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LABELLE WAWA

City of Labelle, FL

TRAFFIC IMPACT STATEMENT

PREPARED FOR:

ComTerra Acquisitions, LLC

JOB NO. 24-147

DATE: 03/18/2025
REVISED: 07/08/2025
REVISED: 10/29/2025

Bryan G. Kelley, Professional Engineer, State of Florida, License No. 74006 This item has been digitally signed and sealed by Bryan G. Kelley, P.E., on 10/29/2025. Printed Copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.	Digitally signed by Bryan Kelley Date: 2025.10.29 10:18:25-04'00'

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SITE PLAN

1.0 SITE DATA

The subject parcel is located at the northwest corner of the Hickpochee Avenue and Main Street intersection in the City of Labelle, Florida. The proposed plan of development on the currently unimproved parcel consists of a 6,500 SF convenience store and a 12 fuel position gas station. Per coordination with the City of LaBelle, the 6,500 SF of convenience store area has been divided into 1,162 SF of fast food restaurant with a drive through and 5,338 SF of convenience store space to conservatively account for drive through trip generation.

Site access is proposed via two (2) full access driveway connections to College Street, one (1) full access driveway connection to Main Street and one (1) right in/right out driveway connection to Main Street. For additional information on site layout, please refer to the Site Plan prepared by Bohler. The site location map is shown in Figure 1 attached to this report.

2.0 TRAFFIC GENERATION

The traffic generated by the proposed site has been calculated in accordance with the traffic generation rates listed in the *ITE Trip Generation Manual, 11th Edition.*

Table 1 shows the daily traffic generation associated with the proposed development in trips per day (tpd). Tables 2 and 3 show the AM and PM peak hour traffic generation, respectively, in peak hour trips (pht). The traffic to be generated by the proposed development may be summarized as follows:

Proposed Development Net Trips

Weekday Daily Traffic Generation	=	1,001 tpd
A.M. Peak Hour Traffic Generation	=	78 pht (39 In/39 Out)
P.M. Peak Hour Traffic Generation	=	70 pht (35 In/35 Out)

Proposed Development Driveway Trips

Weekday Daily Traffic Generation	=	3,832 tpd
A.M. Peak Hour Traffic Generation	=	282 pht (142 In/140 Out)
P.M. Peak Hour Traffic Generation	=	268 pht (135 In/133 Out)

3.0 ROADWAY SEGMENT ANALYSIS

The roadway study area was evaluated for all major roadways within a 1-mile radius of the site. The project trips were distributed on the surrounding roadway network as shown in Figure 1 for all roadways within the study area. Table 4 documents the area-wide growth based on FDOT AADT data from 2019 to 2023 for the surrounding roadways. Tables 5 and 6 shows the peak hour project impact on each of the impacted roadways for the AM and PM peak hours, respectively. The roadway classification was derived from the FDOT Preliminary Context Classification website. The LOS D volumes thresholds are from the FDOT 2023 Multimodal Q/LOS Handbook.

All roadways in which the proposed development trips resulted in less than a 1.0% of the LOS D capacity meet Level of Service requirements. As shown in Tables 5 and 6, the project will have a significant impact on Hickpochee Avenue and Main Street. Therefore, these roadway segments were analyzed further.

3.0 ROADWAY SEGMENT ANALYSIS (CONTINUED)

The peak hour roadway link analysis for Hickpochee Avenue and Main Street is provided in Tables 7 and 8. The area wide growth calculated in Table 4 was utilized to grow the existing 2024 traffic to the anticipated 2029 traffic conditions. As shown in Tables 7 and 8, all significantly impacted roadway segments meet the required LOS standards inclusive of the proposed development.

4.0 INTERSECTION ANALYSIS

The Hickpochee Avenue and Main Street intersection has been analyzed for the 2024 existing conditions, 2029 background, and 2029 total traffic conditions for the A.M. and P.M. peak hours.

The 2029 total traffic has been calculated using the higher value between the background growth rate and the combination of a 1.0% background growth rate and the approved committed development trips. The operational analyst was prepared using Synchro 12 software with HCM 7th edition methodology. The existing signal timing obtained from the FDOT was also utilized. The Synchro results are included in Appendix D through F and may be summarized as follows:

Existing Conditions Analysis

The results of the existing conditions analysis are provided below in Table 9 and are included in Appendix D.

Table 9
2024 Existing Conditions Operational Analysis

Intersection	Intersection Control	Approach / Turn Lane	AM Peak Hour		PM Peak Hour	
			Delay (s)	LOS	Delay (s)	LOS
Hickpochee Ave at Main Street	Signal	Northbound Approach	44.4	D	44.9	D
		Southbound Approach	74.0	E	78.8	E
		SBL	52.8	D	50.1	D
		SBT/SBR	79.1	E	82.8	F
		Eastbound Approach	18.5	B	23.9	C
		EBL	14.4	B	16.1	B
		Westbound Approach	17.8	B	21.1	C
		Overall	30.1	C	33.1	C

As shown above, the study intersections currently operate at LOS D or better for both peak hours.

4.0 INTERSECTION ANALYSIS (CONTINUED)

Background Conditions

The 2029 background traffic volumes were determined by using an area wide 4.7% historical growth rate. Note this is a conservative analysis since it is unlikely the 4.7% historical rate will continue on an annual basis for the next five years.

The results of the background conditions analysis are provided below in Table 9 and are included in Appendix E.

Table 9
2029 Background Conditions Operational Analysis

Intersection	Intersection Control	Approach	AM Peak Hour		PM Peak Hour	
			Delay (s)	LOS	Delay (s)	LOS
Hickpochee Ave at Main Street	Signal	Northbound Approach	43.1	D	52.0	D
		Southbound Approach	81.0	F	106.7	F
		SBL	49.5	D	51.5	D
		SBT/SBR	88.4	F	114.4	F
		Eastbound Approach	25.0	C	33.0	C
		EBL	18.9	B	20.2	C
		Westbound Approach	23.7	C	27.1	C
		Overall	35.5	D	43.3	D

As shown above, all study intersections will continue to operate overall at LOS D or better for both peak hours.

4.0 INTERSECTION ANALYSIS (CONTINUED)

Total Traffic Conditions

The project trips were added to the background conditions based on the trip distribution/assignment to determine the projected 2029 total traffic conditions. The results of the total traffic conditions analysis are provided below in Table 10 and included in Appendix F.

Table 10
2029 Total Traffic Conditions Operational Analysis

Intersection	Intersection Control	Approach	AM Peak Hour		PM Peak Hour	
			Delay (s)	LOS	Delay (s)	LOS
Hickpochee Ave at Main Street	Signal	Northbound Approach	46.7	D	58.5	E
		Southbound Approach	99.1	F	130.8	F
		SBL	51.5	D	55.6	E
		SBT/SBR	114.1	F	147.0	F
		Eastbound Approach	25.7	C	32.8	C
		EBL	20.3	C	21.3	C
		Westbound Approach	25.8	C	28.8	C
		Overall	41.0	D	49.2	D

As shown above, all study intersections will continue to operate overall at LOS D or better for both peak hours.

4.0 INTERSECTION ANALYSIS (CONTINUED)

However, due to the higher delays on the minor street, an optimized timings scenario was also included. The overall cycle length remained the same but the green phase times were optimized to better balance the anticipated traffic demand. The results of the optimized conditions scenario are shown in Table 11 below and included in Appendix G.

Table 11
2029 Optimized Conditions Operational Analysis

Intersection	Intersection Control	Approach	AM Peak Hour		PM Peak Hour	
			Delay (s)	LOS	Delay (s)	LOS
Hickpochee Ave at Main Street	Signal	Northbound Approach	39.8	D	45.9	D
		Southbound Approach	64.4	E	71.5	E
		SBL	47.8	D	48.3	D
		SBT/SBR	69.7	E	76.5	E
		Eastbound Approach	28.7	C	40.4	D
		EBL	22.7	C	25.4	C
		Westbound Approach	28.8	C	34.7	C
		Overall	36.3	D	43.8	D

5.0 DRIVEWAY VOLUMES

The AM and PM peak hour volumes at the project entrances for the overall development with no reduction for pass by credits are shown in Tables 2 and 3 and may be summarized as follows:

Directional Distribution (Trips IN/OUT)

AM = 142 / 140
PM = 135 / 133

The Turning Movement figure presents the AM and PM peak turning movement volume assignments at the project driveway based on the directional distributions. As previously mentioned, access is proposed via two (2) full access driveway connections to College Street, one (1) full access driveway connection to Main Street and one (1) right in/right out driveway connection to Main Street.

6.0 SITE OPERATIONS

The proposed site plan includes additional parking spaces above minimum code requirements to account for the Wawa business model. The site has been specifically designed to ensure sufficient traffic circulation, parking, and stacking. Wawa is sensitive to the specific issues, and the site plan is designed intentionally to ensure efficient operations.

The proposed pick up window (referred to as Wawa FlyThru) will reduce the amount of parking required. It is not a typical drive-thru with a menu board in which the visitor places an order at the drive-thru. Instead, all orders are placed online prior to arriving at the site and the Wawa FlyThru is only used for pick-up. This results in much lower queuing since the order is placed ahead of time. There is also no payment transaction at the FlyThru window as that is done ahead of time. At other Wawa's with a FlyThru, the average queue is only 2-3 vehicles at peak times with an average wait time of 50 seconds. If a customer places an order in-line and it is not ready, they will be directed to park and wait for the mobile verification that their order is ready. The site is designed to accommodate a 4-vehicle queue with a by-pass lane.

7.0 CONCLUSION

The attached tables document the daily, A.M. peak hour and P.M. peak hour traffic generation for the existing and proposed development. The proposed development will generate 1,001 daily trips, 78 A.M. peak hour trips, and 70 P.M. peak hour trips. The analysis provided within this report demonstrates the surrounding roadway network will operate at an acceptable Level of Service.

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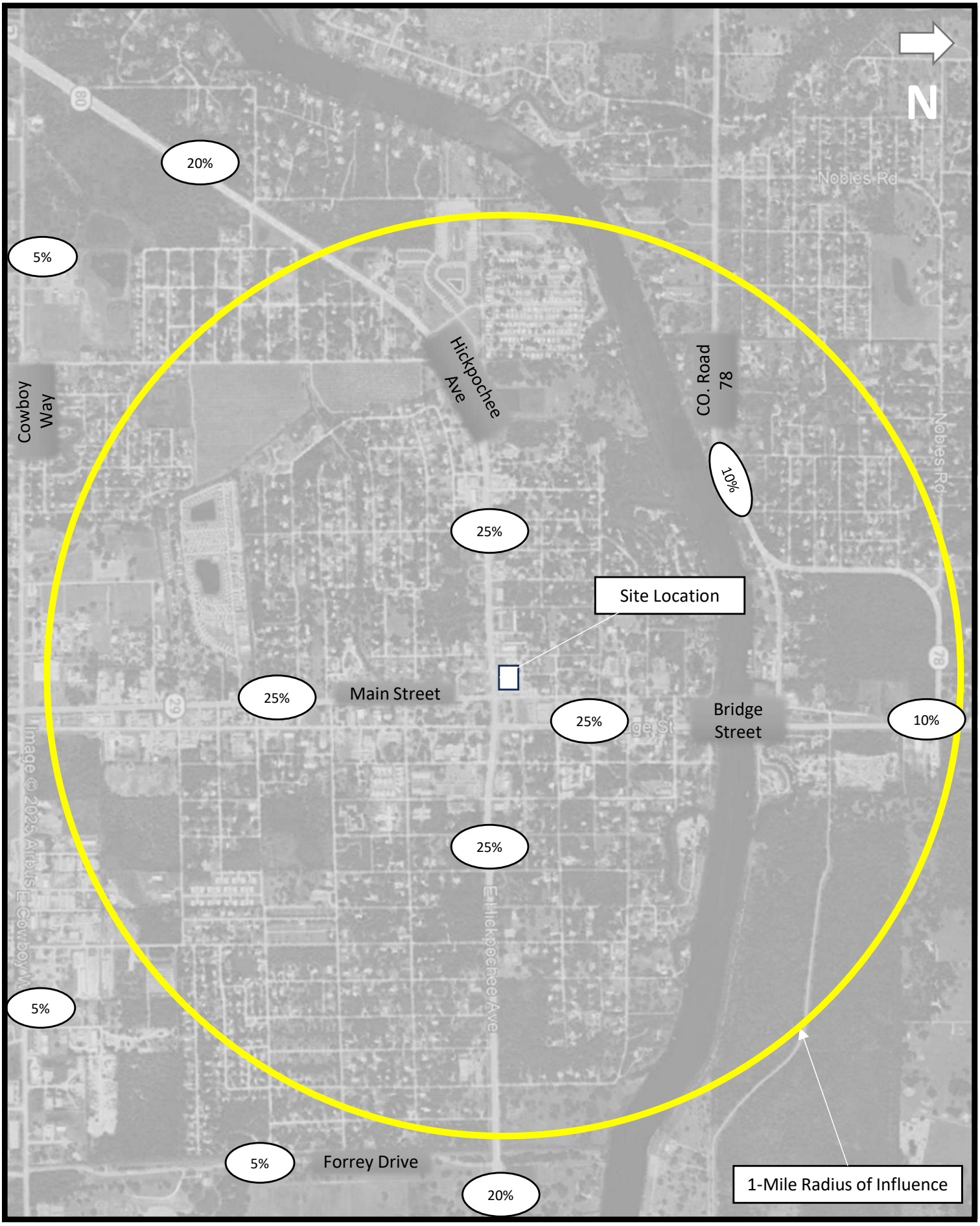

Figure 1 - Trip Distribution
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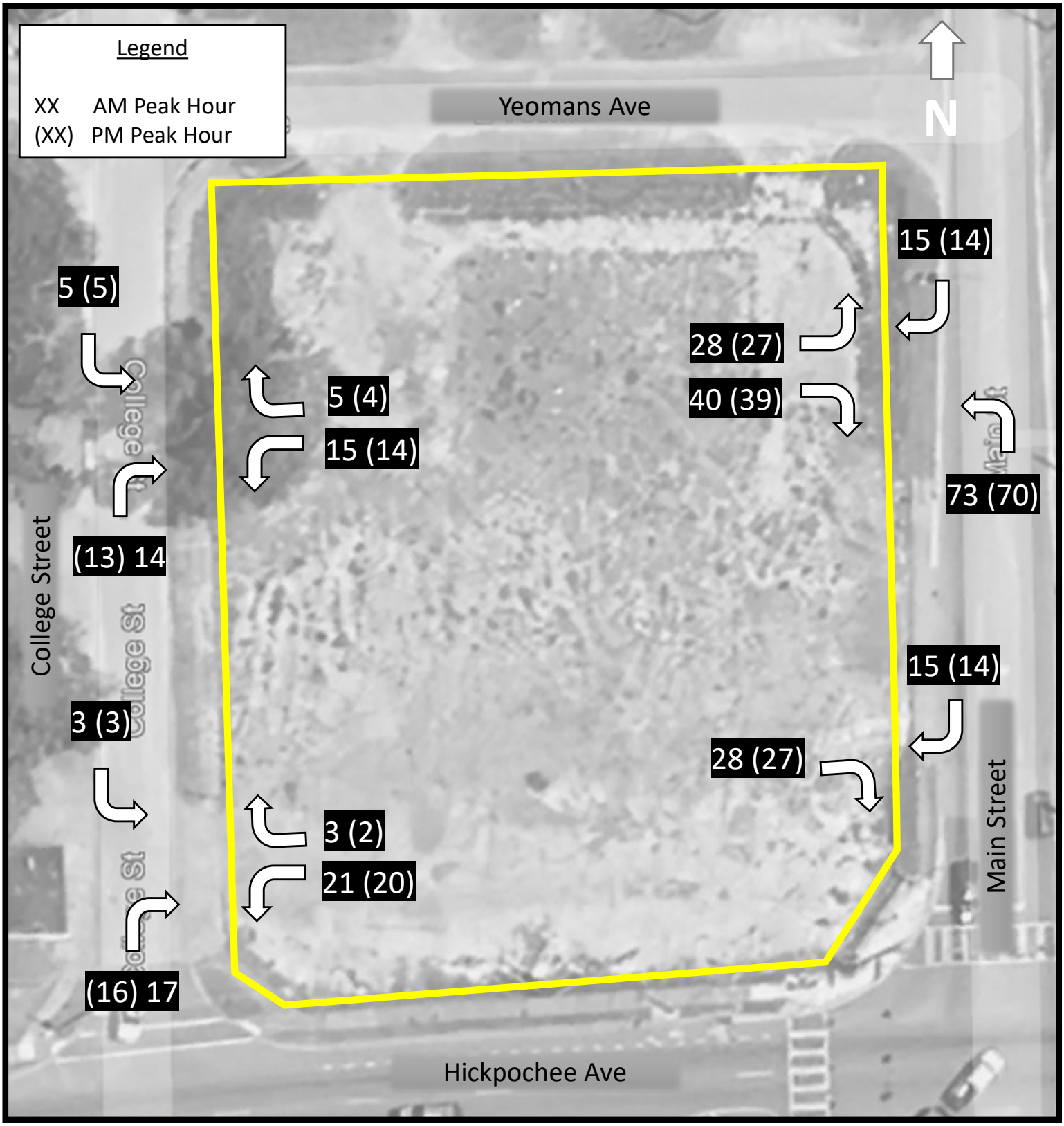

