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Intersection Control Evaluation (ICE) Report

Susan Drive and Margaret Avenue

Marshall, Lyon County, MN

February 18, 2025

Submitted by:

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Certification

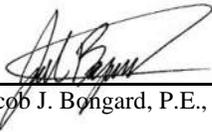
Intersection Control Evaluation Report

Susan Drive and Margaret Avenue

in

City of Marshall
Lyon County, Minnesota

I hereby certify that this report was prepared by me or under my direct supervision and that I am a duly Registered Professional Engineer under the laws of the State of Minnesota.



Jacob J. Bongard, P.E., PTOE

52210
Reg. No.

2/18/2025
Date

Approved:

Marshall City Engineer

Date

Introduction

An analysis of the intersection control alternatives at Susan Drive and Margaret Avenue was completed to evaluate and identify necessary improvements to vehicle and pedestrian infrastructure at the intersections due to safety concerns and the close proximity of the two intersections. The intersection is located in Marshall, Lyon County, Minnesota. See **Figure 1** for a project location map.

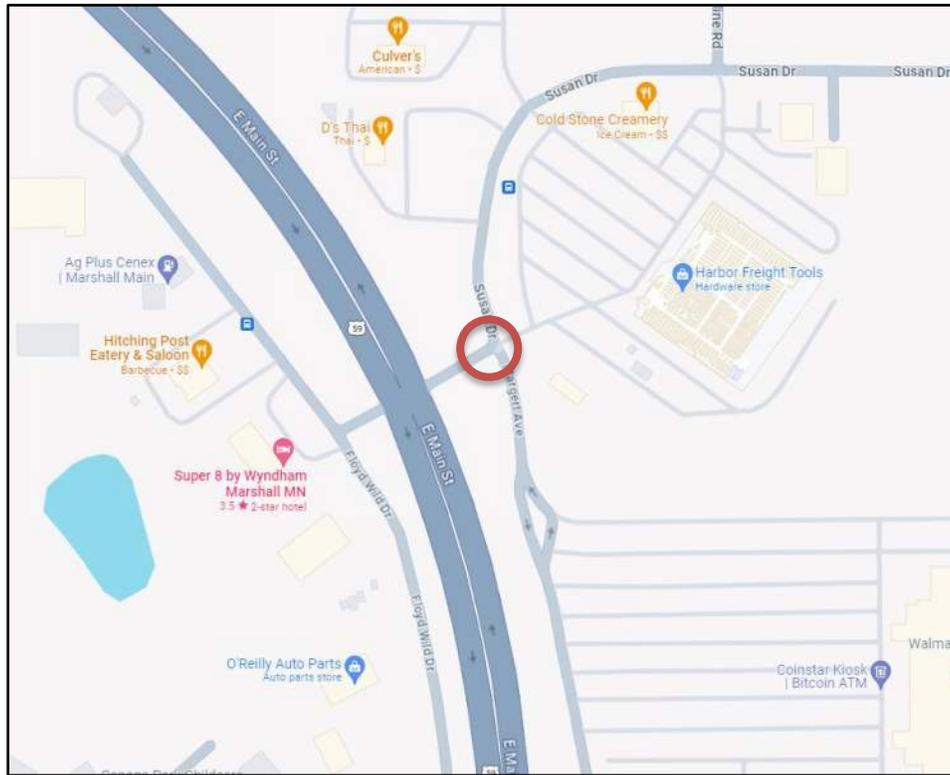


Figure 1. Project Location Map (Map Courtesy of Google)

Existing Conditions

The study area is in the City of Marshall, Lyon County, Minnesota. Susan Drive is a 2-lane undivided roadway that is 38ft from curb to curb. It is classified as a local street and has no posted speed limit but is designed for 30mph. Margaret Avenue is a 2-lane undivided roadway that is 38ft from curb to curb. It is classified as a local street and has no posted speed limit.

The intersection of Susan Drive and Margaret Avenue is a four-legged intersection that is three-way stop controlled with the eastbound approach being a free movement. The segment of Susan Drive between US 59 and Margaret Avenue has 105ft of stacking distance between the adjacent intersections. The north leg is the continuation of Susan Drive. The east leg is the entrance to the commercial parking lot for Harbor Freight Tools. The south leg is Margaret Avenue which leads to the Wal-Mart parking lot. Pedestrian facilities exist on the north leg only.

The study intersection is closely spaced to the US 59 (E Main St) and Susan Drive intersection. US 59 is a 4-lane, divided roadway with curb and gutter classified as a principal arterial that runs north-south through Marshall. The speed limit is 40 mph at the Susan Drive intersection. The intersection of US 59 and Susan Dr is signalized with US 59 considered the major roadway. The west leg of the intersection has 50ft of stacking distance and then opens into a frontage road system that services a variety of businesses including a gas station, hotel, restaurant, and other commercial businesses. Pedestrian

accommodations are provided for the the north and east legs of the intersection. See **Figure 2** for the existing intersection layout and lane configuration.

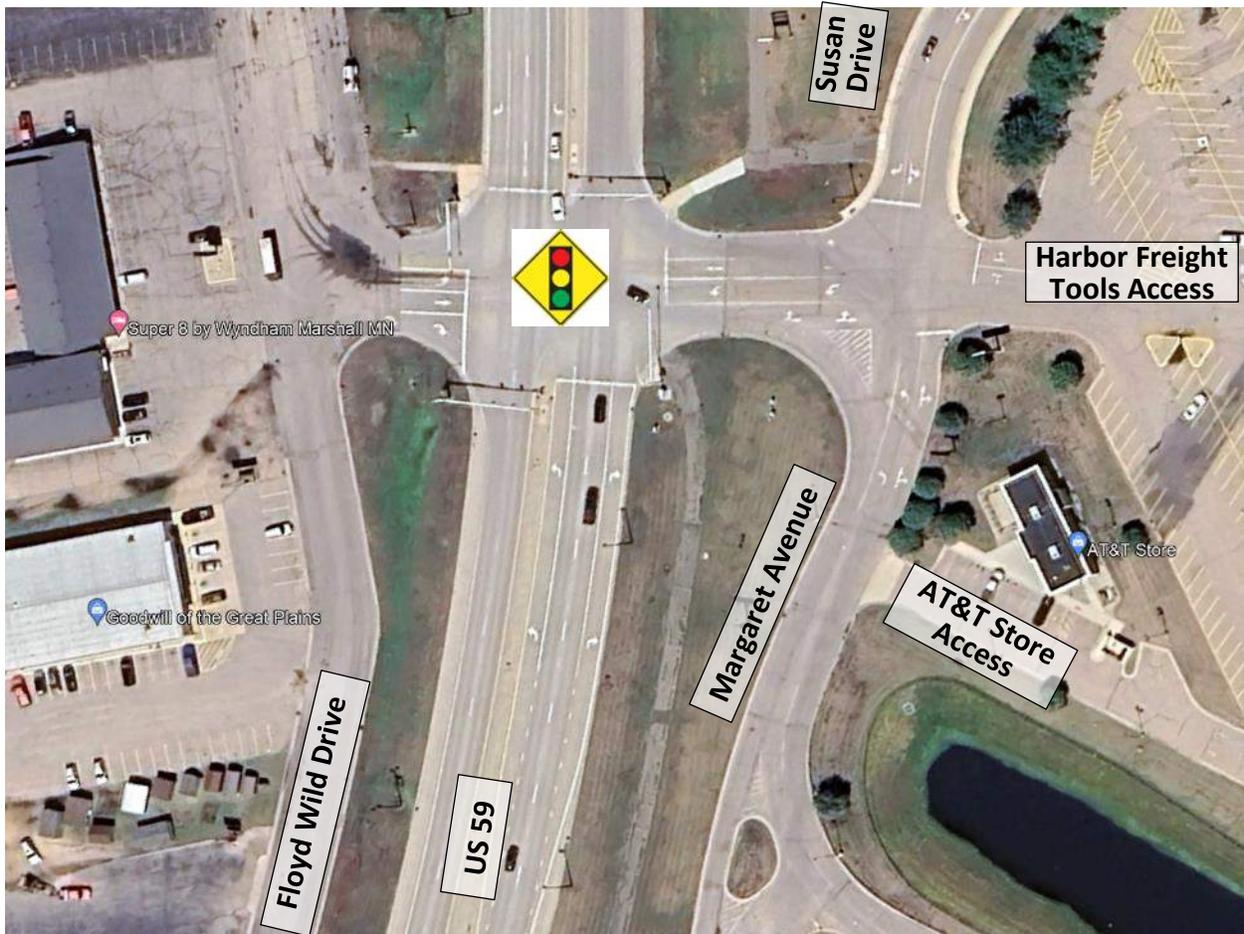


Figure 2: Existing Intersection Layout

Data Collection

Traffic counts were collected in October of 2023 at the intersections of US 59/Susan Drive and Susan Drive/Margaret Avenue. A 13-hour turning movement count was collected on Thursday, October 12th between the hours of 6 am and 7 pm. The details of the count can be found in **Appendix A**. AM and PM peak hours were identified as:

Susan Drive and Margaret Avenue

- 11:00 AM to 12:00 PM for the AM Peak Hour
- 12:45 PM to 1:45 PM for the PM Peak Hour

US 59 and Susan Drive

- 11:00 AM to 12:00 PM for the AM Peak Hour
- 3:45 PM to 4:45 PM for the PM Peak Hour

Though the PM peak hours for the two intersections differ, peak volumes were analyzed together to be conservative and capture the greatest volume threshold.

Safety Analysis

A crash review was completed for the intersection using 2018-2022 crash data obtained from the Minnesota Crash Mapping Analysis Tool (MnCMAT2).

Crash data from 2018-2022 was analyzed to determine the observed crash rate, statewide average crash rate, critical crash rate and critical index. The same parameters for injury crashes were also calculated. The observed crash rate is the number of crashes per million entering vehicles (MEV). The critical rate is a statistical comparison based on similar intersections statewide. An observed crash rate greater than the critical rate indicates that the intersection operates outside the expected, normal range. The critical index reports the magnitude of this difference (i.e. observed crash rate ÷ critical crash rate); a critical index greater than 1.0 indicates that the observed crash rate is greater than the critical rate and that the intersection crash history is above the expected, normal range. **Tables 1 and 2** summarize the safety analysis results for the intersection.

Table 1. Intersection Crash Data (2018-2022)

Intersection	Traffic Control	Total Crashes (5 Years)	Total Crash Rate				Fatal & Serious Injury Crash Rate			
			Observed	Statewide Average	Critical Rate	Crash Index	Observed	Statewide Average	Critical Rate	Crash Index
US 59 & Susan Dr	Signal	9	0.777	0.508	1.090	0.710	0.00	0.69	8.13	0.00
Susan Dr & Margaret Ave	TWSC	5	0.989	0.128	0.640	1.550	0.00	0.311	13.38	0.00

For the US 59 and Susan Drive intersection, the observed total crash rate for this period is 0.78 per MEV; this is 29% below the critical rate. Based on similar statewide intersections, an additional 4 crashes over the five years would indicate this intersection operates outside the normal range.

For the Susan Drive and Margaret Avenue intersection, the observed total crash rate for this period is 0.99 per MEV; this is 1.6 times the critical rate. If crashes were reduced by 2 over five years, this intersection would perform within normal range.

Table 2. Crash Severity & Type Summary (2018-2022)

Intersection	Crash Severity					Crash Type					
	Fatal	A Injury	B Injury	C Injury	PDO	Angle	Rear End	Left Turn	Sideswipe Opposing	Run off Road	Total
US 59 & Susan Dr	0	0	2	2	5	3	1	3	1	0	9
Susan Dr & Margaret Ave	0	0	0	0	5	2	1	1	0	1	5

An analysis of crash types and severities may help in identifying common safety issues. For the Susan Drive and Margaret Avenue intersection, the analysis of the crash types reveals that two of the five observed crashes during the five-year period were reported as right-angle collisions. An intersection safety screening worksheet and crash summaries for this intersection are shown in **Appendix B**.

Warrant Analysis

All-way stop control and traffic control signal warrant analysis was completed for the intersection of Susan Drive and Margaret Avenue using the 2023 traffic volumes.

Traffic Control Signal Warrant Analysis

Traffic signal warrants have been developed as national guidelines to promote continuity of traffic control devices to ensure that traffic signals are installed at intersections that would benefit from their use.

The MnMUTCD (Chapter 4C) states that the investigation of the need for a traffic control signal shall include an analysis of the applicable factors contained in the following traffic signal warrants:

- Warrant 1: Eight-Hour Vehicular Volume
- Warrant 2: Four-Hour Vehicular Volume

- Warrant 3: Peak Hour
- Warrant 4: Pedestrian Volume
- Warrant 5: School Crossing
- Warrant 6: Coordinated Signal System
- Warrant 7: Crash Experience
- Warrant 8: Roadway Network
- Warrant 9: Intersection Near a Grade Crossing

A traffic control signal should not be installed unless one or more of the warrants can be met, however the satisfaction of a traffic signal warrant or warrants shall not in itself require the installation of a traffic signal. Furthermore, a traffic control signal should not be installed unless an engineering study indicates that the traffic control signal will improve the overall safety and operation of the intersection. Finally, the signal should not disrupt the progressive flow of traffic. The following assumptions were made when analyzing Warrant 1:

- North and south legs considered the major approaches due to higher volume.
- Right turn lane volumes for the east and west legs were omitted
- Right turn lanes were omitted in the approach lane counts

The warrant analysis shows that no warrants are met for the Susan Dr and Margaret Ave intersection with existing traffic volumes. A traffic signal is not warranted at the intersection. The results of the signal warrant analysis are documented in **Appendix C**.

All-Way Stop Control Warrant Analysis

All-way stop control can be useful as a safety measure at intersections if safety concerns exist because of high traffic volumes in multiple directions or if there is insufficient sight distance available to see conflicting traffic on an approach to an intersection. The decision to install an all-way stop control should be based on an engineering study.

The MnMUTCD identifies the following criteria that should be considered in the engineering study for an all-way stop control installation:

- Condition A: Where traffic control signals are justified, an all-way stop can be installed as an interim measure.
- Condition B: Five or more crashes are reported in a 12-month period.
- Condition C: The volume of either vehicles or a combination of vehicles, pedestrians and bicycles entering the intersection from all approaches for any eight hours of an average day meets the minimum volume requirements set forth in section 2B.7 of the 2018 MnMUTCD.

