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# Drainage Report

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## Manor Downs Industrial

**Bowman**  
807 Las Cimas Parkway  
Building 2 Suite 350  
Austin, TX 78746  
512.327.1180 | [bowman.com](http://bowman.com)

*August 1, 2025*  
*Project No. 070422-01-002*

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## 1.0 INTRODUCTION

The proposed project is located at Hill Lane, Manor, TX, 78653. It is located within the City of Manor City Limits, in Travis County. The total project area is approximately 146.92 acres. The proposed development includes six IN-2 Heavy Industrial zoned lots and one lot to be dedicated to the City of Manor as parkland. There is public right of way dedication to the City of Manor for the Murray Dalfen Drive roadway that bisects the site. There is also public right of way dedication along the southern property line to Travis County. Associated water, wastewater, grading and drainage improvements are proposed with this project to service the development. A site Location Map is included in the appendix of this report as **Exhibit 1**.

The proposed improvements for Manor Downs Industrial are shown in the Overall Site & Dimension Plan in **Exhibit 2**.

### 1.1 ZONING

This site is located within the City of Manor City Limits and is zoned as IN-2 Heavy Industrial. This zoning allows for a maximum impervious cover of 85% and requires at least 15% of each lot to be landscaped. Six out of the seven proposed lots will have industrial buildings with the seventh lot being parkland dedication to the City of Manor.

### 1.2 EXISTING CONDITIONS

The existing tract is predominantly open grass area with some tree coverage. The site drains to the south towards Hill Lane with three distinct point of analysis, which include culverts under Hill Lane. The site contains two existing drainage easements to be released by the City of Manor with document numbers, 2000100887 and 20004163233.

## 2.0 HYDROLOGY AND HYDRAULICS

### 2.1 WATERSHED

The site is located within the Gilleland Creek Watershed and is classified as a Suburban Watershed in the ETJ. See **Exhibit 3** for the City of Austin Watershed Map. The subject site is not located within the Edwards Aquifer Recharge Zone, or the Recharge Transition Zone as defined by TCEQ or the City of Austin. Refer to **Exhibit 4** for Edwards Aquifer Map.

### 2.2 FLOOD PLAIN

No portion of the subject tract is located within a 100-year FEMA designated flood plain as shown on FEMA Map Panel No. 48453C0290J effective August 18, 2014. A copy of the FEMA Floodplain Map has been included in the Appendix of this report as **Exhibit 5**.

### 2.3 TOPOGRAPHY AND SOILS

The entire site drains to the south towards three different points of analysis, which include proposed culverts with the Hill Lane Roadway Project, COA #SP-2023-0488D. The soil on-site consists of mostly

clay soils, which correlate to Group D Hydrologic Soils. This group of soils has slow infiltration rates during storm events. See **Exhibit 6** for the soil map and soil descriptions from Natural Resources Conservation Service.

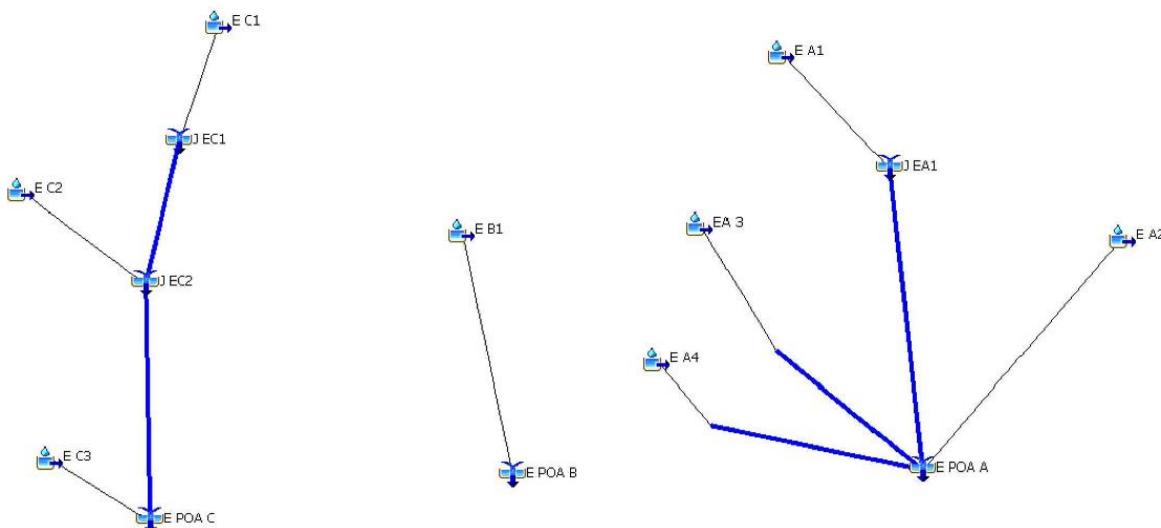
## 2.4 EXISTING DRAINAGE AREA MAP

The Manor Downs Industrial project is proposed in two phases. Phase 1 includes the subdivision construction plans for the public roadway and public Manville waterline as well as Site Plan A, which contains proposed industrial buildings 3-6. Phase 2 includes Site Plan B, which includes Industrial Buildings 1 and 2. Therefore, there are two different proposed drainage conditions for this project: intermediate conditions for the Subdivision and Site Plan A and ultimate conditions which include the Subdivision, Site Plan A, and Site Plan B.

The following rainfall data from the City of Austin Drainage Criteria Manual was used for HEC-HMS.

The site resides North of the Colorado River, which is in Zone 2, according to the City of Austin Study Area Map. The precipitation factor in Zone 2 for the 24-hr duration is 4.06 inches with a curve number of 84 for fair condition (grass cover 50% to 75%). Upstream, offsite areas are considered to be fully developed per COA DCM 25-7-8. See **Exhibit 7** for the existing drainage area map for the ultimate conditions. The intermediate conditions existing drainage area map contains the same delineations without POA C since no improvements are proposed that affect that POA. The pre-development conditions consist of eight basins that total 355.2 acres, which drain to three different POA's. POA C is an 18" and 24" corrugated metal pipe, CMP, POA B is a 36" and 42" CMP, and POA C is two 30" CMP. CMPs drain under the existing Hill Lane Roadway towards the south. The table below shows the existing flows at each points of analysis.

Existing Conditions - HEC-HMS Model & Results:



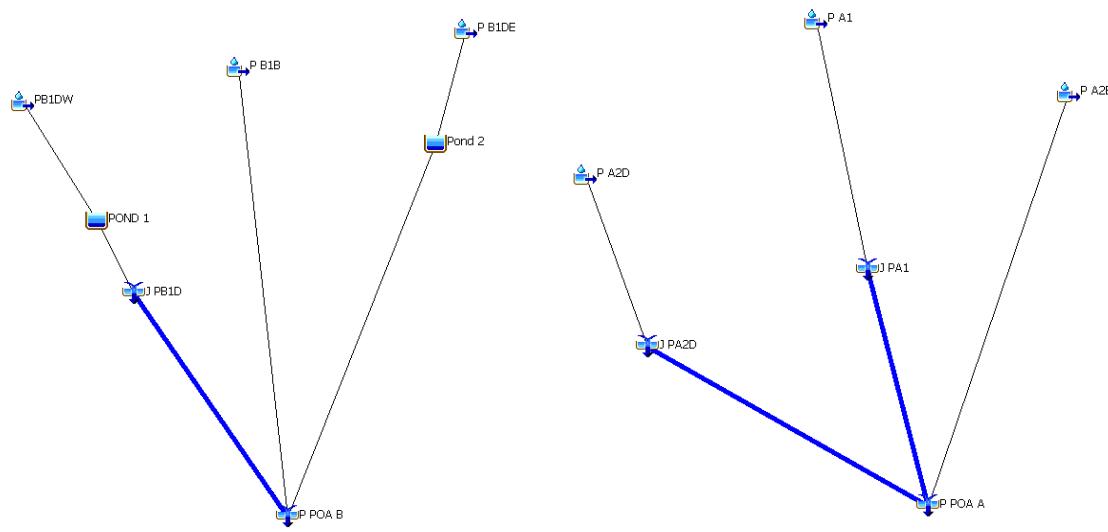
BASIN	Existing Development Conditions - SCS Method - Via HEC-HMS											
	AREA			CN	IMPERVIOUS COVER		LAG min	Q (CFS)				
	sf	ac	sq mi		sf	%		2-yr	5-yr	10-yr	25-yr	100-yr
EA1	2,283,711	52.4	0.0819	84	1,428,166	62.5%	27.1	98.8	130.0	157.9	198.7	267.6
EA2	4,696,199	107.8	0.1685	84	2,456,560	52.3%	40.4	159.2	212.7	260.8	331.8	452.9
EA3	1,761,118	40.4	0.0632	84	7,433		15.2	71.1	99.7	125.2	161.9	224.0
EA4	703,689	16.2	0.0252	84	6,356	0.9%	66.7	14.2	20.5	26.4	35.1	50.3
Reach EA1							6.6	93.4	122.7	149.0	187.5	252.7
Reach EA3							1.0	70.9	99.5	125.0	161.8	222.7
Reach EA4								14.1	20.4	26.3	35.0	50.1
Junction EA1								98.8	130.0	157.9	198.7	267.6
E-POA A	<b>9,444,717</b>	<b>216.8</b>	<b>0.3388</b>					<b>305.0</b>	<b>410.0</b>	<b>504.8</b>	<b>644.5</b>	<b>883.3</b>
EB1	1,146,885	26.3	0.0124	84	85,459	7.5%	13.6	49.3	69.6	87.9	114.3	158.1
E-POA B	<b>1,146,885</b>	<b>26.3</b>	<b>0.0124</b>					<b>49.3</b>	<b>69.6</b>	<b>87.9</b>	<b>114.3</b>	<b>158.1</b>
					-							
EC1	2,787,609	64.0	0.1000	84	1,811,946	65.0%	28.6	116.9	153.6	186.4	234.5	315.9
EC2	403,953	9.3	0.0145	84	196,647	48.7%	13.5	20.7	27.6	33.8	42.8	57.8
EC3	1,689,471	38.8	0.0606	84	84,323	5.0%	17.3	69.8	97.5	122.3	158.0	217.2
Reach EC1							0.8	116.9	153.6	186.4	234.5	315.9
Reach EC2							5.7	125.7	165.1	200.5	252.4	340.4
Junction EC1								116.9	153.6	186.4	234.5	315.9
Junction EC2								135.7	178.5	216.6	272.6	366.9
E-POA C	<b>4,881,033.1</b>	<b>112.1</b>	<b>0.2</b>					<b>191.4</b>	<b>258.1</b>	<b>317.7</b>	<b>404.8</b>	<b>551.0</b>

## 2.5 PROPOSED DRAINAGE AREA MAP

The precipitation factor in Zone 2 for the 24-hr duration is 4.06 inches with a curve number of 84 for fair condition (grass cover 50% to 75%). Upstream, offsite areas are considered to be fully developed per COA DCM 25-7-8 for both the intermediate and ultimate conditions. It is also assumed that Hill Lane Roadway has been improved by others and the new improved culverts exist at each POA.

The proposed drainage area map for the intermediate conditions was designed for the fully developed Subdivision and Site plan A. The post-development conditions consist of 6 basins totaling about 243.15 acres. Due to the complexity of the site's drainage patterns, storm drains, culverts, channels, and ponds were used to divert flow to ensure that existing flows were matched and/or decreased. See **Exhibit 8** for the proposed drainage area map that shows the intermediate proposed drainage routing for the site. The table below shows the discharge of the proposed points of analysis with a decrease in flows at POA A and B.

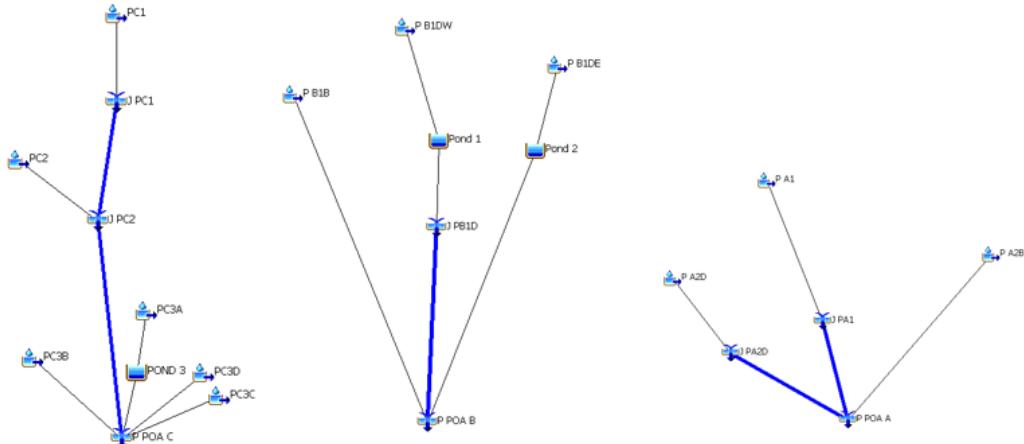
Proposed Intermediate Conditions - HEC-HMS Model & Results:



BASIN	AREA			CN	IMPERVIOUS COVER		LAG min	Q (CFS)						
	sf	ac	sq mi					2-yr	5-yr	10-yr	25-yr	100-yr		
					sf	%								
PA1	2249743	51.6	0.0807	84	1,431,765	63.6%	26.75	98.5	129.5	157.2	197.6	265.9		
PA2B	4,252,218	97.6	0.1525	84	2,376,513	55.9%	39.94	147.2	195.8	239.5	304.0	413.9		
PA2D	1,845,372	42.4	0.0662	84	1,209,351	65.5%	12.42	106.3	139.0	168.2	210.4	281.1		
REACH PA1							5.49	91.9	120.7	146.4	184.1	247.9		
REACH PA2D							1.47	102.2	133.6	161.7	202.5	270.8		
<b>PPOA A</b>								<b>296.5</b>	<b>392.6</b>	<b>478.6</b>	<b>605.7</b>	<b>821.9</b>		
PB1B	977613	22.44	0.0351	84	355563	36%	13.86	46.9	63.5	78.4	100.0	136.1		
PB1DW	360395	8.27	0.0129	84	8314	2%	7.63	20.2	28.4	35.8	46.2	63.6		
PB1DE	906385	20.81	0.0325	84	553976	61%	8.62	62.7	81.8	99.3	124.0	165.5		
POND 1								0.2	0.8	4.3	13.6	34.8		
POND 2								0.3	0.3	0.3	3.5	45.3		
REACH PB1D							1.35	0.20	0.80	4.30	13.30	33.60		
<b>PPOAB</b>								<b>47.20</b>	<b>63.90</b>	<b>78.90</b>	<b>100.50</b>	<b>157.70</b>		

The proposed drainage area map for the ultimate conditions was designed for the fully developed industrial site with six industrial buildings. The post-development conditions consist of 12 basins totaling about 355.2 acres. Due to the complexity of the site's drainage patterns, storm drains, culverts, channels, and ponds were used to divert flow to ensure that existing flows were matched and/or decreased. See **Exhibit 9** for the proposed drainage area map that shows the ultimate proposed drainage routing for the fully developed site conditions. The table below shows the discharge of the proposed points of analysis with a decrease in flows at POA A, B, and C.

## Proposed Ultimate Conditions - HEC-HMS Model & Results:



Proposed Development Conditions - SCS Method - Via HEC-HMS											
BASIN	AREA			CN	IMPERVIOUS COVER		LAG min	Q (CFS)			
	sf	ac	sq mi		sf	%		2-yr	5-yr	10-yr	25-yr
PA1	2,244,375	51.5	0.0805	84	1,428,275	63.6%	26.75	98.3	129.2	156.8	197.1
PA2B	4,252,218	97.6	0.1525	84	2,376,513	55.9%	39.94	147.2	195.8	239.5	304.0
PA2D	1,845,372	42.4	0.0662	84	1,209,351	65.5%	12.42	106.3	139.0	168.2	210.4
REACH PA1							5.49	91.7	120.4	146.0	183.6
REACH PA2D							1.47	102.2	133.6	161.7	202.5
<b>P POA A</b>	<b>8,341,964</b>	<b>191.5</b>	<b>0.2992</b>		<b>5,014,139</b>	<b>60.1%</b>		<b>296.3</b>	<b>392.3</b>	<b>478.2</b>	<b>605.2</b>
PB1B	892568	20.5	0.0320	84	390015	43.7%	11.95	48.8	65.3	80.2	101.5
PB1DW	148014	3.4	0.0053	84	89891	60.7%	3.00	10.7	14.0	16.9	21.1
PB1DE	938133	21.5	0.0337	84	553976	59.1%	8.62	64.5	84.5	102.6	128.3
REACH PB1D							1.41	0.2	0.2	0.2	0.2
<b>P POA B</b>	<b>1,978,715</b>	<b>45</b>	<b>0.0710</b>		<b>1,033,881</b>	<b>52.3%</b>		<b>49.2</b>	<b>65.7</b>	<b>80.6</b>	<b>102.0</b>
PC 1	2,787,609	64.0	0.1000	84	1,811,946	65.0%	28.64	116.9	153.6	186.4	234.5
PC 2	332,575	7.6	0.0119	84	196,639	59.1%	12.94	18.2	23.9	29.1	36.5
PC3A	619,735	14.2	0.0222	84	355,885	57.4%	13.88	32.1	42.4	51.6	65.0
PC3B	431,604	9.9	0.0155	84	30,972	7.2%	7.72	25.7	35.7	44.8	57.6
PC3C	107,012	2.5	0.0038	84	94,453	88.3%	8.43	8.0	10.2	12.2	15.1
PC3D	873,264	20.0	0.0313	84	643,781	73.7%	12.72	50.9	66.0	79.4	99.0
REACH PC1							2.40	111.6	146.3	177.4	223.6
REACH PC2							4.47	121.6	159.6	193.8	243.9
<b>P POA C</b>	<b>5,151,800</b>	<b>118.3</b>	<b>0.1848</b>		<b>3,133,677</b>	<b>61%</b>		<b>189.9</b>	<b>247.8</b>	<b>299.0</b>	<b>374.3</b>
											<b>550.7</b>

## 2.6 STORMWATER DETENTION

Based on the existing and proposed HEC-HMS Results, storm water detention is required due to the increase of area and impervious cover being routed to POA B and C. There are two ponds proposed at POA B, named Pond 1 and Pond 2, and one pond proposed at POA C, named Pond 3. Each pond was designed using the City of Austin Drainage Criteria Manual as guide for side slopes, berm width, bottom slopes, and spillway/outlet calculations. Pond 2 is greater than six feet in height and therefore has associated probable maximum flood calculations with the pond.

For the intermediate conditions, Pond 1 is proposed to ensure that flows do not increase at POA B for proposed conditions. The pond footprint, bottom and side slopes, and outlet structure remain the same from intermediate to proposed conditions. The difference between intermediate and ultimate conditions for Pond 1 is the contributing drainage area, inlet structure, and temporary channel. In the intermediate conditions, there are 8.27 acres routed to Pond 1 with 2.31% impervious cover while in the ultimate conditions, there is 3.28 acres with 62.86% impervious cover. In the intermediate conditions, there is a temporary channel constructed to ensure water is routed into the pond. Similarly, there is a temporary entrance headwall provided to the pond from the temporary channel. This headwall is to be demoed and replaced with the storm drain infrastructure proposed in Site Plan B. Refer to **Exhibit 10** for the proposed detention Pond 1 sheets that further detail the pond for intermediate conditions. Refer to **Exhibit 11** for the proposed detention pond sheets that further detail each pond for the ultimate conditions. Note that Pond 2 remains the same from the intermediate conditions to the ultimate conditions and can be found in Exhibit 11.

The proposed channels for Manor Downs Industrial were also analyzed. **Exhibit 12** shows the cross sections for Channel East for the 2 yr, 25 yr, and 100 yr storm events as well as the internal cross sections near Murray-Dalfen Drive for the same storm events. For Channels A-C on the interior of the site near Murray Dalfen Drive, the flows through the channel were taken from Storm CAD headwall results. For Channel D, the flow in the cross section was determined using the HEC HMS results for Pond 1. For Channel E, a combination of StormCAD headwall results and Pond 2 HEC HMS results were utilized to determine the total flow through the cross section. The table below shows the difference in flows from existing to proposed conditions at each POA.

Existing vs. Proposed Flow (CFS) - HEC-HMS Results:

EXISTING VS PROPOSED FLOWS					
	2YR	5 YR	10 YR	25 YR	100 YR
EXISTING POA A	305.0	410.0	504.8	644.5	883.3
PROPOSED POA A	296.3	392.3	478.2	605.2	821.3
Δ	-8.7	-17.7	-26.6	-39.3	-62.0
EXISTING POA B	49.3	69.6	87.9	114.3	158.1
PROPOSED POA B	49.2	65.7	80.6	102.0	137.7
Δ	-0.1	-3.9	-7.3	-12.3	-20.4
EXISTING POA C	191.4	258.1	317.7	404.8	551.0
PROPOSED POA C	189.9	247.8	299.0	374.3	550.7
Δ	-1.5	-10.3	-18.7	-30.5	-0.3

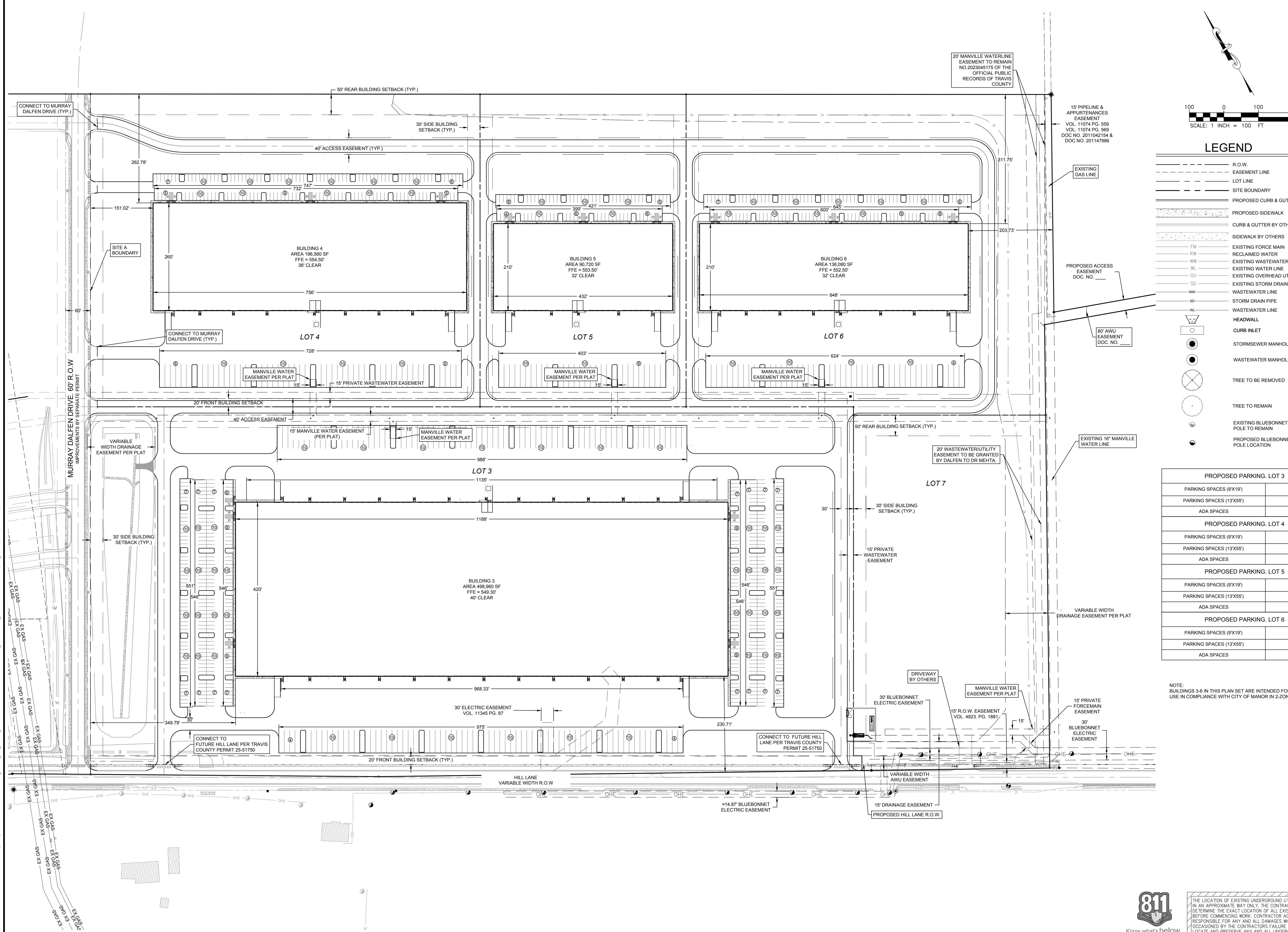
## 2.7 DRAINAGE EASEMENTS

There are two existing drainage easements on site that are to be vacated by the City of Manor with document numbers 2000100887 and 2004163233. There are four proposed drainage easements to be dedicated via plat in order to convey offsite drainage through the site. The drainage easements cover the proposed ponds and channels throughout the site.

EXHIBIT 1 – LOCATION MAP



EXHIBIT 2 – OVERALL SITE & DIMENSION PLAN



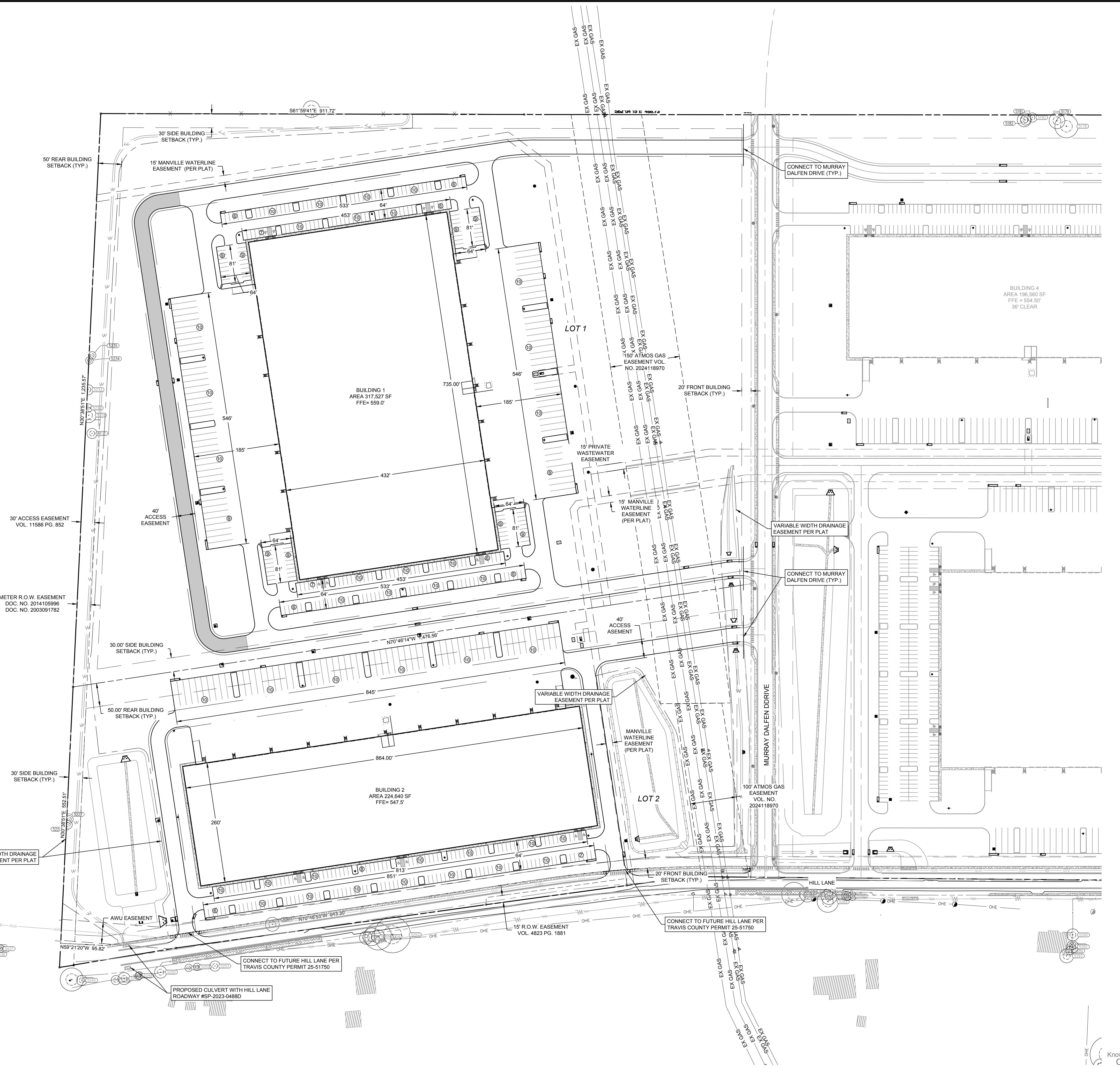
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VIEW, MARK-UP, AND/OR  
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AUTHORITY OF  
MARISA KEISER, P.E.  
#141397  
ON AUGUST 1 2025.  
IS NOT TO BE USED FOR  
STRUCTION, BIDDING, OR  
PERMIT PURPOSES.

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GN	DRAWN LB	CHKD MK
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No. 070422-01-002  
SHEET

5 OF 86  
OR CONSTRUCTION



OCCASION  
LOCATE A

LOCATION OF EXISTING UNDERGROUND UTILITIES ARE SHOWN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES COMMENCING WORK. CONTRACTOR AGREES TO BE FULLY LIABLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE CAUSED BY THE CONTRACTORS FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

HIS DOCUMENT IS RELEASED  
OR THE PURPOSE OF INTERIM  
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DRAFTING UNDER THE  
AUTHORITY OF  
MARISA KEISER, P.E.  
#141397  
ON AUGUST 1 2025.  
IT IS NOT TO BE USED FOR  
CONSTRUCTION, BIDDING, OR  
PERMIT PURPOSES.