Figure 2 – Driveway Trips
Labelle Wawa
Project # 24-147

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3/11/2025
10/28/2025

PROPOSED DEVELOPMENT

TABLE 1 - Daily Traffic Generation

Landuse	ITE Code	Intensity		Rate/Equation	Dir Split In	Dir Split Out	Gross Trips	Internalization %	Internalization Total	External Trips (Driveway Trips)	Pass-by %	Pass-by Trips	Net Trips
Fast Food Rest. + DT	934	1,162	S.F.	467.48			543		0	543	49%	266	277
Gas Station w/ Convenience Store[e]	FDOT	12	Fuel Positions	14.3*PM Trips			3,289		0	3,289	78%	2,565	724
		5,338	S.F.										
			Grand Totals:				**3,832**	**0.0%**	**0**	**3,832**	**74%**	**2,831**	**1,001**

TABLE 2 - AM Peak Hour Traffic Generation

Landuse	ITE Code	Intensity		Rate/Equation	Dir Split In	Dir Split Out	Gross Trips In	Gross Trips Out	Gross Trips Total	Internalization %	Internalization In	Internalization Out	Internalization Total	External Trips (Driveway Trips) In	External Trips (Driveway Trips) Out	External Trips (Driveway Trips) Total	Pass-by %	Pass-by Trips	Net Trips In	Net Trips Out	Net Trips Total
Fast Food Rest. + DT	934	1,162	S.F.	44.61	0.51	0.49	27	25	52	0.0%	0	0	0	27	25	52	49%	25	14	13	27
Gas Station w/ Convenience Store[e]	FDOT	12	Fuel Positions	12.3*FP+15.5*(X)	0.50	0.50	115	115	230	0.0%	0	0	0	115	115	230	78%	179	25	26	51
		5,338	S.F.																		
			Grand Totals:				**142**	**140**	**282**	**0.0%**	**0**	**0**	**0**	**142**	**140**	**282**	**72%**	**204**	**39**	**39**	**78**

TABLE 3 - PM Peak Hour Traffic Generation

Landuse	ITE Code	Intensity		Rate/Equation	Dir Split In	Dir Split Out	Gross Trips In	Gross Trips Out	Gross Trips Total	Internalization %	Internalization In	Internalization Out	Internalization Total	External Trips (Driveway Trips) In	External Trips (Driveway Trips) Out	External Trips (Driveway Trips) Total	Pass-by %	Pass-by Trips	Net Trips In	Net Trips Out	Net Trips Total
Fast Food Rest. + DT	934	1,162	S.F.	33.03	0.52	0.48	20	18	38	0.0%	0	0	0	20	18	38	49%	19	10	9	19
Gas Station w/ Convenience Store[e]	FDOT	12	Fuel Positions	12.3*FP+15.5*(X)	0.50	0.50	115	115	230	0.0%	0	0	0	115	115	230	78%	179	25	26	51
		5,338	S.F.																		
			Grand Totals:				**135**	**133**	**268**	**0.0%**	**0**	**0**	**0**	**135**	**133**	**268**	**74%**	**198**	**35**	**35**	**70**

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TABLE 4
AREA WIDE GROWTH RATE CALCULATION

STATION	ROADWAY	FROM	TO	2019 PEAK SEASON DAILY TRAFFIC	2023 PEAK SEASON DAILY TRAFFIC	IND. (%)
	HICKPOCHEE AVENUE	COWBOY WAY	DR. MLK BLVD	14,400	17,700	5.29%
	HICKPOCHEE AVENUE	DR. MILK BLVD	MAIN STREET/SITE	18,700	25,000	7.53%
	HICKPOCHEE AVENUE	MAIN STREET/SITE	BRIDGE STREET	22,500	25,000	2.67%
	HICKPOCHEE AVENUE	BRIDGE STREET	FORREY DRIVE	13,100	16,800	6.42%
	COUNTY ROAD 78	CRECENT AVENUE	BRIDGE STREET	2,000	2,600	6.78%
	COWBOY WAY	HICKPOCHEE AVENUE	MAIN STREET	7,300	6,700	-2.12%
	COWBOY WAY	MAIN STREET	FORREY DRIVE	8,900	10,400	3.97%
	MAIN STREET	OKLAHOMA AVENUE	HICKPOCHEE AVENUE/SITE	3,000	3,400	3.18%
	MAIN STREET	HICKPOCHEE AVENUE/SITE	PALMDALE AVENUE	7,100	8,600	4.91%
	MAIN STREET	PALMDALE AVENUE	COWBOY WAY	7,400	9,400	6.16%
	BRIDGE STREET	COUNTY ROAD 78	HICKPOCHEE AVENUE	15,900	18,900	4.42%
	FORREY DRIVE	HICKPOCHEE AVENUE	COWBOY WAY	NA	NA	NA
			∑ =	**120,300**	**144,500**	**4.69%**

AREA WIDE GROWTH RATE = 4.7%

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TABLE 5

TEST 1 - PROJECT SIGNIFICANCE CALCULATION
AM PEAK HOUR

2029 BUILD OUT
1 MILE RADIUS OF DEVELOPMENT INFLUENCE
TOTAL AM PEAK HOUR PROJECT TRIPS (ENTERING) = 39
TOTAL AM PEAK HOUR PROJECT TRIPS (EXITING) = 39

STATION	ROADWAY	FROM	TO	DIRECTION	PROJECT DISTRIBUTION	AM PEAK HOUR DIRECTIONAL PROJECT TRIPS	EXISTING LANES	CLASS	LOS D STANDARD	TOTAL PROJECT IMPACT	PROJECT SIGNIFICANT
	HICKPOCHEE AVENUE	COWBOY WAY	DR. MLK BLVD	EB	20%	8	4L	C2	2910	0.27%	NO
				WB	20%	8	4L	C2	2910	0.27%	NO
	HICKPOCHEE AVENUE	DR. MLK BLVD	MAIN STREET/SITE	EB	25%	10	4L	C2T	1640	0.61%	NO
				WB	25%	10	4L	C2T	1640	0.61%	NO
	HICKPOCHEE AVENUE	MAIN STREET/SITE	BRIDGE STREET	EB	50%	20	4L	C2T	1640	1.22%	YES
				WB	50%	20	4L	C2T	1640	1.22%	YES
	HICKPOCHEE AVENUE	BRIDGE STREET	FORREY DRIVE	EB	25%	10	4L	C2T	1640	0.61%	NO
				WB	25%	10	4L	C2T	1640	0.61%	NO
	COUNTY ROAD 78	CRECENT AVENUE	BRIDGE STREET	EB	10%	4	2L	C2	730	0.55%	NO
				WB	10%	4	2L	C2	730	0.55%	NO
	COWBOY WAY	HICKPOCHEE AVENUE	MAIN STREET	EB	5%	2	2L	C2	730	0.27%	NO
				WB	5%	2	2L	C2	730	0.27%	NO
	COWBOY WAY	MAIN STREET	FORREY DRIVE	EB	5%	2	2L	C2	730	0.27%	NO
				WB	5%	2	2L	C2	730	0.27%	NO
	MAIN STREET	OKLAHOME AVENUE	HICKPOCHEE AVENUE/SITE	NB	5%	2	2L	C2	730	0.27%	NO
				SB	5%	2	2L	C2	730	0.27%	NO
	MAIN STREET	HICKPOCHEE AVENUE/SITE	PALMDALE AVENUE	NB	25%	10	2L	C2T	940	1.06%	YES
				SB	25%	10	2L	C2T	940	1.06%	YES
	MAIN STREET	PALMDALE AVENUE	COWBOY WAY	NB	20%	8	2L	C2	730	1.10%	YES
				SB	20%	8	2L	C2	730	1.10%	YES
	BRIDGE STREET	COUNTY ROAD 78	HICKPOCHEE AVENUE	NB	25%	10	2L	C3C	1070	0.93%	NO
				SB	25%	10	2L	C3C	1070	0.93%	NO
	FORREY DRIVE	HICKPOCHEE AVENUE	COWBOY WAY	NB	5%	2	2L	C2	730	0.27%	NO
				SB	5%	2	2L	C2	730	0.27%	NO

TABLE 6

TEST 1 - PROJECT SIGNIFICANCE CALCULATION
PM PEAK HOUR

2029 BUILD OUT
1 MILE RADIUS OF DEVELOPMENT INFLUENCE
TOTAL PM PEAK HOUR PROJECT TRIPS (ENTERING) = 35
TOTAL PM PEAK HOUR PROJECT TRIPS (EXITING) = 35

STATION	ROADWAY	FROM	TO	DIRECTION	PROJECT DISTRIBUTION	PM PEAK HOUR DIRECTIONAL PROJECT TRIPS	EXISTING LANES	CLASS	LOS D STANDARD	TOTAL PROJECT IMPACT	PROJECT SIGNIFICANT
	HICKPOCHEE AVENUE	COWBOY WAY	DR. MLK BLVD	EB	20%	7	4L	C2	2910	0.24%	NO
				WB	20%	7	4L	C2	2910	0.24%	NO
	HICKPOCHEE AVENUE	DR. MLK BLVD	MAIN STREET/SITE	EB	25%	9	4L	C2T	1640	0.55%	NO
				WB	25%	9	4L	C2T	1640	0.55%	NO
	HICKPOCHEE AVENUE	MAIN STREET/SITE	BRIDGE STREET	EB	50%	18	4L	C2T	1640	1.10%	YES
				WB	50%	18	4L	C2T	1640	1.10%	YES
	HICKPOCHEE AVENUE	BRIDGE STREET	FORREY DRIVE	EB	25%	9	4L	C2T	1640	0.55%	NO
				WB	25%	9	4L	C2T	1640	0.55%	NO
	COUNTY ROAD 78	CRECENT AVENUE	BRIDGE STREET	EB	10%	4	2L	C2	730	0.55%	NO
				WB	10%	4	2L	C2	730	0.55%	NO
	COWBOY WAY	HICKPOCHEE AVENUE	MAIN STREET	EB	5%	2	2L	C2	730	0.27%	NO
				WB	5%	2	2L	C2	730	0.27%	NO
	COWBOY WAY	MAIN STREET	FORREY DRIVE	EB	5%	2	2L	C2	730	0.27%	NO
				WB	5%	2	2L	C2	730	0.27%	NO
	MAIN STREET	OKLAHOME AVENUE	HICKPOCHEE AVENUE/SITE	NB	5%	2	2L	C2	730	0.27%	NO
				SB	5%	2	2L	C2	730	0.27%	NO
	MAIN STREET	HICKPOCHEE AVENUE/SITE	PALMDALE AVENUE	NB	25%	9	2L	C2T	940	0.96%	NO
				SB	25%	9	2L	C2T	940	0.96%	NO
	MAIN STREET	PALMDALE AVENUE	COWBOY WAY	NB	20%	7	2L	C2	730	0.96%	NO
				SB	20%	7	2L	C2	730	0.96%	NO
	BRIDGE STREET	COUNTY ROAD 78	HICKPOCHEE AVENUE	NB	25%	9	2L	C3C	1070	0.84%	NO
				SB	25%	9	2L	C3C	1070	0.84%	NO
	FORREY DRIVE	HICKPOCHEE AVENUE	COWBOY WAY	NB	5%	2	2L	C2	730	0.27%	NO
				SB	5%	2	2L	C2	730	0.27%	NO

TABLE 7
AM PEAK HOUR - TEST 1

2029 BUILD OUT
1 MILE RADIUS OF DEVELOPMENT INFLUENCE
AREA WIDE GROWTH RATE = 4.70%
TOTAL AM PEAK HOUR PROJECT TRIPS (ENTERING) = 39
TOTAL AM PEAK HOUR PROJECT TRIPS (EXITING) = 39

ROADWAY	FROM	TO	DIRECTION	TRAFFIC COUNT YEAR	AM PEAK HOUR TRAFFIC	PROJECT DISTRIBUTION	AM PEAK HOUR PROJECT TRIPS	LINK GROWTH	1.0% GROWTH	TOTAL BACKGROUND TRAFFIC USED	2029 TRAFFIC WITHOUT PROJECT	2029 TOTAL TRAFFIC	ASSURED LANES	CLASS	LOS D	2029 WITHOUT PROJECT MEETS LOS STD.	MEETS LOS STD.
HICKPOCHEE AVENUE	MAIN STREET/SITE	BRIDGE STREET	EB	2024	624	50%	20	161	32	161	785	805	4L	C2T	1640	**YES**	**YES**
			WB	2024	802	50%	20	207	41	207	1009	1029	4L	C2T	1640	**YES**	**YES**
MAIN STREET	HICKPOCHEE AVENUE/SITE	PALMDALE AVENUE	NB	2023	261	25%	10	83	16	83	344	354	2L	C2T	940	**YES**	**YES**
			SB	2023	337	25%	10	107	21	107	444	454	2L	C2T	940	**YES**	**YES**
MAIN STREET	PALMDALE AVENUE	COWBOY WAY	NB	2023	249	20%	8	79	15	79	328	336	2L	C2	730	**YES**	**YES**
			SB	2023	308	20%	8	98	19	98	406	414	2L	C2	730	**YES**	**YES**

Notes:

TABLE 8
PM PEAK HOUR - TEST 1

2029 BUILD OUT
1 MILE RADIUS OF DEVELOPMENT INFLUENCE
AREA WIDE GROWTH RATE = 4.70%
TOTAL PM PEAK HOUR PROJECT TRIPS (ENTERING) = 35
TOTAL PM PEAK HOUR PROJECT TRIPS (EXITING) = 35

ROADWAY	FROM	TO	DIRECTION	TRAFFIC COUNT YEAR	PM PEAK HOUR TRAFFIC	PROJECT DISTRIBUTION	PM PEAK HOUR PROJECT TRIPS	LINK GROWTH	1.0% GROWTH	TOTAL BACKGROUND TRAFFIC USED	2029 TRAFFIC WITHOUT PROJECT	2029 TOTAL TRAFFIC	ASSURED LANES	CLASS	LOS D	2029 WITHOUT PROJECT MEETS LOS STD.	MEETS LOS STD.
HICKPOCHEE AVENUE	MAIN STREET/SITE	BRIDGE STREET	EB	2024	1053	50%	18	272	54	272	1325	1343	4L	C2T	1640	**YES**	**YES**
			WB	2024	782	50%	18	202	40	202	984	1002	4L	C2T	1640	**YES**	**YES**

Notes:

INTERSECTION VOLUME DEVLOPMENT

LABELLE WAWA

HICKPOCHEE AVENUE AND MAIN STREET

03/11/25
10/28/25

TRIPS

	IN	OUT
AM	142	140
PM	135	133

INPUT DATA

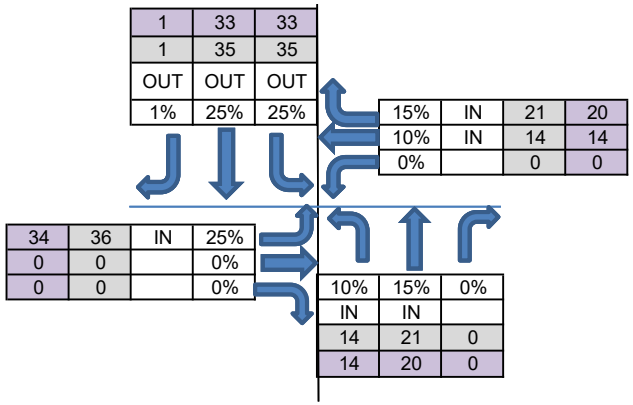
Comments:

Growth Rate = 4.70% Peak Season = 1.00 Current Year = 2024 Buildout Year = 2029

AM Peak Hour

INTERSECTION VOLUME DEVELOPMENT

	Northbound			Southbound			Eastbound			Westbound				
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
Existing Volume (2024)	112	105	61	56	157	88	32	507	56	110	644	48	1976	Existing Total
Peak Season Adjustment	0	0	0	0	0	0	0	0	0	0	0	0		
Background Traffic Growth	29	27	16	14	41	23	8	131	14	28	166	12	510	BG Total
1.0% Background Growth	6	5	3	3	8	4	2	26	3	6	33	2	101	1% + MP Total
Major Projects Traffic	0	0	0	0	0	0	0	0	0	0	0	0		
1% BGR + Major Projects	6	5	3	3	8	4	2	26	3	6	33	2	101	1% + MP Total
Background Traffic w/o Project	141	132	77	70	198	111	40	638	70	138	810	60		
Project Traffic	14	21	0	35	35	1	36	0	0	0	14	21		
Total	**155**	**153**	**77**	**105**	**233**	**112**	**76**	**638**	**70**	**138**	**824**	**81**		
Approach Total		**385**			**450**			**785**			**1,044**			



