORK STAGES BY A QUALIFIED SPECIAL INSPECTOR. INSPECTION SHALL INCLUDE CHECKING FOR COMPLIANCE WITH CONTRACT DRAWINGS AND SPECIFICATIONS, INCLUDE, BUT NOT LIMITED TO RECORDING OF THE FOLLOWING:
- QUALITY AND TESTING OF MASONRY UNITS AND
- MATERIALS FOR PROPORTIONING, MIXING AND CONSISTENCY OF MORTAR
- PROPORTIONING, MIXING AND CONSISTENCY OF MORTAR
- AND GROUT. CONDITION, GRADE, SIZE, SPACING AND PLACEMENT OF REINFORCING STEEL.
- INSPECTION OF GROUT SPACING IMMEDIATELY PRIOR TO
- CLOSING OF CLEANOUT. SIGNIFICANT OR UNUSUAL CONSTRUCTION LOADS ON
- MASONRY ELEMENTS. WHEN AMBIENT TEMPERATURE FALLS BELOW 40 DEGREES F OR RISES ABOVE 100 DEGREES F, A COMPLETE RECORD OF WEATHER CONDITIONS AND OF RECONDITIONING AND PROTECTION GIVEN TO MASONRY

MATERIALS, AND PROTECTION AND CURING OF COMPLETE

WORK. SHALL BE MAINTAINED. GENERAL PROGRESS OF WORK.



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**RELLINGWOOD** 

**K·FRIESE** + ASSOCIATES PUBLIC PROJECT ENGINEERING 1120 S. Capital of Texas Highway CityView 2, Suite 100 Austin, Texas 78746 P - 512.338.1704 F - 512.338.1784 TBPE Firm #6535 www.kfriese.com

NOTES NAME DATE **SURVEY BY** DRAWN BY DESIGNED BY CHECKED BY REVIEWED BY

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TBPE REG. NO. F-5499 505 E. HUNTLAND DRIVE, STE. 335 2431 EAST EVANS ROAD AUSTIN, TEXAS 78752 SAN ANTONIO, TEXAS 78259 (512) 579-0722 FAX: (512) 579-0734 (210) 641-6003 FAX: (210) 641-8279