All-way stop control is met for 2 of the required 8 hours with the 2023 traffic volumes for the existing conditions. All-way stop control is not warranted at the intersection. The results of the all-way stop warrant analysis are documented in **Appendix C**.

Operations Analysis

The operational analysis results are described as a Level of Service (LOS) ranging from A to F as shown in **Table 3**. These letters serve to describe a range of operating conditions for different types of facilities. Levels of service are calculated based on the Highway Capacity Manual 6th edition, which base the level of service on control delay. Control delay is the delay experienced by vehicles slowing down as they are approaching the intersection, the wait time at the intersection, and the time for the vehicle to speed up through the intersection and enter the traffic stream. The average intersection control delay is a volume weighted average of delay experienced by all motorists entering the intersection on all intersection approaches for signalized and roundabout intersections. Level of Service D is commonly taken as an acceptable design year LOS.

Table 3: Level of Service Criteria

	Signalized Intersection	Unsignalized Intersection
LOS	Control Delay per Vehicle (sec.)	Control Delay per Vehicle (sec.)
A	≤ 10	≤ 10
B	>10 and ≤ 20	>10 and ≤ 15
C	>20 and ≤ 35	>15 and ≤ 25
D	>35 and ≤ 55	>25 and ≤ 35
E	>55 and ≤ 80	>35 and ≤ 50
F	>80	>50

Traffic operations and queuing details for 2023 traffic volumes can be found in the **Appendix D**.

Existing Conditions

Both intersections operate with acceptable delays on all approaches in 2023. **Table 3** shows that all approaches of the Susan Drive and Margaret Avenue intersection operate with LOS A except northbound which has LOS B.

Table 4. Existing Conditions Traffic Operations Analysis

Existing Condition					
Intersection	Approach Leg	AM Peak LOS	PM Peak LOS	AM Peak Intersection LOS	PM Peak Intersection LOS
US Highway 59 & Susan Dr	NB	B	B	B	B
	SB	C	C		
	EB	C	C		
	WB	A	A		
Susan Dr & Margaret Ave	NB	B	B	A	A
	SB	A	A		
	EB	A	A		
	WB	A	A		

Alternatives Analysis

A range of potential alternatives were explored for the US 59 at Susan Drive and Susan Drive at Margaret Avenue intersections to improve safety and support efficient movement of traffic through the area. This exercise included a comprehensive review of the following characteristics to identify a well-rounded improvement supportive of the many competing needs and interests at the intersection:

- *Business Access* – Sufficient access must be maintained to area businesses including Harbor Freight Tools, AT&T, D’S Thai Restaurant, and Walmart (via Margaret Avenue to the south).
- *Pedestrian/Bicyclist Accommodations* – Reducing overall exposure and simplifying decision-making for pedestrians and bicyclists in the area is a priority. Lane reductions, defined marked pedestrian crossings, and improved sidewalk/trail connectivity were considered.
- *Traffic Control* – The traffic signal at US 59 and Susan Drive will be maintained. Review of the existing three-way stop at Susan Drive and Margaret Avenue is needed to identify if other alternatives provide improved traffic flow and safety.
- *Intersection Geometry* – The current spacing between US 59 and Margaret Avenue allows for limited stacking distance and requires complex decision-making for drivers navigating the area. Re-alignment of Susan Drive or Margaret Avenue, lane reductions, access modifications, and other elements were reviewed to simplify movements at the intersection.

Several alternatives were developed, each with varying levels of accommodation for the competing needs at the intersection. The following two alternatives were viewed as best accommodating the needs of the area:

- Alternative #1: Susan Drive T-Intersection - The Susan Drive T-Intersection Alternative involves relocating the harbor freight entrance and realigning Susan Drive, aiming to improve traffic flow. See **Figure 3** for details.
- Alternative #2: Margaret Avenue Closure – The Margaret Avenue Closure Alternative recommends closing the access to Walmart on Margaret Avenue. See **Figure 4** for details.

Both alternatives will be analyzed for their feasibility and impact on the transportation system.

Alternative #1: Susan Drive T-Intersection

The T-Intersection Alternative assumes relocating the harbor freight entrance and realigning Susan Drive so that the existing southbound right and eastbound left become the through movements. Margaret Avenue then Ts into Susan Dr and is stop controlled. The lane configuration on the WB approach of US 59 is modified to combine the left and thru movements. This change allows room for a southbound left turn lane at the new Susan Drive and Margaret Avenue intersection. The pedestrian crossing of Susan Drive is shifted north to the new Harbor Freight entrance. This concept is shown in **Figure 3** and a standalone figure is included in **Appendix E**.



Figure 3. T-Intersection Alternative

Operations Analysis

The Build Conditions analysis for the T-intersection alternative shows that the revised configuration is expected to operate with acceptable delays on all approaches with existing traffic volumes. **Table 5** shows that all approaches operate with LOS A except northbound which operates at a LOS B during the AM and PM peak hours. The findings also show that modifications proposed at the Susan Drive and Margaret Avenue intersection do not negatively impact the function of the signalized US Highway 59 and Susan Drive intersection.

Table 5. Build Conditions Traffic Operations Analysis

Build Condition					
Condition	Approach Leg	AM Peak LOS	PM Peak LOS	AM Peak Intersection LOS	PM Peak Intersection LOS
US Highway 59 & Susan Dr	NB	B	B	B	B
	SB	C	C		
	EB	C	C		
	WB	B	A		
Susan Dr & Margaret Ave	NB	B	B	A	A
	SB	A	A		
	EB	A	A		

Queues for the westbound approach of Susan Drive at US Highway 59 were analyzed to identify if issues arise between the closely spaced intersections. As noted, the westbound approach is reduced from three lanes of approach (left, thru, and right) to two lanes of approach (left-thru and right). Storage length in the Existing Conditions there is separate left, through and right turn lanes. Storage length for the thru-left lane is extended from 105 feet to 115 with the build condition. **Table 6** shows expected queuing between the intersections. The maximum queue for both alternatives is 50ft. This is approximately 2 car lengths and does not extend beyond the available storage in either condition.

Table 6. Build Conditions Traffic Queuing Analysis

US Highway 59 & Susan Dr					
Condition	Peak Hour	Approach Leg	Movement	Queue (ft)	
				Average	Maximum
Existing	AM	WB	L	25	25
			T	25	25
			R	0	50
	PM		L	25	50
			T	25	25
			R	0	50
Build	AM	LT	25	50	
		R	0	50	
	PM	LT	25	50	
		R	0	50	

Alternative #2: Margaret Avenue Closure

The Margaret Avenue Closure Alternative assumes severing the Margaret Avenue connection between Susan Drive and Walmart south of the AT&T access off Margaret Avenue. The lane configuration on the WB approach of US Highway 59 is changed to combine the left and thru movements. For the Susan Drive and Margaret Avenue intersection, the eastbound, northbound, and westbound approaches are combined into a single lane. An exclusive right turn lane is maintained for the southbound approach to the intersection. The existing pedestrian crossing of the north leg is maintained. This concept is shown in **Figure 4** and a standalone figure is included in **Appendix E**.

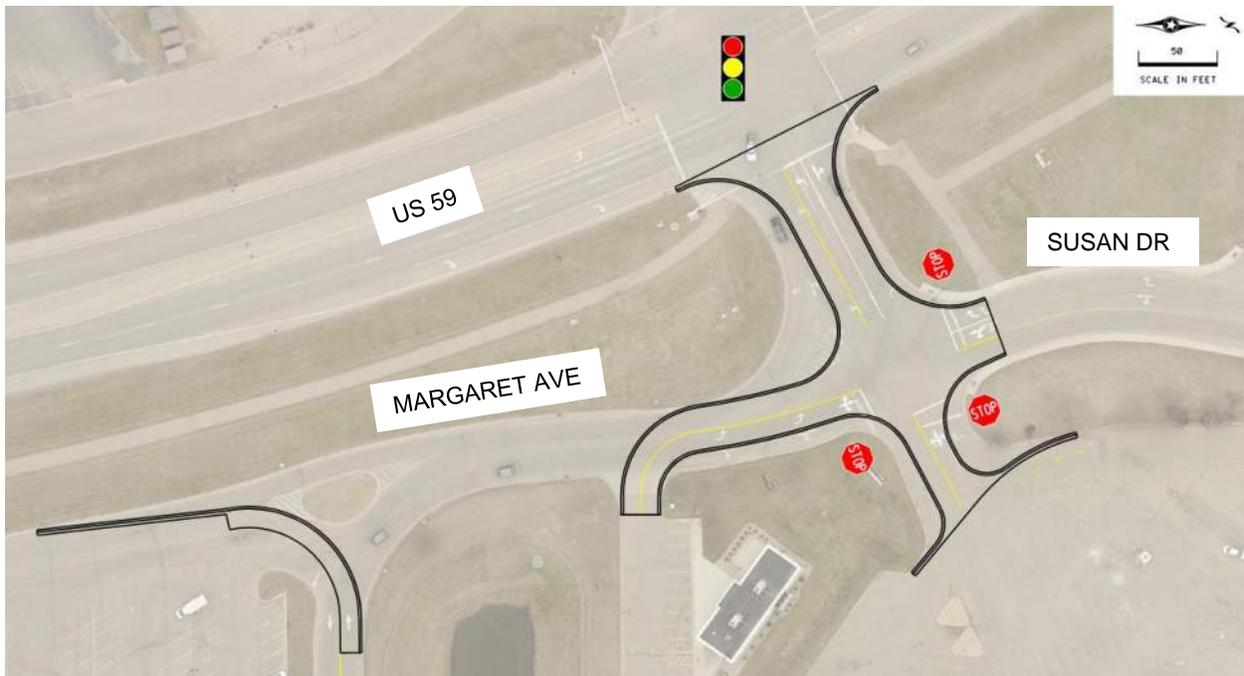


Figure 4. Margaret Avenue Closure Alternative

Operations Analysis

The Build Conditions analysis for the Margaret Avenue Closure Alternative shows acceptable operations for all approaches. Traffic levels are expected to be reduced for the west and south legs of the Susan Drive and Margaret Avenue intersection with all Walmart traffic now redirected to Boyer Drive to the south.

Additional Alternatives Reviewed

In addition to Alternatives 1 and 2 outlined above, the following alternatives were explored and documented as potential intersection treatments for the Susan Drive and Margaret Avenue but are not recommended to be carried forward.

Alternative #3: Tightened Intersection Alternative

The Tightened Intersection Alternative minimizes pavement needs for the intersection. The lane configuration on the WB approach of US Highway 59 is modified to combine the left and thru approach lanes. For the Susan Drive and Margaret Avenue intersection, the lanes are combined to one left/thru/right for the eastbound and westbound approaches. The existing pedestrian crossing of the north leg is maintained. This concept is shown in **Figure 5** and a standalone figure is included in **Appendix E**.

This alternative was dismissed as it is not expected to provide the safety and operational benefits of Alternatives 1 and 2 outlined previously.

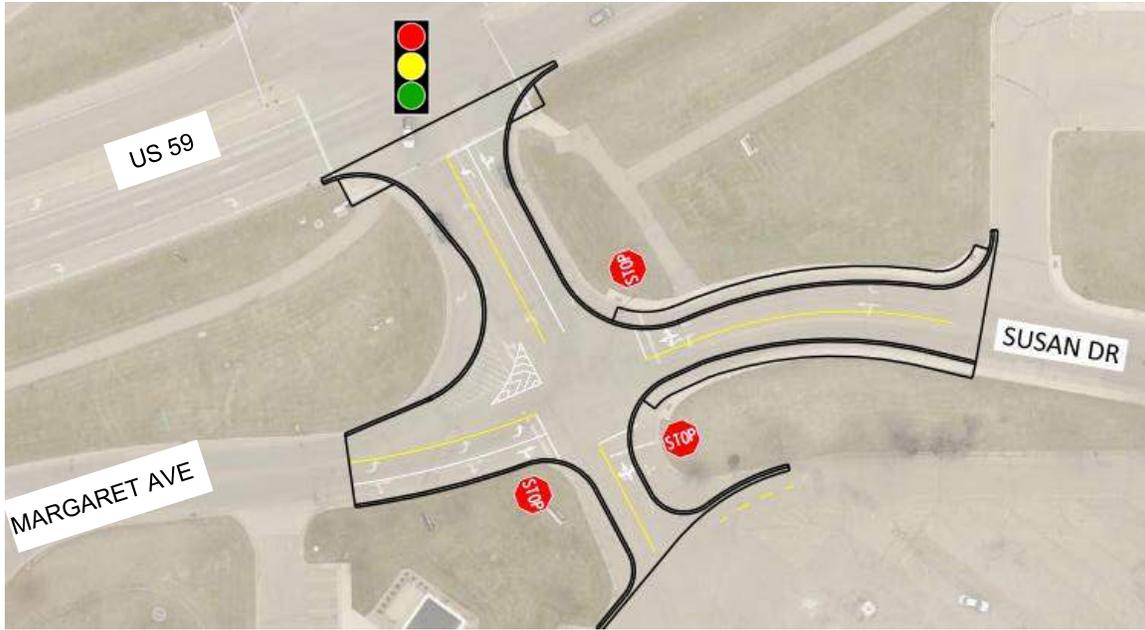


Figure 5. Tightened Intersection Alternative

Alternative #4: Eastbound Free Right Alternative

The Eastbound Free Right Alternative assumes relocating the Harbor Freight Tools entrance and realigns Susan Drive to allow the existing southbound right and eastbound left to function as the through movements. Margaret Avenue then Ts into Susan Dr and is stop controlled. The lane configuration on the westbound approach of US Highway 59 is changed to combine the left and thru movements. This change allows room for a southbound left turn lane at the new Susan Drive and Margaret Ave. The pedestrian crossing of Susan Drive is shifted north to driveway entrance. This concept is shown in **Figure 6** and a standalone figure is included in **Appendix E**.

This alternative was dismissed as it is not expected to provide the safety and operational benefits of Alternatives 1 and 2 outlined previously.



Figure 6. Eastbound Free Right Alternative

Recommendations

The operations and warrant analysis for the Susan Drive and Margaret Avenue intersection indicate that a variety of intersection alternatives could effectively serve the level of traffic traveling through the intersection with existing conditions. Alternative #1 and Alternative #2 are the preferred improvements due to their ability to best manage the competing interests of the intersection. It will be the decision of City officials as to whether access closures are permissible or if those presented in varying alternatives represent fatal flaws in carrying forward a preferred vision. All alternatives outlined in this document are expected to simplify decision-making at the intersection while maintaining sufficient operations.

