## LEGAL DESCRIPTION

Land Situated in the State of Michigan, County of Oakland, City of Madison Heights.

art of the Northwest 1/4 of Section 2, Town 1 North, Range 11 East described as: ommencing at the North 1/4 corner of Section 2: thence along the North-South 1/4 mer of said Section, South 00 degrees 09 minutes 23 seconds West 890.17 feet; thence of said Section, South 00 degrees 09 minutes 23 seconds West 180.17 feet; thence of said Section 1 the Westerly 1/4 feet 1/4

THE LANDS SURVEYED, SHOWN AND DESCRIBED HEREON ARE THE SAME LANDS AS DESCRIBED IN THE TITLE COMMITMENT PROVIDED BY FIDELITY NATIONAL TITLE INSURANCE COMPANY, COMMITMENT NO. A0714833, DATED MARCH 30, 2017 AT 7:59 AM.

### BENCHMARKS

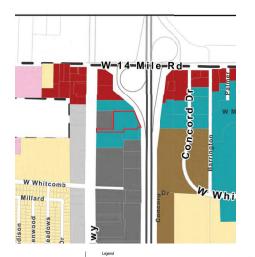
SITE BENCHMARK #1: ARROW ON HYDRANT ELEV.= 615.62 (NAVD8)

SITE BENCHMARK #2: ARROW ON HYDRANT FLEV.= 615.62 (NAVD88)

SITE BENCHMARK #3: ARROW ON HYDRANT ELEV.= 615.62 (NAVD88)

# UTILITY STATEMENT

ALL EXISTING UTILITIES HAVE BEEN FIELD LOCATED WHERE VISIBLE. UNDERGROUND LOCATIONS HAVE BEEN TAKEN FROM RECORD DOCUMENTS AND NO GUARANTEE CAN BE MADE TO THE COMPLETENESS, EXACINESS OF CORRECTIVESS OF LOCATIONS. THE CONTRACTOR SHALL FIELD VERIFY THE LOCATION OF EXISTING UTILITIES AND RELOCATE AS INCESSEDY TO A LYDIO CONSILICITY MAY LAW CONSTITUTION.



# Zoning Map City of Madison Heights

City of Madison Heights, Michigan

May 13th, 2024

# SITE PLAN FOR:

# **RESIDENCE INN - REDEVELOPMENT**

A MULTI FAMILY REDEVELOPMENT SECTION 2,TOWN 1N, RANGE 11E, CITY OF MADISON HEIGHTS, OAKLAND COUNTY, MICHIGAN

PREPARED FOR:

# L2L MADISON HEIGHTS LLC,

7700 OLD GEORGETOWN ROAD, SUITE 700 BETHESDA, MD 20814



NOT TO SCALE



BOUNDARY & TOPOGRAPHIC SURVEY BY:



NOVI, MICHIGAN 48377



### **Sheet List Table**

 Sheet Number
 Sheet Title

 01
 COVER SHEET

 02
 BOUNDARY TOPOGRAPHIC SURVEY

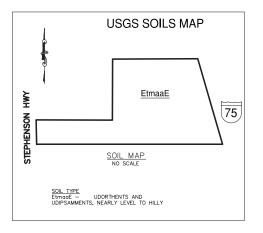
 03
 DEMOLITION PLAN

 04
 SITE PLAN

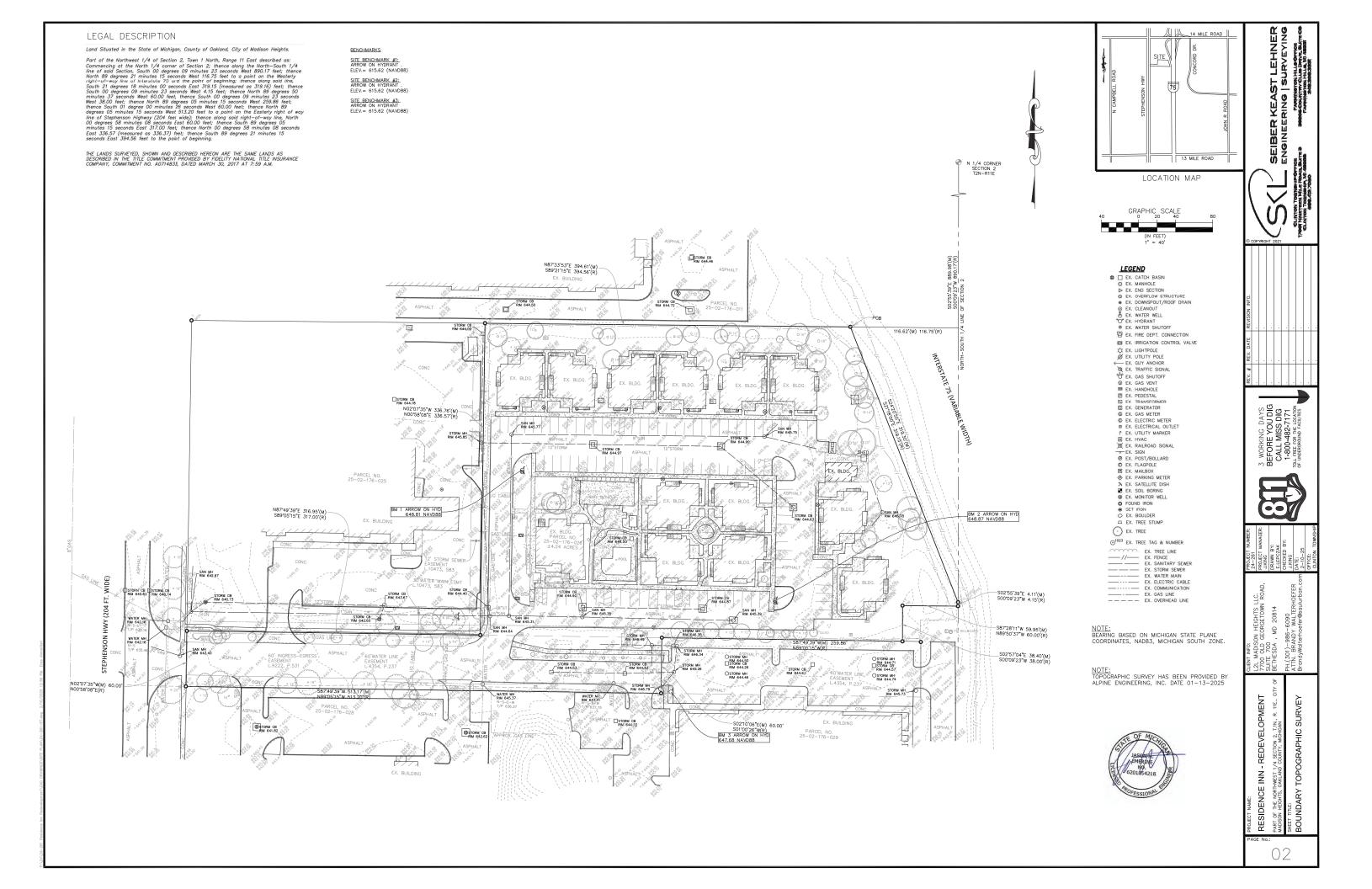
 05
 RAMP DETAILS

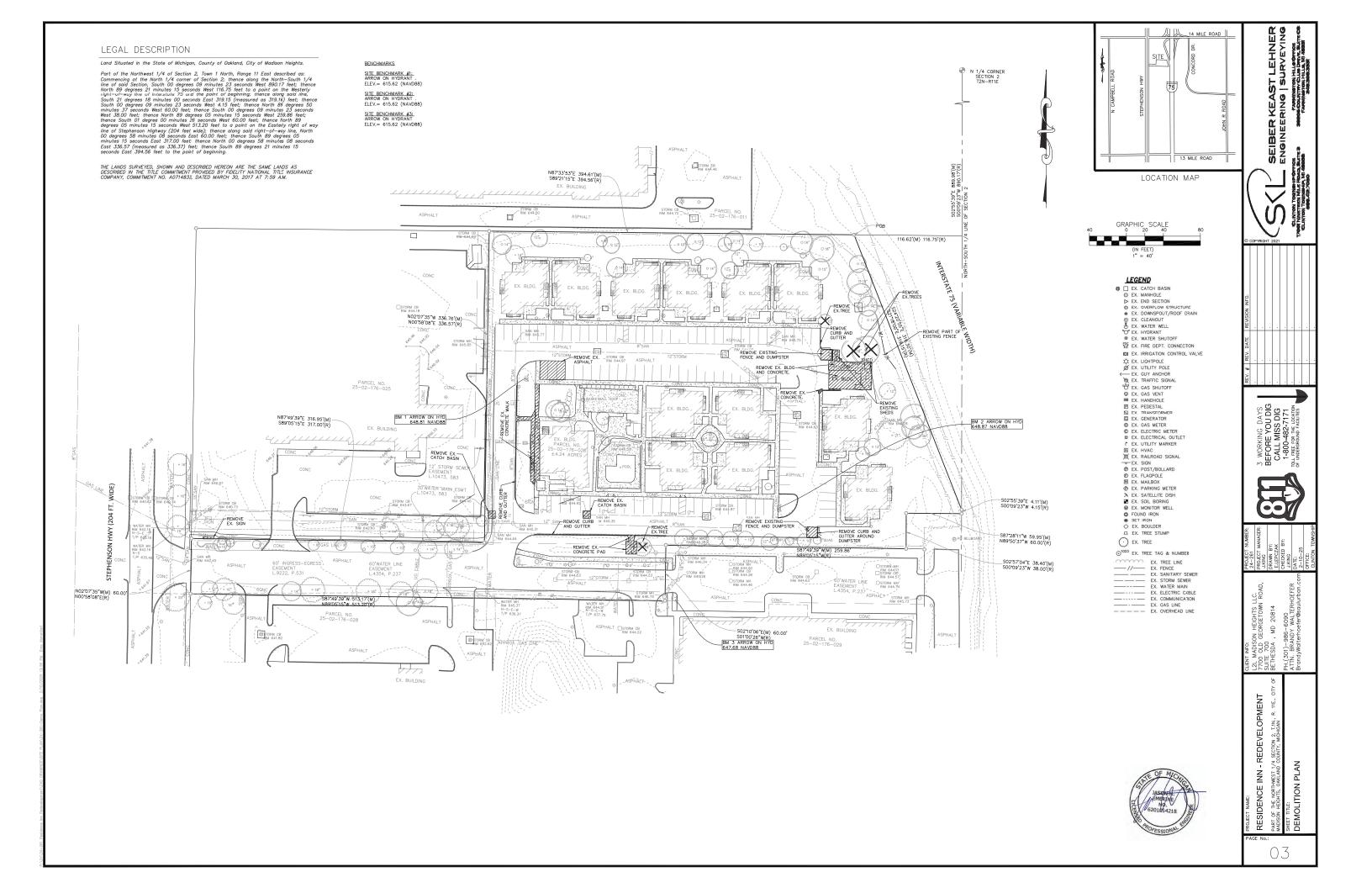
 06
 UTILITY PLAN

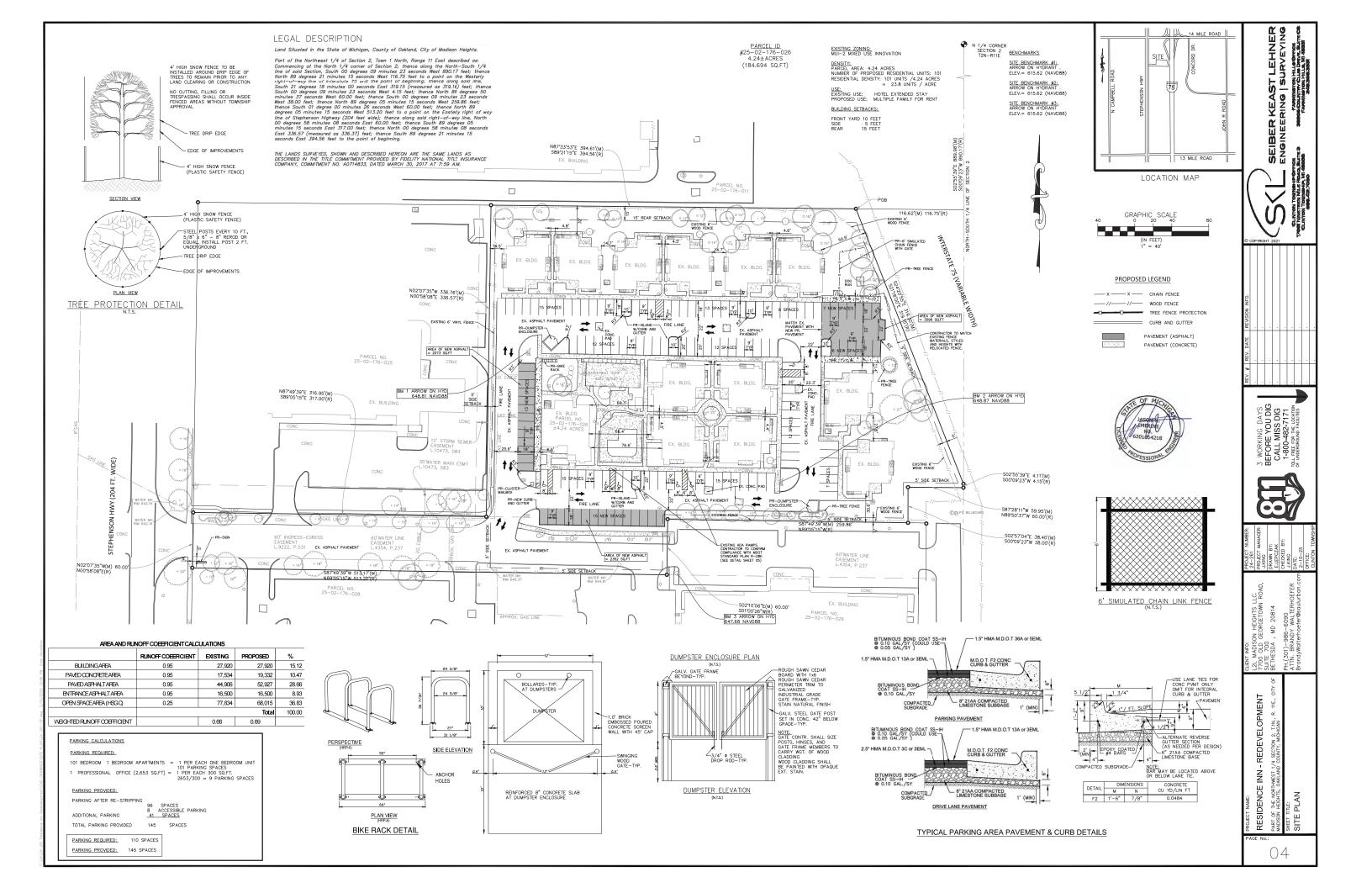
07-08 SOIL INVESTIGATIONS
L1-L2 LANDSCAPE PLAN
SPA.a01-04 ARCHITECTURAL
1 of 1 PHOTOMETRIC PLAN

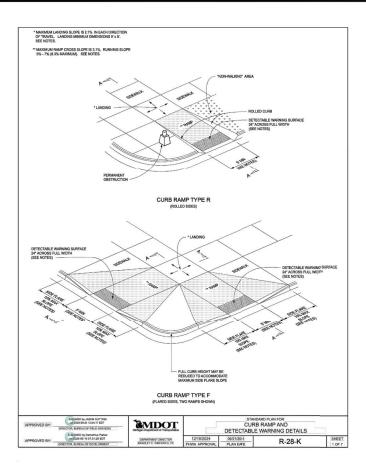


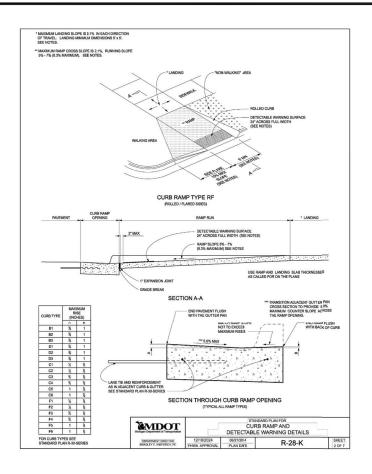
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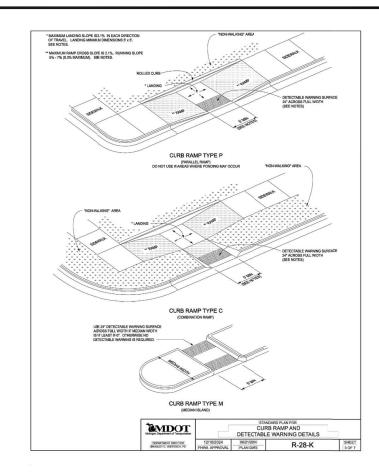


