



September 21, 2026

Mr. Abdul Muqsit
Nexamp Inc.
101 Summer Street, 2
Boston, MA 02110

RE: Jerry Smith Storage: Geotechnical Services
Jerry Smith Road, Lansing, New York
Nexamp Project Number: 133809
LaBella Project Number: 2265555

Dear Mr. Abdul:

LaBella Associates, D.P.C. has completed the geotechnical engineering services for the above referenced project. The study was performed in general accordance with LaBella's "Jerry Smith Storage: Geotechnical Services" proposal dated July 23, 2026.

Our report presents the results of the geotechnical subsurface investigation and provides geotechnical recommendations for the energy storage area at the above address in the Town of Lansing, Tompkins County, New York hereinafter referred to as the "project site".

We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report, please contact us.

Respectfully submitted,
LABELLA ASSOCIATES, D.P.C.


Leonela Lorenzo
Civil Designer


Matthew A. Korn, PE
Senior Geotechnical Engineer

Prepared For:

Nexamp Inc.
101 Summer Street, 2
Boston, MA 02110

Submitted By:

LaBella Associates, D.P.C.
4 British American Boulevard
Latham, New York 12110
(518) 439-8235

Certificate of authorization number:
Professional Engineering: 0021272



Geotechnical Subsurface Investigation and Report
Jerry Smith Storage

September 21, 2026
Project No. 2265555

NOTE

This report is written using **U.S. Customary Units** unless otherwise noted.

The professional services provided in this report consider only the site-specific geotechnical aspects of the subsurface conditions at the project site. The presence or implications of possible surface or subsurface contaminants from any source are outside the terms of reference for this geotechnical study and have not been investigated or addressed herein. Coal seam hazard evaluation, fire and gas hazard evaluation, site subsidence hazard evaluation, wetland impact study, septic field hazard or impact evaluation, slope stability and landslide hazard analysis, are beyond the scope of work for this project.

The subsurface soil profile and design parameters provided in this report are estimated based on the results of the test explorations as indicated on our exploration logs, which include our visual classification of the recovered soil samples, geotechnical analytical laboratory results (where applicable) and/or generally published soil property correlations. Actual subsurface conditions beyond the test explorations and below the depths explored may vary, as well as subsurface conditions encountered in the field during and/or as a result of construction activity.

The recommendations contained within this report are based on the subsurface conditions encountered and the Civil Permit Plan Set prepared by The Environmental Design Partnership, LLP titled "Jerry Smith Energy Storage System" last revised on May 27, 2026. If subsurface conditions or the arrangement of the planned equipment pads varies from those presented within this report, the geotechnical engineer shall be notified to identify if the recommendations provided herein are still applicable.

Provided the accompanying geotechnical interpretive report is read in its entirety, and the recommendations and construction considerations outlined in the report are incorporated in the design and during construction activities, the recommended foundation systems are suitable for the planned structures. Please contact LaBella with questions regarding the information provided herein.

This report was prepared by **LaBella Associates, D.P.C.**

Written by


Leonela Lorenzo
Civil Designer

Reviewed and approved by:

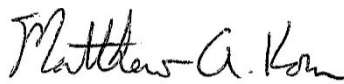

Matthew A. Korn, PE
Senior Geotechnical Engineer



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1.0 INTRODUCTION

LaBella Associates, D.P.C. (LaBella) is pleased to present this report for the subsurface exploration and geotechnical engineering evaluation for the energy storage area and associated access road located at Jerry Smith Road in the Town of Lansing, Tompkins County, New York. For this investigation, five (5) test borings were advanced and two (2) infiltration tests completed at the locations depicted on **Figure 1 – Exploration Location Plan**.

The overall project site is bound by Jerry Smith Road (north), agricultural land (west, and south) and woods and a solar array field to the east. An existing solar array field is located to the south and west of the project area. The project area is located in the northeastern quadrant of the project site. The planned energy storage area will be located in the southwest corner of the project area with an access drive started at Jerry Smith Road and extending south turning east and curving around the energy storage area.

Topographically the project area descends from northwest (El 1088 ft) to southeast (El 1079 ft). Elevations noted herein are based on a topographic survey completed by The Environmental Design Partnership, LLP, dated (May 27, 2026), referencing an assumed datum. Existing conditions are depicted in **Figure 1**.

LaBella's Scope of Services included advancing test explorations, preparing subsurface exploration logs, a location plan, and preparing this report that outlines geotechnical recommendations for the development of the project area.

2.0 SUBSURFACE EXPLORATIONS

This section summarizes the results of the subsurface explorations performed at the project area on May 12, 2026, and May 13, 2026 in support of the interpretations made herein.

Test explorations were witnessed by a LaBella Engineer to advise the operator regarding location and depth, to record subsurface conditions and to modify the subsurface exploration as necessary. Soil samples collected during the subsurface explorations were visually classified in the field in accordance with the Unified Soil Classification System (USCS) and ASTM D 2488. Summary logs detailing the observed conditions within each test exploration are included within **Appendix B - Exploration Logs**. As-drilled locations are documented on **Figure 1**.

LaBella conducted subsurface explorations to characterize the in-situ conditions and to collect representative soil samples. Samples were used for both visual classification and laboratory testing as a basis for determining design criteria cited in this report.

2.1 Test Boring Explorations

Five (5) test boring explorations designated B1 through B5 were performed in the project area by LaBella Environmental, LLC utilizing a CME 55 tracked drill rig capable of advancing a 2.25-inch internal diameter (I.D.) hollow stem augers. The test boring explorations were advanced to depths of 10.9-feet (B4), 12-feet (B5), and 22-feet (B1, B2, and B3).

During soil sample collection, a 2-inch split spoon sampler was driven approximately 2-feet and the number of blows required to drive the sampler every 6-inches were recorded in accordance with ASTM D 1586 to measure the resistance of the soil penetration of the sampler.

Based on the bioretention area detail within the Civil Permit Set, two infiltration tests were performed at a depth of 7 feet (FH1 and FH2) to comply with the current NYSDEC stormwater design manual. The two (2) infiltration tests, designated FH1 and FH2, were conducted on August 13, 2026 as close to the planned area as permitted due to the woods. Summary data detailing the test depth and trials for each infiltration test are included within **Appendix B**.

2.2 Subsurface Stratigraphy

Subsurface explorations indicate at least two distinct stratum are present within the depth of explorations in the project area. The observed strata across the project area were generally **Reworked Material, and Loamy Till**. Each stratum is described in greater detail below using the percentage descriptions per ASTM D 2488.

Reworked Material: An approximate 2-foot thick stratum of Reworked material was encountered within test explorations B1 through B4 and was visually classified as Silty Sand (SM) and Silty Sand with Gravel (SM). This stratum consisted of dry, brown, loose to medium dense sand with some to a little percentage of silt, some to a few percentage of gravel, and a few to no percentage of clay.

Loamy Till: A stratum of Loamy Till was encountered within all test explorations. Typically this stratum was visually classified as Silty Sand with Gravel (SM) and Clayey Gravel with Sand (GC), consisting of brown and gray, dry to wet, medium dense to very dense sand or gravel with some to a few percentage of silt, and some to no percentage of clay.

2.3 Groundwater Conditions

Groundwater was observed within three (3) of the test explorations at depths of 20.5-feet (B1), and 21-feet (B2 and B3) below existing ground surface. Groundwater readings taken at termination of the explorations are typically considered un-stabilized readings.

Groundwater levels recorded on the exploration logs are based off field observations and visual soil classification of soil samples. Groundwater will fluctuate with season, precipitation, nearby construction activity, and other factors.

2.4 Laboratory Results

Representative soil samples were selected for geotechnical laboratory testing. The samples were delivered to Atlantic Testing Laboratories for basic classification testing and analytical tests. The following laboratory test were conducted, and the results are provided in **Appendix C – Laboratory Results**.

- Two (2) moisture content tests (ASTM D2216),
- One (1) Atterberg limit test (ASTM D4318),
- Two (2) particle-size distribution tests (ASTM D6913/D7928),
- One (1) corrosivity suite test, and
- One (1) thermal conductivity test was assigned on soils within the upper 7-feet.

The laboratory tests included within the corrosivity suite are as follows: pH (ASTM G51), Electrical Resistivity (ASTM G187), Water-Soluble Sulfate (ASTM C1580), Oxidation-Reduction Potential (ASTM G200), Water-Soluble Chloride Ion Content (AASHTO T291 method A).

Based on our visual observations, and the laboratory analysis, the soils in the project area and within the upper 10-ft were classified as dry to moist. The test soil at B3 has a moisture point value of 1 on the 10-point soil test evaluation scale for moisture per ANSI/AWWA C105/A21.5.