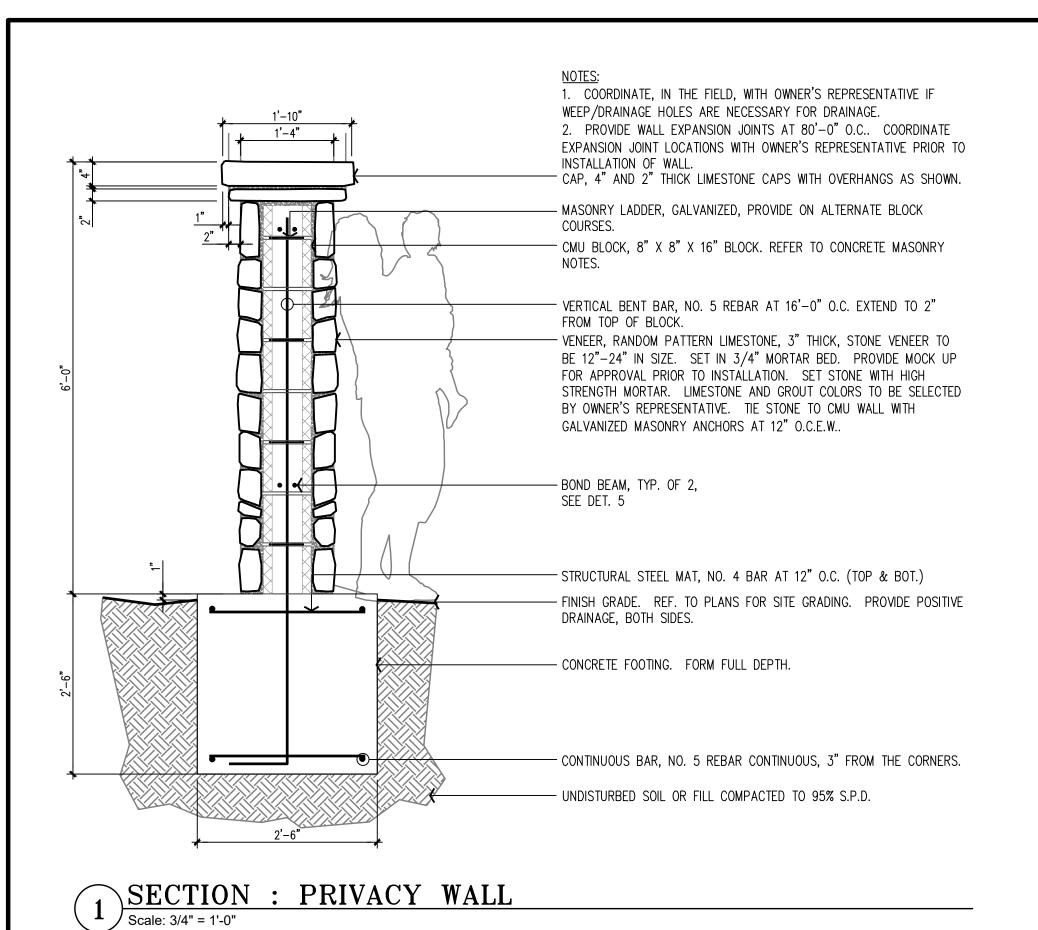
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