GSE Engineering & Consulting, Inc.

SUMMARY REPORT OF A LIMITED FORENSIC EVALUATION

MICANOPY TOWN HALL
MICANOPY, ALACHUA COUNTY, FLORIDA

GSE PROJECT NO. 17318

Prepared For:

TOWN OF MICANOPY

MAY 2026

GSE Engineering & Consulting, Inc.

May 14, 2026

Sara S. Samario, Town Administrator
Town of Micanopy
706 NE Cholokka Boulevard, P.O. Box 137
Micanopy, Florida 32667

Subject: Summary Report of a Limited Forensic Evaluation
Micanopy Town Hall
Micanopy, Alachua County, Florida
GSE Project No. 17318

GSE Engineering & Consulting, Inc. (GSE) is pleased to submit this report of a limited forensic evaluation of the Micanopy Town Hall in Micanopy, Alachua County, Florida. This report summarizes an evaluation by GSE and provides our opinion of the cause of damage in the building and provides recommendations for stabilizing the foundations of the building.

GSE appreciates this opportunity to have assisted you on this project. If you have any questions or comments concerning this report, please contact us.

Sincerely,
GSE Engineering & Consulting, Inc.

This item has been digitally signed and sealed by Kenneth L Hill
Digitally signed by Kenneth L Hill
Date: 2026.05.14 17:03:50 -04'00'
on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

KENNETH L. HILL LICENSE No.40146 STATE OF FLORIDA PROFESSIONAL ENGINEER

Kenneth L. Hill, P.E.
Principal Engineer – Geotechnical
Florida Registration No. 40146

This item has been digitally signed and sealed by John C Newcombe
Digitally signed by John C Newcombe
DN: C=John C Newcombe, dnQualifier=A01410C00000198FFEE3A2400583FC7, O=Unaffiliated, C=US
Date: 2026.05.14 15:46:13-04'00'
on the date adjacent to the seal. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

JOHN C. NEWCOMBE LICENSE No.93594 STATE OF FLORIDA PROFESSIONAL ENGINEER

John C. Newcombe, P.E.
Senior Engineer – Structural
Florida Registration No. 93594

KLH/JCN:tlf
Q:\Projects\17318 Micanopy Town Hall\17318 Forensic Site Exploration.docx

Distribution: Addressee (1 – Electronic); File (1)

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1.0 INTRODUCTION

1.1 General

GSE Engineering & Consulting, Inc. (GSE) was retained to conduct a limited forensic evaluation of the Micanopy Town Hall located at 706 NE Cholokka Boulevard in Micanopy, Alachua County, Florida (Figure 1).

1.2 Project Description

The Micanopy Town Hall is two-story, brick and wood frame construction. According to the Alachua County Property Appraiser's web site, the structure was constructed in 1895 and has a total area of about 10,290 square feet.

Cracking damage developed in the western end of the ground floor of the building on the north side of the hallway over the past few months. The damage consists of a diagonal crack above a door frame. We understand there are doors on the second floor at the east end of the building that are out of square and bind in the frame.

1.3 Purpose

The purpose of this evaluation was to determine the severity of the damage in the building and to determine whether any remedial repairs are warranted.

2.0 SITE RECONNAISSANCE

Mr. Everett L. Skipper E.I., Adam Davis, and Chauncey L. Garrison II, with GSE, visited the site on March 18, 2026 to perform our site reconnaissance. Members of the Micanopy Town Hall staff were present during our site visit.

2.1 General Site Observations

The building is two-story, brick and wood frame construction supported by brick and mortar piers on shallow foundations. The building faces east. The floor systems consist of wood beams and joists. The exterior walls are brick construction. The interior walls are covered with plastered wallboard, the drop ceiling consists of mineral fiber ceiling tiles, and the flooring within the building consists of laminate vinyl plank, hardwood flooring, ceramic tile, and carpet. There are historical remedial floor repairs within the crawl space consisting of floor beam repairs and newer CMU pier foundations.

Gutters are present along portions of the rooflines with downspouts that discharge the water away from the building. Appreciable erosion was not observed near the foundations. Landscaping consists of shrubs along the perimeter of the building and small to large trees around the majority of the property.

2.2 Summary of Damage Observations

The maintenance personnel onsite gave historical testimony relating to the building's condition, going back multiple decades. GSE was informed that the current condition of the building had remained relatively unchanged for (approximately) the last 50 years. Minor water damage events and repairs were described without major structural concerns. The main concern relayed to GSE was concentrated at the area below and surrounding a level 2 storage room and the corresponding level 1 council room below.

Most of the concerns described were related to doors that were previously operable are now difficult to open and close, as well as floors that have substantial elevation changes.

During our discussions with maintenance personnel, GSE was informed that a level 2 room was used as storage for a large number (approximately 200) file boxes. These file boxes were estimated to be 24"x12"x10" each and were described as being completely filled with paper files and documents.

Cracking and separation damage were noted on both the interior and exterior of the building. Figure 2 illustrates the overall layout and observed representative damage to the interior and exterior of the building.

Damage within the interior of the building consists of separations between some of the doors and door frames (1/4" to 1/2" in width), cracks in the wallboards (hairline to 1/16" in width), cracks in the walls above the corners of some of the windows (hairline to 1/16" in width), and separations between the floors and walls in the level 1 council room and level 2 storage room (1/4" to 1/2" in width).

Exterior damage consists of cracks in the brick walls (approximately hairline to 1/16" in width) and cracking and voids within the mortar joints of the brick knee wall at the main entrance.

3.0 PUBLISHED DATA REVIEW

This section presents a review of readily available published information related to topography and soil survey information.

3.1 Review of Published Topographic Data

The Alachua County Growth Management website indicates the ground surface elevations in the area of the building are near 110 feet[1]. The building is located on a fairly level lot. Regional topography is gently to moderately sloping hills.

3.2 Review of Published Soil Information

The Alachua County Soil Survey[2] maps as one soil type in the vicinity of the building. The following soil description is from the County soil survey.

Newnan sand - This nearly level, somewhat poorly drained soil is in small to relatively large areas in the flatwoods. Slopes are nearly level to slightly convex and range from 0 to 2 percent. The areas generally range from about 10 to 250 acres.

Typically, the surface layer is dark gray sand about 5 inches thick. The subsurface layer is light brownish gray sand to a depth of 12 inches. The upper part of the subsoil is 4 inches of dark brown sand, in which the sand grains are well coated with organic material, and 4 inches of dark brown sand that is mottled. Below this is a leached layer of light gray to white sand to a depth of 56 inches. The lower part of the subsoil is loamy, light gray, and mottled. The upper 3 inches is loamy sand, the next 16 inches is fine sandy loam, and the lower 7 inches is sandy clay loam.