DESIGN MC	DRAWN MG	CHKD MK
B No. 070422-01-002		
SHEET 07 OF 43		

## OVERALL SITE PLAN

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**OVERALL SITE PLAN**

**ANOR DOWNS INDUSTRIAL - SITE PLAN B**

**8500 HILL LN**

**MANOR, TEXAS, 78653**

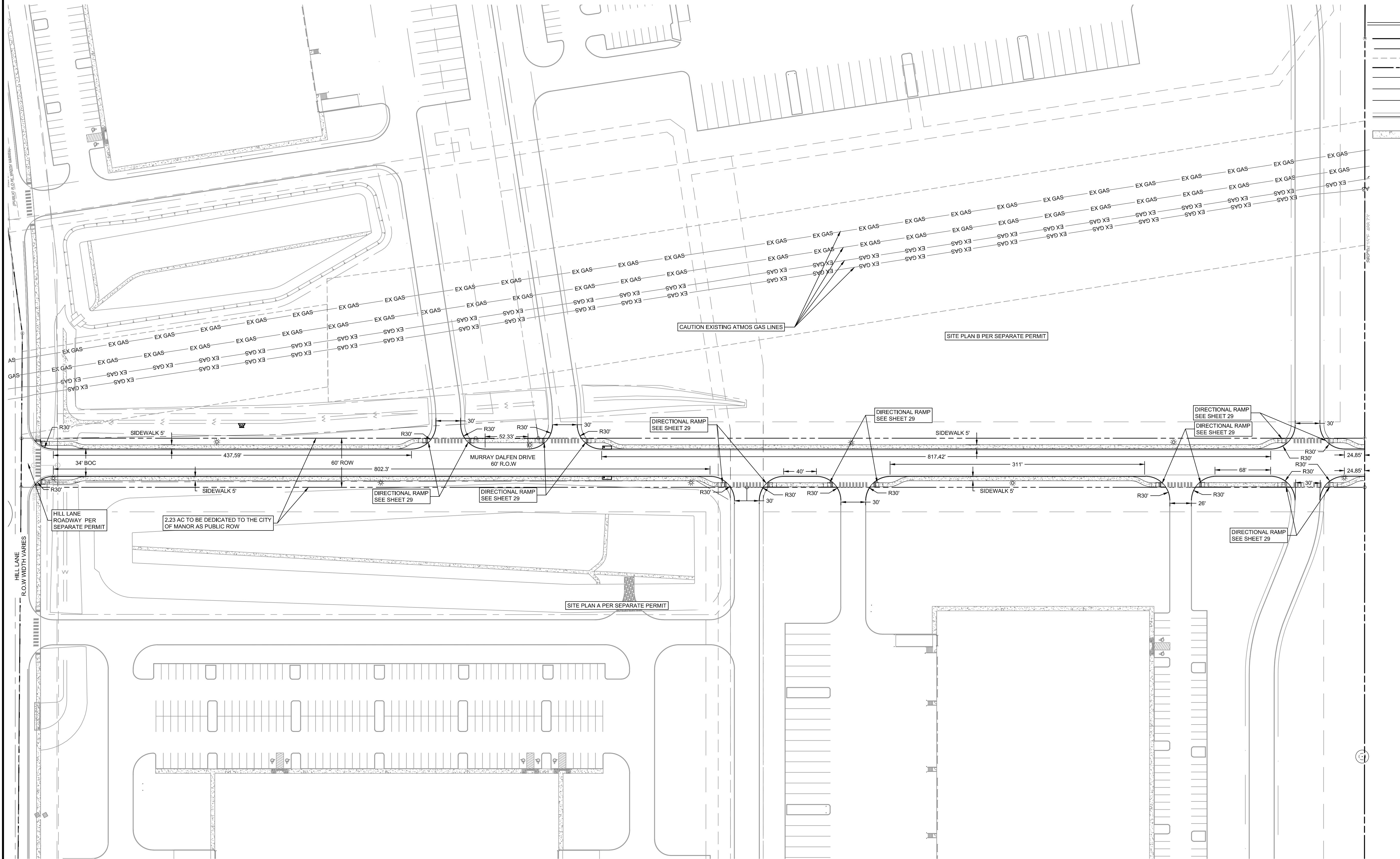
PROPOSED PARKING. LOT 1	
PARKING SPACES (9'X19')	254
PARKING SPACES (13'X55')	78
ADA SPACES	8
PROPOSED PARKING. LOT 2	
PARKING SPACES (9'X19')	155
PARKING SPACES (13'X55')	60
ADA SPACES	6

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**DESCRIPTION**

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omas Parkway  
exas 78746  
Phone: (512) 327-1180  
Fax: (512) 327-4062  
[www.bownman.com](http://www.bownman.com)



60 0 60 200  
SCALE: 1 INCH = FT 60

## LEGEND

- PROPERTY BOUNDARY LINE
- R.O.W.
- EASEMENT LINE
- CREEK CENTERLINE
- FIRE LANE
- WL
- FM
- CURB AND GUTTER
- PROPOSED SIDEWALK
- MANHOLE
- WATER METER
- FIRE HYDRANT
- BACKFLOW PREVENTER
- CURB INLET
- HEADWALL

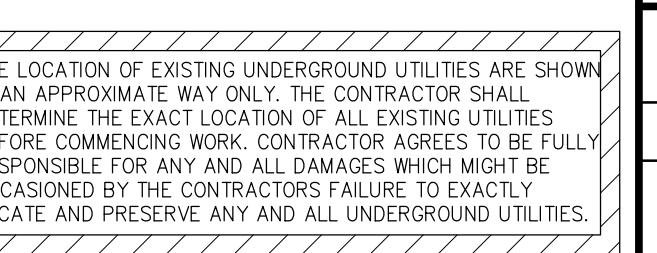
## REVISIONS

REVISION	DESCRIPTION	DATE
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## OVERALL SUBDIVISION DIMENSION PLAN

MANOR DOWNS INDUSTRIAL  
8916 HILL LN  
MANOR, TEXAS, 78653

THIS DOCUMENT IS RELEASED FOR THE PURPOSE OF INTERIM REVIEW, MARK-UP, AND/OR DRAFTING UNDER THE AUTHORITY OF MARISA KEISER, P.E.  
#141397  
ON JULY 30, 2025.  
IT IS NOT TO BE USED FOR CONSTRUCTION, BIDDING, OR PERMIT PURPOSES.



DESIGN DRAWN CHKD  
MC MG MK

JOB No. 070422-01-002

SHEET

08 OF 34

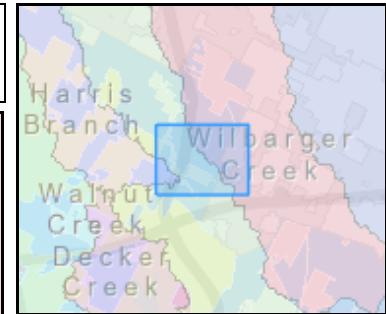
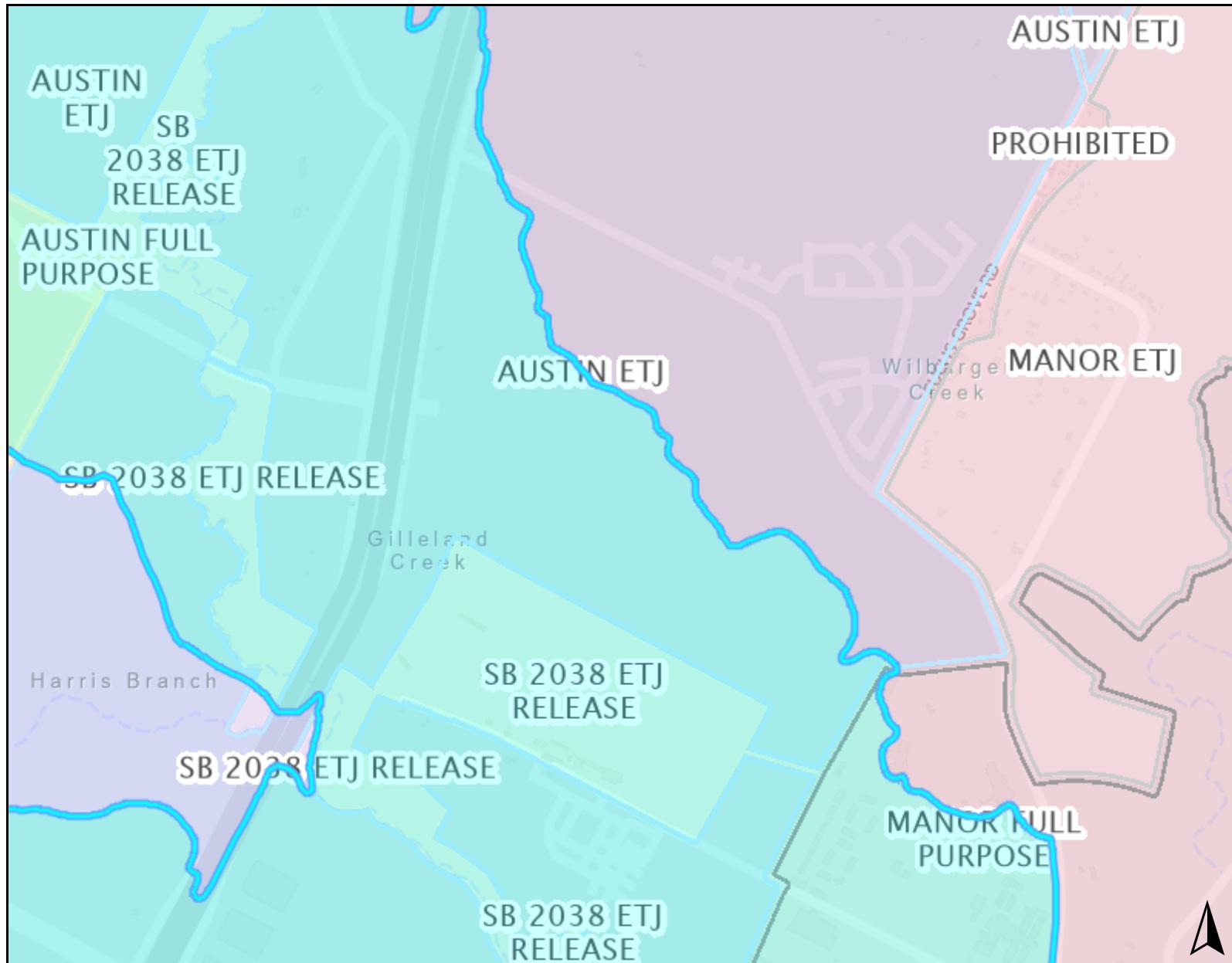


Know what's below.  
Call before you dig.

EXHIBIT 3 – WATERSHED MAP



# Property Profile



## Legend

### Property

- Jurisdiction
  - FULL PURPOSE
  - EXTRATERRITORIAL
  - JURISDICTION
  - EXTRATERRITORIAL
  - JURISDICTION
- OTHER CITY LIMITS
- OTHER CITIES ETJ

### Jurisdictions Fill

- Jurisdiction
  - FULL PURPOSE
  - EXTRATERRITORIAL
  - JURISDICTION
  - EXTRATERRITORIAL
  - JURISDICTION
- OTHER CITY LIMITS
- OTHER CITIES ETJ

### Environmental 3

- Watershed
  - Gilleland Creek
  - Harris Branch
  - Wilbarger Creek

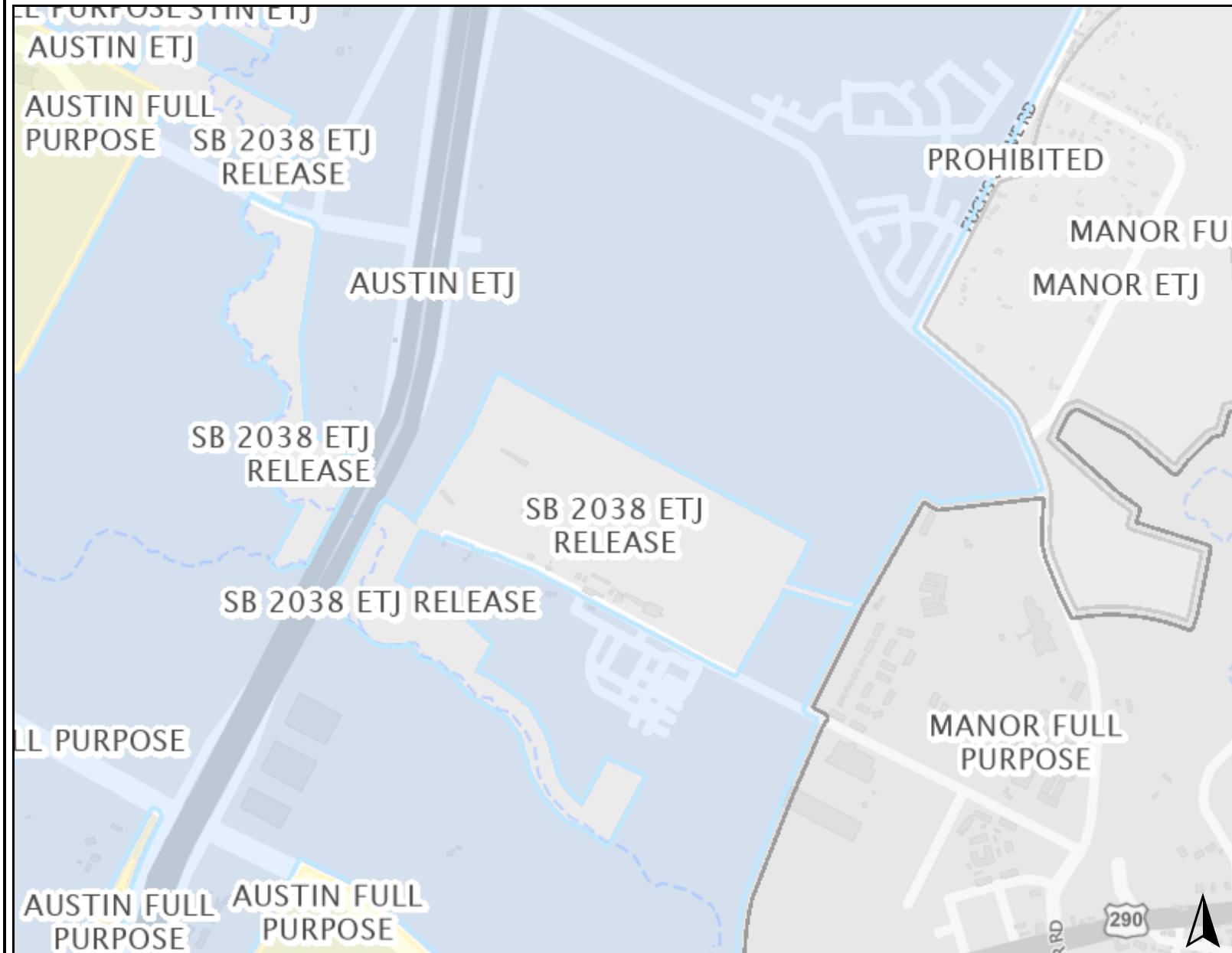
## Notes

This product is for informational purposes and may not have been prepared for or be suitable for legal, engineering, or surveying purposes. It does not represent an on-the-ground survey. This product has been produced by the City of Austin for the sole purpose of geographic reference. No warranty is made by the City of Austin regarding specific accuracy or completeness.

EXHIBIT 4 – EDWARDS AQUIFER MAP



# Property Profile



## Legend

### Property

- Jurisdiction
  - FULL PURPOSE
  - EXTRATERRITORIAL
  - JURISDICTION
  - OTHER CITY LIMITS
  - OTHER CITIES ETJ

### Jurisdictions Fill

- Jurisdiction
  - FULL PURPOSE
  - EXTRATERRITORIAL
  - JURISDICTION
  - OTHER CITY LIMITS
  - OTHER CITIES ETJ

## Notes

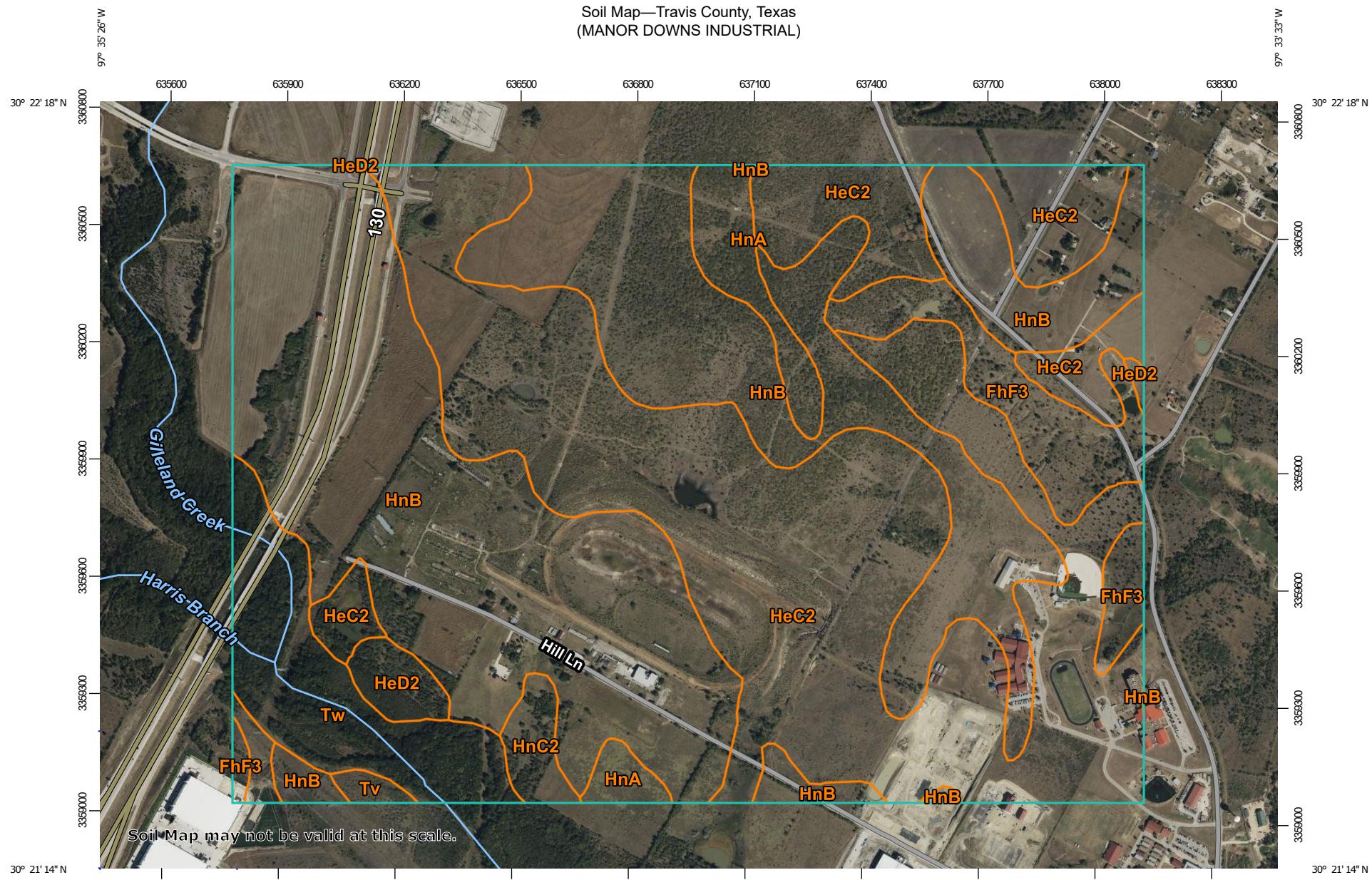
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EXHIBIT 5 – FEMA FLOODPLAIN MAP



EXHIBIT 6 – NRCS SOILS MAP

Soil Map—Travis County, Texas  
(MANOR DOWNS INDUSTRIAL)



Map Scale: 1:13,900 if printed on A landscape (11" x 8.5") sheet.

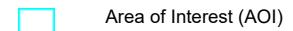
A horizontal scale bar representing distance in meters. The bar is divided into four segments by vertical tick marks at 200, 400, 600, and 800 meters. The word "Meters" is written in black capital letters at the far right end of the bar.

Man projection: Web Mercator. Corner coordinates: WGS84. Edge ticks: UTM Zone 14N WGS84



## MAP LEGEND

### Area of Interest (AOI)



Area of Interest (AOI)

### Soils



Soil Map Unit Polygons



Soil Map Unit Lines



Soil Map Unit Points

### Special Point Features



Blowout



Borrow Pit



Clay Spot



Closed Depression



Gravel Pit



Gravelly Spot



Landfill



Lava Flow



Marsh or swamp



Mine or Quarry



Miscellaneous Water



Perennial Water



Rock Outcrop



Saline Spot



Sandy Spot



Severely Eroded Spot



Sinkhole



Slide or Slip



Sodic Spot

Spoil Area

Stony Spot

Very Stony Spot

Wet Spot

Other

Special Line Features

### Water Features

Streams and Canals

### Transportation

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

### Background

Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Travis County, Texas

Survey Area Data: Version 26, Aug 30, 2024

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Data not available.

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
FhF3	Ferris-Heiden complex, 8 to 20 percent slopes, severely eroded	46.6	4.9%
HeC2	Heiden clay, 3 to 5 percent slopes, eroded	373.5	39.4%
HeD2	Heiden clay, 5 to 8 percent slopes, eroded	10.4	1.1%
HnA	Houston Black clay, 0 to 1 percent slopes	25.2	2.7%
HnB	Houston Black clay, 1 to 3 percent slopes	423.3	44.6%
HnC2	Houston Black clay, 3 to 5 percent slopes, moderately eroded	10.0	1.1%
Tv	Tinn clay, 0 to 1 percent slopes, occasionally flooded	3.1	0.3%
Tw	Tinn clay, 0 to 1 percent slopes, frequently flooded	56.5	6.0%
<b>Totals for Area of Interest</b>		<b>948.7</b>	<b>100.0%</b>

EXHIBIT 7 – EXISTING DRAINAGE AREA MAP

# Bowman

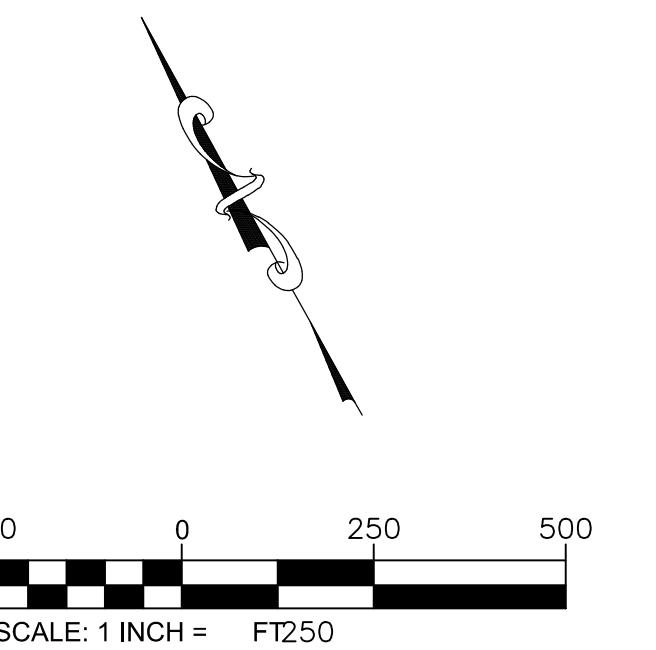
©2024 Bowman Consulting Group Ltd  
TBE Firm Registration No. F-114309  
807 Las Cimas Parkway  
Austin, Texas 78746

Phone: 512-327-1160  
Fax: 512-327-4002  
www.bowman.com

## EXISTING DRAINAGE AREA MAP

MANOR DOWNS INDUSTRIAL  
8916 HILL LN  
MANOR, TEXAS, 78653

Existing Development Conditions - SCS Method - Via HEC-HMS																														
BASIN	AREA			IMPERVIOUS COVER		SHEET FLOW			SHALLOW CONCENTRATED FLOW		CHANNEL FLOW			PIPE FLOW		TC		CN	IC%											
	sf	ac	sq mi	sf	%	2yr-24hr	Tc	L	S	Tc	L	n	S	R	V*	Tc	L	n	%	In	sf	in	fps	min	lag					
EA1	2288711.08	52.43	0.0819	1,428,166	62.54%	100	0.240	0.50%	4.06	22.1	2693	1.60%	Unpaved	16.135	2.20	304	0.045	1.90%	1.00	0.56	1.1	45.2	27.10	84	62.5%					
EA2	4696199.48	107.81	0.1685	2,456,560	52.31%	100	0.240	0.10%	4.06	42.0	1808	2.10%	Unpaved	16.135	12.9	2812	0.045	1.28%	1.00	3.75	12.5	67.4	40.43	84	52.3%					
EA3	1761118.04	40.43	0.0632	7,433	0.42%	100	0.240	5.00%	4.06	8.8	2054	1.90%	Unpaved	16.135	15.4		483	0.014	0.10%	40.0	8.72	125.6	8.42	1.12	25.3	15.18	84	0.4%		
EA4	703688.62	16.15	0.0252	6,356	0.90%	50	0.240	0.10%	4.06	24.1	1506	0.04%	Unpaved	16.135	77.8	1998	0.045	0.98%	1.14	3.58	9.3	111.2	66.72	84	0.9%					
REACH EA1																														
REACH EA3																														
REACH EA4																														
EB1	1146885.28	26.33	0.0411	85,459	7.45%	100	0.240	3.00%	4.06	10.8	1276	3.10%	Unpaved	16.135	7.5	791	0.045	0.70%	1.14	3.02	4.4					22.6	13.57	84	7.5%	
EC 1	2287609.20	63.99	0.1000	1,811,946	65.00%	100	0.240	0.50%	4.06	22.1	3145	1.60%	Unpaved	16.135	25.7												47.7	28.64	84	65.0%
EC 2	403952.78	9.27	0.0145	196,647	48.08%	100	0.240	1.70%	4.06	13.5	1004	1.35%	Unpaved	16.135	8.9												22.4	13.47	84	48.7%
EC 3	1689471.13	38.78	0.0606	84,323	4.99%	100	0.240	2.30%	4.06	12.0	1175	2.49%	Unpaved	16.135	7.7	1998	0.045	1.00%	1.14	3.61	9.2					28.9	17.33	84	5.0%	
REACH EC1																														
REACH EC2																														
TOTAL AREA	15472635.61	355.20	0.56	6,076,889	39.28%																									



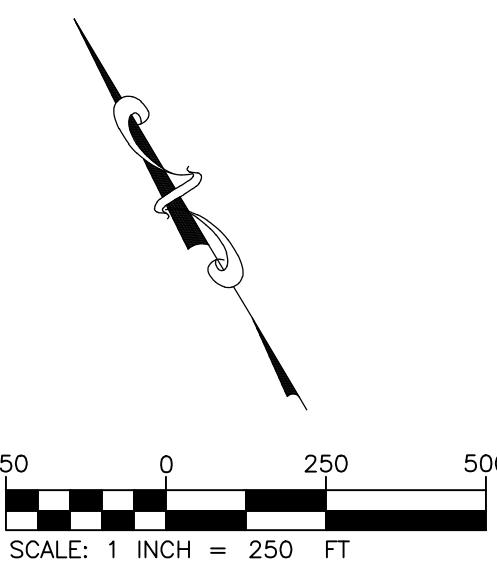
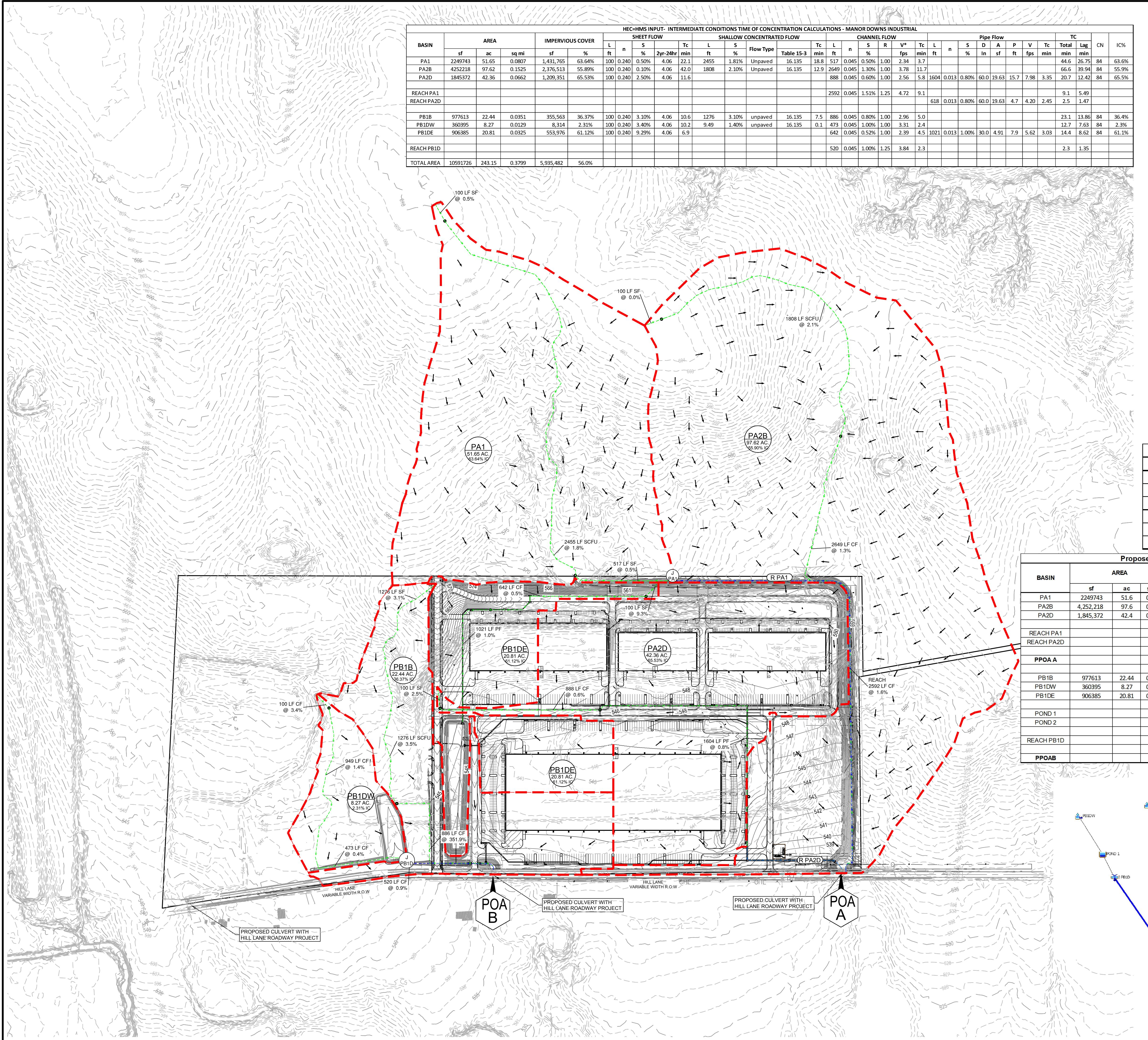
## LEGEND

- PROPERTY BOUNDARY
- EXISTING R.O.W.
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- EXISTING DRAINAGE AREA
- TIME OF CONCENTRATION
- REACH
- DRAINAGE AREA IDENTIFIER
- FLOW ARROW

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#H1397  
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DESIGN MC DRAWN MG CHKD MK  
JOB NO. 070422-01-002 SHEET 17 OF 33  
PRELIMINARY NOT FOR CONSTRUCTION