Appendix A

Turning Movement Counts

US 59 & Susan Dr, Marshall MN

0 0

Thursday, October 12, 2023

Time	Southbound						Westbound						Northbound						Eastbound						VEHICLE TOTAL	
	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total		
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:00 AM	0	3	13	7	0	23	0	0	0	6	0	6	0	0	19	1	0	20	0	10	0	1	0	11	60	60
6:15 AM	0	5	17	11	0	33	0	1	0	5	0	6	0	2	26	0	28	0	8	0	3	0	11	78	78	
6:30 AM	0	5	23	6	0	34	0	0	0	5	0	5	0	4	34	1	39	0	12	1	1	0	14	92	92	
6:45 AM	0	16	24	6	0	46	0	0	2	6	0	8	0	1	39	3	43	0	5	0	1	0	6	103	103	
Hourly Total	0	29	77	30	0	136	0	1	2	22	0	25	0	7	118	5	130	0	35	1	6	0	42	333	333	
7:00 AM	0	9	27	10	0	46	0	1	0	13	1	14	0	1	31	0	32	0	1	1	0	0	2	94	94	
7:15 AM	0	6	32	12	1	50	0	0	0	11	0	11	0	4	68	1	73	0	11	1	3	0	15	149	149	
7:30 AM	0	10	32	11	0	53	0	1	1	13	0	15	0	5	86	2	93	0	9	2	1	0	12	173	173	
7:45 AM	0	23	43	21	0	87	0	0	2	11	0	13	0	5	78	3	86	0	16	2	4	0	22	208	208	
Hourly Total	0	48	134	54	1	236	0	2	3	48	1	53	0	15	263	6	284	0	37	6	8	0	51	624	624	

US 59 & Susan Dr, Marshall MN
0 0

Thursday, October 12, 2023

Time	Southbound 0						Westbound 0						Northbound 0						Eastbound 0						VEHICLE TOTAL
	U Turns	Left Turns	Straight Through	Right Turns	Crosswal k Crossings	Vehicle Approac h Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswal k Crossings	Vehicle Approac h Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswal k Crossings	Vehicle Approac h Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswal k Crossings	Vehicle Approac h Total	
8:00 AM	0	19	51	16	0	86	0	3	3	17	1	23	0	2	47	1	0	50	0	12	0	3	0	15	174
8:15 AM	0	24	46	12	0	82	0	2	0	15	0	17	0	2	47	3	0	52	0	16	2	1	0	19	170
8:30 AM	0	23	36	14	0	73	0	2	2	27	0	31	0	3	32	3	0	38	0	7	0	2	0	9	151
8:45 AM	1	24	35	13	0	73	0	2	2	17	0	21	0	2	43	3	0	48	0	13	2	6	0	21	163
Hourly Total	1	90	168	55	0	314	0	9	7	76	1	92	0	9	169	10	0	188	0	48	4	12	0	64	658
9:00 AM	0	23	50	18	0	91	0	2	4	22	0	28	0	2	47	5	0	54	0	9	0	4	0	13	186
9:15 AM	1	22	45	17	0	85	0	1	1	25	0	27	0	4	44	6	0	54	0	16	1	4	0	21	187
9:30 AM	0	33	40	13	0	86	0	2	3	24	0	29	0	4	39	3	0	46	0	16	3	3	0	22	183
9:45 AM	0	36	43	23	0	102	0	3	2	30	0	35	0	4	71	5	0	80	0	15	1	4	0	20	237
Hourly Total	1	114	178	71	0	364	0	8	10	101	0	119	0	14	201	19	0	234	0	56	5	15	0	76	793
10:00 AM	0	23	50	13	0	86	0	2	0	28	0	30	0	1	55	3	0	59	0	20	3	2	0	25	200
10:15 AM	0	46	37	19	0	102	0	2	1	36	0	39	0	3	54	4	0	61	0	12	6	1	0	19	221
10:30 AM	0	45	32	20	3	97	0	1	2	33	0	36	0	2	60	6	0	68	0	13	1	2	0	16	217
10:45 AM	0	48	43	16	0	107	0	2	1	34	0	37	0	1	44	5	0	50	0	17	7	3	0	27	221
Hourly Total	0	162	162	68	3	392	0	7	4	131	0	142	0	7	213	18	0	238	0	62	17	8	0	87	859
11:00 AM	0	36	49	18	0	103	0	3	4	28	0	35	0	4	48	4	0	56	0	21	2	3	0	26	220
11:15 AM	0	40	51	19	0	110	0	4	5	40	0	49	0	2	46	6	0	54	0	19	6	4	0	29	242
11:30 AM	0	42	49	15	0	106	0	7	4	40	0	51	0	2	49	4	0	55	0	11	5	3	0	19	231
11:45 AM	0	41	70	14	0	125	0	4	2	42	0	48	0	3	54	6	0	63	0	16	4	4	0	24	260
Hourly Total	0	159	219	66	0	444	0	18	15	150	0	183	0	11	197	20	0	228	0	67	17	14	0	98	953
12:00 PM	0	55	72	16	0	143	0	4	2	44	0	50	0	2	67	9	0	78	0	23	4	2	0	29	300
12:15 PM	0	42	59	23	0	124	0	5	4	48	0	57	0	1	69	3	0	73	0	16	3	4	0	23	277
12:30 PM	1	53	51	23	0	128	0	5	2	43	0	50	0	0	59	4	0	63	0	15	3	3	0	21	262
12:45 PM	0	52	51	29	0	132	0	7	3	50	0	60	0	2	62	5	0	69	0	16	2	4	0	22	283
Hourly Total	1	202	233	91	0	527	0	21	11	185	0	217	0	5	257	21	0	283	0	70	12	13	0	95	1122
1:00 PM	0	48	61	17	0	126	0	12	2	72	0	86	0	5	50	11	0	66	0	14	3	6	0	23	301
1:15 PM	0	51	54	20	0	125	0	4	5	45	0	54	0	1	50	7	0	58	0	21	3	1	0	25	262
1:30 PM	0	45	41	21	0	107	0	3	3	52	0	58	0	2	49	5	0	56	0	22	3	2	0	27	248
1:45 PM	0	38	52	26	0	116	0	8	7	39	0	54	0	3	41	3	0	47	0	28	5	4	0	37	254
Hourly Total	0	182	208	84	0	474	0	27	17	208	0	252	0	11	190	26	0	227	0	85	14	13	0	112	1065
2:00 PM	0	43	37	16	0	96	0	5	3	63	0	71	0	5	43	6	0	54	0	21	3	4	0	28	249
2:15 PM	0	38	66	16	0	120	0	3	5	47	0	55	0	3	47	1	0	51	0	22	4	3	0	29	255
2:30 PM	0	45	49	18	0	112	0	3	5	45	0	53	0	2	84	6	0	92	0	17	2	4	0	23	280
2:45 PM	0	39	53	17	0	109	0	4	4	48	0	56	0	0	66	2	0	68	0	9	2	3	0	14	247
Hourly Total	0	165	205	67	0	437	0	15	17	203	0	235	0	10	240	15	0	265	0	69	11	14	0	94	1031
3:00 PM	0	27	55	10	0	92	0	3	0	47	0	50	0	1	50	2	0	53	0	19	3	0	0	22	217
3:15 PM	0	44	62	32	0	138	0	4	5	41	0	50	0	2	50	0	0	52	0	18	5	2	0	25	265
3:30 PM	0	48	54	23	0	125	0	4	2	42	0	48	0	2	59	1	0	62	0	23	5	3	0	31	266
3:45 PM	0	61	75	22	0	158	0	4	2	59	0	65	0	3	88	3	0	94	0	17	3	3	0	23	340
Hourly Total	0	180	246	87	0	513	0	15	9	189	0	213	0	8	247	6	0	261	0	77	16	8	0	101	1088

US 59 & Susan Dr, Marshall MN

0 0

Thursday, October 12, 2023

Time	Southbound						Westbound						Northbound						Eastbound						VEHICLE TOTAL
	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	
4:00 PM	0	54	67	24	0	145	0	8	3	47	2	58	0	2	78	8	0	88	0	17	4	5	0	26	317
4:15 PM	0	45	58	23	0	126	0	5	2	52	0	59	0	5	55	3	0	63	0	14	5	4	0	23	271
4:30 PM	0	40	88	18	0	146	0	4	2	43	0	49	0	3	66	3	0	72	0	16	6	1	0	23	290
4:45 PM	0	47	84	6	0	137	0	6	3	71	0	80	0	1	70	5	0	76	0	17	7	2	0	26	319
Hourly Total	0	186	297	71	0	554	0	23	10	213	2	246	0	11	269	19	0	299	0	64	22	12	0	98	1197
5:00 PM	0	45	89	5	0	139	0	6	6	54	0	66	0	4	68	4	0	76	0	25	4	5	0	34	315
5:15 PM	0	45	60	5	0	110	0	7	4	41	0	52	0	4	68	3	0	75	0	19	2	3	0	24	261
5:30 PM	0	40	60	4	0	104	0	5	2	46	0	53	0	3	45	4	0	52	0	14	1	3	0	18	227
5:45 PM	0	40	60	0	0	100	0	2	4	45	0	51	0	2	58	6	0	66	0	8	4	7	0	19	236
Hourly Total	0	170	269	14	0	453	0	20	16	186	0	222	0	13	239	17	0	269	0	66	11	18	0	95	1039
6:00 PM	0	37	44	4	0	85	0	10	2	57	0	69	0	2	58	4	0	64	0	14	2	2	0	18	236
6:15 PM	0	30	42	1	0	73	0	3	4	44	0	51	0	3	41	2	0	46	0	12	3	2	0	17	187
6:30 PM	0	48	34	0	0	82	0	5	1	45	0	51	0	1	40	5	0	46	0	13	4	2	0	19	198
6:45 PM	0	40	26	0	0	66	0	5	0	39	0	44	0	3	28	0	0	31	0	10	0	1	0	11	152
Hourly Total	0	155	146	5	0	306	0	23	7	185	0	215	0	9	167	11	0	187	0	49	9	7	0	65	773
7:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DAILY TOTAL	3	1842	2542	763	4	5150	0	189	128	1897	4	2214	0	130	2770	193	0	3093	0	785	145	148	0	1078	11535
Cars	3	1818	2310	687	4	4818	0	184	125	1847	4	2156	0	118	2538	186	0	2842	0	704	144	132	0	980	10796
Heavy Vehicles	0	24	232	76	0	332	0	5	3	50	0	58	0	12	232	7	0	251	0	81	1	16	0	98	739
Heavy Vehicle %	0.00%	1.30%	9.13%	9.96%	0.00%	6.45%	0.00%	2.65%	2.34%	2.64%	0.00%	2.62%	0.00%	9.23%	8.38%	3.63%	0.00%	8.12%	0.00%	10.32%	0.69%	10.81%	0.00%	9.09%	6.41%

US 59 & Susan Dr, Marshall MN
0 0
Thursday, October 12, 2023

AM Peak Hour

Time	Southbound						Westbound						Northbound						Eastbound						VEHICLE TOTAL
	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	
11:00 AM	0	36	49	18	0	103	0	3	4	28	0	35	0	4	48	4	0	56	0	21	2	3	0	26	220
11:15 AM	0	40	51	19	0	110	0	4	5	40	0	49	0	2	46	6	0	54	0	19	6	4	0	29	242
11:30 AM	0	42	49	15	0	106	0	7	4	40	0	51	0	2	49	4	0	55	0	11	5	3	0	19	231
11:45 AM	0	41	70	14	0	125	0	4	2	42	0	48	0	3	54	6	0	63	0	16	4	4	0	24	260
Peak Hour Total	0	159	219	66	0	444	0	18	15	150	0	183	0	11	197	20	0	228	0	67	17	14	0	98	953
PHF	0.000	0.946	0.782	0.868	0.000	0.888	0.000	0.643	0.750	0.893	0.000	0.897	0.000	0.688	0.912	0.833	0.000	0.905	0.000	0.798	0.708	0.875	0.000	0.845	0.916

PM Peak Hour

Time	Southbound						Westbound						Northbound						Eastbound						VEHICLE TOTAL
	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	
3:45 PM	0	61	75	22	0	158	0	4	2	59	0	65	0	3	88	3	0	94	0	17	3	3	0	23	340
4:00 PM	0	54	67	24	0	145	0	8	3	47	2	58	0	2	78	8	0	88	0	17	4	5	0	26	317
4:15 PM	0	45	58	23	0	126	0	5	2	52	0	59	0	5	55	3	0	63	0	14	5	4	0	23	271
4:30 PM	0	40	88	18	0	146	0	4	2	43	0	49	0	3	66	3	0	72	0	16	6	1	0	23	290
Peak Hour Total	0	200	288	87	0	575	0	21	9	201	2	231	0	13	287	17	0	317	0	64	18	13	0	95	1218
PHF	0.000	0.820	0.818	0.906	0.000	0.910	0.000	0.656	0.750	0.852	0.250	0.888	0.000	0.650	0.815	0.531	0.000	0.843	0.000	0.941	0.750	0.650	0.000	0.913	0.896

Total Vehicles On Leg		10605	
Vehicles Entering Intersection	5150	Vehicles Exiting Intersection	5455
Southbound			
Cars	687	2310	1818
Heavy	76	232	24
Total	763	2542	1842



Total Vehicles on Leg 2099	Vehicles Entering Intersection 1078	Eastbound	Cars	Heavy	Total
			0	0	0
	0		0	0	
	704		81	785	
	144		1	145	
Vehicles Exiting Intersection 1021			132	16	148



Daily Volumes

Cars	Heavy	Total	Westbound	Vehicles Entering Intersection 2214	Total Vehicles on Leg 4394
1847	50	1897			
125	3	128			
184	5	189			
0	0	0			
4	0	4	Vehicles Exiting Intersection 2180		



Cars	0	0	118	2538	186
Heavy	0	0	12	232	7
Total	0	0	130	2770	193
Northbound					
Vehicles Entering Intersection 3093			Vehicles Exiting Intersection 2879		
Total Vehicles On Leg			5972		