Included with this soil in some areas are Mulat, Pomona, Sparr, and Wauchula soils. In some areas are soils that have characteristics similar to Newnan soils except that they have a brown, organically stained layer directly below the surface layer or have only 1 to 3 inches of leached, light gray or white material between the surface layer and the stained layer. About 65 acres mapped as Newnan soil is within the flood plain of the Santa Fe River and is occasionally flooded. Total included areas are about 20 percent or less.

This Newnan soil has a water table that is at a depth of 18 to 30 inches for 1 to 2 months during most years and at a depth of 30 to 60 inches for 2 to 5 months. During drier periods, it is at a depth of more than 60 inches. The available water capacity is very low to low to a depth of about 12 inches and low to medium from 12 to 82 inches. Permeability is rapid to a depth of about 12 inches, moderately rapid to rapid from 12 to 16 inches, rapid from 16 to 56 inches, moderately rapid from 56 to 59 inches, and slow to moderately slow from 59 to 82 inches. Natural fertility is low in the sandy upper 56 inches and medium in the loamy subsoil below. Organic matter content is moderately low.

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[1] Alachua County Growth Management website, http://mapgenius.alachuacounty.us/.

[2] Soil Survey of Alachua County, Florida. Soil Conservation Service, U.S. Department of Agriculture.

4.0 FIELD AND LABORATORY TESTS

4.1 General Description

The procedures used for field sampling and testing are in general accordance with industry standards of care and established engineering and geological investigation practices for this geographic region. This exploration consisted of performing a relative floor elevation survey, a foundation test pit, Standard Penetration Test borings, and laboratory tests on samples recovered from the site. The following sections describe our field and laboratory testing program in more detail.

4.2 Auger Borings

The auger borings were performed in accordance with ASTM D1452. The borings were performed with hand auger equipment that was rotated into the ground in a manner that reduces soil disturbance. After penetrating to the required depth, the auger was retracted and the soils collected on the auger flights were field classified and placed in sealed containers. Representative samples of each stratum were retained from the auger boring. Results from the auger borings are provided in Section 5.1. The auger boring locations are indicated on Figure 2.

4.3 Relative Floor Elevation Surveys

A relative floor elevation survey of the interior was performed using a Zip Level Pro®. Data for the floor elevation survey was collected at random points in the rooms that were readily accessible. GSE does not move furniture to obtain the floor elevation data. The data is accurate to approximately 0.1 inch. The data is used in a computer model that plots contours of the relative elevation of the floor slab. The floor elevation survey map is not prepared by a licensed surveyor, and is not to be considered a survey as regulated by §472, Florida Statutes. The results of the relative floor elevation survey are provided on Figures 3 and 4.

4.4 Soil Laboratory Tests

The soil samples recovered from the soil borings were returned to our laboratory, and examined to confirm the field descriptions. Representative samples were then selected for laboratory testing. The laboratory tests consisted of the percent soil fines passing the No. 200 sieve determinations, natural moisture content determinations, Atterberg Limits tests, and an organic content determination. These tests were performed in order to aid in classifying the soils and to further evaluate their engineering properties. The laboratory tests are provided in Section 8.2.

5.0 FINDINGS

This section summarizes the findings of the field and laboratory services.

5.1 Relative Floor Elevation Survey Results

The first-floor interior relative floor elevation survey indicates approximately 4.5 inches of elevation change occurs across the floor. The approximate northern half of the building is relatively level, with elevations generally ranging from about 3.0 to 3.5 inches, indicating less than approximately 0.5 inches of variation across that area. The southeastern portion of the building exhibits a gradual slope downward toward the south, with approximately 2.5 to 3.0 inches of elevation change across that portion of the building. The lowest elevations, on the order of approximately 0.5 to 1.0 inches, are observed in the southeastern portion of the floor.

The second-floor interior relative floor elevation survey indicates approximately 4.9 inches of elevation change occurs across the floor. The floor elevations generally range from about 3.0 to 4.5 inches across much of the building, indicating moderate variability. A distinct low area, with elevations on the order of approximately 0.5 to 1.0 inches, is present in the southeastern portion of the building, consistent with the first-floor trends. Additionally, a secondary low area with elevations of approximately 1.0 to 2.0 inches is observed in the northwestern portion of the second floor. These areas have about 2.5 to 3.5 inches of elevation difference relative to nearby higher areas, indicating movement across the building, with the greatest settlement in the southeastern portion and some variation toward the northwest.

The relative floor elevation survey results are provided on Figures 3 and 4.

5.2 Auger Boring Results

Three (3) hand auger borings were performed at the site around the perimeter of the building. The selected boring locations considered the damage to the building and relative floor elevation survey. The soil boring locations are shown on Figure 2. Descriptions for the soils encountered are based on visual observations of the recovered soil samples and the laboratory testing performed. Stratification boundaries between the soil types should be considered approximate, as the actual transition between soil types may be gradual. The complete boring logs are provided in Section 8.1. The key to soil classifications is provided in Section 8.3.

The auger borings typically penetrated 3 to 3.7 feet of sand with silt (SP-SM). This was underlain by interbedded strata consisting of sand with silt and trace clay, silty to very silty sand, clayey sand, and cemented sand (SP-SM, SM, SC, SP) to depths of 6 to 6.5 feet bls. Boring HA-1 encountered brick debris approximately 5.25 feet bls. Borings HA-2 and HA-3 encountered cemented sand at approximately 6 feet bls.

The groundwater table was not encountered within a depth of 6 feet bls at the boring locations.

5.3 Laboratory Soil Analysis

Selected soil samples recovered from the soil borings were analyzed in order to aid in classifying the soils and to further evaluate their engineering properties. The laboratory tests consisted of three (3) percent passing the No. 200 sieve determinations, three (3) natural moisture content determinations, two (2) Atterberg Limits tests and one (1) organic content determination. Soil samples selected for laboratory testing were collected from depths of 3 to 5.5 feet bls. The complete laboratory report is provided in Section 8.2.

The laboratory tests indicate the tested soils consist of clayey sand, sand with silt and organics, and very silty sand.

The tested clayey sand (SC) contains 26 percent soil fines passing the No. 200 sieve and has a natural moisture content of about 15 percent. The tested sand with silt (SP-SM) contains approximately 16 percent soil fines passing the No. 200 sieve with a natural moisture content of about 15 percent. The tested very silty sand (SM) contains approximately 32 percent soil fines passing the No. 200 sieve and has a natural moisture content of about 18 percent.

The Atterberg Limits test indicates the clayey sand has a Liquid Limit (LL) value of 26, Plastic Limit (PL) value of 11, and Plasticity Index (PI) value of 15. This corresponds to materials with low potential (LL < 50 and PI < 25) for expansive behavior[3]. The tested very silty sand (SM) indicates it is nonplastic.

