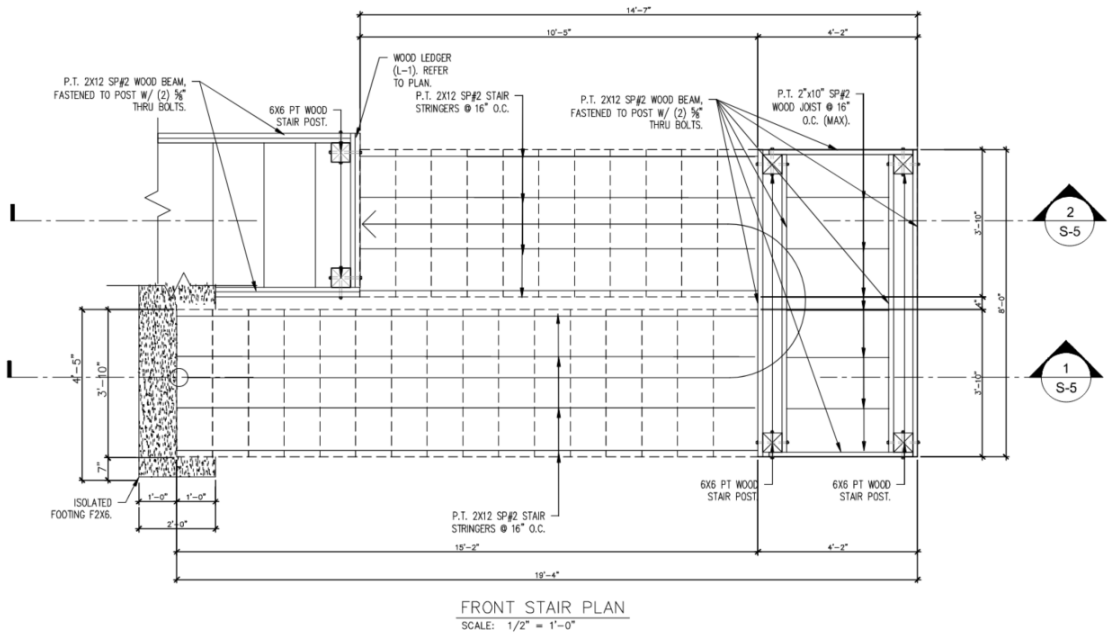




RENDER PROPOSED.
NTS



510 Crystal Drive, Madeira
Beach, FL 33708.

SCOPE OF WORK:
-Lift the existing house

SHEET INDEX	
SHEET NUMBER	SHEET TITLE
A-0	COVER.
N-1	GENERAL NOTES.
C-1	SITE PLAN.
A-1	EXISTING FIRST FLOOR PLAN.
A-2	EXISTING ELEVATIONS.
A-3	NEW FLOOR PLAN.
A-4	NEW ELEVATIONS.
S-1	FOUNDATION PLAN.
S-2	FLOOR FRAMING PLAN.
S-3	ROOF FRAMING
S-4	SECTIONS & DETAILS.
S-5	FRONT STAIR DETAILS.
S-6	REAR STAIR DETAILS.

FLORIDA PRODUCT APPROVAL			
PRODUCT	ITEM	TYPE	APPROVAL NUMBER
WINDOWS	EXTERIOR WINDOWS	STATE	PROVIDED BY CONTRACTOR
DOORS	EXTERIOR DOORS	STATE	PROVIDED BY CONTRACTOR
SIMPSON STRONG-TIE	H1 / H2.5A / SP1 / SP4	STATE	FL10456
SIMPSON STRONG-TIE	LSTA / MSTA	STATE	FL10852 / FL13872
SIMPSON STRONG-TIE	LTS / MTS / HTS	STATE	FL10852 / FL13872
SIMPSON STRONG-TIE	SDWC15600	STATE	FL15895
SIMPSON STRONG-TIE	H10A / H10A-2	STATE	FL11478
SIMPSON STRONG-TIE	HGA10KT	STATE	FL11470 / FL11478
SIMPSON STRONG-TIE	H16 / H16-2 / LGT / MGT	STATE	FL11470
SIMPSON STRONG-TIE	GBC	STATE	FL10861
SIMPSON STRONG-TIE	HH4 / HH6	STATE	FL10446
SIMPSON STRONG-TIE	HGT	STATE	FL10456 / FL10866
J-BOLTS			ASTM F1554
THREADED ROD			ASTM A307 (SAE 1018)
NUTS			ASTM A563
WASHERS			ASTM F463



GOOGLE MAPS ELEVATION.
NTS

MARK		
CD	03/07/25	PERMIT
SHEET MANAGEMENT		
PROJECT NO: 2025035		
SHEET TITLE		
COVER		
SHEET NUMBER		
A-0		



PROJECT
This form has been digitally
signed and sealed by Michael Diaz,
P.E. on the date adjacent to seal.
Printed copies of this document
are not considered signed and
sealed and no signature must be
verified on any electronic copies.

**510 Crystal
Drive,
Madeira
Beach, FL
33708.**

MARK	DATE	DESCRIP.
CD	03/07/25	PERMIT

SHEET MANAGEMENT
PROJECT NO: 2025035

SHEET TITLE

GENERAL NOTES

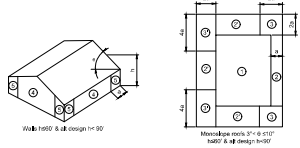
SHEET NUMBER

N-1

STRUCTURAL DATA: MADEIRA BEACH					
SUBSYSTEM: A. BEAMS					
WIND SPEED (HURRICANE)		147.9	mph		
EXPOSURE		II			
MIN. CEILING HEIGHT (FT)		20.0			
INTERNAL PRESS. COEFFICIENT (Cp)		0.2			
Roof					
AREA	10	20	50	100	
Negative Zone 1	-116.7	-116.7	-43.3	-43.3	
Negative Zone 2	-154.2	-154.2	-44.2	-44.2	
Negative Zone 3	-202.4	-172.4	-186.0	-186.0	
Positive All Zones	-13.6	-38.0	-39.6	-35.7	
Overlaping Zone 1 & 2	-166.0	-163.1	-182.1	-182.9	
Overlaping Zone 2 & 3	-203.4	-179.6	-124.0	-149.1	
Walls					
AREA	10	20	50	100	
Negative Zone 1	-68.8	-65.7	-41.0	-41.1	
Negative Zone 2	-84.4	-78.9	-71.4	-65.7	
Positive Zone 1 & 2	61.2	65.3	10.6	11.7	

Notes:
1) If an effective stress between two girders above the load may be interpolated, otherwise use the load associated with the lower effective area.
2) If an effective stress rapidly increases using downward away from the building surfaces, respectively.
3) If an effective stress diagram for corresponding zones.
4) If an effective stress, tension, etc., shall be designed for the full length of design pressure.
5) If an effective stress diagram shall be designed to resist the net uplift design pressure specified.
6) If a value is not specified, modify value in the table by 1.47.

LOCATION OF CAC WIND PRESSURE ZONES - ASCE 7-22



STRUCTURAL SPECIFICATIONS

- GENERAL CONTRACTOR (G.C.) IS RESPONSIBLE TO VERIFY AND COORDINATE ALL DIMENSIONS AND DETAILS BEFORE PROCEEDING WITH WORK. ANY DISCREPANCIES MUST BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ALL ENGINEERS OF RECORD. THE CONTRACTOR IS ALSO SOLELY RESPONSIBLE FOR INTERVIEW, MAINTAINING AND SUPERVISING ALL SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. DMD & COMPANY, LLC IS NOT RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION OR FOR RELATED SAFETY PRECAUTIONS AND PROGRAMS.
- TYPICAL DETAILS AND WALL SECTIONS SHOWN APPLY TO ALL SIMILAR SECTIONS AND CONDITIONS UNLESS NOTED OTHERWISE.
- CONTRACTOR MUST FULLY PROTECT AND PROTECT ALL WORK IN PROGRESS UNTIL THE STRUCTURE IS COMPLETED.
- THE STRUCTURE AND ALL APPLICABLE COMPONENTS FOR THIS PROJECT HAVE BEEN DESIGNED IN ACCORDANCE WITH APPROPRIATE PROVISIONS OF EACH OF THE FOLLOWING:
 - THE 2015 EDITION (2015) FLORIDA BUILDING CODE
 - ACI 308.7M-11B BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE
 - BUILDING CODE REQUIREMENTS FOR WOOD JOINTS (2015) (2015) (2015)
 - ACI 308.7M-11B BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE
 - ACI 308.7M-11B BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE
 - ACI 308.7M-11B BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE
- IF THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO FIELD VERIFY ALL DIMENSIONS AND COORDINATE WITH ARCHITECTURAL AND STRUCTURAL CONSTRUCTION DOCUMENTS AND WITH ANY AND ALL APPLICABLE EQUIPMENT MANUFACTURERS (E.G. WINDOW, DOOR, AIR HANDLER, ETC.) IF THERE ARE ANY CONFLICTS THE GENERAL CONTRACTOR IS REQUIRED TO REQUEST AND RECEIVE AN ARCHITECTURAL DIRECTIVE PRIOR TO SHOWING WORK.
- IF THERE ARE ANY DIMENSIONS NOT SHOWN ON THE STRUCTURAL CONSTRUCTION DOCUMENTS REQUIRED FOR CONSTRUCTION THE GENERAL CONTRACTOR MAY REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND/OR FOR CONSTRUCT THE STRUCTURAL ENGINEER OF RECORD FOR ADDITIONAL INFORMATION.

STRUCTURAL FOUNDATION NOTES

- THE FOUNDATIONS FOR THIS PROJECT HAS BEEN ENGINEERED ASSUMING THE SOIL IS SUITABLE TO SUPPORT 3000 PSF (3000 PSF) FOOTING. G.C./OWNER SHOULD OBTAIN A SITE SPECIFIC SOIL REPORT FOR THIS LOCATION. ANY DISCREPANCY, IN ANY SOIL CONDITION, MUST BE INFORMED TO ENGINEER OF RECORD ALONG WITH THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS. THIS MAY RESULT IN REVISIONS TO THE FOUNDATION PLAN.
- SUBGRADE PREPARATION MUST BE AS NEEDED TO OBTAIN THE SAFE BEARING PRESSURE DEFINED ABOVE. ALL CHANGES AND UNDESIRABLE SOIL MUST BE REMOVED AND A MINIMUM OF 18" COMPACTED FILL MUST BE OBTAIN UNLESS GEOTECHNICAL ENGINEER RECOMMENDATIONS ALLOW A LOWER PERCENT OF COMPACTION.