EXHIBIT 8 – PROPOSED DRAINAGE AREA MAP: INTERMEDIATE CONDITIONS

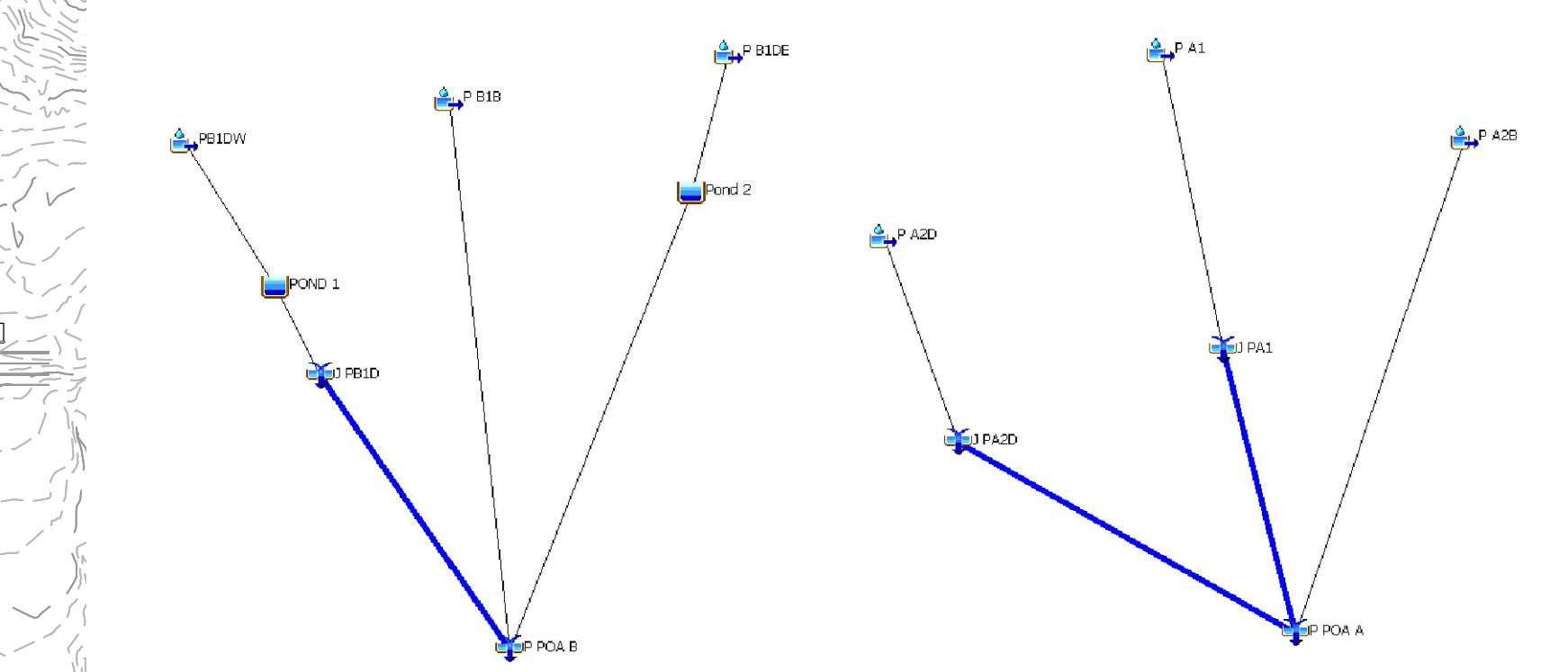


### LEGEND

- PROPERTY BOUNDARY
- PROPOSED R.O.W.
- PROPOSED LOT LINE
- EASEMENT LINE
- PROPOSED CURB & GUTTER
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- DRAINAGE AREA
- TIME OF CONCENTRATION
- REACH
- XX XX.XX DRAINAGE AREA IDENTIFIER
- FLOW ARROW
- HEADWALL
- CURB INLET
- AREA INLET
- JUNCTION BOX

EXISTING VS INTERMEDIATE CONDITIONS FLOWS					
	2YR	5YR	10YR	25YR	100YR
EXISTING POA A	305.0	410.0	504.8	644.5	883.3
PROPOSED POA A	296.5	392.6	478.6	605.7	821.9
Δ	-8.5	-17.4	-26.2	-38.8	-61.4
EXISTING POA B	49.3	69.6	87.9	114.3	158.1
PROPOSED POA B	47.2	63.9	78.9	100.5	157.7
Δ	-2.1	-5.7	-9.0	-13.8	-0.4

BASIN	AREA			CN	IMPERVIOUS COVER		LAG min	Q (CFS)					
	sf	ac	sq mi		sf	%		2-yr	5-yr	10-yr	25-yr	100-yr	
PA1	2249743	51.6	0.0807	84	1,431,765	63.6%	26.75	98.5	129.5	157.2	197.6	265.9	
PA2B	4252218	97.62	0.1525	84	2,376,513	55.9%	39.94	147.2	195.8	239.5	300.4	413.9	
PA2D	1845372	42.36	0.0662	84	1,209,351	65.53%	12.42	106.3	139.0	168.2	210.4	281.1	
REACH PA1								5.49	91.9	120.7	146.4	184.1	247.9
REACH PA2D								1.47	102.2	133.6	161.7	202.5	270.8
PPOA A								296.5	392.6	478.6	605.7	821.9	
PB1B	977613	22.44	0.0351	84	355,563	36%	13.86	46.9	63.5	78.4	100.0	136.1	
PB1DW	360395	8.27	0.0129	84	8314	2%	7.63	20.2	28.4	35.8	46.2	63.6	
PB1DE	906385	20.81	0.0325	84	553,976	61%	8.62	62.7	81.8	99.3	124.0	165.5	
POND 1								0.2	0.8	4.3	13.6	34.8	
POND 2								0.3	0.3	0.3	3.5	45.3	
REACH PB1D								1.35	0.20	0.80	4.30	13.30	33.60
PPOAB								47.20	63.90	78.90	100.50	157.70	

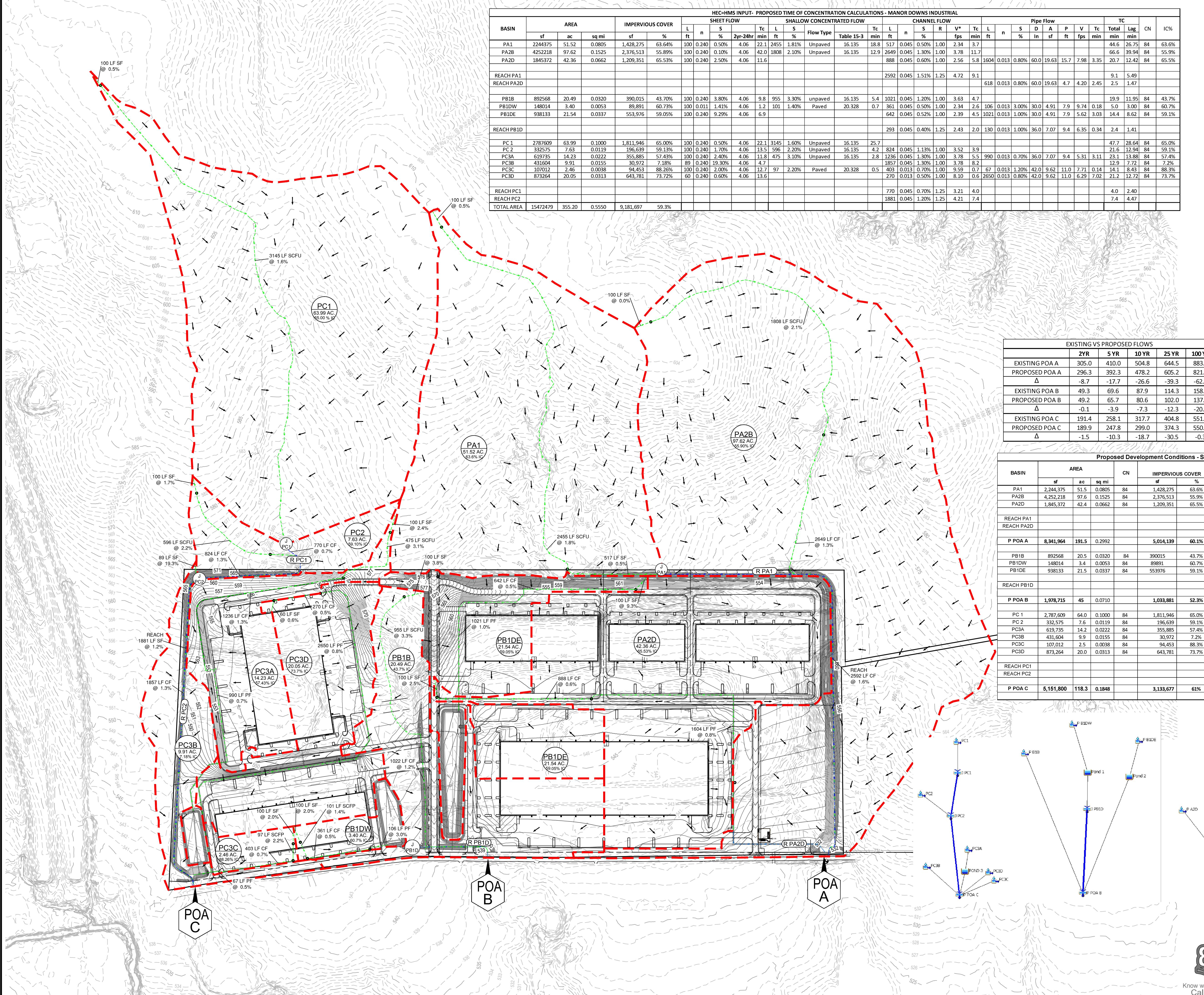


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JOB No. 070422-01-002 SHEET 47 OF 79  
PRELIMINARY NOT FOR CONSTRUCTION

**Bowman**  
TBE Firm Registration No. F-11439  
Phone: 512-327-1160  
Email: info@bowman.com  
807 Las Cimas Parkway  
Austin, Texas 78746

EXHIBIT 9 – PROPOSED DRAINAGE AREA MAP: ULTIMATE CONDITIONS



A detailed map showing a river system with various tributaries and a main channel. The map includes a scale bar at the bottom.

Scale: 1 INCH = 250 FT



**Owman**

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Fort Lauderdale, Florida 33301  
Phone: (512) 327-1180

EXISTING VS PROPOSED FLOWS					
	2YR	5 YR	10 YR	25 YR	100 YR
EXISTING POA A	305.0	410.0	504.8	644.5	883.3
PROPOSED POA A	296.3	392.3	478.2	605.2	821.3
Δ	-8.7	-17.7	-26.6	-39.3	-62.0
EXISTING POA B	49.3	69.6	87.9	114.3	158.1
PROPOSED POA B	49.2	65.7	80.6	102.0	137.7
Δ	-0.1	-3.9	-7.3	-12.3	-20.4
EXISTING POA C	191.4	258.1	317.7	404.8	551.0
PROPOSED POA C	189.9	247.8	299.0	374.3	550.7
Δ	-1.5	-10.3	-18.7	-30.5	-0.3

Proposed Development Conditions - SCS Method - Via HEC-HMS										
AREA		CN	IMPERVIOUS COVER		LAG min	Q (CFS)				
			sf	%		2-yr	5-yr	10-yr	25-yr	100-yr
51.5	0.0805	84	1,428,275	63.6%	26.75	98.3	129.2	156.8	197.1	265.2
97.6	0.1525	84	2,376,513	55.9%	39.94	147.2	195.8	239.5	304.0	413.9
42.4	0.0662	84	1,209,351	65.5%	12.42	106.3	139.0	168.2	210.4	281.1
					5.49	91.7	120.4	146.0	183.6	247.3
					1.47	102.2	133.6	161.7	202.5	270.8
<b>191.5</b>	<b>0.2992</b>		<b>5,014,139</b>	<b>60.1%</b>		<b>296.3</b>	<b>392.3</b>	<b>478.2</b>	<b>605.2</b>	<b>821.3</b>
20.5	0.0320	84	390015	43.7%	11.95	48.8	65.3	80.2	101.5	137.2
3.4	0.0053	84	89891	60.7%	3.00	10.7	14.0	16.9	21.1	28.2
21.5	0.0337	84	553976	59.1%	8.62	64.5	84.5	102.6	128.3	171.3
					1.41	0.2	0.2	0.2	0.2	0.2
<b>45</b>	<b>0.0710</b>		<b>1,033,881</b>	<b>52.3%</b>		<b>49.2</b>	<b>65.7</b>	<b>80.6</b>	<b>102.0</b>	<b>137.7</b>
64.0	0.1000	84	1,811,946	65.0%	28.64	116.9	153.6	186.4	234.5	315.9
7.6	0.0119	84	196,639	59.1%	12.94	18.2	23.9	29.1	36.5	49.0
14.2	0.0222	84	355,885	57.4%	13.88	32.1	42.4	51.6	65.0	87.4
9.9	0.0155	84	30,972	7.2%	7.72	25.7	35.7	44.8	57.6	79.0
2.5	0.0038	84	94,453	88.3%	8.43	8.0	10.2	12.2	15.1	19.8
20.0	0.0313	84	643,781	73.7%	12.72	50.9	66.0	79.4	99.0	131.7
					2.40	111.6	146.3	177.4	223.6	301.8
					4.47	121.6	159.6	193.8	243.9	329.1
<b>118.3</b>	<b>0.1848</b>		<b>3,133,677</b>	<b>61%</b>		<b>189.9</b>	<b>247.8</b>	<b>299.0</b>	<b>374.3</b>	<b>550.7</b>

# PROPOSED DRAINAGE AREA MAP

MOR DOWNS INDUSTRIAL - SIT  
8500 HILL LN  
MANOR, TEXAS, 78653

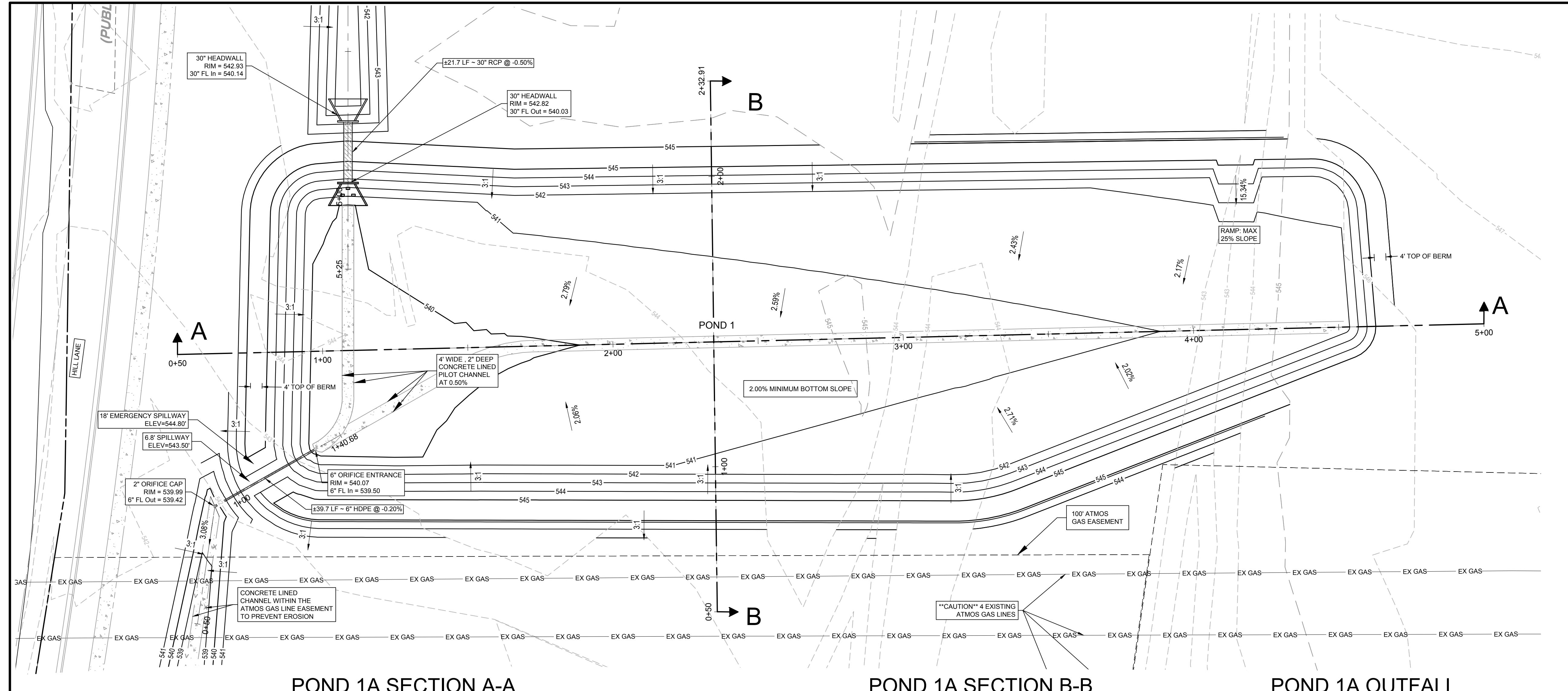
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DESIGN MC	DRAWN MG	CHKD MK
JOB No. 070422-01-002		

SHEET  
21 OF 43

# OT FOR CONSTRUCTION

EXHIBIT 10 – DETENTION POND 1: INTERMEDIATE CONDITIONS



SCALE: 1 INCH = 20 FT

## LEGEND

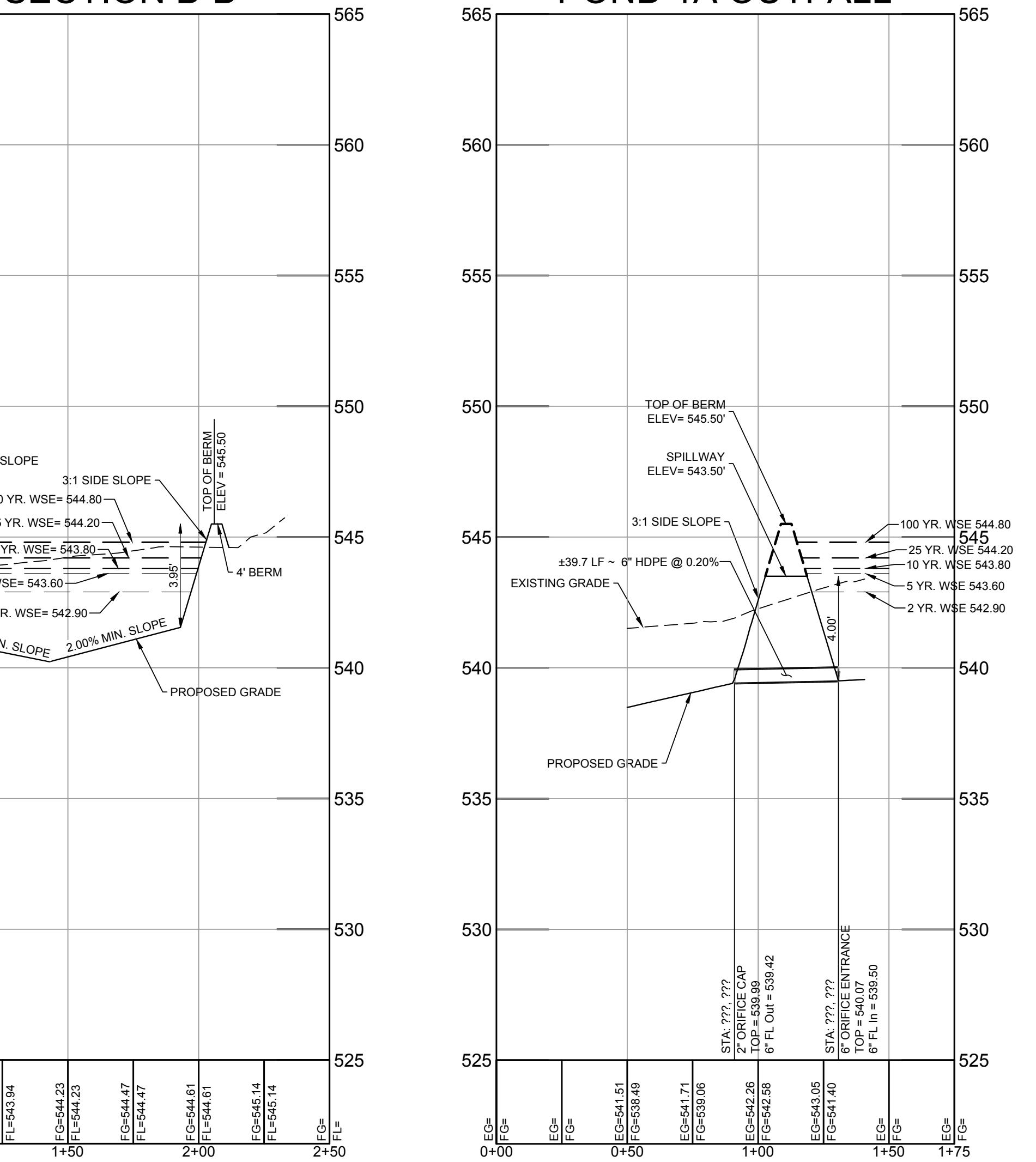
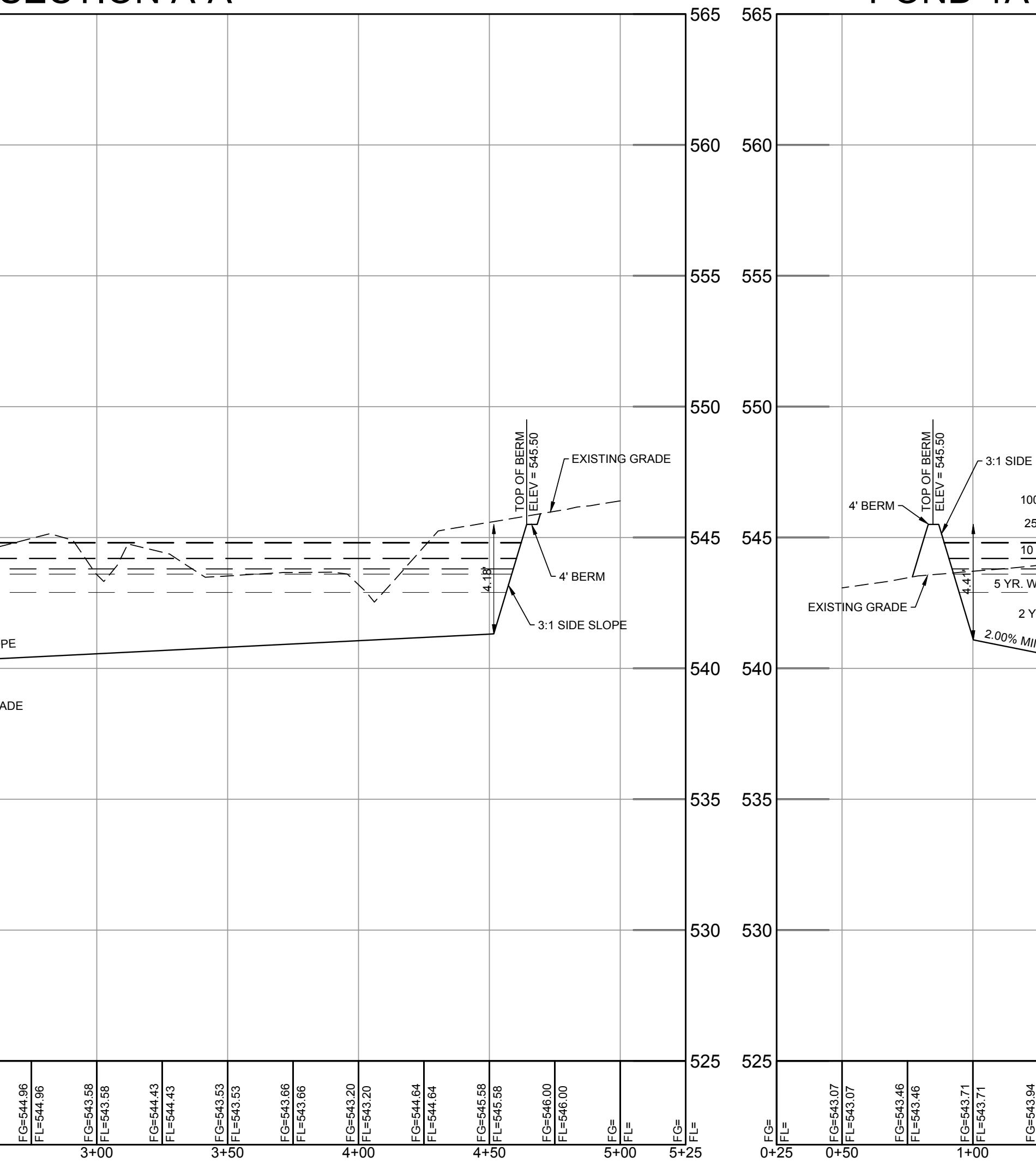
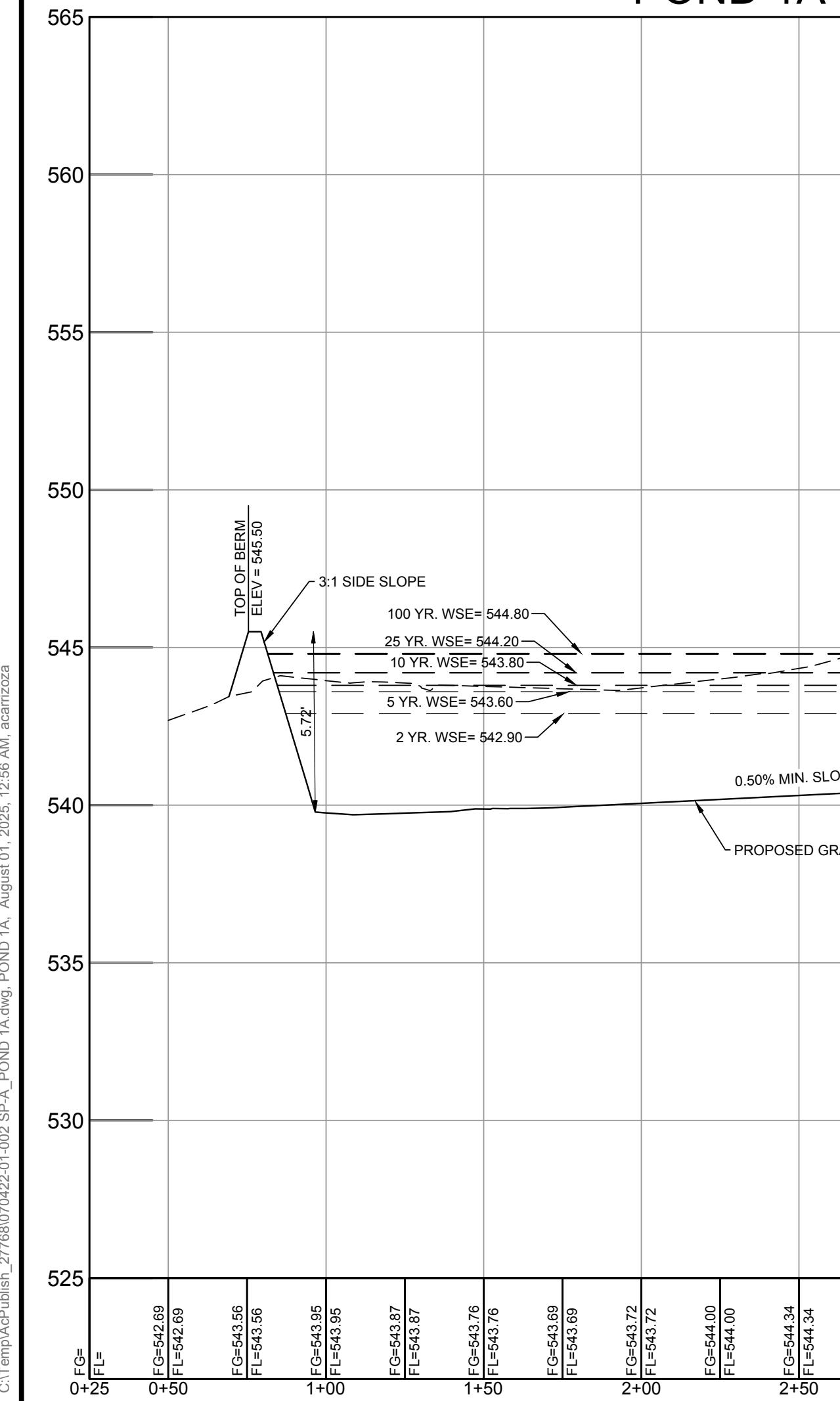
- PROPOSED R.O.W.
- PROPOSED LOT LINE
- SITE BOUNDARY
- EASEMENT LINE
- PROPOSED CURB & GUTTER
- EXISTING MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- WW - WASTEWATER LINE
- WL - WATER LINE
- STORM DRAIN LINE
- PROPOSED BERM
- PROPOSED SWALE
- PROPOSED ROCK BERM
- HEADWALL
- CURB INLET
- AREA INLET
- STORMSEWER MANHOLE

- NOTES:**
- ALL EARTHEN EMBANKMENTS SHALL BE COMPAKTED TO 95% OF MAXIMUM DENSITY IN ACCORDANCE WITH COA STANDARD SPECIFICATIONS.
  - EARTHEN EMBANKMENTS SIDE SLOPES SHALL BE NO STEEPER THAN 3 HORIZONTAL TO 1 VERTICAL.