The organic content determinations indicate the tested sand with silt (SP-SM) contains approximately 3.9 percent organic matter. Typically, soils with greater than 5 percent organic content are considered unsuitable for shallow foundation support.

[3] U.S. Department of the Army USA, 1983, Foundations in Expansive Soils, TM 5-818-7, p. 4-1.

6.0 EVALUATION

In this section of the report, we present our evaluation of the site and subsurface conditions, in addition to our evaluation of the damage to the building.

6.1 Summary Evaluation of Site and Subsurface Conditions

The material types and depths encountered by the soil borings are generally consistent with the County Soil Survey mappings. The borings indicate the subsurface soils beneath the building consist primarily of sand with silt, silty sand, sand with clay, and clayey sand within the upper approximately 5 to 6 feet, underlain by cemented sand.

The investigation identified the presence of small amounts of organic materials, including roots and minor organic content, interbedded within the near-surface soils beneath the building. In addition, red brick fragments were encountered within the subsurface soils, indicating the possible presence of buried debris. Organic materials at shallow depths are susceptible to decomposition over time, which can result in a reduction in soil volume and localized settlement. Additionally, the presence of roots may lead to the formation of voids as they decay, further reducing the soil's ability to provide uniform support. Similarly, buried debris may not provide uniform support and can result in localized weak zones. The variability in the distribution of these materials can contribute to differential settlement across the building.

The investigation also identified the presence of clayey sand within the shallow subsurface soils. Laboratory testing indicates these soils exhibit low potential for expansive behavior and are not considered a primary contributor to the observed movement; however, they may still exhibit minor volume change or strength variation with changes in moisture conditions.

6.2 Structural Summary Evaluation of Observed Damage

The observed damage at the Micanopy Town Hall building is mainly attributed to differential floor and foundation movement, likely caused by localized settlement associated with the overloading of the foundations and floor members below the level 2 file storage room and level 1 council room.

Other types of damage observed included areas of historical settlements, wall cracking in both the exterior and interior walls, cracking and voids in the mortar joints of a brick knee wall, and separations between the wall and floor of the level 2 file storage room and level 1 council room.

The weight attributed to the number of file boxes in the level 2 storage room exceeds the allowable deadload that the floor systems of the Micanopy Town Hall are intended to support. Because of this concentrated loading, the foundations below this storage room have settled, along with differential deflections and settlements in the supporting floor members.

Historical differential settlements were observed within the floor systems of the subject building. Maintenance personnel described that these settlements had been observed decades ago and were not a concern. Refer to Figures 3 & 4 for the results of the Relative Floor Elevation Surveys.

The cracking and separation damage observed within the walls of the structure were within the expected range for a building of this age and construction type. Additionally, the observed differential settlements could have contributed to this cracking and separation damage. This cracking is cosmetic and is not considered a structural concern.

The cracking and voids observed within the brick knee wall at the main entrance is cosmetic and is not considered a structural concern. This is a maintenance issue that should be corrected to prevent future deterioration.

The observed cosmetic damage is attributed to material shrinkage, thermal expansion and contraction, construction workmanship, influence of age, material deterioration, and differential foundation movement. Contributing factors to the differential foundation movement that cannot be ruled out within a reasonable professional probability include post-construction settlement due to very loose to loose near-surface sandy soils, the consolidation and compression of organic-rich materials, and effects of highly expansive clay-rich soils.

It is GSE's opinion that the Micanopy Town Hall structure is suitable for everyday use. For usability and serviceability, GSE recommends that improvements be made to alleviate some of the concerns described including doors not opening or operating correctly as well as uneven floors.

7.0 RECOMMENDATIONS

In order to improve the serviceability of the Micanopy Town Hall structure, GSE recommends that new pier foundations be installed below existing deflected supporting floor beams or that new beams and pier foundations be constructed to supplement the existing foundation system. Bottle jacks should be installed at the top of each new pier. These bottle jacks would be used to methodically and uniformly support and lift the areas of the greatest differential settlement.

GSE recommends that a foundation repair contractor submit a shop drawing for GSE to review. This shop drawing should describe the construction of the new pier foundations to be installed. For the lifting operations, GSE recommends a maximum of ½" of lift. Any lifting performed beyond this limit is likely to cause additional structural damage to the structure.

Lifting operations should be monitored and performed in a controlled and methodical process. Please be aware that all lifting procedures have a chance to cause cosmetic damage in the walls, floors, and ceilings of the existing structure. Should unexpected shifting, cracking, or any other damage be observed during the lifting procedures, lifting operations should cease immediately and the engineer of record should be notified.

8.0 FIELD DATA

8.1 Auger Boring Logs

GSE Engineering & Consulting, Inc.
GSE Engineering
5590 SW 64th St
Gainesville, FL 32608
Telephone: 3523773233

CLIENT Town of Micanopy **PROJECT NAME** Micanopy Town Hall

PROJECT NUMBER 17318 **PROJECT LOCATION** Micanopy, Alachua County, Florida

DATE PERFORMED 3/18/2026 **BORING NUMBER HA-1**

DRILLING CONTRACTOR GSE

GROUND WATER LEVELS: LOGGED BY CLG

▼ AT TIME OF DRILLING NE CHECKED BY CLG

▽ ESTIMATED SEASONAL HIGH 1.5 ft

NOTES

DEPTH (ft)	GRAPHIC LOG	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0		AU 1	(SP-SM) Dark gray SAND with silt and small roots
1.0			(SP-SM) Pale gray SAND with silt
		AU 2	▽
2.5			
3.0		AU 3	(SM) Brown silty SAND
3.5		AU 4	(SP-SC) Gray, brown, and orange SAND with clay
5.0		AU 5	(SC) Gray and orange clayey SAND *Red brick identified at 5.25 ft bls.* *%PASS-200 = 26* *MC = 15* *LL = 26 PL = 11 PI = 15*
6.0			Bottom of borehole at 6.0 feet. *Boring terminated due to auger refusal*

DATE PERFORMED 3/18/2026 **BORING NUMBER HA-2**

DRILLING CONTRACTOR GSE

GROUND WATER LEVELS: LOGGED BY CLG

▼ AT TIME OF DRILLING NE CHECKED BY CLG

▽ ESTIMATED SEASONAL HIGH 1.5 ft

NOTES

DEPTH (ft)	GRAPHIC LOG	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0		AU 1	(SP-SM) Gray SAND with silt and roots
1.5			▽
		AU 2	(SP-SM) Pale gray SAND with silt and roots
2.5			
		AU 3	(SP-SM) Dark brown SAND with silt and organics *%PASS-200 = 16* *MC = 15* *ORG = 3.9*
3.5		AU 4	(SP-SM) Brown and tan SAND with silt and trace clay
4.5			(SC) Gray clayey SAND with cemented sand fragments
5.0		AU 5	
6.0		AU 6	(SP) Gray cemented SAND
6.5			Bottom of borehole at 6.5 feet. *Boring terminated due to auger refusal.*