Results of the pH test identified that the soil has a pH value of 7.38, and a redox potential value of 187 mV which both equated to 0 points on the 10-point soil test evaluation scale per ANSI/AWWA C105/A21.5. The calculated resistivity value was 2,451 Ω cm which equates to 2 points on the 10-point soil test evaluation scale per ANSI/AWWA C105/A21.5.

The one laboratory tested sample had a sulfate by mass of 2600 ppm and a chloride value of 100 ppm. Generally, soils with sulfate and chloride values less than 200 ppm and 100 ppm respectively are considered non-corrosive. The sulfate value equates to 3.5 points on the 10-point soil test evaluation scale per ANSI/AWWA C105/A21.5.

The project area soils are generally considered non-corrosive with a total point value of 6.5 on the 10-point soil test evaluation scale, with ten points or greater indicating a corrosive soil.

One bulk sample was collected to produce a thermal dry-out curve. The calculated thermal resistivity value for the project area was 52°C-cm/W at 14.4% moisture content. More detailed results and the thermal dry-out curve graph is included in **Appendix C**.

2.5 Field Ground Resistivity

Concurrent with the subsurface investigation, LaBella performed a field electrical resistivity test within the project area using an AEMC Model 4630 Resistivity Meter to obtain representative values. The test was performed in accordance with ASTM G57 utilizing probe spacing of 0.5, 1, 2, 3, 5, 6, 10, 30 and 40-feet. The calculated soil resistivity values varied between 40,014 ohm-cm and 125,522 ohm-cm. These values indicate the subsurface is generally non corrosive. More detailed results are included in **Appendix D – Field Resistivity Data**.

Based on results of field resistivity testing, when also considering the results of the corrosion suite tests, soils in the project area are considered to have a generally non corrosive environment. Therefore, protection of direct buried metal is not required, but may extend the life of the piles.

3.0 SEISMIC CONSIDERATIONS

Soils were calculated as not susceptible to liquefaction when examined under the following conditions: USGS published mean peak ground acceleration (0.075g), a maximum earthquake magnitude of 5.0, site-recorded standard blow count values, percentage of fines in the underlying soils, and depth to groundwater.

The soils across the project area have been characterized for seismic conditions in accordance with ASCE/SEI 7-22, “Minimum Design Loads and Associated Criteria for Buildings and Other Structures”, using appropriate generalized correlations. Direct shear wave velocity (V_s) measurements were not collected as part of the scope of work therefore, values were estimated from standard penetration test (SPT) blow counts using published empirical correlations. The estimated V_s values are subject to the inherent uncertainty associated with SPT-based correlations.

Based on the subsurface conditions observed, our analysis, interpretation and correlations, LaBella recommends an average shear wave velocity of approximately 860 ft/sec for the project area. This corresponds to a Site Class D, an SS of 0.15g and S1 of 0.042g.

4.0 GEOTECHNICAL RECOMMENDATIONS

This section presents our geotechnical recommendations for foundation design and construction of the energy storage area based on the subsurface investigation. We have provided our recommendations in accordance with the related provisions of the 2025 Building Code of New York State (BCNYS). At the time of this report and based on the Permit Plan Set, LaBella understands that the proposed energy storage area consists of a gravel access road, equipment pads and battery energy storage systems.

4.1 Equipment Pad Foundations

Based on the observed subsurface conditions, shallow reinforced concrete slabs are suitable foundation systems to support the proposed equipment. LaBella recommends an allowable bearing capacity of **3 kips per square foot (ksf)**. All foundation excavations shall be protected from frost during construction.

For a frost protected slab, with a project area frost depth of 4-feet, LaBella recommends a slab bearing on non-frost susceptible (NFS) material (AASHTO No. 57 stone) extending to a minimum depth of 4 feet below the lowest finished grade. Additionally, because the underlying soils are medium dense to dense with greater than 15% fines an underdrain (placed at the lowest point practicable) that has a positive pitch to daylight is recommended.

As an alternative to the NFS material, to allow compaction verification testing, the use of NYSDOT Type 2 subbase may be substituted. This material should be compacted to 95-percent of the maximum dry density per ASTM D1557. Note the allowed gradation allows for greater than 5-percent of material passing the number 200 sieve (fines). When the percentage of fines exceeds 5-percent, the potential for frost heave slightly increases.

4.2 Pavement Recommendations

Based on our understanding that the access drive will see minimal daily traffic and be limited to lightly loaded vehicles and an occasional WB-67 vehicle or fire truck, we recommend a 12-inch gravel bed, consisting of NYSDOT Type 2 Subbase, over a separation fabric such as Mirafi 140N.

LaBella recommends the gravel road is founded on proof-rolled site soils after removal of all topsoil, loose, disturbed, or unsuitable soils at the subgrade surface. The subgrade should be sloped toward the roadway edges to prevent water from ponding below the road (e.g. to drain toward roadside swales on slopes that drain away the road section). Installation of proper surface drainage practices are anticipated (e.g. proper road drainage practices). This access road is not intended to withstand frequent heavy construction traffic and associated equipment loads, which the contractor may need during the construction phase.

4.3 Utilities

Where underground utilities are required, trench excavation widths should extend a minimum of 12-inches beyond the outer edges of the utility elements to be installed. Exposed soil subgrades should be lightly compacted (proof rolled) and filled with placed and compacted pipe bedding material extending 6-inches (minimum) below and above each utility. Minimum pipe zone bedding and backfill widths shall comply with related pipe installation standards to ensure proper conditions for uniform load distribution.

On-site soils may be used as trench backfill above the pipe zone bedding material. Placement may be affected if the moisture content is greater or less than 3 percent of the optimum moisture content. Installation of an underground trace line is recommended along the utility line to facilitate location of the utility in the future.

Utilities should be located below frost level or connectors to the battery storage area need to be flexible to allow for potential freeze-thaw movement.

4.4 Temporary Excavations

We anticipate that excavation of the on-site soils can be accomplished using conventional earthwork equipment and techniques (i.e. backhoes, scrapers, excavators, or dozers) based on the physical characteristics, relative density of the strata observed and the anticipated excavation limits.

Generally, all temporary cut slope excavations should not be left open or unbraced for extended periods of time. Temporary cuts should be sloped as required for stability in accordance with OSHA regulations and protected from erosion. OSHA requires each type of material be benched at the following slope for temporary excavations:

- Type "A" – 3/4 Horizontal: 1 Vertical (3/4H: 1V),
- Type "B" – 1 Horizontal: 1 Vertical (1H: 1V), and
- Type "C" – 1-1/2 Horizontal: 1 Vertical (1-1/2H: 1V).

Based on the subsurface explorations, overburden soil is to be considered OSHA Type "C". However, this should be verified for each excavation by an OSHA competent person.

4.5 Control of Water

Based on the observed groundwater, seepage into open excavations is not anticipated. However, temporary dewatering measures (e.g., sumps, barriers) should be readily available during construction to remove surface water and precipitation if needed in open excavations.

Surface water must be controlled during construction and earthwork operations by using temporary swales, ditches or other means necessary. All surfaces shall be restored to vegetated conditions upon completion of construction. Upon completion of rough grading activities, the final grade should be set to promote positive drainage away from the structures.

5.0 FILL MATERIALS

In general, fill materials shall be free of unsuitable material such as organics, construction debris, cobbles/boulders, frozen material, etc. Fill areas shall be cleared of all vegetation, roots, and other organic materials prior to placement of fill. Stockpiled soils may require installation of run-off protection between drainage channels and the stockpile.

We recommend compaction consist of at least 4 systematic passes using a vibratory roller with a minimum operating weight of 8-tons. In confined areas, hand guided vibratory equipment shall be used to compact the soil to the specified criteria. If soil weaving or other disturbance is noticed during compaction, vibratory compaction should be discontinued. Heavy compaction equipment shall not be used within 3-feet of the

transformer foundation system. Compaction shall meet the requirements stated below or as approved by a qualified engineer.

5.1 On-Site Borrow Material

Based on LaBella's visual classification of the materials to be excavated (SM), they may be reused as Common Fill, if screened to remove any 4-inch or greater particles, any deleterious materials and debris and meeting the gradation requirements noted for same.

When utilizing on-site material having more than 15-percent passing the No. 200 sieve (SM), placement is susceptible to minor fluctuations in moisture content. If utilized, the on-site soils should stay within 2-percent of optimum moisture content in addition to meeting the required compaction criteria.

5.2 Common Fill

Common Fill similar to NYSDOT Select Fill should consist of inorganic, sand based, granular soils, free of debris and other deleterious material that meet the following gradation:

Sieve Size	Percent Passing by Weight
4-inch	100
No. 40	0-70
No. 200	0-15

Common Fill used for site grading and landscaping should be placed in lifts not exceeding 9-inches loose, measured and compacted to 90% of the maximum dry density as determined by ASTM D 1557. All fill should be placed to promote positive drainage away from structures. Common Fill can be placed up to 2-feet below the foundation elements. Compaction testing should be performed periodically (at least once every 25,000 sq-ft or a minimum of once per lift).

