A DIVISION OF NUCOR CORPORATION

PROJECT NUMBER: <u>U1R-211</u>81 PROJECT NAME: Project Zebra COUNTY: Whatcom PROJECT LOCATION: Lynden, WA CUSTOMER: H&H Steel Erectors Lake Stevens, WA

MBMA

ACCREDITED

PROJECT LOADS

	DESIGN CODE: IBC 2018	BUILDING END USE: 3B			
	ROOF LIVE LOAD: 20 PS NOT REDUCIBLE PER CODE	MBMA OCC. CLASS: II - Standard Buildings			
	GROUND SNOW LOAD: 25 PS SNOW IMPORTANCE FACTOR, Is:	SF SNOW EXP. FACTOR, Ce: 1			
	WIND: 110 WIND I	MPORTANCE FACTOR, IW:1_			
	EXPOSURE: C WITHIN	HURRICANE COASTLINE ☐ YES ☒ NO			
	UL 90 🗌 YES 🗵 NO	RAIN INTENSITY (in/hr)4_			
	SEISMIC INFORMATION Ss:0.943, Design Sds/Sd1:				
	-				
Seismic Imp. Factor Ie:1 Seismic Design Category:					
3	Analysis Procedure: Equivalent Lateral Force Method				
1	Basic SFRS:				

NOTES:
AJ COLLATERAL DEAD LOADS, UNLESS OTHERWISE NOTED, ARE ASSUMED TO BE UNIFORMLY DISTRIBUTED.
JULICILATERAL DEAD LOADS, UNLESS OTHERWISE NOTED, ARE ASSUMED TO BE UNIFORMLY DISTRIBUTED.
WHEN SUSPENDED SPRINGLER SYSTEMS, LIGHTING, HVAC EQUIPMENT, CELLINGS, ETC., ARE SUSPENDED
FROM ROOF MEMBERS, CONSULT THE MA. B. IS THESE CONCENTRATED LOAD EXCEED 200 POUNDS, OR IF
INDIVIDUAL MEMBERS ARE LOADED SIGNIFICANTLY MORE THAN OTHERS.

2) THE DESIGN OF STRUCTURAL MEMBERS SUPPORTING GRAVITY LOADS IS CONTROLLED BY THE MORE CRITICAL EFFECT OF ROOF LIVE LOAD OR ROOF SNOW LOAD, AS DETERMINED BY THE APPLICABLE CODE.

	BUILDING	BUILDING	BUILDING
	Main	Loading Dock	High Bay
ROOF DEAD (PSF):	5	5	5.9
PRI. COL. (PSF):	5	5	5
SEC. COL. (PSF):	5	5	5
SNOW Ct:	1.2	1.2	1.2
SNOW Cs:			
ROOF SNOW (PSF):	25	25	25
WIND ENCLOSURE:	Enclosed	Enclosed	Enclosed
GCpi:			
SEISMIC R:			
SEISMIC Cs:			
BASE SHEAR (KIPS):			

GENERAL NOTES

ASTM DESCRPTION ASTM DESCRPTION 1. MATERIALS MATERIALS STRUCTURAL STEEL PLATE ROOF AND WALL SHEETING HOT ROLLED MILL SHAPES A36 / A529 / A572 / A500 BOLTS A307 / A325 / A490 HSS ROUND A500 HSS RECTANGULAR COLD FORM SHAPES A653 / A1011

 STRUCTURAL PRIMER NOTES:
 SHOP COAT PRIMER IS INTENDED TO PROTECT THE STEEL FRAMING FOR A SHORT PERIOD OF TIME. STORAGE IN EXTREME COLD TEMPERATURES OR WINTER SNOW CONDITIONS, INCLUDING TRANSPORTATION ON SALTED OR CHEMICALLY TREATED ROADS WILL ADVERSELY AFFECT THE DURABILITY AND LONGEVITY OF THE PRIMER. THE COAT OF SHOP PRIMER DOES NOT PROVIDE THE UNIFORMITY OF APPEARANCE, OR THE DURABILITY AND CORROSION RESISTANCE OF A FIELD APPLIED FINISH COAT OF PAINT OVER A SHOP PRIMER. MINOR ABRASIONS TO THE SHOP COAT PRIMER CAUSED BY HANDLING, (JADING, SHIPPING, UNLOADING AND ERECTION ARE UNAVOIDABLE AND ARE NOT THE RESPONSIBILITY OF THE METAL BUILDING MANUFACTURER.
METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DETERMENT OF THE PRIMER OR CORSION THAT MAY RESULT FROM ATMOSPHERIC
AND ENVIRONMENTAL FARIORS NOT THE COMPATIBILITY OF THE RIMER OR OWN FIELD APPLIED CATING.