(Continued Next Page)

GSE Engineering
5590 SW 64th St
Gainesville, FL 32608
Telephone: 3523773233

CLIENT Town of Micanopy **PROJECT NAME** Micanopy Town Hall

PROJECT NUMBER 17318 **PROJECT LOCATION** Micanopy, Alachua County, Florida

DATE PERFORMED 3/18/2026 **BORING NUMBER HA-3**

DRILLING CONTRACTOR GSE

GROUND WATER LEVELS: LOGGED BY CLG

AT TIME OF DRILLING NE CHECKED BY CLG

ESTIMATED SEASONAL HIGH 1.5 ft

NOTES

DEPTH (ft)	GRAPHIC LOG	SAMPLE TYPE NUMBER	MATERIAL DESCRIPTION
0.0		AU 1	(SP-SM) Dark gray SAND with silt and roots
			▽ 1.5
		AU 2	(SP-SM) Gray SAND with silt
2.5			2.5
		AU 3	(SP-SM) Brown and tan SAND with silt
			3.7
		AU 4	(SM) Gray silty SAND with trace clay
			4.5
		AU 5	(SM) Gray and orange very silty SAND
5.0			*%PASS-200 = 32* *MC = 18* *LL = NP PL = NP PI = NP*
			5.5
		AU 6	(SP) Gray cemented SAND
			6.5
			Bottom of borehole at 6.5 feet.
			Borehole terminated due to auger refusal

8.2 Laboratory Results

GSE Engineering & Consulting, Inc.

SUMMARY REPORT OF LABORATORY TEST RESULTS

Project Number: 17318

Project Name: Micanopy Town Hall

Boring Number	Depth (ft)	Soil Description	Natural Moisture Content (%)	Liquid Limit	Plastic Limit	Plasticity Index	Percent Passing No. 200 Sieve	Organic Content (%)	Hydraulic Conductivity (ft/day)	Unified Soil Classification
HA-1	5-5.5	Gray and orange clayey SAND	15	26	11	15	26			SC
HA-2	3-3.5	Dark brown SAND with silt and organics	15				16	3.9		SP-SM
HA-3	4.5-5	Gray and orange very silty SAND	18	NP	NP	NP	32			SM

8.3 Key to Soil Classification

KEY TO SOIL CLASSIFICATION CHART

Criteria for Assigning Group Symbols and Group Names Using Laboratory Tests				SYMBOLS		GROUP NAME
				GRAPHIC	LETTER	
COARSE-GRAINED SOILS	Gravels	Clean Gravels	Cu ≥ 4 and 1 ≤ Cc ≤ 3		**GW**	Well graded GRAVEL
More than 50% retained on No. 200 sieve	More than 50% of coarse fraction retained on No. 4 sieve	Less than 5% fines	Cu < 4 and/or 1 > Cc > 3		**GP**	Poorly graded GRAVEL
		Gravels with fines	Fines classify as ML or MH		**GM**	Silty GRAVEL
		More than 12% fines	Fines classify as CL or CH		**GC**	Clayey GRAVEL
	Sands	Clean Sands	Cu ≥ 6 and 1 ≤ Cc ≤ 3		**SW**	Well graded SAND
	50% or more of coarse fraction passes No. 4 sieve	Less than 5% fines	Cu < 6 and/or 1 > Cc > 3		**SP**	Poorly graded SAND
		Sand with fines	Fines classify as ML or MH		**SP-SM**	SAND with silt
		5% ≤ fines < 12%	Fines classify as CL or CH		**SP-SC**	SAND with clay
		Sand with fines	Fines classify as ML or MH		**SM**	Silty SAND
		12% ≤ fines < 30%	Fines classify as CL or CH		**SC**	Clayey SAND
		Sand with fines	Fines classify as ML or MH		**SM**	Very silty SAND
		30% fines or more	Fines classify as CL or CH		**SC**	Very clayey SAND
FINE-GRAINED SOILS	Clays	inorganic	50% ≤ fines < 70%		**CL/CH**	Sandy CLAY
50% or more passes the No. 200 sieve			70% ≤ fines < 85%		**CL/CH**	CLAY with sand
			fines ≥ 85%		**CL/CH**	CLAY
	Silts and Clays	inorganic	PI > 7 and plots on/above "A" line		**CL**	Lean CLAY
	Liquid Limit less than 50		PI < 4 or plots below "A" line		**ML**	SILT
		organic	Liquid Limit - oven dried / Liquid Limit - not dried < 0.75		**OL**	Organic clay / Organic silt
	Silts and Clays	inorganic	PI plots on or above "A" line		**CH**	Fat CLAY
	Liquid Limit 50 or more		PI plots below "A" line		**MH**	Elastic SILT
		organic	Liquid Limit - oven dried / Liquid Limit - not dried < 0.75		**OH**	Organic clay / Organic silt
HIGHLY ORGANIC SOILS	Primarily organic matter, dark in color, and organic odor				**PT**	PEAT

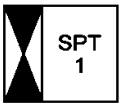
CORRELATION OF PENETRATION RESISTANCE WITH RELATIVE DENSITY AND CONSISTENCY

	No. OF BLOWS, N	RELATIVE DENSITY
	0 - 4	Very Loose
	5 - 10	Loose
SANDS:	11 - 30	Medium dense
	31 - 50	Dense
	OVER 50	Very Dense

	No. OF BLOWS, N	CONSISTENCY
	0 - 2	Very Soft
SILTS	3 - 4	Soft
&	5 - 8	Firm
CLAYS:	9 - 15	Stiff
	16 - 30	Very Stiff
	31 - 50	Hard
	OVER 50	Very Hard

	No. OF BLOWS, N	RELATIVE DENSITY
	0 - 8	Very Soft
	9 - 18	Soft
LIMESTONE:	19 - 32	Moderately Hard
	33 - 50	Hard
	OVER 50	Very Hard

SAMPLE GRAPHIC TYPE LEGEND



Location of SPT Sample

Location of Auger Sample

PARTICLE SIZE IDENTIFICATION

BOULDERS:		Greater than 300 mm
COBBLES:		75 mm to 300 mm
GRAVEL:	Coarse -	19.0 mm to 75 mm
	Fine -	4.75 mm to 19.0 mm
SANDS:	Coarse -	2.00 mm to 4.75 mm
	Medium -	0.425 mm to 2.00 mm
	Fine -	0.075 mm to 0.425 mm
SILTS & CLAYS:		Less than 0.075 mm

LABORATORY TEST LEGEND

LL	=	Liquid Limit, %
PL	=	Plastic Limit, %
PI	=	Plasticity Index, %
% PASS - 200	=	Percent Passing the No. 200 Sieve
MC	=	Moisture Content, %
ORG	=	Organic Content, %
k_h	=	Horizontal Hydraulic Conductivity, ft/day

9.0 LIMITATIONS

9.1 Warranty

This report has been prepared for our client for their exclusive use, in accordance with generally accepted soil and foundation engineering practices, and makes no other warranty either expressed or implied as to the professional advice provided in the report.