5.3 Stone Fill

Stone Fill with not more than 10-percent material passing the number 4 sieve, such as a ¾-inch crushed stone, is recommended for prepared subgrades for equipment pad construction. Stone Fill should be placed in loose lifts not to exceed 14-inches in thickness for heavy compaction equipment and 10-inches for lighter compaction equipment.

When Stone Fill is used as a drainage medium, it should be uniformly graded. A non-woven, geotextile meeting AASHTO M288 Survivability Class 3, such as a Mirafi 140N or equivalent, should be placed between the Stone Fill and adjacent soils to prevent the migration of fines into the stone void space.

5.4 Pipe Bedding Material

Pipe Bedding Material should be a clean, granular, bedding fill meeting the following gradation and be placed in lifts not exceeding 8-inches loose, measured and compacted to 95% of maximum dry density as defined by ASTM D 1557:

Sieve Size	Percent Passing by Weight
¾-inch	100
No. 60	0-35
No. 100	0-10

6.0 CLOSING

LaBella has prepared this report for the exclusive use by Nexamp Inc. and their design team. LaBella's recommendations for the energy storage area and foundations are based upon our understanding of the proposed construction and the information obtained from our subsurface exploration. Variations in the subsurface conditions may occur between test exploration locations or there may be changes in the planned construction during the design phase. As this may be the case, changes to our recommendations may be warranted.

Generally accepted soil mechanics and geotechnical engineering practices were used to develop the recommendations stated in this report. Our services were conducted in a manner that is in accordance with generally accepted geotechnical engineering practice. The geotechnical engineer of record should review the final plans and specifications to evaluate their consistency with LaBella's recommendations. Prospective bidders should understand that this report was prepared for design purposes only and may not contain sufficient information to prepare an accurate bid.

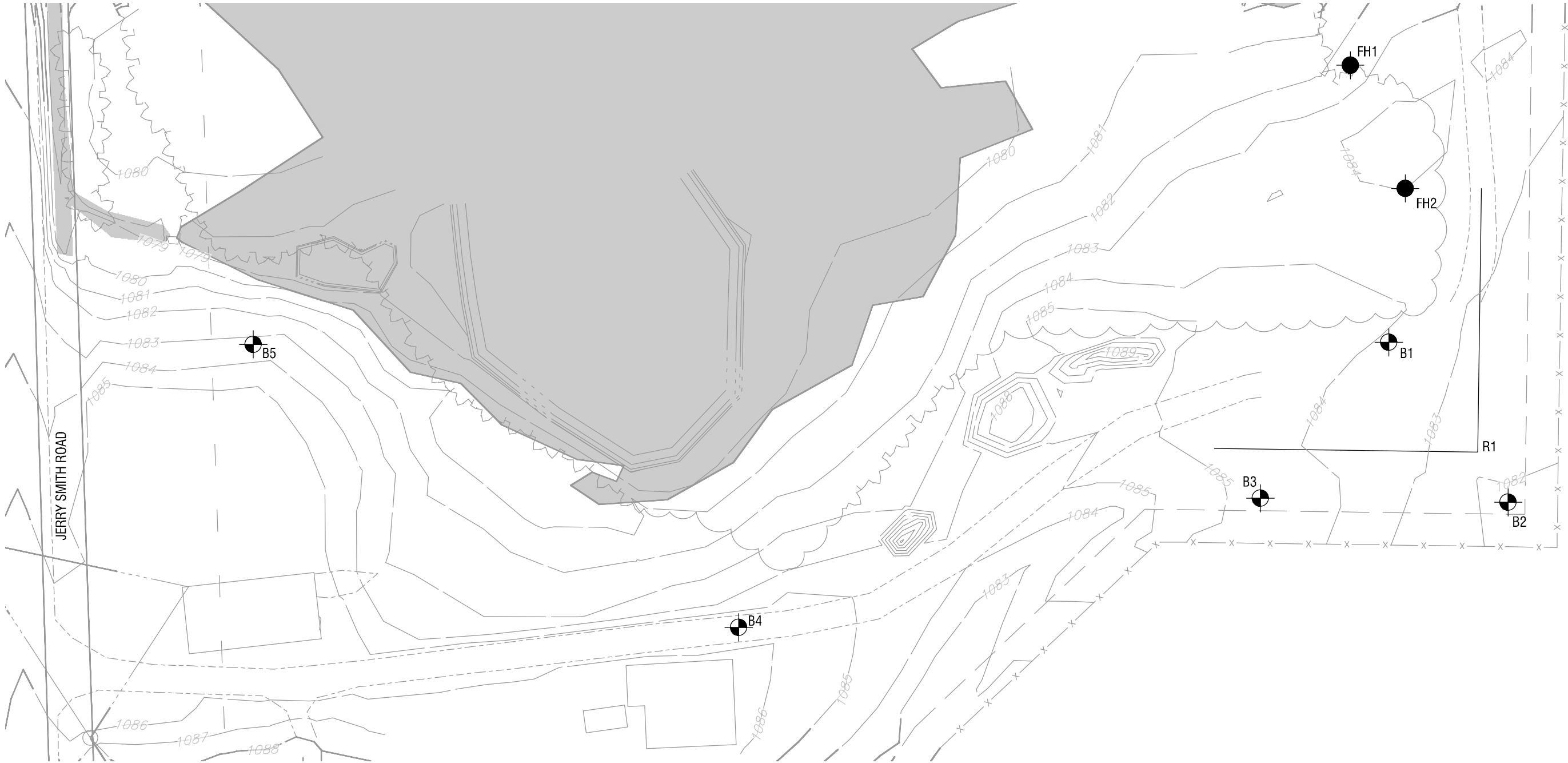
7.0 DISPOSITION OF SAMPLES

LaBella will hold all soil samples for 60-days after the date of this report. If the Client desires that these samples be retained for a longer period, the Client shall notify LaBella in writing and make arrangements to obtain the samples from LaBella prior to the expiration of the 60-day time period; otherwise, the samples will be properly disposed by LaBella.

APPENDIX A

FIGURE

9/18/2026 10:19:12 AM
B:\GLOBAL\Projects\Nexamp\2265555 - Jerry Smith Energy Storage\06_Drawings\Geotechnical\Fig1_2265555_Location_Plan.dwg



- NOTES**
- TOPOGRAPHIC INFORMATION SHOWN HEREON IS COMPILED FROM A FIELD SURVEY COMPLETED BY THE ENVIRONMENTAL DESIGN PARTNERSHIP, LLP DATED MAY 27, 2026 REFERENCING AN ASSUMED DATUM.
 - ELECTRICAL RESISTIVITY TEST LOCATION SHOWN HEREON WAS COMPLETED BY LABELLA ON AUGUST 12, 2026.
 - TEST BORING LOCATIONS SHOWN HEREON WERE COMPLETED BY LABELLA ON AUGUST 12, 2026 AND AUGUST 13, 2026.
 - FALLING HEAD TEST LOCATIONS SHOWN HEREON WERE COMPLETED BY LABELLA ON AUGUST 13, 2026.

- LEGEND**
- TEST BORING LOCATION
B1
 - ELECTRICAL RESISTIVITY TEST
R1
 - FALLING HEAD TEST LOCATION
FH1

1
FIG1

EXPLORATION LOCATION PLAN

SCALE: 1" = 50'

North Arrow

LaBella
Powered by partnership

4 British American Boulevard
Latham, NY 12110
518-273-0055
labellapc.com

C.A. NUMBER:
GEOLOGICAL: 018750
LAND SURVEYING: 017976
PROFESSIONAL ENGINEERING: 018281

It is a violation of New York Education Law Art. 145 Sec. 7209 & Art. 147 Sec. 7307, for any person, unless acting under the direction of a licensed architect, professional engineer, or land surveyor, to alter an item in any way. If an item bearing the seal of an architect, engineer, or land surveyor is altered; the altering architect, engineer, or land surveyor shall affix to the item their seal and notation "altered by:" followed by their signature and date of such alteration, and a specific description of the alteration.

DRAWING NAME:
EXPLORATION LOCATION PLAN

PROJECT NAME:
JERRY SMITH ENERGY STORAGE
JERRY SMITH ROAD, LANSING

ISSUED FOR:
ISSUED FOR REPORT

DRAWN BY:
LL

DATE:
08/13/2026

PROJECT NO.:
2265555

DRAWING NUMBER:
FIG1

APPENDIX B

EXPLORATION LOGS

SOIL GENERAL INFORMATION SHEET

Soil Identification Terms

The exploration logs present observations and the results of tests performed in the field by the Geotechnical Engineers, Geologists, and Driller as noted. The identification of soils or soil like material is subject to limitations imposed by the field conditions and equipment used.