3. BUILDING ERECTION NOTES:
THE GENERAL CONTRACTOR AND/OR ERECTOR IS RESPONSIBLE TO SAFELY AND PROPERLY ERECT THE METAL BUILDING SYSTEM IN CONFORMANCE THE SEMENAL CONTINUE OF AND/OF CREET UPS A SEASON SERVICE AND TO PERSON THE RECLE FROM THE WORLD AND SHORT SERVICE AND THE RECLE AND THE RECLE FROM THE RECLE FROM THE PROPERTY SUCH AS OUTS, BRACES, FALSEWORK, CHIEF AND OF THER ELEMENTS FOR EXPERTION ARE TO BE DETERMINED, FURNISHED AND INSTALLED BY THE ERECTION THE PROPERTY SHORT AN ANCHORAGE POINT FOR A FALL ARREST /SAFETY TIE OFF.\P

4. A325 & A490 BOLT TIGHTENING REQUIREMENTS:

IT IS THE RESPONSIBILITY OF THE ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE REGULATIONS. FOR PROJECTS IN THE UNITED STATES SEE THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS OR FOR PROJECTS IN CANADA, SEE THE CAN/CSA S16 LIMIT STATES DESIGN OF STEEL STRUCTURES FOR MORE INFORMATION. THE FOLLOWING CRITERIA MAY BE USED TO DETERMINE THE BOLT TIGHTNESS (I.E., "SNUG-TIGHT" OR "FULLY-PRETENSIONED"), UNLESS REQUIRED OTHERWISE BY LOCAL JURISDICTION OR CONTRACT REQUIREMENTS:

A) ALL A490 BOLTS SHALL BE "FULLY-PRETENSIONED". B) ALL A325 BOLTS IN PRIMARY FRAMING (RIGID FRAMES AND BRACING) MAY BE "SNUG-TIGHT", EXCEPT AS FOLLOWS: "FULLY-PRETENSION" A325 BOLTS IF: a) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5 TONS.

- b) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS-REVERSALS ON THE CONNECTIONS
- a) BOLLINING SUPPORTS MALCHINERY THAT CREATES YEAR TOWN, IMPACT OR STRESS-REVENDALS ON THE CONNECTION THE ENGINEER-OF-RECORD FOR THE PROJECT SHOULD BE CONSULTED TO EVALUATE FOR THIS CONDITION.
 c) THE PROJECT SITE IS LOCATED IN A HIGH SEISMIC AREA, FOR IBC-BASED CODES, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC DESIGN CATEGORY" OF "D", "E", "OR "F", SEE THE "BUILDING LOADS" SECTION OF THIS PAGE FOR THE DEFINED SEISMIC DESIGN CATEGORY FOR THIS PROJECT.

d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A325-SC". "SLIP-CRITICAL (SC)" CONNECTIONS MUST BE FREE OF PAINT, OIL, OR OTHER MATERIALS THAT REDUCE FRICTION AT CONTACT SURFACES. GALVANIZED OR LIGHTLY RUSTED SURFACES ARE ACCEPTABLE.

C) IN CANADA, ALL A325 AND A490 BOLTS SHALL BE "FULLY PRE-TENSIONED", EXCEPT FOR SECONDARY MEMBERS (PURLINS, GIRTS, OPENING FRAMING, ETC.) AND FLANGE BRACES.

SECONDARY MEMBERS (PURLINS, GIRTS, OPENING FRAMING, ETC.) AND FLANGE BRACE CONNECTIONS MAY ALWAYS BE "SNUG-TIGHT", UNLESS INDICATED OTHERWISE IN THESE DRAWINGS.