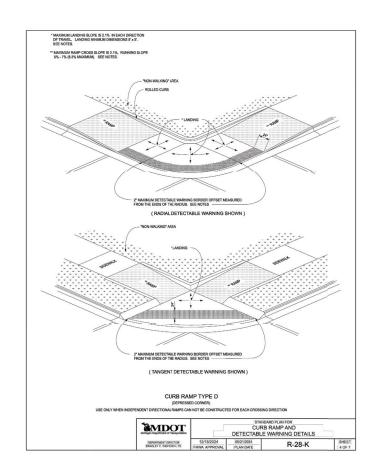


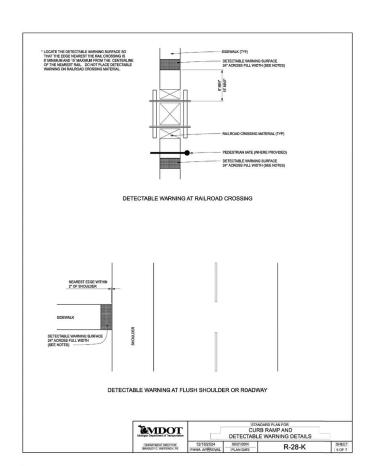


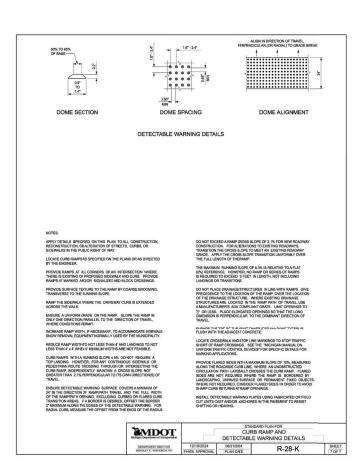










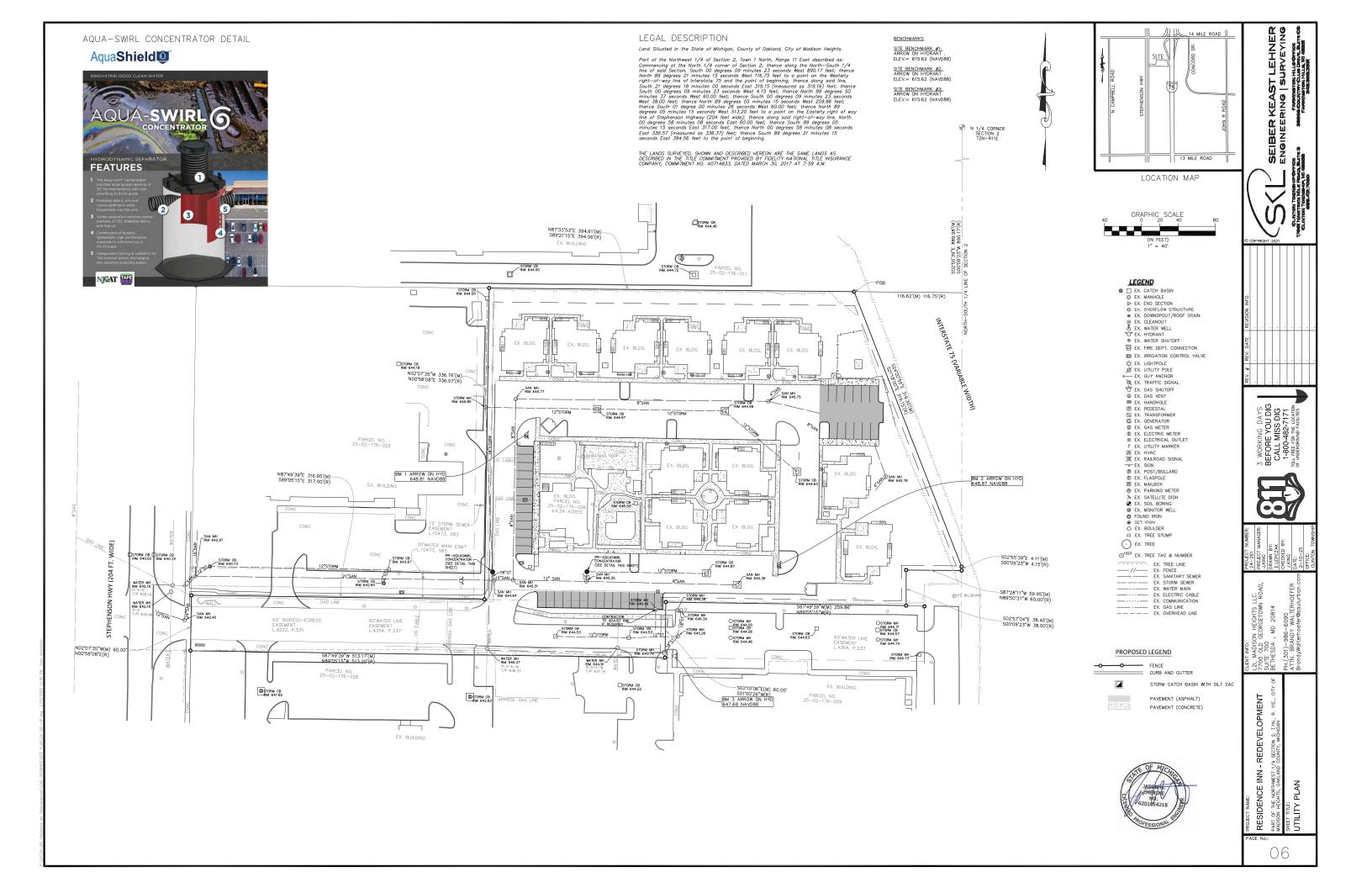






RESIDENCE INN - REDEVELOPMENT

05



L2L MADISON HEIGHTS, LLC 7700 OLD GEORGETOWN ROAD SHITE 700 BETHESDA, MARYLAND 20814

FEBRUARY 6, 2025 McDOWELL & ASSOCIATES

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February 6, 2025

Light Duty Pavement Medium Duty Payement Pavement Layers (Parking)

Hot Mix Asphalt 1.5" 36A or 5EML (Wearing Course) 1.5" 36A or 5EML (Wearing Course)

1.5" 13A or 3EML (Base Course) 2.5" 3C or 3EML (Base Course) 8" MDOT 21AA 8" MDOT 21AA Aggregate Base

Note: Final design should conform to Oakland County Road Commission and City of Madison Heights design standards.

If any existing structures are found, they should be entirely removed from the proposed pavement area. Buried utilities should be removed or grouted in place. Resulting excavations should be backfilled with engineered fill meeting the requirements outlined above.

The stabilization procedures outlined above should result in reasonably stable payements. It should Ine stabilization procedures outlined above should result in reasonably stable pavements. It should be recognized, however, that all asphalt pavements need repairs from time to time as a result of progressive yielding under repeated traffic loads for a prolonged period of time. Placing the new pavement materials over existing non-regineered fill soils may result in long-term differential settlement and some potential cracking of the pavement.

Groundwater was not encountered in any of the borings. It is recommended that pavements be properly graded to promote effective drainage of water and prevent the ponding of surface water in the low areas. As a minimum, a system of stub drains or one drain paced around the catch basin and backfilled with pea stone should be installed at any catch basin or manhole structures to drain and outchfied with pea stories stories to be instanced at any callection of surface water runoff and thus minimize the possibility of frost penetration and heave. The finished pavement surface should be free of depressions and sloped to provide effective surface drainage towards catch basins, where applicable. The drain tile should be wrapped with a geotechnical filter fabric. Edge drains should be installed in watered landscaped areas

Experience indicates that actual subsurface conditions at the site could vary from those found at Experience indicates that actual subsurface conditions at the site could vary from those found at the five test borings performed at specific locations. It is, therefore, essential that a qualified geotechnical engineering testing firm be retained to provide soils engineering services during the site preparation, excavation, earthmoving, and paving phases of the proposed project. This is to observe compliance with the design concepts, specifications, and recommendations and to allow design changes in the event that subsurface conditions differ from those anticipated prior to the start of construction.

### McDowell & Associates

nmental & Hydrogeological Services • Materials Testing & Inspection 21355 Hatcher Avenue, Ferndale, MI 48220 Phone: (248) 399-2066 • Fax: (248) 399-2157

February 6, 2025

L2L Madison Heights, LLC 7700 Old Georgetown Road

Bethesda, Maryland 20814

Job No. 25-034

Attention: Ms. Brandy Walterhoefer

> Soils Investigation Proposed Parking Lot Reconstruction Residence Inn Hotel 32650 Stephenson Highway Madison Heights, Michigan

Dear Ms. Walterhoefer:

In accordance with your request, we have conducted a Soils Investigation at the subject project.

### Project Description

It is understood that the project will consist of a full-depth reconstruction of an existing parking lot at the subject property. It is anticipated that the pavement will support primarily automobile traffic with occasional delivery and sanitation trucks.

### Field Work and Laboratory Testing

Five Soil Test Borings, designated as 1 through 4 and 4A, were performed at the subject property at the approximate locations shown on the Soil Boring Location Plan which accompanies this report. The boring locations were field located by our drillers. Boring 1, 2, 3, and 4A were advanced to a depth of five feet six inches (56") below the existing ground surface at the boring locations. Due to parked cars blocking access to the location of Boring 3 with a drill rig, a hand boring had to be performed. Boring 4 was advanced to a depth of two feet one inch (2'1") before encountering auger refusal on a concrete obstruction. The drillers moved six feet (6') south of Boring 4 and drilled Boring 4A.

Soil descriptions, groundwater observations and the results of field and laboratory tests are to be found on the accompanying Logs of Soil Test Borings.