POND 1A SECTION A-A

POND 1A SECTION B-B

POND 1A OUTFALL



PROFILE SCALE 1:40

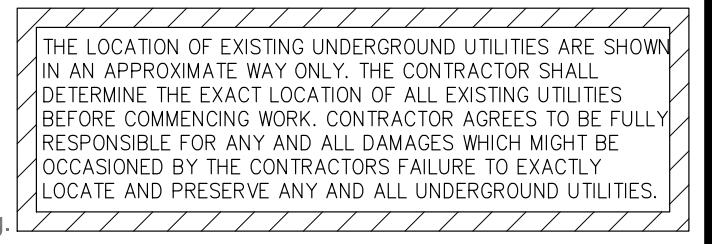
**POND 1A**  
**MANOR DOWNS INDUSTRIAL - SITE PLAN A**  
**8916 HILL LN**  
**MANOR, TEXAS, 78653**

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DESIGN DRAWN CHKO  
MC LB MK

JOB No. 070422-01-002

SHEET 55 OF 79



## REVISIONS

REVISION	DESCRIPTION	DATE
***	***	***

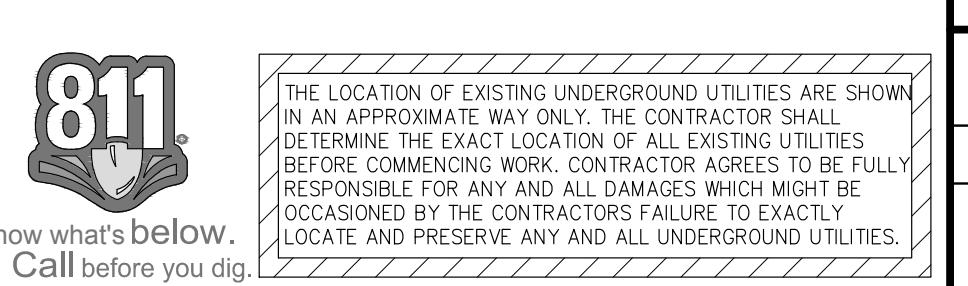
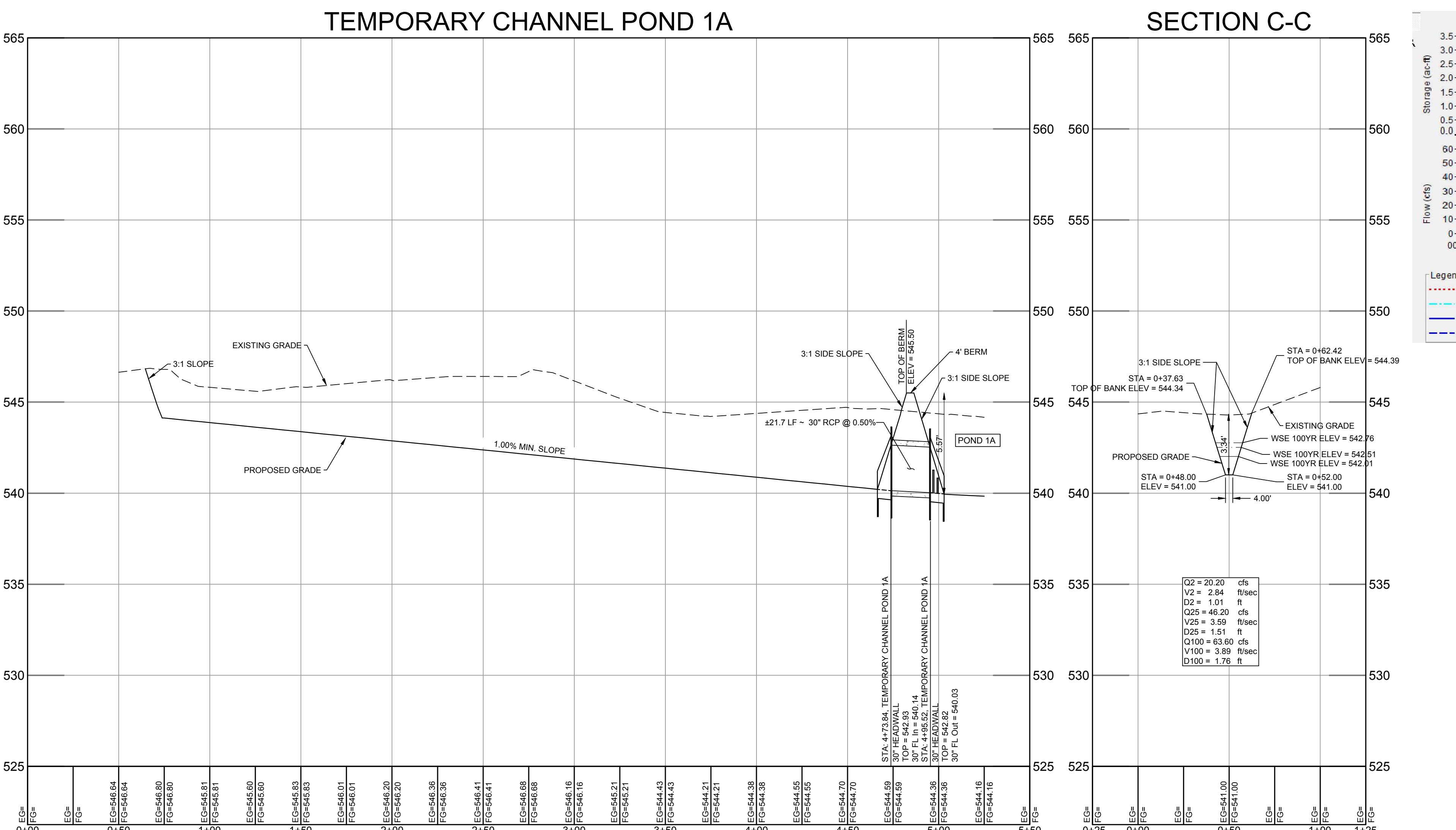
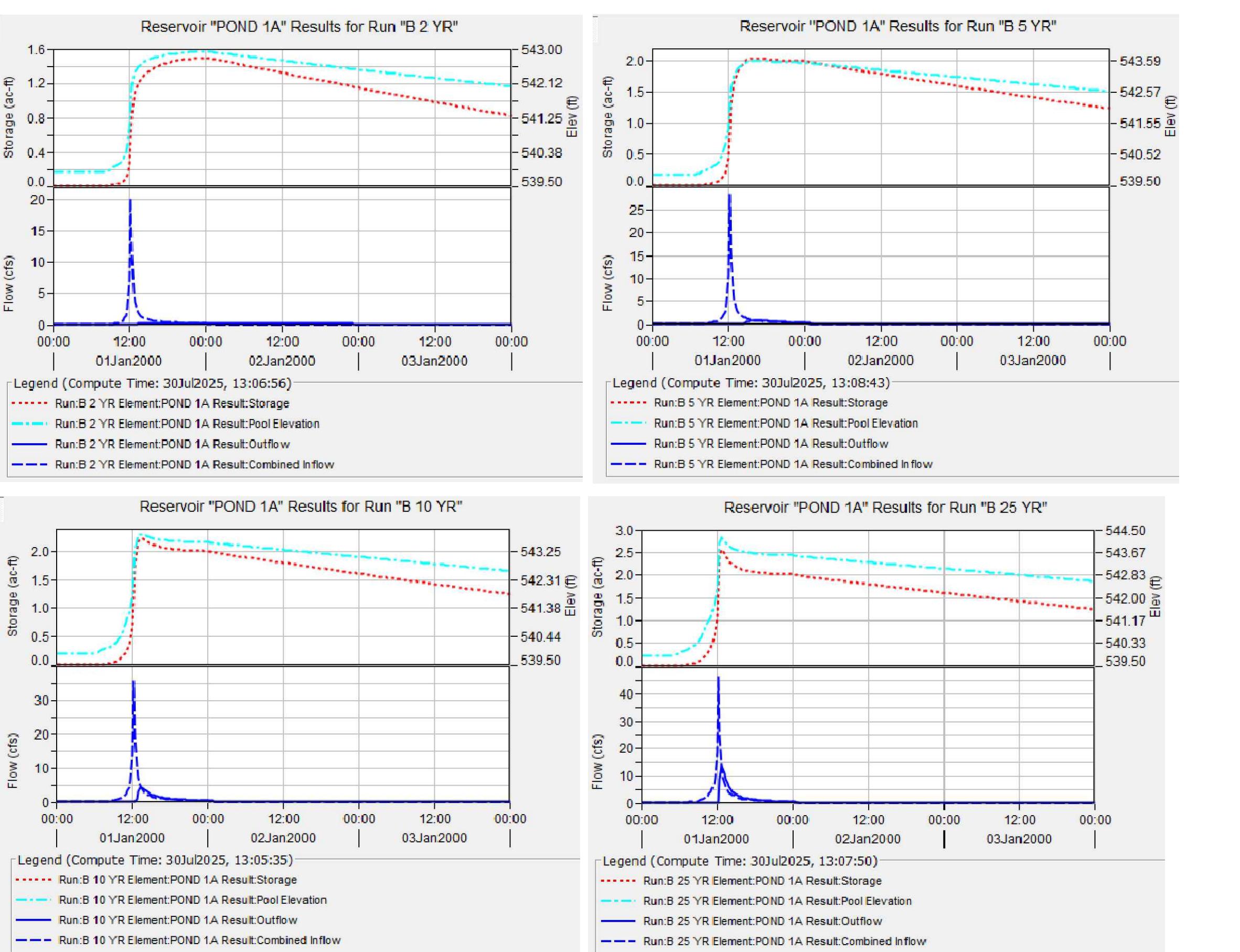
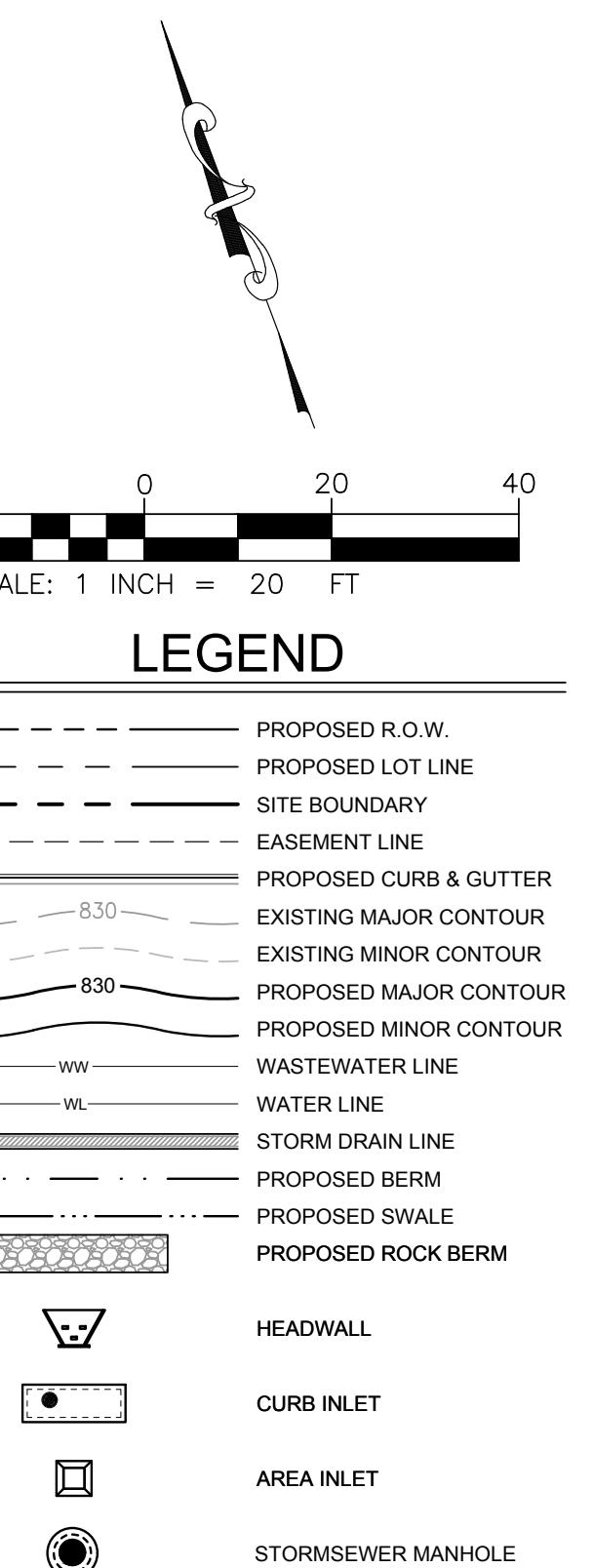
## POND 1A CHANNEL AND CALCULATIONS

MANOR DOWNS INDUSTRIAL - SITE PLAN A  
8916 HILL LN  
MANOR, TEXAS, 78653

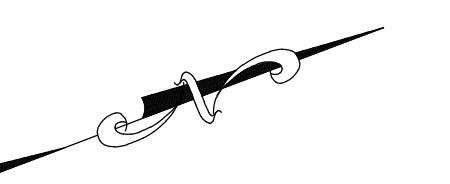
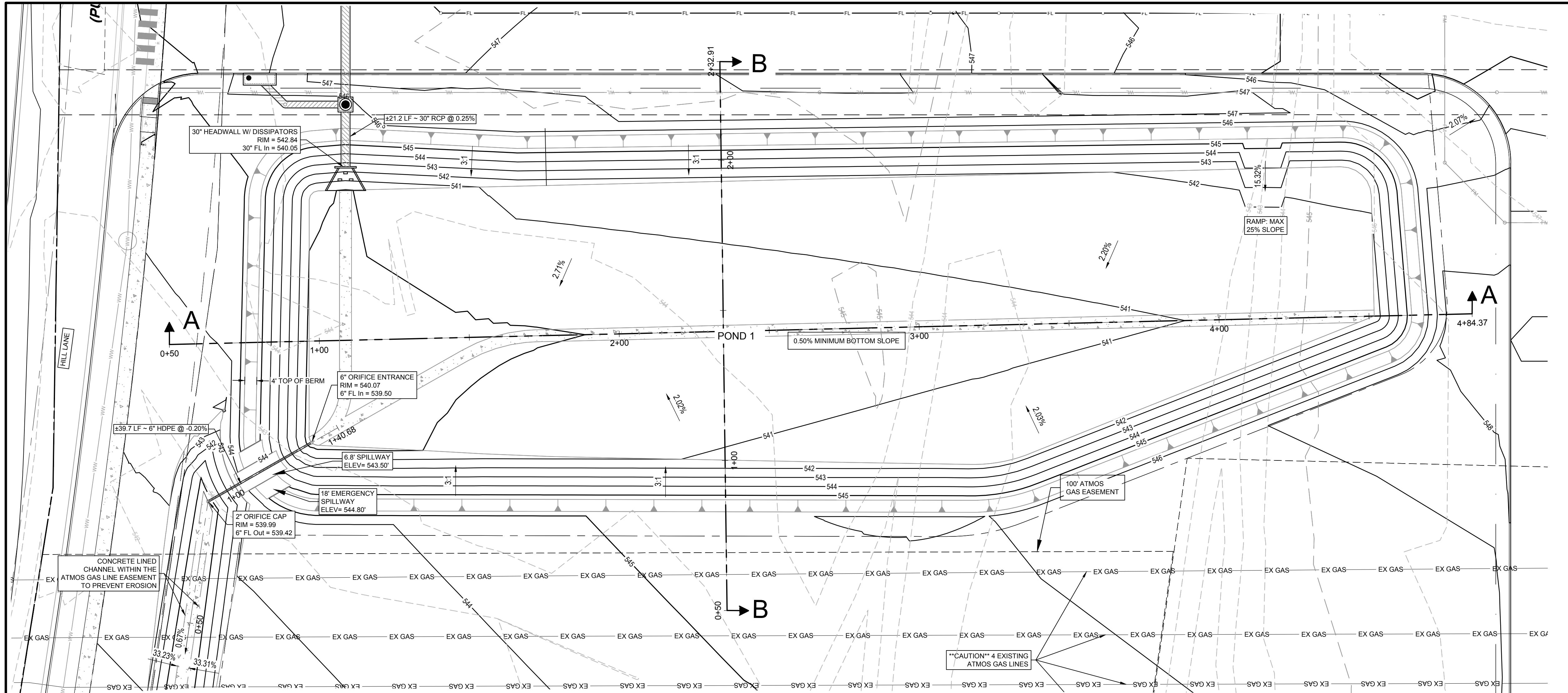
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DESIGN DRAWN CHKO  
MC LB MK  
JOB No. 070422-01-002  
SHEET

56 OF 79



**EXHIBIT 11 – DETENTION PONDS: ULTIMATE CONDITIONS**



A scale bar diagram for architectural drawings. It features a horizontal line with tick marks at 0, 20, and 40. The segment from 0 to 20 is divided into four equal parts by intermediate tick marks. The segment from 20 to 40 is also divided into four equal parts. Below the line, the text "SCALE: 1 INCH = 20 FT" is printed.

## LEGEND

---

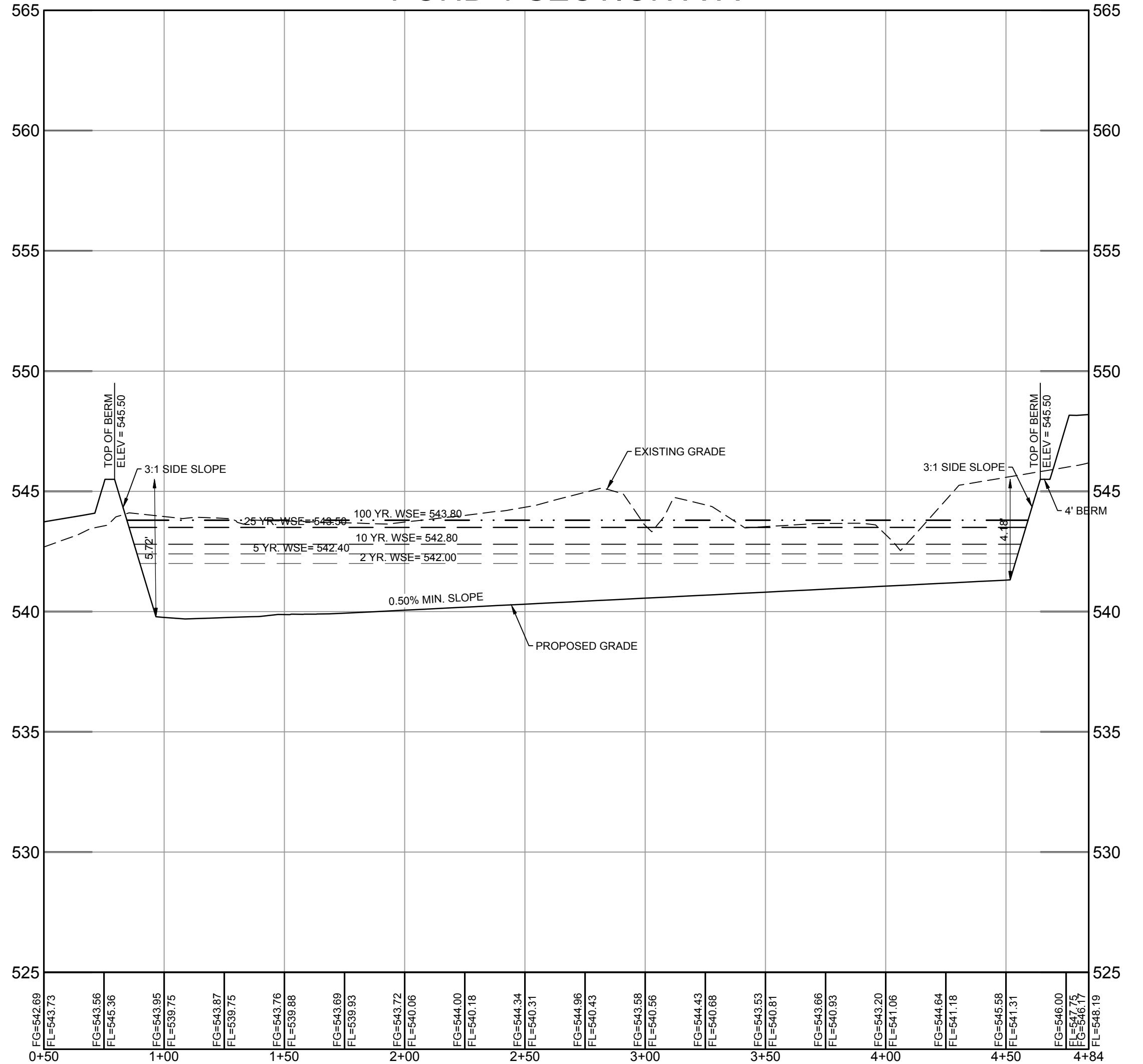
- The legend includes the following entries:

  - PROPOSED R.O.W. (dashed line)
  - PROPOSED LOT LINE (dashed line)
  - EASEMENT LINE (dashed line)
  - PROPOSED CURB & GUTTER (solid line)
  - EXISTING MAJOR CONTOUR (dashed line)
  - EXISTING MINOR CONTOUR (dashed line)
  - PROPOSED MAJOR CONTOUR (solid line)
  - PROPOSED MINOR CONTOUR (solid line)
  - WW (WASTEWATER LINE) (solid line)
  - WL (WATER LINE) (solid line)
  - STORM DRAIN LINE (hatched line)
  - PROPOSED BERM (solid line)
  - PROPOSED SWALE (dotted line)
  - PROPOSED ROCK BERM (hatched rectangle)

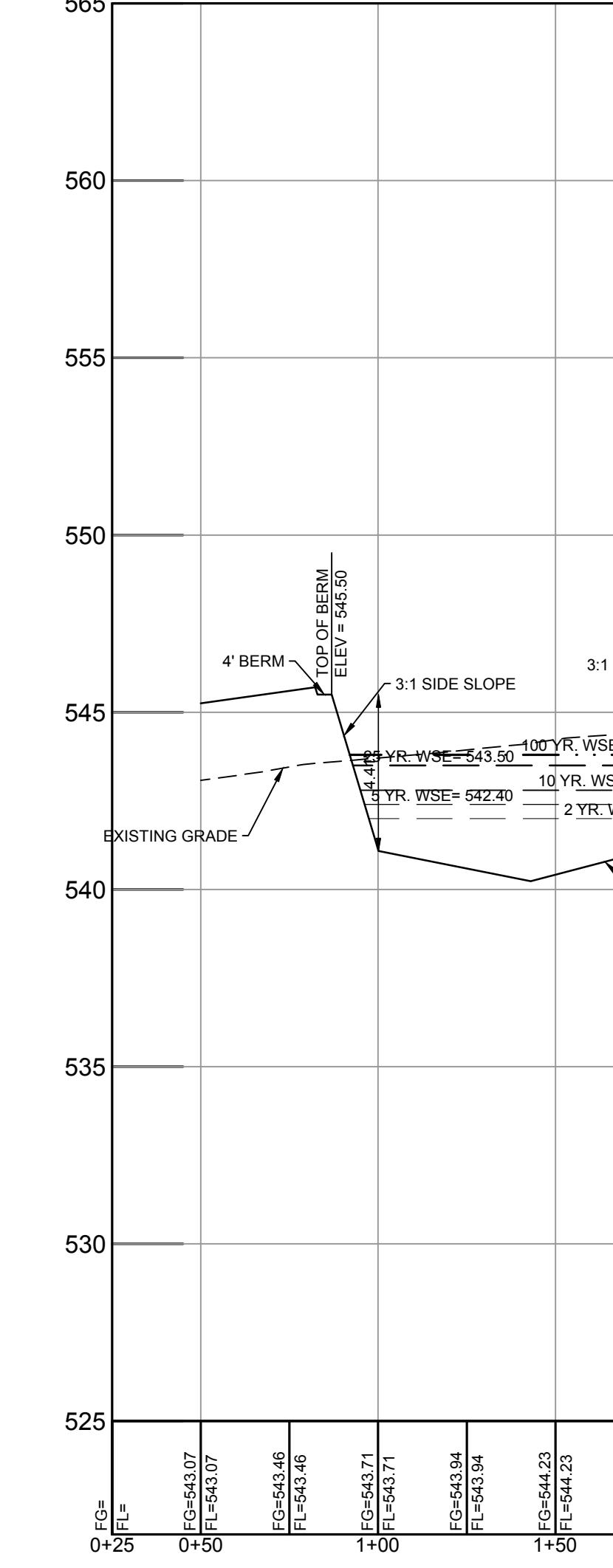
Below the legend are four symbols with labels:

  - 
  - HEADWALL
  - 
  - CURB INLET
  - 
  - AREA INLET
  - 
  - STORMSEWER MANHOLE

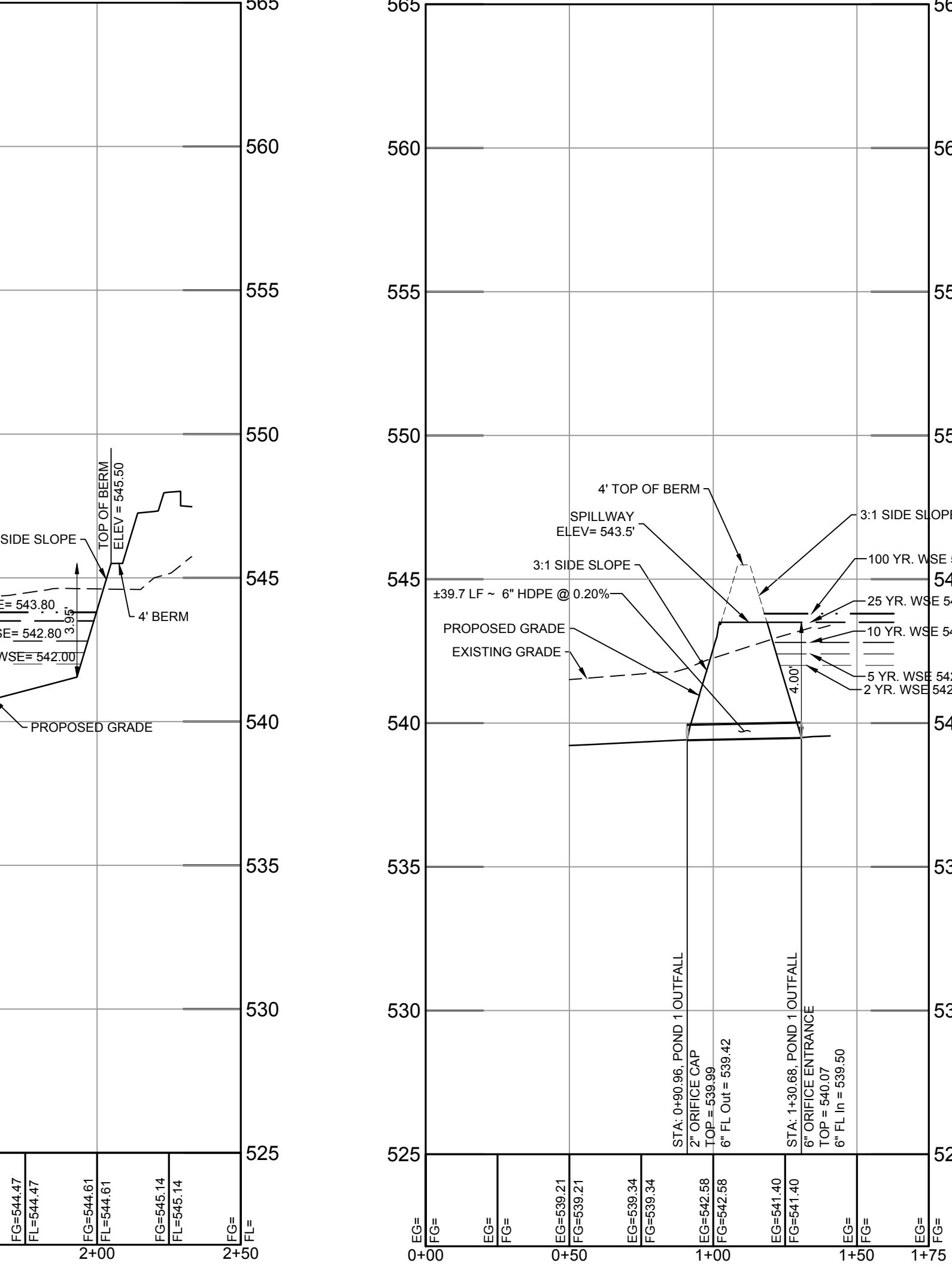
## POND 1 SECTION A-



## POND 1 SECTION B



# POND 1 OUTFALL



PROFILE SCALE 1:40

Know what's below.  
**Call** before you dig.

KNOW WHAT'S BELOW.  
CALL BEFORE YOU DIG.

DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK. CONTRACTOR AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

**POND 1**

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**ANOR DOWNS INDUSTRIAL - SITE PLAN B**  
8500 HILL LN  
MANOR, TEXAS, 78653

M		
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DESIGN MC	DRAWN MG	CHKD MK
B No. 070422-01-002		
SHEET		
27 OF 43		

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 #141397  
 ON AUGUST 2025.  
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DESIGN DRAWN CHKD  
 MC MG MK