5. GENERAL DESIGN NOTES:

1) ALL STRUCTURAL STEEL SECTIONS AND WELDED PLATE MEMBERS ARE DESIGNED IN ACCORDANCE WITH ANS/AISC 360 "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS" OR THE CAN/CSA 516 "LIMIT STATES DESIGN OF STEEL STRUCTURES", AS REQUIRED BY THE SPECIFICE BUILDING CODE.
2) ALL WELDING OF STRUCTURAL STEEL IS BASED ON EITHER AWS D1.1 "STRUCTURAL WELDING CODE – STEEL" OR CAN/CSA W59 "WELDED

STEEL CONSTRUCTION (METAL ARC WELDING)", AS REQUIRED BY THE SPECIFIED BUILDING CODE.

3) ALL COLD FORMED MEMBERS ARE DESIGNED IN ACCORDANCE WITH ANSI/AISI 100 OR THE CAN/CSA \$136 "SPECIFICATIONS FOR THE DESIGN

OF COLD FORMED STEEL STRUCTURAL MEMBERS" , AS REQUIRED BY THE SPECIFIED BUILDING CODE

4) ALL WELDING OF COLD FORMED STEEL IS BASED ON AWS D1.3 "STRUCTURAL WELDING CODE" - SHEET STEEL" OR CAN/CSA W59 "WELDED STEEL CONSTRUCTION (METAL ARK WELDING) AS REQUIRED BY THE SPECIFIED BUILDING CODE.

5) THIS MANUFACTURING FACILITY IS AS AC-47 SACREDITED AND CAN/CSA AGEO AND W47.1 CERTIFIED (IF APPLICABLE) FOR THE DESIGN

AND MANUFACTURING OF METAL BUILDING SYSTEMS.
6) IF JOISTS ARE INCLUDED WITH THIS PROJECT, THEY ARE SUPPLIED AS A PART OF THE SYSTEMS ENGINEERED METAL BUILDING AND ARE FABRICATED IN ACCORDANCE WITH THE REQUIREMENTS OF SECTION 1926.758 OF OSHA SAFETY STANDARDS FOR STEEL ERECTION, DATED JANUARY 18, 2001.

6. GLOSSARY OF ABBREVIATIONS:

A.B. = ANCHOR BOLTS BS = BOTH SIDES Max = MAXIMUM M.B. = MACHINE BOLTS B.U. = BUILT-UP MBS = METAL BUILDING SUPPLIER Dia = DIAMETER Min = MINIMUM FIG = FLANGE
F.S =FAR SIDE
Ga. = GAUGE
H.S.B. = HIGH STRENGTH BOLTS N/A = NOT APPLICABLE O.A.L. = OVERALL LENGTH O.C. = ON CENTER

BS = BOTH SIDES

Req'd = REQUIRED Rev. = REVISION SIM = SIMILAR SL = STEEL LINE SLV = SHORT LEG VERTICAL TBD = TO BE DETERMINED

Typ = TYPICAL

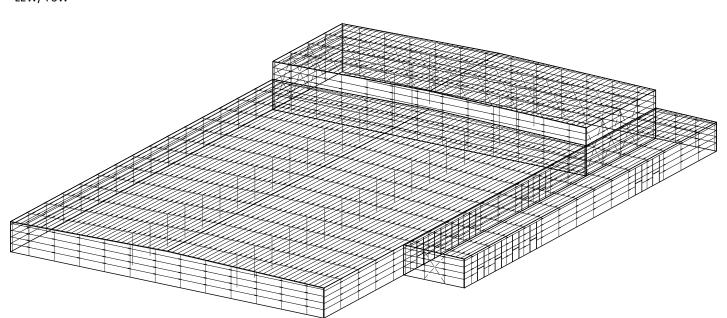
U.N.O. = UNLESS NOTED OTHERWISE

LLV = LONG LEG VERTICAL

?? = PART MARK TO BE DETERMINED AND WILL BE UPDATED ON FOR CONSTRUCTION DRAWINGS

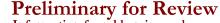


REW/ BSW



MAX. BUILDING

HEIGHT 50'



Information found herein, such as clearances, depths, connections, details, etc., have not been finalized and are subject to change based on final design.