9.2 Auger Borings

The determination of soil type and conditions was performed from the ground surface to the maximum depth of the borings, only. Any changes in subsurface conditions that occur between or below the borings would not have been detected or reflected in this report.

Soil classifications that were made in the field are based upon identifiable textural changes, color changes, changes in composition or changes in resistance to penetration in the intervals from which the samples were collected. Abrupt changes in soil type, as reflected in boring logs and/or cross sections may not actually occur, but instead, be transitional.

Depth to the water table is based upon observations made during the performance of the auger borings. This depth is an estimate and does not reflect the annual variations that would be expected in this area due to fluctuations in rainfall and rates of evapotranspiration.

9.3 Site Figures

The measurements used for the preparation of the figures in this report were made using the provided site plan and by estimating distances from existing structures and site features. Figures in this report were not prepared by a licensed land surveyor and should not be interpreted as such.

FIGURES

NORTH

NOT TO SCALE

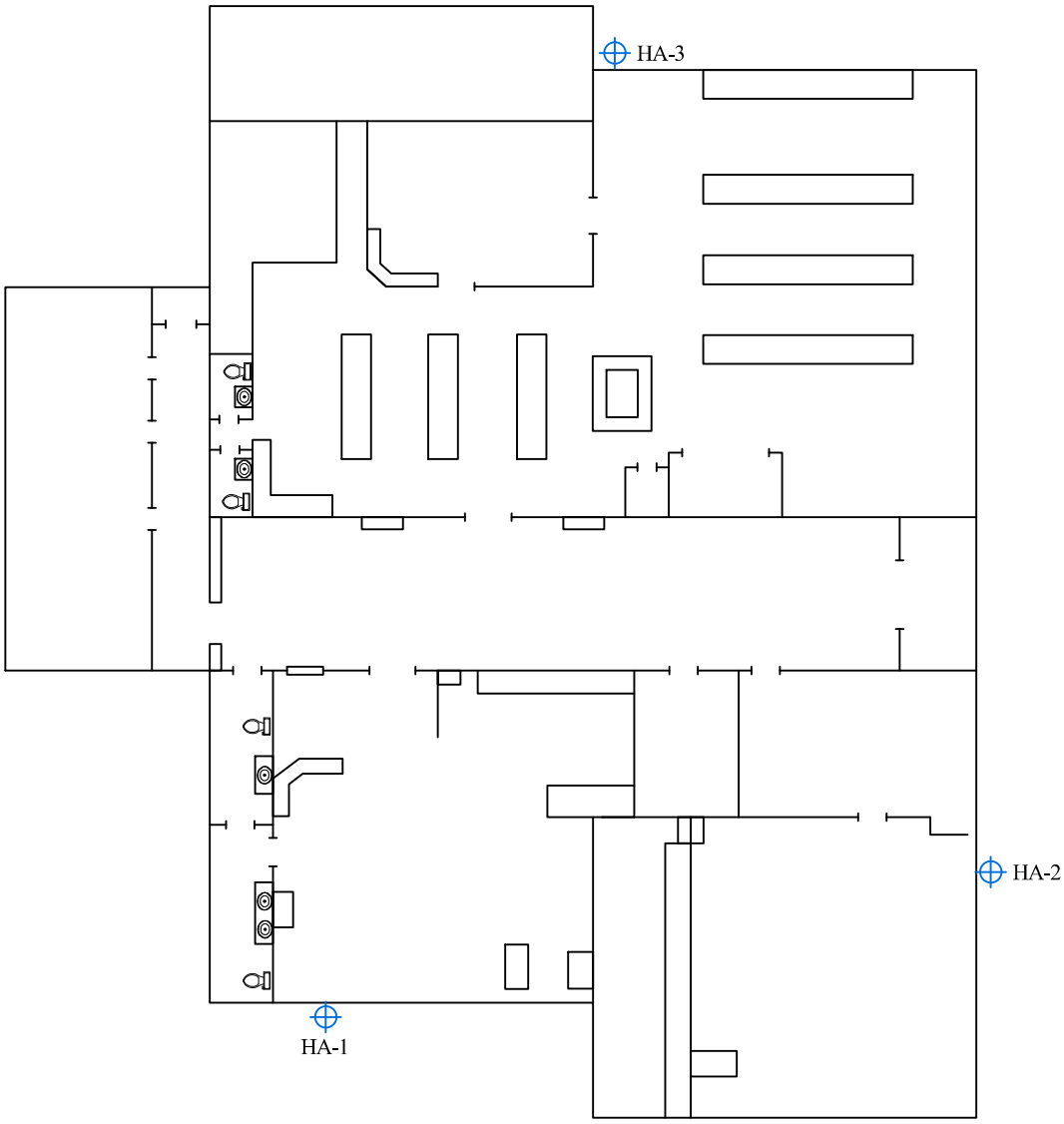
MICANOPY TOWN HALL
MICANOPY, ALACHUA COUNTY, FLORIDA
GSE PROJECT NO. 17318

PROJECT SITE LOCATION MAP

DESIGNED BY: CLG
CHECKED BY : KLH
DRAWN BY : EEG

GSE Engineering & Consulting, Inc.

FIGURE 1



LEGEND:

AUGER BORING

NORTH

0 15'

SCALE: 1" = 15' APPROX.