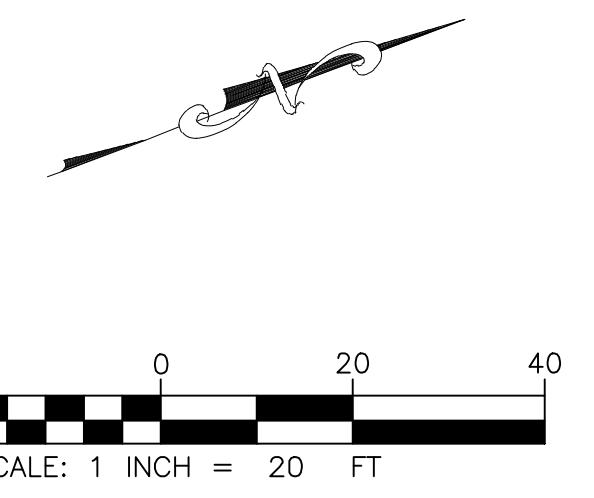
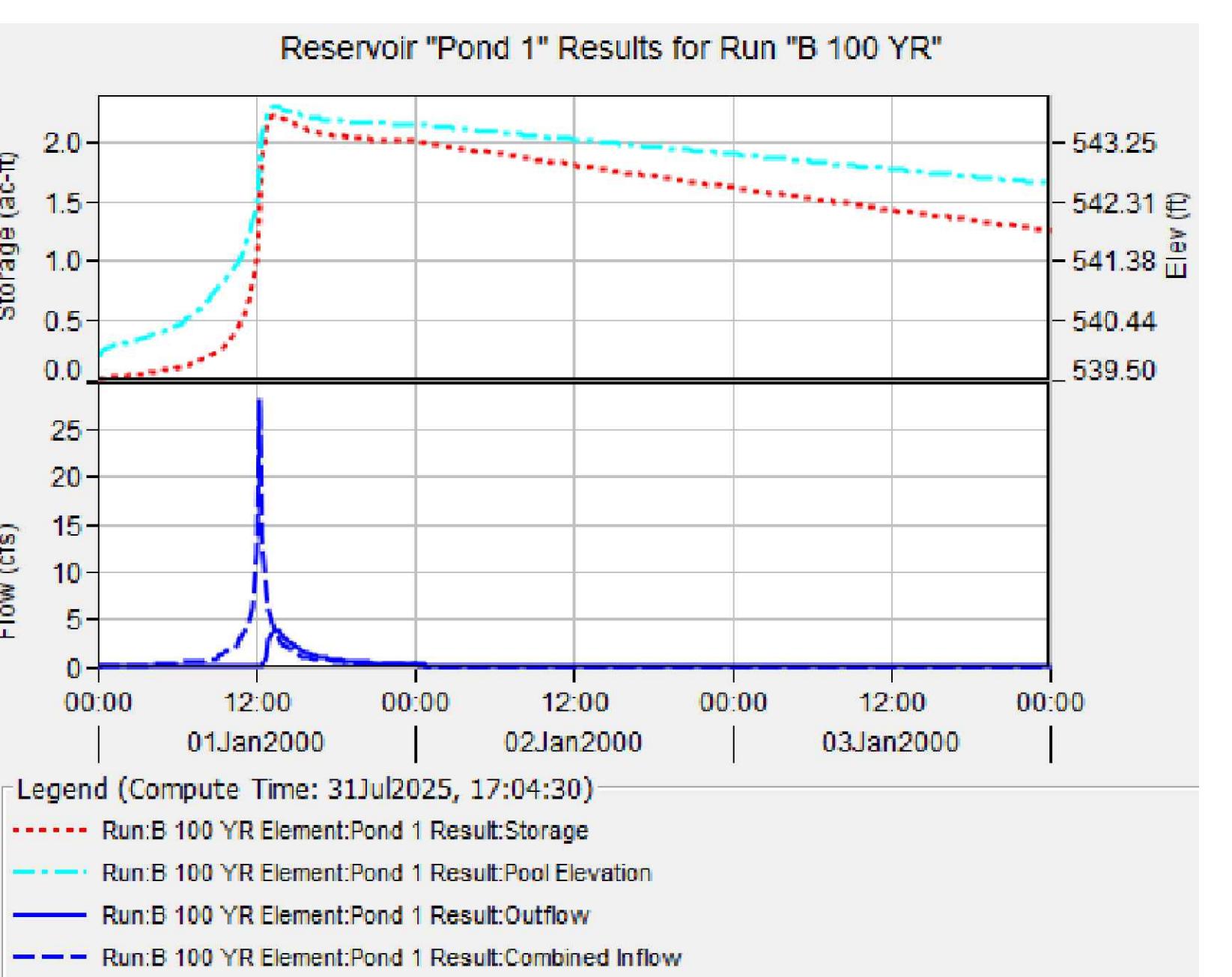
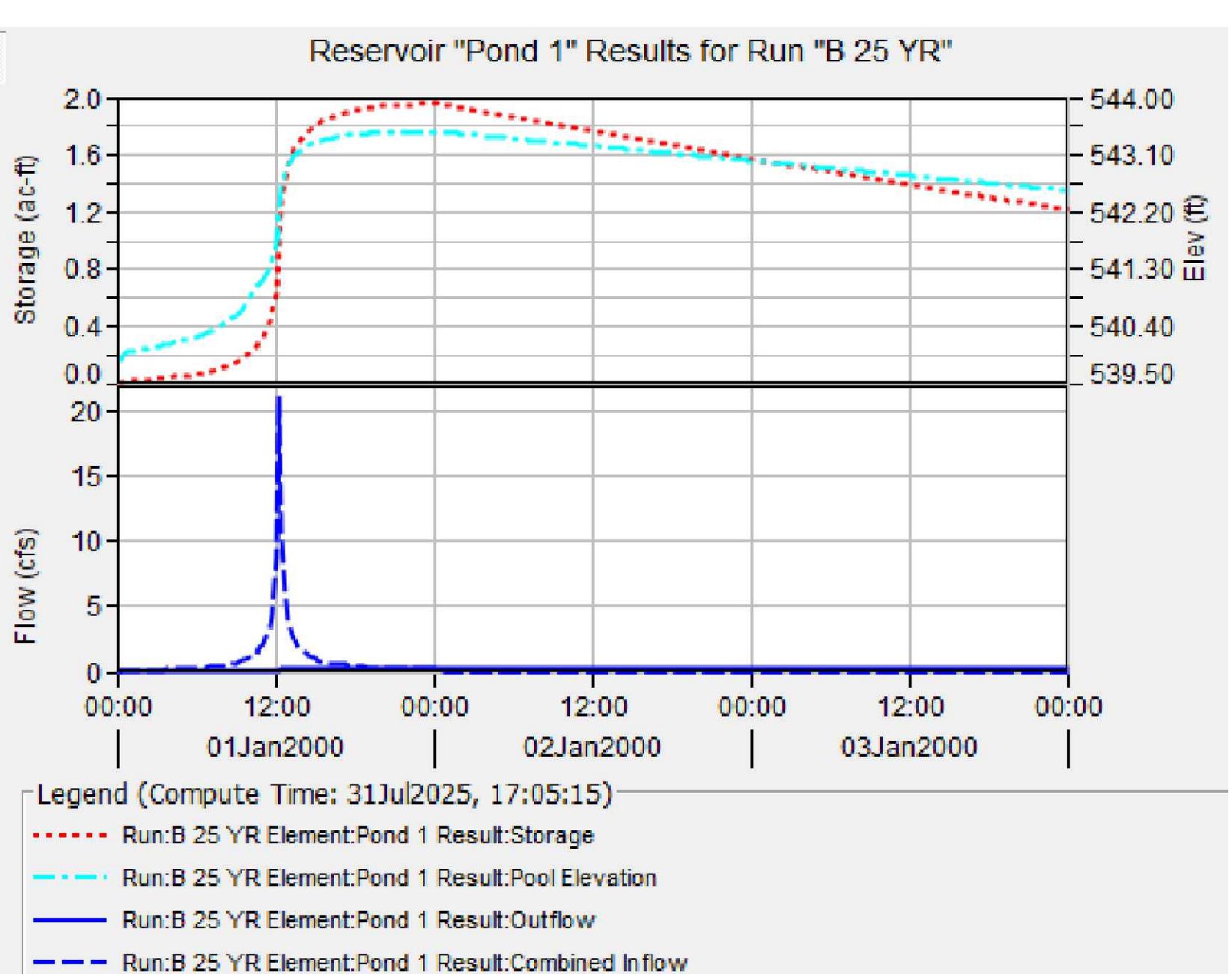
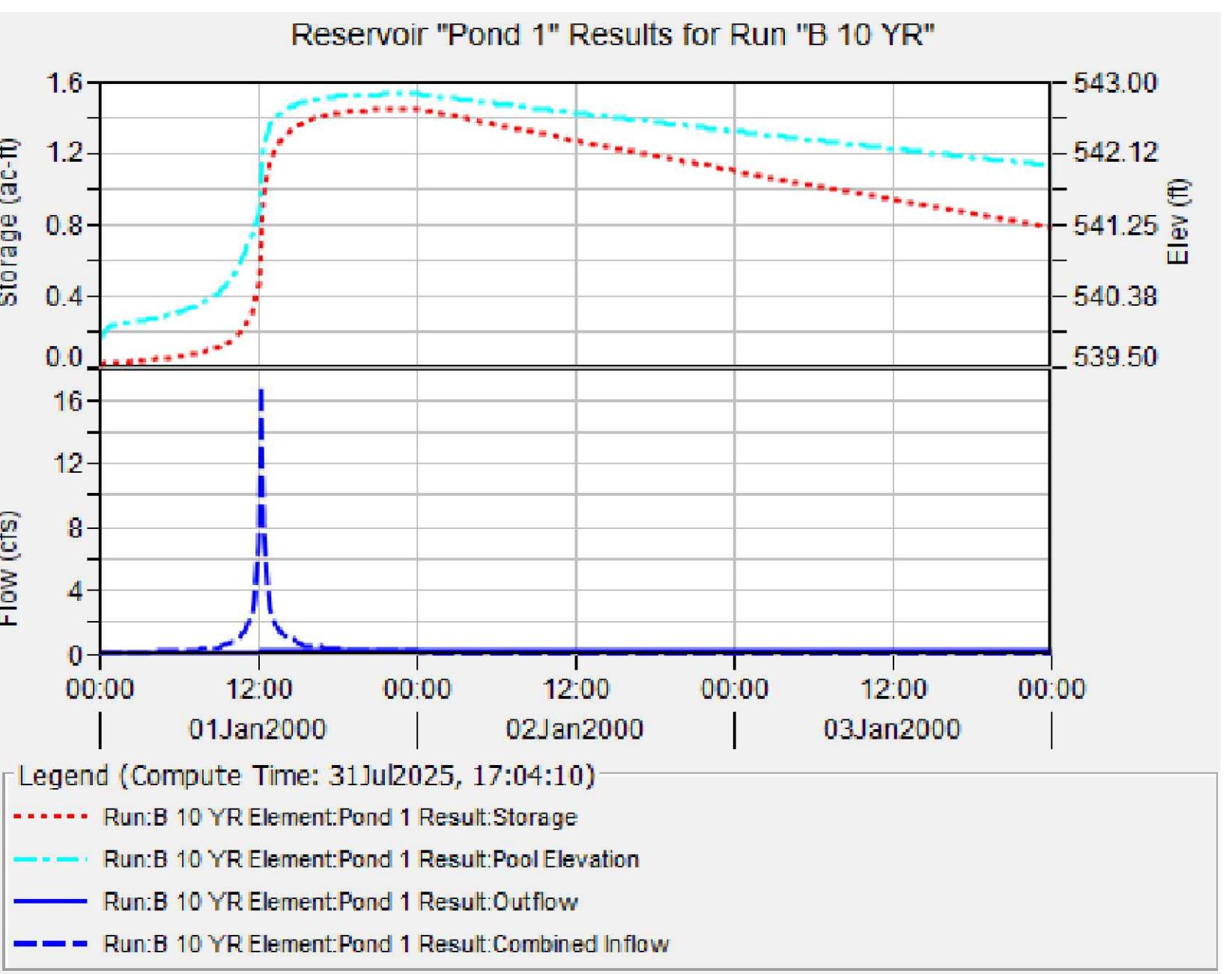
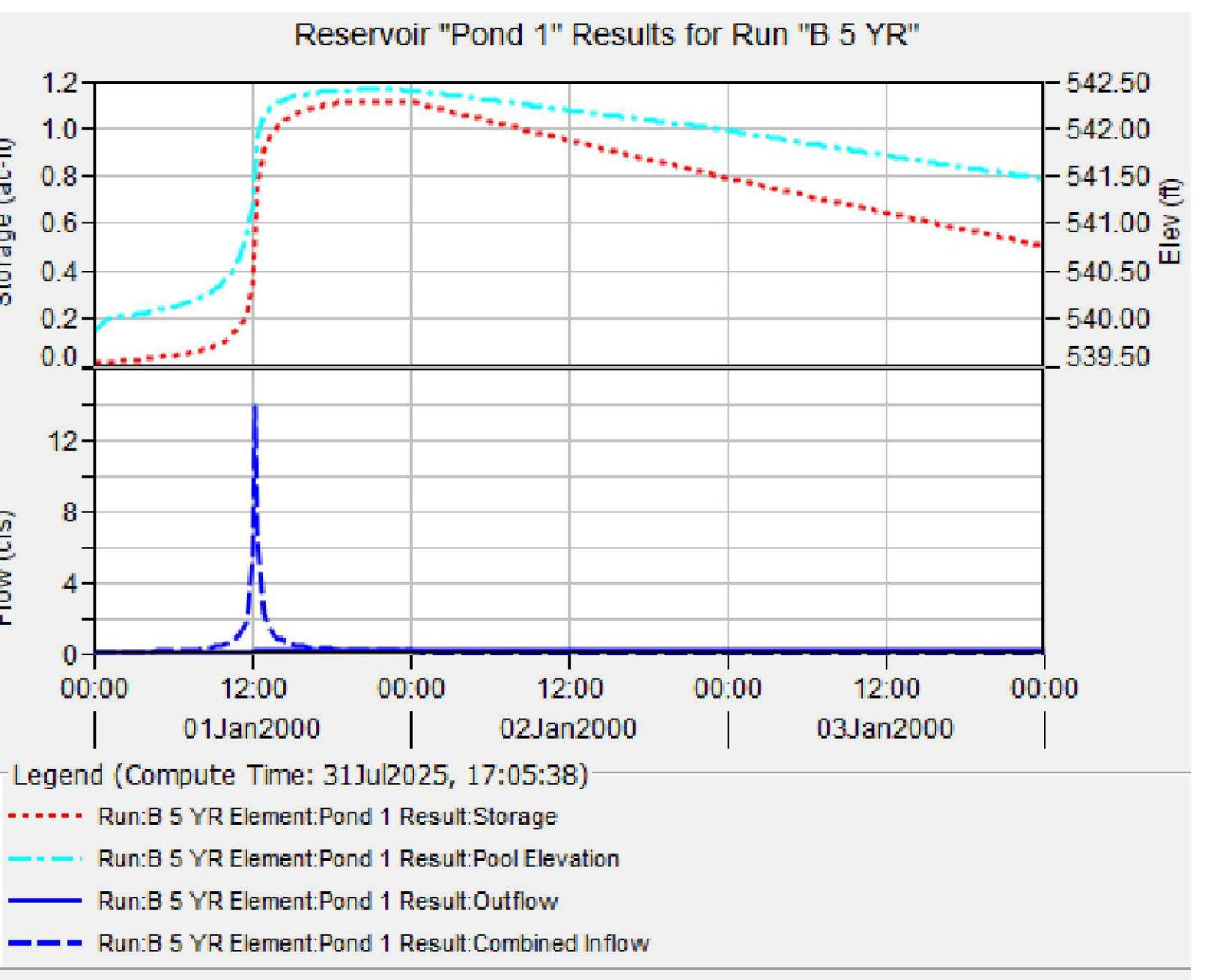
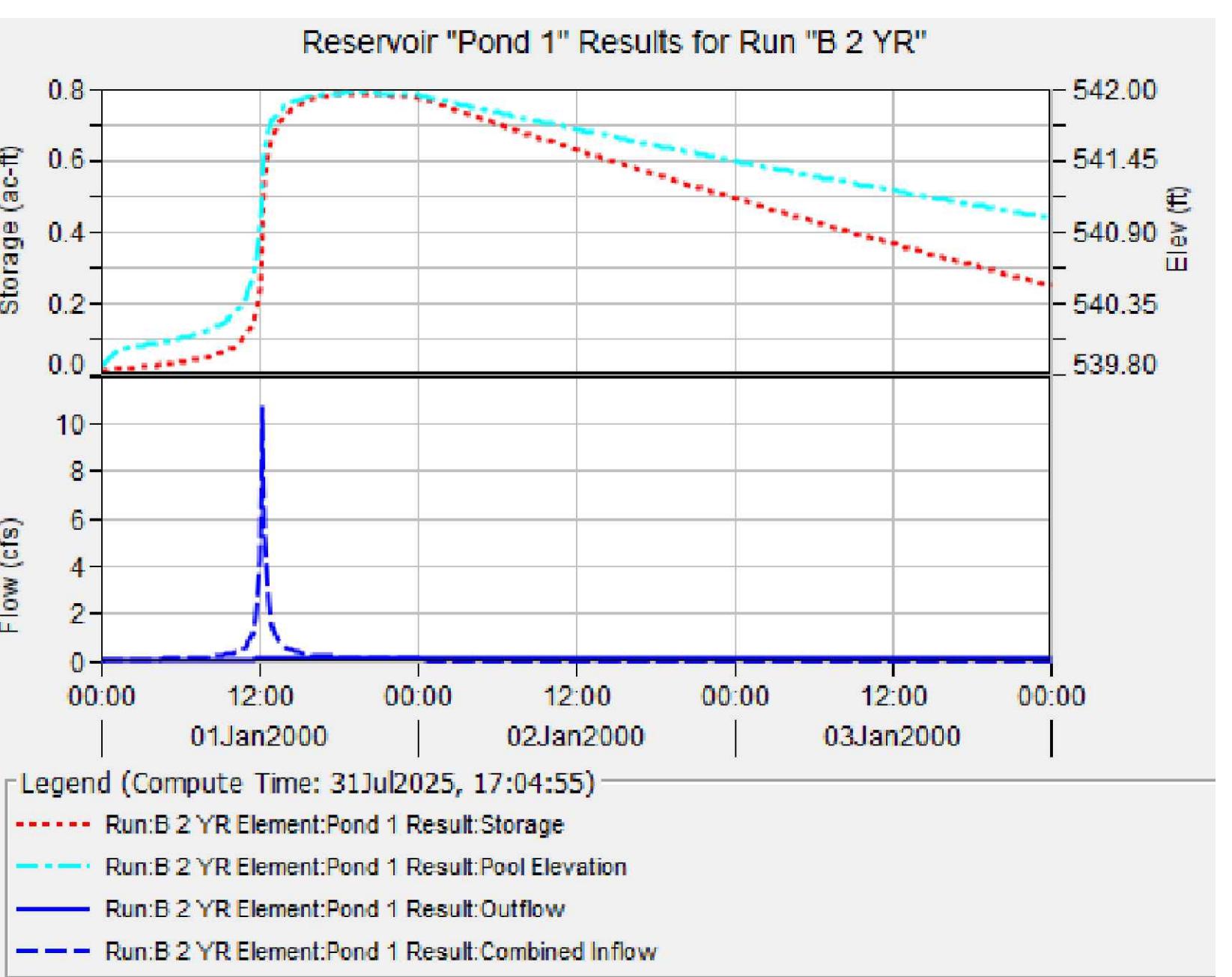
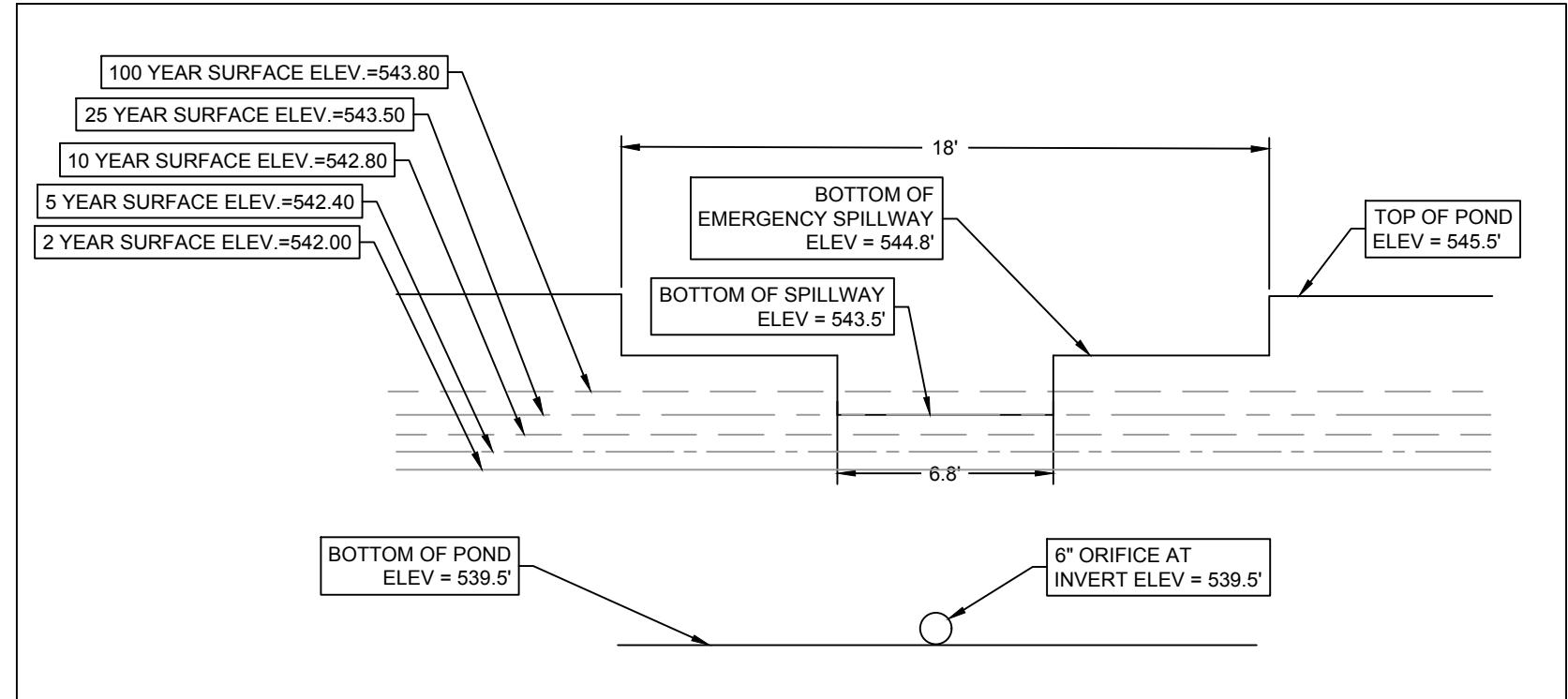
JOB No. 070422-01-002

SHEET

28 OF 43

POND 1 SUMMARY TABLE					
EVENT	INFLOW (cfs)	OUTFLOW (cfs)	DELTA (cfs)	WSE Ft	STORAGE Ac-ft
2-year	10.7	0.2	10.5	542.0	0.80
5-year	14.0	0.2	13.8	542.4	1.10
10-year	16.9	0.2	16.7	542.8	1.40
25-Year	21.1	0.2	20.9	543.5	2.00
100-Year	27.2	0.3	26.9	543.8	2.20

POND 1 STAGE-AREA-STORAGE TABLE						
ELEV	AREA (sq. ft.)	DEPTH (ft)	Avg End Inc. Vol. (cu. ft.)	Avg End Total Vol. (cu. ft.)	Conic Inc. Vol. (cu. ft.)	Conic Total Vol. (cu. ft.)
539.5	0	N/A	N/A	0	N/A	0
540.0	4076	0.5	1019	1019	679	679
541.0	17551	1.0	10814	11833	10028	10708
542.0	31669	1.0	24610	36443	24265	34973
543.0	34805	1.0	33237	69679	33224	68197
544.0	37551	1.0	36178	105857	36169	104367
545.0	40324	1.0	38938	144795	38929	143296
545.5	41744	0.5	20517	165312	20516	163812



SCALE: 1 INCH = 20 FT

LEGEND

- PROPOSED R.O.W.
- PROPOSED LOT LINE
- EASEMENT LINE
- PROPOSED CURB & GUTTER
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- WASTEWATER LINE
- WATER LINE
- STORM DRAIN LINE
- PROPOSED BERM
- PROPOSED SWALE
- PROPOSED ROCK BERM
- HEADWALL
- CURB INLET
- AREA INLET
- STORMSEWER MANHOLE

REVISIONS

DESCRIPTION

DATE

REVISION

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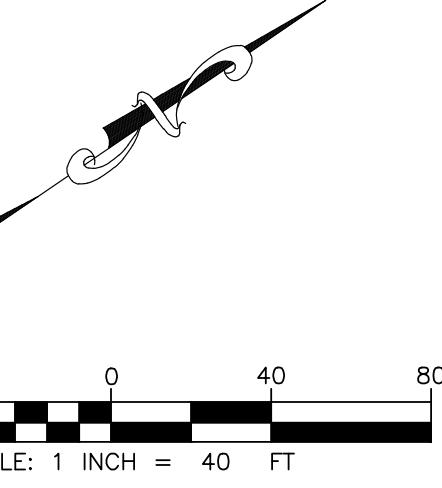
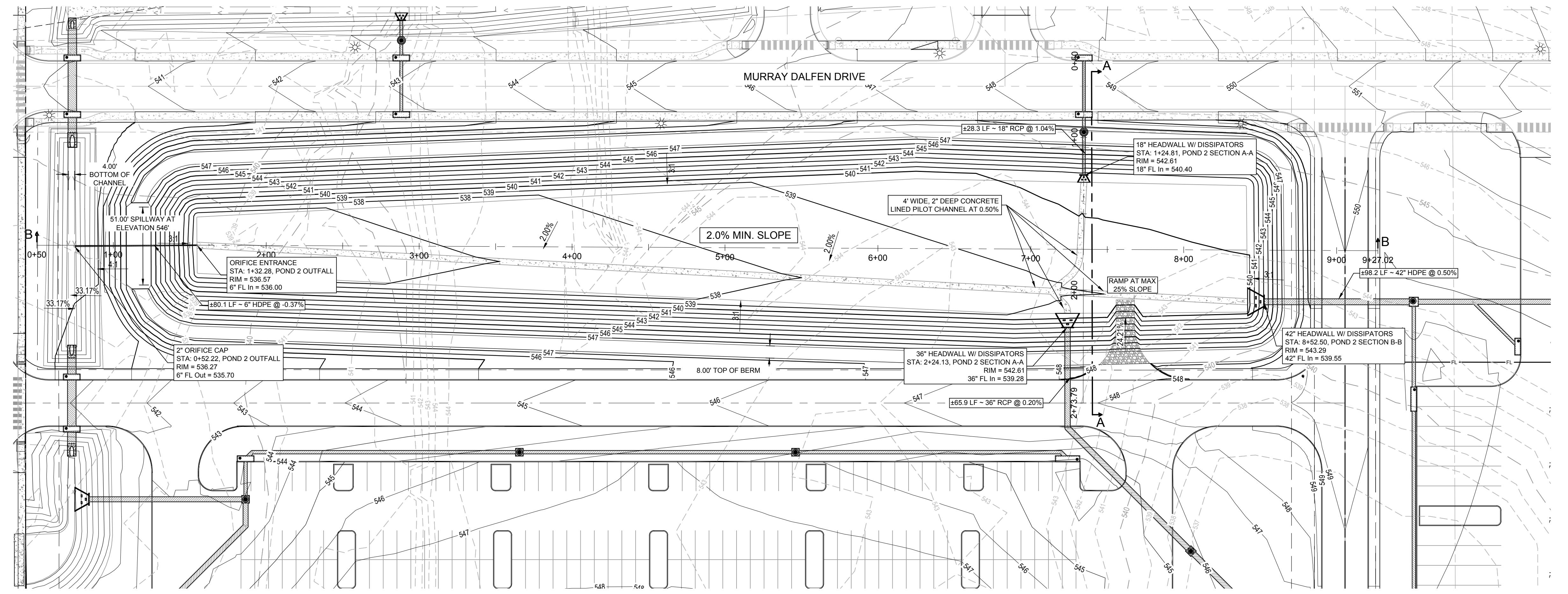
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A scale bar diagram for architectural drawings. It features a horizontal line with tick marks at 0, 40, and 80. The first 40 units are marked with a pattern of black and white squares, while the remaining 40 units are solid black. Below the line, the text "SCALE: 1 INCH = 40 FT" is printed.

## LEGEND

---

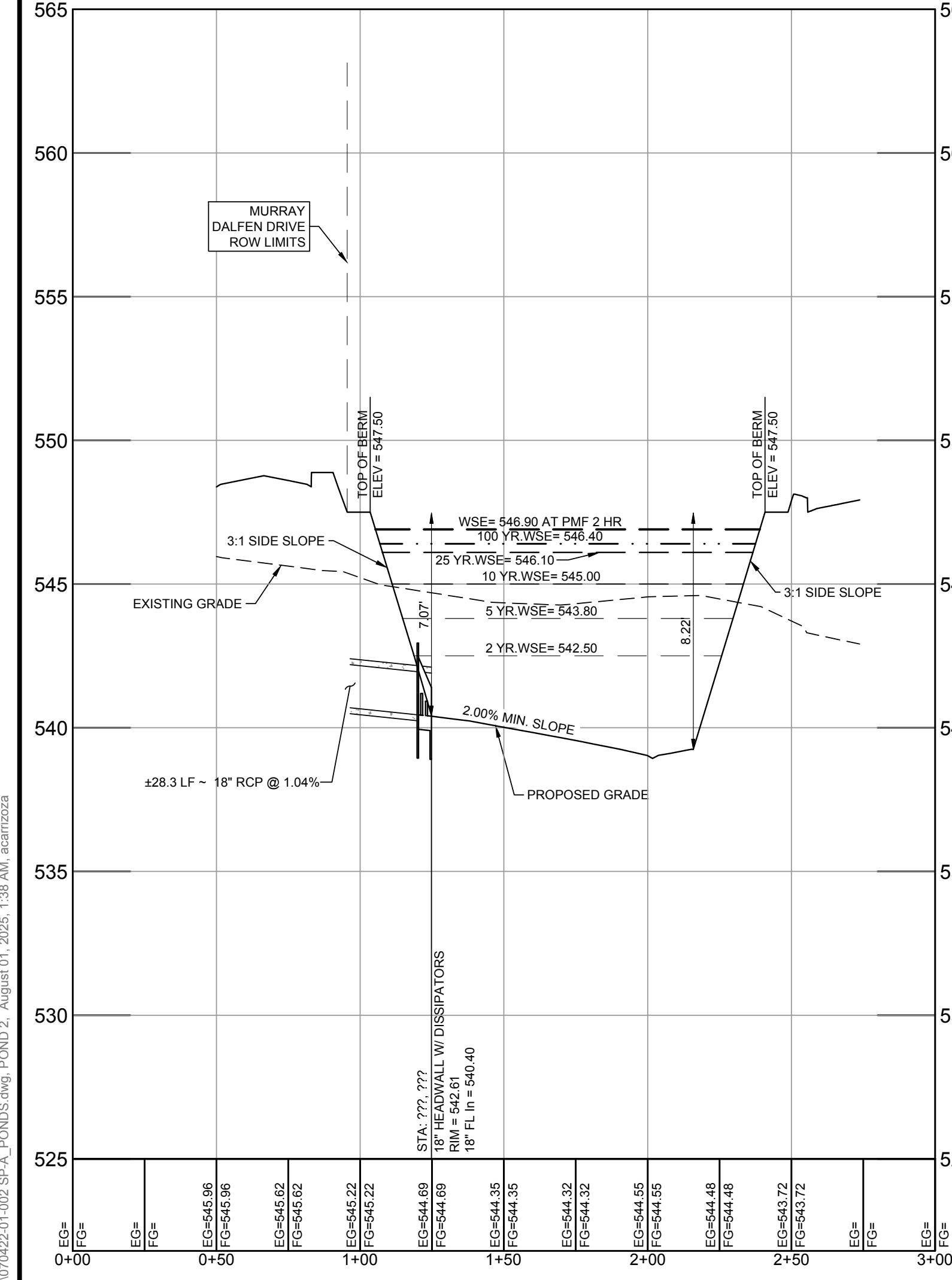
- This legend provides a key for interpreting the symbols and line types used in the site plan:

  - Dash-dot line: PROPOSED R.O.W.
  - Solid line: PROPOSED LOT LINE
  - Solid line: SITE BOUNDARY
  - Dash-dot line: EASEMENT LINE
  - Solid line: PROPOSED CURB & GUTTER
  - Solid line: EXISTING MAJOR CONTOUR
  - Dashed line: EXISTING MINOR CONTOUR
  - Solid line: PROPOSED MAJOR CONTOUR
  - Solid line: PROPOSED MINOR CONTOUR
  - Solid line: WW: WASTEWATER LINE
  - Solid line: WL: WATER LINE
  - Shaded line: STORM DRAIN LINE
  - Solid line: PROPOSED BERM
  - Solid line: PROPOSED SWALE
  - Block with circles: PROPOSED ROCK BERM

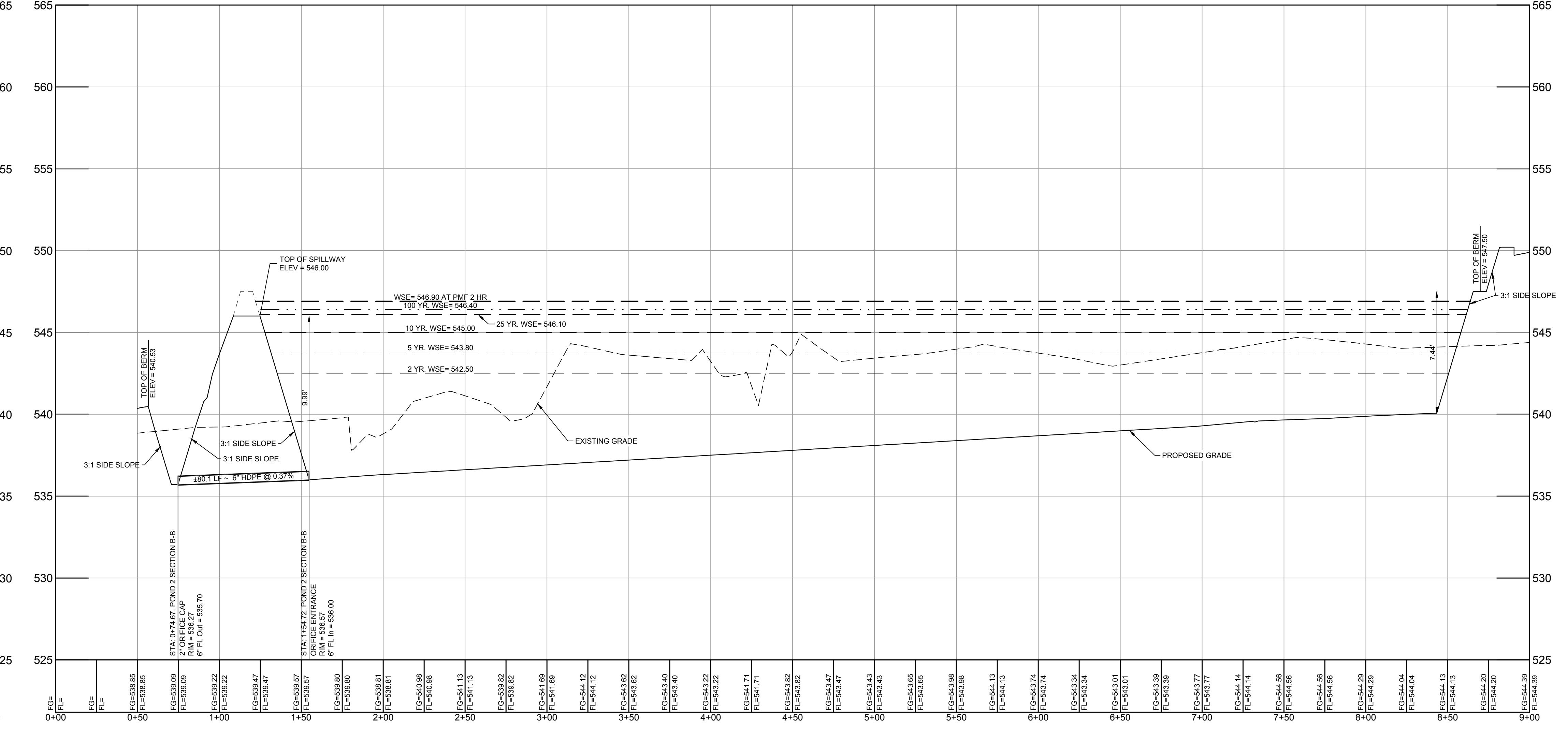
Below the legend are two symbols: a triangle pointing down-right labeled "HEADWALL" and a downward-pointing arrowhead symbol.

ES:  
ALL EARTHEN EMBANKMENTS SHALL BE COMPAKTED TO 95% OF MAXIMUM DENSITY IN ACCORDANCE WITH COA STANDARD SPECIFICATIONS.  
EARTHEN EMBANKMENTS SIDE SLOPES SHALL BE NO STEEPER THAN 3 HORIZONTAL TO 1 VERTICAL.

## POND 2 SECTION A-A



## POND 2 SECTION B-B



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ON AUGUST 1 2025.  
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PERMIT PURPOSES.