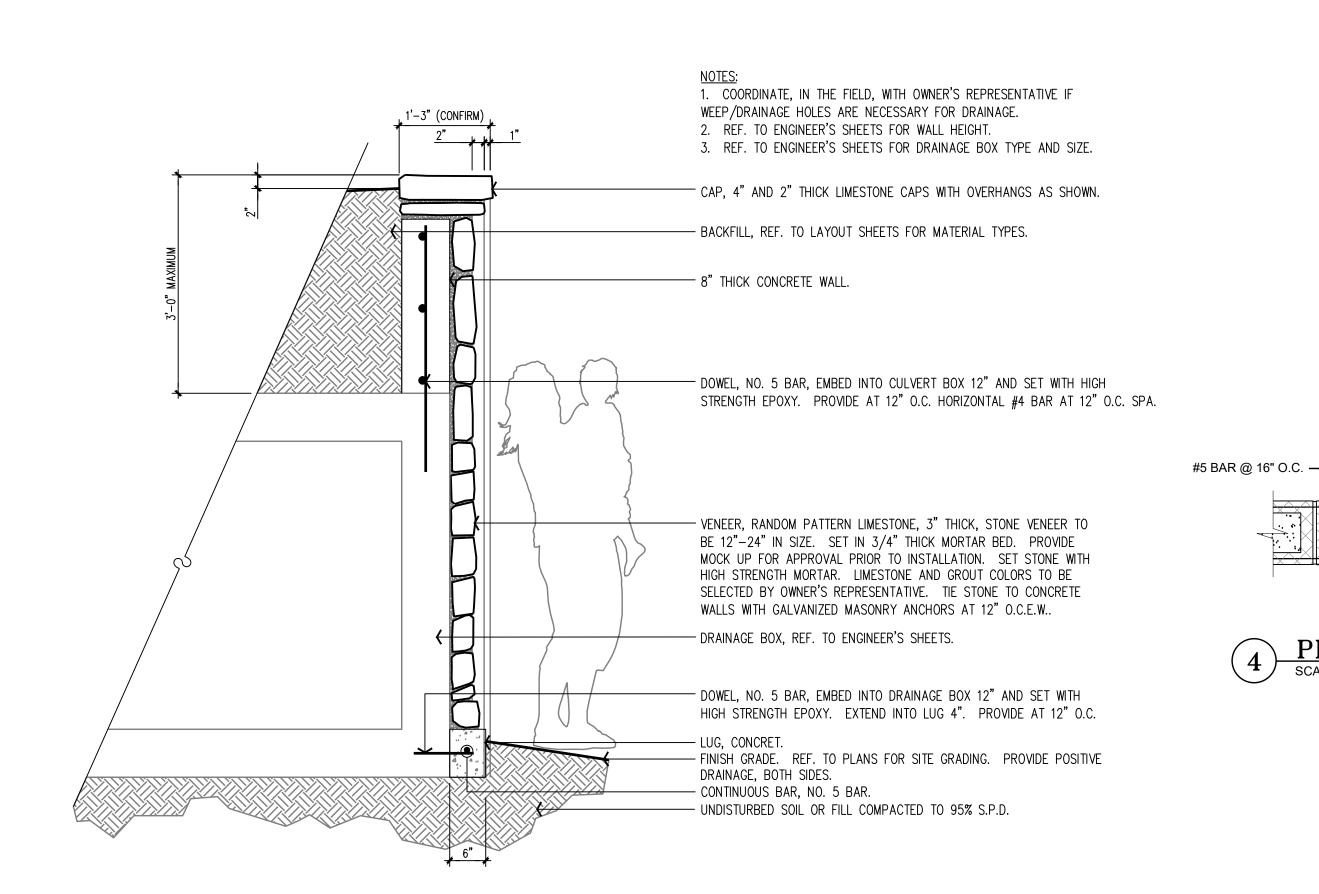
**UNINTECH CONSULTING** 