PRELIMINARY COVERSHEET DRAWING

DO NOT USE FOR FINAL CONSTRUCTION

STEEL ERECTORS

H&H

U1R-21181

DUOTE NUMBER:

SHEET NUMBER: CV-1



PRELIMINARY PERSPECTIVE RENDERING DO NOT USE FOR FINAL CONSTRUCTION

H&H STEEL ERECTORS LAKE STEVENS, WA

PROJECT ZEBRA

U1R-21181 SHEET NUMBER:

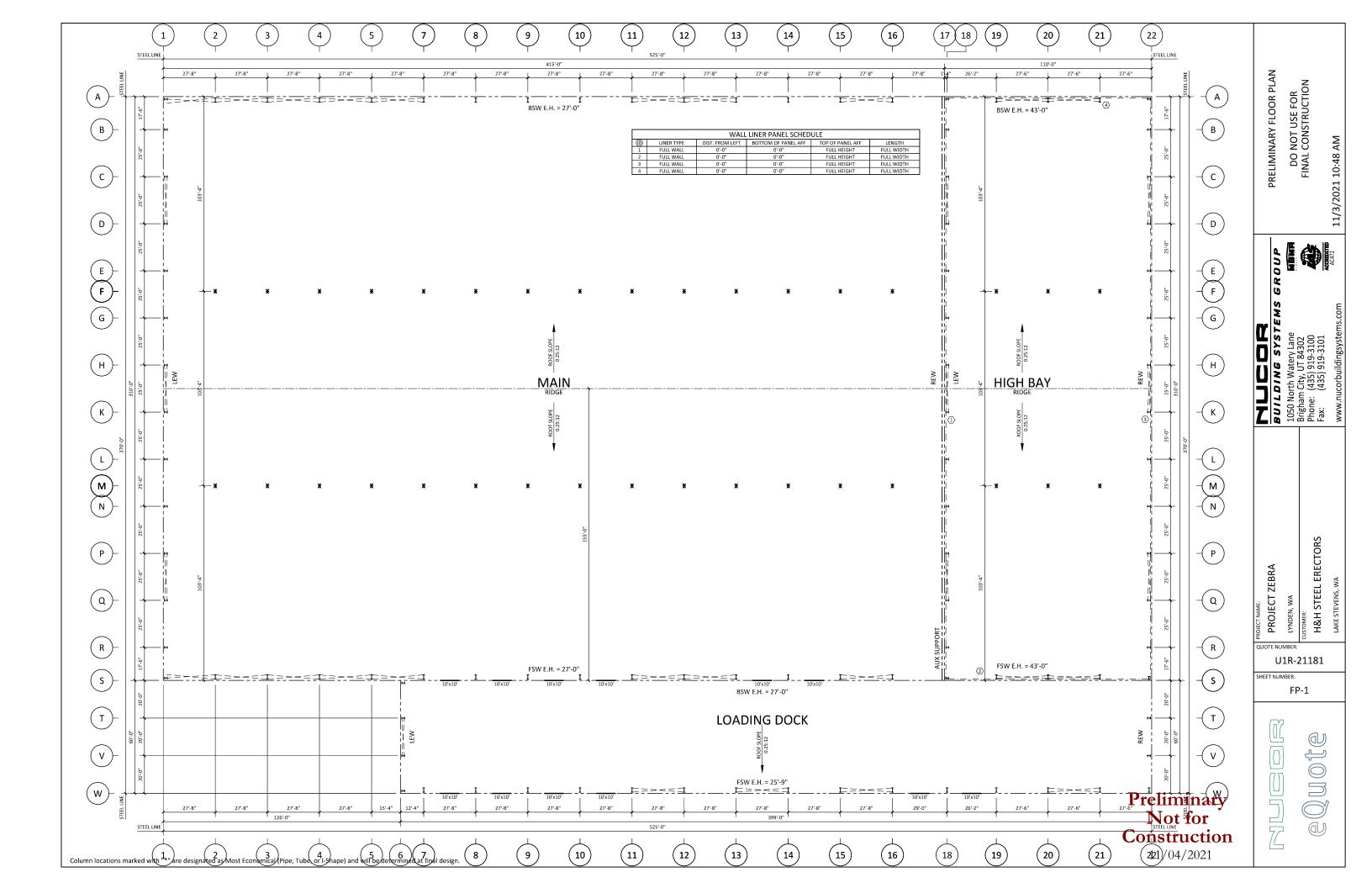
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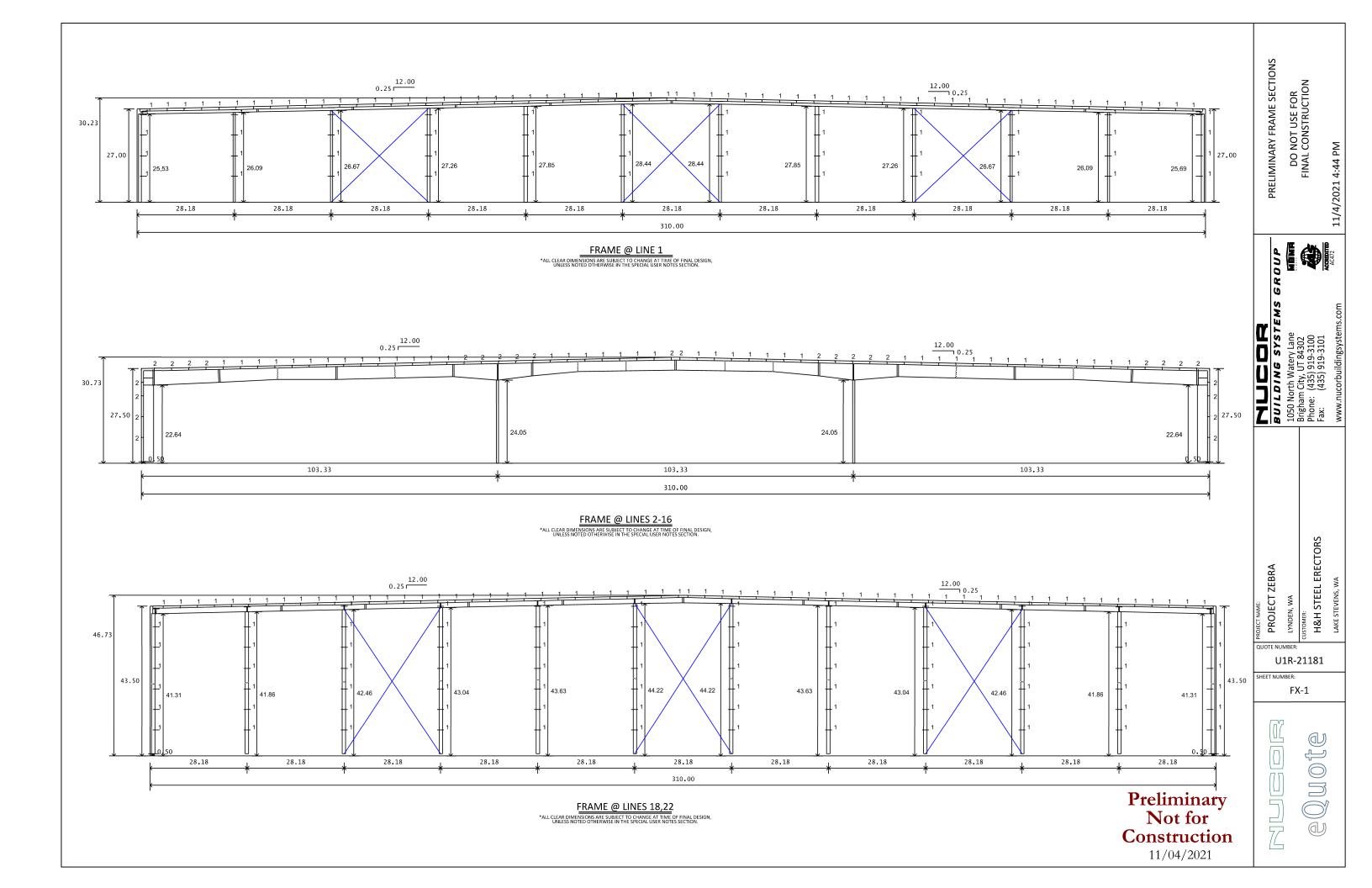