DESIGN MC	DRAWN LB	CHKD MK
--------------	-------------	------------

B No. 070422-01-002  
SHEET

57 OF 79  
- FOR CONSTRUCTION

## POND 2 CALCULATIONS

MANOR DOWNS INDUSTRIAL - SITE PLAN A  
 8916 HILL LN  
 MANOR, TEXAS, 78653

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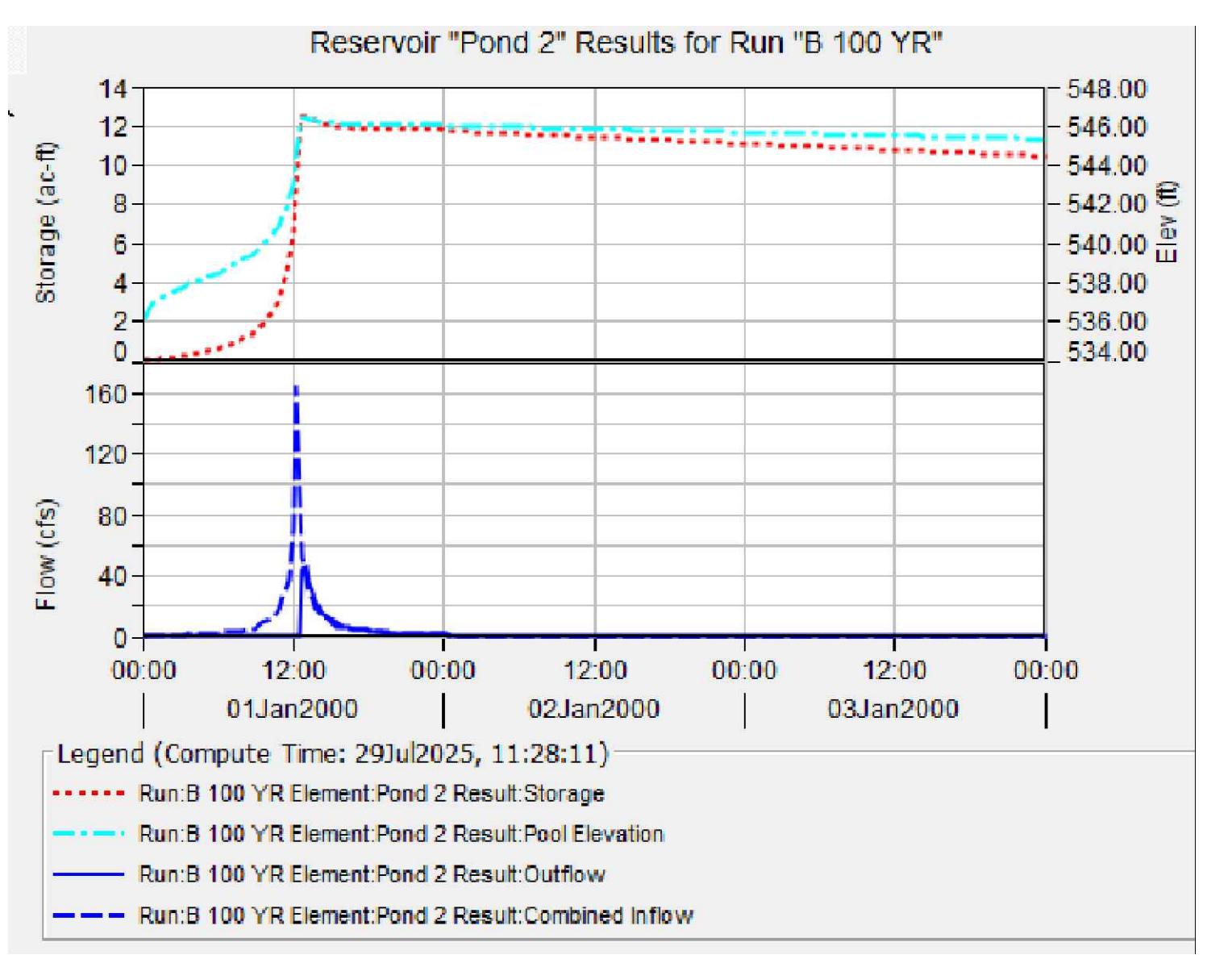
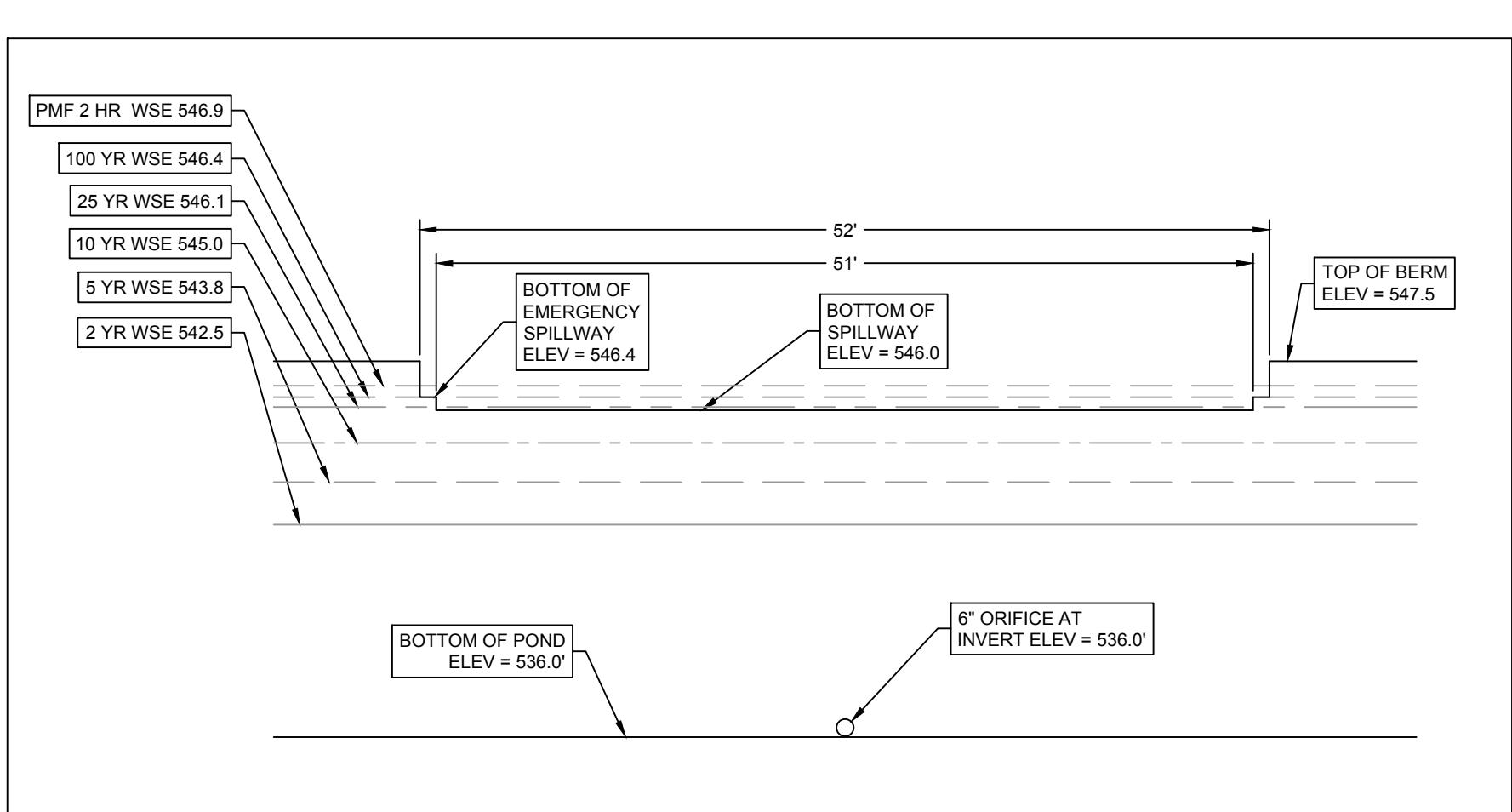
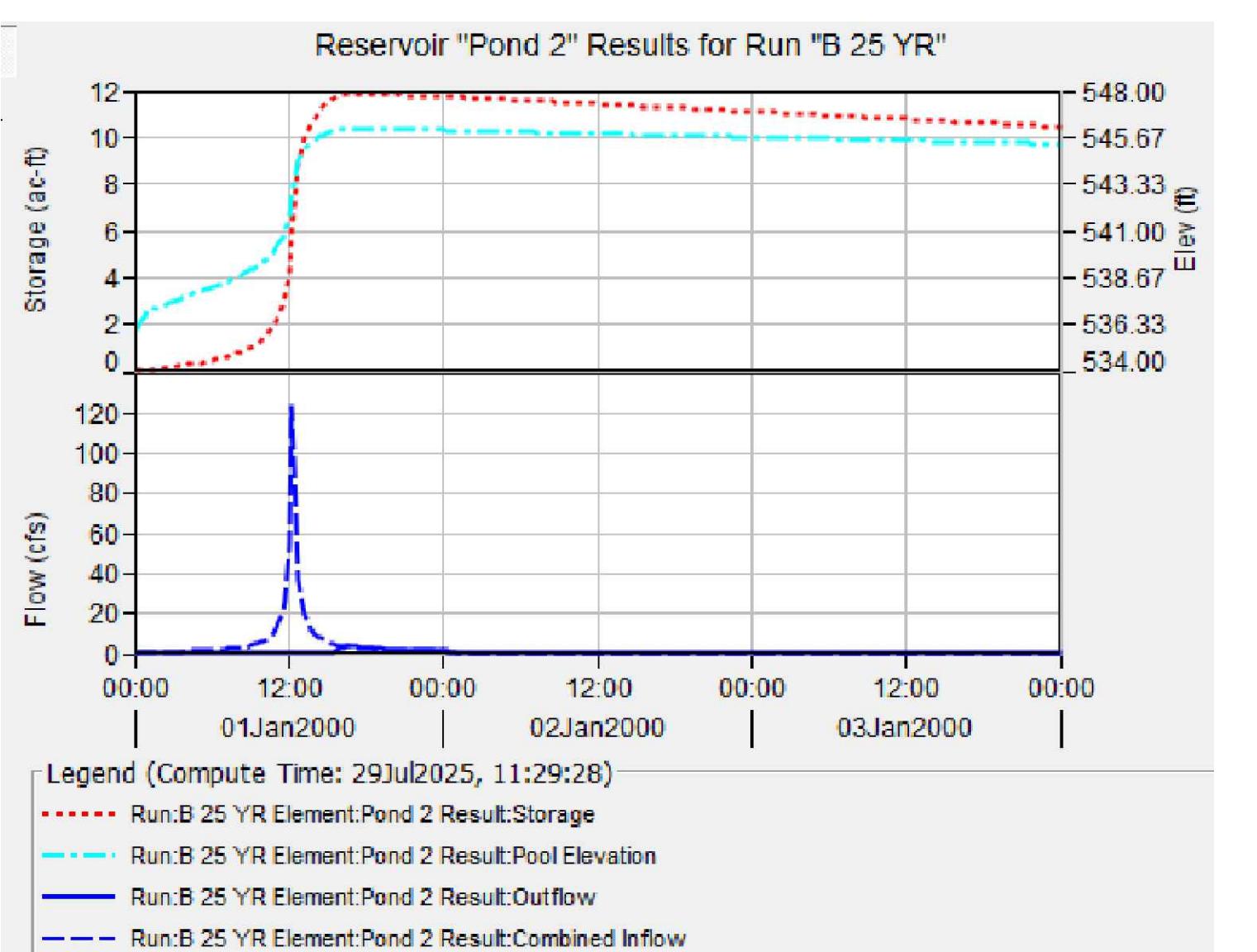
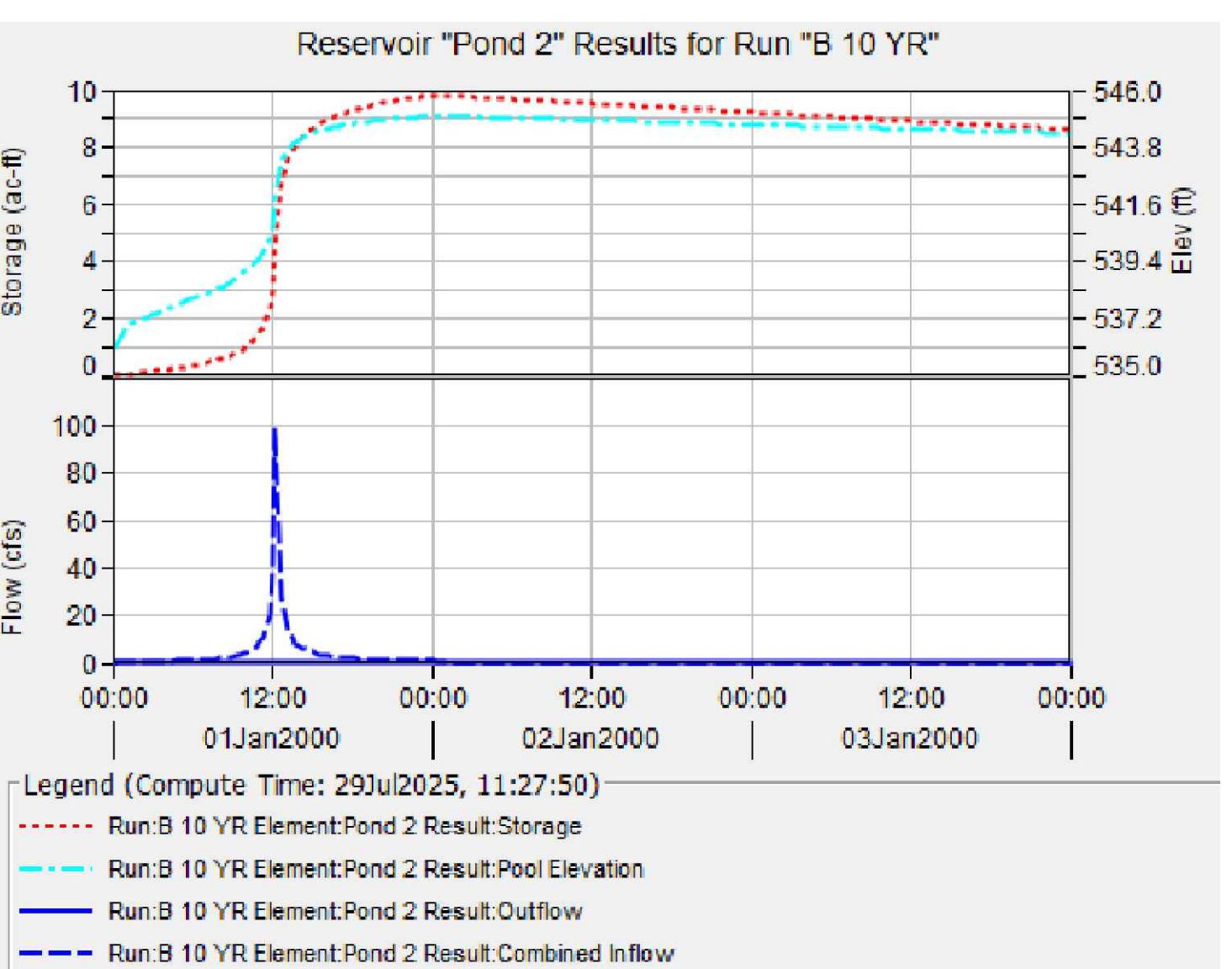
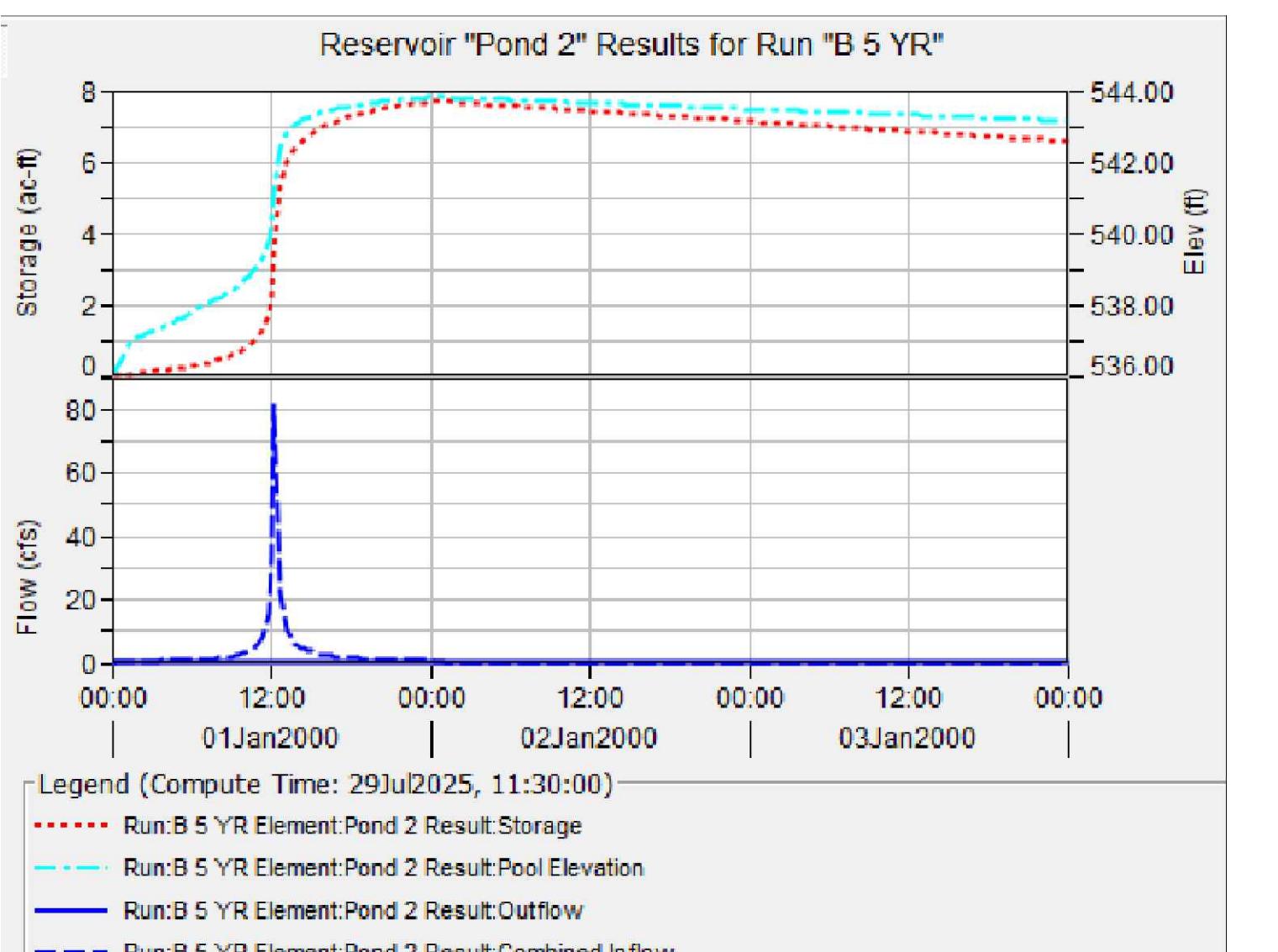
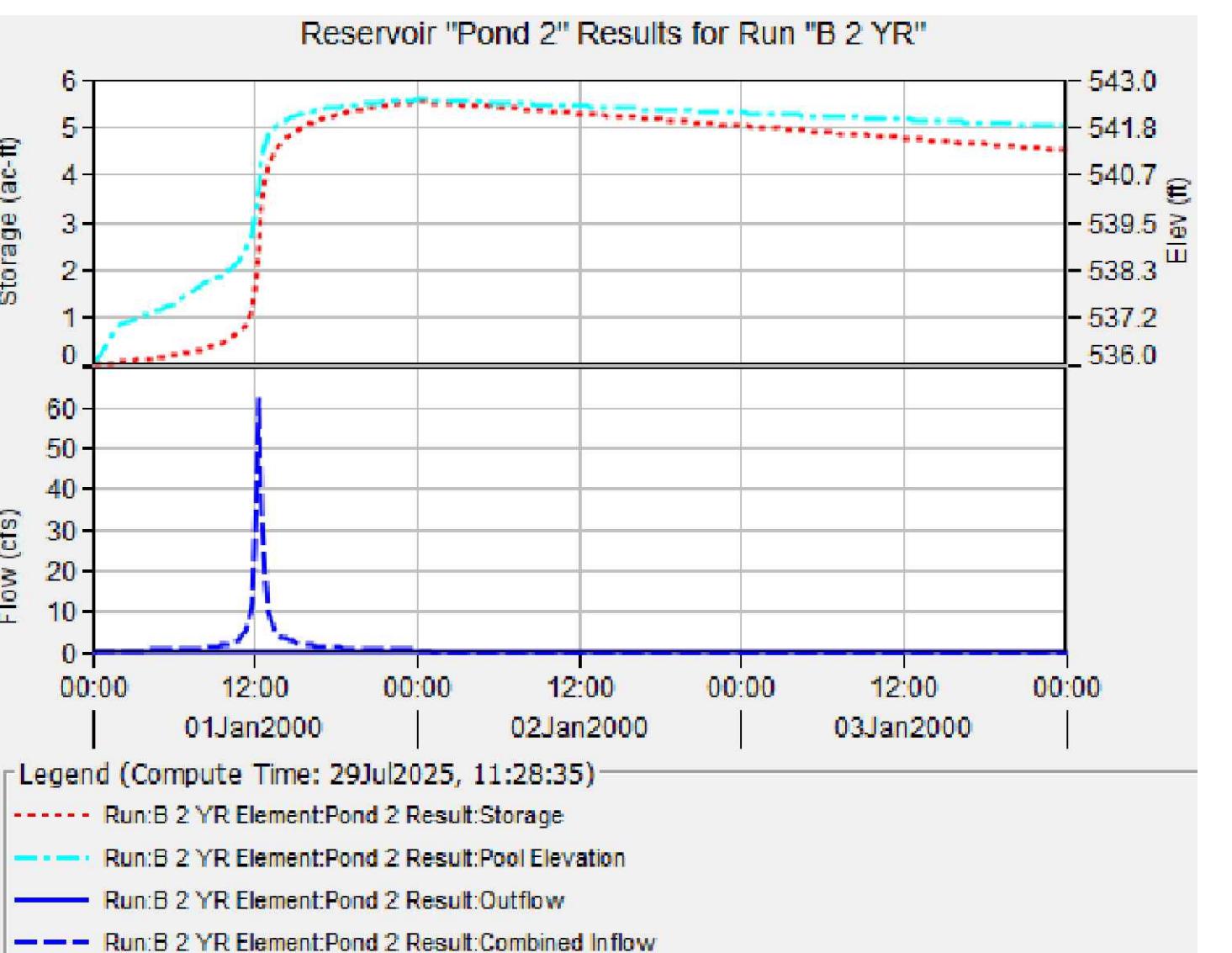
DESIGN MC DRAWN LB CHKD MK  
 SHEET

JOB No. 070422-01-002  
 SHEET  
 58 OF 79

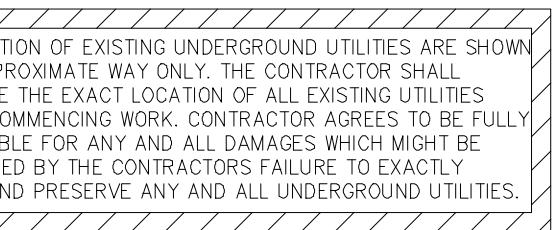
POND 2 SUMMARY					
EVENT	INFLOW (cfs)	OUTFLOW (cfs)	DELTA (cfs)	WSE Ft	STORAGE Ac-ft
2-year	62.70	0.30	62.40	542.50	5.50
5-year	81.80	0.30	81.50	543.80	7.70
10-year	99.30	0.30	99.00	545.00	9.80
25-Year	124.00	3.50	120.50	546.10	11.90
100-Year	165.50	45.20	120.30	546.40	12.60

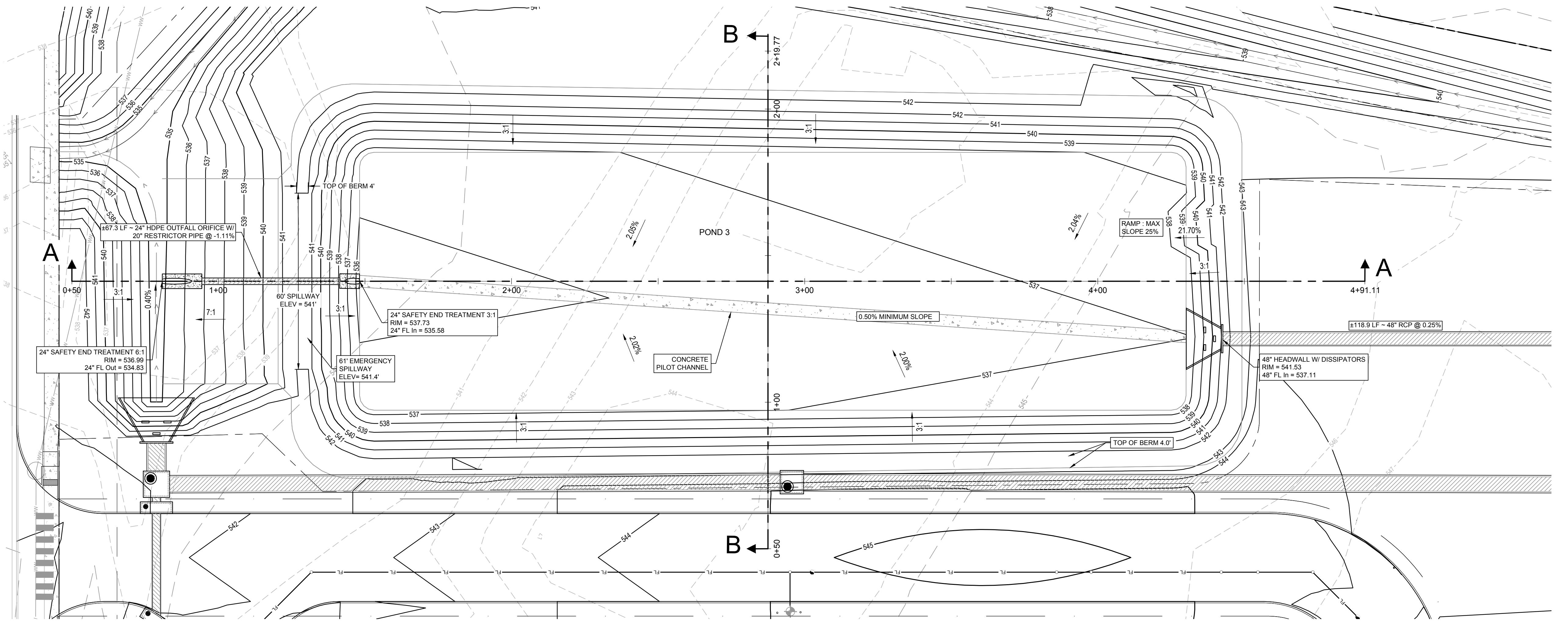
STAGE STORAGE TABLE						
ELEV	AREA (sq. ft.)	DEPTH (ft)	Avg End Inc. Vol. (cu. ft.)	Avg End Total Vol. (cu. ft.)	Conic Inc. Vol. (cu. ft.)	Conic Total Vol. (cu. ft.)
536.000	0.00	N/A	N/A	0.00	N/A	0.00
537.000	6,912.30	1.00	3456.15	3456.15	2304.10	2304.10
538.000	19,183.10	1.00	13047.70	16503.85	12536.86	14840.96
539.000	35,373.29	1.00	27278.19	43782.04	26868.58	41709.54
540.000	52,304.75	1.00	43839.02	87621.06	43563.96	85273.50
541.000	61,714.68	1.00	57009.71	144630.77	56944.88	142218.39
542.000	66,445.97	1.00	64080.32	208711.10	64065.76	206284.15
543.000	71,231.30	1.00	68838.64	277549.74	68824.77	275108.93
544.000	76,071.00	1.00	73651.15	351200.89	73637.90	348746.82
545.000	80,965.89	1.00	78518.44	429719.33	78505.73	427252.55
546.000	85,917.06	1.00	83441.47	513160.81	83429.23	510681.78
547.000	90,854.45	1.00	88385.76	601546.56	88374.26	599056.04
547.500	93,386.85	0.500	46060.33	647606.89	46058.88	645114.92

POND 2 PMF SUMMARY					
PMP FREQUENCY	DEPTH (in)	MAX. ELEV. (ft)	DAY	TIME (hr:min)	PEAK DISCHARGE
1 hr	15.1	546.9	0	1:15	198.3
2 hr	19.9	546.9	0	1:30	213.1
3 hr	22.4	546.8	0	1:30	166.4
6 hr	26.6	546.8	0	2:00	168.0
12 hr	35.4	546.7	0	2:15	131.1
24 hr	42.5	546.6	0	7:45	90.1
48 hr	46.6	546.4	0	8:00	52.6
72 hr	46.6	546.3	0	20:30	35.1



Know what's below.  
Call before you dig.