Susan Dr & Margert Ave, Marshall MN

0 0

Thursday, October 12, 2023

Time	Southbound						Westbound						Northbound						Eastbound						VEHICLE TOTAL	
	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total		
12:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
2:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
3:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:00 AM	0	0	2	2	0	4	0	0	0	0	0	0	4	0	0	0	4	0	0	0	4	0	0	4	4	12
6:15 AM	0	0	0	2	0	2	0	0	0	0	0	0	4	4	0	0	8	0	0	0	5	0	0	5	5	15
6:30 AM	0	0	0	0	0	0	0	1	0	0	1	0	4	0	1	0	5	0	2	0	5	0	0	7	7	13
6:45 AM	0	0	2	3	0	5	0	1	0	0	1	0	5	0	0	5	0	5	2	12	0	0	19	19	30	
Hourly Total	0	0	4	7	0	11	0	1	1	0	2	0	17	4	1	0	22	0	7	2	26	0	0	35	35	70
7:00 AM	0	0	1	3	0	4	0	0	1	0	1	0	10	1	0	0	11	0	2	0	8	0	0	10	10	26
7:15 AM	0	0	1	3	1	4	0	0	0	0	0	0	9	1	0	0	10	0	2	1	5	0	0	8	8	22
7:30 AM	0	0	3	6	0	9	0	1	0	0	1	0	8	0	0	0	8	0	5	0	9	0	0	14	14	32
7:45 AM	0	0	1	7	0	8	0	0	0	0	0	0	6	1	0	0	7	0	14	2	12	0	0	28	28	43
Hourly Total	0	0	6	19	1	25	0	1	1	0	2	0	33	3	0	0	36	0	23	3	34	0	0	60	60	123

Susan Dr & Margert Ave, Marshall MN

0 0

Thursday, October 12, 2023

Time	Southbound						Westbound						Northbound						Eastbound						VEHICLE TOTAL	
	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total		
8:00 AM	0	0	2	8	0	10	0	1	1	1	0	3	0	14	2	0	0	16	0	5	1	14	0	20	0	49
8:15 AM	0	0	1	6	0	7	0	0	0	1	0	1	0	11	3	0	0	14	0	7	3	18	0	28	0	50
8:30 AM	0	0	1	13	0	14	0	1	4	0	0	5	0	15	1	1	0	17	0	9	1	16	0	26	0	62
8:45 AM	0	0	1	9	0	10	0	0	0	0	0	0	0	11	3	1	0	15	0	5	3	21	0	29	0	54
Hourly Total	0	0	5	36	0	41	0	2	5	2	0	9	0	51	9	2	0	62	0	26	8	69	0	103	0	215
9:00 AM	0	1	2	7	0	10	0	0	4	0	0	4	0	18	1	0	0	19	0	12	1	14	0	27	0	60
9:15 AM	0	0	3	7	0	10	0	0	1	1	0	2	0	18	2	1	0	21	0	6	4	19	1	29	0	62
9:30 AM	0	0	2	14	0	16	0	3	2	0	0	5	0	13	5	0	0	18	0	10	3	26	0	39	0	78
9:45 AM	0	1	4	6	0	11	0	1	0	0	0	1	0	28	3	1	0	32	0	9	2	31	0	42	0	86
Hourly Total	0	2	11	34	0	47	0	4	7	1	0	12	0	77	11	2	0	90	0	37	10	90	1	137	0	286
10:00 AM	0	0	5	7	0	12	0	1	1	0	0	2	0	24	1	1	0	26	0	6	1	23	0	30	0	70
10:15 AM	0	0	2	11	0	13	0	0	1	0	0	1	0	26	2	0	0	28	0	17	5	34	0	56	0	98
10:30 AM	0	0	6	9	0	15	0	1	0	1	0	2	0	27	7	1	0	35	0	16	3	33	0	52	0	104
10:45 AM	0	0	4	13	0	17	0	1	4	0	0	5	0	22	4	1	0	27	0	20	3	37	0	60	0	109
Hourly Total	0	0	17	40	0	57	0	3	6	1	0	10	0	99	14	3	0	116	0	59	12	127	0	198	0	381
11:00 AM	0	0	8	11	0	19	0	0	0	0	0	0	0	23	9	0	0	32	0	12	5	24	0	41	0	92
11:15 AM	0	1	6	20	0	27	0	1	0	0	2	1	0	25	5	1	0	31	0	26	3	23	0	52	0	111
11:30 AM	0	0	5	24	0	29	0	1	1	1	0	3	0	26	4	3	0	33	0	23	2	29	0	54	0	119
11:45 AM	0	0	5	19	0	24	0	0	1	0	0	1	0	30	9	3	0	42	0	14	5	32	0	51	0	118
Hourly Total	0	1	24	74	0	99	0	2	2	1	2	5	0	104	27	7	0	138	0	75	15	108	0	198	0	440
12:00 PM	0	0	5	22	0	27	0	0	5	1	0	6	0	21	11	0	0	32	0	29	6	35	0	70	0	135
12:15 PM	0	0	9	24	0	33	0	0	3	0	0	3	0	30	12	1	0	43	0	22	0	25	0	47	0	126
12:30 PM	0	1	8	22	0	31	0	1	1	0	0	2	0	27	5	0	0	32	0	26	2	32	0	60	0	125
12:45 PM	0	1	7	32	0	40	0	0	1	0	0	1	0	30	13	1	0	44	0	22	5	31	0	58	0	143
Hourly Total	0	2	29	100	0	131	0	1	10	1	0	12	0	108	41	2	0	151	0	99	13	123	0	235	0	529
1:00 PM	0	0	7	36	1	43	0	0	5	0	1	5	0	41	13	4	0	58	0	25	1	34	0	60	0	166
1:15 PM	0	0	1	17	0	18	0	1	7	0	0	8	0	31	6	0	0	37	0	23	4	31	0	58	0	121
1:30 PM	0	0	7	26	0	33	0	3	1	0	0	4	0	32	11	1	0	44	0	12	5	37	0	54	0	135
1:45 PM	0	0	5	23	0	28	0	1	0	1	0	2	0	31	13	3	0	47	0	16	1	28	0	45	0	122
Hourly Total	0	0	20	102	1	122	0	5	13	1	1	19	0	135	43	8	0	186	0	76	11	130	0	217	0	544
2:00 PM	0	0	4	27	0	31	0	1	7	0	0	8	0	37	8	0	0	45	0	25	3	25	1	53	0	137
2:15 PM	0	0	9	27	0	36	0	0	2	0	0	2	0	25	5	0	0	30	0	7	5	31	0	43	0	111
2:30 PM	0	0	9	23	0	32	0	3	2	0	0	5	0	27	10	2	0	39	0	19	2	30	0	51	0	127
2:45 PM	0	0	4	15	0	19	0	0	4	0	0	4	0	37	11	1	0	49	0	17	4	26	0	47	0	119
Hourly Total	0	0	26	92	0	118	0	4	15	0	0	19	0	126	34	3	0	163	0	68	14	112	1	194	0	494
3:00 PM	0	0	6	11	0	17	0	2	4	0	0	6	0	36	3	1	0	40	0	8	3	19	0	30	0	93
3:15 PM	0	0	4	16	0	20	0	2	3	0	0	5	0	31	7	0	0	38	0	10	7	32	0	49	0	112
3:30 PM	0	0	5	18	0	23	0	2	0	0	0	2	0	30	7	1	0	38	0	25	1	26	0	52	0	115
3:45 PM	0	0	7	18	0	25	0	1	2	0	0	3	0	45	7	2	0	54	0	24	3	39	0	66	0	148
Hourly Total	0	0	22	63	0	85	0	7	9	0	0	16	0	142	24	4	0	170	0	67	14	116	0	197	0	468

Susan Dr & Margert Ave, Marshall MN

0 0

Thursday, October 12, 2023

Time	Southbound						Westbound						Northbound						Eastbound						VEHICLE TOTAL
	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	
4:00 PM	0	0	6	19	0	25	0	1	2	0	0	3	0	37	8	1	0	46	0	22	0	45	0	67	141
4:15 PM	0	0	8	23	0	31	0	2	1	0	0	3	0	36	10	0	0	46	0	16	1	36	0	53	133
4:30 PM	0	0	6	24	0	30	0	2	1	0	0	3	0	23	8	1	0	32	0	18	3	29	0	50	115
4:45 PM	0	0	9	32	0	41	0	2	4	0	0	6	0	43	9	4	0	56	0	18	4	37	0	59	162
Hourly Total	0	0	29	98	0	127	0	7	8	0	0	15	0	139	35	6	0	180	0	74	8	147	0	229	551
5:00 PM	0	0	10	24	0	34	0	3	6	0	1	9	0	38	9	1	0	48	0	19	3	30	0	52	143
5:15 PM	0	0	4	17	0	21	0	1	2	0	0	3	0	33	10	2	0	45	0	17	2	33	0	52	121
5:30 PM	0	0	11	12	0	23	0	1	5	0	1	6	0	36	6	0	0	42	0	16	3	28	0	47	118
5:45 PM	0	1	3	14	0	18	0	0	0	0	0	0	0	37	9	0	0	46	0	15	6	28	0	49	113
Hourly Total	0	1	28	67	0	96	0	5	13	0	2	18	0	144	34	3	0	181	0	67	14	119	0	200	495
6:00 PM	0	0	6	25	0	31	0	0	6	0	0	6	0	39	2	0	0	41	0	18	3	21	0	42	120
6:15 PM	0	0	3	20	0	23	0	0	2	0	0	2	0	31	6	2	0	39	0	11	0	22	0	33	97
6:30 PM	0	0	3	18	0	21	0	0	9	1	0	10	0	30	4	1	0	35	0	13	2	41	0	56	122
6:45 PM	0	0	5	14	0	19	0	1	3	0	0	4	0	25	6	0	0	31	0	10	3	28	0	41	95
Hourly Total	0	0	17	77	0	94	0	1	20	1	0	22	0	125	18	3	0	146	0	52	8	112	0	172	434
7:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
9:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Hourly Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DAILY TOTAL	0	6	238	809	2	1053	0	43	110	8	5	161	0	1300	297	44	0	1641	0	730	132	1313	2	2175	5030
Cars	0	6	236	791	2	1033	0	33	109	8	5	150	0	1259	296	44	0	1599	0	716	129	1295	1	2140	4922
Heavy Vehicles	0	0	2	18	0	20	0	10	1	0	0	11	0	41	1	0	0	42	0	14	3	18	1	35	108
Heavy Vehicle %	0.00%	0.00%	0.84%	2.22%	0.00%	1.90%	0.00%	23.26%	0.91%	0.00%	0.00%	6.83%	0.00%	3.15%	0.34%	0.00%	0.00%	2.56%	0.00%	1.92%	2.27%	1.37%	50.00%	1.61%	2.15%

Susan Dr & Margert Ave, Marshall MN

0 0

Thursday, October 12, 2023

AM Peak Hour

Time	Southbound						Westbound						Northbound						Eastbound						VEHICLE TOTAL
	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	
11:00 AM	0	0	8	11	0	19	0	0	0	0	0	0	0	23	9	0	0	32	0	12	5	24	0	41	92
11:15 AM	0	1	6	20	0	27	0	1	0	0	2	1	0	25	5	1	0	31	0	26	3	23	0	52	111
11:30 AM	0	0	5	24	0	29	0	1	1	1	0	3	0	26	4	3	0	33	0	23	2	29	0	54	119
11:45 AM	0	0	5	19	0	24	0	0	1	0	0	1	0	30	9	3	0	42	0	14	5	32	0	51	118
Peak Hour Total	0	1	24	74	0	99	0	2	2	1	2	5	0	104	27	7	0	138	0	75	15	108	0	198	440
PHF	0.000	0.250	0.750	0.771	0.000	0.853	0.000	0.500	0.500	0.250	0.250	0.417	0.000	0.867	0.750	0.583	0.000	0.821	0.000	0.721	0.750	0.844	0.000	0.917	0.924

PM Peak Hour

Time	Southbound						Westbound						Northbound						Eastbound						VEHICLE TOTAL
	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	U Turns	Left Turns	Straight Through	Right Turns	Crosswalk Crossings	Vehicle Approach Total	
12:45 PM	0	1	7	32	0	40	0	0	1	0	0	1	0	30	13	1	0	44	0	22	5	31	0	58	143
1:00 PM	0	0	7	36	1	43	0	0	5	0	1	5	0	41	13	4	0	58	0	25	1	34	0	60	166
1:15 PM	0	0	1	17	0	18	0	1	7	0	0	8	0	31	6	0	0	37	0	23	4	31	0	58	121
1:30 PM	0	0	7	26	0	33	0	3	1	0	0	4	0	32	11	1	0	44	0	12	5	37	0	54	135
Peak Hour Total	0	1	22	111	1	134	0	4	14	0	1	18	0	134	43	6	0	183	0	82	15	133	0	230	565
PHF	0.000	0.250	0.786	0.771	0.250	0.779	0.000	0.333	0.500	0.000	0.250	0.563	0.000	0.817	0.827	0.375	0.000	0.789	0.000	0.820	0.750	0.899	0.000	0.958	0.851

Total Vehicles On Leg		2088			
Vehicles Entering Intersection		1053	Vehicles Exiting Intersection		1035
Southbound					
Cars	791	236	6	0	2
Heavy	18	2	0	0	0
Total	809	238	6	0	2



Total Vehicles on Leg 4394	Vehicles Entering Intersection 2175	Eastbound	Cars	Heavy	Total
			1	1	2
	0		0	0	
	716		14	730	
	129		3	132	
	Vehicles Exiting Intersection 2219		1295	18	1313



Daily Volumes

Cars	Heavy	Total	Westbound	Vehicles Entering Intersection 161	Total Vehicles on Leg 343
8	0	8			
109	1	110			
33	10	43			
0	0	0			
5	0	5		Vehicles Exiting Intersection 182	



Cars	0	0	1259	296	44	
Heavy	0	0	41	1	0	
Total	0	0	1300	297	44	
Northbound						
Vehicles Entering Intersection			1641	Vehicles Exiting Intersection		1594
Total Vehicles On Leg			3235			



Appendix B

Crash Analysis

Intersection Safety Screening

Intersection: US 59 and Susan Dr, Marshall, MN

Statewide Averages based on 2016-2020 crashes

Crashes by Crash Severity	
Fatal (K)	0
Serious Injury (A)	0
Minor Injury (B)	2
Possible Injury (C)	2
Property Damage (PDO)	5
Total Crashes	9

Intersection Characteristics	
Entering Volume	6,342
Environment	Urban
Lighting	Lit
Traffic Control	Signal

Annual crash cost = \$153,000

Statewide comparison = Signal, Low Volume (<=20K)

Total Crash Rate	
Observed	0.777
Statewide Average	0.508
Critical Rate	1.090
Critical Index	0.71

Fatal & Serious Injury Crash Rate	
Observed	0.000
Statewide Average	0.690
Critical Rate	8.130
Critical Index	0.00

The observed crash rate is the number of crashes per million entering vehicles (MEV). The critical rate is a statistical comparison based on similar intersections statewide. An observed crash rate greater than the critical rate indicates that the intersection operates outside the expected, normal range. The critical index reports the magnitude of this difference (i.e. observed crash rate ÷ critical crash rate).