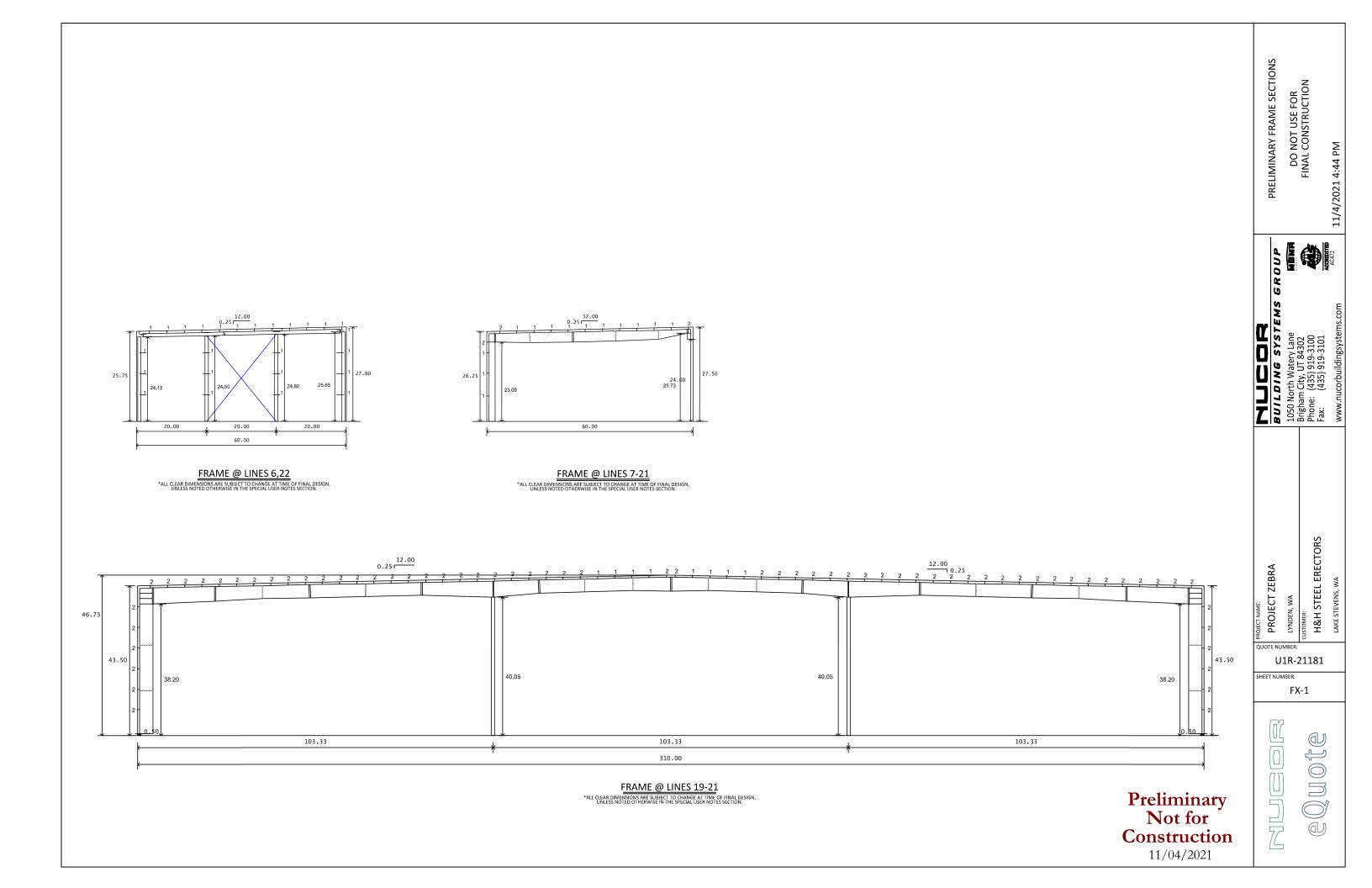
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