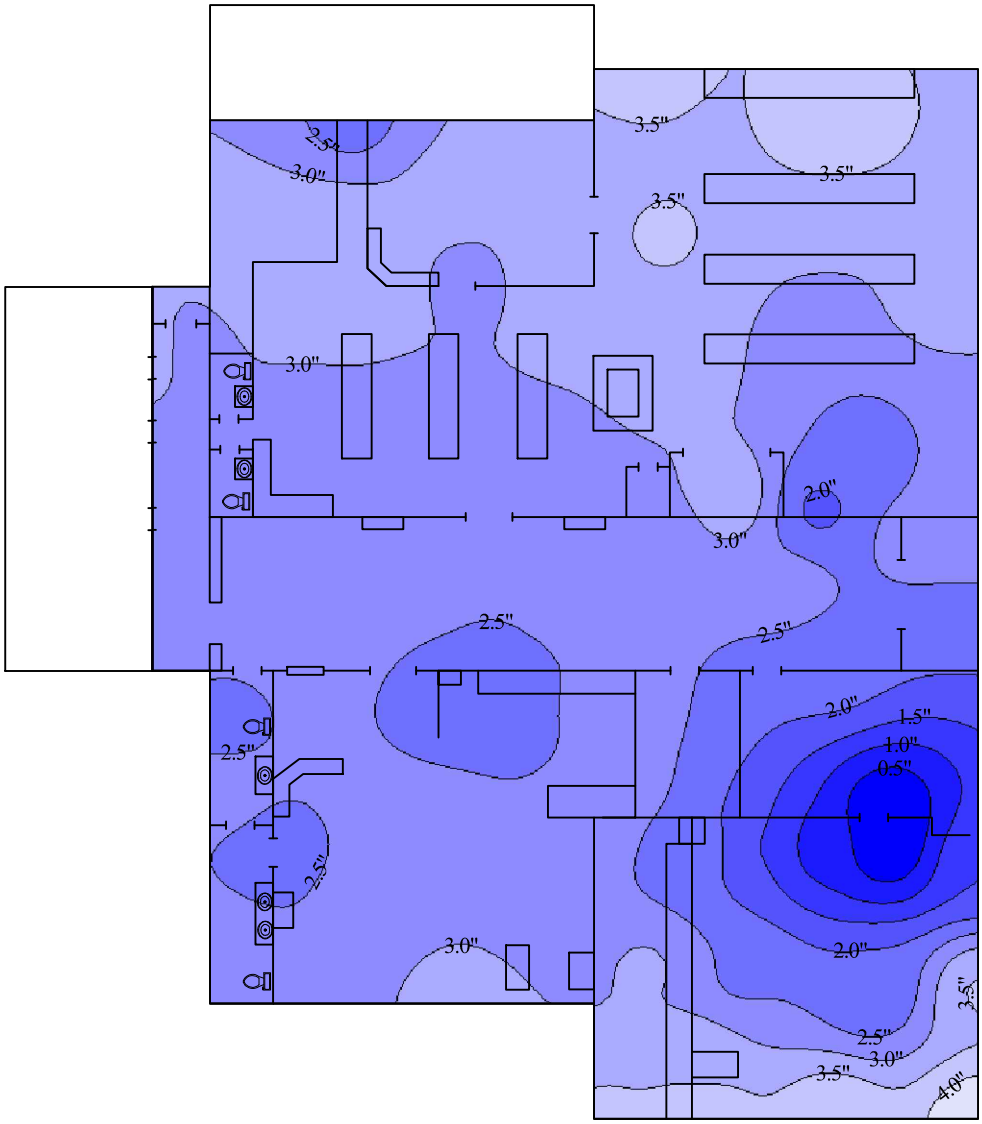
MICANOPY TOWN HALL
MICANOPY, ALACHUA COUNTY, FLORIDA
GSE PROJECT NO. 17318

SITE PLAN SHOWING APPROXIMATE LOCATIONS OF FIELD TESTS

DESIGNED BY: CLG
CHECKED BY : KLH
DRAWN BY : EEG

GSE Engineering & Consulting, Inc.

FIGURE 2



LEGEND:

- 4.5"*
- 4.0"
- 3.5"
- 3.0"
- 2.5"
- 2.0"
- 1.5"
- 1.0"
- 0.5"
- 0.0"

*MAXIMUM RELATIVE ELEVATION FOR CORRESPONDING AREA

NORTH

0 15'

SCALE: 1" = 15' APPROX.

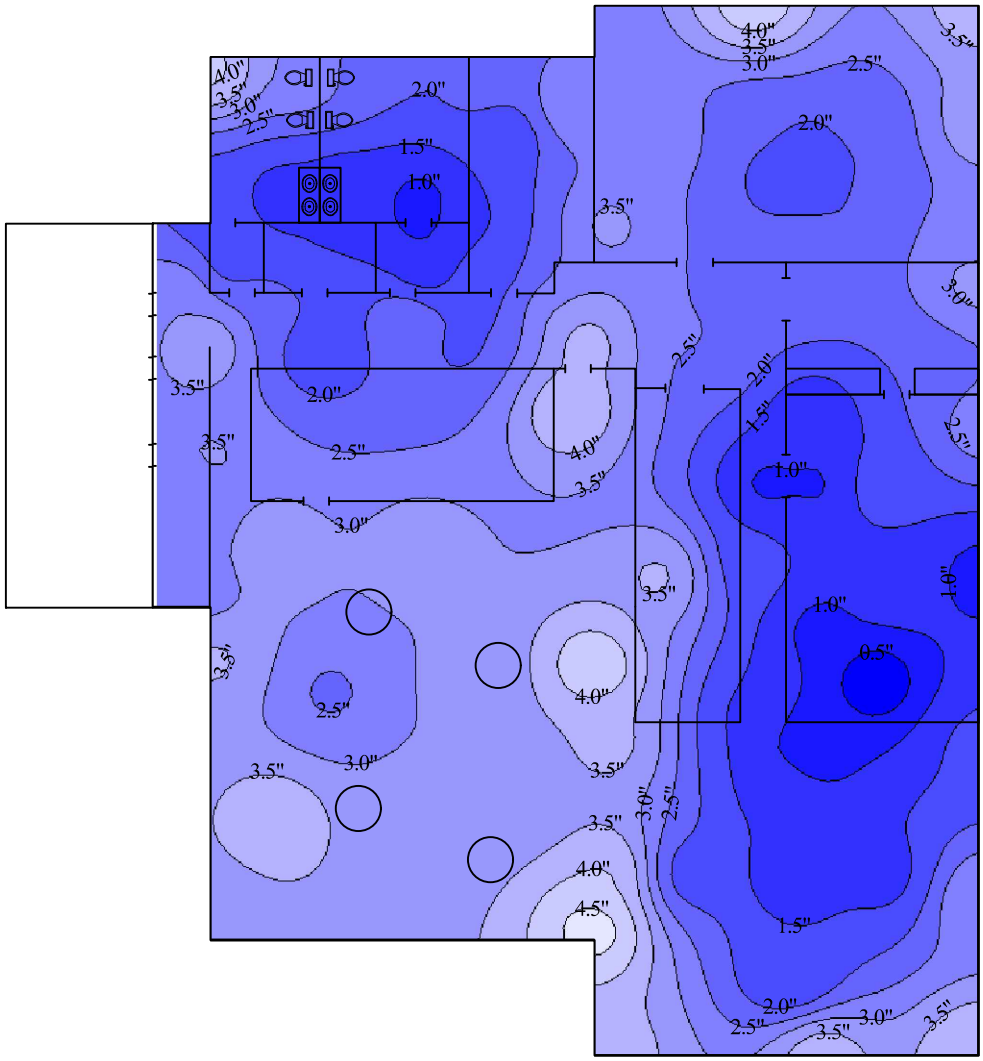
MICANOPY TOWN HALL
MICANOPY, ALACHUA COUNTY, FLORIDA
GSE PROJECT NO. 17318

RELATIVE FLOOR ELEVATION SURVEY
FIRST FLOOR

DESIGNED BY: CLG
CHECKED BY : KLH
DRAWN BY : EEG

GSE Engineering & Consulting, Inc.