Borings 1 and 2 encountered four inches (4") and four and one-half inch (4.5") of asphalt pavement underlain by fill and possible fill soils consisting of asphalt millings, silty sand, and/or silty clay to three feet two inches (3'2") and three feet six inches (3'6") followed by very stiff silty clay which was ercountered to boring completions. Borings 3 and 4A encountered three feet six inches (3'6") and five feet two inches (5'2") of fill soils consisting of surficial topsoil and clay-type soils

Mid-Michigan Office 3730 James Savage Road, Midland, MI 48642 Phone: (989) 496-3610 • Fax: (989) 496-3190

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Job No. 25-034 February 6, 2025

If we can be of any further service, please feel free to call.

Very truly yours,

Loran Stenzel-Sebastian Staff Geologist

McDOWELL & ASSOCIATES Loran Stungel - Selastia

David Quintal, P.E.

Page -2-

followed by firm and very stiff silty clay which was encountered throughout the remainder of the borings. Boring 4 encountered one foot (1") of surficial topsoil underlain by silty clay fill soils to two feet one inch  $(2^*1")$  where a concrete obstruction was encountered.

Soil descriptions and depths shown on the boring logs are approximate indications of change from one soil type to another and are not intended to represent an area of exact geologic change or stratification. The transition from one soil type to the next may be gradual rather than abrupt and subsurface conditions may be different from those found by the borings at locations between or beyond the actual boring locations. Also, the site shows some signs of modification which could indicate fill and soil conditions different from those encountered at the boring locations.

No groundwater was encountered in any of the borings. It should be noted that short-term groundwater observations may not provide a reliable indication of the depth of the water table. In groundwater observations may not provide a reliable indication of the depth of the water table. In soils with simificant fines content (clay and/or silt), this is due to the slow rate of infiltration of water into the borehole as well as the potential for water to become trapped in overlying layers of granular soils during periods of heavy rainfall. Water levels in granular soils fluctuate with seasonal and climatic changes as well as the amount of rainfall in the area immediately prior to the measurements. It should be expected that groundwater fluctuations could occur on a seasonal basis and that seams of water-bearing sands or silts could be found within the various clay strata at the

Standard Penetration Tests (SPTs) performed during the sampling operation indicate that the site soils have fair to very good strengths and densities. The tests performed on the fill and possible fill soils at a depth of two feet six inches (2's") resulted in penetration indices ranging from 7 to 24 blows per fost. The five-foot (5') test values varied from 6 to 15 blows per foot. The SPTs for Borings 1, 2, 4, and 4A were performed with a rope and cathead safety hammer. The tests for Boring 3 were performed with a 70-lb donut hammer and the values were divided by two (2) to same safety for the safety hammer. approximate SPT values.

Loss on Ignition Tests were performed on the discolored sand-type materials in Samples 2A and 3B to confirm discoloration of soils was not due to the presence of organic soils. Organic contents on the order of 1.9% and 3.0% were obtained. Organic contents of around 4% or greater are often considered to be unacceptable for the support of floor slabs or pavements.

Based on historic aerials, it appears that the existing parking lot has been in place since sometime between 1985 and 1987. It is unknown how long the existing asphalt has been in place. Considering the assumed amount of time the existing fill soils have been in place, we would anticipate that most of the potential settlement (from pavement and traffic loads) of these soils has already occurred, and future long-term settlement should theoretically be minor. If the possibility of more than normal differential settlement can be tolerated, it is anticipated that the fill soils could be left in place.

If the possibility of more than normal differential settlement cannot be tolerated, then the fill soils should be removed and replaced with the engineered fill. All existing non-engineered fill, organic soils, soft soils, and loose granular soils should be excavated and removed from the proposed roadway area. The excavations should extend beyond the edge of the parking lot at least one foot

Job No. 25-034

(1') for every foot below the proposed grade. The removal of the unsaitable soils should be done in the presence of a qualified soils engineer or technician to limit the potential for uncontrolled fill or highly organic soils being left behind before the placement of engineered fill. After the unsuitable scils have been removed, the excavation should preferably be filled with compacted bank run sand similar to MDOT Class I or II granular soils. If clay material is utilized, it should be placed within 3% of its optimum moisture content. If the bottom of the excavation is not sufficiently stable to install the fill material, then a layer of coarse stone fill such as MDOT 6AA crushed stone could be installed. Geotextile filter fabric should be placed between the coarse stone engineered fill material and any encountered lower native granular soils to minimize the amount engineered fill material and any encountered lower native granular soils to minimize the amount of fines infill-tating into the aggregate material. If granular material is to be placed above the stone, a six-inch (6°) layer of MDOT 21AA or an additional layer of filter fabric should be placed above the stone, overlapping the underlying fabric to further minimize the anount of material infiltrating into the aggregate material. The fill soils should be deposited in horizontal lifts not to exceed nine inches (9°) is thickness with each lift being compacted uniformly to a minimum density of 95% of its maximum value as determined by the Modified Proctor Test (ASTM D-1557).

Job No. 25-034

February 6, 2025

One-inch by three-inch (1" x 3") size crushed stone or crushed concrete could be used in lieu of the MDOT 6AA aggregate and bank run sand that we recommended above. The crushed material would need to be placed and compacted in lifts not exceeding nine irches (9") up to the planned pavement section. The crushed stone or crushed concrete material should not contain significant amounts of trick and should be relatively clean of lime or cement dust which could potentially foul up or clog the edge drain. We suggest that the brick content should be less than 5% and cement/lime dust should be less than 3%. The large, crushed material will need to be separated from any site granular soils by a geotextile filter fabric. We suggest that a filter fabric be placed along the bottom and sides of the engineered fill excavation in an effort to minimize fines from migrating into the voids within the crushed material. It should be noted that crushed concrete should not be used for any project within 500 feet of any watercourse.

In the areas to be paved, the site should be prepared in a manner similar to that recommended In the areas o be paved, the site should be prepared in a manner similar to that recommended above. The subgrade should be reworked until approximately the upper one foot (1') of the subgrade is compacted to at least 95% of its maximum value as determined by the Modified Proctor Test (ASTM D-1557). The aggregate base material should be compacted to 98% of its maximum value as determined by the Modified Proctor Test (ASTM D-1557). The asphalt should be placed in accordance with MDOT specifications as well as applicable local requirements and compacted to a minimum of 92% of the Theoretical Maximum Density (TMD).

It appears that the subgrade will consist of sand and clay-type fill soils. Once compacted, we would expect the clay-type fill soils to have California Bearing Ratios on the order of 3% and a modulus of subgrade reaction of about 100pci. It appears these soils have a high percentage of silt-sized particles which would indicate they would tend to have a severe frost heave potential.

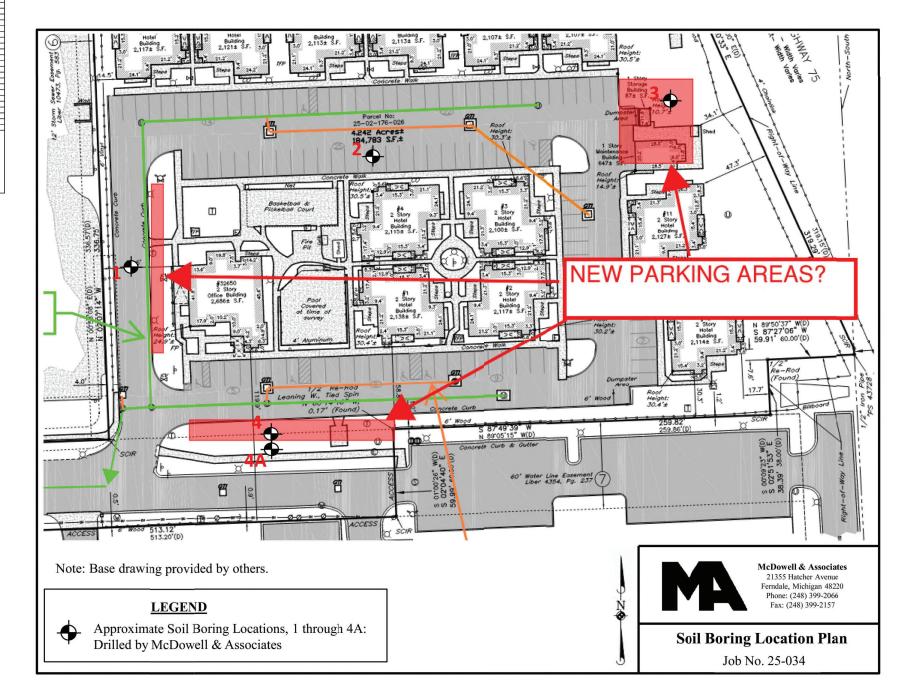
After the subgrade has been properly compacted and any additional fill has been properly placed and/or compacted, the asphalt pavement material could be installed. The following pavement thicknesses are suggested:

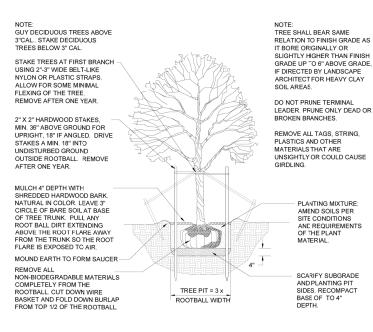
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Type	Depth	Legend			SOIL DESC	CRIPTION		Penetraion Blows for 6"	Moisture %	Natural Wt. P.C.F.	Dry Den Wt.	Unc. Comp. Strength PSF.	Str.
- 1			D'4"	ASPHALT							P.C.F.		-
-H	1_		0'6"	- ASPHALT MIL Moiet brown si	LINGS	ND with occasional	brick fill	_					-
Α	2		15 —			n to discolored bro		9					
UL	3		2'10"	with traces of				9	9.3	104			$\vdash$
		/////	3'2"	Extremely con	npact moist	brown silty fine SA	ND, possible fill						
В	4				t blue silty (	CLAY with traces o	f sand and	6					
UL	5			pebbles				7	12.3	134		(9000+)	
	6	//////	E-6*					8			-	(9000+)	$\vdash$
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TYPE	OF SAM	<b>IPLE</b>	Notes:	1) Used track	ria.				GROUN	D WATER	OBSERV	ATIONS	_
	TURBED					completion with col	d patch asphalt.						
	DIST. LINE								.W. ENCOU			FT.	INS.
	LBY TUB								.W. ENCOU			FT.	INS.
	JT SPOO								AFTER CO			FT.	INS.
	IBRATED		s	PT Hammer: Safe	ety	Crew Chief:	TC	1 '	A. H. AFTER	into			_70.
	ETROME			d Penetration Test	t - Driving 2	O.D. Sampler 1' Wit		9.W	. VOLUMES		N	one	
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									_		_	Dry Den		
Sample & Type	Depth	Legend			SOIL DESC	CRIPTION			Penetration Blows for 6"	Moisture %	Natural Wt. P.C.F.	Wt. P.C.F.	Unc. Comp. Strength PSF.	Str.
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	1		0'5.5	ASPHALT MIL	LINGS									
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UL	3	/////		sandy CLAY w possible fill	ith traces of	of sand, pebbles	s and top	soil,	11	10.4	117			
	3		0108	possible IIII					13					_
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B UL	5	/////		Very stiff moist pebbles	Diue sifty (	JLAY WITH TRACE	es of san	a and	7	13.6	130			
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	NETROME			Penetration Test	- Driving 2	O.D. Sampler 1	' With 140		G.W	VOLUMES		N	one	
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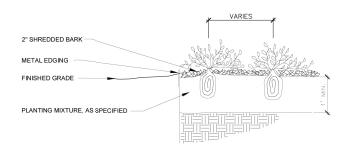
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Sample & Type	Depth	Legend		SOIL DESCRIPTION		Penetration Blows for 6"	Moisture %	Natural Wt. P.C.F.	Dry Den Wt.	Unc. Comp. Strength PSF.	Str.	Sample & Type	Depth	Legend			SOIL DESC	CRIPTION		Penetration Blows for 6"		Natural Wt. P.C.F.		Unc. Comp. Strength PSF.	Str.
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	1		57155	Moist brown silty CLAY with traces of sand and	d pebbles, fill								1		1'0"								$\Box$		
Α			1'4"			3					-	A	2				t brown silty	CLAY with trace	s of sand and	4	-		$\vdash$		⊢
ÚL	2			Firm moist brown silty CLAY with traces of sar	nd and	3	11.4	122			$\vdash$	ÜL			2'1"	pebbles, fill				11/ 1"	18.4	126	$\vdash$		-
	3			pebbles and occasional moist sand lenses, fill		4				(9000+)			3	1									*	(4200)	
-H	4	4////	3'5"	Firm moist dark brown to discored brown silty							$\vdash$	-	4			Auger refusal	at 2"1"			_	_		$\vdash$		⊢
В	-			with traces of sand and topsol and a seam of moist fine sand, fill	discolored	2					Н		-										$\vdash$		-
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TYPE	OF SAN	IDI E	Note:	Hand boring.		_	GROUN	D WATER	OBSEBV	ATIONIO	Щ		PE OF SA	MPLE	Note:		Could trans	and the same and		-	GROUN	D WATER	OBSERV	ATIONS	
D DIS				Used 70 pound hammer. 3low counts are	divided by 2	1	SHOOM	- ANIER	ODOLI (V)				ISTURBED					struction. Moved 6	5' south and		0.10011	_ /********			
J.L UNI		R		to approximate standard renetration value		l a	W. ENCOUR	TERED AT		FT.	INS.		NDIST. LIN		l	drilling Boring	4A.			c	.W. ENCOUR	NTERED AT		FT.	NS.
S.T SHE							W. ENCOUR				INS.		HELBY TU		l						.W. ENCOUR			FT.	NS.
S.S SPL							AFTER CO				INS.		PLIT SPOO		l						W. AFTER CO			FT.	NS.
R.C RO	CK CORE						3.W. AFTER	HRS		FT.	INS.		OCK COR							J 7	G.W. AFTER	HRS		FT.	NS.
· - CAL				SPT Hammer: Safety Crew Chief:		1							ALIBRATE			PT Hammer: Saf		Crew Chief:		4			-		
	ETROME		\$tanda	rd Penetration Test - Driving 2" O.C. Sampler 1" With Falling 30": Count Made at 6" Intervals	140lb Hammer	G.W	. VOLUMES		No	one			ENETROME		Standan	d Penetration Tes	t - Driving 2'	O.D. Sampler 1' W fade at 6" Intervals	ith 140lb Hammer	G.W	V. VOLUMES		No	one	
(POCKET	PENETR	OMETER)		railing 30 : Count Made at 6" Intervals		<u> </u>						(POCK	ET PENETF	ROMETER)		railing	SU . COUNT IV	naue at o Intervals							

Georg 2135	DCWELL & ASSOCIATES tecinical, Environmental, & Hydrogeologic Services 55 latcher Avenue • Ferndale, MI 48220 nc. 2483 1992-1056 • Fax: (248) 1992-157	LOG OF SOIL BORING NO.	4	20 00				
Phot	1e: 246) 399-2006 • Fax: (246) 399-2157		Proposed	Parking I	Lot Recons	struction		
		LOCATION	Residenc	e Inn Hote	el			
JOB NO.	25-034		32650 St	ephenson	Highway			
SURFACE EL	EVATION DATE 1/31/2025		Madison					
SURFACE EL	EVATION DATE 1/31/2023		mauisoii	neignis, i	nichigan			_
Sample & Type Depth Legend	SOIL DESCRIPTION		Penetration Blows for 6"	Moisture %	Natural Wt. P.C.F.	Dry Den Wt. P.C.F.	Unc. (omp. Strengti PSF.	Str. %
1 1 1 1 1 1 1 1 1	Moist dark brown clayey TOPSOIL, fill		_					$\vdash$
1////	1'0" Extremely stiff moist brown silty CLAY with tr	aces of sand						$\vdash$
A 2 /////	1'10" and concrete, fill		8					
UL 3	Extremely stiff moist brown silty sandy CLAY	with trace of	8	12.5	124		(9000+)	$\vdash$
	pebbles and topsoil, fill 3'6"		11			2027	(8000+)	$\vdash$
4 /////								
В /////	Very stiff moist blue silty CLAY with traces of	sand and	5	40.0	407			$\vdash$
UL 5	pebbles		6 7	13.0	137		(90(0+)	$\vdash$
6 /////	D-O*		<b>-</b> '-				(9000-)	-
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TYPE OF SAMPLE	Note:			GROUN	D WATER	OBSERV	ATION\$	$\vdash$
- DISTURBED								
L - UNDIST, LINER				W. ENCOU	NTERED AT		FT.	INS.
T SHELBY TUBE					NTERED AT		FT.	INS.
.s SPLIT SPOON					OMPLETION		FT.	INS.
.C ROCK CORE				S.W. AFTER			FT.	INS.
* - CALIBRATED	SPT Hammer: Safety Crew Chief:	TC	1 `					- 741
PENETROMETER	Standard Penetration Test - Driving 2" O.D. Sampler 1' Wit		G M	. VOLUMES		N	one	
(POCKET PENETROMETER)	Falling 30": Count Made at 6" Intervals		3.11	·······································		14		
(FOOKET PENETROWETER)								

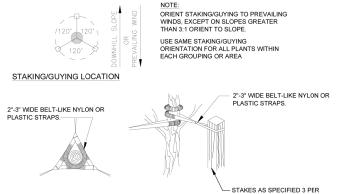




# **DECIDUOUS TREE PLANTING DETAIL**



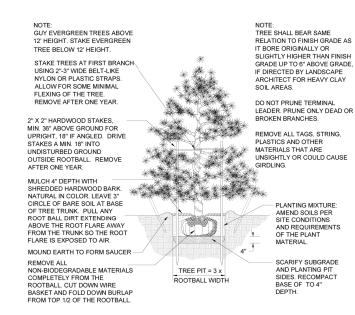
# PERENNIAL PLANTING DETAIL



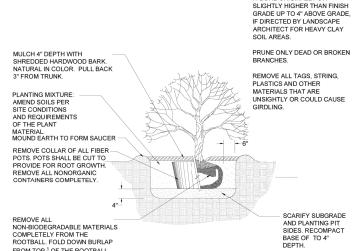
**GUYING DETAIL** 

STAKING DETAIL

# TREE STAKING DETAIL



## **EVERGREEN TREE PLANTING DETAIL**



# SHRUB PLANTING DETAIL

# LANDSCAPE NOTES

FROM TOP 1/3 OF THE ROOTBALL

- All plants shall be north Midwest American region grown, No. 1 grade plant materials, and shall be true to name, free from physical damage and wind burn.
   Plants shall be full, well-branched, and in healthy vigorous growing

- condition.