CONCRETE MASONRY UNITS (CMU)

- THE DESIGN AND CONSTRUCTION MUST CONFORM TO BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES AS 530 / ASCE 5, AND THE SPECIFICATIONS FOR MASONRY STRUCTURES AS 530 / ASCE 6.
- MINIMUM NET COMPRESSIVE STRENGTH OF BLOCK ASSEMBLY MUST BE 1500 P.S.I. (75) MORTAR FOR MASONRY MUST BE TYPE "N" OR "M".
- FOR ALL EXTERIOR AND INTERIOR BEARING, BED JOINTS ARE TO COVER 100% OF THE MASONRY SURFACES AND ALL HEAD JOINTS ARE TO COVER 100% OF THE PROTECTED AREA OF THE FACE SHELLS.
- CONCRETE FILL FOR ALL CMU CELLS, AS REQUIRED, MUST BE WITH 3000 P.S.I. GROUT WITH A SLUMP BETWEEN 8 AND 11 INCHES. G.C. MUST SUBMIT DESIGN MIX TO STRUCTURAL ENGINEER OF RECORD FOR APPROVAL.
- THE MINIMUM HORIZONTAL REINFORCING FOR ALL CMU WALLS MUST BE 9 GAUGE HOT DIP GALVANIZED TRUSS OR LADDER TYPE JOINT REINFORCING AT 16" O.C., PROVIDE MANUFACTURE "Y" AND "L" SHAPES FOR INTERSECTIONS AND CORNERS, (MINIMUM LAP 8").
- PROVIDE ADDITIONAL VERTICAL REINFORCING BAR AT EVERY CORNER, INTERSECTION, CONTROL JOINT, AND OPENING SIZES (E.G.).
- THE MINIMUM SPACING FOR ALL REINFORCING IS TO CONFORM TO THE LATEST ACI CODE ALONG WITH THE DBI SPECIFICATIONS FOR REBAR.
- ALL CMU WALLS HAVE BEEN ENGINEERED TO BE BRACED BY FLOOR/ROOF/BEAM MEMBERS. THEREFORE CONTRACTOR MUST PROVIDE TEMPORARY BRACING DURING CONSTRUCTION.
- ALL CMU BEAM FIRST FLOOR FINISHED ELEVATION MUST BE FULLY ABOUT FULLED.
- ALL INDOOR OUT BLOCK HORIZONTAL BARS MUST HAVE CORNER BARS AT ALL CORNERS AND WALL INTERSECTIONS. SIZE AND NUMBER OF CORNER BARS MUST BE SAME AS HORIZONTAL BARS.
- ALL INTERSECTING WALLS AND CORNER WALLS MUST BE LAD IN AN OVERLAPPING MASONRY BRICKING PATTERN, WITH ALTERNATE UNITS HAVING A BEARING OF NOT LESS THAN 3 INCHES ON EYE BELL.

MISCELLANEOUS

- THE STRUCTURAL SYSTEM IS UNDESIRABLE UNTIL ALL CONNECTIONS HAVE BEEN MADE AND ALL CONCRETE HAS REACHED ITS MINIMUM DESIGN STRENGTH, AS SHOWN IN THE STRUCTURAL DOCUMENTS.
- CONTRACTOR IS RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION TO ENSURE THE SAFETY OF THE BUILDING UNTIL THE SYSTEM IS COMPLETED. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF WHATEVER TEMPORARY BRACING, SHORNS, OR TIE-DOWNS THAT MAY BE NECESSARY. SUCH MATERIAL SHALL BE REMOVED AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.
- CONTRACTOR TO SUPPORT, BRACE, AND SECURE EXISTING STRUCTURE AS REQUIRED. CONTRACTOR IS SOLELY RESPONSIBLE FOR THE SAFETY OF THE BUILDING DURING CONSTRUCTION.
- APPLICABLE BUILDING CODE: 2015 EDITION (2015) FLORIDA BUILDING CODE.
- ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2015 EDITION (2015) FLORIDA BUILDING CODE.
- DESIGN LOADS FOR PRE-ENGINEERED METAL ROOFING: 20 PSF SUPERIMPOSED LIVE LOAD AND 15 PSF DEAD LOAD.
- COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.
- CONTACT ENGINEER WITH ANY QUESTIONS OR DISCREPANCIES FOUND ON DRAWINGS.
- CONTRACTOR TO SUPPORT, BRACE, AND SECURE EXISTING STRUCTURE AS REQUIRED. CONTRACTOR IS SOLELY RESPONSIBLE FOR THE SAFETY OF THE BUILDING DURING CONSTRUCTION.
- CONTRACTOR SHALL NOT BE RELEASED FROM RESPONSIBILITY FOR ERRORS OR OMISSIONS IN SHOP DRAWINGS OR NO DESIGN BY THE ENGINEER'S REVIEW THEREOF.

STRUCTURAL CONCRETE

- ALL STRUCTURAL CONCRETE CONSTRUCTION MUST CONFORM TO THE LATEST ACI "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI-318".
- ALL STRUCTURAL CONCRETE FOR THIS PROJECT MUST HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH AS INDICATED IN THE TABLE BELOW:

CONCRETE	MIX RATIO	AGGREGATE	USE FOR
3000 PSI	0.52	STONE	FOUNDATIONS
4000 PSI	0.48	STONE	SLAB ON GRADE
5000 PSI	0.42	STONE	CONCRETE WALLS
- ALL REINFORCING STEEL (REBAR) MUST BE HOT ROLLED STEEL, INTERMEDIATE GRADE DEFORMED BARS, CONFORMING TO ASTM A-615, GRADE 60. ALL REBAR MUST BE SECURELY SUPPORTED AND TIED IN PLACE PRIOR TO CONCRETE PLACEMENT. USE OF CONCRETE BRICK TO SUPPORT BARS IS NOT ALLOWED.
- IF STRUCTURAL CONSTRUCTION DOCUMENTS CALL FOR REBAR TO BE WELDED ALL WELDING MUST CONFORM TO ASTM A-706.
- ANY WELDED WIRE FABRIC (W.W.F.) DESIGNED IN THE STRUCTURAL CDS MUST BE PROVIDED IN FLAT SHEETS ONLY AND MUST CONFORM TO ASTM A-185.
- ALL BARS WELDED CONTINUOUS MUST BE SPICED AT ALL LAP POINTS AND CORNERS AND DEVELOPED AT NON-CONTINUOUS ENDS AS PER LAP SPICE LENGTHS DEFINED IN ATTACHED TYPICAL DETAILS (UNLESS OTHERWISE NOTED IN PLANS). IF A SPICE IS REQUIRED, G.C. MUST SPICE CONTINUOUS TOP BARS AT MID-SPAN, BETWEEN SUPPORTS AND MUST SPICE CONTINUOUS BOTTOM BARS OVER SUPPORTS.
- THE MINIMUM CLEAR COVER FOR REBAR MUST BE A MINIMUM OF THE VALUE INDICATED BELOW:

SLABS:	(LIGHTWEIGHT & STONE CONG.)
GRIDS & BEAMS: <td>(TO STIRRUPS)</td>	(TO STIRRUPS)
JOISTS: <td>(LIGHTWEIGHT & STONE CONG.)</td>	(LIGHTWEIGHT & STONE CONG.)
COLUMNS AND PIERS: <td></td>	
SURFACES EXPOSED TO EARTH & WEATHER <td>(TO STIRRUPS)</td>	(TO STIRRUPS)
ALL OTHER SURFACES <td>(TO STIRRUPS)</td>	(TO STIRRUPS)

- UNLESS NOTED, TEMPORARY REINFORCING (ASTM A-615) MUST BE 10#10 X CONCRETE AREA.
- PROVIDE #4 @ 12" O.C. WITH STANDARD HOOK, TOP BARS IN ALL SLABS AT DISCONTINUOUS ENDS UNLESS OTHERWISE NOTED ON PLANS. LENGTH OF BARS 1/4 OF SPAN, MINIMUM 3'-0". UNLESS OTHERWISE NOTED PROVIDE #4 @ 12" O.C. IN ALL CANTILEVERS. BAR LENGTH MUST BE CANTILEVER SPAN PLUS 15'-0" PLUS STANDARD HOOK AT CANTILEVER ENDS.
- WHERE PIPE SLEEVES (UP TO 2" IN DIAMETER) PASS THROUGH CONCRETE BEAMS, PROVIDE ADDITIONAL STIRRUP EACH SIDE OF SLEEVES, SLEEVES FOR PIPES 2" IN DIAMETER OR LARGER MUST BE STEEL OR CAST IRON, AND THE LOCATION MUST BE APPROVED BY THE STRUCTURAL ENGINEER.
- ALL CONSTRUCTION JOINTS MUST BE THOROUGHLY CLEANED JUST BEFORE PLACING NEW CONCRETE IN ACCORDANCE WITH THIS PROJECT.
- UNDER NO CIRCUMSTANCES MAY CONCRETE BE PUMPED THROUGH ALUMINUM PIPES. CONCRETE MUST NOT BE PLACED IN CONTACT WITH ALUMINUM, ALUMINUM BRASS, BRASS, TRUCK WHEELS, RUBBERS, CHUTES, CONVEYORS, TRUCK PIPES, AND OTHER EQUIPMENT MADE OF ALUMINUM MUST NOT BE USED ON THIS PROJECT.
- SLUMPS OF OVER 4 INCHES WILL NOT BE PERMITTED UNLESS THE HARRY ARMATURE (SHUTTER PLASTICIZER) IS USED. MAXIMUM SLUMP WITH HARRY IS 8 INCHES - UNLESS OTHERWISE NOTED BY THE STRUCTURAL ENGINEER OF RECORD.
- NO ADJUSTMENT MUST BE USED IN CONCRETE EXCEPT WITH THE PERMISSION OF THE STRUCTURAL ENGINEER OF RECORD AND ONLY AFTER LABORATORY DESIGN MIX APPROVAL. ALL ADJUSTMENTS MUST CONTAIN NO MORE CHLORIDE IONS THAN ARE PRESENT IN MUNICIPAL POTABLE DRINKING WATER.
- ANY WATER REDUCING ADJUSTMENTS MUST CONFORM TO ASTM C-494, TYPE A, AND MUST BE USED IN ALL CONCRETE.
- ANY AIR ENTRAINING ADJUSTMENTS MUST CONFORM TO ASTM C-260. THE AIR CONTENT OF CONCRETE MUST BE USED AS FOLLOWS:

FOR CONCRETE EXPOSED TO SOIL OR WEATHER:	5%.
FOR INTERIOR WALLS, COLUMNS, AND SLABS:	3%.
- ALL FLY ASH USED IN THE CONCRETE MIX DESIGN MUST CONFORM TO ASTM208, TYPE C OR TYPE F AND MUST NOT EXCEED 5% OF THE TOTAL CEMENTitious MAT.
- ANY EXPOSED CONCRETE SLABS MUST RECEIVE A CURING COMPOUND. THE CURING COMPOUND MUST CONFORM TO ASTM C-209 AND MUST HAVE A MINIMUM OF 30% SOLIDS.