20 0 20 40  
SCALE: 1 INCH = 20 FT

## LEGEND

- PROPOSED R.O.W.
- PROPOSED LOT LINE
- EASEMENT LINE
- PROPOSED CURB & GUTTER
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- WASTEWATER LINE
- WATER LINE
- STORM DRAIN LINE
- PROPOSED BERM
- PROPOSED SWALE
- PROPOSED ROCK BERM
- HEADWALL
- CURB INLET
- AREA INLET
- STORMSEWER MANHOLE

## REVISIONS

REVISION	DESCRIPTION	DATE
***	***	***

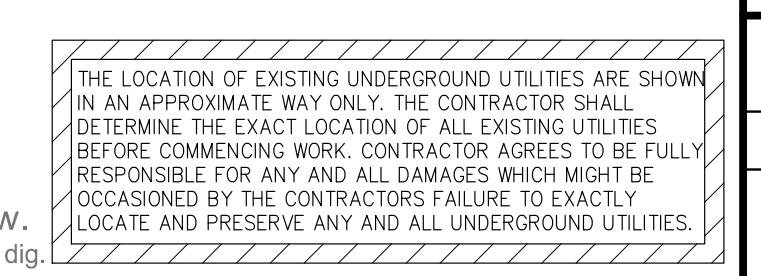
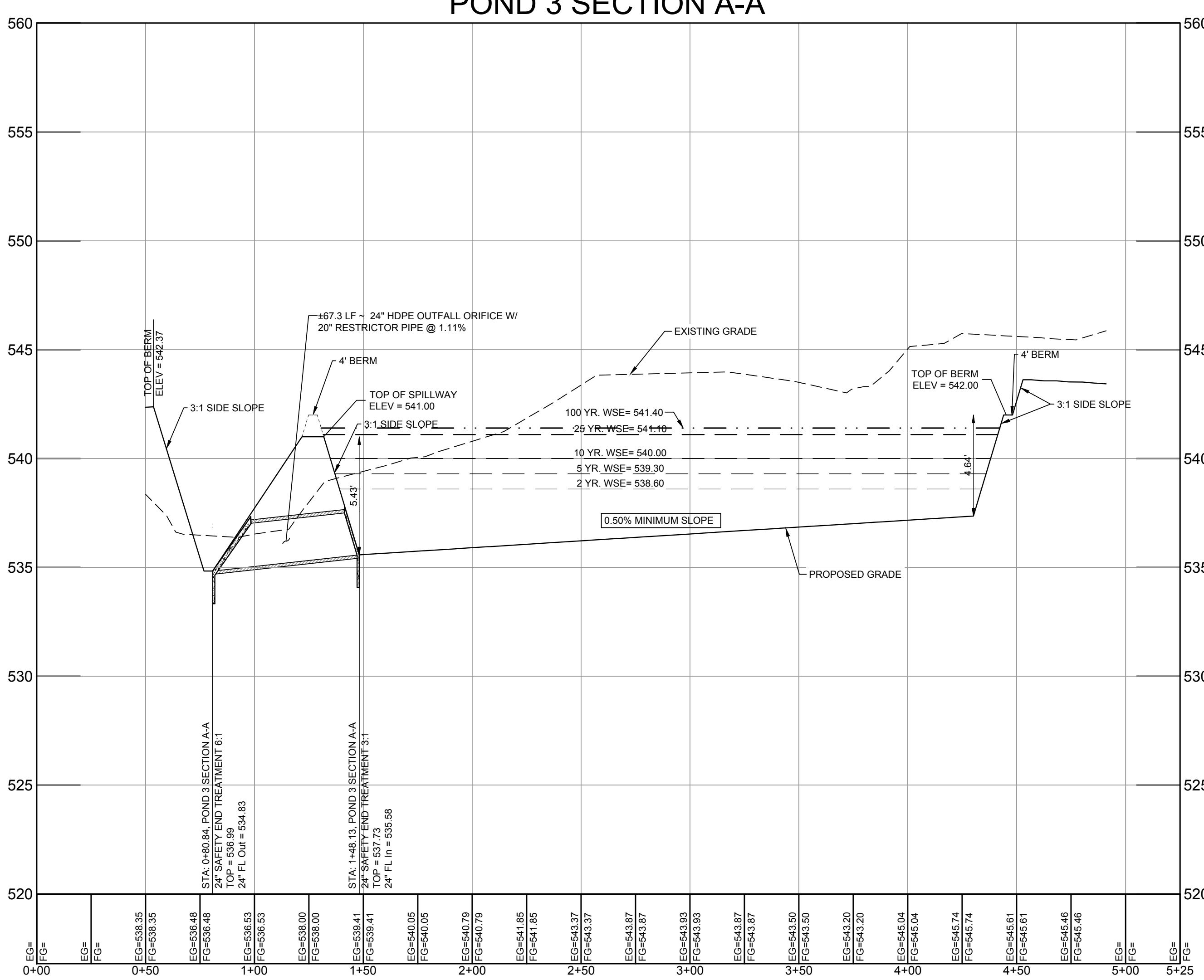
## POND 3

MANOR DOWNS INDUSTRIAL - SITE PLAN B  
8500 HILL LN  
MANOR, TEXAS, 78653

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DESIGN DRAWN CHKD  
MC MG MK  
JOB No. 070422-01-002 SHEET

29 OF 43



## POND 3 CALCULATIONS

MANOR DOWNS INDUSTRIAL - SITE PLAN B

8500 HILL LN

MANOR, TEXAS, 78653

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MC MG MK

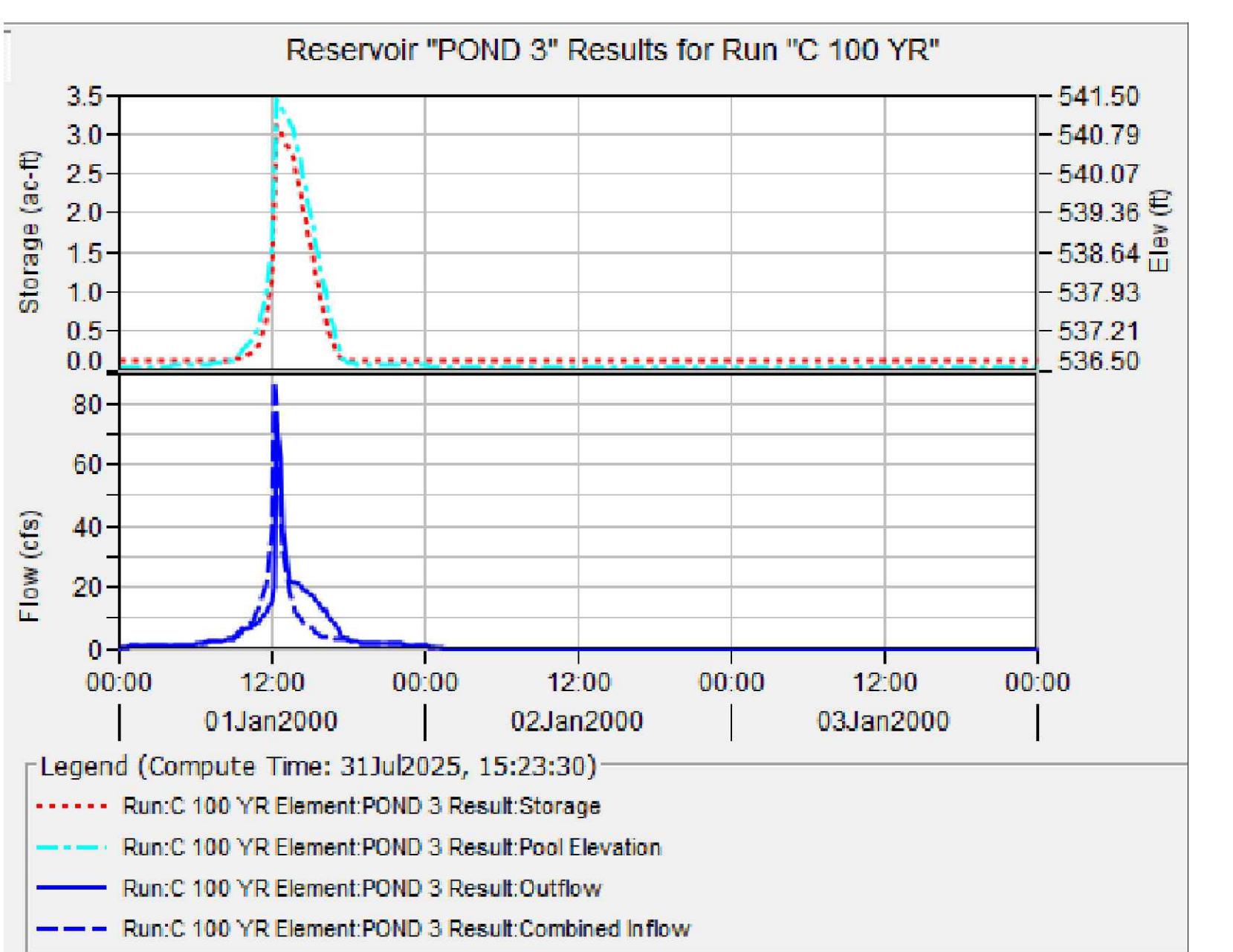
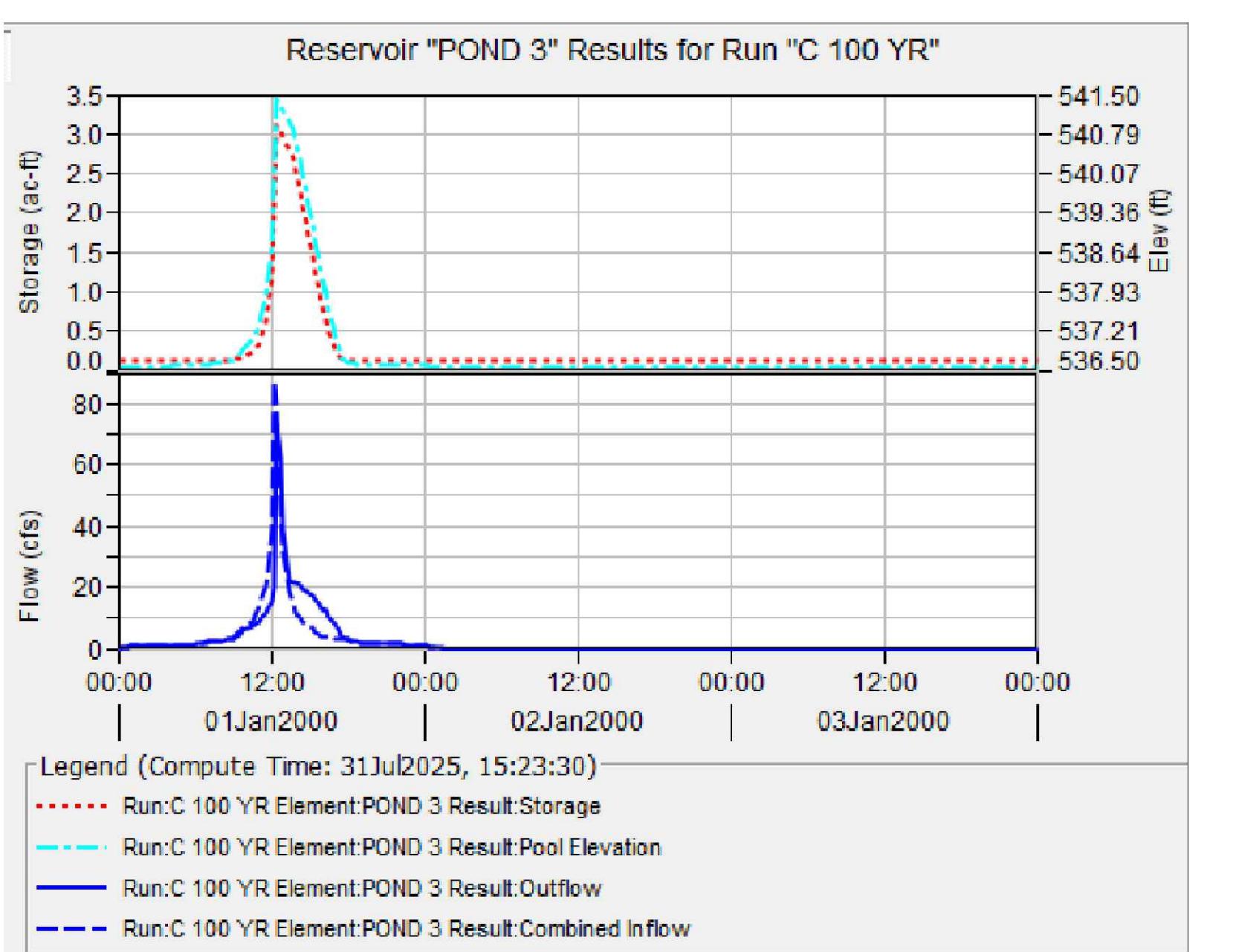
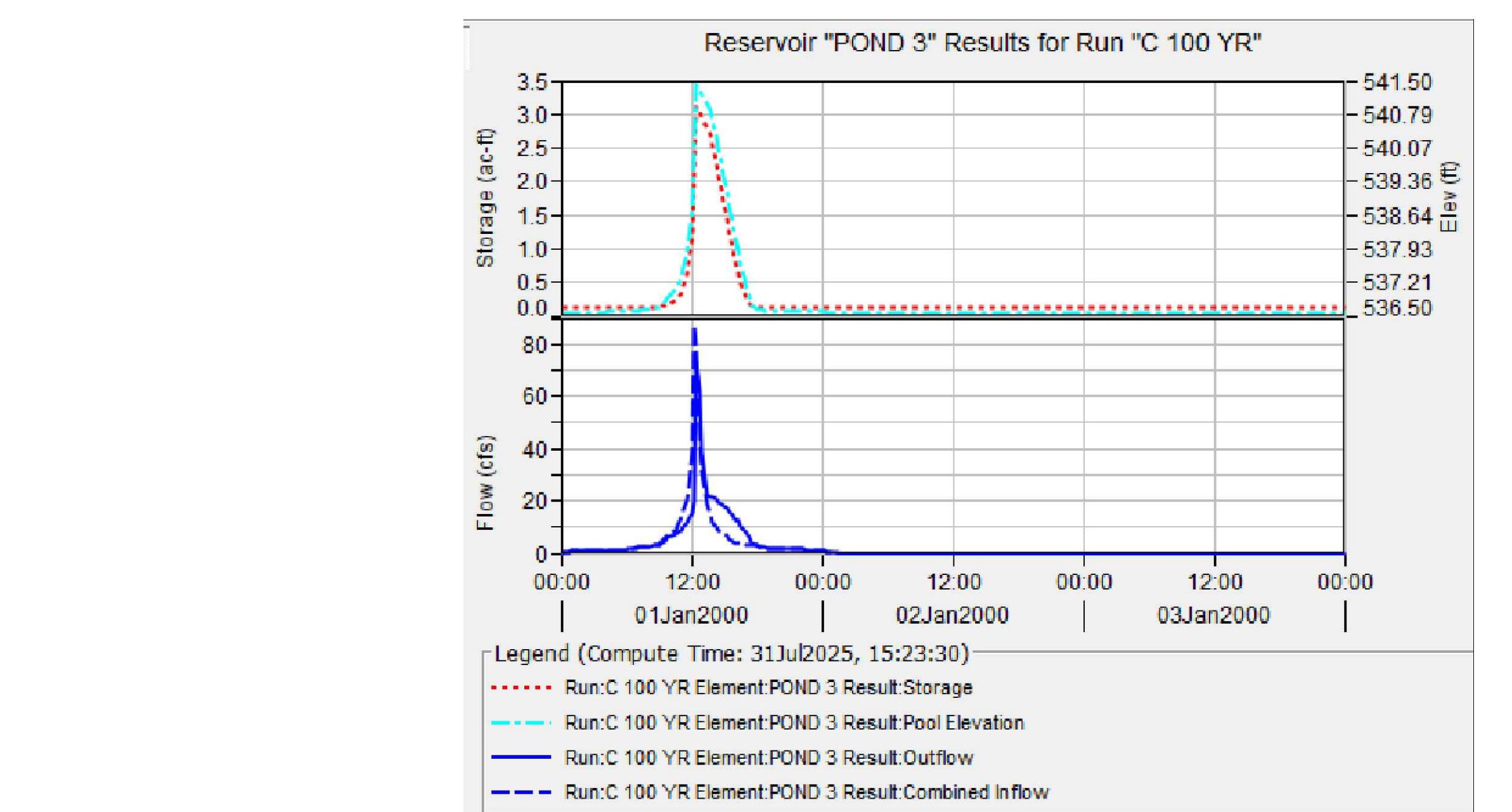
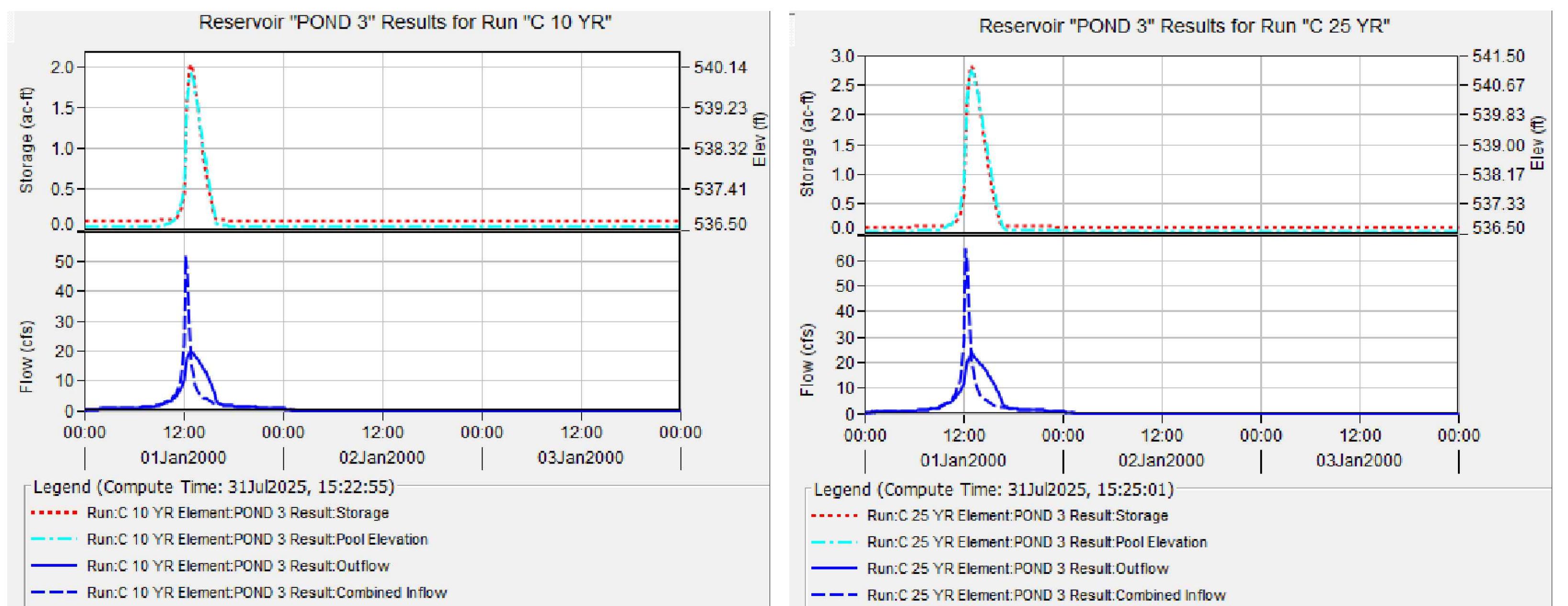
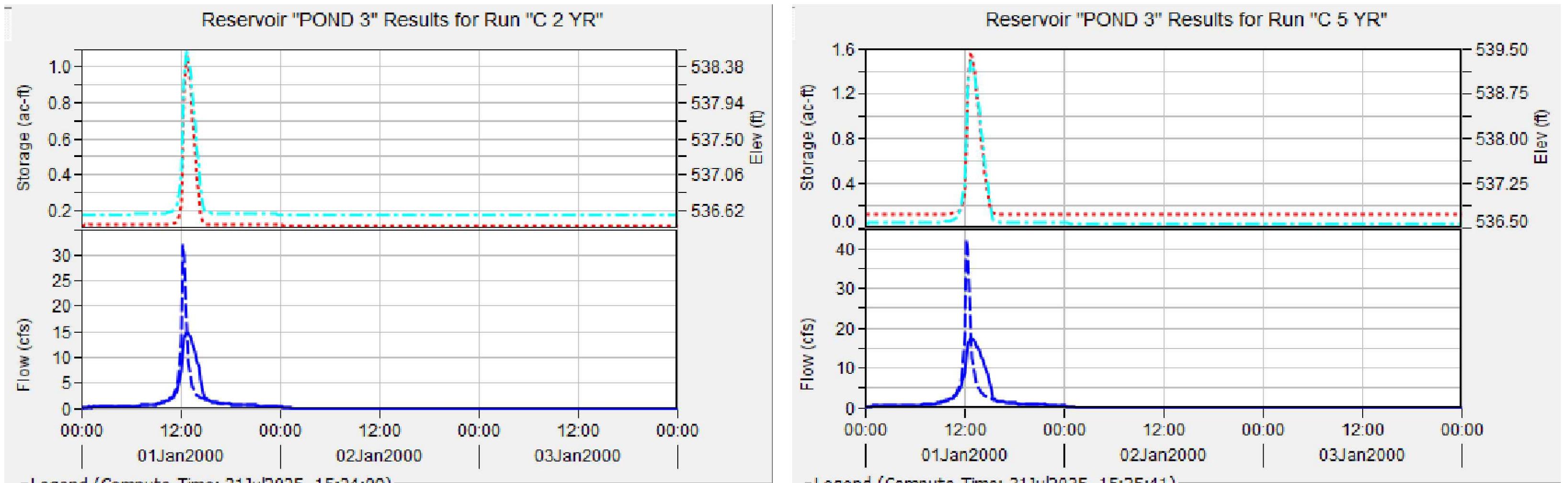
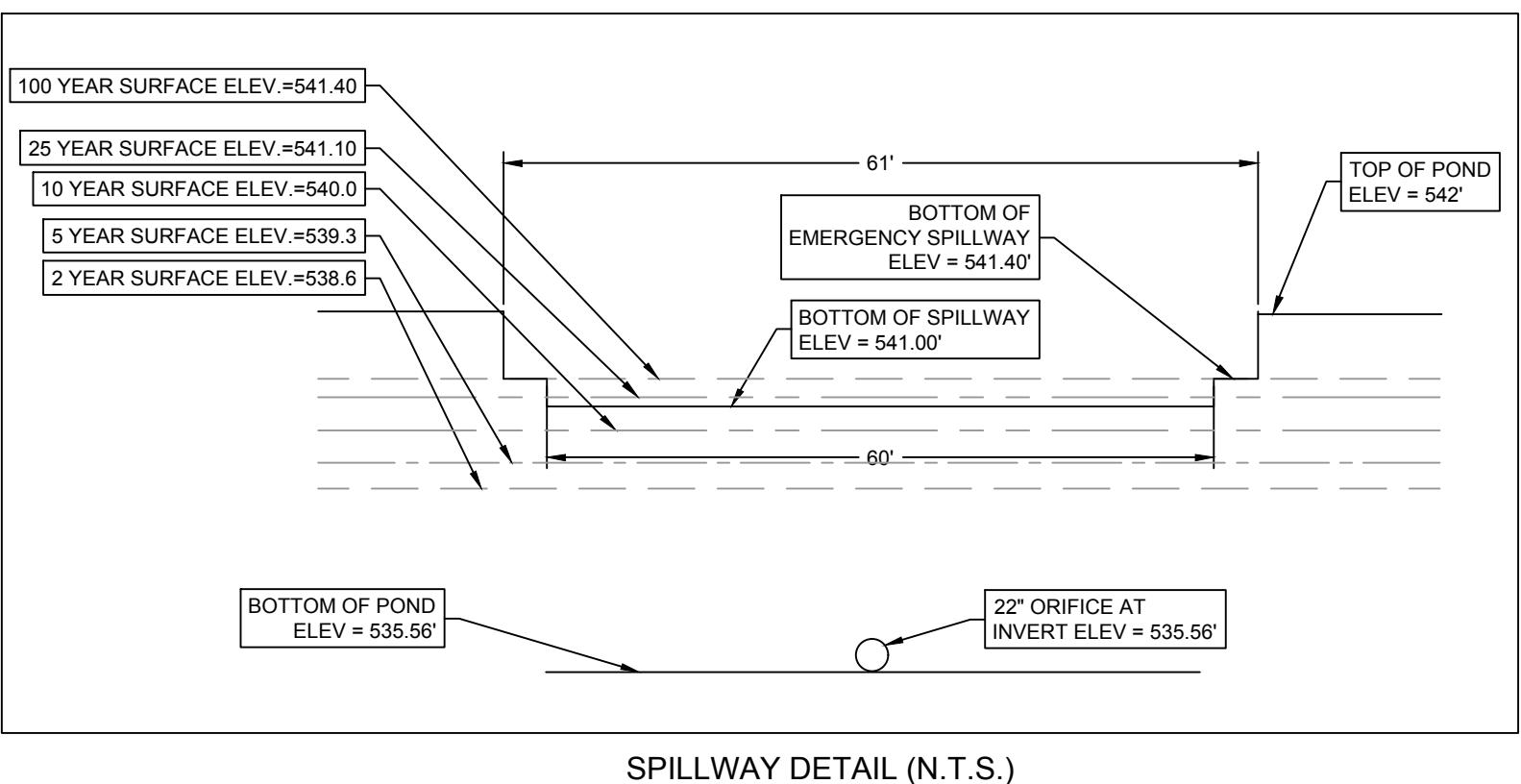
JOB No. 070422-01-002

SHEET

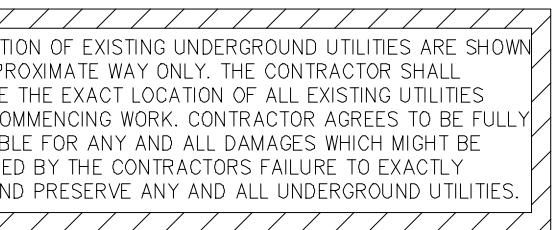
30 OF 43

POND 3 SUMMARY					
EVENT	INFLOW (cfs)	OUTFLOW (cfs)	DELTA (cfs)	WSE Ft	STORAGE Ac-ft
2-year	32.1	14.8	17.3	538.6	1.10
5-year	42.4	17.3	25.1	539.3	1.50
10-year	51.6	19.4	32.2	540.0	2.00
25-Year	65	24.5	40.5	541.1	2.80
100-Year	87.4	73.5	13.9	541.4	3.10

POND 3 STAGE-AREA-STORAGE TABLE						
ELEV	AREA (sq. ft.)	DEPTH (ft)	Avg End Inc. Vol. (cu. ft.)	Avg Total Vol. (cu. ft.)	Conic Inc. Vol. (cu. ft.)	Conic Total Vol. (cu. ft.)
536	1,839.81	0	386	386	258	258
537	17,601.03	1	9,720	10,107	8,377	8,635
538	26,717.10	1	22,159	32,266	22,001	30,636
539	29,163.15	1	27,940	60,206	27,931	58,567
540	31,488.69	1	30,326	90,532	30,318	88,886
541	33,869.61	1	32,679	123,211	32,672	121,557
542	36,320.18	1	35,095	158,306	35,088	156,645



Know what's below.  
Call before you dig.



THE LOCATION OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DEFEND AND EXHAUST ALL LOCATIONS OF THESE UTILITIES BEFORE COMMENCING WORK. CONTRACTOR AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTORS FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.

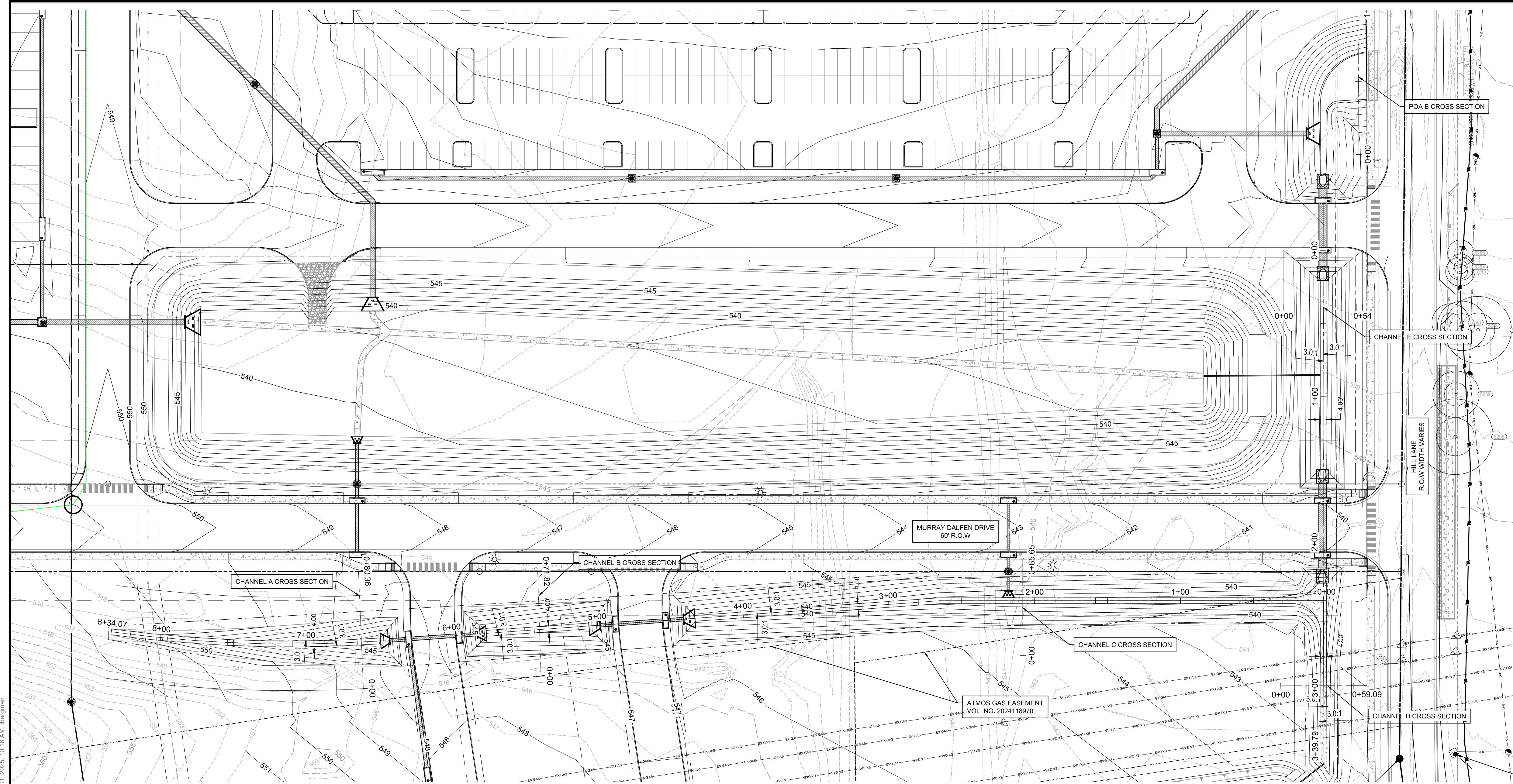
DESIGN DRAWN CHKD  
MC MG MK

JOB No. 070422-01-002

SHEET

30 OF 43

EXHIBIT 12 – CHANNEL CROSS SECTIONS



40  
0  
40  
80  
SCALE: 1 INCH = 40 FT

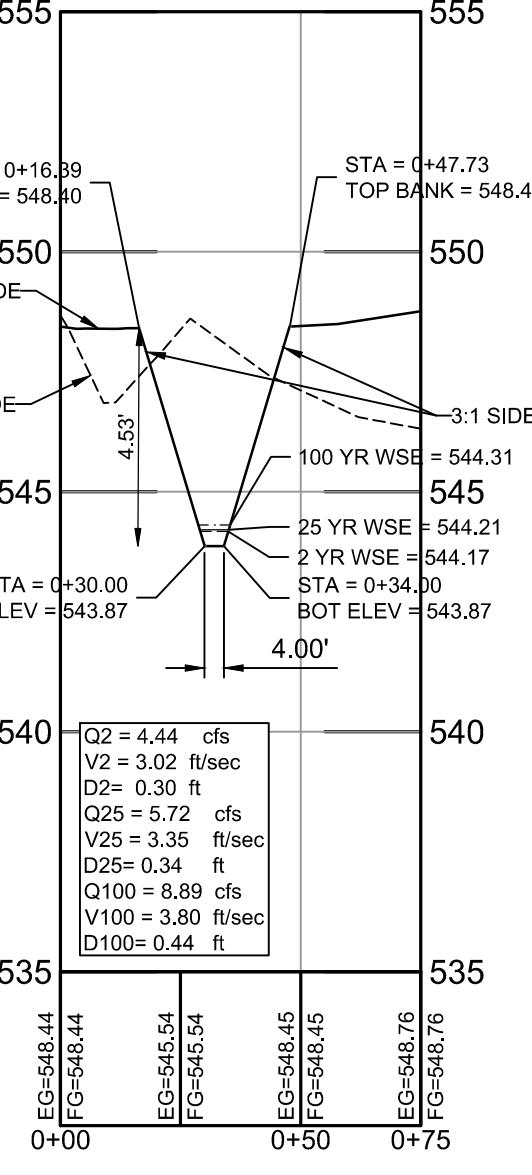
### LEGEND

PROPOSED R.O.W.
PROPOSED LOT LINE
EASEMENT LINE
PROPOSED CURB & GUTTER
EXISTING MAJOR CONTOUR
EXISTING MINOR CONTOUR
PROPOSED MAJOR CONTOUR
PROPOSED MINOR CONTOUR
STORM DRAIN LINE
HEADWALL
CURB INLET
AREA INLET
STORMSEWER MANHOLE