  Plants shall be watered before and after planting is complete.

  All trees must be staked, fertilized and mulched and shall be guaranteed to exhibit a normal growth cycle for at least two (2) full years following to exhibit a normal growth cycle for at least two (2) full years following Township approval.

  5. All material shall conform to the guidelines established in the most recent edition of the American Standard for Nursery Stock.

  6. Provide clean backfill soil, using material stockpiled on site. Soil shall be screened and free of any debris, foreign material, and stone.

  7. "Agriform" tabs or similar slow-release fertilizer shall be added to the planting pits before being backfilled.

  8. Amended planting mix shall consist of 1/3 screened topsoil, 1/3 sand and 1/3 peat, mixed well and spread to the depth as indicated in planting details.

  8. All indiriums shall be mulched per notating details, sheet

- All plantings shall be mulched per planting details located on this sheet. The Landscape Contractor shall be responsible for all work shown on the
- landscape drawings and specifications.
- landscape drawings and specifications.

  1. No substitutions or changes of location, or plant types shall be made without the approval of the Landscape Architect.

  12. The Landscape Architect shall be notified in writing of any discrepancies between the plans and field conditions prior to installation.

  13. The Landscape Contractor shall be responsible for maintaining all plant material in a vertical condition throughout the guaranteed period.

  14. The Landscape Architect shall have the right, at any stage of the installation, to spice to succeed the material that deep set meat the regular regular of the.

- 14. The Landscape Architect shall have the hight, at any stage of the installation, to reject any work or material that does not meet the requirements of the plans and specifications, if requested by owner.

  15. Contractor shall be responsible for checking plant quantities to ensure quantities on drawings and plant list are the same. In the event of a discrepancy, the quantities on the plans shall prevail.

  16. The Landscape Contractor shall seed and mulch or sod (as indicated on plans)

- The Landscape Contractor shall seed and mulch or sod (as indicated on plans) all areas disturbed during construction, throughout the contract limits.
   A pre-emergent weed control agent, "Preen" or equal, shall be applied uniformly on top of all mulching in all planting beds.
   Sod shall be two year old "Baron/Cheriadelphi" Kentucky Blue Grass grown in a sod nursery on loam soil.
   All Proposed Landscaping to be Provided Water with an Automatic Underground Irrigation System.

TREE SHALL BEAR SAME RELATION TO FINISH GRADE AS IT BORE ORIGINALLY OR SLIGHTLY HIGHER THAN FINISH GRADE UP TO 4" ABOVE GRADE,

Seal:

# Landscape Details

t. 248.467.4668

Project:

32650 Stephenson Hwy. Madison Heights, Michigan

Prepared for:

L2L Madison Heights, LLC 7700 Old Georgetown Road, Suite 700 Bethesda, Maryland 20814

Revision: Issued: February 10, 2025

Job Number:

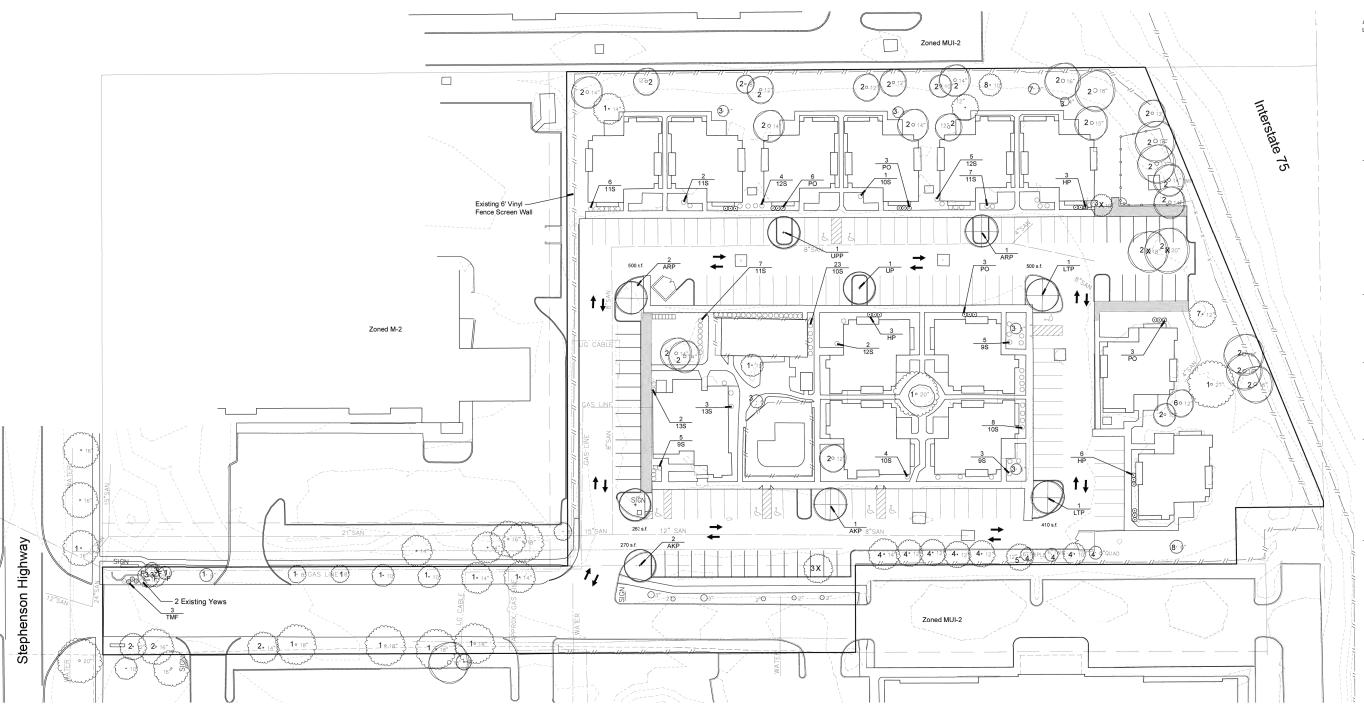
25-008

Drawn By Checked By:

Sheet No.

L-2

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DESIGN

Seal:



Landscape Plan

Project:

32650 Stephenson Hwy. Madison Heights, Michigan

### Prepared for:

L2L Madison Heights, LLC 7700 Old Georgetown Road, Suite 700 Bethesda, Maryland 20814

Revision: Issued: February 10, 2025

Job Number:

25-008

Drawn By: Checked By:





1"=30'

Sheet No.

Landscape Summary

Frontage Landscaping
Site Frontage
"F" Less Drive
Net Frontage
Trees Required
Trees Provided 24' 0.6 Trees (24 / 40) 1 Tree (1 Existing) 0.2 Trees Ornamental Trees Required Ornamental Trees Provided Shrubs Required Shrubs Provided 3 Trees (3 Existing) 4.8 Shrubs (24 / 5) 5 Shrubs (2 Existing)

Parking Lot Landscaping Parking Spaces Shown Interior Landscaping Required Interior Landscaping Provided Tress Required Trees Provided