STRUCTURAL STEEL

- ALL STRUCTURAL STEEL ELEMENTS SHALL BE FABRICATED AND SPECIFIED IN ACCORDANCE WITH THE LATEST AISC SPECIFICATIONS FOR STEEL CONSTRUCTION.
- STRUCTURAL STEEL ELEMENTS SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS:

	ASTM F1554
ANCHOR BOLTS	ASTM A305 OR A307
FRAMING BOLTS	ASTM A305 OR A307
SHIPS (CLAS)	ASTM A108
SHARP STUDS	ASTM A108
STEEL PIPE (HSS)	ASTM A500 (42 KSI)
STRUCTURAL TUBE (HSS)	ASTM A500 (44 KSI)
WELDING ELECTRODES	E70XX
WIDE FLANGE (W)	ASTM A992 (50 KSI)
- SHOP CONNECTIONS MAY BE WELDED OR FASTENED BY HIGH STRENGTH BOLTS. ALL CONNECTIONS SHALL CONFORM TO STANDARD CONNECTION DETAILS ON THE PLANS, UNLESS EXPLICIT APPROVAL FROM THE ENGINEER HAS BEEN OBTAINED. ALL CONNECTIONS TO BE DOUBLE ANGLE FRAMED BEAM CONNECTION PER AISC, UNLESS NOTED OTHERWISE.
- DESIGN CONNECTIONS FOR THE MAIN BEAM SHEAR (V) IN WPS USED IN THE TABLES FOR "ALLOWABLE UNIFORM LOADS IN WPS FOR BEAMS LATERALLY SUPPORTED", AT THE BOTTOM OF EACH PAGE IN THE "PROPERTIES AND REACTION VALUES", PART TWO OF THE LATEST EDITION OF THE AISC, "MANUAL OF STEEL CONSTRUCTION".
- ALL BOLTS SHALL BE A MINIMUM OF 3/4" IN DIAMETER, AND BE PROVIDED WITH HARDENED WASHERS UNDER THE WELDED ELEMENT (BOLT HEAD OR NUT).
- ALL CONNECTIONS PERFORMED IN THE FIELD SHALL BE MADE WITH HIGH STRENGTH BOLTS, SLIP-CRITICAL (FRICTION) TYPE.
- INSTALLATION AND TIGHTENING OF ALL HIGH STRENGTH BOLTS MUST CONFORM TO "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A305 OR A307 BOLTS".
- ALL WELDING OPERATIONS MUST CONFORM TO THE AMERICAN WELDING SOCIETY CODE, AWS D1.1, AND ALL WELDS SHALL BE PERFORMED USING E70XX, 1/8" WELDS ARE TO BE EQUAL IN STRENGTH TO SHEAR BOLTED CONNECTIONS.
- WELDING STRUCTURAL STEEL MEMBERS TO EMBED PLATES IN CONCRETE MUST BE PERFORMED WITH EXTREME CARE, OVERHEATING THE EMBED PLATE DURING THE WELDING PROCESS WILL CAUSE SEVERE DEGRADATION OF THE PERFORMANCE OF THE CONNECTION. WELD IN SHORT, SINGLE PASSES (NO MORE THAN 1" OF LENGTH FOR 1/4" THICK WELDS) AND ALLOW AMPLIFIED COOLING-OFF TIME BETWEEN EACH WELD. THICKER WELDS SHOULD BE PERFORMED IN MULTIPLE PASSES, WITH A COOLING-OFF PERIOD FOR EACH PASS.
- GAUZZAGE ALL STEEL MEMBERS EXPOSED TO WEATHER (SUCH AS LINTELS, DOWE JAMBS, ETC.).
- SEE SPECIFICATIONS FOR PAINTING OR COATING OF THE CONNECTION. ALL FABRICATION AND ERECTION MARKS SHALL BE COVERED DURING FIELD TOUCH-UP PAINTING.
- CUTS, HOLES, CORNERS, AND ALL OTHER MODIFICATIONS REQUIRED TO BE MADE FOR THE WORK OF OTHER TRADES SHALL BE SHOWN IN THE STRUCTURAL STEEL SHOP DRAWINGS, AND SHALL BE PERFORMED ONLY BY THE SHOP ISSUING SUCH DRAWINGS, OR OTHER FACILITY APPROVED BY THE ENGINEER TO PERFORM SHOP WORK. HOLES IN STRUCTURAL ELEMENTS SHALL BE REINFORCED AS REQUIRED BY THE ENGINEER.
- CUTTING, BURNING OF HOLES, OR OTHER MODIFICATIONS TO STEEL MEMBERS IN THE FIELD IS NOT PERMITTED WITHOUT THE EXPLICIT APPROVAL OF THE ENGINEER.
- STEEL MEMBERS REQUIRED BY OTHER TRADES TO SUPPORT THEIR EQUIPMENT, WHICH ARE NOT ALREADY PRESENT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS, SHALL BE PROVIDED BY THE TRADES REQUIRING SUCH WORK.
- FOR MISCELLANEOUS STEEL ELEMENTS, SEE ARCHITECTURAL DRAWINGS.

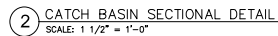
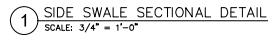
OPEN WEB STEEL JOISTS

- STEEL JOIST CONSTRUCTION SHALL CONFORM TO THE LATEST SPECIFICATIONS OF THE STEEL JOIST INSTITUTE. ALL STEEL JOISTS MUST BE APPROVED BY THE STEEL JOIST INSTITUTE. MAXIMUM LIVE AND TOTAL DEFLECTION OF JOISTS = L/360.
- JOIST MANUFACTURER MUST SUBMIT THE CHALLOUS USED FOR THE MANUFACTURE OF THE JOISTS WITH THE SHOP DRAWINGS, WITH ALL DESIGN SPECIFICATIONS (LOAD TABLES, ETC.) FOR ALL JOIST MEMBERS.
- SHORT-SPAN JOISTS MUST HAVE A MINIMUM BEARING OF 2-1/2" ON STEEL BEAMS. LONG-SPAN JOISTS MUST HAVE A MINIMUM BEARING OF 4" ON STEEL BEAMS. IN CASES WHERE JOISTS BEAR ON STEEL BEAMS ON ONLY ONE SIDE OF THE BEAM, JOISTS SEATS SHALL EXTEND A MINIMUM OF 1" PAST THE CENTERLINE OF THE SUPPORTING BEAM.
- ALL SHORT-SPAN JOISTS, AND DEEP LONG-SPAN JOISTS SHALL HAVE UNIFORM CROSS-SECTIONS, WITH STANDARD DEAD-LOAD CAMBER. ROOF PITCH TO BE ACCOMPLISHED BY SLOPED JOISTS AND SUPPORT BEAMS. ADJUST JOIST SEATING AS SHOWN ON DRAWINGS.
- ALL JOISTS BEARING ON STEEL BEAMS SHALL BE BOLTED OR WELDED TO THOSE BEAMS USING STANDARD CONNECTION PROCEDURES.
- ROAD CONNECTIONS OF BOTTOM CHORDS OF STEEL JOISTS TO COLUMNS SHALL BE PERFORMED SHOULD BE LOOSE DURING ERECTION.
- STEEL JOIST SUPPLIER TO PROVIDE AND INSTALL ALL BRACING WORK, AS SHOWN ON PLAN, BUT NOT LESS THAN AS REQUIRED BY THE STEEL JOIST INSTITUTE OR THE STEEL JOIST DESIGNER. BRACING SHALL CONSIST OF A MINIMUM OF 1-1/4" x 1-1/4" x 1/8" ANGLES, UNLESS NOTED OTHERWISE.
- ALL CUTS AND CONNECTIONS SHALL BE WELDED BY AN APPROVED SHOP.
- NO FIELD WELDING TO BAR JOISTS IS PERMITTED WITHOUT THE EXPLICIT APPROVAL BY THE ENGINEER. JOIST SHALL BE SPICED IN THE FIELD.
- FOR SPECIFIC JOIST ENDS, SEE THE ROOF SECTION DRAWINGS.
- SEE SPECIFICATIONS FOR PAINTING OR COATING OF STEEL JOISTS.
- TRADES ARE NOT ALLOWED TO HAVE EQUIPMENT EXCEEDING 40 POUNDS FROM JOISTS WITHOUT THE EXPLICIT APPROVAL OF THE ENGINEER. LOADS LESS THAN 40 POUNDS MAY BE HUNG AT PANEL POINTS ONLY. ADDITIONAL COSTS IN REINFORCING JOISTS SHALL BE BORNE BY THE TRADES REQUIRING THE WORK.

1. THE PROJECT CONSISTS OF RAISING THE EXISTING CONCRETE-SLAB RESIDENTIAL ON MASONRY PLASTERERS AND STEEL BEAMS, BUILDING TWO WOOD-FRAMED STAIRWAYS FOR MAIN FLOOR, SECONDARY AREAS, AND INCLUDING CICKS AS NECESSARY FOR ACCESSIBILITY.
2. ALL NEW WORK IS TO COMPLY WITH THE FLORIDA RESIDENTIAL BUILDING CODE, 2018, 8TH EDITION AND WITH ALL APPLICABLE CODES AND LOCAL ORDINANCES.
3. INFORMATION ON SITE PLAN, AREA, LENGTH DESCRIPTION HAVE BEEN PROVIDED BY THE OWNER / CONTRACTOR AND ASSUMED TO BE CORRECT.
4. FOUNDATION SHALL BEAR ON FIRM STABLE COMPACTED SOIL. IF POOR SOIL CONDITIONS ARE FOUND, CONTACT THE OWNER BEFORE PROCEEDING.
5. ALL REINFORCING SHALL BE HELD SECURELY IN PLACE WITH STANDARD ACCESSORIES DURING PLACING OF CONCRETE; IF REQUIRED, ADDITIONAL TIES OR STRUTS MAY BE REQUIRED BY THE CONTRACTOR TO FURNISH ADEQUATE SUPPORT.
6. THE CONTRACTOR SHALL COORDINATE THE SIZE, NUMBER & LOCATION OF ALL ANCHOR BOLTS, INSERTS, WELD PLATES AND OTHER ITEMS TO BE EMBEDDED IN THE CONCRETE AS REQUIRED BY ALL TRADES. THE ACTUAL LENGTH OF THE ANCHOR BOLT REQUIRED SHALL TAKE INTO ACCOUNT THE THICKNESS OF THE ATTACHED PART, THE NUT THICKNESS, ETC.
7. THE CONTRACTOR SHALL MAINTAIN ALL SHORING AND BRACING NECESSARY TO MAINTAIN THE STRUCTURE INTEGRITY OF THE WORK AREAS PERMANENTLY AND / OR DURING THE CONSTRUCTION PROCESS AS MAY BE REQUIRED.
8. ALL CONDITIONS SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION, ALL DIMENSIONS AND ELEVATIONS SHALL BE CHECKED BY THE GENERAL CONTRACTOR PRIOR TO CONSTRUCTION, DO NOT SCALE THE DRAWING. INTERFERENCES AND CORRECTIONS SHALL BE MARKED AND BROUGHT TO THE ATTENTIONS OF THE OWNER.
9. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR COMPLYING WITH ALL CITY PROCEDURES, ORDINANCES AND MEANS OF CONSTRUCTION AND ALL REQUIREMENTS BY APPLICABLE FEDERAL, STATE AND LOCAL LAWS, REGULATIONS AND CODES.
10. ALL WOOD EXPOSED TO WEATHER SHALL BE SMOOTH, CLEAR & FREE OF DEFECTS, SPLITTING, ETC.
11. DOOR HARDWARE IS TO BE FIRST GRADE RESIDENTIAL AS SELECTED OR APPROVED BY OWNER.
12. PAINT INTERIOR AND EXTERIOR EXPOSED SURFACES THAT DO NOT RECEIVE PRE-FINISHED MATERIAL, (3 COATS) COLOR SELECTED BY OWNER.
13. ALL CONNECTORS TO BE SIMPSON UNLESS NOTED OTHERWISE, WITH NEC.