PM Peak Hour

INTERSECTION VOLUME DEVELOPMENT

	Northbound			Southbound			Eastbound			Westbound				
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right		
Existing Volume (2024)	142	101	154	37	188	87	67	862	60	84	656	42	2480	Existing Total
Peak Season Adjustment	0	0	0	0	0	0	0	0	0	0	0	0		
Background Traffic Growth	37	26	40	10	49	22	17	223	15	22	169	11	640	BG Total
1.0% Background Growth	7	5	8	2	10	4	3	44	3	4	33	2	127	1% + MP Total
Major Projects Traffic	0	0	0	0	0	0	0	0	0	0	0	0		
1% BGR + Major Projects	7	5	8	2	10	4	3	44	3	4	33	2	127	1% + MP Total
Background Traffic w/o Project	179	127	194	47	237	109	84	1085	75	106	825	53		
Project Traffic	14	20	0	33	33	1	34	0	0	0	14	20		
Total	**193**	**147**	**194**	**80**	**270**	**110**	**118**	**1085**	**75**	**106**	**839**	**73**		
Approach Total		**533**			**460**			**1,278**			**1,018**			

APPENDIX A

FDOT TRAFFIC COUNTS

FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2023 HISTORICAL AADT REPORT

COUNTY: 07 - HENDRY

SITE: 0023 - SR 29, N OF CR 80A/COWBOY WAY LABELLE

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2023	9400 S	N 4400	S 5000	9.00	55.20	9.20
2022	8600 F	N 4000	S 4600	9.00	57.50	9.20
2021	8400 C	N 3900	S 4500	9.00	58.30	9.20
2020	7200 C	N 3300	S 3900	9.00	56.80	19.80
2019	7400 C	N 3600	S 3800	9.00	57.40	18.80
2018	7200 F	N 3900	S 3300	9.00	58.50	13.80
2017	6900 C	N 3700	S 3200	9.00	58.20	6.10
2016	9900 F	N 5000	S 4900	9.00	57.30	6.10
2015	9300 C	N 4700	S 4600	9.00	58.40	6.10
2014	8400 F	N 4000	S 4400	9.00	58.90	6.10
2013	8400 C	N 4000	S 4400	9.00	58.60	6.10
2012	8500 C	N 3900	S 4600	9.00	58.80	5.60
2011	8200 F	N 3900	S 4300	9.00	61.10	5.80
2010	8400 C	N 4000	S 4400	9.69	59.27	5.80
2009	8600 C	N 4000	S 4600	9.99	59.95	4.60
2008	8300 C	N 3900	S 4400	10.28	60.86	6.10

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
*K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES

FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2023 HISTORICAL AADT REPORT

COUNTY: 07 - HENDRY

SITE: 0006 - SR 29, N OF CALOOSAHATCHEE RIVER & S OF CR 78

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2023	18900 F	N 9300	S 9600	9.00	55.20	17.40
2022	17300 C	N 8500	S 8800	9.00	57.50	17.40
2021	16600 C	N 8300	S 8300	9.00	58.30	17.00
2020	14500 C	N 7200	S 7300	9.00	56.80	15.90
2019	15900 C	N 7900	S 8000	9.00	57.40	15.00
2018	15300 C	N 7600	S 7700	9.00	58.50	15.00
2017	14500 C	N 7200	S 7300	9.00	58.20	11.80
2016	11800 C	N 7000	S 4800	9.00	57.30	14.90
2015	13900 C	N 6800	S 7100	9.00	58.40	11.60
2014	13900 C	N 6800	S 7100	9.00	58.90	10.60
2013	11800 F	N 5800	S 6000	9.00	58.60	12.50
2012	12000 C	N 5900	S 6100	9.00	58.80	12.50
2011	13000 S	N 6500	S 6500	9.00	61.10	11.90
2010	13200 F	N 6600	S 6600	9.69	59.27	11.90
2009	13800 C	N 6900	S 6900	9.99	59.95	11.90
2008	14500 C	N 7200	S 7300	10.28	60.86	13.40

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
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V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
*K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES

FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2023 HISTORICAL AADT REPORT

COUNTY: 07 - HENDRY

SITE: 4139 - NORTH RIVER ROAD/CR 78, EAST OF FORT DENAUD BRIDGE WAY

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2023	2600 C	E 1300	W 1300	9.50	55.70	15.90
2022	2300 C	E 1200	W 1100	9.50	57.50	16.30
2021	2000 T	E 1000	W 1000	9.50	55.80	12.60
2020	2000 S	E 1000	W 1000	9.50	55.80	12.10
2019	2000 F	E 1000	W 1000	9.50	56.80	12.10
2018	2000 C	E 1000	W 1000	9.50	57.40	12.10
2017	1850 T	E 950	W 900	9.50	56.80	9.70
2016	1750 S	E 900	W 850	9.50	56.30	7.90
2015	1650 F	E 850	W 800	9.50	56.90	7.90
2014	1550 C	E 800	W 750	9.50	56.70	7.90
2013	1300 S	0	0	9.50	56.30	11.60
2012	1300 F	0	0	9.50	56.50	11.50
2011	1300 C	E 0	W 0	9.50	56.50	15.00

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
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FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2023 HISTORICAL AADT REPORT

COUNTY: 07 - HENDRY

SITE: 4125 - COWBOY WAY, EAST OF S.R. 80

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2023	6700 C	E 3300	W 3400	9.00	55.70	9.50
2022	7100 C	E 3400	W 3700	9.00	57.50	12.80
2021	7600 T	E 3700	W 3900	9.00	55.80	12.60
2020	7300 S	E 3600	W 3700	9.00	55.80	9.90
2019	7300 F	E 3600	W 3700	9.00	56.80	9.90
2018	7100 C	E 3500	W 3600	9.00	57.40	9.90
2017	6500 T	E 3200	W 3300	9.00	56.80	9.70
2016	6100 S	E 3000	W 3100	9.00	56.30	11.50
2015	5700 F	E 2800	W 2900	9.00	56.90	11.50
2014	5500 C	E 2700	W 2800	9.00	56.70	11.50
2013	4400 S	0	0	9.00	56.30	11.60
2012	4400 F	0	0	9.00	56.50	11.50
2011	4500 C	E 0	W 0	9.00	56.50	15.00

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
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FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2023 HISTORICAL AADT REPORT

COUNTY: 07 - HENDRY

SITE: 4032 - E COWBOY WAY, 0.5 MI E OF SR 29 HC 32

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2023	10400 C	E 5100	W 5300	9.00	55.70	11.60
2022	9500 E			9.00	57.50	18.40
2021	9300 S	E 4600	W 4700	9.00	55.80	10.70
2020	8900 F	E 4400	W 4500	9.00	55.80	10.70
2019	8900 C	E 4400	W 4500	9.00	56.80	10.70
2018	10500 X	0	0	9.00	57.40	13.80
2017	9900 T			9.00	56.80	10.80
2016	9300 S	E 4600	W 4700	9.00	56.30	9.80
2015	8700 F	E 4300	W 4400	9.00	56.90	9.80
2014	8300 C	E 4100	W 4200	9.00	56.70	9.80
2013	7600 S	E 3800	W 3800	9.00	56.30	7.60
2012	7600 F	E 3800	W 3800	9.00	56.50	7.60
2011	7800 C	E 3900	W 3900	9.00	56.50	7.60
2010	6700 S	E 3300	W 3400	9.81	57.36	8.20
2009	6900 F	E 3400	W 3500	10.07	57.54	8.20
2008	7100 C	E 3500	W 3600	10.16	56.49	8.20

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
*K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES

FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2023 HISTORICAL AADT REPORT

COUNTY: 07 - HENDRY

SITE: 5003 - SR 80, EAST OF SR 29/BRIDGE STREET LABELLE

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2023	16800 S	E 8300	W 8500	9.00	56.40	9.40
2022	15400 F	E 7600	W 7800	9.00	57.50	9.40
2021	14600 C	E 7200	W 7400	9.00	55.80	9.40
2020	11500 C	E 5700	W 5800	9.00	55.80	10.30
2019	13100 C	E 6600	W 6500	9.00	56.50	11.10
2018	11900 C	E 5900	W 6000	9.00	56.20	9.50
2017	12000 C	E 6000	W 6000	9.00	55.40	9.20
2016	11400 C	E 5400	W 6000	9.00	55.40	12.00
2015	11000 C	E 5500	W 5500	9.00	55.70	9.50
2014	11200 F	E 5700	W 5500	9.00	54.80	8.80
2013	11000 C	E 5600	W 5400	9.00	54.50	8.80
2012	10300 C	E 5100	W 5200	9.00	54.70	9.40
2011	12500 F	E 6400	W 6100	9.00	51.90	7.90
2010	12700 C	E 6500	W 6200	9.91	56.65	7.90
2009	13300 C	E 6600	W 6700	10.14	55.96	8.80
2008	14300 C	E 7200	W 7100	10.10	54.31	15.70

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
*K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES

FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2023 HISTORICAL AADT REPORT

COUNTY: 07 - HENDRY

SITE: 5000 - SR 80, NE OF COWBOY WAY

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2023	17700 S	E 8900	W 8800	9.00	56.40	16.20
2022	16300 F	E 8200	W 8100	9.00	57.50	16.20
2021	15500 C	E 7800	W 7700	9.00	55.80	16.20
2020	13700 C	E 6800	W 6900	9.00	55.80	14.60
2019	14400 C	E 7100	W 7300	9.00	56.50	15.50
2018	13400 C	E 6600	W 6800	9.00	56.20	11.60
2017	13100 C	E 6600	W 6500	9.00	55.40	10.80
2016	13600 C	E 6600	W 7000	9.00	55.40	11.90
2015	11100 C	E 5500	W 5600	9.00	55.70	13.30
2014	10500 F	E 5200	W 5300	9.00	54.80	11.70
2013	10300 C	E 5100	W 5200	9.00	54.50	11.70
2012	9700 C	E 4800	W 4900	9.00	54.70	11.40
2011	11800 F	E 6000	W 5800	9.00	51.90	7.90
2010	12000 C	E 6100	W 5900	9.91	56.65	7.90
2009	11100 C	E 5600	W 5500	10.14	55.96	12.60
2008	10300 C	E 5100	W 5200	10.10	54.31	13.10

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
*K FACTOR: STARTING WITH YEAR 2011 IS STANDARDK, PRIOR YEARS ARE K30 VALUES

FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2023 HISTORICAL AADT REPORT

COUNTY: 07 - HENDRY

SITE: 5002 - SR 80, WEST OF SR 29/MAIN STREET, LABELLE

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2023	25000 F	E 12500	W 12500	9.00	56.40	10.90
2022	23000 C	E 11500	W 11500	9.00	57.50	10.90
2021	20000 C	E 10000	W 10000	9.00	55.80	10.60
2020	16600 C	E 8300	W 8300	9.00	55.80	10.80
2019	18700 C	E 9300	W 9400	9.00	56.50	11.70
2018	17200 C	E 8600	W 8600	9.00	56.20	9.40
2017	16800 C	E 8300	W 8500	9.00	55.40	9.50
2016	18200 C	E 9000	W 9200	9.00	55.40	10.70
2015	15000 C	E 7600	W 7400	9.00	55.70	10.00
2014	15900 F	E 8000	W 7900	9.00	54.80	9.50
2013	15700 C	E 7900	W 7800	9.00	54.50	9.50
2012	15200 C	E 7800	W 7400	9.00	54.70	8.60
2011	14900 F	E 7400	W 7500	9.00	51.90	7.70
2010	15100 C	E 7500	W 7600	9.91	56.65	7.70
2009	16300 C	E 8100	W 8200	10.14	55.96	9.60
2008	16700 C	E 8400	W 8300	10.10	54.31	10.30

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
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FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2023 HISTORICAL AADT REPORT

COUNTY: 07 - HENDRY

SITE: 0011 - SR 80, WEST OF SR 29/BRIDGE STREET, LABELLE

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2023	25000 S	E 12500	W 12500	9.00	56.40	8.10
2022	23000 F	E 11500	W 11500	9.00	57.50	8.10
2021	22000 C	E 11000	W 11000	9.00	55.80	8.10
2020	21000 C	E 10500	W 10500	9.00	55.80	14.00
2019	22500 C	E 11500	W 11000	9.00	56.50	13.90
2018	21500 C	E 10500	W 11000	9.00	56.20	13.80
2017	19300 F	E 10000	W 9300	9.00	55.40	13.80
2016	18000 C	E 9300	W 8700	9.00	55.40	13.80
2015	13100 C	E 6600	W 6500	9.00	55.70	8.20
2014	15200 F	E 7500	W 7700	9.00	54.80	8.20
2013	15000 C	E 7400	W 7600	9.00	54.50	8.20
2012	14300 C	E 7100	W 7200	9.00	54.70	8.60
2011	12700 F	E 6400	W 6300	9.00	51.90	6.60
2010	12900 C	E 6500	W 6400	9.91	56.65	6.60
2009	17600 C	E 9600	W 8000	10.14	55.96	8.60
2008	16700 C	E 9000	W 7700	10.10	54.31	9.00

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
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FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2023 HISTORICAL AADT REPORT

COUNTY: 07 - HENDRY

SITE: 5005 - SR 29/MAIN STREET, SOUTH OF SR 80

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2023	8600 F	N 4300	S 4300	9.00	55.20	10.80
2022	7800 C	N 3900	S 3900	9.00	57.50	10.80
2021	7700 C	N 3800	S 3900	9.00	58.30	8.90
2020	6200 C	N 3100	S 3100	9.00	56.80	19.80
2019	7100 C	N 3600	S 3500	9.00	57.40	18.80
2018	7100 F	N 3400	S 3700	9.00	58.50	13.80
2017	6800 C	N 3300	S 3500	9.00	58.20	7.60
2016	8600 F	N 4300	S 4300	9.00	57.30	7.60
2015	8200 C	N 4100	S 4100	9.00	58.40	7.60
2014	7300 F	N 3600	S 3700	9.00	58.90	6.20
2013	7300 C	N 3600	S 3700	9.00	58.60	6.20
2012	7800 C	N 3800	S 4000	9.00	58.80	6.10
2011	6700 F	N 3300	S 3400	9.00	61.10	5.80
2010	6700 C	N 3300	S 3400	9.69	59.27	5.80
2009	8300 C	N 4200	S 4100	9.99	59.95	5.00
2008	8600 C	N 4300	S 4300	10.28	60.86	5.80

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
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FLORIDA DEPARTMENT OF TRANSPORTATION
TRANSPORTATION STATISTICS OFFICE
2023 HISTORICAL AADT REPORT

COUNTY: 07 - HENDRY

SITE: 4100 - MAIN ST, NORTH OF SR 80

YEAR	AADT	DIRECTION 1	DIRECTION 2	*K FACTOR	D FACTOR	T FACTOR
2023	3400 C	N 1600	S 1800	9.00	55.70	16.30
2022	3300 E			9.00	57.50	15.60
2021	3200 S	N 1500	S 1700	9.00	55.80	12.60
2020	3000 F	N 1400	S 1600	9.00	55.80	14.00
2019	3000 C	N 1400	S 1600	9.00	56.80	13.90
2018	4000 X	0	0	9.00	57.40	10.50
2017	3800 T			9.00	56.80	9.70
2016	3600 S	N 1700	S 1900	9.00	56.30	4.50
2015	3400 F	N 1600	S 1800	9.00	56.90	4.50
2014	3200 C	N 1500	S 1700	9.00	56.70	4.50
2013	2800 S	N 1300	S 1500	9.00	56.30	2.90
2012	2800 F	N 1300	S 1500	9.00	56.50	2.90
2011	2800 C	N 1300	S 1500	9.00	56.50	2.90

AADT FLAGS: C = COMPUTED; E = MANUAL ESTIMATE; F = FIRST YEAR ESTIMATE
S = SECOND YEAR ESTIMATE; T = THIRD YEAR ESTIMATE; R = FOURTH YEAR ESTIMATE
V = FIFTH YEAR ESTIMATE; 6 = SIXTH YEAR ESTIMATE; X = UNKNOWN
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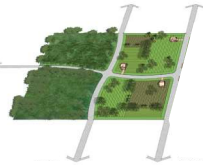
APPENDIX B

FDOT 2023 MULTIMODAL Q/LOS TABLES

C1 & C2

Motor Vehicle Highway Generalized Service Volume Tables



(C1-Natural & C2-Rural)

Peak Hour Directional

	B	C	D	E
1 Lane	240	430	730	1,490
2 Lane	1,670	2,390	2,910	3,340
3 Lane	2,510	3,570	4,370	5,010

Peak Hour Two-Way

	B	C	D	E
2 Lane	440	780	1,330	2,710
4 Lane	3,040	4,350	5,290	6,070
6 Lane	4,560	6,490	7,950	9,110

AADT

	B	C	D	E
2 Lane	4,600	8,200	14,000	28,500
4 Lane	32,000	45,800	55,700	63,900
6 Lane	48,000	68,300	83,700	95,900

Adjustment Factors

2 Lane Divided Roadway with Exclusive Left Turn Adjustment: Multiply by 1.05
Multilane Undivided Highway with Exclusive Left Turn Adjustment: Multiply by 0.95
Multilane Undivided Highway without Exclusive Left Turn Adjustment:: Multiply by 0.75

This table does not constitute a standard and should be used only for general planning applications. The table should not be used for corridor or intersection design, where more refined techniques exist.