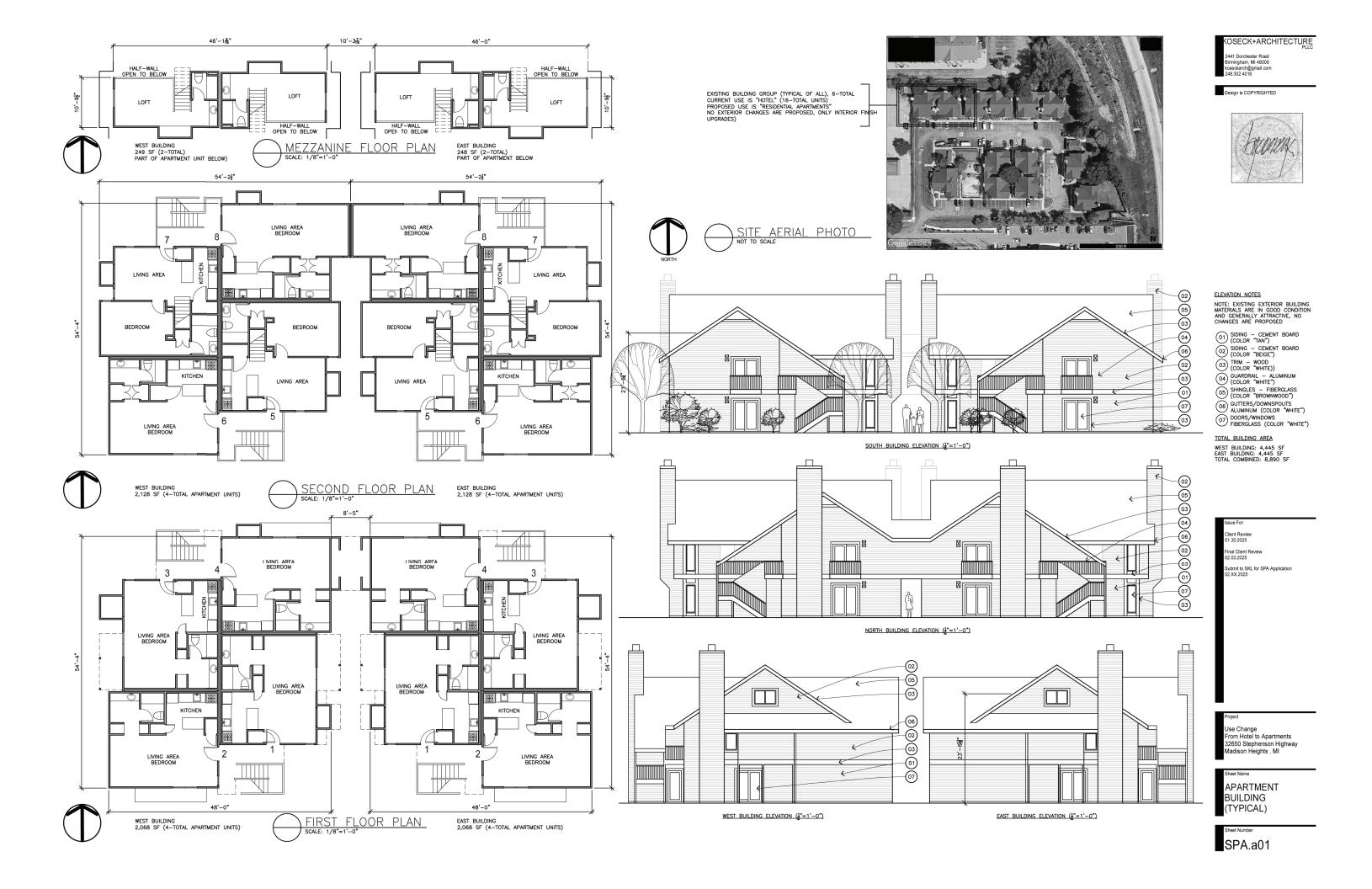
Interior Landscaping Interior Landscaping
Impervious Site Area
Landscape Area Required
Landscape Area Provided
Trees Required
Trees Repoided
Shrubs Required
Shrubs Provided 143 Spaces 715 s.f. (143 x 5) 1,943 s.f. 7.2 Trees (715 / 100) 8 Trees 115,602 s.f. (As Shown on Sheet 4) 5,780 s.f. (115,602 x.5%) 69,062 s.f. (As Snown on Sheet 4) 14.4 Trees (5,780 / 400) 20 Trees (20 Existing) 30,9 Shubs (5,780 / 400) x.2) +2 87 Shrubs (87 Existing) **Existing Vegetation** 

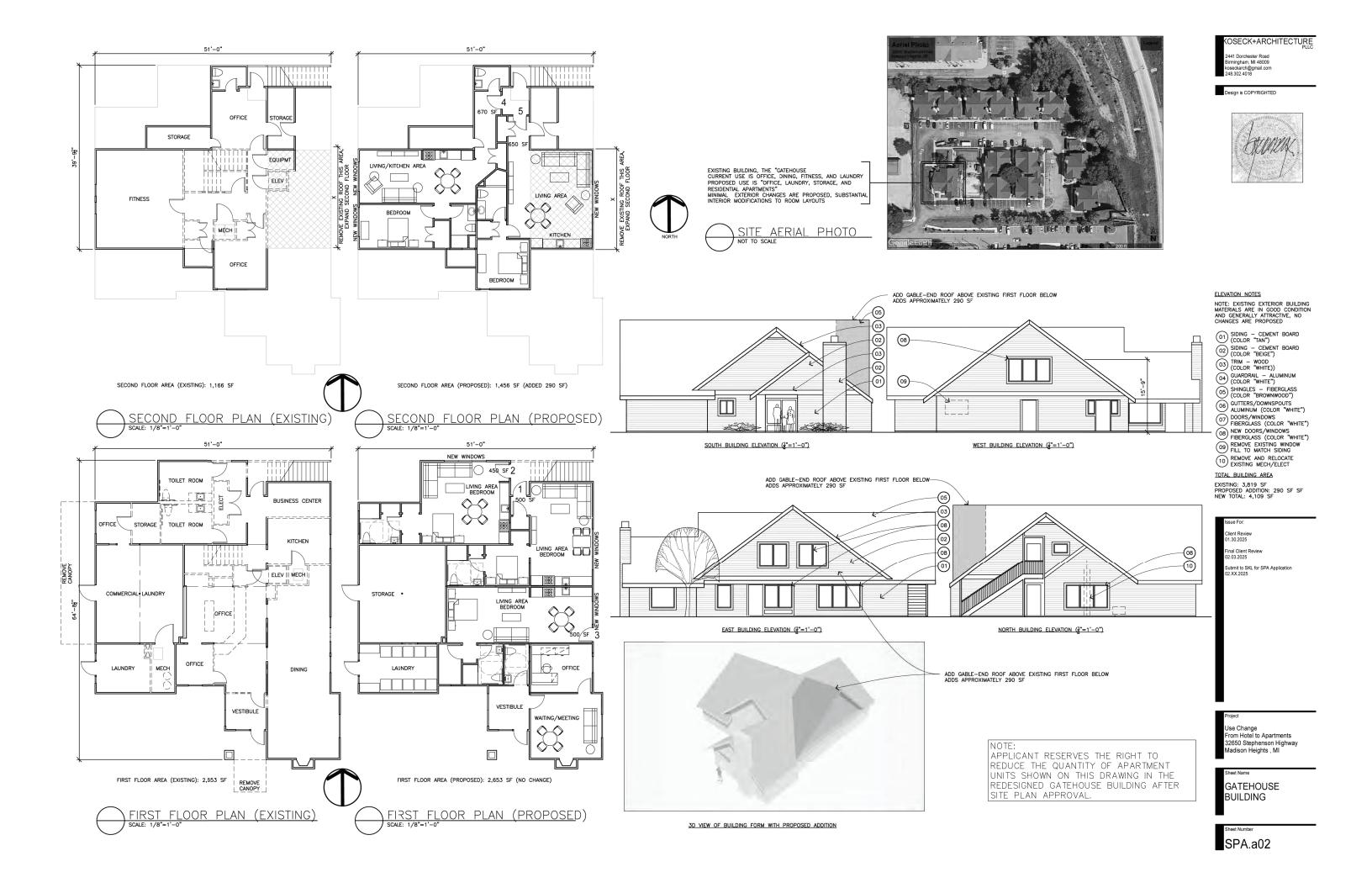
	ng vegetation
Symbol	Species
1	Honey Locust (17 Trees)
2	Austrian Pine (31 Trees)
3	Crabapple (11 Trees)
4	Russian Olive (9 Trees, Exempt from Requirements)
5	Siberian Elm (1 Tree, Exempt from Requirements)
6	Scotch Pine (1 Tree)
7	Sugar Maple (2 Trees)
8	Red Maple (2 Trees)
9S	Rose (13 Shrubs)
10S	Privet (36 Shrubs)
11S	Burning Bush (22 Shrubs)
12S	Forsythia (11 Shrubs)
13S	Arborvitae (5 Shrubs)
"X"	Tree to be Removed

rontag	e Land	scaping					
TMF	3	Taxus x media 'Densiformis'	Dense Yew		as shown	B&B	36"
sym.	qty.	botanical name	common name	caliper	spacing	root	heigh
arking	Lot La	indscaping					
AKP	3	Acer x. freemanii 'Autumn Blaze'	Autumn Blaze Maple	3.0"	as shown	B&B	
ARP	3	Acer rubrum 'Red Point'	Red Point Maple	3.0"	as shown	B&B	
LTP	2	Liriodendron tulipifera	Tulip Tree	3.0"	as shown	B&B	
UPP	1	Ulmus x 'Princeton'	Princeton Elm	3.0"	as shown	B&B	
	9	Trees Provided					
sym.	qty.	botanical name	common name	caliper	spacing	root	heigh
Senera	l Planti	ngs					
UP	1	Ulmus x 'Princeton'	Princeton Elm	3.0"	as shown	B&B	
HP	12	Hydrangea paniculata 'Little Quickfire'	Little Quickfire Hydrangea		as shown	cont	36"
PO	15	Physocarpus opulifolius 'Coppertina'	Coppertina Ninebark		as shown	cont	36"