Soil identifications are visual descriptions on the basis of ASTM D2488. The soil density or consistency is based on the penetration resistance determined by ASTM D1586.

The following defines some of the terms utilized in the preparation of the exploration logs.

Abbreviations

N-Value - Standard Penetration Value	WOH - Weight of Rods & Hammer
WOR - Weight of Rods	▼ - Observed Groundwater Surface
NWE - No Water Encountered	bgs - below ground surface
HSG - Hydrologic Soil Group	N/A - Not Applicable

Relative Density of Coarse Grained Soils		Consistency of Fine Grained Soils		
Soil Density	N Value	Soil Consistency	Unconfined Compressive Strength, (tsf)	N Value
Very Loose	0 to 4	Very Soft	≤ 0.25	0 to 2
Loose	5 to 10	Soft	0.25 to 0.5	3 to 4
Medium Dense	11 to 30	Medium Stiff	0.5 to 1.0	5 to 8
Dense	31 to 50	Stiff	1.0 to 2.0	9 to 15
Very Dense	> 50	Very Stiff	2.0 to 1.0	16 to 30
		Hard	> 4.0	>30

Size Description	
Soil Type	Particle Size
Boulder	> 12"
Cobble	12" - 3"
Gravel-Coarse	3" - 3/4"
Gravel-Fine	3/4" - #4
Sand-Coarse	#4 - #10
Sand-Medium	#10 - #40
Sand-Fine	#40 - #200
Silt/Clay	< #200

> 3", Cobble/Boulder Size Percent Ranges	
Quantifier	Percent
Occasional	< 15
Frequent	15 - 35
Very	35 - 60
Extremely	60 - 90

Moisture Condition	
Term	Description
Dry	No moisture to touch.
Moist	Damp but no visible water.
Wet	Free water in sample.

< 3", Soil Particle Size Percent Ranges	
Quantifier	Percent
Mostly	> 50
Some	30 - 45
Little	15 - 25
Few	5 - 10
Trace	< 5

Field Plasticity	
Term	Description
Non-plastic	A 1/8-in. thread cannot be rolled at any water content.
Low	A thread can barely be rolled and a lump cannot be formed.
Medium	A thread is easy to roll and not much time is required to reach the plastic limit. A lump crumbles when drier than the plastic limit.
High	It takes considerable time rolling and kneading to reach the plastic limit.

Dilatancy	
Term	Description
None	No visible change in the specimen.
Slow	Water appears slowly on the specimen surface during shaking.
Rapid	Water appears quickly on the specimen surface during shaking.

Notes:

1. The N-Value is the summation of the second and the third SPT values.
2. Split-spoon refusal is considered 50 blows over six inches.
3. If 50 blows is reached prior to advancing 6-inches, the penetration is reported as 50 over the nearest 1 inch value.
4. 100 is the maximum reported N-Value on the Exploration Logs.
5. Auger, Roller Bit, and/or Casing refusal occurs if the driller is unable to advance the boring.

TEST BORING LOG

Page 1 of 8

 4 British American Blvd. Latham, NY 12110 Phn: (518) 273-0055 Fax: (518) 273-8391							PROJECT: Jerry Smith Storage LOCATION: Jerry Smith Road, Lansing, NY CLIENT: Nexamp Inc PROJECT NO.: 2265555			Test Boring No.: B1																	
Contractor: LaBella Environmental, LLC Drill Rig: CME 55 Driller: Wayne Farris Inspector: Michael Thomas							Start Date: 12-Aug-2026 Finish Date: 12-Aug-2026 EL. Datum: Assumed Datum G.S. Elevation: 1,084			Northing: See Figure Easting: Latitude: 42.59283 Longitude: -76.56402																	
							Total Depth: 22 ft.			Borehole Dia.: 2.25 in. Water Depth: 20.5 ft. Bedrock Depth: NE ft. Sample Hammer: Automatic																	
Depth (Ft)	Elevation (Ft)	Sample No.	SPT Blows	Recovery (in)	Groundwater	Group Symbol	Stratum Descriptions:			Field Notes, Comments:																	
1	1083	SS-1	2	8		SM	Silty Sand (SM): Mostly Sand, little Silt, few Gravel, trace Clay, brown, dry (Reworked Material)			Moisture Content: 18%																	
2	1082		5							Approximate Strata Change																	
3	1081		6																								
4	1080		7																								
5	1079	SS-2	22	16		SM	4" Silty Sand (SM): Mostly Sand, some Silt, few Gravel, trace Clay, brown, dry (Loamy Till)																				
6	1078		9																								
7	1077		23			SM	12" Silty Sand w/ Gravel (SM): Mostly Sand, some Gravel, little Silt, few Clay, brown/gray, dry																				
8	1076		35																								
9	1075																										
10	1074	SS-3	32	0			Similar to SS-4 based on cuttings			Gravel in spoon tip																	
11	1073		48																								
12	1072		50/5																								
13	1071																										
14	1070																										
15	1069	SS-4	10	18		GC	Clayey Gravel w/ Sand (GC): Mostly Gravel, some Clay, little Sand, few Silt, gray, dry to moist																				
16	1068		23																								
17	1067		16																								
18	1066		22																								
19	1065																										
20	1064	SS-5	10	12	▼	GC	Clayey Gravel w/ Sand (GC)																				
METHODS: HA- Hollow Stem Auger, RWH- Rotary Wash, SSA- Solid Stem Auger, DC-Diamond Core, DP-Direct Push SAMPLE TYPES: SS-Split Spoon, RC-Bedrock Core, GS-Geoprobe Sleeve, ST-Shelby Tube STANDARD 1. Samples classified in accordance with ASTM D-2488 unless otherwise noted. NOTES: 2. Test Boring Log Page 1: 0 - 20 feet. Each subsequent page: Additional 25 feet. 3. Refer to the "Interpretation of Subsurface Logs" for additional symbology and abbreviation definitions.										DRILLING INFORMATION Method: HA 0 to 20.0 Method: <table border="1"> <tr> <th></th> <th>Sample</th> <th>Core</th> </tr> <tr> <td>Type</td> <td>SS</td> <td></td> </tr> <tr> <td>Int Diam.</td> <td>2.25</td> <td></td> </tr> <tr> <td>Weight</td> <td>140 lb</td> <td></td> </tr> <tr> <td>Fall</td> <td>30"</td> <td></td> </tr> </table>				Sample	Core	Type	SS		Int Diam.	2.25		Weight	140 lb		Fall	30"	
	Sample	Core																									
Type	SS																										
Int Diam.	2.25																										
Weight	140 lb																										
Fall	30"																										
ADDITIONAL NOTES:																											

TEST BORING LOG

Page 2 of 8

 4 British American Blvd. Latham, NY 12110 Phn: (518) 273-0055 Fax: (518) 273-8391							PROJECT: Jerry Smith Storage LOCATION: Jerry Smith Road, Lansing, NY CLIENT: Nexamp Inc PROJECT NO.: 2265555		Test Boring No.: B1
							Total Depth: 22 ft.		
Depth (Ft)	Elevation (Ft)	Sample No.	SPT Blows	Recovery (in)	Groundwater	Group Symbol	Stratum Descriptions:	Field Notes, Comments:	
21	1063		32			GC	Clayey Gravel w/ Sand (GC): Mostly Gravel, some Clay, little Sand, few Silt, gray, moist to wet		
			20						
			17						
22	1062						Boring Terminated at 22-ft bgs		
23	1061								
24	1060								
25	1059								
26	1058								
27	1057								
28	1056								
29	1055								
30	1054								
31	1053								
32	1052								
33	1051								
34	1050								
35	1049								
36	1048								
37	1047								
38	1046								
39	1045								
40	1044								
41	1043								
42	1042								
43	1041								
44	1040								
45	1039								

ADDITIONAL NOTES:

TEST BORING LOG

Page 3 of 8

 LaBella Powered by partnership.							4 British American Blvd. Latham, NY 12110 Phn: (518) 273-0055 Fax: (518) 273-8391			PROJECT: Jerry Smith Storage LOCATION: Jerry Smith Road, Lansing, NY CLIENT: Nexamp Inc PROJECT NO.: 2265555			Test Boring No.: B2														
Contractor: LaBella Environmental, LLC Drill Rig: CME 55 Driller: Wayne Farris Inspector: Michael Thomas							Start Date: 12-Aug-2026 Finish Date: 12-Aug-2026 EL. Datum: Assumed Datum G.S. Elevation: 1,082			Northing: See Figure Easting: Latitude: 42.59263 Longitude: -76.56433			Total Depth: 22 ft. Borehole Dia.: 2.25 in. Water Depth: 21 ft. Bedrock Depth: NE ft. Sample Hammer: Automatic														
Depth (Ft)	Elevation (Ft)	Sample No.	SPT Blows	Recovery(in)	Groundwater	Group Symbol	Stratum Descriptions:			Field Notes, Comments:																	
1	1081	SS-1	1	13		SM	Silty Sand w/ Gravel (SM): Mostly Sand, some Silt, little Gravel, few Clay, brown, dry (Reworked Material)																				
2	1080		3							Approximate Strata Change																	
3	1079		5																								
4	1078		6																								
5	1077																										
6	1076	SS-2	8	15		SC - SM	Silty Clayey Sand w/ Gravel (SC-SM): Mostly Sand, little Gravel, few Silt, few Clay, brown, dry (Loamy Till)			Moisture Content = 8%																	
7	1075		16																								
8	1074		15																								
9	1073		25																								
10	1072																										
11	1071	SS-3	7	14		SM	Silty Sand w/ Gravel (SM): Mostly Sand, some Gravel, little Silt, few Clay, brown, dry																				
12	1070		10																								
13	1069		10																								
14	1068		19																								
15	1067																										
16	1066	SS-4	5	10		SM	Silty Sand w/ Gravel (SM): Mostly Sand, some Gravel, little Silt, few Clay, brown, dry																				
17	1065		12																								
18	1064		12																								
19	1063		14																								
20	1062																										
		SS-5	21	12		SM	Silty Sand w/ Gravel (SM)																				
METHODS: HA- Hollow Stem Auger, RWH- Rotary Wash, SSA- Solid Stem Auger, DC-Diamond Core, DP-Direct Push SAMPLE TYPES: SS-Split Spoon, RC-Bedrock Core, GS-Geoprobe Sleeve, ST-Shelby Tube STANDARD 1. Samples classified in accordance with ASTM D-2488 unless otherwise noted. NOTES: 2. Test Boring Log Page 1: 0 - 20 feet. Each subsequent page: Additional 25 feet. 3. Refer to the "Interpretation of Subsurface Logs" for additional symbology and abbreviation definitions.										DRILLING INFORMATION Method: HA 0 to 20.0 Method: <table border="1"> <tr> <th></th> <th>Sample</th> <th>Core</th> </tr> <tr> <td>Type</td> <td>SS</td> <td></td> </tr> <tr> <td>Int Diam.</td> <td>2.25</td> <td></td> </tr> <tr> <td>Weight</td> <td>140 lb</td> <td></td> </tr> <tr> <td>Fall</td> <td>30"</td> <td></td> </tr> </table>				Sample	Core	Type	SS		Int Diam.	2.25		Weight	140 lb		Fall	30"	
	Sample	Core																									
Type	SS																										
Int Diam.	2.25																										
Weight	140 lb																										
Fall	30"																										
ADDITIONAL NOTES:																											

TEST BORING LOG

Page 4 of 8

 4 British American Blvd. Latham, NY 12110 Phn: (518) 273-0055 Fax: (518) 273-8391						PROJECT: Jerry Smith Storage LOCATION: Jerry Smith Road, Lansing, NY CLIENT: Nexamp Inc PROJECT NO.: 2265555		Test Boring No.: B2	
						Total Depth: 22 ft.			
Depth (Ft)	Elevation (Ft)	Sample No.	SPT Blows	Recovery (in)	Groundwater	Group Symbol	Stratum Descriptions:	Field Notes, Comments:	
21	1061	21			▼	SM	Silty Sand w/ Gravel (SM): Mostly Sand, some Gravel, little Silt, few Clay, brown, wet		
		10							
		12							
22	1060						Boring Terminated at 22-ft bgs		
23	1059								
24	1058								
25	1057								
26	1056								
27	1055								
28	1054								
29	1053								
30	1052								
31	1051								
32	1050								
33	1049								
34	1048								
35	1047								
36	1046								
37	1045								
38	1044								
39	1043								
40	1042								
41	1041								
42	1040								
43	1039								
44	1038								
45	1037								
ADDITIONAL NOTES:									

TEST BORING LOG

Page 5 of 8

 LaBella Powered by partnership.							4 British American Blvd. Latham, NY 12110 Phn: (518) 273-0055 Fax: (518) 273-8391			PROJECT: Jerry Smith Storage LOCATION: Jerry Smith Road, Lansing, NY CLIENT: Nexamp Inc PROJECT NO.: 2265555			Test Boring No.: B3														
Contractor: LaBella Environmental, LLC Drill Rig: CME 55 Driller: Wayne Farris Inspector: Michael Thomas							Start Date: 13-Aug-2026 Finish Date: 13-Aug-2026 EL. Datum: Assumed Datum G.S. Elevation: 1,085			Northing: See Figure Easting: Latitude: 42.59298 Longitude: -76.56433			Total Depth: 22 ft. Borehole Dia.: 2.25 in. Water Depth: 21 ft. Bedrock Depth: NE ft. Sample Hammer: Automatic														
Depth (Ft)	Elevation (Ft)	Sample No.	SPT Blows	Recovery(in)	Groundwater	Group Symbol	Stratum Descriptions:			Field Notes, Comments:																	
1	1084	SS-1	1	12		SM	Silty Sand w/ Gravel (SM): Mostly Sand, some Silt, little Gravel, few Clay, brown, dry (Reworked Material)																				
2	1083		2							Approximate Strata Change																	
3	1082		6																								
4	1081		6																								
5	1080																										
6	1079	SS-2	10	13		SM	Silty Sand w/ Gravel (SM): Mostly Sand, some Silt, little Gravel, few Clay, brown, moist (Loamy Till)																				
7	1078		12																								
8	1077		18																								
9	1076		19																								
10	1075																										
11	1074	SS-3	10	15		SM	Silty Sand w/ Gravel (ML): Mostly Sand, some Silt, little Gravel, few Clay, brown, moist																				
12	1073		26																								
13	1072		50/5																								
14	1071																										
15	1070																										
16	1069	SS-4	10	9		SM	Silty Sand w/ Gravel (SM): Mostly Sand, some Gravel, little Silt, few Clay, brown, moist																				
17	1068		13																								
18	1067		16																								
19	1066		13																								
20	1065																										
		SS-5	4	12		SM	Silty Sand w/ Gravel (SM)																				
METHODS: HA- Hollow Stem Auger, RWH- Rotary Wash, SSA- Solid Stem Auger, DC-Diamond Core, DP-Direct Push SAMPLE TYPES: SS-Split Spoon, RC-Bedrock Core, GS-Geoprobe Sleeve, ST-Shelby Tube STANDARD 1. Samples classified in accordance with ASTM D-2488 unless otherwise noted. NOTES: 2. Test Boring Log Page 1: 0 - 20 feet. Each subsequent page: Additional 25 feet. 3. Refer to the "Interpretation of Subsurface Logs" for additional symbology and abbreviation definitions.										DRILLING INFORMATION Method: HA 0 to 20.0 Method: <table border="1"> <tr> <th></th> <th>Sample</th> <th>Core</th> </tr> <tr> <td>Type</td> <td>SS</td> <td></td> </tr> <tr> <td>Int Diam.</td> <td>2.25</td> <td></td> </tr> <tr> <td>Weight</td> <td>140 lb</td> <td></td> </tr> <tr> <td>Fall</td> <td>30"</td> <td></td> </tr> </table>				Sample	Core	Type	SS		Int Diam.	2.25		Weight	140 lb		Fall	30"	
	Sample	Core																									
Type	SS																										
Int Diam.	2.25																										
Weight	140 lb																										
Fall	30"																										
ADDITIONAL NOTES:																											

TEST BORING LOG

Page 6 of 8

 4 British American Blvd. Latham, NY 12110 Phn: (518) 273-0055 Fax: (518) 273-8391							PROJECT: Jerry Smith Storage LOCATION: Jerry Smith Road, Lansing, NY CLIENT: Nexamp Inc PROJECT NO.: 2265555		Test Boring No.: B3
							Total Depth: 22 ft.		
Depth (Ft)	Elevation (Ft)	Sample No.	SPT Blows	Recovery (in)	Groundwater	Group Symbol	Stratum Descriptions:	Field Notes, Comments:	
21	1064		9		▼	SM	Silty Sand w/ Gravel (SM): Mostly Sand, some Gravel, little Silt, few Clay, brown, wet		
			10						
			18						
22	1063						Boring Terminated at 22-ft bgs		
23	1062								
24	1061								
25	1060								
26	1059								
27	1058								
28	1057								
29	1056								
30	1055								
31	1054								
32	1053								
33	1052								
34	1051								
35	1050								
36	1049								
37	1048								
38	1047								
39	1046								
40	1045								
41	1044								
42	1043								
43	1042								
44	1041								
45	1040								