C3C & C3R

Motor Vehicle Arterial Generalized Service Volume Tables

(C3C-Suburban Commercial)

Peak Hour Directional

	B	C	D	E
1 Lane	*	760	1,070	**
2 Lane	*	1,520	1,810	**
3 Lane	*	2,360	2,680	**
4 Lane	*	3,170	3,180	**

Peak Hour Two-Way

	B	C	D	E
2 Lane	*	1,380	1,950	**
4 Lane	*	2,760	3,290	**
6 Lane	*	4,290	4,870	**
8 Lane	*	5,760	5,780	**

AADT

	B	C	D	E
2 Lane	*	15,300	21,700	**
4 Lane	*	30,700	36,600	**
6 Lane	*	47,700	54,100	**
8 Lane	*	64,000	64,200	**

(C3R-Suburban Residential)

	B	C	D	E
1 Lane	*	970	1,110	**
2 Lane	*	1,700	1,850	**
3 Lane	*	2,620	2,730	**

	B	C	D	E
2 Lane	*	1,760	2,020	**
4 Lane	*	3,090	3,360	**
6 Lane	*	4,760	4,960	**

	B	C	D	E
2 Lane	*	19,600	22,400	**
4 Lane	*	34,300	37,300	**
6 Lane	*	52,900	55,100	**

Adjustment Factors

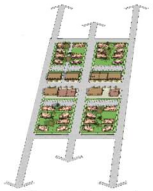
The peak hour directional service volumes should be adjust by multiplying by 1.2 for one-way facilities
The AADT service volumes should be adjusted by multiplying 0.6 for one way facilities 2 Lane Divided Roadway with an Exclusive Left Turn Lane(s): Multiply by 1.05
2 lane Undivided Roadway with No Exclusive Left Turn Lane(s): Multiply by 0.80

Exclusive right turn lane(s): Multiply by 1.05
Multilane Undivided Roadway with an Exclusive Left Turn Lane(s): Multiply by 0.95
Multilane Roadway with No Exclusive Left Turn Lane(s): Multiply by 0.75
Non-State Signalized Roadway: Multiply by 0.90

This table does not constitute a standard and should be used only for general planning applications. The table should not be used for corridor or intersection design, where more refined techniques exist.
* Cannot be achieved using table input value defaults.
** Not applicable for that level of service letter grade. For the automobile mode, volumes greater than level of service D become F because intersection capacities have been reached.

C2T, C4, C5, & C6

Motor Vehicle Arterial Generalized Service Volume Tables



(C2T-Rural Town)

Peak Hour Directional

	B	C	D	E
1 Lane	*	720	940	**
2 Lane	*	1,140	1,640	**
3 Lane	*	2,120	2,510	**

Peak Hour Two-Way

	B	C	D	E
2 Lane	*	1,310	1,710	**
4 Lane	*	2,070	2,980	**
6 Lane	*	3,850	4,560	**

AADT

	B	C	D	E
2 Lane	*	13,800	18,000	**
4 Lane	*	21,800	31,400	**
6 Lane	*	40,500	48,000	**

(C4-Urban General)

	B	C	D	E
1 Lane	*	*	870	1,190
2 Lane	*	1,210	1,790	2,020
3 Lane	*	2,210	2,810	2,990
4 Lane	*	2,590	3,310	3,510

	B	C	D	E
2 Lane	*	*	1,580	2,160
4 Lane	*	2,200	3,250	3,670
6 Lane	*	4,020	5,110	5,440
8 Lane	*	4,710	6,020	6,380

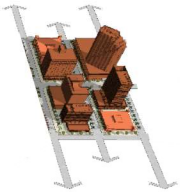
	B	C	D	E
2 Lane	*	*	17,600	24,000
4 Lane	*	24,400	36,100	40,800
6 Lane	*	44,700	56,800	60,400
8 Lane	*	52,300	66,900	70,900

(C5-Urban Center)

	B	C	D	E
1 Lane	*	*	690	1,080
2 Lane	*	1,290	1,900	2,130
3 Lane	*	1,410	2,670	3,110
4 Lane	*	2,910	3,560	3,640

	B	C	D	E
2 Lane	*	*	1,250	1,960
4 Lane	*	2,350	3,450	3,870
6 Lane	*	2,560	4,850	5,650
8 Lane	*	5,290	6,470	6,620

	B	C	D	E
2 Lane	*	*	13,900	21,800
4 Lane	*	26,100	38,300	43,000
6 Lane	*	28,400	53,900	62,800
8 Lane	*	58,800	71,900	73,600



(C6-Urban Core)

	B	C	D	E
1 Lane	*	***	790	1,030
2 Lane	*	***	1,490	1,920
3 Lane	*	***	2,730	2,940
4 Lane	*	***	3,250	3,490

	B	C	D	E
2 Lane	*	***	1,440	1,870
4 Lane	*	***	2,710	3,490
6 Lane	*	***	4,960	5,350
8 Lane	*	***	5,910	6,350

	B	C	D	E
2 Lane	*	***	16,000	20,800
4 Lane	*	***	30,100	38,800
6 Lane	*	***	55,100	59,400
8 Lane	*	***	65,700	70,600

Adjustment Factors

The peak hour directional service volumes should be adjust by multiplying by 1.2 for one-way facilities
The AADT service volumes should be adjusted by multiplying 0.6 for one way facilities 2 Lane Divided Roadway with an Exclusive Left Turn Lane(s): Multiply by 1.05
2 lane Undivided Roadway with No Exclusive Left Turn Lane(s): Multiply by 0.80

Exclusive right turn lane(s): Multiply by 1.05
Multilane Undivided Roadway with an Exclusive Left Turn Lane(s): Multiply by 0.95
Multilane Roadway with No Exclusive Left Turn Lane(s): Multiply by 0.75
Non-State Signalized Roadway: Multiply by 0.90

This table does not constitute a standard and should be used only for general planning applications. The table should not be used for corridor or intersection design, where more refined techniques exist.
*Cannot be achieved using table input value defaults. **Not applicable for that level of service letter grade. For the automobile mode, volumes greater than level of service D become F because intersection capacities have been reached.
***LOS C thresholds are not applicable for C6 as C6 roadway facilities are neither planned nor designed to achieve automobile LOS C.

APPENDIX C

SIGNAL TIMING SHEET

FDOT - DISTRICT 1

Signal System Timing Report

System ID: ***07010A***

Section: ***07010000***

Arterial: ***SR 80 / Hickpochee Ave***

From: ***Main Street***

MP: ***9.355***

To: ***Lee Street***

MP: ***9.500***

County: ***Hendry***

Implementation Start Date: March 7, 2024
Fine Tuning Completion Date: March 11, 2024

Name		Date	Approved By:
Designed By:	Michael Munson	3/26/2024	
Checked By:	Phillip Kurth	3/26/2024	
Drawn By:	Michael Munson	3/26/2024	
Checked By:	Phillip Kurth	3/26/2024	

Time of Day Plan

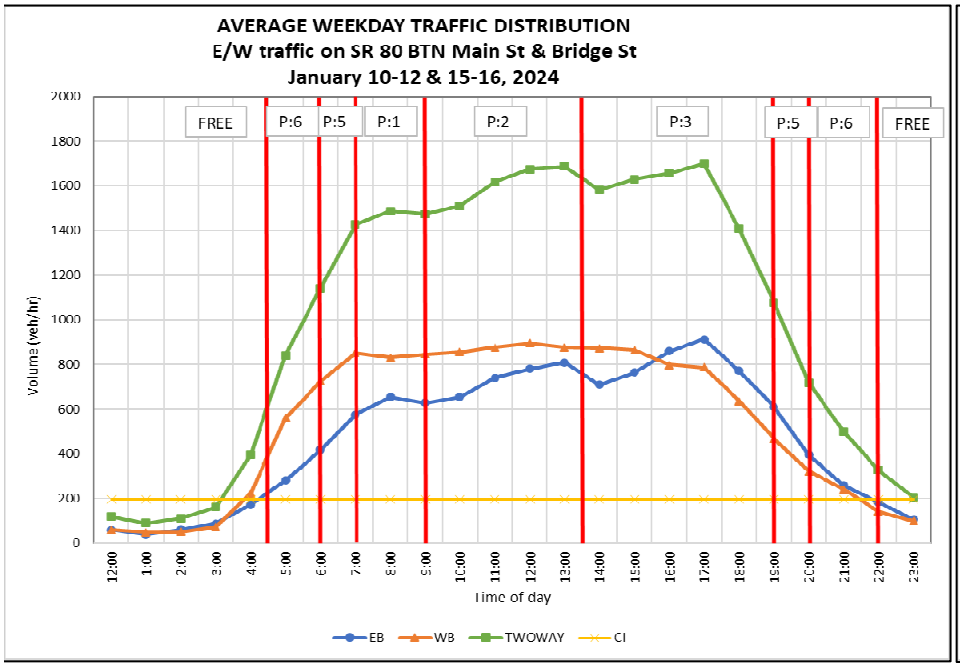
Designed By:	M. Munson
Date:	3/26/2024
Checked By:	P. Kurth
Date:	3/26/2024

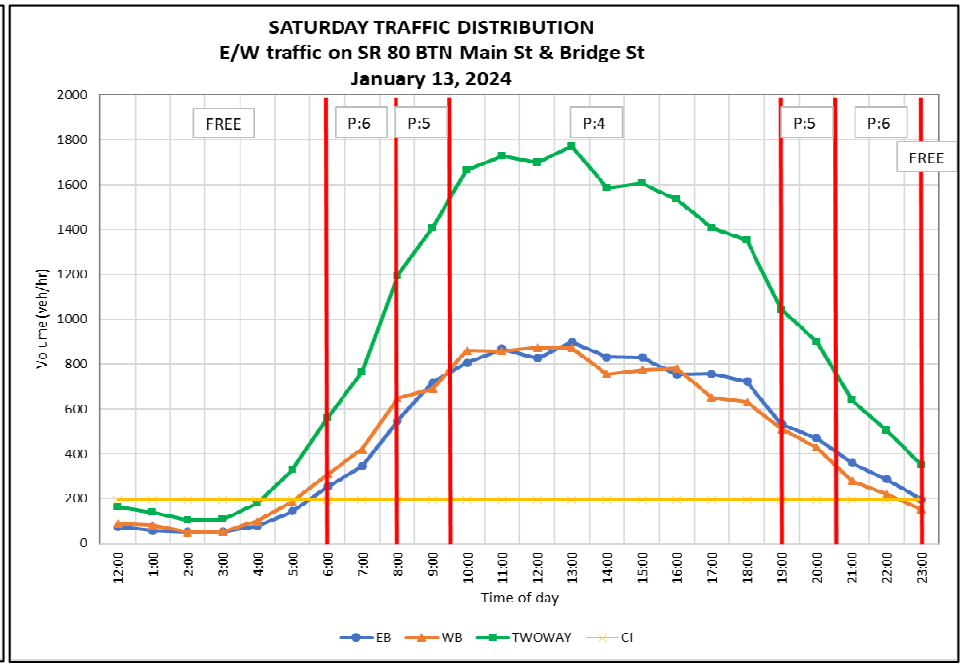
System ID: ***07010A***
Section: ***07010000***
Segment: ***SR 80 / Hickpochee Ave***
From: ***Main Street***
To: ***Lee Street***

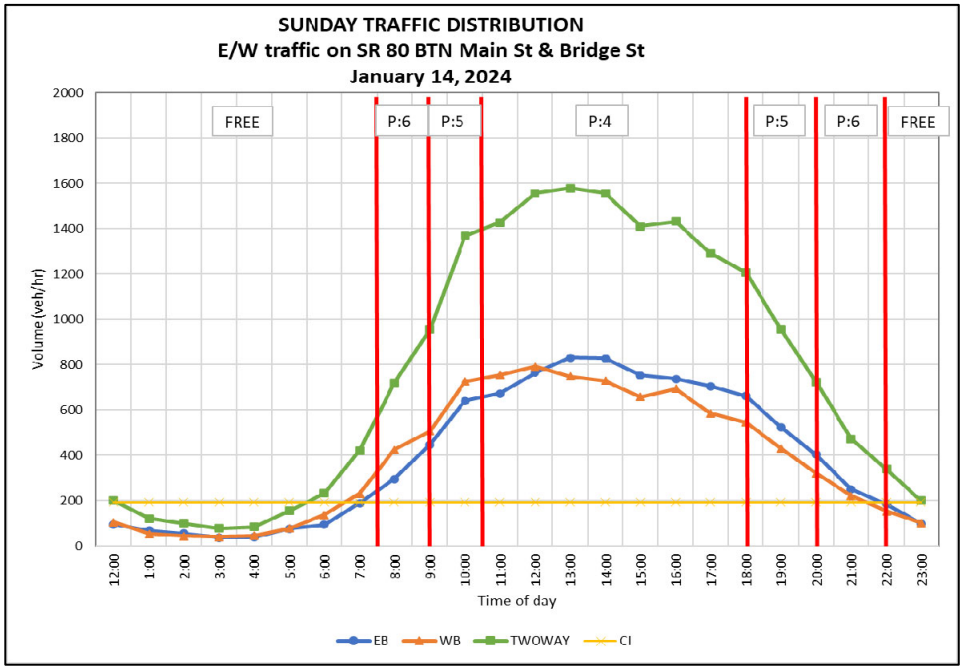
ALL SEASON PLAN

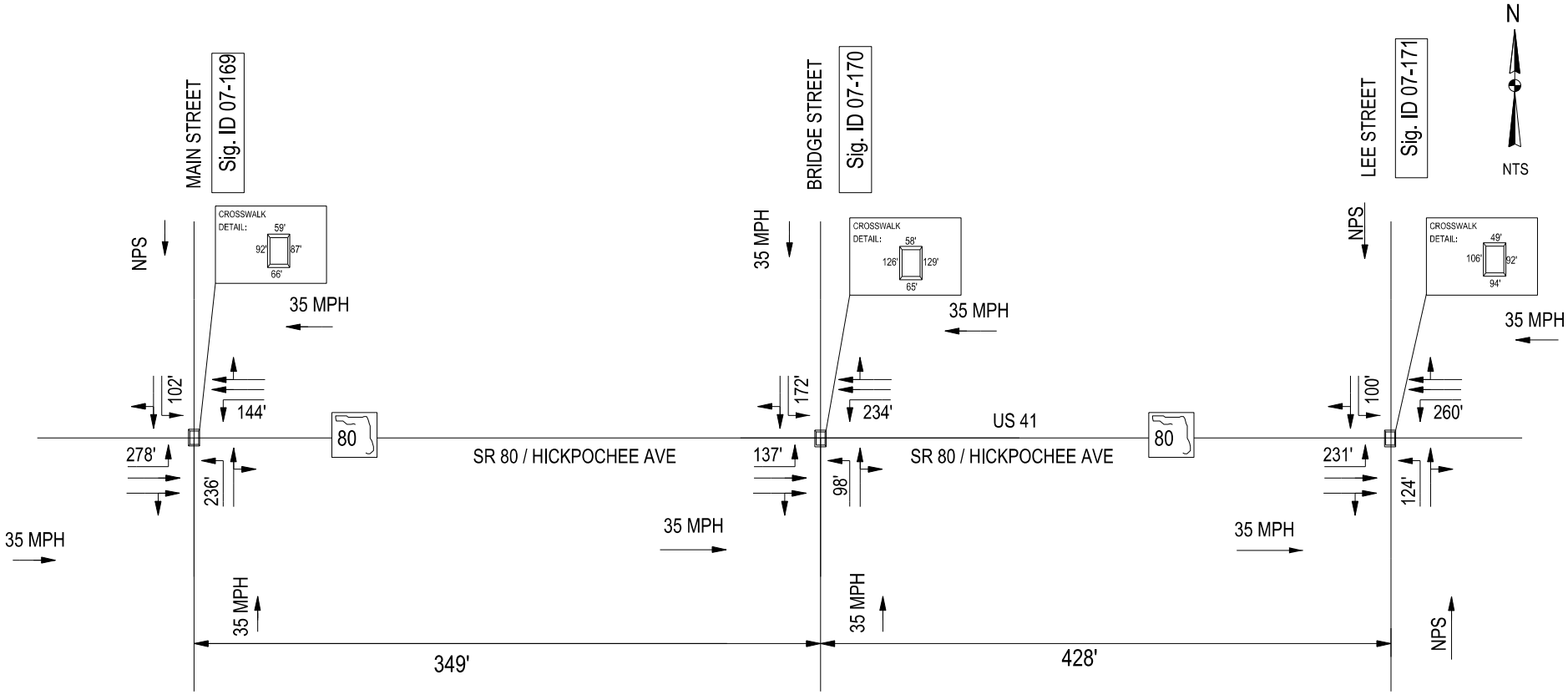
Day	Time	Pattern (C/S/O)	Cycle Length
Monday Thru Friday	0000	-	FREE
	0430	6	80
	0600	5	100
	0700	1	140
	0900	2	140
	1330	3	140
	1900	5	100
	2000	6	80
	2200	-	FREE
Saturday	0000	-	FREE
	0600	6	80
	0800	5	100
	0930	4	120
	1900	5	100
	2030	6	80
	2300	-	FREE
Sunday	0000	-	FREE
	0730	6	80
	0900	5	100
	1030	4	120
	1800	5	100
	2000	6	80
	2200	-	FREE