The observed total crash rate for this period is 0.78 per MEV; this is 29% below the critical rate. Based on similar statewide intersections, an additional 4 crashes over the five years would indicate this intersection operates outside the normal range.

The observed fatal and serious injury crash rate for this period is 0.00 per 100 MEV; this is 100% below the critical rate. The intersection operates within the normal range.

Intersection Safety Screening

Intersection: Susan Dr and Margaret Ave, Marshall, MN

Statewide Averages based on 2016-2020 crashes

Crashes by Crash Severity	
Fatal (K)	0
Serious Injury (A)	0
Minor Injury (B)	0
Possible Injury (C)	0
Property Damage (PDO)	5
Total Crashes	5

Intersection Characteristics	
Entering Volume	2,767
Environment	Urban
Lighting	Lit
Traffic Control	Thru-Stop

Annual crash cost = \$13,000

Statewide comparison = Urban, Thru/STOP

Total Crash Rate	
Observed	0.989
Statewide Average	0.128
Critical Rate	0.640
Critical Index	1.55

Fatal & Serious Injury Crash Rate	
Observed	0.000
Statewide Average	0.311
Critical Rate	13.380
Critical Index	0.00

The observed crash rate is the number of crashes per million entering vehicles (MEV). The critical rate is a statistical comparison based on similar intersections statewide. An observed crash rate greater than the critical rate indicates that the intersection operates outside the expected, normal range. The critical index reports the magnitude of this difference (i.e. observed crash rate ÷ critical crash rate).

The observed total crash rate for this period is 0.99 per MEV; this is 1.6 times the critical rate. If crashes were reduced by 2 over five years, this intersection would perform within normal range.

The observed fatal and serious injury crash rate for this period is 0.00 per 100 MEV; this is 100% below the critical rate. The intersection operates within the normal range.

Appendix C

Warrant Analysis

SIGNAL WARRANTS ANALYSIS

2023 Volumes

Minor Rights Excluded

LOCATION:
 COUNTY: LYON
 REF. POINT:
 DATE: 1/17/2024
 OPERATOR: KMH

Speed	Approach Description	Lanes
30	Major App1: NB Margaret Ave	2
30	Major App3: SB Susan Dr	2
30	Minor App2: EB Susan Dr	1
30	Minor App4: WB Harbor Freight Entrance	2

0.70 FACTOR USED?

No

POPULATION < 10,000?

No

EXISTING SIGNAL ?

No

THRESHOLDS 1A/1B:

600/900

150/75

200/100

HOUR	MAJOR APP. 1	MAJOR APP. 3	TOTAL 1+3	MAJOR 1A/1B	MINOR APP. 2	MINOR 2 1A/1B	MINOR APP. 4	MINOR 4 1A/1B	MET SAME 1A/1B
0:00 - 1:00	0	0	0	/	0	/	0	/	/
1:00 - 2:00	0	0	0	/	0	/	0	/	/
2:00 - 3:00	0	0	0	/	0	/	0	/	/
3:00 - 4:00	0	0	0	/	0	/	0	/	/
4:00 - 5:00	0	0	0	/	0	/	0	/	/
5:00 - 6:00	0	0	0	/	0	/	0	/	/
6:00 - 7:00	22	11	33	/	9	/	2	/	/
7:00 - 8:00	36	25	61	/	26	/	2	/	/
8:00 - 9:00	62	41	103	/	34	/	7	/	/
9:00 - 10:00	90	47	137	/	47	/	11	/	/
10:00 - 11:00	116	57	173	/	71	/	9	/	/
11:00 - 12:00	138	99	237	/	90	/X	4	/	/
12:00 - 13:00	151	131	282	/	112	/X	11	/	/
13:00 - 14:00	186	122	308	/	87	/X	18	/	/
14:00 - 15:00	163	118	281	/	82	/X	19	/	/
15:00 - 16:00	170	85	255	/	81	/X	16	/	/
16:00 - 17:00	180	127	307	/	82	/X	15	/	/
17:00 - 18:00	181	96	277	/	81	/X	18	/	/
18:00 - 19:00	146	94	240	/	60	/	21	/	/
19:00 - 20:00	0	0	0	/	0	/	0	/	/
20:00 - 21:00	0	0	0	/	0	/	0	/	/
21:00 - 22:00	0	0	0	/	0	/	0	/	/
22:00 - 23:00	0	0	0	/	0	/	0	/	/
23:00 - 24:00	0	0	0	/	0	/	0	/	/

	Met (Hr)	Required (Hr)	
Warrant 1A	0	8	Not satisfied
Warrant 1B	0	8	Not satisfied
Warrant 2	0	4	Not satisfied
Warrant 3	0	1	Not satisfied
Warrant 7	0	8	Not satisfied

LOCATION:
 COUNTY: LYON

REF. POINT:
 DATE: 1/17/2024

OPERATOR: KMH

Speed	Approach Description	Lanes
30	Major App1: NB Margaret Ave	2
30	Major App3: SB Susan Dr	2
30	Minor App2: EB Susan Dr	1
30	Minor App4: WB Harbor Freight Entrance	2

0.70 FACTOR USED? No
 POPULATION < 10,000? No
 EXISTING SIGNAL ? No

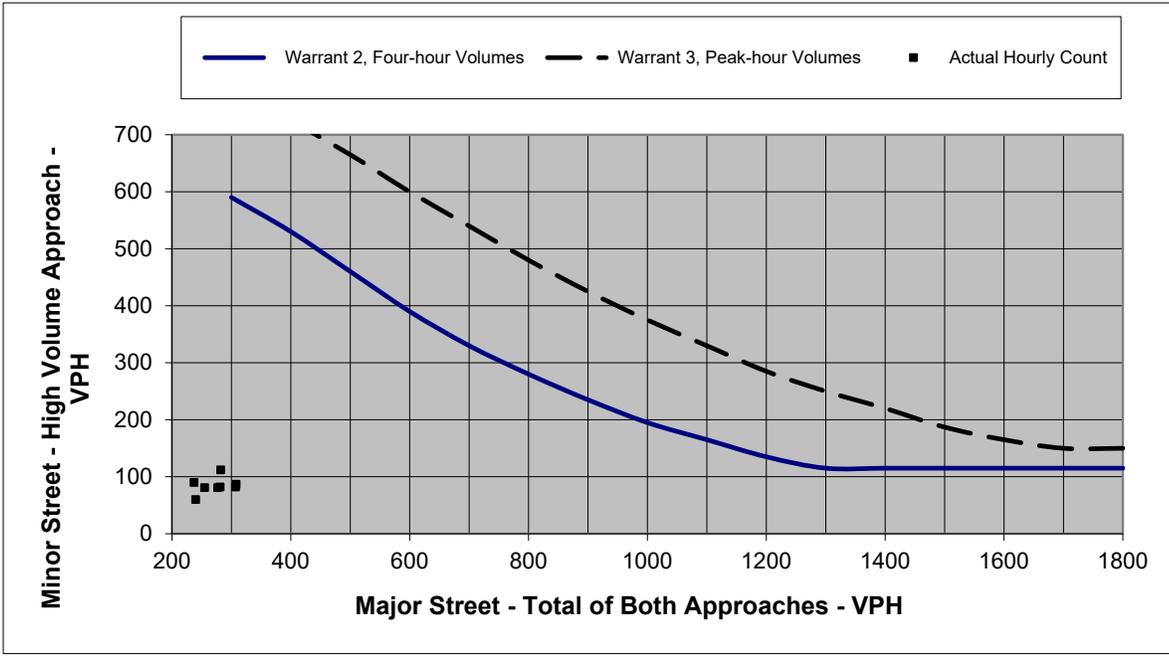


Figure 1. Four Hour and Peak Hour Warrant Analysis

Note: For data points outside the graph range, check the minor street volume against the lower thresholds

Major	Warrant Criteria		Actual Hourly Count	
	Warrant 2, F	Warrant 3, Pe	Major	Actual Hourly Count
200			0	0
300	590		0	0
400	530	725	0	0
500	460	665	0	0
600	390	600	0	0
700	330	540	0	0
800	280	480	33	9
900	235	425	61	26
1000	195	375	103	34
1100	165	330	137	47
1200	135	285	173	71
1300	115	250	237	90
1400	115	220	282	112
1500	115	187	308	87
1600	115	165	281	82
1700	115	150	255	81
1800	115	150	307	82
			277	81
			240	60
			0	0
			0	0
			0	0
			0	0
			0	0
			0	0

ALL WAY STOP WARRANT

2023 Volumes

LOCATION:

COUNTY: LYON

REF. POINT:

DATE: 1/17/2024

OPERATOR: KMH

0.70 FACTOR USED? No

Speed	Approach Description	Lanes
30	Major App1: NB Margaret Ave	2
30	Major App3: SB Susan Dr	1
30	Minor App2: EB Susan Dr	1
30	Minor App4: WB Harbor Freight Entrance	2

300

200

HOUR	MAJOR APP. 1	MAJOR APP. 3	MINOR APP. 2	MINOR APP. 4	MAJOR TOTAL Σ (APP. 1 & APP. 3)	MINOR TOTAL APP. 2 + APP. 4	WARRANT MET
0:00 - 1:00	0	0	0	0	0	0	/
1:00 - 2:00	0	0	0	0	0	0	/
2:00 - 3:00	0	0	0	0	0	0	/
3:00 - 4:00	0	0	0	0	0	0	/
4:00 - 5:00	0	0	0	0	0	0	/
5:00 - 6:00	0	0	0	0	0	0	/
6:00 - 7:00	22	11	35	2	33	37	/
7:00 - 8:00	36	25	60	2	61	62	/
8:00 - 9:00	62	41	103	9	103	112	/
9:00 - 10:00	90	47	137	12	137	149	/
10:00 - 11:00	116	57	198	10	173	208	/X
11:00 - 12:00	138	99	198	5	237	203	/X
12:00 - 13:00	151	131	235	12	282	247	/X
13:00 - 14:00	186	122	217	19	308	236	X/X
14:00 - 15:00	163	118	194	19	281	213	/X
15:00 - 16:00	170	85	197	16	255	213	/X
16:00 - 17:00	180	127	229	15	307	244	X/X
17:00 - 18:00	181	96	200	18	277	218	/X
18:00 - 19:00	146	94	172	22	240	194	/
19:00 - 20:00	0	0	0	0	0	0	/
20:00 - 21:00	0	0	0	0	0	0	/
21:00 - 22:00	0	0	0	0	0	0	/
22:00 - 23:00	0	0	0	0	0	0	/
23:00 - 24:00	0	0	0	0	0	0	/

Met (Hr) Required (Hr)

Allway Stop Warrant: 2 8 Not satisfied

REMARKS: _____

Appendix D

Traffic Operations Analysis

HCM 6th Signalized Intersection Summary

3: US Highway 59 & Susan Drive

11/24/2023

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	64	18	13	21	9	201	13	287	17	200	288	87
Future Volume (veh/h)	64	18	13	21	9	201	13	287	17	200	288	87
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	68	24	20	32	12	236	20	354	32	244	351	96
Peak Hour Factor	0.94	0.75	0.65	0.66	0.75	0.85	0.65	0.81	0.53	0.82	0.82	0.91
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	408	564	478	471	564	478	713	1541	687	502	937	418
Arrive On Green	0.30	0.30	0.30	0.30	0.30	0.30	0.26	0.43	0.43	0.09	0.26	0.26
Sat Flow, veh/h	1132	1870	1585	1362	1870	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	68	24	20	32	12	236	20	354	32	244	351	96
Grp Sat Flow(s),veh/h/ln	1132	1870	1585	1362	1870	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	4.5	0.9	0.9	1.7	0.5	12.2	0.5	6.3	1.2	9.4	8.1	4.8
Cycle Q Clear(g_c), s	4.9	0.9	0.9	2.6	0.5	12.2	0.5	6.3	1.2	9.4	8.1	4.8
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	408	564	478	471	564	478	713	1541	687	502	937	418
V/C Ratio(X)	0.17	0.04	0.04	0.07	0.02	0.49	0.03	0.23	0.05	0.49	0.37	0.23
Avail Cap(c_a), veh/h	408	564	478	471	564	478	713	1541	687	502	937	418
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	26.3	24.7	24.7	25.6	24.6	28.7	10.9	17.8	16.4	24.0	30.1	28.9
Incr Delay (d2), s/veh	0.9	0.1	0.2	0.3	0.1	3.6	0.1	0.3	0.1	3.3	1.1	1.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.3	0.4	0.4	0.6	0.2	5.1	0.2	2.5	0.4	4.6	3.5	1.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	27.2	24.9	24.9	25.9	24.6	32.3	11.0	18.2	16.5	27.3	31.3	30.2
LnGrp LOS	C	C	C	C	C	C	B	B	B	C	C	C
Approach Vol, veh/h		112			280			406			691	
Approach Delay, s/veh		26.3			31.2			17.7			29.7	
Approach LOS		C			C			B			C	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	15.0	49.0		36.1	32.0	32.0		36.1				
Change Period (Y+Rc), s	* 5.6	* 5.6		5.9	* 5.7	* 5.6		* 5.9				
Max Green Setting (Gmax), s	* 9.4	* 43		30.1	* 26	* 26		* 30				
Max Q Clear Time (g_c+I1), s	11.4	8.3		6.9	2.5	10.1		14.2				
Green Ext Time (p_c), s	0.0	2.4		0.4	0.0	2.2		0.8				
Intersection Summary												
HCM 6th Ctrl Delay				26.5								
HCM 6th LOS				C								
Notes												
* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.												