ADDITIONAL NOTES:

TEST BORING LOG

Page 7 of 8

 4 British American Blvd. Latham, NY 12110 Phn: (518) 273-0055 Fax: (518) 273-8391							PROJECT: Jerry Smith Storage LOCATION: Jerry Smith Road, Lansing, NY CLIENT: Nexamp Inc PROJECT NO.: 2265555			Test Boring No.: B4																	
Contractor: LaBella Environmental, LLC Drill Rig: CME 55 Driller: Wayne Farris Inspector: Michael Thomas							Start Date: 13-Aug-2026 Finish Date: 13-Aug-2026 El. Datum: Assumed Datum G.S. Elevation: 1,086			Northing: See Figure Easting: Latitude: 42.59363 Longitude: -76.56454																	
							Total Depth: 10.9 ft.			Borehole Dia.: 2.25 in. Water Depth: NWE ft. Bedrock Depth: NE ft. Sample Hammer: Automatic																	
Depth (Ft)	Elevation (Ft)	Sample No.	SPT Blows	Recovery(in)	Groundwater	Group Symbol	Stratum Descriptions:			Field Notes, Comments:																	
1	1085	SS-1	1	13		SM	Silty Sand w/ Gravel (SM): Mostly Sand, some Gravel, little Silt, few Clay, brown, dry (Reworked Material)																				
2	1084		4							Approximate Strata Change																	
3	1083		5																								
4	1082		7																								
5	1081																										
6	1080	SS-2	6	19		SM	Silty Sand w/ Gravel (SM): Mostly Sand, some Silt, little Gravel, trace Clay, brown, dry (Loamy Till)																				
7	1079		12																								
8	1078		23																								
9	1077		41																								
10	1076																										
11	1075	SS-3	18	11		SM	Silty Sand w/ Gravel (SM): Mostly Sand, some Silt, little Gravel, few Clay, brown, dry																				
12	1074		50/5				Boring Terminated at 10.9-ft bgs																				
13	1073																										
14	1072																										
15	1071																										
16	1070																										
17	1069																										
18	1068																										
19	1067																										
20	1066																										
METHODS: HA- Hollow Stem Auger, RWH- Rotary Wash, SSA- Solid Stem Auger, DC-Diamond Core, DP-Direct Push SAMPLE TYPES: SS-Split Spoon, RC-Bedrock Core, GS-Geoprobe Sleeve, ST-Shelby Tube STANDARD 1. Samples classified in accordance with ASTM D-2488 unless otherwise noted. NOTES: 2. Test Boring Log Page 1: 0 - 20 feet. Each subsequent page: Additional 25 feet. 3. Refer to the "Interpretation of Subsurface Logs" for additional symbology and abbreviation definitions.							DRILLING INFORMATION Method: HA 0 to 10.0 Method: <table border="1"> <tr> <th></th> <th>Sample</th> <th>Core</th> </tr> <tr> <td>Type</td> <td>SS</td> <td></td> </tr> <tr> <td>Int Diam.</td> <td>2.25</td> <td></td> </tr> <tr> <td>Weight</td> <td>140 lb</td> <td></td> </tr> <tr> <td>Fall</td> <td>30"</td> <td></td> </tr> </table>							Sample	Core	Type	SS		Int Diam.	2.25		Weight	140 lb		Fall	30"	
	Sample	Core																									
Type	SS																										
Int Diam.	2.25																										
Weight	140 lb																										
Fall	30"																										
ADDITIONAL NOTES:																											

TEST BORING LOG

Page 8 of 8

 LaBella Powered by partnership.							4 British American Blvd. Latham, NY 12110 Phn: (518) 273-0055 Fax: (518) 273-8391			PROJECT: Jerry Smith Storage LOCATION: Jerry Smith Road, Lansing, NY CLIENT: Nexamp Inc PROJECT NO.: 2265555			Test Boring No.: B5		
Contractor: LaBella Environmental, LLC Drill Rig: CME 55 Driller: Wayne Farris Inspector: Michael Thomas							Start Date: 13-Aug-2026 Finish Date: 13-Aug-2026 El. Datum: Assumed Datum G.S. Elevation: 1,083			Northing: See Figure Easting: Latitude: 42.59426 Longitude: -76.56415			Total Depth: 12 ft. Borehole Dia.: 2.25 in. Water Depth: NWE ft. Bedrock Depth: NE ft. Sample Hammer: Automatic		
Depth (Ft)	Elevation (Ft)	Sample No.	SPT Blows	Recovery(in)	Groundwater	Group Symbol	Stratum Descriptions:			Field Notes, Comments:					
1	1082	SS-1	4	7		SM	Silty Sand w/ Gravel (SM): Mostly Sand, some Gravel, little Silt, brown, dry (Loamy Till)								
			5												
			9												
2	1081		9												
3	1080														
4	1079														
5	1078														
6	1077														
7	1076						Silty Sand w/ Gravel (SM): Mostly Sand, some Gravel, little Silt, brown, moist								
		SS-2	5	4		SM									
			6												
8	1075		7												
9	1074														
10	1073														
11	1072														
12	1071														
13	1070						Silty Sand w/ Gravel (SM): Mostly Sand, some Gravel, little Silt, brown, moist								
		SS-3	6	20		SM									
			13												
			22												
14	1069														
15	1068														
16	1067														
17	1066														
18	1065						Boring Terminated at 12-ft bgs								
19	1064														
20	1063														

METHODS: HA- Hollow Stem Auger, RWH- Rotary Wash, SSA- Solid Stem Auger, DC-Diamond Core, DP-Direct Push SAMPLE TYPES: SS-Split Spoon, RC-Bedrock Core, GS-Geoprobe Sleeve, ST-Shelby Tube STANDARD 1. Samples classified in accordance with ASTM D-2488 unless otherwise noted. NOTES: 2. Test Boring Log Page 1: 0 - 20 feet. Each subsequent page: Additional 25 feet. 3. Refer to the "Interpretation of Subsurface Logs" for additional symbology and abbreviation definitions.							DRILLING INFORMATION			
ADDITIONAL NOTES:							Method: HA		0 to 10.0	
							Method:			
									Sample	Core
							Type	SS		
							Int Diam.	2.25		
							Weight	140 lb		
							Fall	30"		

Name: Jerry Smith Road
LaBella Job: 2265555
Attendees: Michael Thomas

Date: 8/13/2026
Address: Jerry Smith, Lansing, NY

Weather: Sunny
Temperature: 75 F

Falling Head Permeability Tests				
Falling Head Test No.	Test Depth (inches)	Trial	Rate	Remarks
FH1	84	Trial 1:	2"/hr	Based on Auger cuttings upper 6.5-ft of soil similar to FH2
		Trial 2:	4"/hr	
		Trial 3:	4"/hr	
FH2	84	Trial 1:	1"/hr	Upper 10-ft, stratum similar to Boring 1 Moved over 5-ft and set casing
		Trial 2:	6"/hr	
		Trial 3:	6"/hr	

APPENDIX C

LABORATORY TEST RESULTS

Albany
22 Corporate Drive
Clifton Park, NY 12065
Phone: 518.383.9144

Client:
Labella Associates
300 State Street
Suite 201
Rochester, NY 14614

Project:
AT3764
2026 Laboratory Services
123
Clifton Park, NY 12065

General Information (for Soil Sample)

Sample Date:	08/18/2026	Sample Number:	214189
Sample From:	Boring	Technician:	Client
Location Details:	LaBella Project No. 2265555. NEXAMP - Jerry Smith - Sample ID: B1, SS-1, (0'-2')		
Material Type:	Native Soil	Location:	

Test Results

Description of Test Material:		B1, SS-1, (0'-2')		
Mass of Test Sample Meet Minimum Requirement:		Yes		
Test Specimen Contain More Than One Sample Type (layered):		No		
Percent of Water Content (Method A):		18		
Test Completed Date:		09/09/2026	Test Completed By:	Aupperle, Michael



Olivia Bartik, Geologist 09/14/2026

Albany
22 Corporate Drive
Clifton Park, NY 12065
Phone: 518.383.9144

Client:
Labella Associates
300 State Street
Suite 201
Rochester, NY 14614

Project:
AT3764
2026 Laboratory Services
123
Clifton Park, NY 12065

General Information (for Soil Sample)

Sample Date:	08/18/2026	Sample Number:	214190
Sample From:	Boring	Technician:	Client
Location Details:	LaBella Project No. 2265555. NEXAMP - Jerry Smith - Sample ID: B2, SS-2, (5'-7')		
Material Type:	Native Soil	Location:	