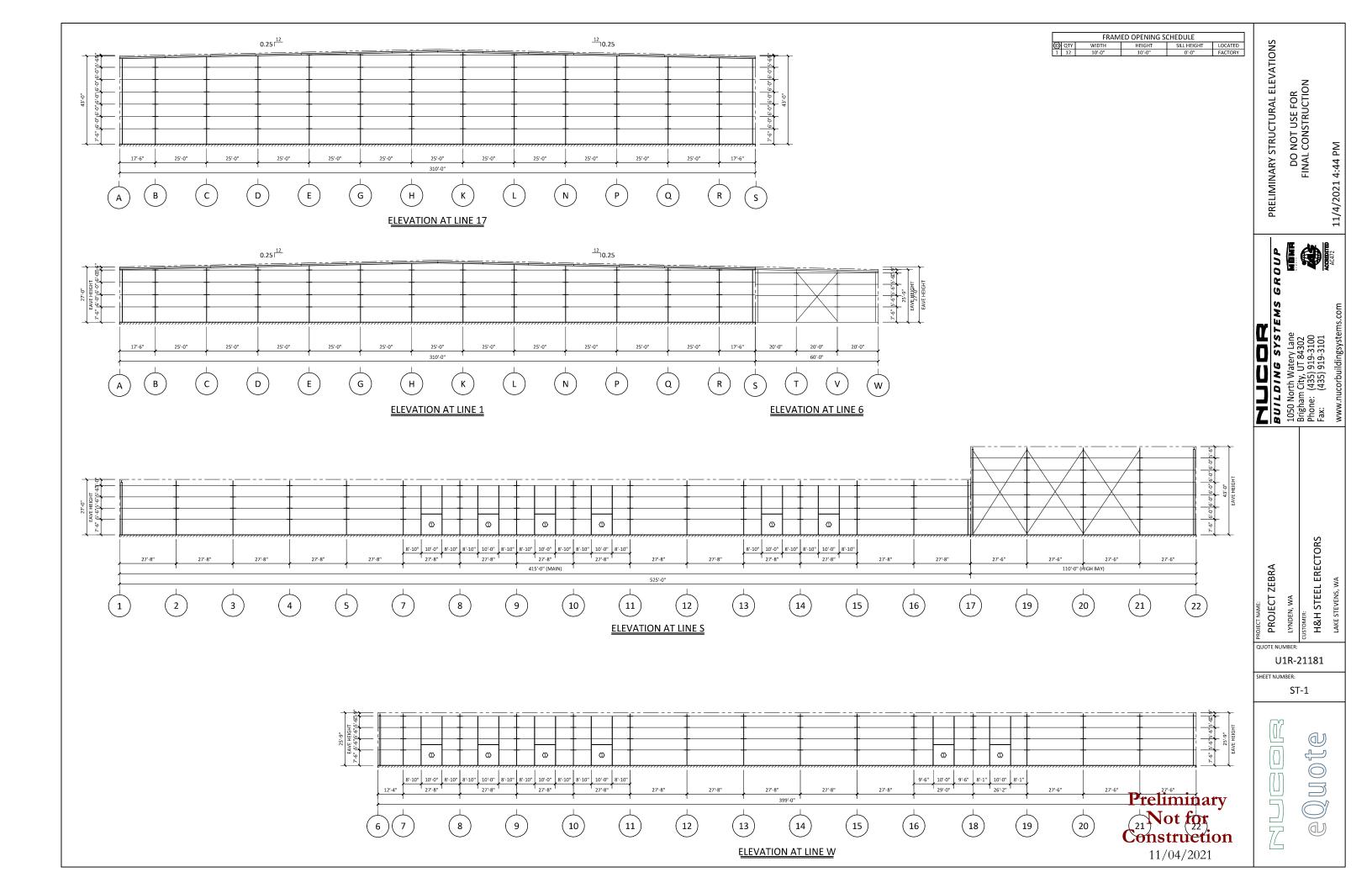
Preliminary
Not for
Construction
11/04/2021