**ENGINEERS, INC.** 

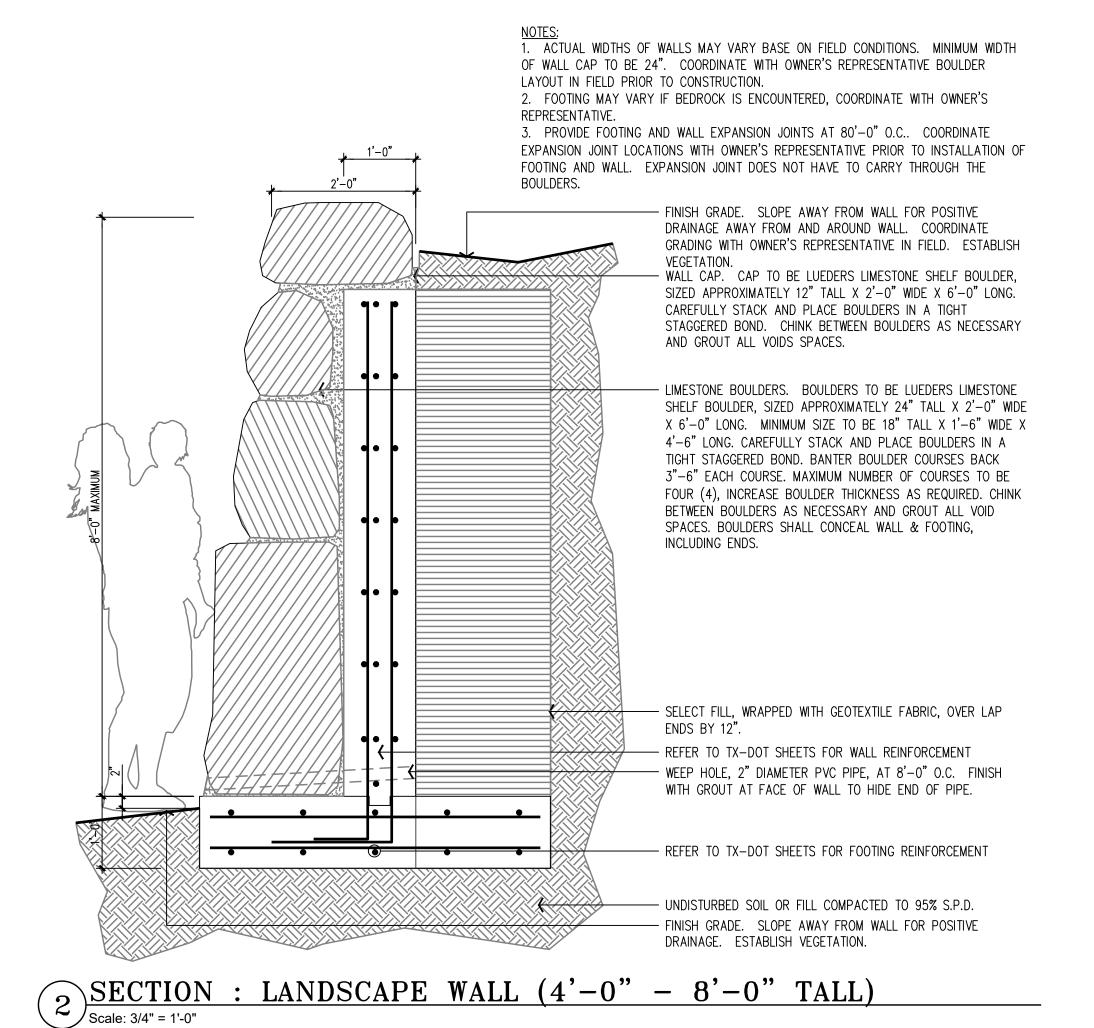
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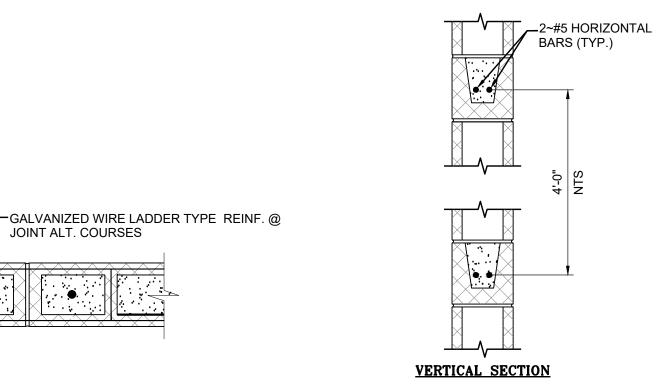
terra design group, inc. san antonio, texas 78217 wheard@terradesignsa.com mzak@terradesignsa.com