COUPLING ANALYSIS (SR 80 from Main St to Lee St)



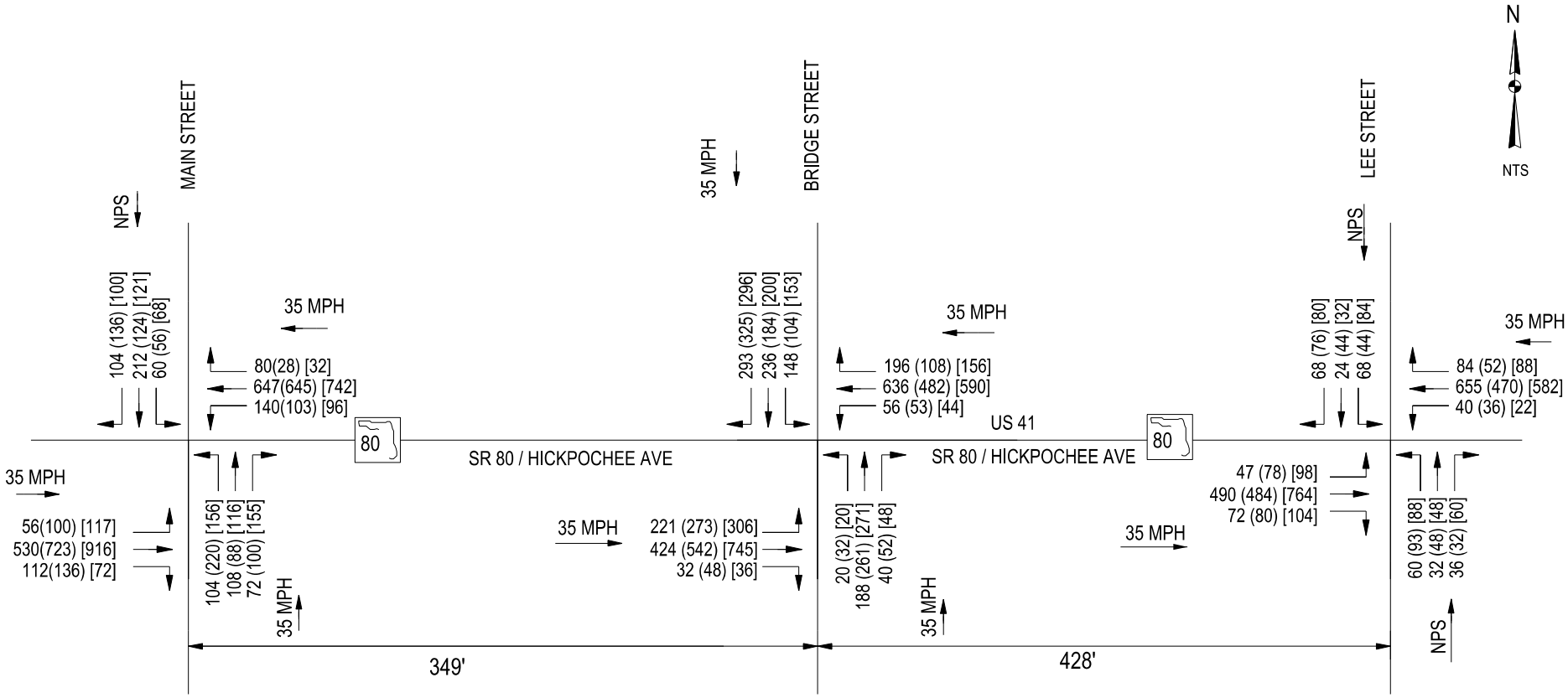






NOTES:
DISTANCES BETWEEN INTERSECTIONS WERE DETERMINED FROM FDOT SLDs

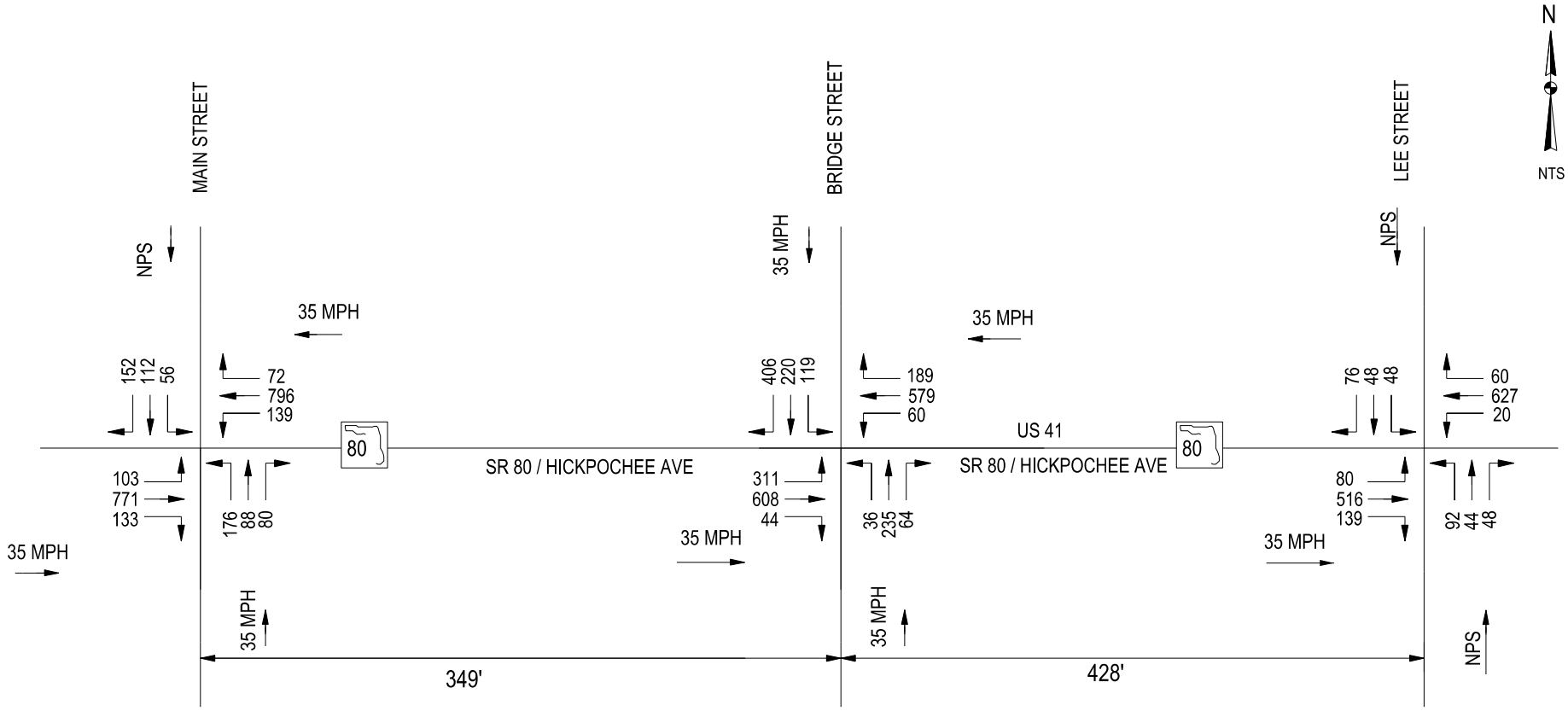
DISTRICT ONE TRAFFIC OPERATIONS SIGNAL SECTION	SR 80 / HICKPOCHEE AVE FROM MAIN STREET TO LEE STREET **LINE SKETCH OF LANE CONFIGURATIONS**	PREPARED BY: FTE DATE: 02/05/2024



NOTES:

- XXX (XXX) [XXX] - AM VOLUME (MIDDAY VOLUME) [PM VOLUME]
- ALL VOLUME VALUES SHOWN IN THIS SHEET ARE FOR TRAFFIC ANALYSIS FOR SIGNAL TIMING DESIGN PURPOSES ONLY

FDOT DISTRICT ONE TRAFFIC OPERATIONS SIGNAL SECTION	SR 80 / HICKPOCHEE AVE FROM MAIN STREET TO LEE STREET **LINE SKETCH OF TRAFFIC VOLUME: WEEKDAY**	PREPARED BY: FTE DATE: 02/19/2024



NOTES:

- SATURDAY VOLUME
- ALL VOLUME VALUES SHOWN IN THIS SHEET ARE FOR TRAFFIC ANALYSIS FOR SIGNAL TIMING DESIGN PURPOSES ONLY

DISTRICT ONE TRAFFIC OPERATIONS SIGNAL SECTION	SR 80 / HICKPOCHEE AVE FROM MAIN STREET TO LEE STREET **LINE SKETCH OF TRAFFIC VOLUME: SATURDAY**	PREPARED BY: FTE DATE: 02/19/2024

Designed By:	M. Munson
Date:	3/26/2024
Checked By:	P. Kurth
Date:	3/26/2024

Location Details	
Section: 07010000	Mile Post: 9.355
Major Street: SR 80 / Hickpochee Ave	Orientation E-W
Minor Street: Main Street	Orientation N-S
Sig ID: 169	System ID: 07010A

Controller Timings (seconds)

Movement # (Controller Phase Ø)	1	2	3	4	5	6	7	8	Notes
Direction	EBL	WB		NB	WBL	EB	NBL	SB	
Turn Type	FYA				FYA		FYA		
Min Green	5	12		7	5	12	5	7	
Ext	3	5		3	3	5	3	3	
Yellow	4.1	4.1		4.1	4.1	4.1	4.1	4.1	
All Red	3.4	3.4		2.0	3.4	3.4	2.0	2.0	
Max I	10	40		20	10	40	10	20	
Max II									
Walk		7		7		7		7	
Flashing Don't Walk		17		25		19		27	
Detector Memory									
Det. Cross Switch.	YES				YES		YES		
Dual Entry				On				On	
Vehicle Recall		Min		Min		Min	Min		
CNA									
Rest in Walk									

Coordination Timings (seconds)

Pattern	C-S-O	Cycle Length	Splits								Offset	Sequence	Coord Phase
1		140	16	72		52	21	67	19	33	9	3	2 & 6
2		140	16	72		52	21	67	19	33	9	3	2 & 6
3		140	16	72		52	21	67	19	33	9	3	2 & 6
4		120	16	49		55	16	49	19	36	9	3	2 & 6
5		100	14	50		36	17	47	14	22	6	1	2 & 6
6		80	14	36		30	14	36	14	16	10	1	2 & 6

Offset Reference Point
End of Main Street Green

SOP 10 (Sequence 1)

Ring 1	1	2	4	
Ring 2	5	6	7	8

SOP 10 (Sequence 3)

Ring 1	2	1	4	
Ring 2	5	6	7	8

Notes:

1) Use 'Max I" during FREE operations and 'Max Inhibit' during coordination
2) Use Fixed Force Offs
3) Max recall phases 2 & 6 during coordination
4) Sequence 1 used during FREE operation
5) Program 3 sec of detection delay for minor street left turn movements
6) Program 8 sec of detection delay for minor street right turn movements
7) Controller: Peek ATC-1000
7.a) Permissive Type: Yield
7.b) Yield Percent: 5%
7.c) Offset Seeking Mode: Short Route
7.d) Pedestrian Override = YES for all patterns
8) Program 3 sec startup delay for FYA indication on movements 1, 5
9) Remove Detector cross switching and phase restrictions for Ø1 & Ø5 during all Patterns

Designed By:	M. Munson
Date:	3/26/2024
Checked By:	P. Kurth
Date:	3/26/2024

Location Details	
Section: 07010000	Mile Post: 9.422
Major Street: SR 80 / Hickpochee Ave	Orientation E-W
Minor Street: SR 29 / Bridge Street	Orientation N-S
Sig ID: 170	System ID: 07010A

Controller Timings (seconds)

Movement # (Controller Phase Ø)	1	2	3	4	5	6	7	8	Notes
Direction	EBL	WB	SBL	NB	WBL	EB		SB	
Turn Type	FYA		FYA		FYA				
Min Green	5	12	5	7	5	12		7	
Ext	3	5	3	4	3	5		6	
Yellow	4.1	4.1	4.1	4.1	4.1	4.1		4.1	
All Red	3.1	3.1	2.6	2.6	3.1	3.1		2.6	
Max I	10	40	10	40	10	40		40	
Max II									
Walk		7		7		7		7	
Flashing Don't Walk		23		33		24		32	
Detector Memory									
Det. Cross Switch.	YES		YES		YES				
Dual Entry				On				On	
Vehicle Recall		Min				Min			
CNA									
Rest in Walk									

Coordination Timings (seconds)

Pattern	C-S-O	Cycle Length	Splits								Offset	Sequence	Coord Phase
1		140	28	50	16	46	13	65		62	0	3	2 & 6
2		140	33	45	16	46	13	65		62	0	3	2 & 6
3		140	33	50	16	41	13	70		57	0	3	2 & 6
4		120	31	37	19	33	13	55		52	5	3	2 & 6
5		100	25	36	16	23	16	45		39	0	3	2 & 6
6		80	17	31	15	17	14	34		32	0	3	2 & 6

Offset Reference Point
End of Main Street Green

SOP 10 (Sequence 1)

Ring 1	1	2	3	4
Ring 2	5	6	8	

SOP 10 (Sequence 3)

Ring 1	2	1	3	4
Ring 2	5	6	8	

Notes:

1) Use 'Max I" during FREE operations and 'Max Inhibit' during coordination
2) Use Fixed Force Offs
3) Max recall phases 2 & 6 during coordination
4) Sequence 1 used during FREE operation
5) Program 3 sec of detection delay for minor street left turn movements
6) Program 8 sec of detection delay for minor street right turn movements
7) Controller: Peek ATC-1000
7.a) Permissive Type: Yield
7.b) Yield Percent: 5%
7.c) Offset Seeking Mode: Short Route
7.d) Pedestrian Override = YES for all patterns
8) Program 3 sec startup delay for FYA indication on movements 1, 5
9) Remove Detector cross switching and phase restrictions for Ø1 & Ø5 during all Patterns

Designed By:	M. Munson
Date:	3/26/2024
Checked By:	P. Kurth
Date:	3/26/2024

Location Details	
Section: 07010000	Mile Post: 9.500
Major Street: SR 80 / Hickpochee Ave	Orientation E-W
Minor Street: Lee Street	Orientation N-S
Sig ID: 171	System ID: 07010A

Controller Timings (seconds)

Movement # (Controller Phase Ø)	1	2	3	4	5	6	7	8	Notes
Direction	*EBL*	*WB*		*NB*	*WBL*	*EB*		*SB*	
Turn Type	*FYA*				*FYA*				
Min Green	5	12		7	5	12		7	
Ext	3	5		3	3	5		3	
Yellow	4.1	4.1		3.7	4.1	4.1		3.7	
All Red	3.2	3.2		2.0	3.2	3.2		2.0	
Max I	*10*	*40*		*20*	*10*	*40*		*20*	
Max II									
Walk		*7*		*7*		*7*		7	
Flashing Don't Walk		*14*		*27*		*27*		31	
Detector Memory									
Det. Cross Switch.	*YES*				*YES*				
Dual Entry				*On*				*On*	
Vehicle Recall		*Min*				*Min*			
CNA									
Rest in Walk									