Test Results

Description of Test Material:		B2, SS-2, (5'-7')		
Mass of Test Sample Meet Minimum Requirement:		Yes		
Test Specimen Contain More Than One Sample Type (layered):		No		
Percent of Water Content (Method A):		8		
Test Completed Date:		09/09/2026	Test Completed By:	Aupperle, Michael



Olivia Bartik, Geologist 09/14/2026

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Client:
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Rochester, NY 14614

Project:
AT3764
2026 Laboratory Services
123
Clifton Park, NY 12065

Sampling Information

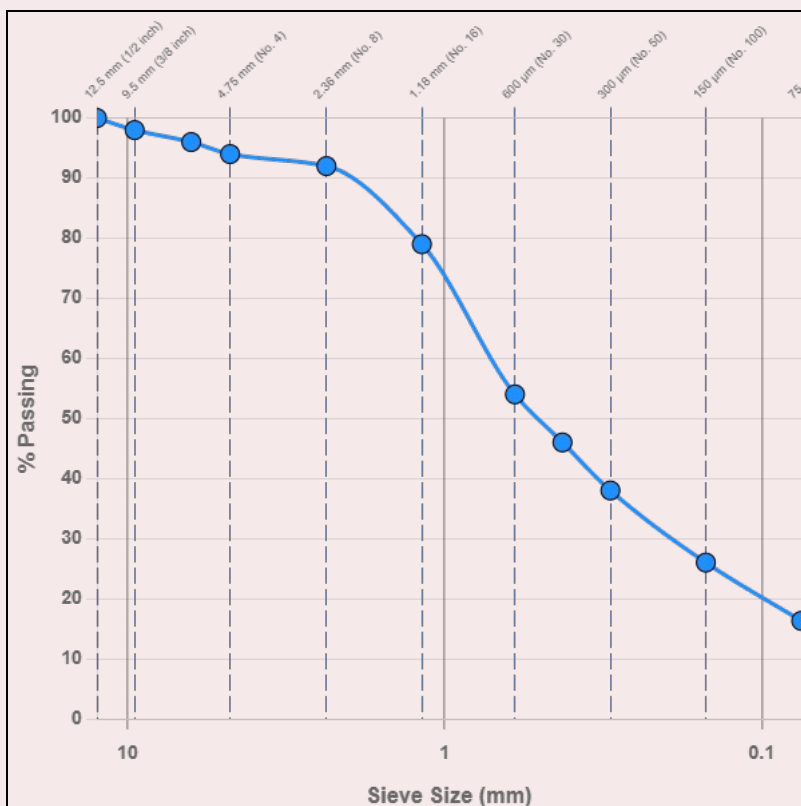
Sample Number: 214189	Sampled By: Client
Sampling Method: Not Provided	Sample Date: 08/18/2026
Sample From: Boring	Material Type: Native Soil
Color/Material Description: B1, SS-1, (0'-2')	
Location Details: LaBella Project No. 2265555. NEXAMP - Jerry Smith - Sample ID: B1, SS-1, (0'-2')	

Test Results

Test Completed By: Aupperle, Michael		Test Completed Date: 09/10/2026	
Sieve Analysis Data			
Sieve Size	Percent Passing	Spec. Range	Spec. Result
12.5 mm (1/2 inch)	100	---	---
9.5 mm (3/8 inch)	98	---	---
6.3 mm (1/4 inch)	96	---	---
4.75 mm (No. 4)	94	---	---
2.36 mm (No. 8)	92	---	---
1.18 mm (No. 16)	79	---	---
600 µm (No. 30)	54	---	---
425 µm (No. 40)	46	---	---
300 µm (No. 50)	38	---	---
150 µm (No. 100)	26	---	---
75 µm (No. 200)	16.3	---	---
Test Specification: ASTM D422			

The graph displays the sieve analysis results for a sample. The y-axis represents the percentage of material passing through the sieve, ranging from 0 to 100. The x-axis represents the sieve size in millimeters on a logarithmic scale, with major ticks at 10, 1, and 0.1 mm. The data points are plotted as blue circles and connected by a smooth blue curve. The curve shows that approximately 92% of the material passes through a 2.36 mm sieve, and only about 16.3% passes through a 75 µm sieve.

Sieve Size (mm)	% Passing
12.5	100
9.5	98
6.3	96
4.75	94
2.36	92
1.18	79
0.60	54
0.425	46
0.30	38
0.15	26
0.075	16.3



Albany

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Project:

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2026 Laboratory Services
123
Clifton Park, NY 12065



Olivia Bartik, Geologist 09/14/2026

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300 State Street
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Rochester, NY 14614

Project:
AT3764
2026 Laboratory Services
123
Clifton Park, NY 12065

Sampling Information

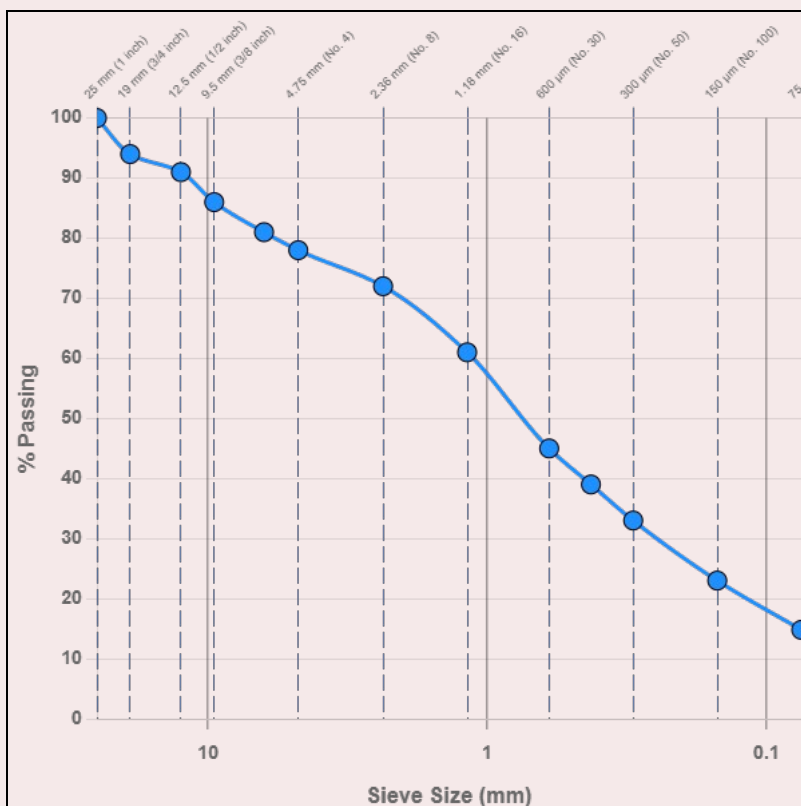
Sample Number: 214190	Sampled By: Client
Sampling Method: Not Provided	Sample Date: 08/18/2026
Sample From: Boring	Material Type: Native Soil
Color/Material Description: B2, SS-2, (5'-7')	
Location Details: LaBella Project No. 2265555. NEXAMP - Jerry Smith - Sample ID: B2, SS-2, (5'-7')	

Test Results

Test Completed By: Aupperle, Michael		Test Completed Date: 09/10/2026	
Sieve Analysis Data			
Sieve Size	Percent Passing	Spec. Range	Spec. Result
25 mm (1 inch)	100	---	---
19 mm (3/4 inch)	94	---	---
12.5 mm (1/2 inch)	91	---	---
9.5 mm (3/8 inch)	86	---	---
6.3 mm (1/4 inch)	81	---	---
4.75 mm (No. 4)	78	---	---
2.36 mm (No. 8)	72	---	---
1.18 mm (No. 16)	61	---	---
600 µm (No. 30)	45	---	---
425 µm (No. 40)	39	---	---
300 µm (No. 50)	33	---	---
150 µm (No. 100)	23	---	---
75 µm (No. 200)	14.8	---	---
Test Specification: ASTM D422			

The graph displays the sieve analysis results for a sample. The y-axis represents the percentage of material passing through the sieve, ranging from 0 to 100. The x-axis represents the sieve size in millimeters on a logarithmic scale, with major ticks at 10, 1, and 0.1 mm. The data points are plotted as blue circles connected by a blue line. The curve shows a gradual decrease in percent passing from 25 mm to 1.18 mm, followed by a steeper decline between 1.18 mm and 0.075 mm.