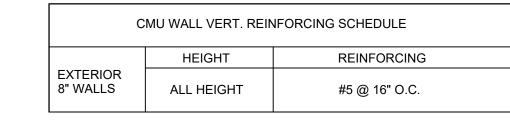
3 SECTION : BOX CULVERT WALL
Scale: 3/4" = 1'-0"





JOINT ALT. COURSES

PLAN-CMU JOINT REINF.
SCALE: NTS



NOTE: GROUT FILL ALL CELLS OF ALL CMU WALLS FULL HEIGHT

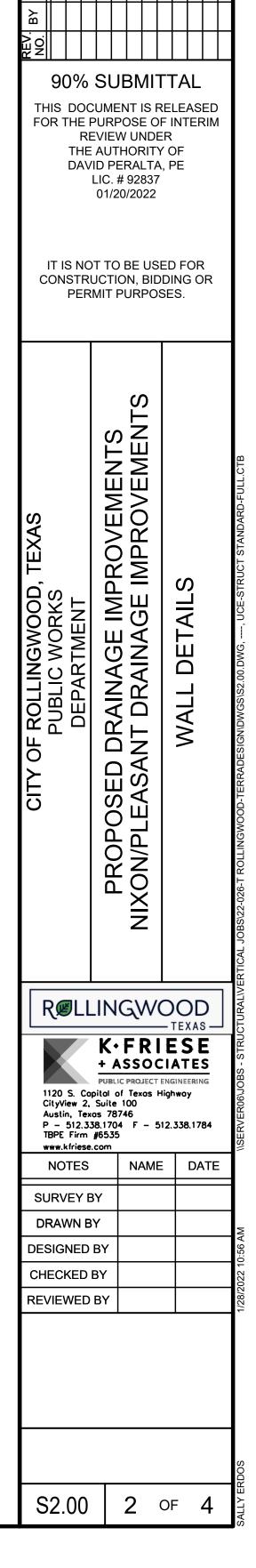




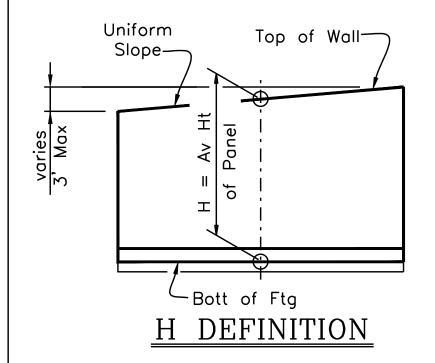


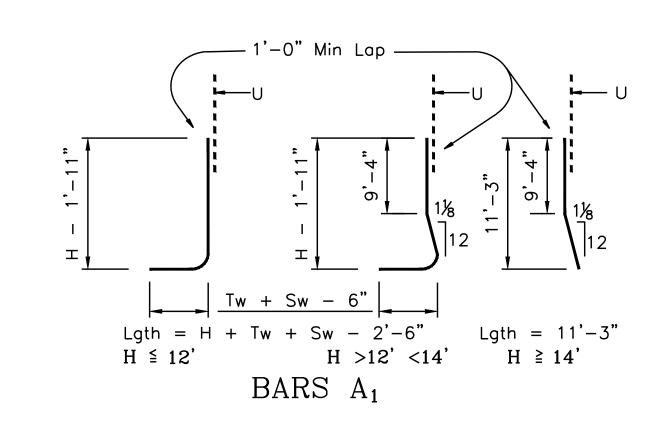
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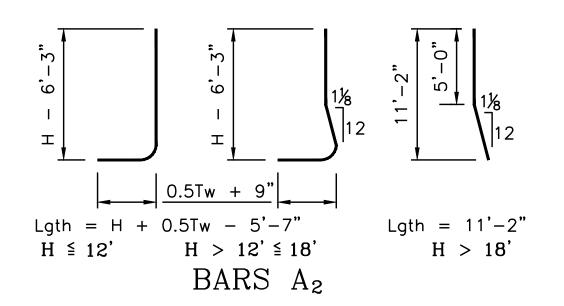
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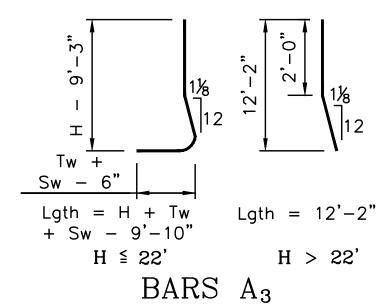


Wall			PRO	PERT	IES												REIN	FORC	ING	STI	CEL	FOR	ON	E 3	32'	PAN	EL									NTITY ONE	Wall
Heigh "H"			WALL DIMI	ENSIONS			Max Soil Press	A <sub>1</sub> ~ 2 at 15"	26 #5 'c-c	A <sub>2</sub> ~ 2 at 15"	5 #6 c-c	Az ~ 2! at 15"	5 #7 A	A <sub>4</sub> ~ 26 at 15"	6 #8 c-c	A <sub>5</sub> ~ at 15	25 #9 "c-c	A <sub>6</sub> ~ 25 at 15"	5 #11 c-c	A 7 ~ 2 at 15"	6 #11 c-c	B ~ 26 at 15" d	#5 :-c			С			it	owel F at 2"c-c	H (#5) at 12" c-c	T (#5) at 12" c-c	a	26 #5 it c-c		PANEL TREINF	Height "H"
(Ft)	Fw	Tw	Sw	Hw	Ft	Kw	T/SF	Lgth	Wt	Lgth	Wt	Lgth	Wt	Lgth	Wt	Lgth	Wt	Lgth	Wt	Lgth	Wt	Lgth	Wt :	Size N	lo Sp	a Lgth	Wt				No Wt			Wt	(CY)	(LB)	(Ft)
2	2'-0"	8"	1'-0"	4"	1'-0"	9"	0.26																			o" 1'-10		4	131 4	32	2 66	2 66	3'-4	"  90	4.2	418	2
3	2'-7"	11"	1'-0"	8"	1'-0"	9"	0.34																	#4 2	6 15	2'-3"	39	6	197	5 40	2 66	2 66	5 5'-4	." 145	6.1	553	3
4	3'-2"	1'-1"	1'-0"	1'-1"	1'-0"	9"	0.42	3'-7"	97													3'-6"	95	#4 2	6 15	<u>" 2'-8"</u>	46	8	263 (	3 48	3 99	2 66	7'-4	" 199	8.0	913	4
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9	5'-10	2'-0"	1'-0"	2'-10"	1'-0"	9"	0.82	9'-6"	' 258	4'-5"	166											8'-6"	231	#6 3	8 10	<b>4'-4"</b>	247	18	591 1	1 88	4 13	3 99	8'-4	" 226	17.1	2037	9