Coordination Timings (seconds)

Pattern	*C-S-O*	*Cycle Length*	*Splits*								*Offset*	*Sequence*	*Coord Phase*
1		*140*	*16*	*94*		*30*	*16*	*94*		*30*	*62*	*1*	*2 & 6*
2		*140*	*16*	*94*		*30*	*16*	*94*		*30*	*62*	*1*	*2 & 6*
3		*140*	*16*	*94*		*30*	*16*	*94*		*30*	*62*	*1*	*2 & 6*
4		*120*	*16*	*74*		*30*	*16*	*74*		*30*	*52*	*1*	*2 & 6*
5		*100*	*16*	*59*		*25*	*16*	*59*		*25*	*25*	*1*	*2 & 6*
6		*80*	*16*	*46*		*18*	*16*	*46*		*18*	*24*	*1*	*2 & 6*

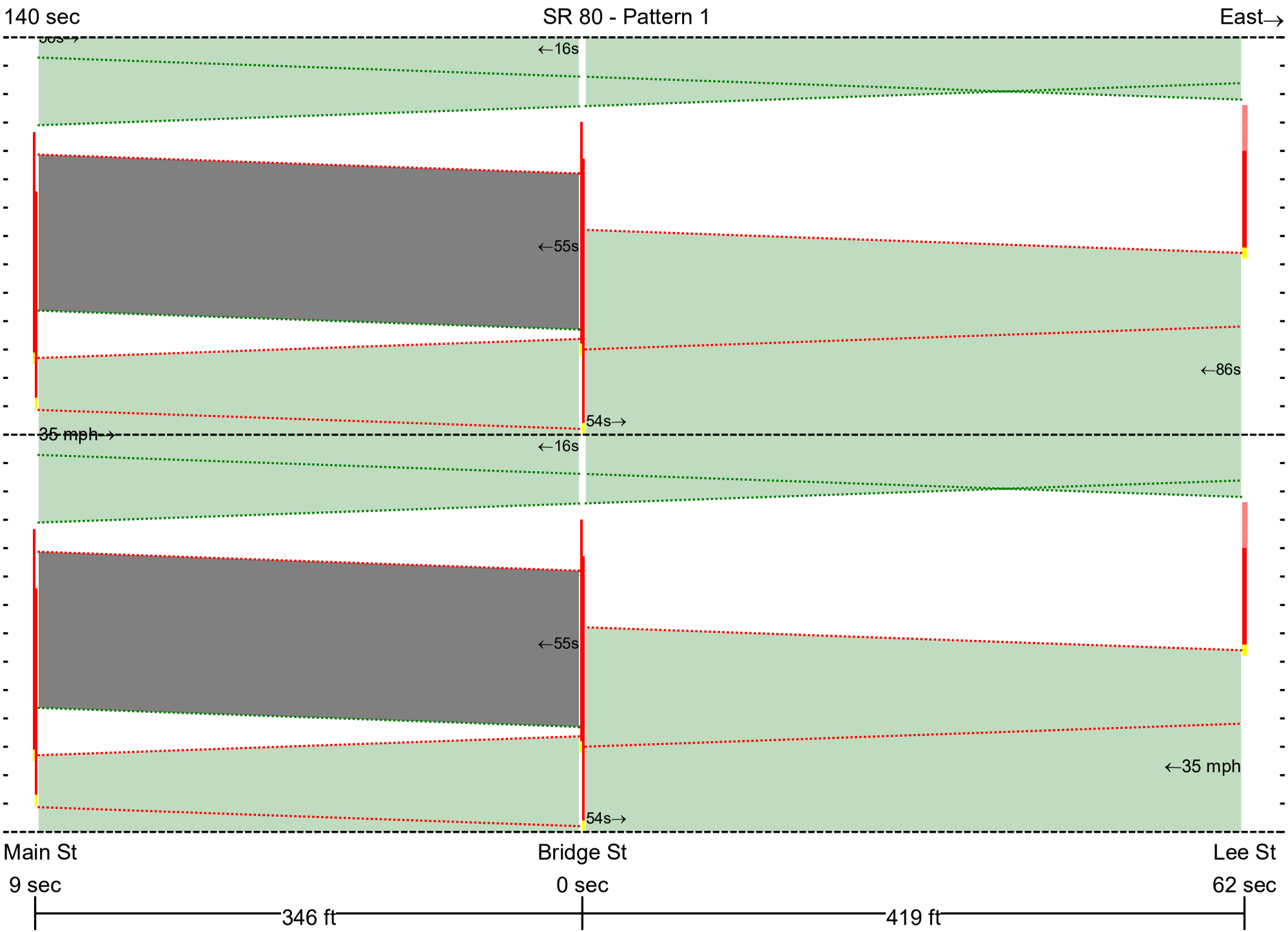
Offset Reference Point
End of Main Street Green

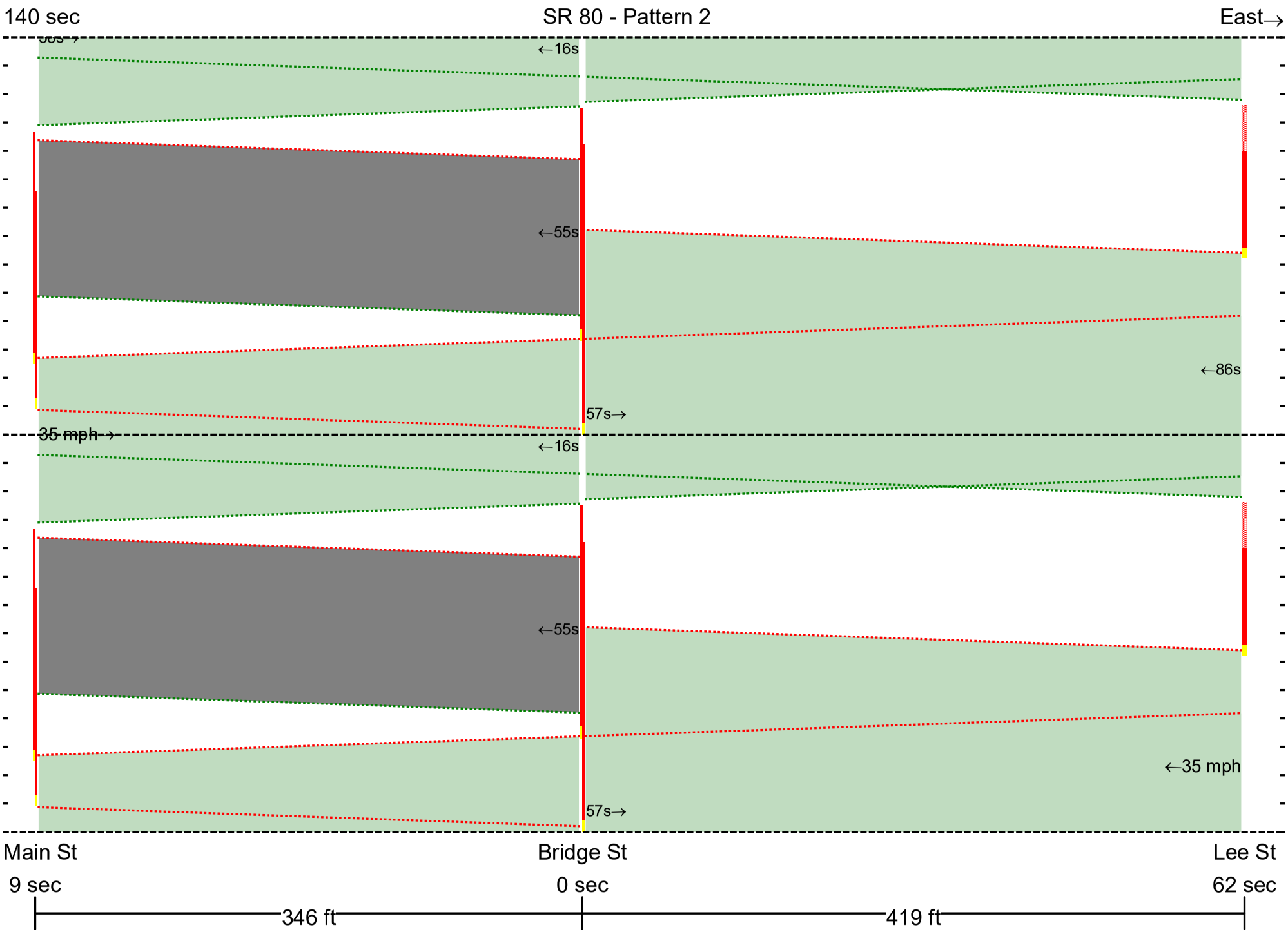
SOP 7 (Sequence 1)

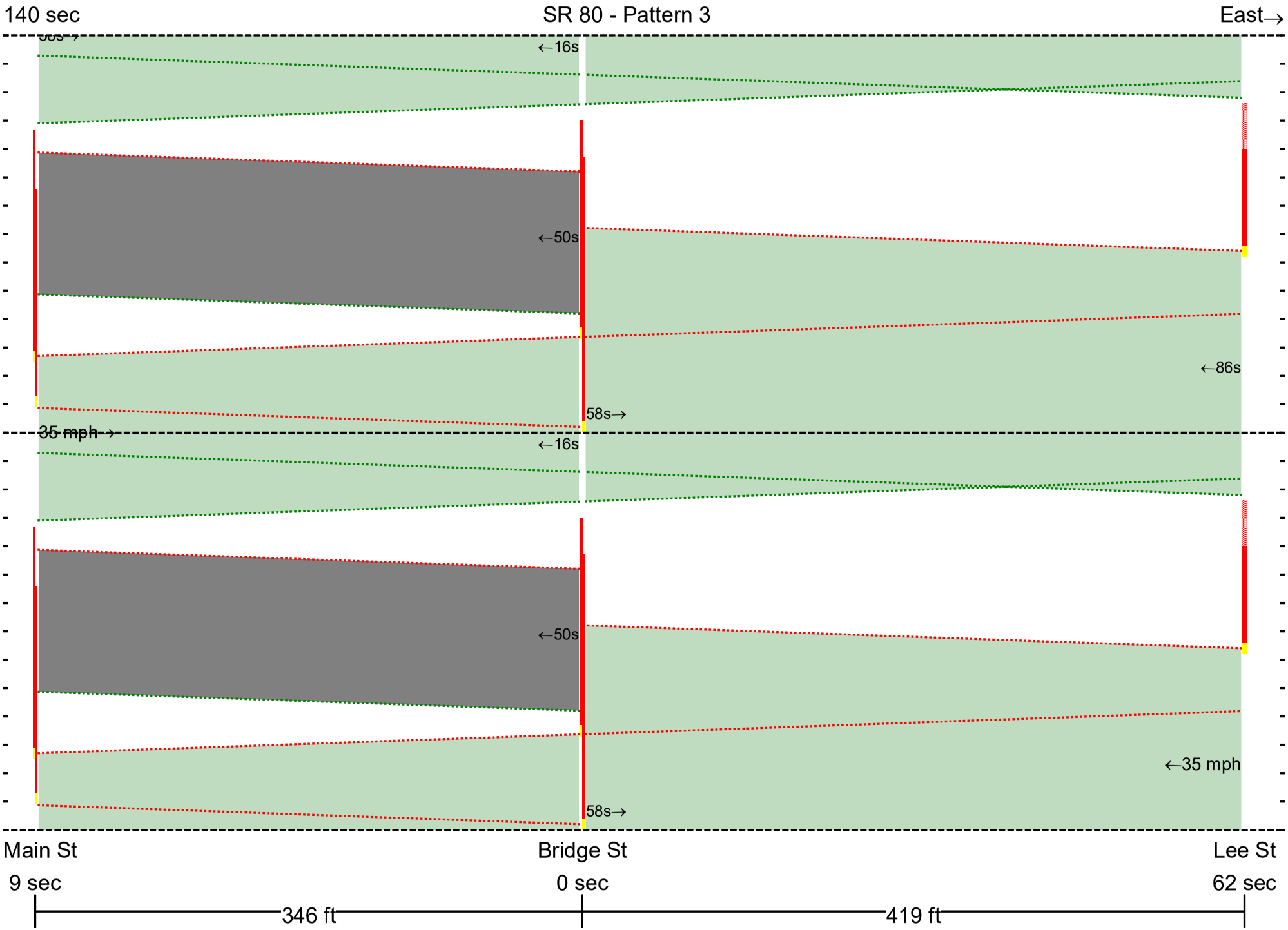
Ring 1	1	2	4
Ring 2	5	6	8

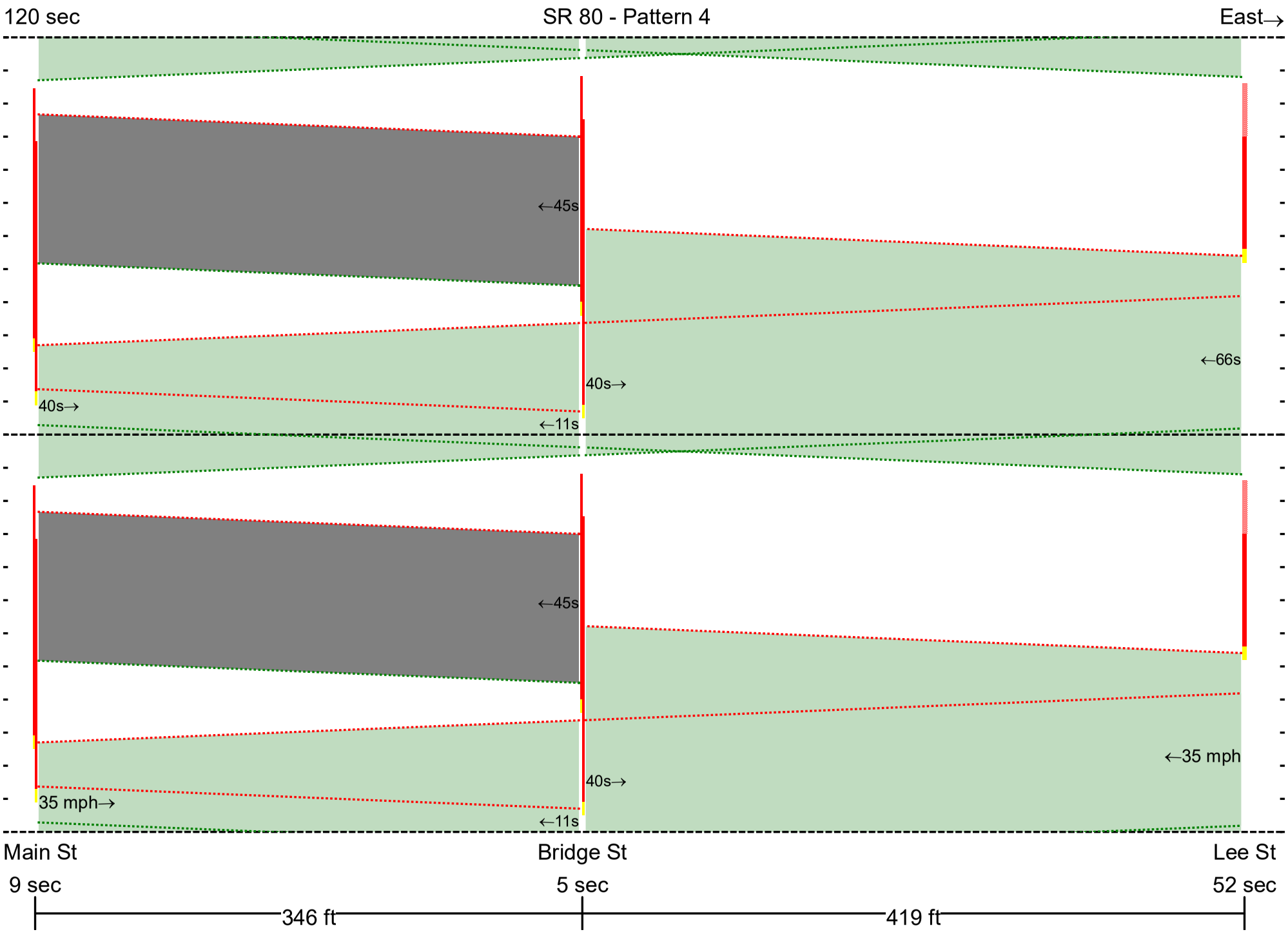
Notes:

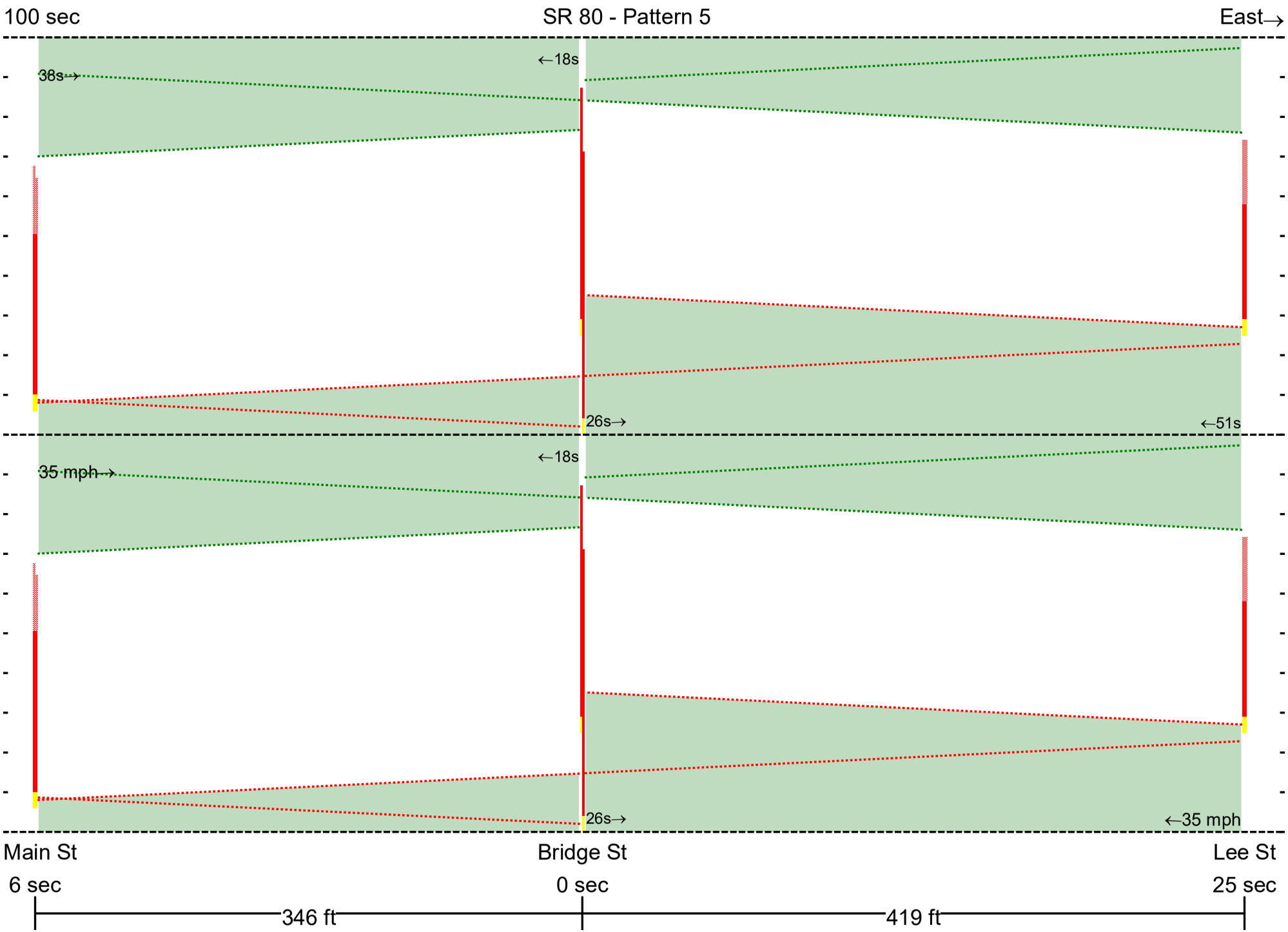
1) Use 'Max I" during FREE operations and 'Max Inhibit' during coordination
2) Use Fixed Force Offs
3) Max recall phases 2 & 6 during coordination
4) Sequence 1 used during FREE operation
5) Program 3 sec of detection delay for minor street left turn movements
6) Program 8 sec of detection delay for minor street right turn movements
7) Controller: Peek ATC-1000
7.a) Permissive Type: Yield
7.b) Yield Percent: 5%
7.c) Offset Seeking Mode: Short Route
7.d) Pedestrian Override = YES for all patterns
8) Program 3 sec startup delay for FYA indication on movements 1, 5

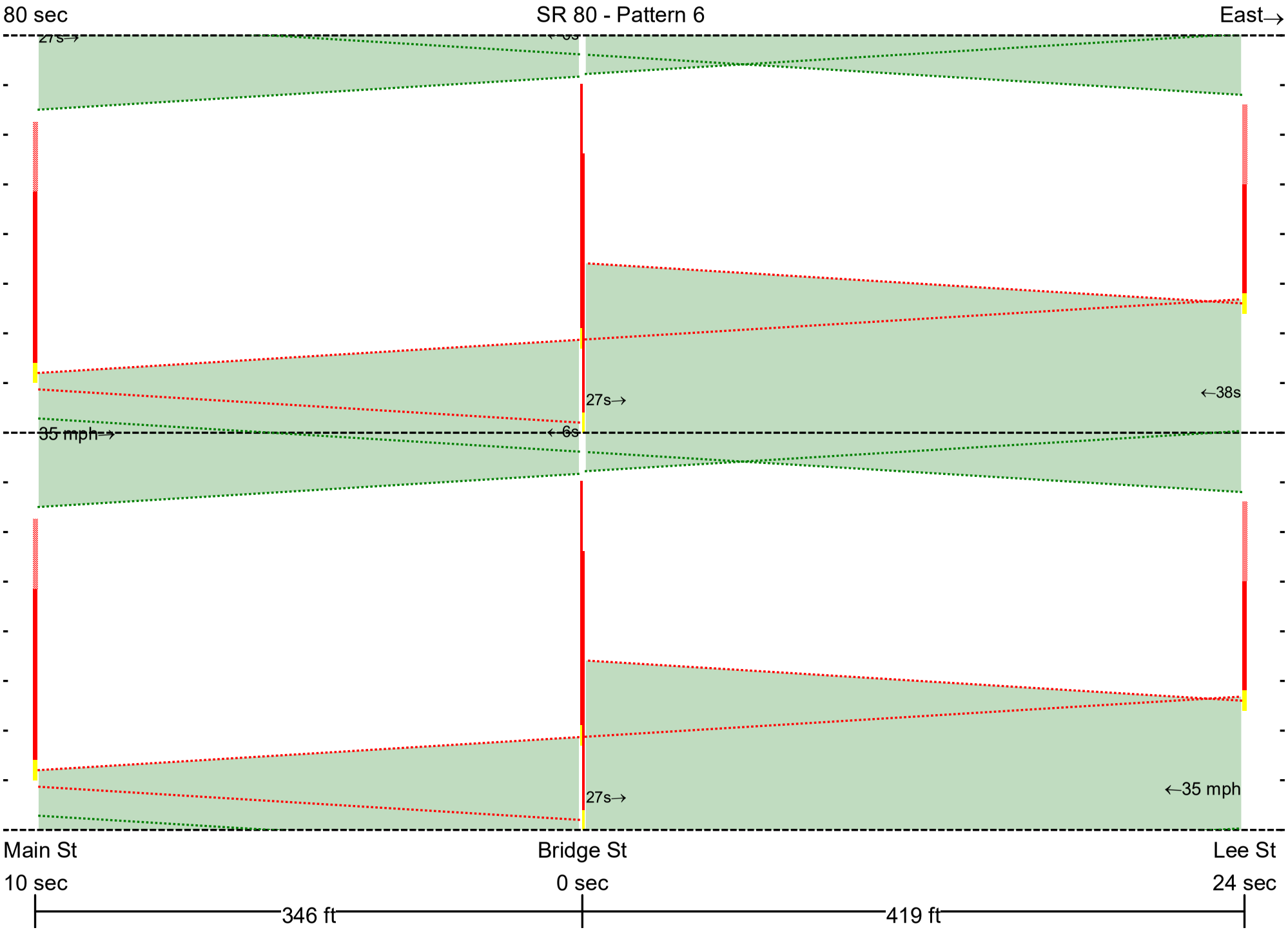

140 sec
SR 80 - Pattern 1
East→
←16s
←55s
←86s
54s→
35 mph→
←16s
←55s
←35 mph
54s→
Main St
Bridge St
Lee St
9 sec
0 sec
62 sec
346 ft
419 ft


140 sec
SR 80 - Pattern 2
East→
←16s
←55s
←86s
57s→
35 mph→
←16s
←55s
←35 mph
57s→
Main St
Bridge St
Lee St
9 sec
0 sec
62 sec
346 ft
419 ft


140 sec
SR 80 - Pattern 3
East→
←16s
←50s
←86s
58s→
35 mph→
←16s
←50s
←35 mph
58s→
Main St
Bridge St
Lee St
9 sec
0 sec
62 sec
346 ft
419 ft


120 sec
SR 80 - Pattern 4
East→
←45s
40s→
←11s
←66s
40s→
←45s
35 mph→
←11s
←35 mph
40s→
Main St
Bridge St
Lee St
9 sec
5 sec
52 sec
346 ft
419 ft


100 sec
SR 80 - Pattern 5
East→
←18s
38s→
26s→
←51s
←18s
35 mph→
26s→
←35 mph
Main St
Bridge St
Lee St
6 sec
0 sec
25 sec
346 ft
419 ft


80 sec
SR 80 - Pattern 6
East→
27s→
←6s
27s→
←38s
35 mph→
←6s
27s→
←35 mph
Main St
Bridge St
Lee St
10 sec
0 sec
24 sec
346 ft
419 ft

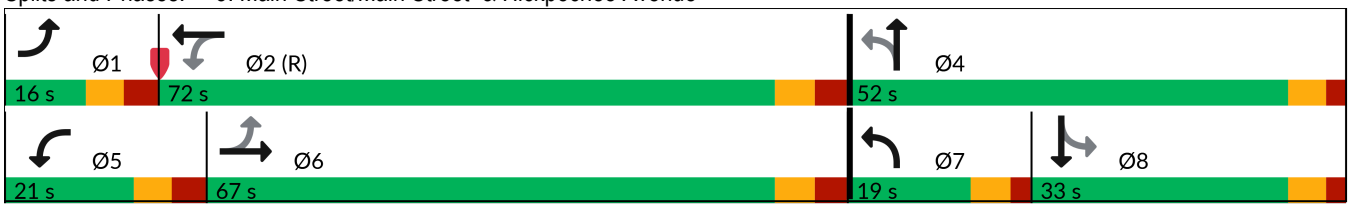
APPENDIX D

EXISTING CONDITIONS SYNCHRO

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	32	507	110	644	112	105	56	157
Future Volume (vph)	32	507	110	644	112	105	56	157
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	NA
Protected Phases	1	6	5	2	7	4		8
Permitted Phases	6		2		4		8	
Detector Phase	1	6	5	2	7	4	8	8
Switch Phase								
Minimum Initial (s)	5.0	12.0	5.0	12.0	5.0	7.0	7.0	7.0
Minimum Split (s)	12.5	23.5	12.5	23.5	11.1	22.1	22.1	22.1
Total Split (s)	16.0	67.0	21.0	72.0	19.0	52.0	33.0	33.0
Total Split (%)	11.4%	47.9%	15.0%	51.4%	13.6%	37.1%	23.6%	23.6%
Yellow Time (s)	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1
All-Red Time (s)	3.4	3.4	3.4	3.4	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.5	7.5	7.5	7.5	6.1	6.1	6.1	6.1
Lead/Lag	Lead	Lag	Lead	Lag	Lead		Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes
Recall Mode	None	Max	None	C-Max	None	None	None	None
Act Effct Green (s)	74.8	68.1	82.8	76.3	41.3	41.3	23.2	23.2
Actuated g/C Ratio	0.53	0.49	0.59	0.55	0.30	0.30	0.17	0.17
v/c Ratio	0.09	0.35	0.25	0.38	0.52	0.33	0.30	0.84
Control Delay (s/veh)	13.5	23.7	14.0	20.8	44.3	34.2	54.0	75.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	13.5	23.7	14.0	20.8	44.3	34.2	54.0	75.6
LOS	B	C	B	C	D	C	D	E
Approach Delay (s/veh)		23.1		19.9		38.3		71.6
Approach LOS		C		B		D		E

Intersection Summary	
Cycle Length: 140	
Actuated Cycle Length: 140	
Offset: 0 (0%), Referenced to phase 2:WBTL, Start of Green	
Natural Cycle: 70	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.84	
Intersection Signal Delay (s/veh): 31.3	Intersection LOS: C
Intersection Capacity Utilization 66.0%	ICU Level of Service C
Analysis Period (min) 15	

Splits and Phases: 3: Main Street/Main Street & Hickpochee Avenue



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	34	593	116	729	118	175	59	258
v/c Ratio	0.09	0.35	0.25	0.38	0.52	0.33	0.30	0.84
Control Delay (s/veh)	13.5	23.7	14.0	20.8	44.3	34.2	54.0	75.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	13.5	23.7	14.0	20.8	44.3	34.2	54.0	75.6
Queue Length 50th (ft)	12	174	44	217	79	106	47	213
Queue Length 95th (ft)	29	238	77	284	129	167	91	311
Internal Link Dist (ft)		990		1144		978		957
Turn Bay Length (ft)								
Base Capacity (vph)	419	1700	504	1911	238	591	231	353
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.35	0.23	0.38	0.50	0.30	0.26	0.73

Intersection Summary

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	32	507	56	110	644	48	112	105	61	56	157	88
Future Volume (veh/h)	32	507	56	110	644	48	112	105	61	56	157	88
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	34	534	48	116	678	40	118	111	53	59	165	82
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	424	1770	159	502	1890	111	195	321	154	243	185	92
Arrive On Green	0.03	0.54	0.54	0.04	0.55	0.55	0.07	0.27	0.27	0.16	0.16	0.16
Sat Flow, veh/h	1781	3298	296	1781	3410	201	1781	1196	571	1222	1179	586
Grp Volume(v), veh/h	34	287	295	116	353	365	118	0	164	59	0	247
Grp Sat Flow(s),veh/h/ln	1781	1777	1817	1781	1777	1834	1781	0	1768	1222	0	1765
Q Serve(g_s), s	1.2	12.5	12.6	4.1	15.5	15.5	7.6	0.0	10.5	6.0	0.0	19.2
Cycle Q Clear(g_c), s	1.2	12.5	12.6	4.1	15.5	15.5	7.6	0.0	10.5	6.0	0.0	19.2
Prop In Lane	1.00		0.16	1.00		0.11	1.00		0.32	1.00		0.33
Lane Grp Cap(c), veh/h	424	953	975	502	985	1017	195	0	475	243	0	277
V/C Ratio(X)	0.08	0.30	0.30	0.23	0.36	0.36	0.60	0.00	0.35	0.24	0.00	0.89
Avail Cap(c_a), veh/h	485	953	975	595	985	1017	238	0	580	286	0	339
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	14.3	17.9	18.0	13.8	17.3	17.4	45.2	0.0	41.3	52.3	0.0	57.8
Incr Delay (d2), s/veh	0.1	0.8	0.8	0.2	1.0	1.0	3.0	0.0	0.4	0.5	0.0	21.3
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	0.9	9.2	9.4	3.0	10.9	11.2	6.4	0.0	8.2	3.4	0.0	15.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	14.4	18.7	18.8	14.1	18.4	18.3	48.1	0.0	41.7	52.8	0.0	79.1
LnGrp LOS	B	B	B	B	B	B	D		D	D		E
Approach Vol, veh/h		616			834			282			306	
Approach Delay, s/veh		18.5			17.8			44.4			74.0	
Approach LOS		B			B			D			E	

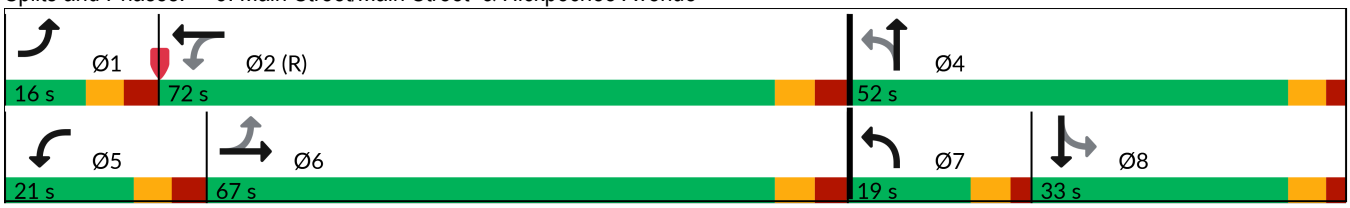
Timer - Assigned Phs	1	2		4	5	6	7	8
Phs Duration (G+Y+Rc), s	11.2	85.1		43.7	13.7	82.6	15.6	28.1
Change Period (Y+Rc), s	7.5	7.5		6.1	7.5	7.5	6.1	6.1
Max Green Setting (Gmax), s	8.5	64.5		45.9	13.5	59.5	12.9	26.9
Max Q Clear Time (g_c+I1), s	3.2	17.5		12.5	6.1	14.6	9.6	21.2
Green Ext Time (p_c), s	0.0	5.2		1.0	0.1	4.0	0.1	0.8