HCM 6th TWSC PM Peak Existing Condition
5: Margaret Ave & Susan Drive

11/24/2023

Intersection										
Int Delay, s/veh	7.9									
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR	NWL	NWR
Lane Configurations		↖	↗		↖↗		↘	↗	↖	↘
Traffic Vol, veh/h	82	15	133	4	14	0	22	111	134	43
Future Vol, veh/h	82	15	133	4	14	0	22	111	134	43
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop
RT Channelized	-	-	Yield	-	-	None	-	None	-	-
Storage Length	-	-	60	-	-	-	150	-	150	-
Veh in Median Storage, #	-	0	-	-	0	-	0	-	0	-
Grade, %	-	0	-	-	0	-	0	-	0	-
Peak Hour Factor	82	75	90	33	50	25	79	77	82	83
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	100	20	148	12	28	0	28	144	163	52

Major/Minor	Major1		Major2		Minor2		Minor1			
Conflicting Flow All	28	0	0	20	0	0	306	14	272	20
Stage 1	-	-	-	-	-	-	52	-	220	-
Stage 2	-	-	-	-	-	-	254	-	52	-
Critical Hdwy	4.13	-	-	4.13	-	-	7.33	6.93	7.33	6.23
Critical Hdwy Stg 1	-	-	-	-	-	-	6.53	-	6.13	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.13	-	6.53	-
Follow-up Hdwy	2.219	-	-	2.219	-	-	3.519	3.319	3.519	3.319
Pot Cap-1 Maneuver	1585	-	-	1595	-	-	635	1063	670	1057
Stage 1	-	-	-	-	-	-	955	-	782	-
Stage 2	-	-	-	-	-	-	750	-	955	-
Platoon blocked, %		-	-		-	-				
Mov Cap-1 Maneuver	1585	-	-	1595	-	-	549	1063	525	1057
Mov Cap-2 Maneuver	-	-	-	-	-	-	549	-	525	-
Stage 1	-	-	-	-	-	-	887	-	726	-
Stage 2	-	-	-	-	-	-	633	-	792	-

Approach	EB		WB		SB		NW	
HCM Control Delay, s	2.8		2.2		9.4		13.8	
HCM LOS					A		B	

Minor Lane/Major Mvmt	NWLn1	NWLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	525	653	1585	-	-	1595	-	-	580	1063
HCM Lane V/C Ratio	0.311	0.104	0.063	-	-	0.008	-	-	0.055	0.136
HCM Control Delay (s)	14.9	11.1	7.4	0	-	7.3	0	-	11.6	8.9
HCM Lane LOS	B	B	A	A	-	A	A	-	B	A
HCM 95th %tile Q(veh)	1.3	0.3	0.2	-	-	0	-	-	0.2	0.5

HCM 6th TWSC AM Peak Existing Condition
5: Margaret Ave & Susan Drive

11/24/2023

Intersection											
Int Delay, s/veh	6.9										
Movement	EBL	EBT	EBR	WBL	WBT	WBR	SBL	SBR	NWL	NWR	
Lane Configurations		↖	↗		↖↗		↘	↗	↖	↘	
Traffic Vol, veh/h	75	15	108	2	2	1	24	74	104	27	
Future Vol, veh/h	75	15	108	2	2	1	24	74	104	27	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	
RT Channelized	-	-	Yield	-	-	None	-	None	-	-	
Storage Length	-	-	60	-	-	-	150	-	150	-	
Veh in Median Storage, #	-	0	-	-	0	-	0	-	0	-	
Grade, %	-	0	-	-	0	-	0	-	0	-	
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	82	16	117	2	2	1	26	80	113	29	

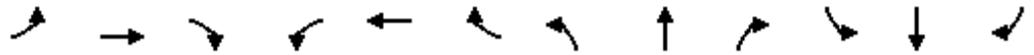
Major/Minor	Major1			Major2			Minor2		Minor1	
Conflicting Flow All	3	0	0	16	0	0	206	2	198	16
Stage 1	-	-	-	-	-	-	7	-	180	-
Stage 2	-	-	-	-	-	-	199	-	18	-
Critical Hdwy	4.13	-	-	4.13	-	-	7.33	6.93	7.33	6.23
Critical Hdwy Stg 1	-	-	-	-	-	-	6.53	-	6.13	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.13	-	6.53	-
Follow-up Hdwy	2.219	-	-	2.219	-	-	3.519	3.319	3.519	3.319
Pot Cap-1 Maneuver	1618	-	-	1601	-	-	743	1081	752	1063
Stage 1	-	-	-	-	-	-	1014	-	821	-
Stage 2	-	-	-	-	-	-	802	-	999	-
Platoon blocked, %		-	-		-	-				
Mov Cap-1 Maneuver	1618	-	-	1601	-	-	682	1081	646	1063
Mov Cap-2 Maneuver	-	-	-	-	-	-	682	-	646	-
Stage 1	-	-	-	-	-	-	957	-	775	-
Stage 2	-	-	-	-	-	-	721	-	897	-

Approach	EB			WB			SB		NW	
HCM Control Delay, s	2.8			2.9			9.1		11.4	
HCM LOS							A		B	

Minor Lane/Major Mvmt	NWLn1	NWLn2	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	SBLn2
Capacity (veh/h)	646	722	1618	-	-	1601	-	-	668	1081
HCM Lane V/C Ratio	0.175	0.051	0.05	-	-	0.001	-	-	0.041	0.074
HCM Control Delay (s)	11.8	10.3	7.3	0	-	7.3	0	-	10.6	8.6
HCM Lane LOS	B	B	A	A	-	A	A	-	B	A
HCM 95th %tile Q(veh)	0.6	0.2	0.2	-	-	0	-	-	0.1	0.2

HCM 6th Signalized Intersection Summary
 3: US Highway 59 & Susan Drive

AM Peak Build Condition
 01/12/2024



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	67	17	14	18	15	150	11	197	20	159	219	66
Future Volume (veh/h)	67	17	14	18	15	150	11	197	20	159	219	66
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	84	24	16	28	20	169	16	216	24	167	281	76
Peak Hour Factor	0.80	0.71	0.88	0.64	0.75	0.89	0.69	0.91	0.83	0.95	0.78	0.87
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	199	245	207	162	101	207	762	2283	1018	876	2417	1078
Arrive On Green	0.13	0.13	0.13	0.13	0.13	0.13	0.02	0.64	0.64	0.05	0.68	0.68
Sat Flow, veh/h	1194	1870	1585	800	775	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	84	24	16	48	0	169	16	216	24	167	281	76
Grp Sat Flow(s),veh/h/ln	1194	1870	1585	1576	0	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	6.8	1.1	0.9	1.1	0.0	10.4	0.3	2.3	0.5	3.1	2.7	1.6
Cycle Q Clear(g_c), s	9.2	1.1	0.9	2.5	0.0	10.4	0.3	2.3	0.5	3.1	2.7	1.6
Prop In Lane	1.00		1.00	0.58		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	199	245	207	263	0	207	762	2283	1018	876	2417	1078
V/C Ratio(X)	0.42	0.10	0.08	0.18	0.00	0.81	0.02	0.09	0.02	0.19	0.12	0.07
Avail Cap(c_a), veh/h	403	565	479	525	0	477	897	2283	1018	1246	2417	1078
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	43.0	38.3	38.2	38.8	0.0	42.3	5.8	6.8	6.5	5.1	5.6	5.4
Incr Delay (d2), s/veh	1.4	0.2	0.2	0.3	0.0	7.6	0.0	0.1	0.0	0.1	0.1	0.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.1	0.5	0.4	1.1	0.0	4.5	0.1	0.8	0.2	0.9	0.9	0.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	44.4	38.4	38.3	39.1	0.0	49.8	5.8	6.9	6.5	5.2	5.7	5.5
LnGrp LOS	D	D	D	D	A	D	A	A	A	A	A	A
Approach Vol, veh/h		124			217			256			524	
Approach Delay, s/veh		42.5			47.5			6.8			5.5	
Approach LOS		D			D			A			A	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	7.4	73.6		19.0	11.2	69.8		19.0				
Change Period (Y+Rc), s	* 5.6	* 5.6		5.9	* 5.7	* 5.6		* 5.9				
Max Green Setting (Gmax), s	* 9.4	* 43		30.1	* 26	* 26		* 30				
Max Q Clear Time (g_c+I1), s	2.3	4.7		12.4	5.1	4.3		11.2				
Green Ext Time (p_c), s	0.0	2.0		0.7	0.4	1.3		0.4				

Intersection Summary

HCM 6th Ctrl Delay	18.0
HCM 6th LOS	B

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

HCM 6th TWSC
5: Susan Drive & Margaret Ave

01/12/2024

Intersection						
Int Delay, s/veh	3.9					
Movement	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations	↶		↷	↶	↷	
Traffic Vol, veh/h	90	108	24	100	104	34
Future Vol, veh/h	90	108	24	100	104	34
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	100	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	98	117	26	109	113	37

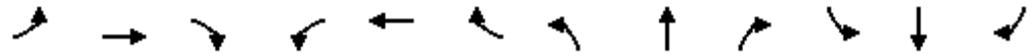
Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	215	0	318	157
Stage 1	-	-	-	-	157	-
Stage 2	-	-	-	-	161	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1355	-	675	889
Stage 1	-	-	-	-	871	-
Stage 2	-	-	-	-	868	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1355	-	662	889
Mov Cap-2 Maneuver	-	-	-	-	662	-
Stage 1	-	-	-	-	871	-
Stage 2	-	-	-	-	852	-

Approach	NB	SB	NW
HCM Control Delay, s	0	1.5	11.5
HCM LOS			B

Minor Lane/Major Mvmt	NBT	NBRNWLn1	SBL	SBT
Capacity (veh/h)	-	-	706	1355
HCM Lane V/C Ratio	-	-	0.212	0.019
HCM Control Delay (s)	-	-	11.5	7.7
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.8	0.1

HCM 6th Signalized Intersection Summary
 3: US Highway 59 & Susan Drive

PM Peak Build Condition
 01/12/2024



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	64	18	13	21	9	201	13	287	17	200	288	87
Future Volume (veh/h)	64	18	13	21	9	201	13	287	17	200	288	87
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	68	24	20	32	12	236	20	354	32	244	351	96
Peak Hour Factor	0.94	0.75	0.65	0.66	0.75	0.85	0.65	0.81	0.53	0.82	0.82	0.91
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	238	324	274	236	79	274	654	2051	915	740	2255	1006
Arrive On Green	0.17	0.17	0.17	0.17	0.17	0.17	0.02	0.58	0.58	0.08	0.63	0.63
Sat Flow, veh/h	1132	1870	1585	1004	457	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	68	24	20	44	0	236	20	354	32	244	351	96
Grp Sat Flow(s),veh/h/ln	1132	1870	1585	1461	0	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	5.5	1.1	1.1	1.6	0.0	14.5	0.5	4.7	0.9	5.2	4.0	2.4
Cycle Q Clear(g_c), s	8.1	1.1	1.1	2.7	0.0	14.5	0.5	4.7	0.9	5.2	4.0	2.4
Prop In Lane	1.00		1.00	0.73		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	238	324	274	315	0	274	654	2051	915	740	2255	1006
V/C Ratio(X)	0.29	0.07	0.07	0.14	0.00	0.86	0.03	0.17	0.03	0.33	0.16	0.10
Avail Cap(c_a), veh/h	384	565	479	503	0	477	784	2051	915	1069	2255	1006
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	38.8	34.6	34.6	35.3	0.0	40.2	8.2	9.9	9.1	6.4	7.4	7.1
Incr Delay (d2), s/veh	0.7	0.1	0.1	0.2	0.0	7.8	0.0	0.2	0.1	0.3	0.1	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.5	0.5	0.4	0.9	0.0	6.2	0.2	1.7	0.3	1.7	1.4	0.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	39.4	34.7	34.7	35.5	0.0	47.9	8.2	10.1	9.2	6.7	7.6	7.3
LnGrp LOS	D	C	C	D	A	D	A	B	A	A	A	A
Approach Vol, veh/h		112			280			406			691	
Approach Delay, s/veh		37.6			46.0			9.9			7.2	
Approach LOS		D			D			A			A	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	7.7	69.1		23.2	13.5	63.3		23.2				
Change Period (Y+Rc), s	* 5.6	* 5.6		5.9	* 5.7	* 5.6		* 5.9				
Max Green Setting (Gmax), s	* 9.4	* 43		30.1	* 26	* 26		* 30				
Max Q Clear Time (g_c+I1), s	2.5	6.0		16.5	7.2	6.7		10.1				
Green Ext Time (p_c), s	0.0	2.6		0.8	0.6	2.1		0.4				

Intersection Summary

HCM 6th Ctrl Delay	17.5
HCM 6th LOS	B

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

HCM 6th TWSC
5: Susan Drive & Margaret Ave

01/12/2024

Intersection						
Int Delay, s/veh	4.8					
Movement	NBT	NBR	SBL	SBT	NWL	NWR
Lane Configurations						
Traffic Vol, veh/h	97	133	22	125	134	49
Future Vol, veh/h	97	133	22	125	134	49
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	100	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	82	90	79	77	82	83
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	118	148	28	162	163	59

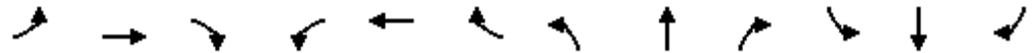
Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	266	0	410	192
Stage 1	-	-	-	-	192	-
Stage 2	-	-	-	-	218	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1298	-	598	850
Stage 1	-	-	-	-	841	-
Stage 2	-	-	-	-	818	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1298	-	585	850
Mov Cap-2 Maneuver	-	-	-	-	585	-
Stage 1	-	-	-	-	841	-
Stage 2	-	-	-	-	800	-

Approach	NB	SB	NW
HCM Control Delay, s	0	1.1	13.6
HCM LOS			B

Minor Lane/Major Mvmt	NBT	NBRNWLn1	SBL	SBT
Capacity (veh/h)	-	-	638	1298
HCM Lane V/C Ratio	-	-	0.349	0.021
HCM Control Delay (s)	-	-	13.6	7.8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	1.6	0.1