THE RESIDENCE AREA SHOULD BE STRIPPED OF TOPSOIL AND ORGANICS, THEN THE STRIPPED BUILDING SUBGRADE SHOULD BE PROOF-ROLLED WITH A HEAVY DRUM-TYPE VIBRATORY COMPACTOR HAVING A MINIMUM STATIC WEIGHT OF 20,000 POUNDS. PROOF-ROLLING OF THE BUILDING AREA TO 7 FEET BEYOND CONSTRUCTION LINES, SHOULD CONSIST OF AT LEAST 10 COMPLETE COVERAGES BY THE COMPACTION EQUIPMENT. COMPACTION SHOULD CONTINUE UNTIL THE SOIL 1 FOOT BELOW THE COMPACTION SURFACE ATTAINS A DENSITY OF AT LEAST 98 PERCENT OF THE MAXIMUM DRY DENSITY AS INDICATED BY THE MODIFIED PROCTOR COMPACTION TEST (ASTM METHOD D 1557)

CONTRACTOR IS TO PROVIDE EROSION CONTROL / SEDIMENTATION BARRIER (HAY BALES OR SILTATION CURTAIN) TO PREVENT SILTATION OF ADJACENT PROPERTY, STREETS, STORM SEWERS AND WATERWAYS. IN ADDITION, CONTRACTOR SHALL PLACE STRAW, MULCH OR OTHER SUITABLE MATERIAL ON GROUND IN AREAS WHERE CONSTRUCTION RELATED TRAFFIC IS TO ENTER AND EXIST SITE. IF, IN THE OPINION OF THE ENGINEER AND / OR LOCAL AUTHORITIES, EXCESSIVE QUANTITIES OF EARTH ARE TRANSPORTED OFF-SITE EITHER BY NATURAL DRAINAGE OR BY VEHICULAR TRAFFIC, THE CONTRACTOR IS TO REMOVE AND CLEAN SAID EARTH TO THE SATISFACTION OF THE ENGINEER / AND / OR AUTHORITIES.

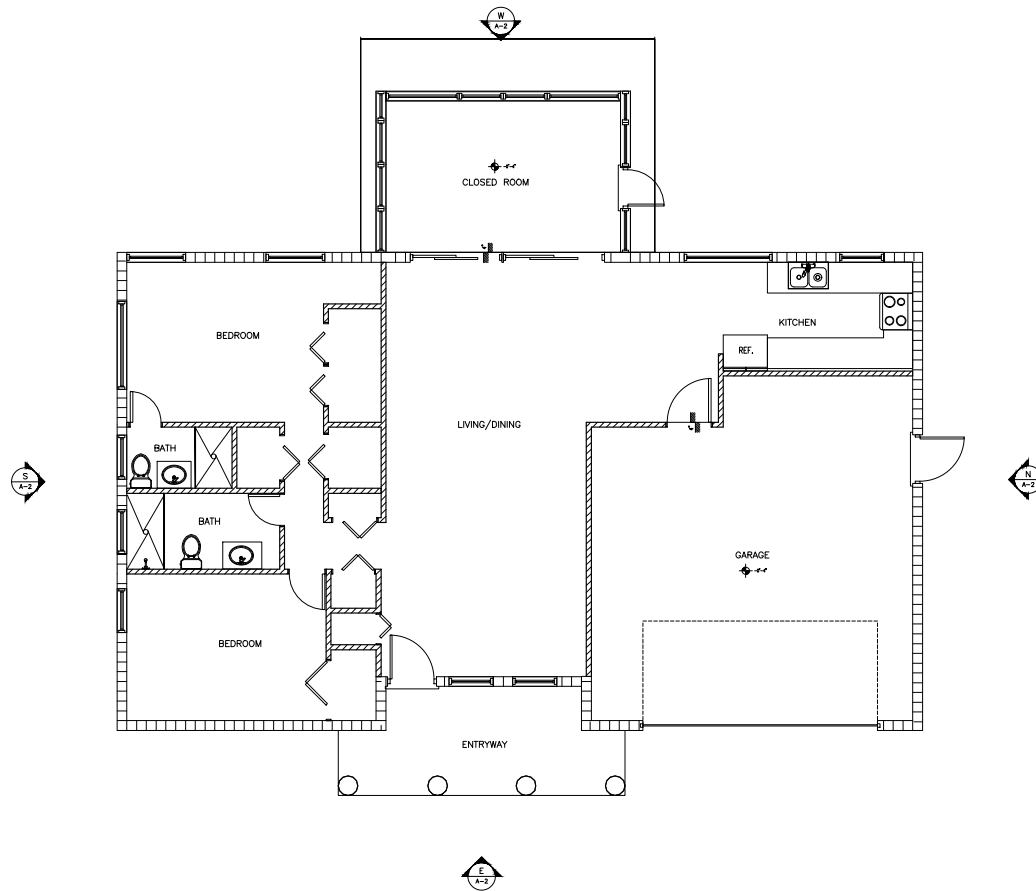


TOTAL IMPERVIOUS = 3217 S.F
% IMPERVIOUS=TOTAL IMPERVIOUS/LOT
% IMPERVIOUS=3217/7473= 52 %
GREEN AREA = 43 %



SITE PLAN
SCALE 1/8"=1'-0"

C-1



EXISTING FIRST FLOOR PLAN
1/4"=1'-0"

LEGEND

- EXISTING EXTERIOR WALL
- EXISTING INTERIOR WALL

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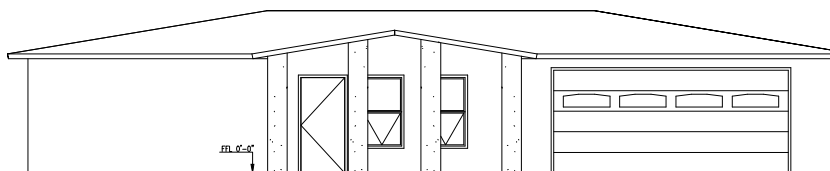
510 Crystal Drive,
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33708.

MARK	DATE	DESCRIP.
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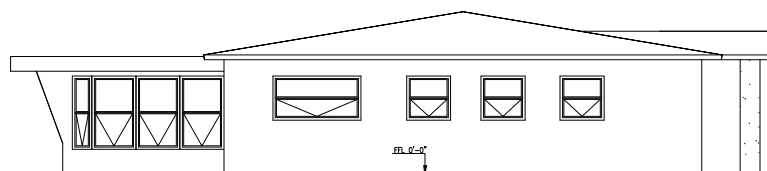
SHEET MANAGEMENT
PROJECT NO: 2025035

SHEET TITLE
EXISTING
FIRST FLOOR PLAN

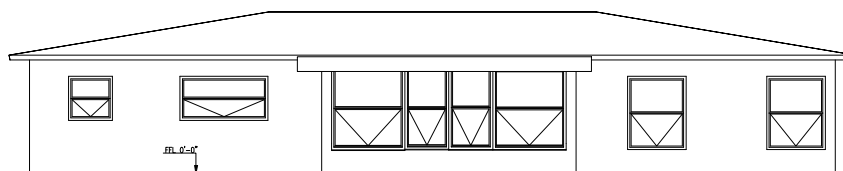
SHEET NUMBER
A-1



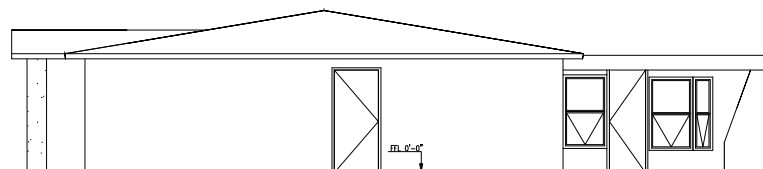
EXISTING EAST ELEVATION
1/4"=1'-0"



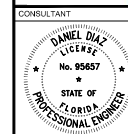
EXISTING SOUTH ELEVATION
1/4"=1'-0"



EXISTING WEST ELEVATION
1/4"=1'-0"



EXISTING NORTH ELEVATION
1/4"=1'-0"