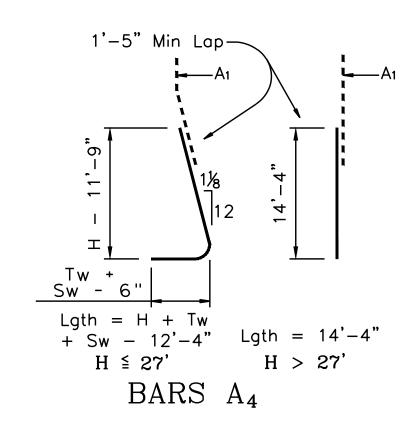


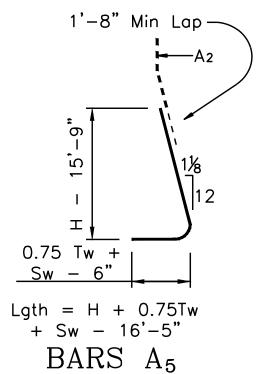


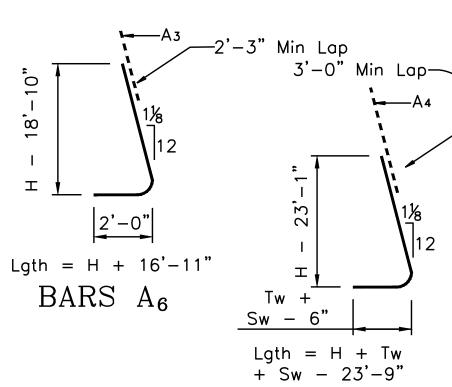


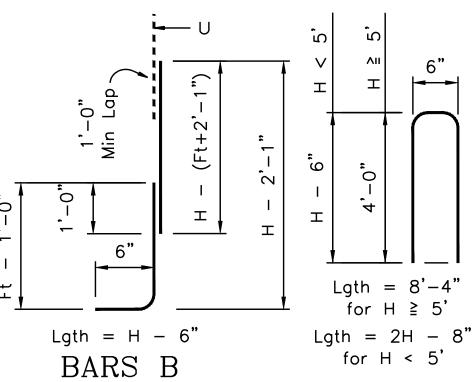


BARS A7





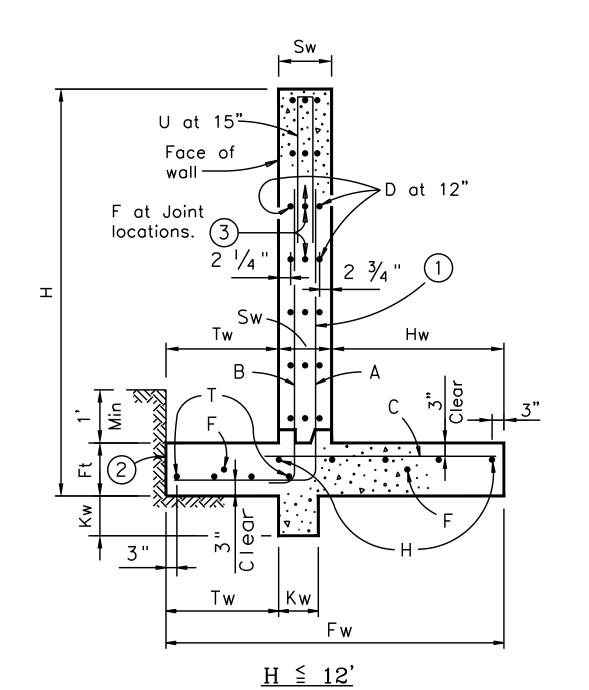




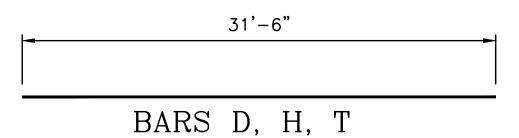
BARS U

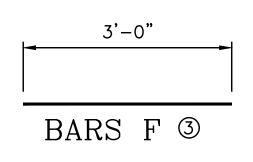
1) Place	vertica	bars	inside	of	horizonta
bars	(Typ bo	oth fac	ces).		

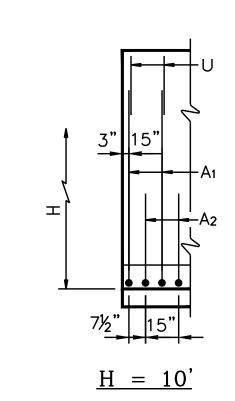
- 2 Place footing toe against undisturbed soil.
- ③ See S2.02 for size.



<u>SECTIONS</u>







PARTIAL WALL ELEVATIONS (Showing vertical reinforcing pattern in back face)

## GENERAL NOTES:

For notes and details not shown on this sheet see sheet S2.02. Quantities are based on "H" being overage height of panel. Retaining walls are designed to be coded as follows on Retaining wall Layout Sheets.



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PROPOSED DRAINAGE IMPROVEMENTS NIXON/PLEASANT DRAINAGE IMPROVEMENTS RETAINING WALLS

## R@LLINGWOOD TEXAS \_

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DESIGNED BY		
CHECKED BY		
REVIEWED BY		

terra design group, inc. 2015 ne loop 410 san antonio, texas 78217 210.220.1400 210.220.1404 wheard@terradesignsa.com mzak@terradesignsa.com

3 of 4 S2.01