FIGURE 3



LEGEND:

- 4.9"*
- 4.5"
- 4.0"
- 3.5"
- 3.0"
- 2.5"
- 2.0"
- 1.5"
- 1.0"
- 0.5"
- 0.0"

*MAXIMUM RELATIVE ELEVATION FOR CORRESPONDING AREA

NORTH

0 15'

SCALE: 1" = 15' APPROX.

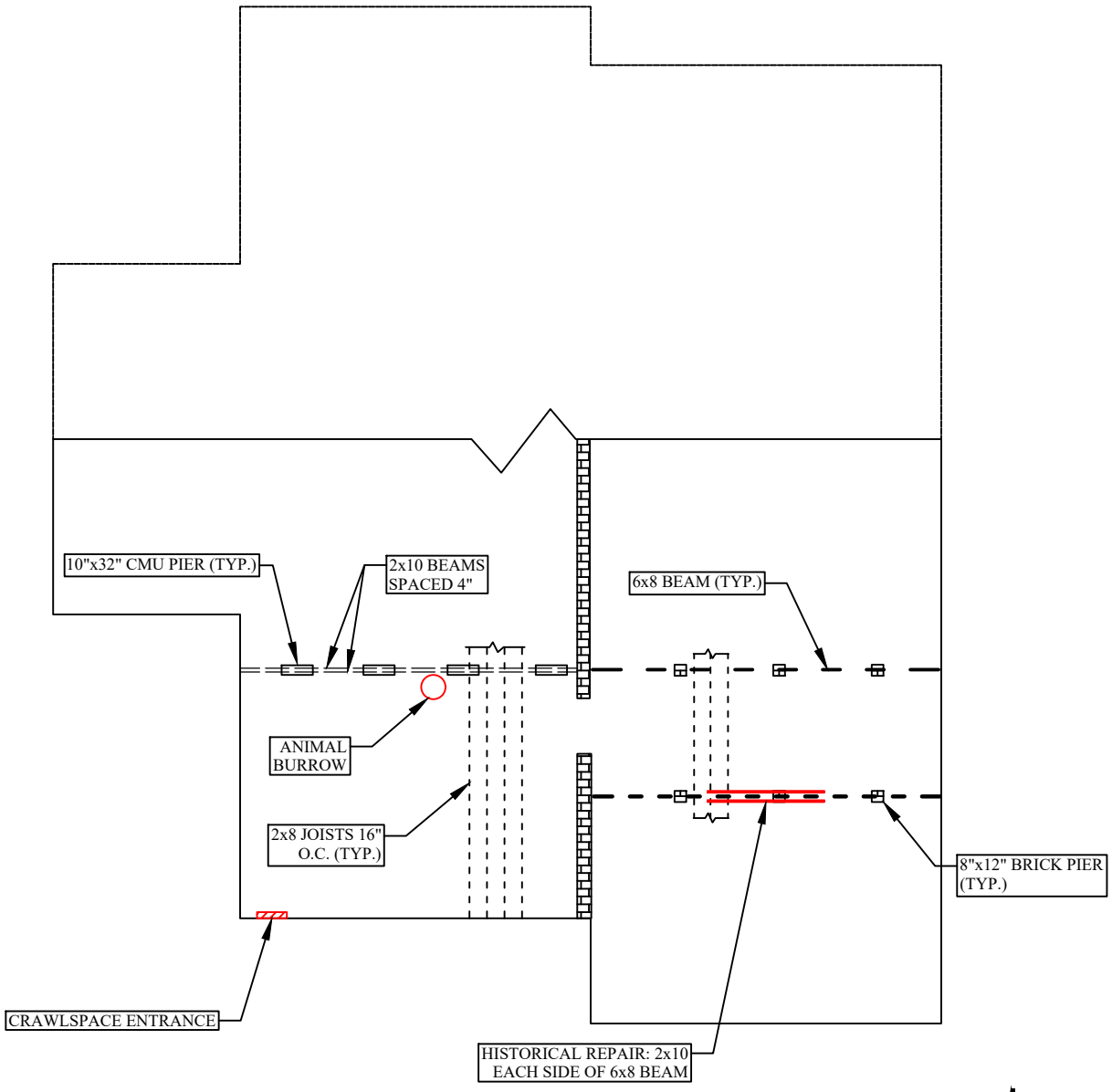
MICANOPY TOWN HALL
MICANOPY, ALACHUA COUNTY, FLORIDA
GSE PROJECT NO. 17318

RELATIVE FLOOR ELEVATION SURVEY
SECOND FLOOR

DESIGNED BY: CLG
CHECKED BY : KLH
DRAWN BY : EEG

GSE Engineering & Consulting, Inc.