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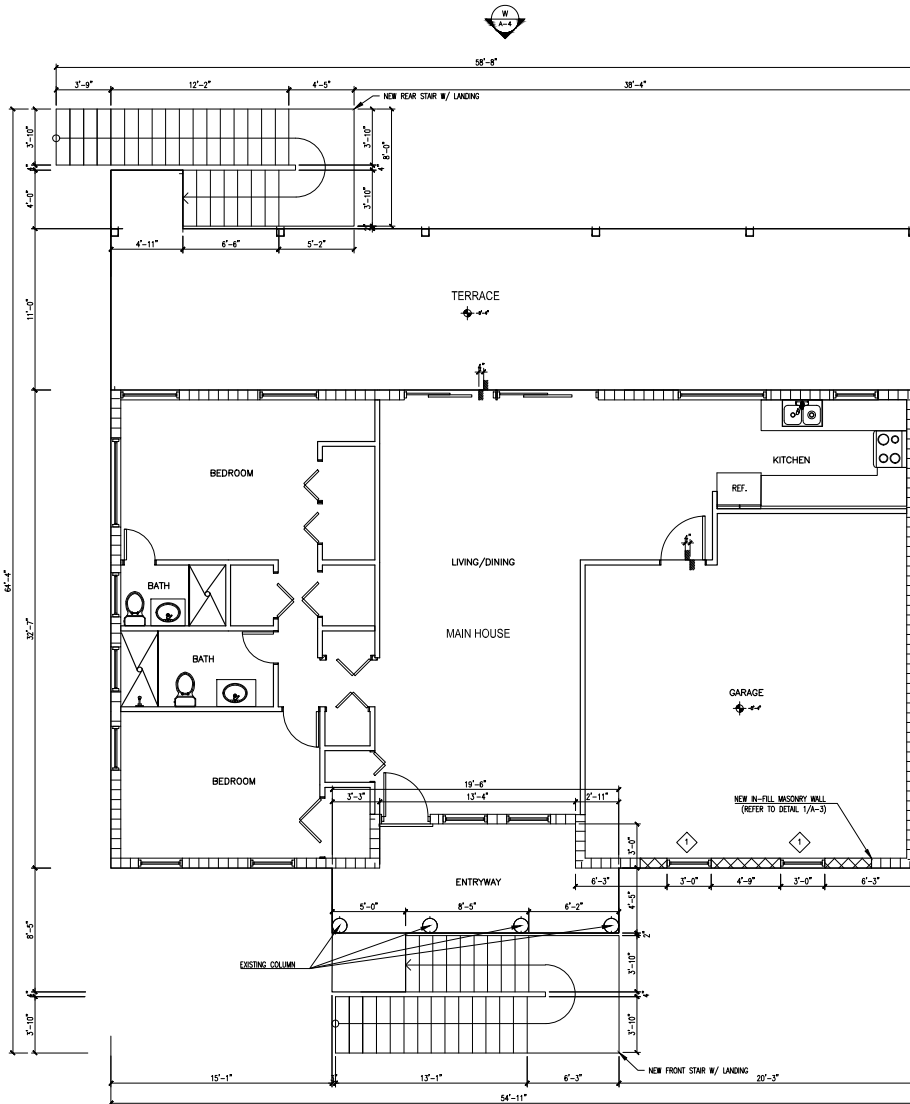
SHEET MANAGEMENT
PROJECT NO: 2025035

SHEET TITLE

EXISTING ELEVATIONS

SHEET NUMBER

A-2



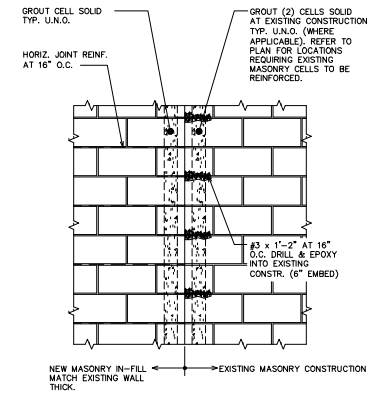
LEGEND

- EXISTING COLUMN TO KEEP
- EXISTING EXTERIOR WALL TO KEEP:
8" CONC. BLOCKWALL
- ▤ NEW EXTERIOR WALL:
8" CONC. (SEE DETAIL O PLAN)

DIMENSION/ELEVATION NOTES:

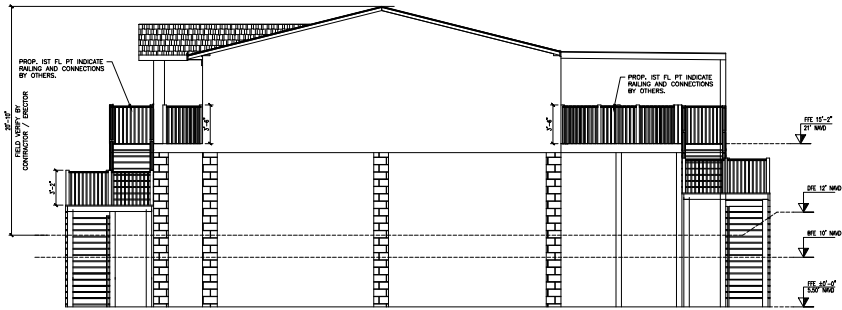
- ALL DIMENSIONS NEED TO BE FIELD VERIFY PER CONTRACTOR/ERECTOR TO COORDINATE ALL DIMENSIONS AND ELEVATIONS NOTIFY ENGINEER OF ANY DISCREPANCIES.

WINDOW SCHEDULE						
ID	TYPE	CODE	WIDTH	HEIGHT	FRAME FINISH	STOOL
1	SINGLE HUNG	25H	36"	60"	VINYL/WHITE	MAR

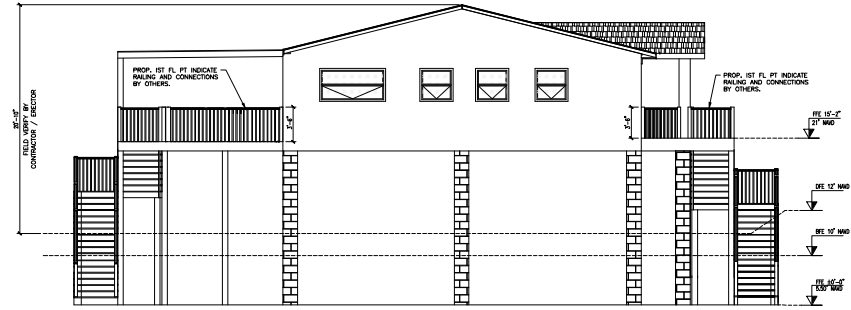


1 TYPICAL MASONRY IN-FILL DETAIL
SCALE: 3/4" = 1'-0"

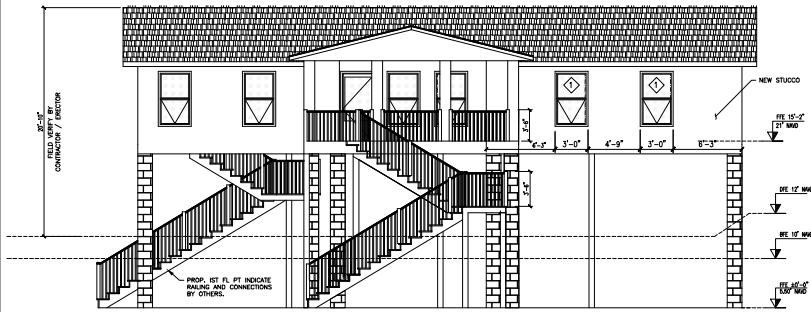
MARK	DATE	DESCRIP.
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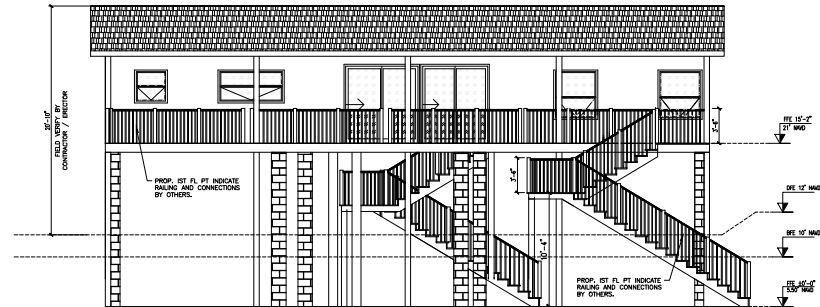
NEW PROPOSED NORTH ELEVATION
3/16"=1'-0"



NEW PROPOSED SOUTH ELEVATION
3/16"=1'-0"



NEW PROPOSED EAST ELEVATION
3/16"=1'-0"



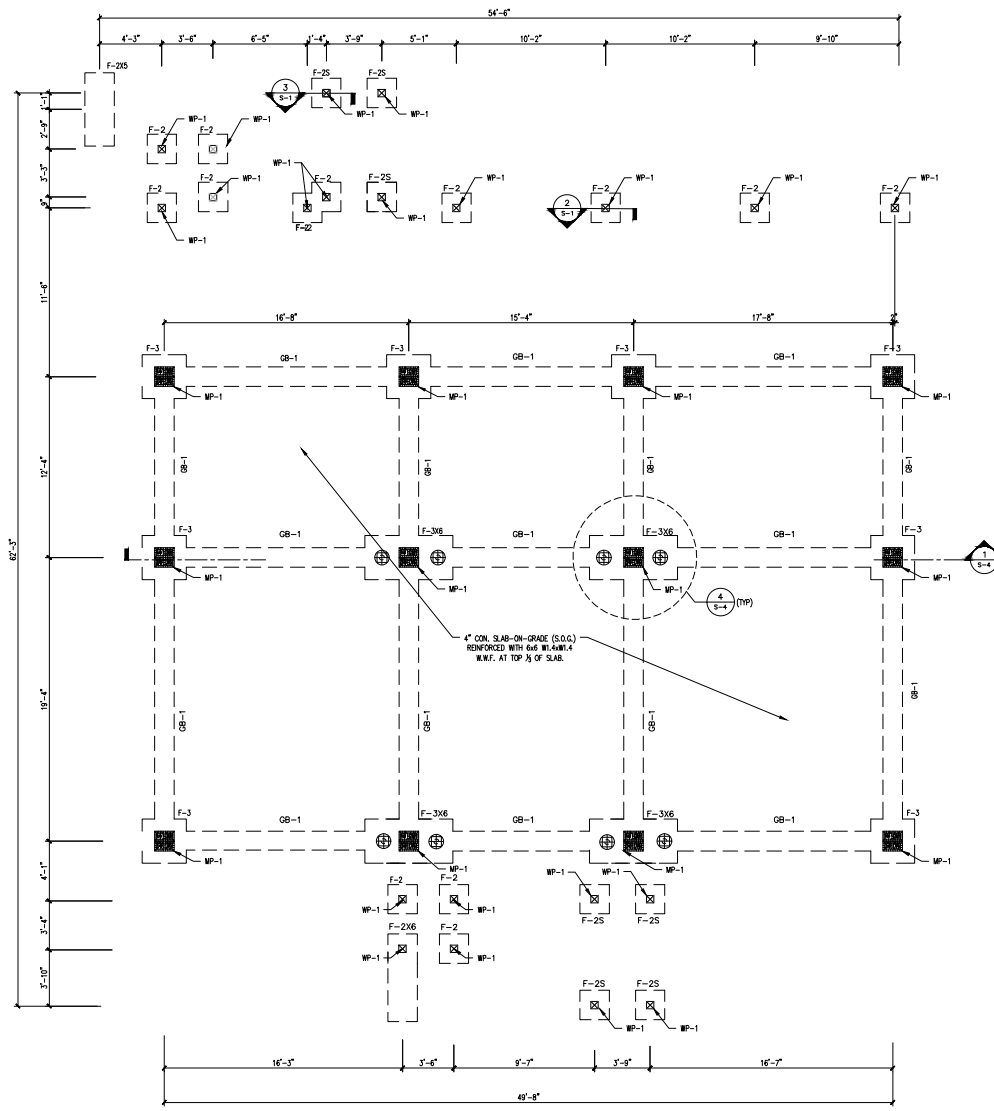
NEW PROPOSED WEST ELEVATION
3/16"=1'-0"