Queues AM Peak Existing Condition
3: US Highway 59

11/24/2023

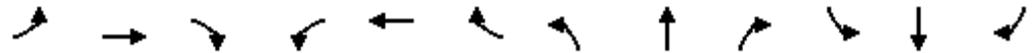


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	84	24	16	28	20	169	16	216	24	167	281	76
v/c Ratio	0.20	0.04	0.03	0.07	0.04	0.28	0.02	0.14	0.03	0.36	0.30	0.14
Control Delay	27.7	25.2	0.1	25.6	25.0	5.5	8.9	17.4	0.1	14.9	30.5	0.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	27.7	25.2	0.1	25.6	25.0	5.5	8.9	17.4	0.1	14.9	30.5	0.6
Queue Length 50th (ft)	40	11	0	13	9	0	4	42	0	46	75	0
Queue Length 95th (ft)	69	23	0	23	22	45	10	66	0	78	95	0
Internal Link Dist (ft)		78			125			1128			972	
Turn Bay Length (ft)	150		150	150		150	150		150	150		150
Base Capacity (vph)	417	560	582	417	562	596	739	1535	739	469	934	531
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.04	0.03	0.07	0.04	0.28	0.02	0.14	0.03	0.36	0.30	0.14

Intersection Summary

Queues PM Peak Existing Condition
3: US Highway 59

11/24/2023



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	68	24	20	32	12	236	20	354	32	244	351	96
v/c Ratio	0.16	0.04	0.03	0.08	0.02	0.37	0.03	0.23	0.04	0.57	0.38	0.18
Control Delay	27.0	25.2	0.1	25.7	24.8	5.3	8.9	18.3	0.1	19.6	31.5	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	27.0	25.2	0.1	25.7	24.8	5.3	8.9	18.3	0.1	19.6	31.5	1.7
Queue Length 50th (ft)	32	11	0	15	5	0	5	72	0	70	96	0
Queue Length 95th (ft)	66	25	0	27	16	44	11	92	0	99	124	9
Internal Link Dist (ft)		78			125			1128			972	
Turn Bay Length (ft)	150		150	150		150	150		150	150		150
Base Capacity (vph)	420	560	582	417	562	642	702	1535	739	431	934	531
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.16	0.04	0.03	0.08	0.02	0.37	0.03	0.23	0.04	0.57	0.38	0.18

Intersection Summary

Queues AM Peak Build Condition
 3: Susan Drive & US Highway 59

11/24/2023



Lane Group	EBL	EBT	EBR	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	84	24	16	48	169	16	216	24	167	281	76
v/c Ratio	0.21	0.04	0.03	0.10	0.28	0.02	0.14	0.03	0.36	0.30	0.14
Control Delay	27.8	25.2	0.1	25.9	5.5	8.9	17.4	0.1	14.9	30.5	0.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	27.8	25.2	0.1	25.9	5.5	8.9	17.4	0.1	14.9	30.5	0.6
Queue Length 50th (ft)	40	11	0	22	0	4	42	0	46	75	0
Queue Length 95th (ft)	69	23	0	41	45	10	66	0	78	95	0
Internal Link Dist (ft)		78		138			1128			972	
Turn Bay Length (ft)	150		150		150	150		150	150		150
Base Capacity (vph)	406	560	582	485	596	739	1535	739	469	934	531
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.21	0.04	0.03	0.10	0.28	0.02	0.14	0.03	0.36	0.30	0.14

Intersection Summary

Queues PM Peak Build Condition
3: Susan Drive & US Highway 59

11/24/2023



Lane Group	EBL	EBT	EBR	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Group Flow (vph)	68	24	20	44	236	20	354	32	244	351	96
v/c Ratio	0.17	0.04	0.03	0.09	0.37	0.03	0.23	0.04	0.57	0.38	0.18
Control Delay	27.2	25.2	0.1	25.9	5.3	8.9	18.3	0.1	19.6	31.5	1.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	27.2	25.2	0.1	25.9	5.3	8.9	18.3	0.1	19.6	31.5	1.7
Queue Length 50th (ft)	32	11	0	20	0	5	72	0	70	96	0
Queue Length 95th (ft)	66	25	0	38	44	11	92	0	99	124	9
Internal Link Dist (ft)		78		138			1128			972	
Turn Bay Length (ft)	150		150		150	150		150	150		150
Base Capacity (vph)	408	560	582	466	642	702	1535	739	431	934	531
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.17	0.04	0.03	0.09	0.37	0.03	0.23	0.04	0.57	0.38	0.18

Intersection Summary

Appendix E

Intersection Alternatives



SUSAN DR

RELOCATE LIGHT



WB-62 TRACKING TRUCK APRON



US 59

MARGARET AVE

80
SCALE IN FEET

T-Intersection
Alternative Concept

SUSAN DR



US 59

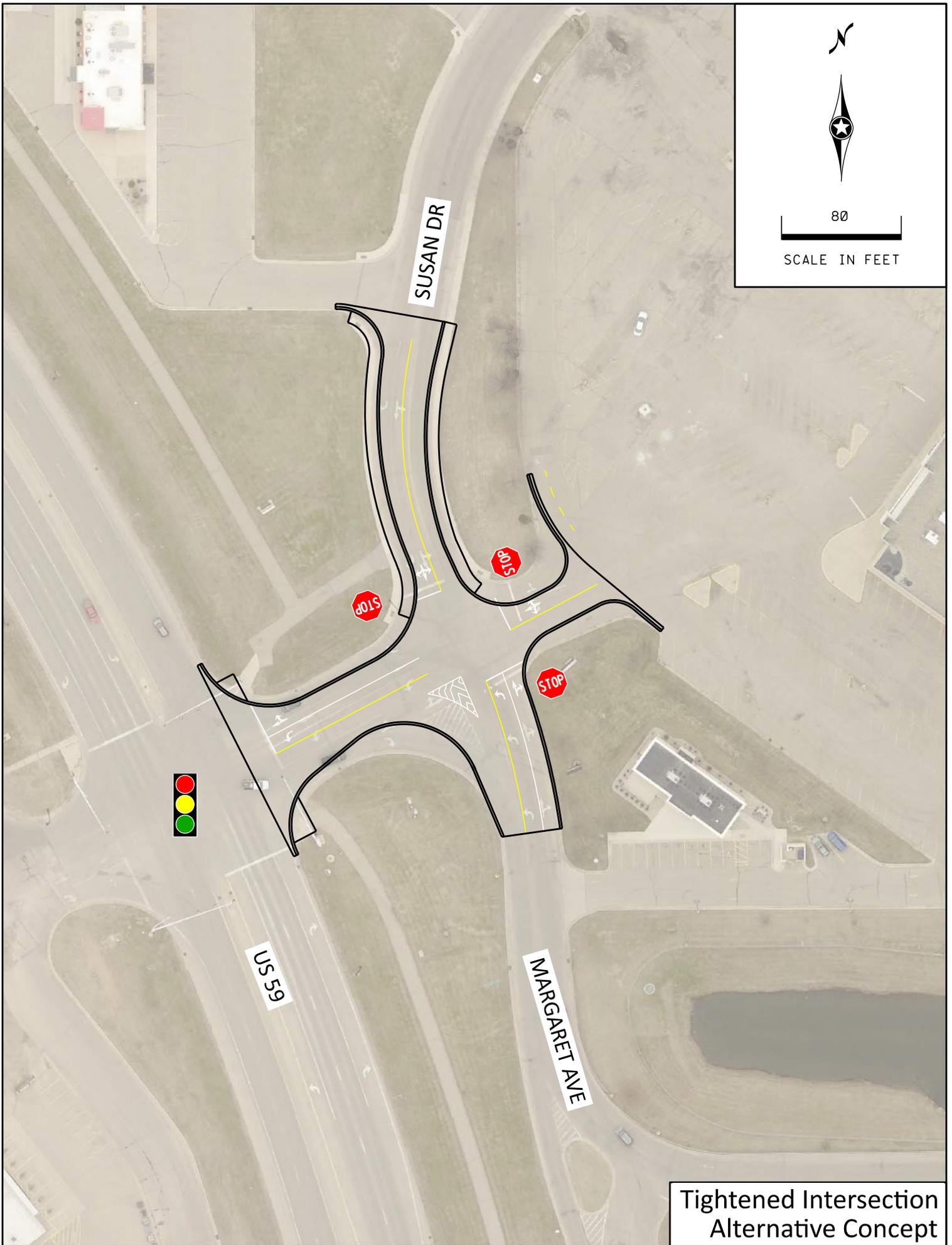
MARGARET AVE



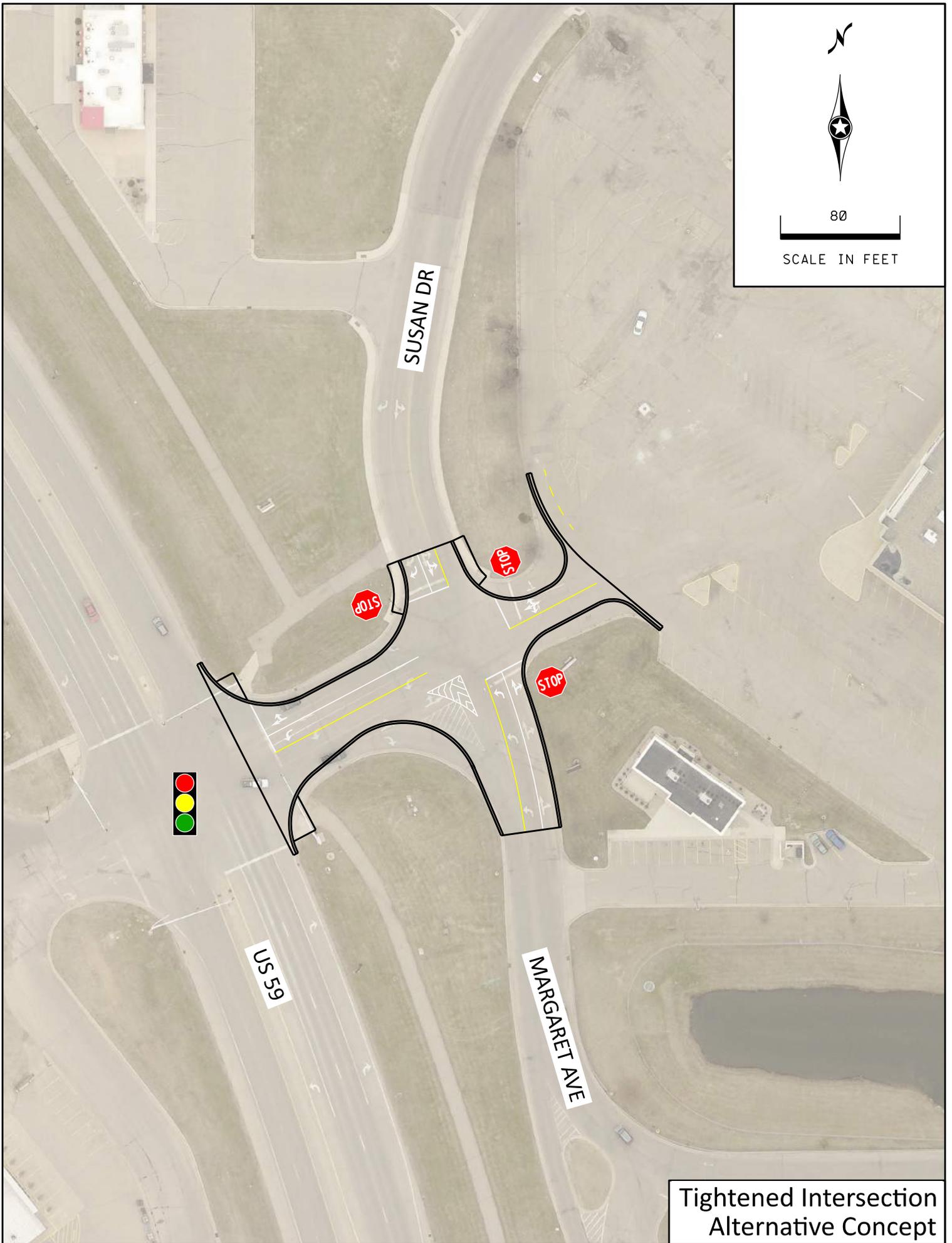
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SCALE IN FEET