Sieve Size (mm)	Percent Passing (%)
25	100
19	94
12.5	91
9.5	86
6.3	81
4.75	78
2.36	72
1.18	61
0.6	45
0.425	39
0.3	33
0.15	23
0.075	14.8



Albany

22 Corporate Drive
Clifton Park, NY 12065
Phone: 518.383.9144

Client:

Labella Associates
300 State Street
Suite 201
Rochester, NY 14614

Project:

AT3764
2026 Laboratory Services
123
Clifton Park, NY 12065



Olivia Bartik, Geologist 09/14/2026

Albany
22 Corporate Drive
Clifton Park, NY 12065
Phone: 518.383.9144

Client:
Labella Associates
300 State Street
Suite 201
Rochester, NY 14614

Project:
AT3764
2026 Laboratory Services
123
Clifton Park, NY 12065

General Information

Sample Number:	214190	Sample Date:	08/18/2026
Sample From:	Boring	Sampling Method:	Not Provided
Location Details:	LaBella Project No. 2265555. NEXAMP - Jerry Smith - Sample ID: B2, SS-2, (5'-7')		
Location:		Technician:	Client

Test Results

Max Grain Size (mm):	19.0	As-Received % Moisture:	8.3
Removed 425 µm Larger Particles:	Pulverized and Screened	% Retain on 425 µm (No. 40) Sieve:	63.0
Prepared Sample:	Oven Dry	Grooving Tool:	Plastic
Liquid Limit Method:	Method A (Multipoint)	Plastic Limit Procedure:	Hand-Rolled
Liquid Limit - NP (Non-Plastic):	No	Plastic Limit - NP (Non-Plastic):	No
Liquid Limit (Method A):	20	Average Plastic Limit:	13
Plasticity Index Result (using LL Method A):		7	
Test Completed Date:	09/14/2026	Lab Technician:	Aupperle, Michael



Olivia Bartik, Geologist 09/14/2026

CORROSION ANALYSIS SUITE

Client: LaBella Associates
Project: NEXAMP - Jerry Smith
Client Project No.: 2265555

ATL Report No. AT3764CL-05-09-26
Report Date: September 14, 2026
Date Received: August 31, 2026

Boring: B3 **Sample:** SS-2 **Depth (ft):** ---

WATER-SOLUBLE SULFATE IN SOIL

ASTM C 1580

Sulfate by Mass of Sample (%)	Sulfate by Mass of Sample (mg/kg)
0.26	2600

MEASURING pH OF SOIL FOR USE IN CORROSION TESTING

ASTM G 51

Type of Test	Soil Temperature (°C)	pH Readings			Average
Laboratory	23.5	7.37	7.39	7.39	7.38

pH of calibration standards used: 7.00

MEASUREMENT OF SOIL RESISTIVITY USING THE TWO-ELECTRODE SOIL BOX METHOD

ASTM G 187 (LABORATORY)

Test Date: 09/11/26 **Performed by:** KAS
Meter Used: Miller 400A **Soil Box Factor:** 1.29

Date Collected	Temperature at Collection (°C)	Measured Resistance (Ω)	Calculated Resistivity (Ω·cm)
Not Provided	Not Provided	1,900	2,451

MEASUREMENT OF OXYGEN-REDUCTION POTENTIAL (ORP) OF SOIL

ASTM G 200

Test Date: 09/11/26 **Time of Measurements:** 10:00 AM
Date and Time Sample Extracted from the Ground: Not Provided
ORP Meter Man.: Extech Instruments **ORP Probe Man.:** Extech Instruments
Model number: SDL100 **Model number:** S500C - ORP/BNC

ORP Measurements (mV)			Ambient Temperature (°C)
188	187	186	25.5

WATER-SOLUBLE CHLORIDE ION CONTENT IN SOIL

AASHTO T 291, Method A

Chloride by Mass of Soil (mg/kg)
100

Reviewed By: Emma Caprood

Date: 09/14/26



ATLANTIC TESTING LABORATORIES

Rochester

3495 Winton Place, Building B
Rochester, New York 14623
585-427-9020 (T)

THERMAL CONDUCTIVITY OF SOIL AND SOFT ROCK BY THERMAL NEEDLE PROBE

IEEE 442

PROJECT INFORMATION

Client: Labella Associates
Project: 2026 Laboratory Services

ATL Report No.: AT3764TH-212324-09-26
Report Date: September 16, 2026
Date Received: August 26, 2026
Page: 1 of 1

SAMPLE INFORMATION

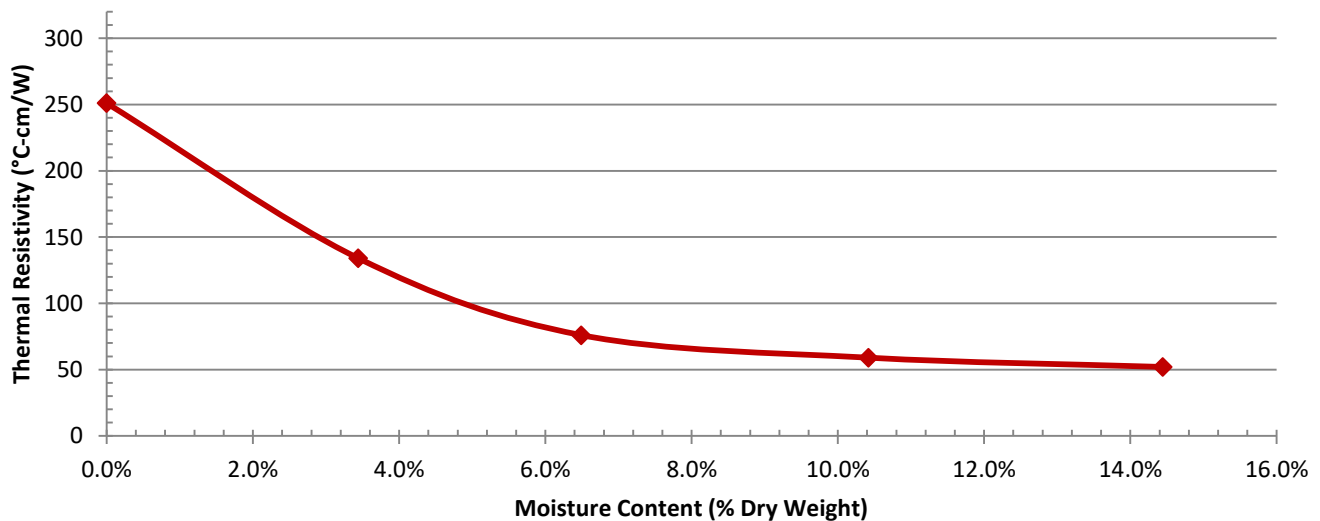
Source: Jerry Smith - Project No. 2265555

Boring Information:	Bulk Sample 0'-5'	Standard Proctor Values:	123.0 lb/ft ³ @ 11.8% M.C.
Sample Condition:	Bulk; Re-molded	Remolded Dry Density:	104.6 lb/ft ³ (85%)
ATL Sample No.:	212324	Received Moisture:	11.3%

Moisture Content (%)	Initial Soil Temperature (°C)	Thermal Resistivity of Sample (°C-cm/W)
0.0%	24.00	251
3.4%	22.60	134
6.5%	20.40	76
10.4%	20.50	59
14.4%	20.40	52

Thermal Dry-out Curve

◆ 212324 @ 85% Standard Proctor Density



Reviewed By: Michael Reed

Date: 9/16/2026

APPENDIX D

FIELD RESISTIVITY DATA

LaBella Associates

Soil Resistivity Test Field Report

JOB NAME/NO: Nexamp - Donner Road Energy Storage 2265558

DATE: 8/12/2026

Field Technician: Michael Thomas

Test Instrument: AEMC Model 4630

Procedure: 4 point soil resistivity test

LOCATION	SPACING	METER READING	CALCULATED SOIL RESISTIVITY
(map attached)	(ft)	(ohms)	$= 2\pi \times \text{SPACING}(\text{cm}) \times R$
R1 90°E	0.5	850.00	81,446
	1	471.00	90,261
	2	222.00	85,087
	3	190.00	109,233
	5	114.00	109,233
	6	78.50	90,261
	10	32.00	61,324
	30	12.50	71,864
	40	5.22	40,014
R1 360°N	0.5	790.00	75,697
	1	501.00	96,010
	2	253.00	96,968
	3	202.00	116,132
	5	131.00	125,522
	6	82.00	94,285
	10	40.00	76,655
	30	18.00	103,484
	40	6.81	52,202

- Very corrosive under 1,000 $\Omega \cdot \text{cm}$
- Aggressive from 1,000 to 5,000 $\Omega \cdot \text{cm}$
- Mildly corrosive from 5,000 to 10,000 $\Omega \cdot \text{cm}$
- Slightly corrosive from 10,000 to 20,000 $\Omega \cdot \text{cm}$
- Progressively less corrosive from 20,000 $\Omega \cdot \text{cm}$ to 30,000 $\Omega \cdot \text{cm}$
- Not corrosive greater than 30,000 $\Omega \cdot \text{cm}$