MARK	DATE	DESCRIP.
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SHEET MANAGEMENT
PROJECT NO: 2025035

SHEET TITLE
NEW ELEVATIONS

SHEET NUMBER



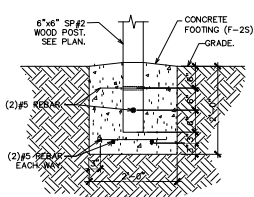
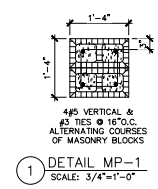
FOUNDATION PLAN
1/4"=1'-0"

LEGEND:

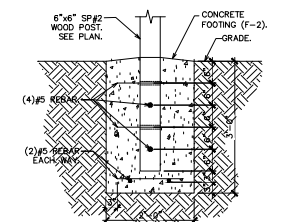
- F-# DENOTES ISOLATED FOOTING. SEE SCHEDULE.
 GB-# DENOTES GRADE BEAM. SEE SCHEDULE.
 WP-1 DENOTES 16"x16" MASONRY PILASTER. SEE DETAIL 1 ON THIS SHEET.
 WP-2 DENOTES 6"x6" SP#2 WOOD POST. SEE DETAIL 2 ON THIS SHEET.
 ⊕ DENOTES HELICAL PILE. FOUNDATION ANCHOR TORQUED TO 5,000 FT-LBS OR 25,000 LBS WORKING LOAD. LTD ATTACHED TO TOP OF HELICAL.

FOOTING SCHEDULE				
MARK	SIZE	DEPTH	REINFORCING EACH WAY	REMARKS
GB-1	1'-4" x CONT.	1'-4"	(2) #5 TOP & BOTTOM #3 STIRRUPS @ 12" O.C.	GRADE BEAM.
F-2	2'-0" x 2'-0"	3'-0"	(2) #5 EACH WAY BOTTOM.	ISOLATED FOOTING. SEE DETAIL 2 ON THIS SHEET.
F-2S	2'-0" x 2'-0"	2'-0"	(2) #5 EACH WAY BOTTOM.	ISOLATED FOOTING. SEE DETAIL 3 ON THIS SHEET.
F-3	3'-0" x 3'-0"	1'-4"	(4) #5 EACH WAY BOTTOM.	ISOLATED FOOTING.
F-2S	2'-0" x 3'-0"	1'-0"	(3) #5 BOTTOM LW (6) #5 BOTTOM SW	ISOLATED FOOTING.
F-2X6	2'-0" x 6'-0"	1'-0"	(3) #5 BOTTOM LW (7) #5 BOTTOM SW	ISOLATED FOOTING.
F-3X6	3'-0" x 6'-0"	1'-4"	(4) #5 BOTTOM LW (7) #5 BOTTOM SW	ISOLATED FOOTING.

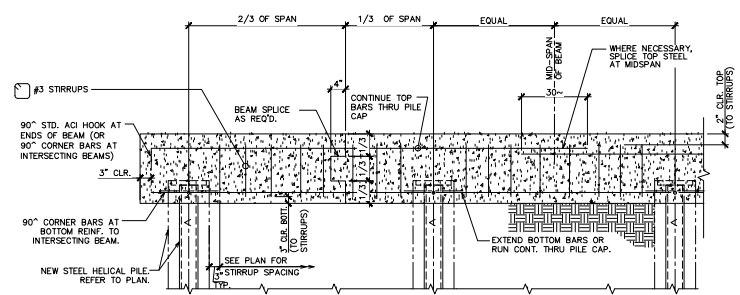
NOTES:
 1. CENTER ALL FOOTINGS BELOW WALL OR COLUMN U.N.O.
 2. ALL FOOTING REINFORCING TO BE BOTTOM BARS U.N.O.
 3. SEE TOP OF FOOTING AT SECTIONS & DETAILS.
 4. INCREASE DEPTH OF THICKENED SLAB EDGE AS REQUIRED TO MAINTAIN 1'-0" (MIN.) EMBEDMENT BELOW GRADE (TYP.) U.N.O.



3 SHORT WOOD POST FOOTING F-2S
SCALE: 3/4"=1'-0"



2 LONG WOOD POST FOOTING F-2
SCALE: 3/4"=1'-0"



- NOTES:
 1. HELICAL PILES NOT TO SCALE.

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STRUCTURAL ENGINEERS
2403 Gulf Breeze Blvd., Ste. C
Cocoa Beach, FL 32905
727.697.0773
dmd-company.com

CONSULTANT

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SHEET MANAGEMENT

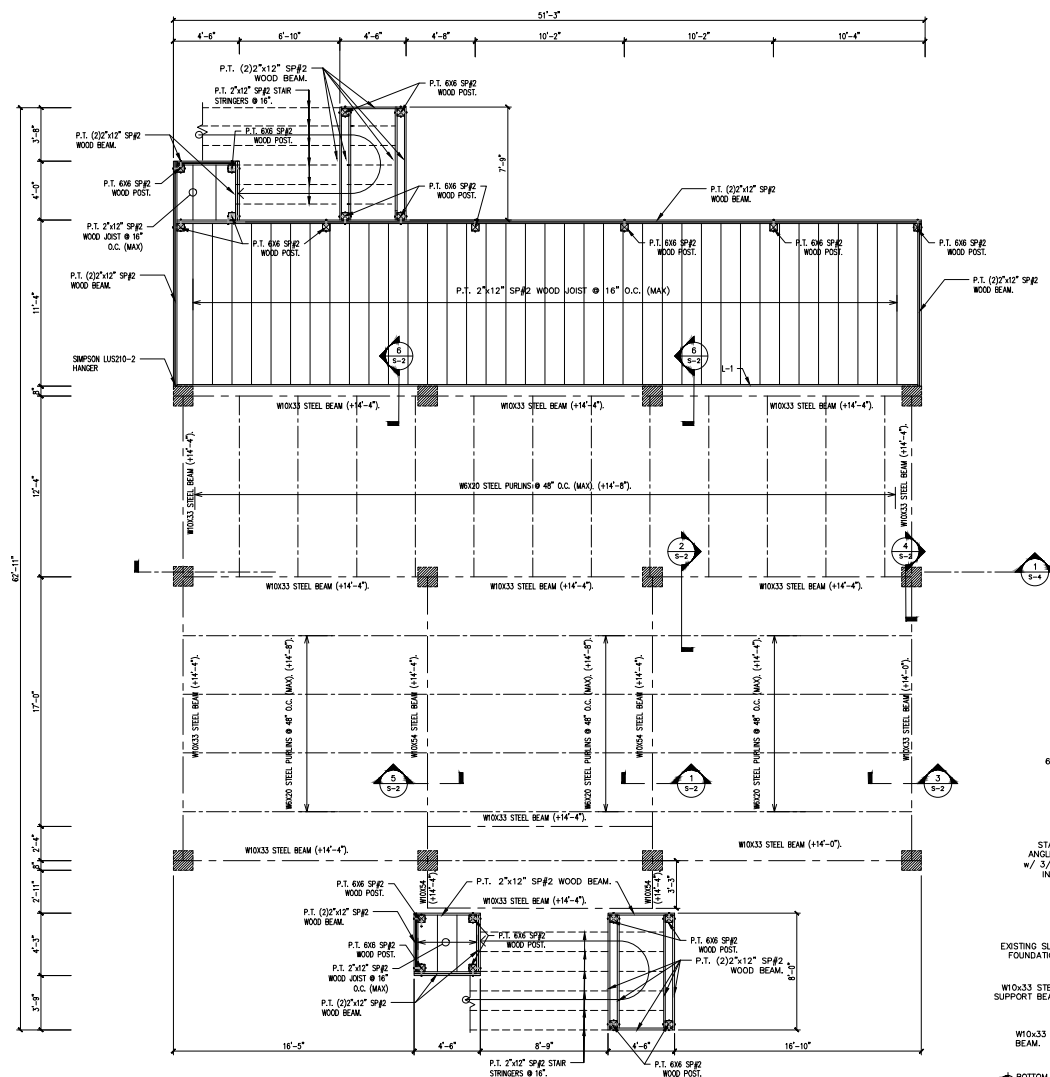
PROJECT NO: 2025035

SHEET TITLE

FOUNDATION PLAN

SHEET NUMBER

S-1



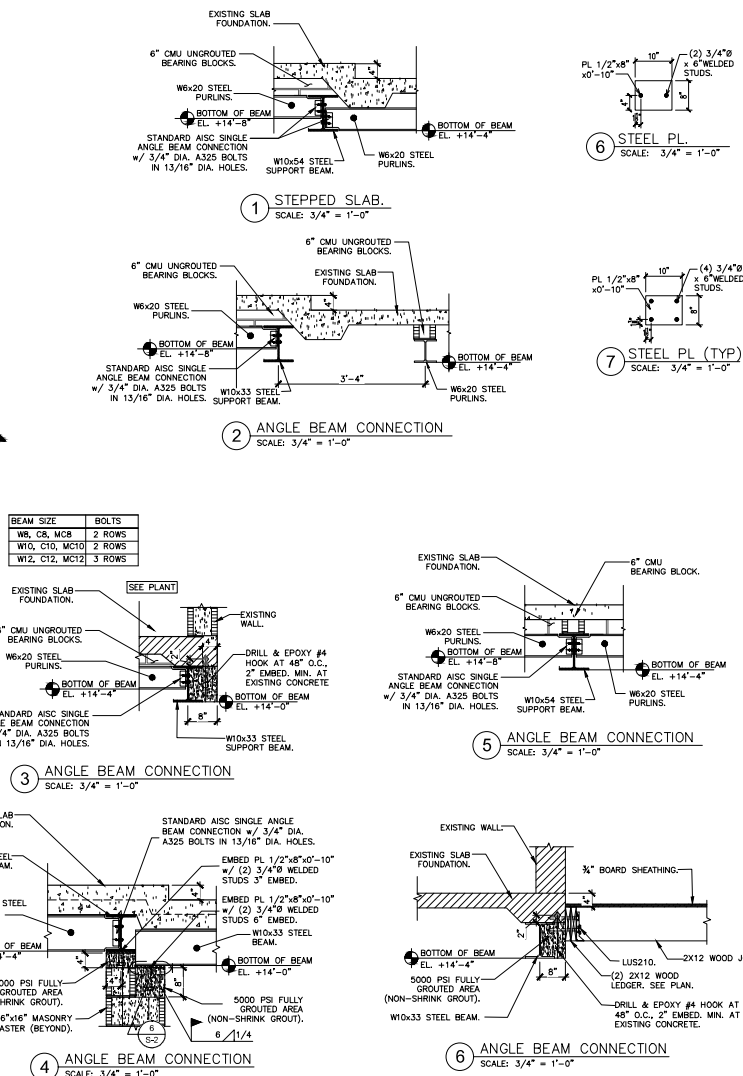
FLOOR FRAMING PLAN
1/4" = 1'-0"

LEGEND:

- DENOTES COLUMN SUPPORT BELLOW.
- L-1 DENOTES P.T. (2) 2X12 WOOD LEDGER w/ 3/4" x SIMPSON STEEL HD. 6-32 EMBED, AT 8" MAX. FROM EA. END AND 16" O.C. MAX., STAGG. TOP AND BOTTOM.
- (+/-) DENOTES BOTTOM OF STEEL BEAM.