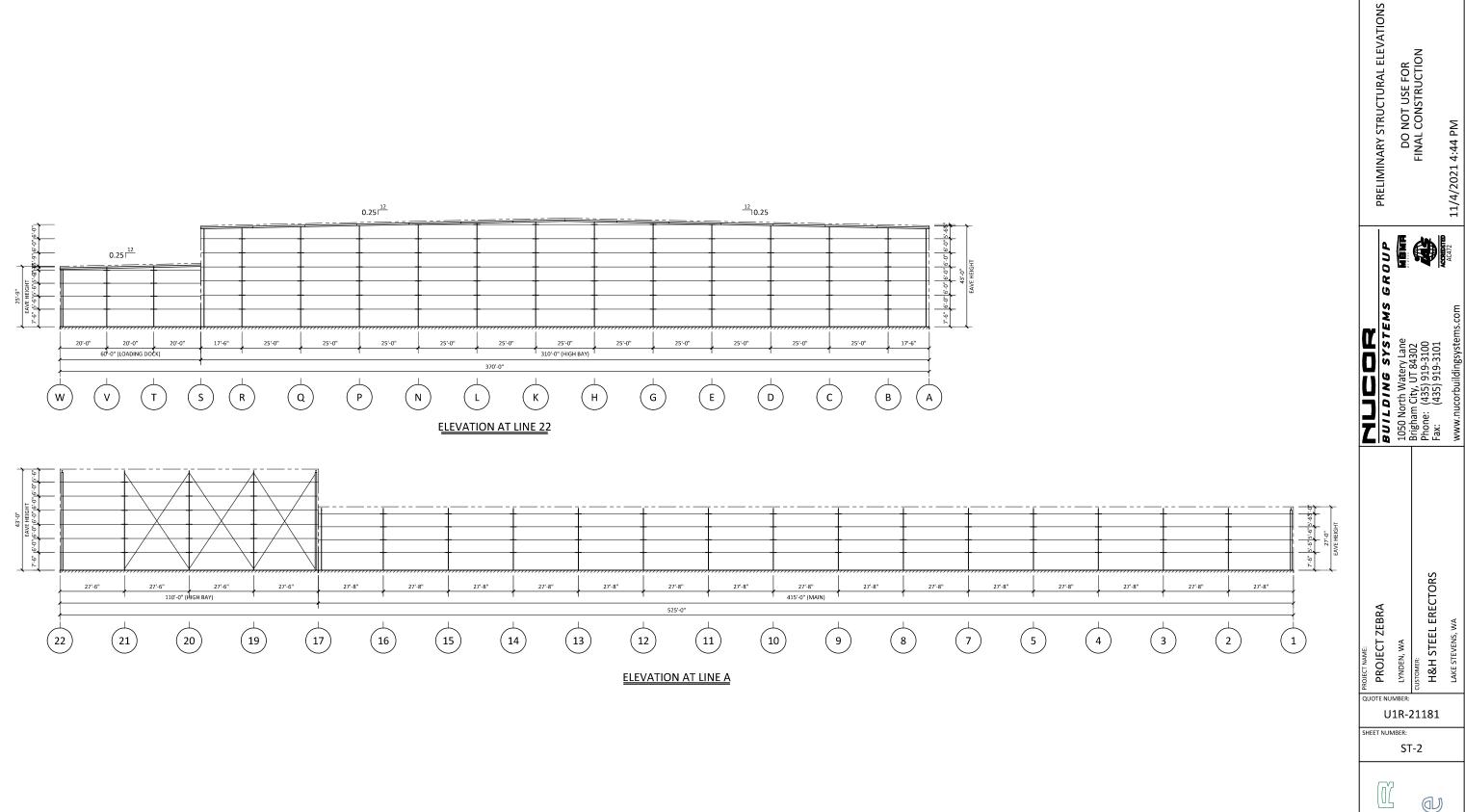












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H&H STEEL ERECTORS LAKE STEVENS, WA