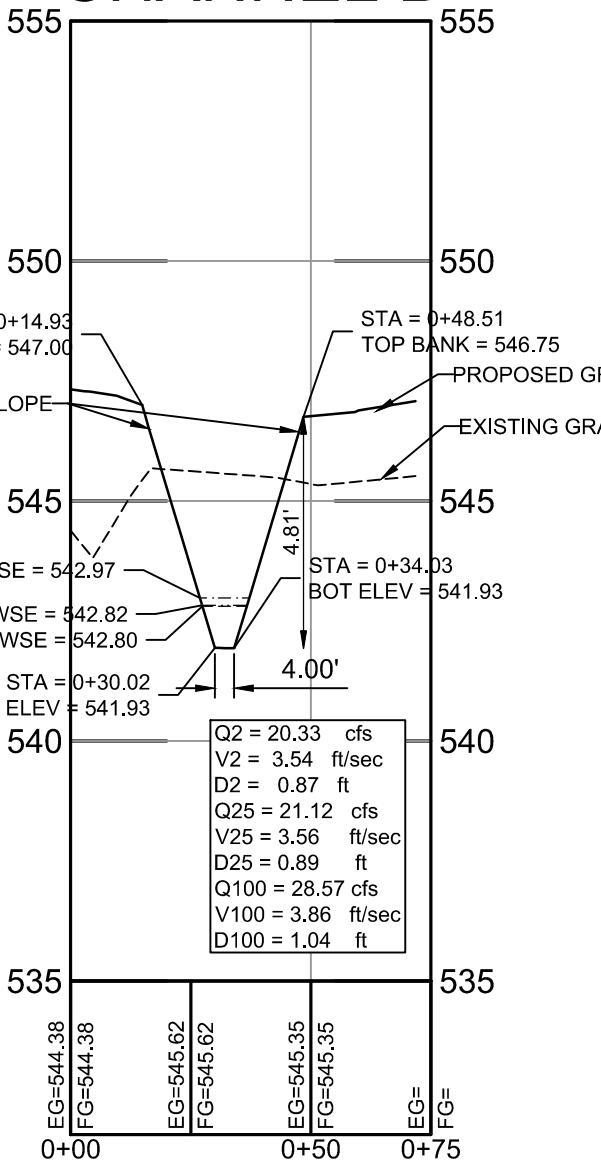
PROFILE LEGEND:	PROFILE SCALE:
EXISTING GRADE CL	1" = 40' HORIZONTAL
PROPOSED GRADE CL	1" = 4' VERTICAL
WSE - 100yr	
VSE - 25yr	
WSE - 2yr	
ELEVATION EXISTING GRADE	ELEVATION PROPOSED FLOW LINE
EG-XXXXXX	FG-XXXXXX

NOTE:  
A MANNING'S N VALUE OF 0.040 WAS USED FOR CHANNEL A, B, C AND E  
A MANNING'S N VALUE OF 0.020 WAS USED FOR CHANNEL D

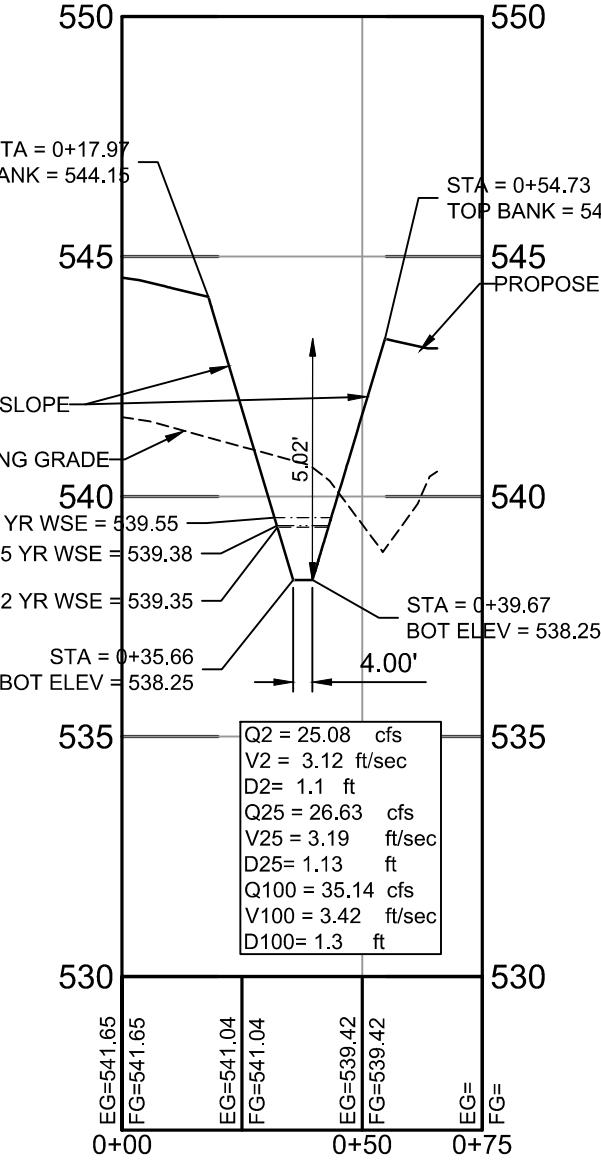
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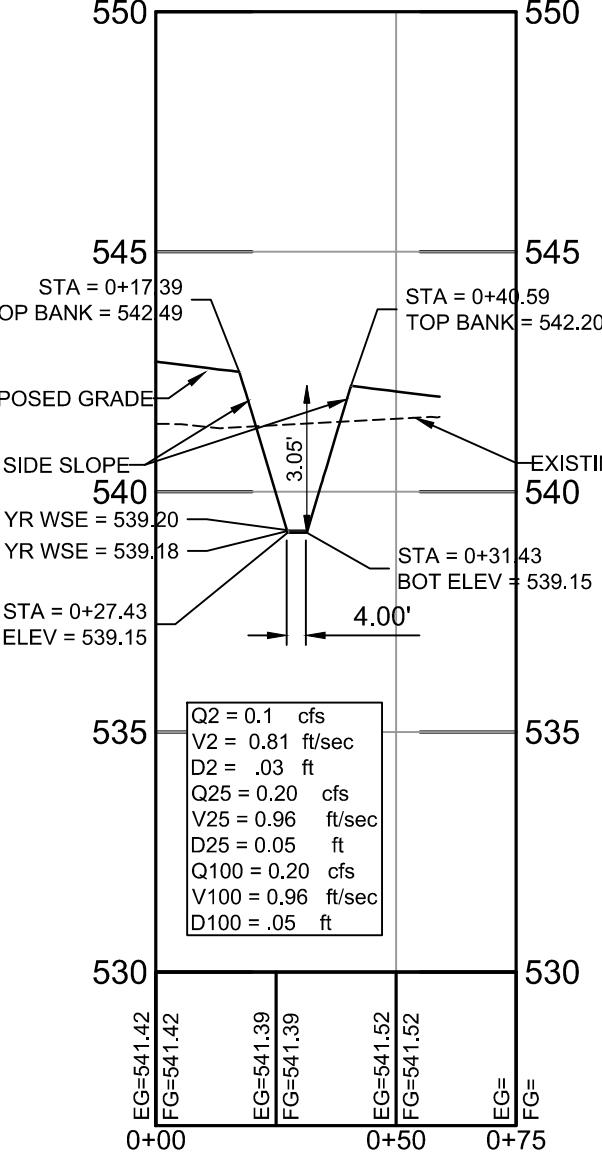
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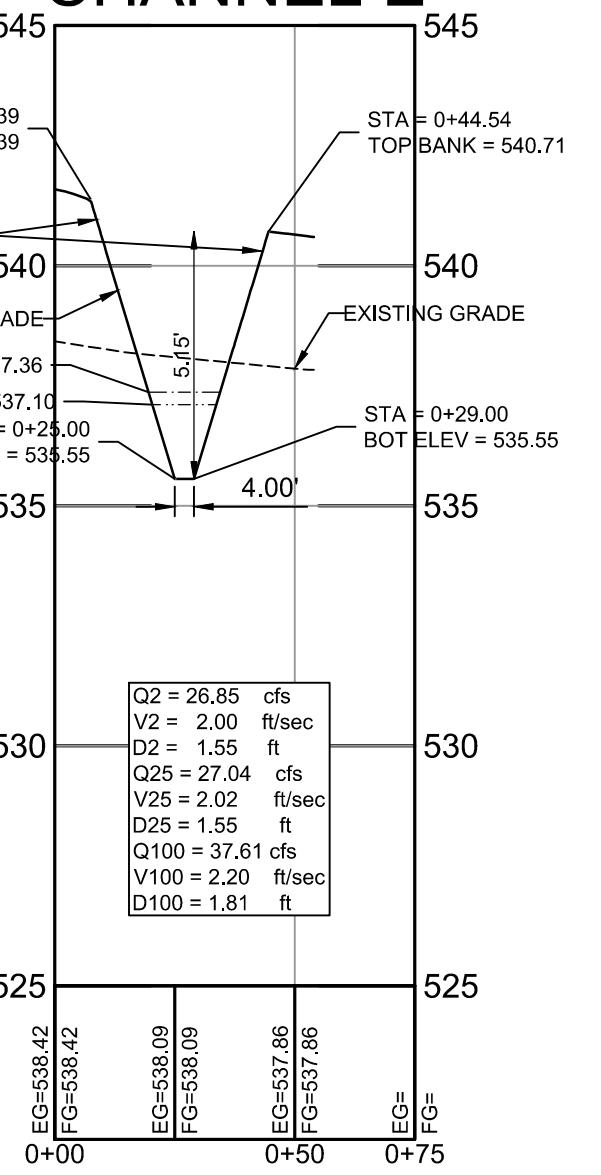
### CHANNEL C



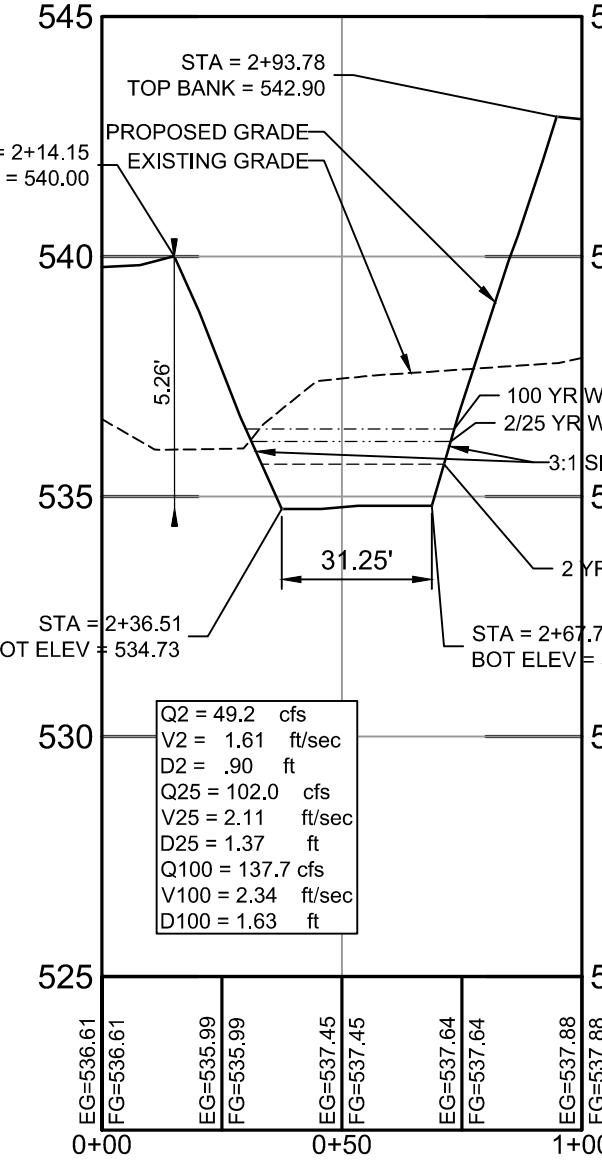
### CHANNEL D



### CHANNEL E



### CHANNEL POA-B



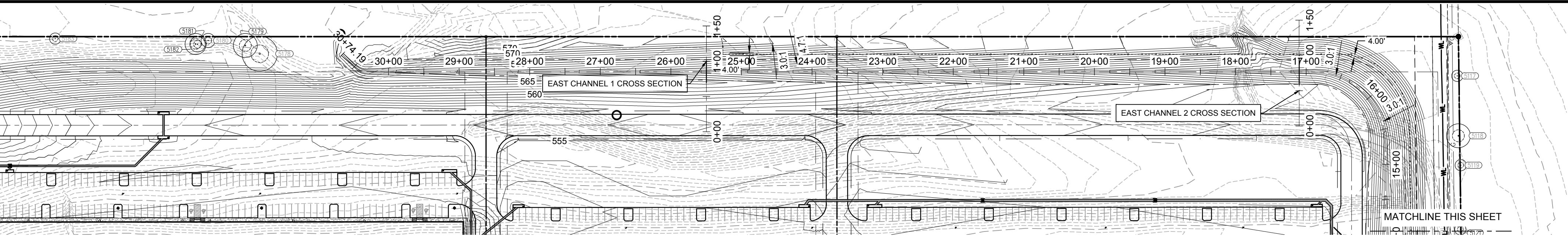
MANOR DOWNS INDUSTRIAL  
8916 HILL LANE  
MURRAY DALFEN DRIVE CHANNEL CROSS SECTIONS

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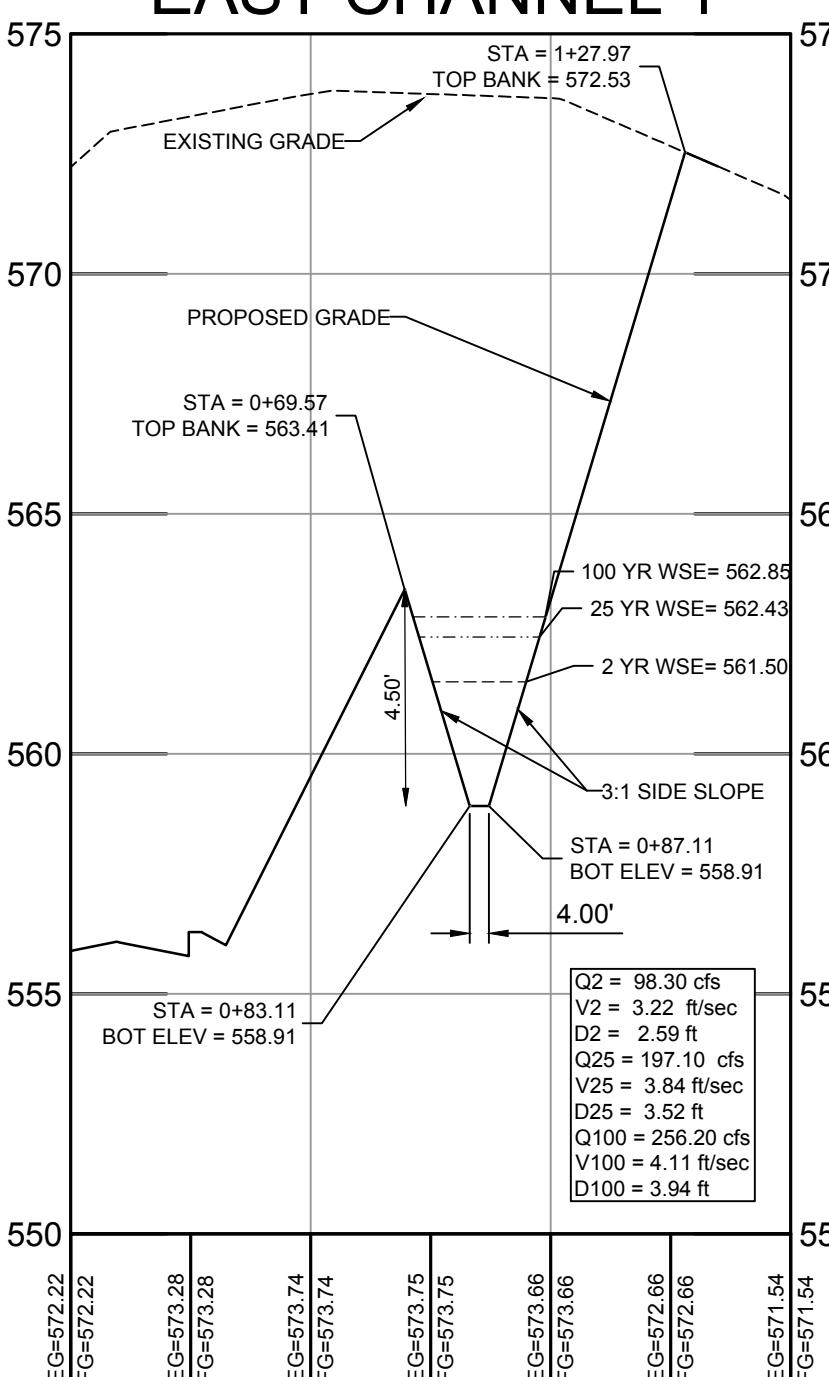
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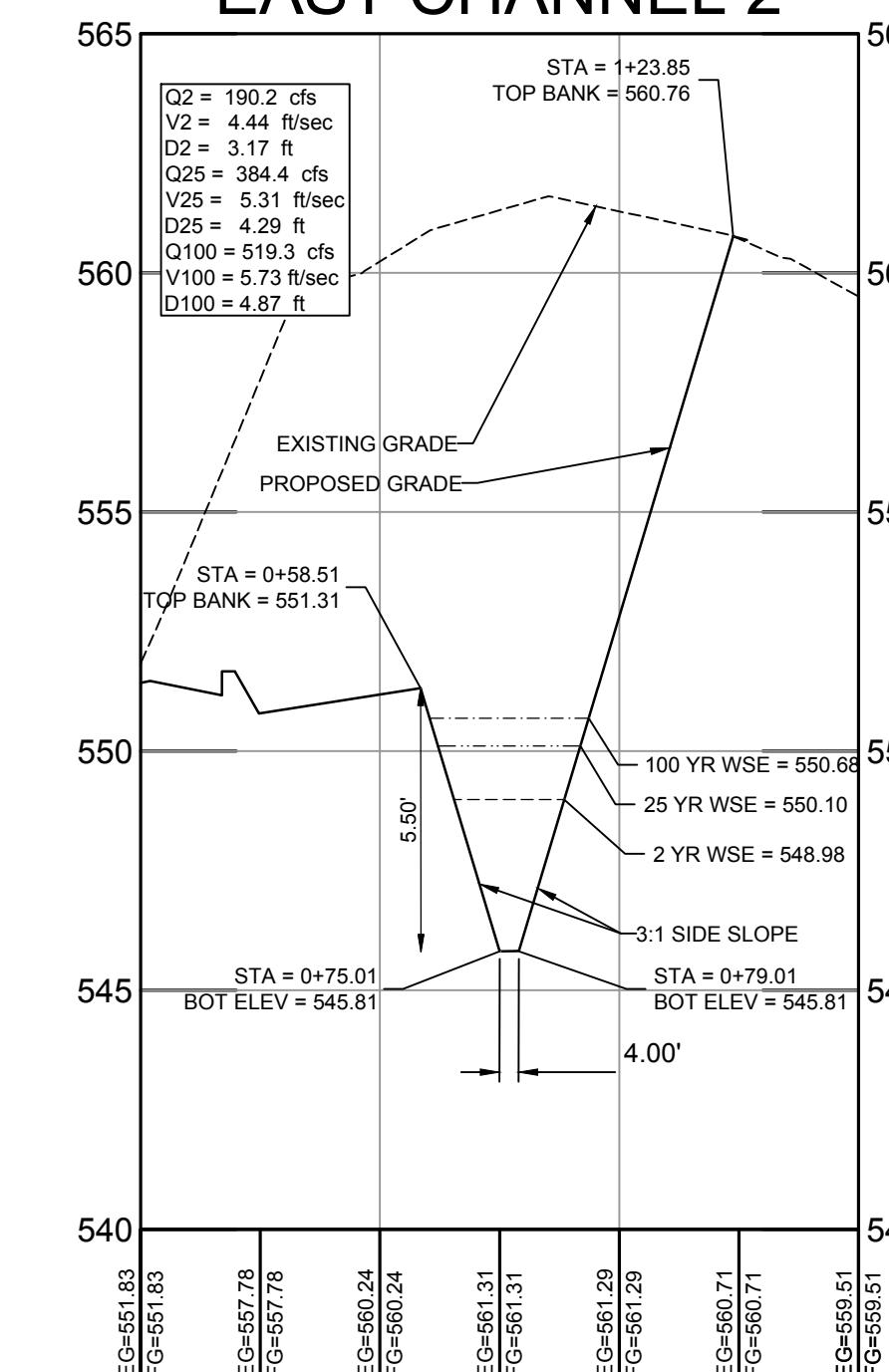


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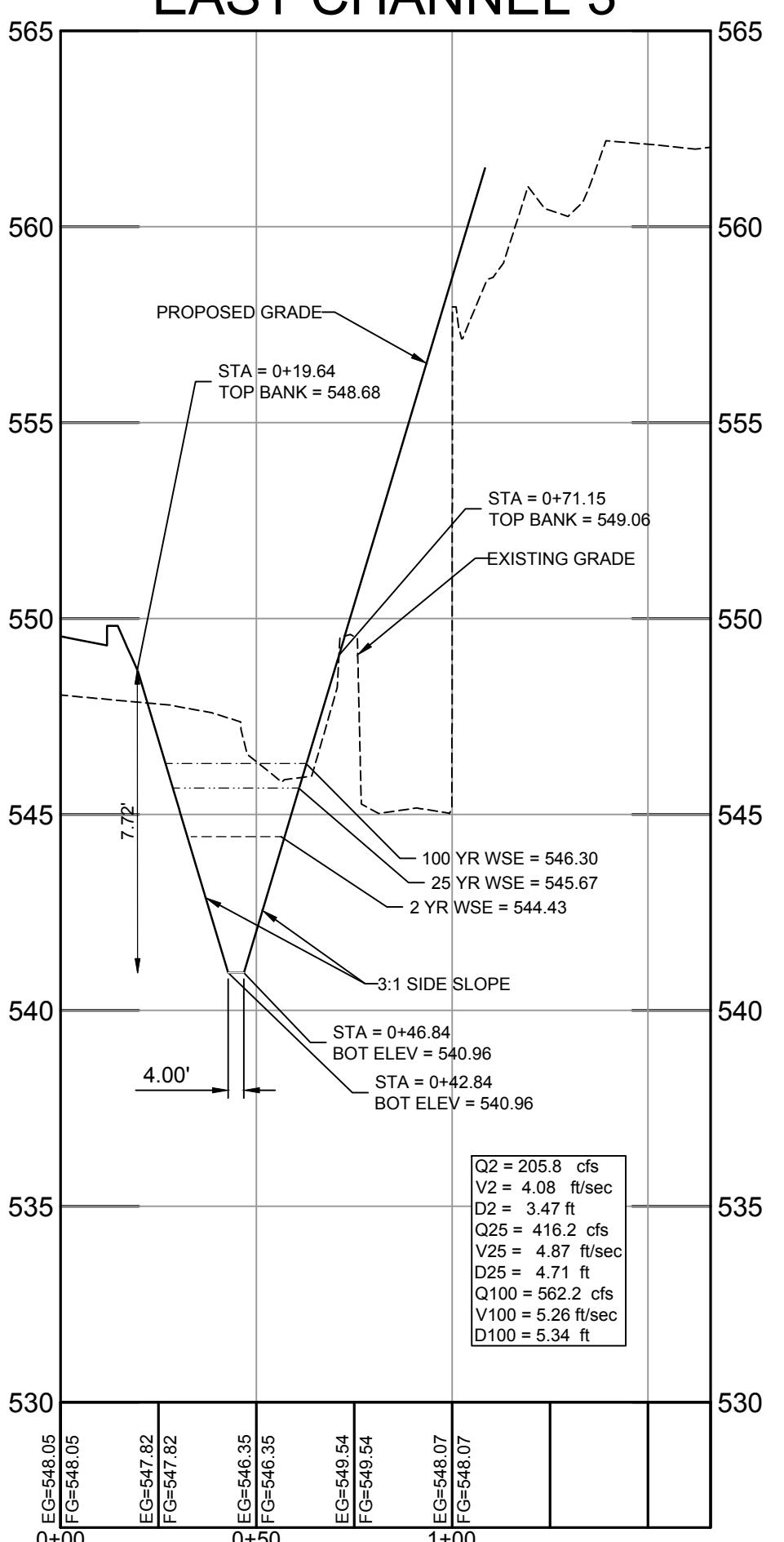
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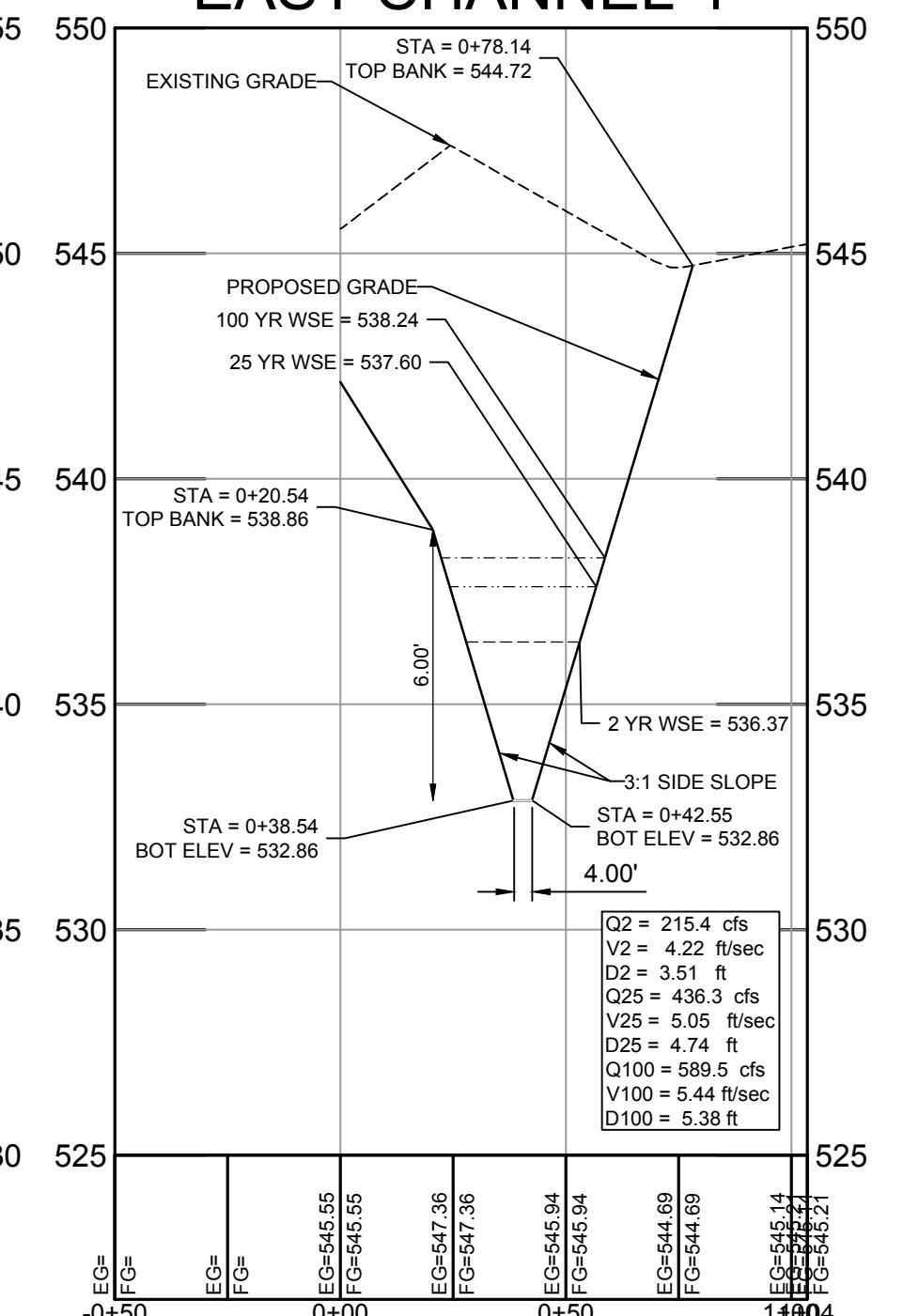
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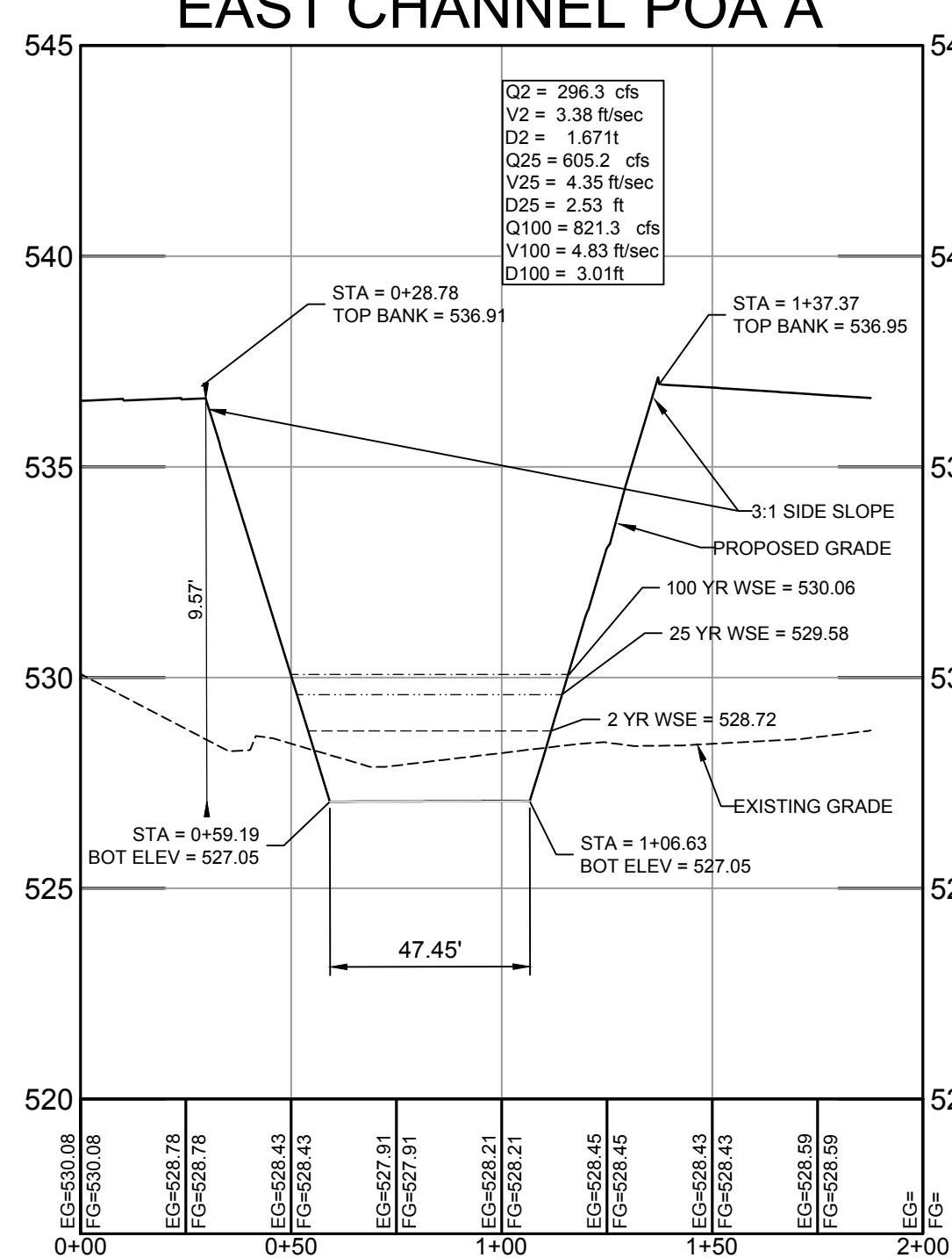
# EAST CHANNEL 3



## EAST CHANNEL 4



# EAST CHANNEL POA



**MANOR DOWNS INDUSTRIAL  
8916 HILL LANE  
EAST CHANNEL CROSS SECTIONS**

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