Margaret Avenue Closure
Alternative Concept



Tightened Intersection
Alternative Concept



N



80



SCALE IN FEET

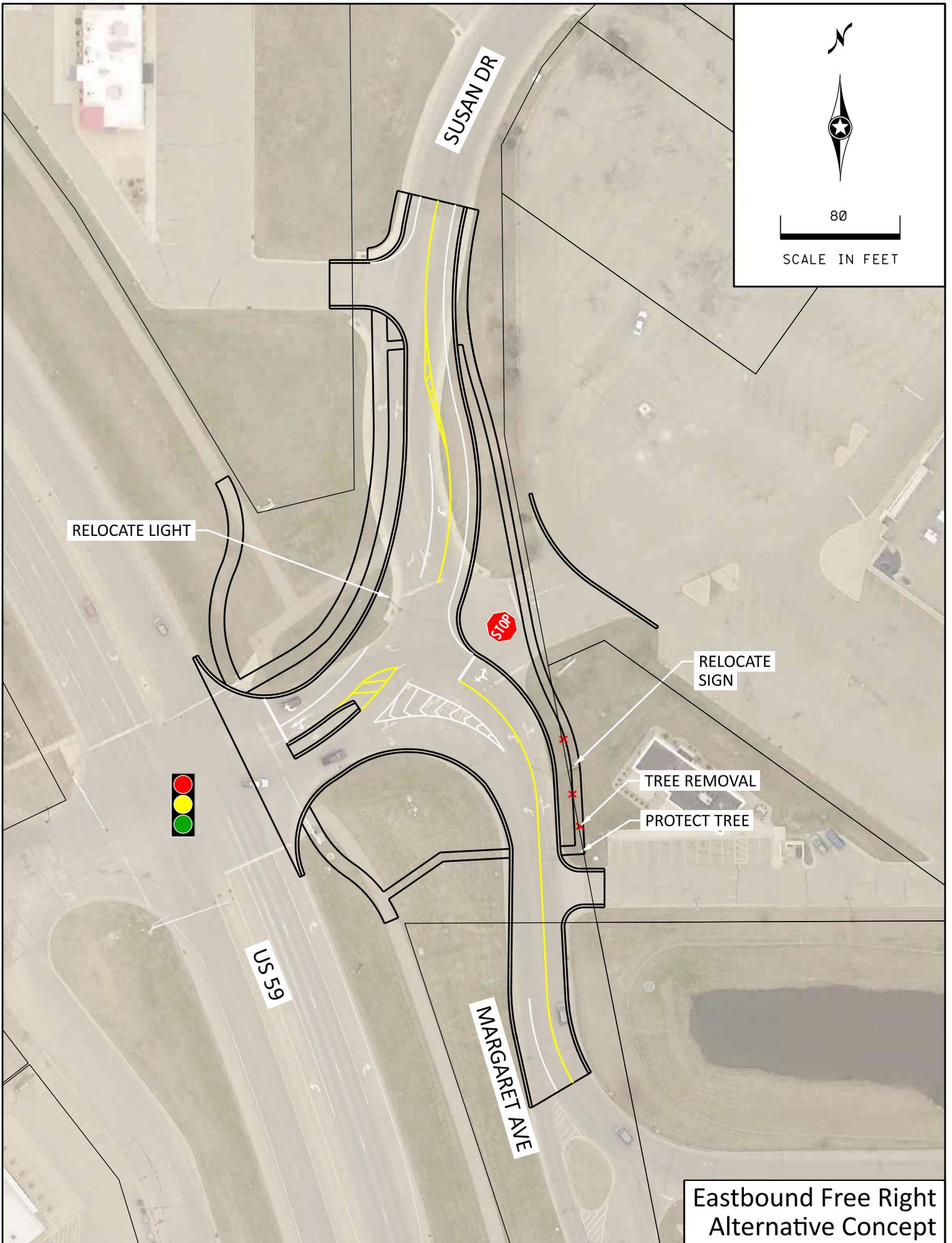
SUSAN DR

US 59

MARGARET AVE



Tightened Intersection
Alternative Concept



SUSAN DR

RELOCATE LIGHT



RELOCATE SIGN

TREE REMOVAL

PROTECT TREE

US 59

MARGARET AVE

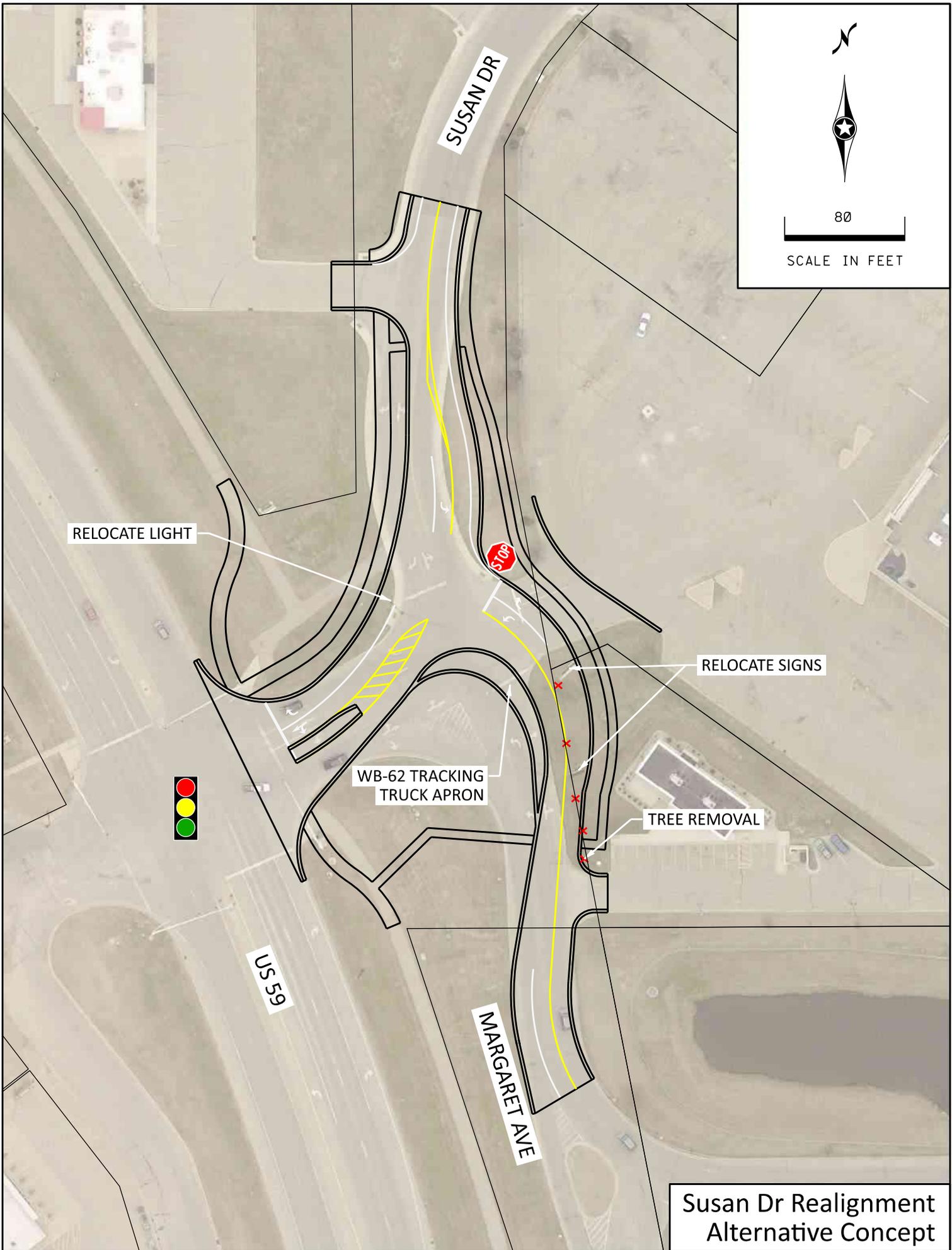
Eastbound Free Right
Alternative Concept



80

SCALE IN FEET





SUSAN DR

US 59

MARGARET AVE

RELOCATE LIGHT



WB-62 TRACKING TRUCK APRON

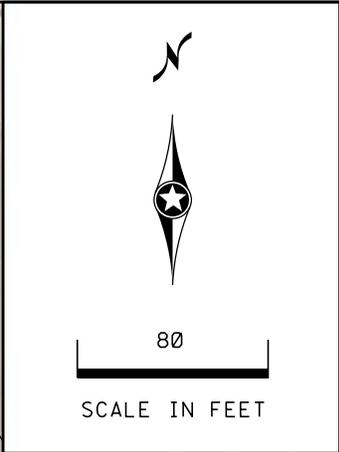
RELOCATE SIGNS

TREE REMOVAL

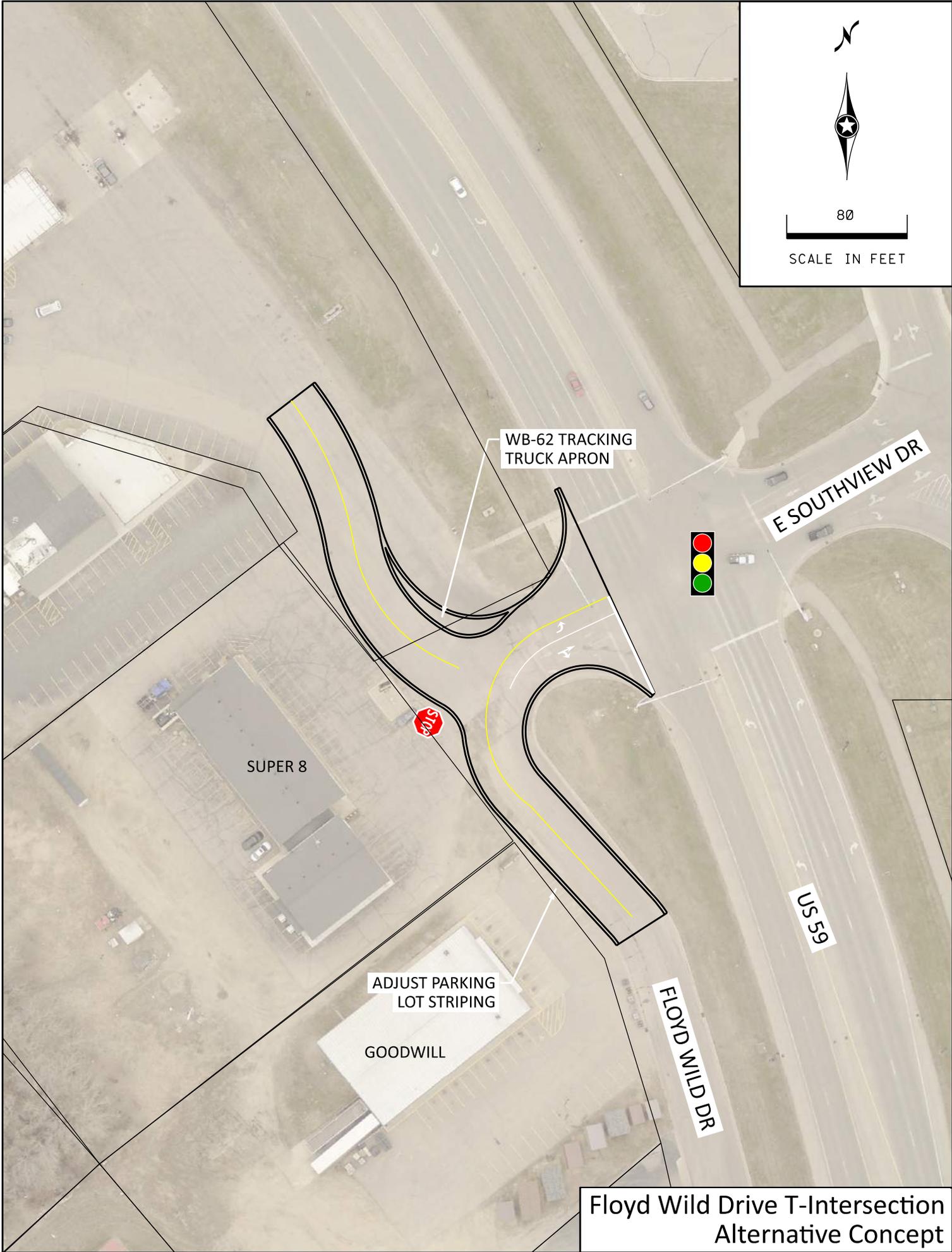
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SCALE IN FEET

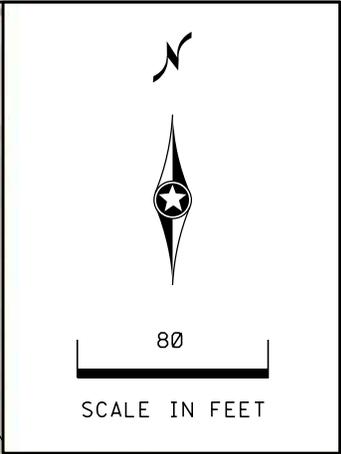
Susan Dr Realignment
Alternative Concept



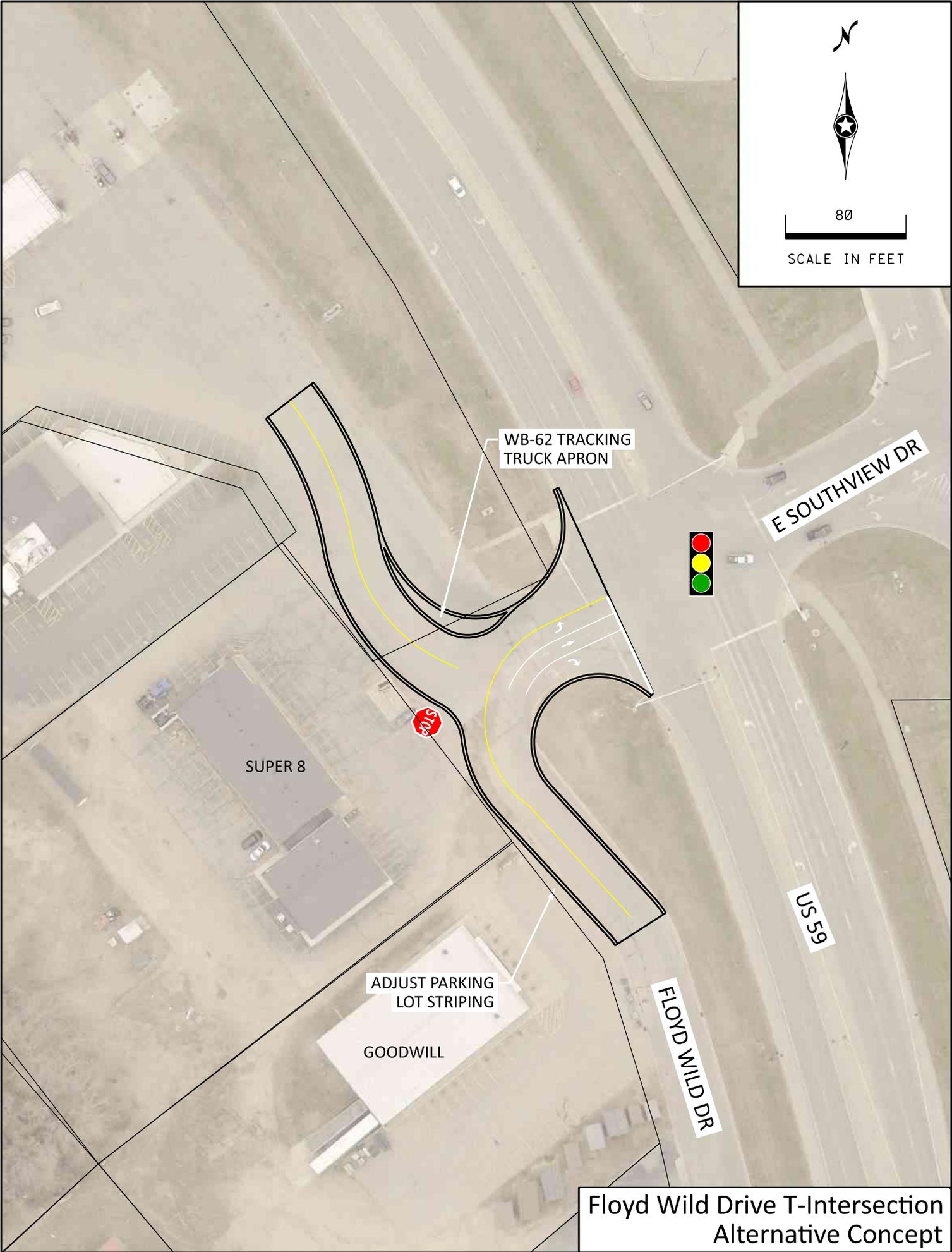
80
SCALE IN FEET



Floyd Wild Drive T-Intersection
Alternative Concept



80
SCALE IN FEET



Floyd Wild Drive T-Intersection
Alternative Concept