FIGURE 4



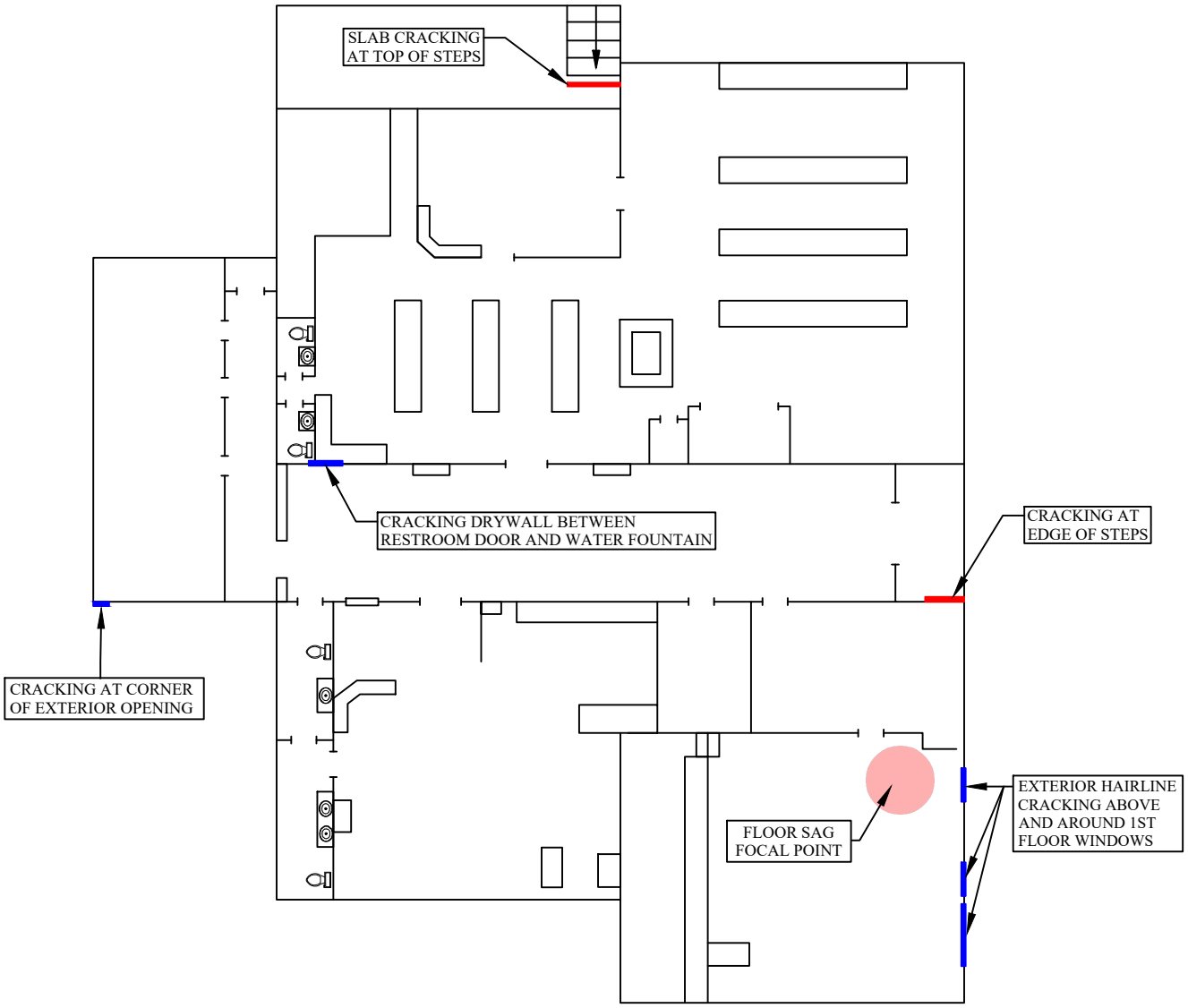
MICANOPY TOWN HALL
MICANOPY, ALACHUA COUNTY, FLORIDA
GSE PROJECT NO. 17318A

CRAWLSPACE OBSERVATION LOCATIONS

DESIGNED BY: AJD
CHECKED BY : JCN
DRAWN BY : IMH

GSE Engineering & Consulting, Inc.

FIGURE 5



SCALE: 1" = 15' APPROX.

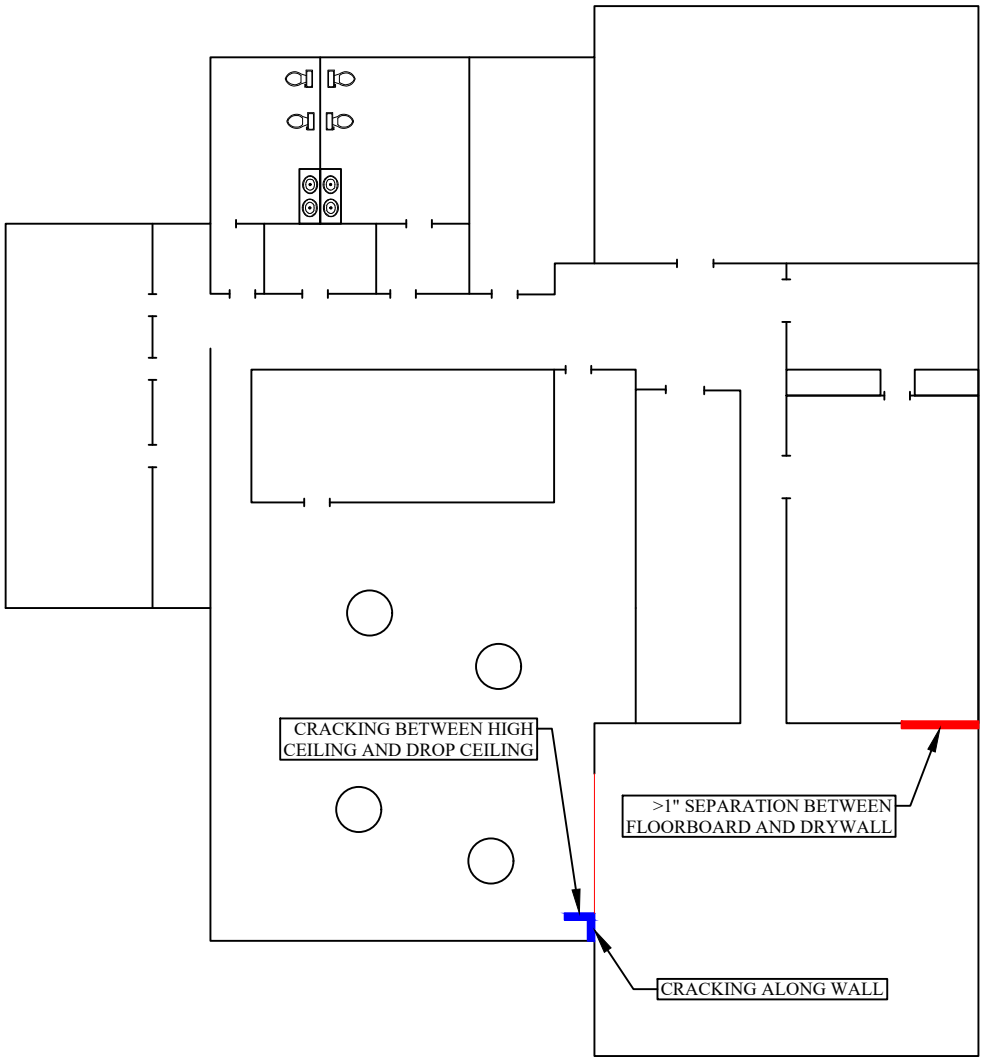
MICANOPY TOWN HALL
MICANOPY, ALACHUA COUNTY, FLORIDA
GSE PROJECT NO. 17318A

FIRST FLOOR AND EXTERIOR OBSERVATION LOCATIONS

DESIGNED BY: AJD
CHECKED BY : JCN
DRAWN BY : IMH

GSE Engineering & Consulting, Inc.

FIGURE 6



NORTH

0 15'

SCALE: 1" = 15' APPROX.

MICANOPY TOWN HALL
MICANOPY, ALACHUA COUNTY, FLORIDA
GSE PROJECT NO. 17318A

SECOND FLOOR OBSERVATION LOCATIONS

DESIGNED BY: AJD
CHECKED BY : JCN
DRAWN BY : IMH

GSE Engineering & Consulting, Inc.

FIGURE 7