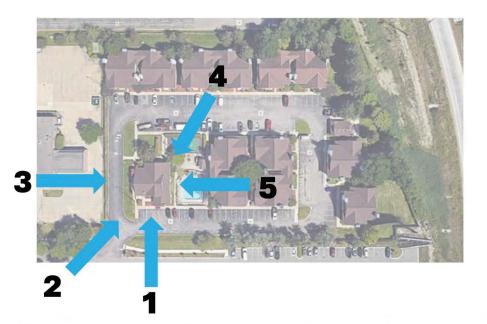
caliper spacing root height

Plant List





# PHOTOS OF THE EXISTING GATEHOUSE













# PROJECT: 32650 STEPHENSON HIGHWAY MADISON HEIGHTS MICHIGAN - TRANSITION FROM HOTEL TO APARTMENTS

# PHOTOS OF THE EXISTING APARTMENTS





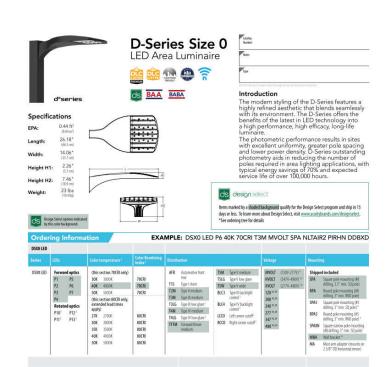






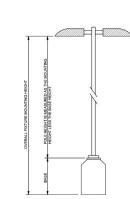








Chris Aina caina@gasserbush.com 734-460-4036



Victoria bin mbi ora, se se o TIP ess. 40

Schedul	e										
Symbol	Label	QTY	Manufacturer	Catalog Number	Description	Lamp	Number Lamps	Lumens per Lamp	LLF	Wattage	Mounting Height
	А	1	Lithonia Lighting	DSX0 LED P4 30K T3M MVOLT	DSX0 LED P4 30K T3M MVOLT	LED	1	9520	0.9	92	20'-0"
	В	2	Lithonia Lighting	DSX0 LED P4 30K TFTM MVOLT	DSX0 LED P4 30K TFTM MVOLT	LED	1	9800	0.9	92	20'-0"
	С	2	Lithonia Lighting	DSX0 LED P4 30K T5M MVOLT HS	DSX0 LED P4 30K T5M MVOLT with houseside shield	LED	1	7339	0.9	92	20'-0"
	D	1	Lithonia Lighting	DSX0 LED P4 30K T5M MVOLT	DSX0 LED P4 30K T5M MVOLT	LED	1	10175	0.9	92	20'-0"
	A-EXISTING	2	Lithonia Lighting	DSX0 LED P4 30K T3M MVOLT	DSX0 LED P4 30K T3M MVOLT	LED	1	9520	0.9	92	20"-0"
	B-EXISTING	6	Lithonia Lighting	DSX0 LED P4 30K TFTM MVOLT	DSX0 LED P4 30K TFTM MVOLT	LED	1	9800	0.9	92	20'-0"
	C-EXISTING	0	Lithonia Lighting	DSX0 LED P4 30K T5M MVOLT HS	DSX0 LED P4 30K T5M MVOLT with houseside shield	LED	1	7339	0.9	92	20'-0"
	D-EXISTING	0	Lithonia Lighting	DSX0 LED P4 30K T5M MVOLT	DSX0 LED P4 30K T5M MVOLT	LED	1	10175	0.9	92	20'-0"

Statistics												
Description	Symbol	Avg	Max	Min	Max/Min	Avg/Min	Avg/Max					
Entrance Drive	+	0.9 fc	3.3 fc	0.0 fc	N/A	N/A	0.3:1					
Parking	+	1.1 fc	2.7 fc	0.0 fc	N/A	N/A	0.4:1					
Western Drive	+	0.7 fc	2.9 fc	0.0 fc	N/A	N/A	0.2:1					
Calc Zone #1	+	0.6 fc	3 3 fc	0.0  fc	N/A	N/A	0.2-1					

### **General Note**

- SEE SCHEDULE FOR LUMINAIRE MOUNTING HEIGHT.
   SEE LUMINAIRE SCHEDULE FOR LIGHT LOSS FACTOR.
   CALCULATIONS ARE SHOWN IN FOOTCANDLES AT: 0' 0"

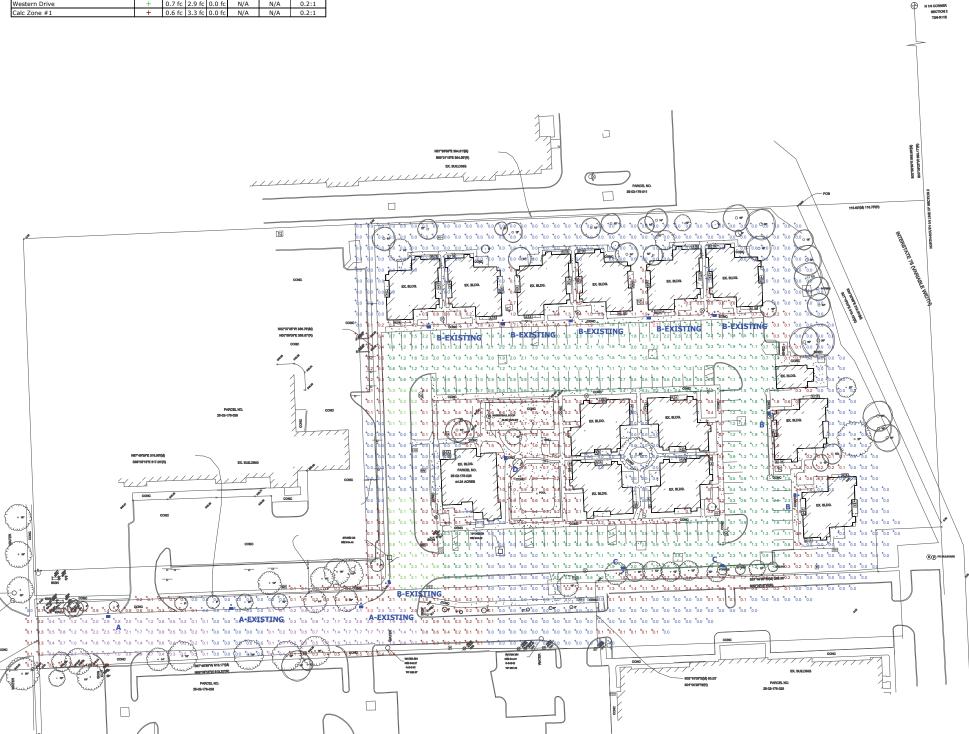
THE ENGINEER AND/OR ARCHITECT MUST DETERMINE APPLICABILITY OF THE LAYOUT TO EXISTING / FUTURE FIELD CONDITIONS. THIS LIGHTING LAYOUT REPRESENTS ILLUMINATION LEVELS CALCULATED FROM LABORATORY DATA TAKEN UNDER CONTROLLED CONDITIONS IN ACCORDANCE WITH ILLUMINATING ENGINEERING SOCIETY APPROVED METHODS. ACTUAL PERFORMANCE OF ANY MANUFACTURER'S LUMINAIRE MAY VARY DUE TO VARIATION IN ELECTRICAL VOLTAGE, TOLERANCE IN LAMPS, AND OTHER VARIABLE FIELD CONDITIONS. MOUNTING HEIGHTS INDICATED ARE FROM GRADE AND/OR FLOOR UP.

THESE LIGHTING CALCULATIONS ARE NOT A SUBSTITUTE FOR INDEPENDENT ENGINEERING ANALYSIS OF LIGHTING SYSTEM SUITABILITY AND SAFETY. THE ENGINEER AND/OR ARCHITECT IS RESPONSIBLE TO REVIEW FOR MICHIGAN

UNLESS EXEMPT, PROJECT MUST COMPLY WITH LIGHTING CONTROLS REQUIRMENTS DEFINED IN ASHRAE 90.1 2013. FOR SPECIFIC INFORMATION CONTACT GBA CONTROLS GROUP AT CONTROLS@GASSERBUSH.COM OR 734-266-6705.

Ordering Note FOR INQUIRIES CONTACT GASSER BUSH AT

Drawing Note
THIS DRAWING WAS GENERATED FROM AN ELECTRONIC IMAGE FOR ESTIMATION PURPOSE ONLY. LAYOUT TO BE VERIFIED IN FIELD BY OTHERS.



Plan View Scale - 1'' = 40ft



Designer DB/KB Date 02/10/2025 Scale Not to Scale Drawing No. QUICK CALC V3 1 of 1