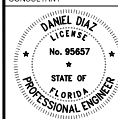
NOTE:
ALL STRUCTURAL STEEL MEMBERS AND CONNECTORS TO BE HOT DIPPED GALVANIZED.

NOTE: PROVIDE HANGER CONNECTOR BY SIMPSON LUS210 AT SIMPLE WOOD JOIST; & HANGER CONNECTOR BY SIMPSON LUS210-2 AT DOUBLE WOOD JOIST. SEE SECTIONS FOR CONNECTOR OF WOOD BEAM TO WOOD POST.



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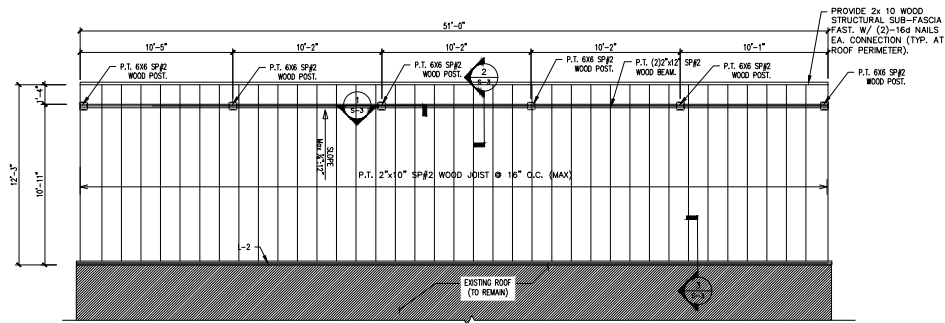
SHEET MANAGEMENT
PROJECT NO: 2025035

SHEET TITLE

FLOOR FRAMING PLAN

SHEET NUMBER

S-2



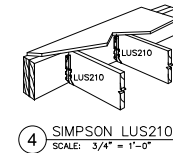
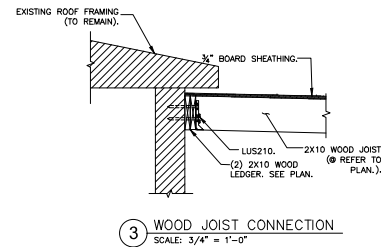
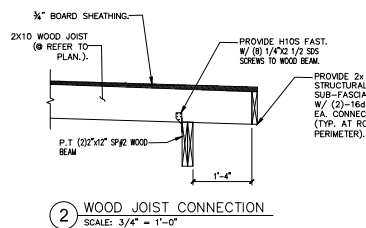
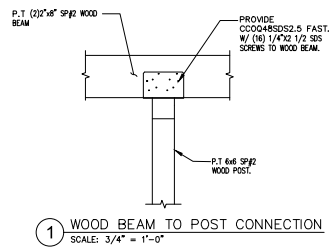
ROOF FRAMING PLAN
1/4"=1'-0"

LEGEND:

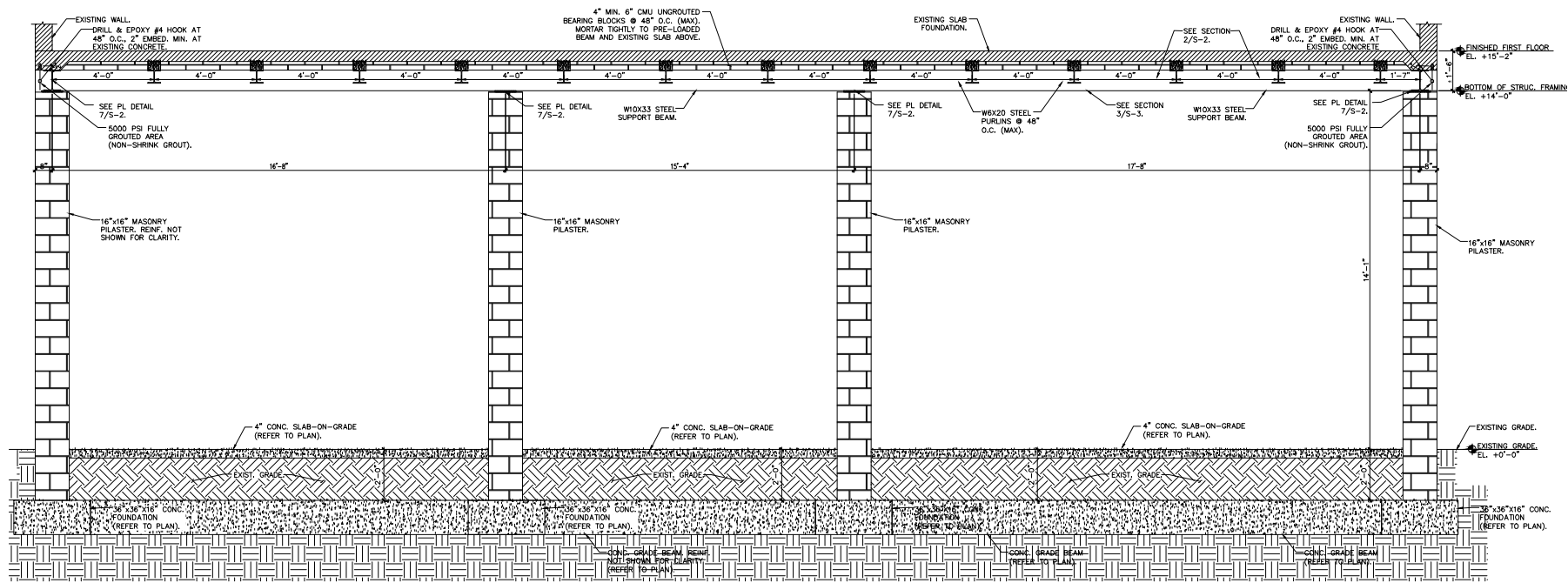
L-2-2 DENOTES P.T. (2) 2X10 WOOD LEDGER W/ 3/4" & SIMPSON TITEN HD, 6-1/2" EMBED, AT 8" MAX. FROM E.A. END AND 18" O.C. MAX., STAGG. TOP AND BOTTOM.

(+/-#-#) DENOTES TOP OF STEEL BEAM.

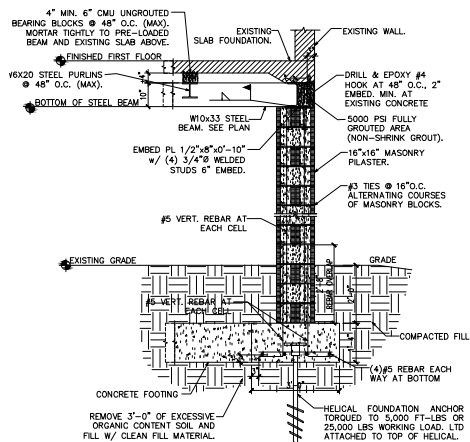
NOTE: PROVIDE HANGER CONNECTOR BY SIMPSON LUS210 AT SIMPLE WOOD JOIST; & HANGER CONNECTOR BY SIMPSON LUS210-2 AT DOUBLE WOOD JOIST. SEE SECTIONS FOR CONNECTOR OF WOOD BEAM TO WOOD POST.



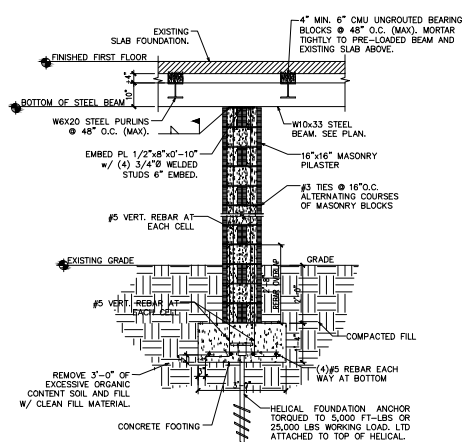
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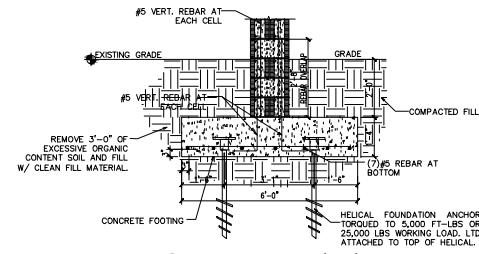
1 MAIN SECTION
SCALE: 1/2" = 1'-0"



2 EXTERIOR PILASTER (TYP.)
SCALE: 1/2" = 1'-0"



3 INTERIOR PILASTER (TYP.)
SCALE: 1/2" = 1'-0"



4 INTERIOR PILASTER (TYP.)
SCALE: 1/2" = 1'-0"

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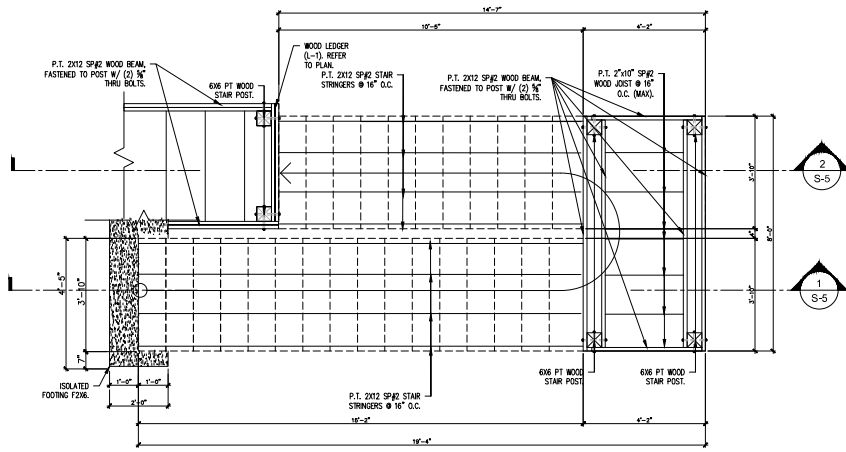
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SHEET MANAGEMENT
PROJECT NO: 2025035

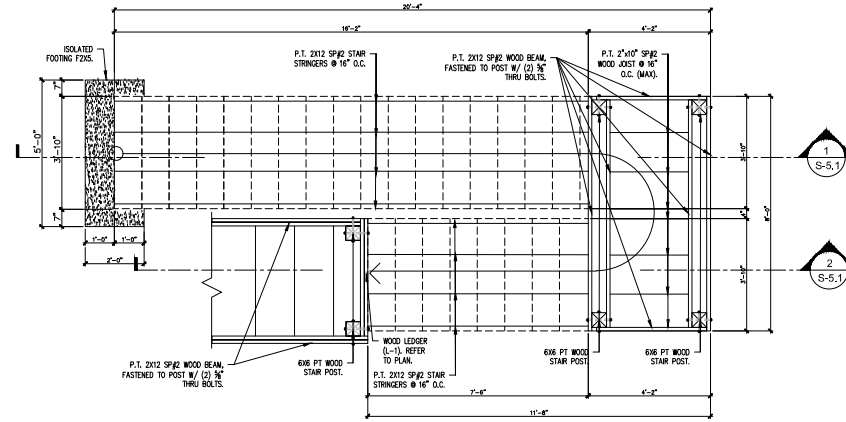
SHEET TITLE
SECTIONS & DETAILS

SHEET NUMBER

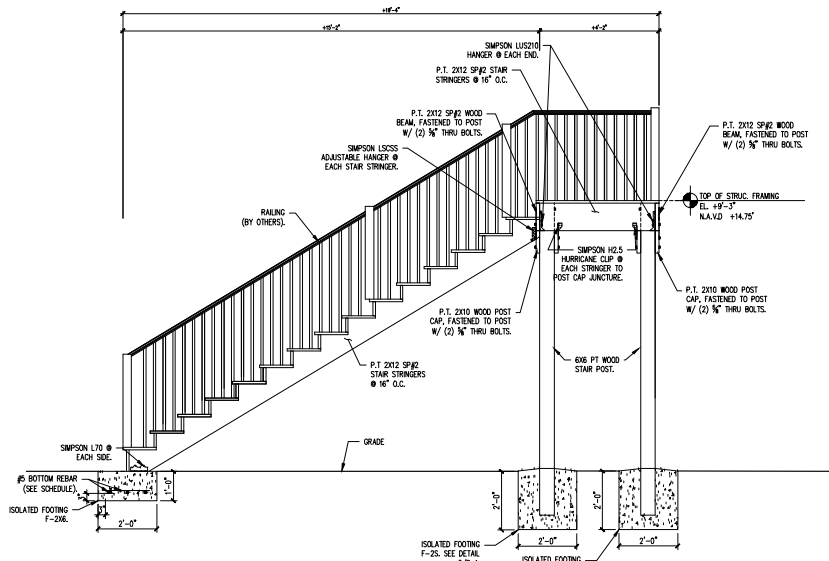
S-4



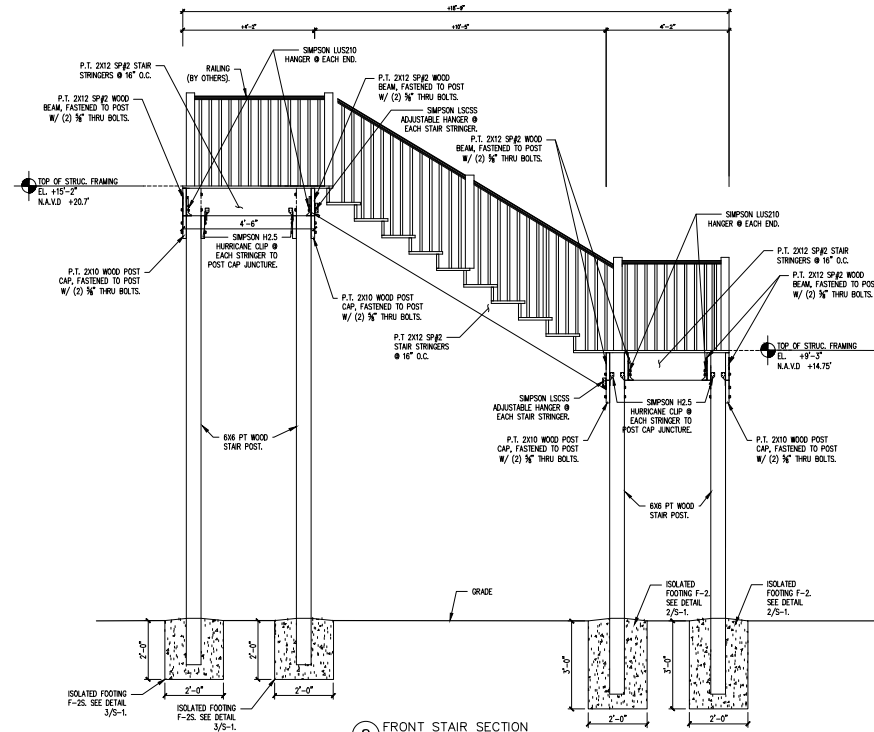
FRONT STAIR PLAN
SCALE: 1/2" = 1'-0"



REAR STAIR PLAN
SCALE: 1/2" = 1'-0"



1 FRONT STAIR SECTION
SCALE: 1/2" = 1'-0"



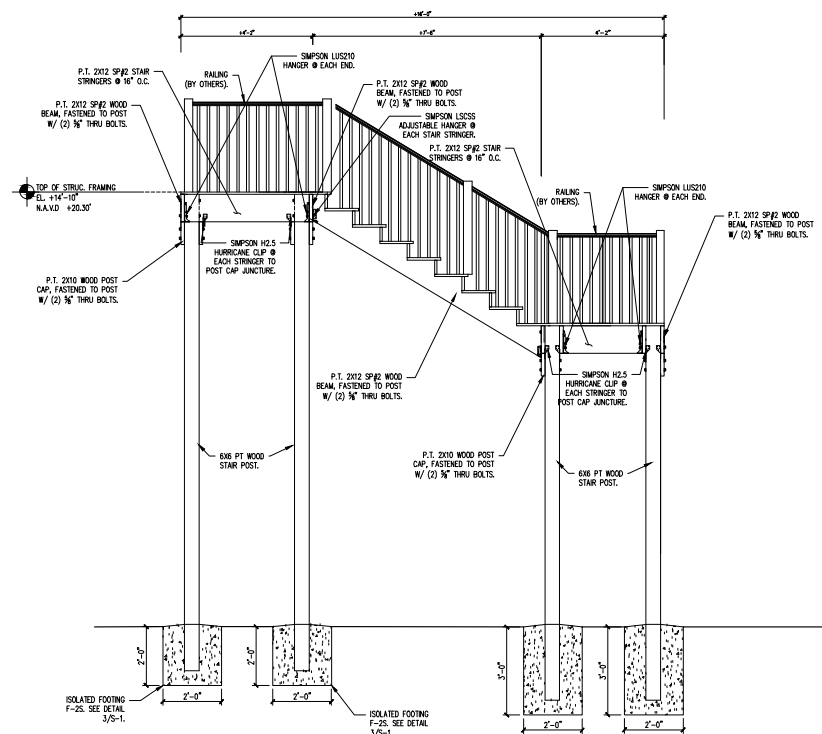
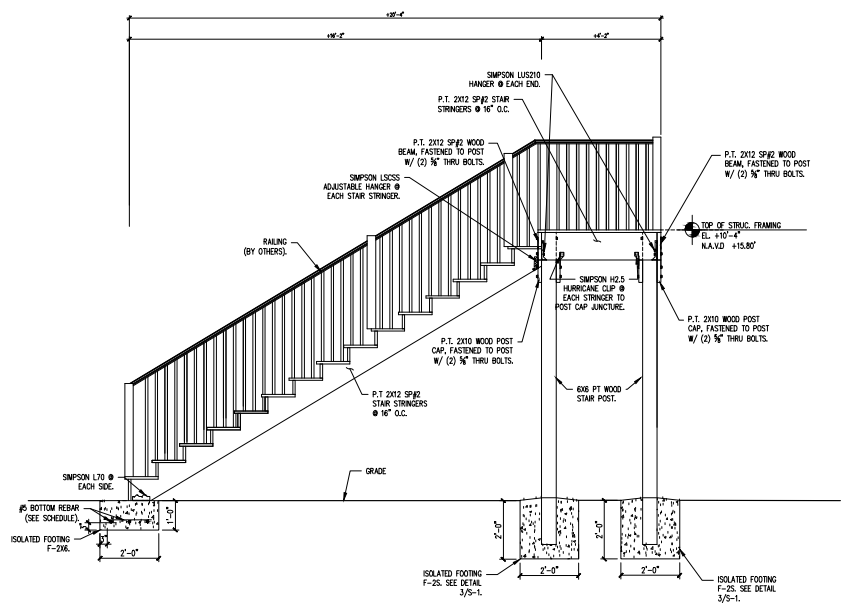
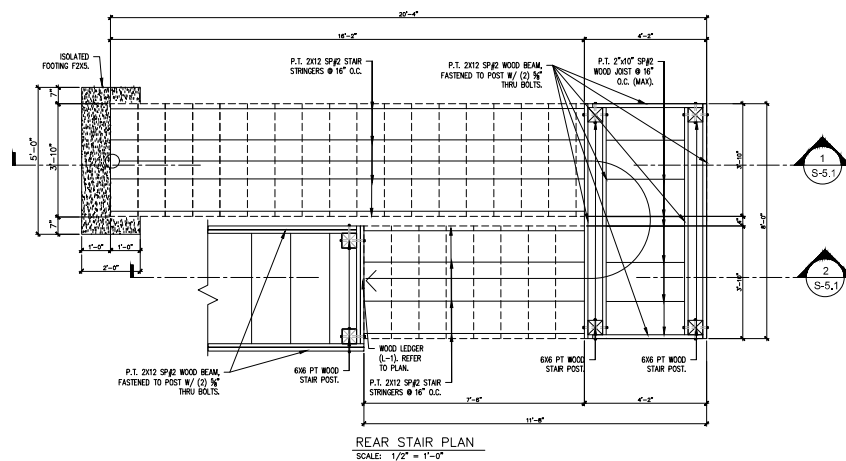
2 FRONT STAIR SECTION
SCALE: 1/2" = 1'-0"

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SHEET MANAGEMENT
PROJECT NO: 2025035

SHEET TITLE
STAIR DETAILS

SHEET NUMBER
S-5



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