Intersection Summary	
HCM 7th Control Delay, s/veh	30.1
HCM 7th LOS	C

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	67	862	84	656	142	101	37	188
Future Volume (vph)	67	862	84	656	142	101	37	188
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	NA
Protected Phases	1	6	5	2	7	4		8
Permitted Phases	6		2		4		8	
Detector Phase	1	6	5	2	7	4	8	8
Switch Phase								
Minimum Initial (s)	5.0	12.0	5.0	12.0	5.0	7.0	7.0	7.0
Minimum Split (s)	12.5	23.5	12.5	23.5	11.1	22.1	22.1	22.1
Total Split (s)	16.0	67.0	21.0	72.0	19.0	52.0	33.0	33.0
Total Split (%)	11.4%	47.9%	15.0%	51.4%	13.6%	37.1%	23.6%	23.6%
Yellow Time (s)	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1
All-Red Time (s)	3.4	3.4	3.4	3.4	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.5	7.5	7.5	7.5	6.1	6.1	6.1	6.1
Lead/Lag	Lead	Lag	Lead	Lag	Lead		Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes
Recall Mode	None	Max	None	C-Max	None	None	None	None
Act Effct Green (s)	74.5	66.9	77.9	70.6	43.4	43.4	24.9	24.9
Actuated g/C Ratio	0.53	0.48	0.56	0.50	0.31	0.31	0.18	0.18
v/c Ratio	0.19	0.58	0.31	0.42	0.66	0.47	0.20	0.89
Control Delay (s/veh)	14.4	28.7	16.0	23.8	50.2	32.7	50.8	80.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	14.4	28.7	16.0	23.8	50.2	32.7	50.8	80.9
LOS	B	C	B	C	D	C	D	F
Approach Delay (s/veh)		27.7		23.0		38.9		77.3
Approach LOS		C		C		D		E

Intersection Summary	
Cycle Length: 140	
Actuated Cycle Length: 140	
Offset: 0 (0%), Referenced to phase 2:WBTL, Start of Green	
Natural Cycle: 80	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.89	
Intersection Signal Delay (s/veh): 34.3	Intersection LOS: C
Intersection Capacity Utilization 76.1%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 3: Main Street/Main Street & Hickpochee Avenue



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	71	970	88	735	149	268	39	290
v/c Ratio	0.19	0.58	0.31	0.42	0.66	0.47	0.20	0.89
Control Delay (s/veh)	14.4	28.7	16.0	23.8	50.2	32.7	50.8	80.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	14.4	28.7	16.0	23.8	50.2	32.7	50.8	80.9
Queue Length 50th (ft)	28	338	34	232	100	150	30	244
Queue Length 95th (ft)	51	423	60	289	159	237	66	#391
Internal Link Dist (ft)		990		1144		978		957
Turn Bay Length (ft)								
Base Capacity (vph)	390	1677	345	1770	230	594	212	352
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.18	0.58	0.26	0.42	0.65	0.45	0.18	0.82

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	67	862	60	84	656	42	142	101	154	37	188	87
Future Volume (veh/h)	67	862	60	84	656	42	142	101	154	37	188	87
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	71	907	52	88	691	33	149	106	151	39	198	81
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	402	1759	101	316	1791	85	216	207	295	244	217	89
Arrive On Green	0.03	0.52	0.52	0.04	0.52	0.52	0.08	0.30	0.30	0.17	0.17	0.17
Sat Flow, veh/h	1781	3416	196	1781	3453	165	1781	698	994	1123	1261	516
Grp Volume(v), veh/h	71	472	487	88	355	369	149	0	257	39	0	279
Grp Sat Flow(s),veh/h/ln	1781	1777	1835	1781	1777	1841	1781	0	1691	1123	0	1777
Q Serve(g_s), s	2.6	24.5	24.5	3.3	16.9	16.9	9.4	0.0	17.6	4.2	0.0	21.6
Cycle Q Clear(g_c), s	2.6	24.5	24.5	3.3	16.9	16.9	9.4	0.0	17.6	4.3	0.0	21.6
Prop In Lane	1.00		0.11	1.00		0.09	1.00		0.59	1.00		0.29
Lane Grp Cap(c), veh/h	402	915	945	316	922	955	216	0	503	244	0	306
V/C Ratio(X)	0.18	0.52	0.52	0.28	0.39	0.39	0.69	0.00	0.51	0.16	0.00	0.91
Avail Cap(c_a), veh/h	450	915	945	422	922	955	236	0	555	266	0	342
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	15.9	22.4	22.4	17.3	20.3	20.3	43.2	0.0	40.8	49.8	0.0	56.9
Incr Delay (d2), s/veh	0.2	2.1	2.0	0.5	1.2	1.2	7.4	0.0	0.8	0.3	0.0	26.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	2.0	16.2	16.6	2.5	11.8	12.1	8.1	0.0	12.0	2.2	0.0	17.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	16.1	24.5	24.4	17.8	21.5	21.5	50.6	0.0	41.6	50.1	0.0	82.8
LnGrp LOS	B	C	C	B	C	C	D		D	D		F
Approach Vol, veh/h		1030			812			406			318	
Approach Delay, s/veh		23.9			21.1			44.9			78.8	
Approach LOS		C			C			D			E	

Timer - Assigned Phs	1	2		4	5	6	7	8
Phs Duration (G+Y+Rc), s	12.2	80.1		47.7	12.7	79.6	17.5	30.2
Change Period (Y+Rc), s	7.5	7.5		6.1	7.5	7.5	6.1	6.1
Max Green Setting (Gmax), s	8.5	64.5		45.9	13.5	59.5	12.9	26.9
Max Q Clear Time (g_c+I1), s	4.6	18.9		19.6	5.3	26.5	11.4	23.6
Green Ext Time (p_c), s	0.0	5.2		1.6	0.1	7.4	0.1	0.6

Intersection Summary		
HCM 7th Control Delay, s/veh		33.1
HCM 7th LOS		C

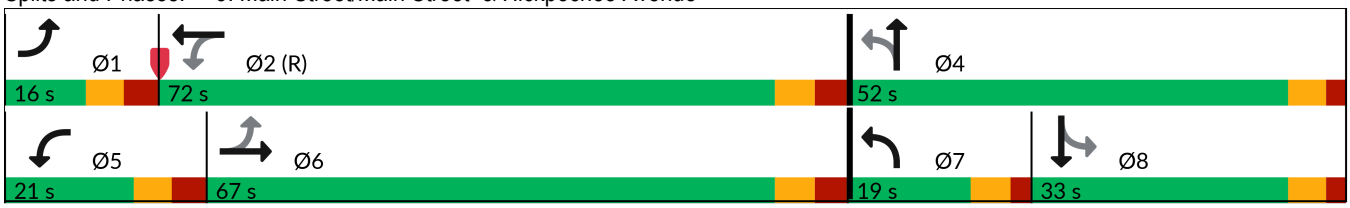
APPENDIX E

BACKGROUND CONDITIONS SYNCHRO

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	40	638	138	810	141	132	70	198
Future Volume (vph)	40	638	138	810	141	132	70	198
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	NA
Protected Phases	1	6	5	2	7	4		8
Permitted Phases	6		2		4		8	
Detector Phase	1	6	5	2	7	4	8	8
Switch Phase								
Minimum Initial (s)	5.0	12.0	5.0	12.0	5.0	7.0	7.0	7.0
Minimum Split (s)	12.5	23.5	12.5	23.5	11.1	22.1	22.1	22.1
Total Split (s)	16.0	67.0	21.0	72.0	19.0	52.0	33.0	33.0
Total Split (%)	11.4%	47.9%	15.0%	51.4%	13.6%	37.1%	23.6%	23.6%
Yellow Time (s)	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1
All-Red Time (s)	3.4	3.4	3.4	3.4	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.5	7.5	7.5	7.5	6.1	6.1	6.1	6.1
Lead/Lag	Lead	Lag	Lead	Lag	Lead		Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes
Recall Mode	None	Max	None	C-Max	None	None	None	None
Act Effct Green (s)	70.3	63.4	79.0	69.8	44.8	44.8	26.3	26.3
Actuated g/C Ratio	0.50	0.45	0.56	0.50	0.32	0.32	0.19	0.19
v/c Ratio	0.14	0.47	0.38	0.52	0.70	0.38	0.34	0.94
Control Delay (s/veh)	14.7	28.0	16.7	26.1	53.0	34.9	54.1	88.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	14.7	28.0	16.7	26.1	53.0	34.9	54.1	88.4
LOS	B	C	B	C	D	C	D	F
Approach Delay (s/veh)		27.3		24.8		42.2		82.1
Approach LOS		C		C		D		F

Intersection Summary	
Cycle Length: 140	
Actuated Cycle Length: 140	
Offset: 0 (0%), Referenced to phase 2:WBTL, Start of Green	
Natural Cycle: 80	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.94	
Intersection Signal Delay (s/veh): 36.7	Intersection LOS: D
Intersection Capacity Utilization 76.1%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 3: Main Street/Main Street & Hickpochee Avenue



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	42	746	145	916	148	220	74	325
v/c Ratio	0.14	0.47	0.38	0.52	0.70	0.38	0.34	0.94
Control Delay (s/veh)	14.7	28.0	16.7	26.1	53.0	34.9	54.1	88.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	14.7	28.0	16.7	26.1	53.0	34.9	54.1	88.4
Queue Length 50th (ft)	16	245	59	305	99	137	59	279
Queue Length 95th (ft)	34	315	94	378	#162	213	110	#463
Internal Link Dist (ft)		990		1144		978		957
Turn Bay Length (ft)								
Base Capacity (vph)	313	1584	409	1750	217	591	222	353
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.13	0.47	0.35	0.52	0.68	0.37	0.33	0.92

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	40	638	70	138	810	60	141	132	77	70	198	111
Future Volume (veh/h)	40	638	70	138	810	60	141	132	77	70	198	111
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	42	672	63	145	853	52	148	139	70	74	208	106
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	314	1575	148	401	1726	105	209	367	185	275	222	113
Arrive On Green	0.03	0.48	0.48	0.06	0.51	0.51	0.08	0.31	0.31	0.19	0.19	0.19
Sat Flow, veh/h	1781	3284	308	1781	3402	207	1781	1173	591	1173	1168	595
Grp Volume(v), veh/h	42	363	372	145	445	460	148	0	209	74	0	314
Grp Sat Flow(s),veh/h/ln	1781	1777	1815	1781	1777	1833	1781	0	1764	1173	0	1763
Q Serve(g_s), s	1.7	18.7	18.8	5.8	23.1	23.1	9.1	0.0	12.9	7.6	0.0	24.6
Cycle Q Clear(g_c), s	1.7	18.7	18.8	5.8	23.1	23.1	9.1	0.0	12.9	7.6	0.0	24.6
Prop In Lane	1.00		0.17	1.00		0.11	1.00		0.33	1.00		0.34
Lane Grp Cap(c), veh/h	314	852	871	401	901	930	209	0	553	275	0	336
V/C Ratio(X)	0.13	0.43	0.43	0.36	0.49	0.49	0.71	0.00	0.38	0.27	0.00	0.94
Avail Cap(c_a), veh/h	371	852	871	473	901	930	231	0	578	277	0	339
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	18.7	23.8	23.8	18.0	22.7	22.7	42.0	0.0	37.5	49.0	0.0	55.8
Incr Delay (d2), s/veh	0.2	1.6	1.5	0.5	1.9	1.9	8.6	0.0	0.4	0.5	0.0	32.5
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	1.3	13.0	13.3	4.4	15.4	15.8	8.0	0.0	9.6	4.1	0.0	20.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	18.9	25.4	25.4	18.5	24.6	24.6	50.6	0.0	37.9	49.5	0.0	88.4
LnGrp LOS	B	C	C	B	C	C	D		D	D		F
Approach Vol, veh/h		777			1050			357			388	
Approach Delay, s/veh		25.0			23.7			43.1			81.0	
Approach LOS		C			C			D			F	

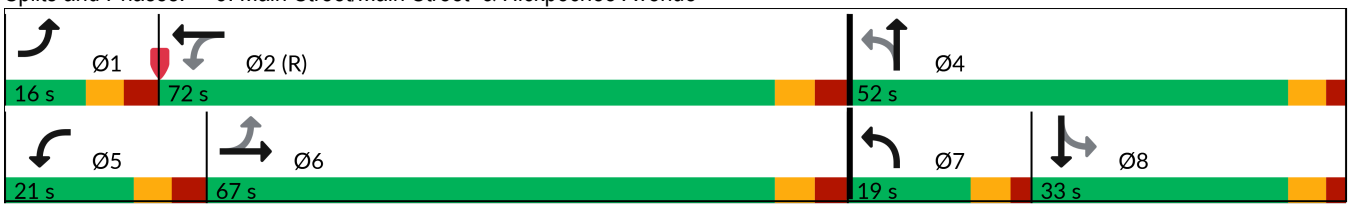
Timer - Assigned Phs	1	2		4	5	6	7	8
Phs Duration (G+Y+Rc), s	11.5	78.5		50.0	15.4	74.7	17.2	32.7
Change Period (Y+Rc), s	7.5	7.5		6.1	7.5	7.5	6.1	6.1
Max Green Setting (Gmax), s	8.5	64.5		45.9	13.5	59.5	12.9	26.9
Max Q Clear Time (g_c+I1), s	3.7	25.1		14.9	7.8	20.8	11.1	26.6
Green Ext Time (p_c), s	0.0	7.0		1.3	0.2	5.3	0.1	0.1

Intersection Summary	
HCM 7th Control Delay, s/veh	35.5
HCM 7th LOS	D

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	84	1085	106	825	179	127	47	237
Future Volume (vph)	84	1085	106	825	179	127	47	237
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	NA
Protected Phases	1	6	5	2	7	4		8
Permitted Phases	6		2		4		8	
Detector Phase	1	6	5	2	7	4	8	8
Switch Phase								
Minimum Initial (s)	5.0	12.0	5.0	12.0	5.0	7.0	7.0	7.0
Minimum Split (s)	12.5	23.5	12.5	23.5	11.1	22.1	22.1	22.1
Total Split (s)	16.0	67.0	21.0	72.0	19.0	52.0	33.0	33.0
Total Split (%)	11.4%	47.9%	15.0%	51.4%	13.6%	37.1%	23.6%	23.6%
Yellow Time (s)	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1
All-Red Time (s)	3.4	3.4	3.4	3.4	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.5	7.5	7.5	7.5	6.1	6.1	6.1	6.1
Lead/Lag	Lead	Lag	Lead	Lag	Lead		Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes
Recall Mode	None	Max	None	C-Max	None	None	None	None
Act Effct Green (s)	71.0	63.1	75.0	65.1	45.9	45.9	26.9	26.9
Actuated g/C Ratio	0.51	0.45	0.54	0.47	0.33	0.33	0.19	0.19
v/c Ratio	0.31	0.77	0.54	0.57	0.87	0.57	0.25	1.03
Control Delay (s/veh)	16.8	36.7	24.0	28.8	72.2	36.2	51.9	108.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	16.8	36.7	24.0	28.8	72.2	36.2	51.9	108.3
LOS	B	D	C	C	E	D	D	F
Approach Delay (s/veh)		35.4		28.3		49.1		101.6
Approach LOS		D		C		D		F

Intersection Summary	
Cycle Length: 140	
Actuated Cycle Length: 140	
Offset: 0 (0%), Referenced to phase 2:WBTL, Start of Green	
Natural Cycle: 90	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 1.03	
Intersection Signal Delay (s/veh): 43.7	Intersection LOS: D
Intersection Capacity Utilization 89.9%	ICU Level of Service E
Analysis Period (min) 15	

Splits and Phases: 3: Main Street/Main Street & Hickpochee Avenue



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	88	1221	112	924	188	338	49	364
v/c Ratio	0.31	0.77	0.54	0.57	0.87	0.57	0.25	1.03
Control Delay (s/veh)	16.8	36.7	24.0	28.8	72.2	36.2	51.9	108.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	16.8	36.7	24.0	28.8	72.2	36.2	51.9	108.3
Queue Length 50th (ft)	34	480	44	316	129	211	38	~343
Queue Length 95th (ft)	60	603	76	383	#260	315	79	#548
Internal Link Dist (ft)		990		1144		978		957
Turn Bay Length (ft)								
Base Capacity (vph)	292	1583	255	1633	216	594	199	353
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.30	0.77	0.44	0.57	0.87	0.57	0.25	1.03

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	84	1085	75	106	825	53	179	127	194	47	237	109
Future Volume (veh/h)	84	1085	75	106	825	53	179	127	194	47	237	109
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	88	1142	68	112	868	45	188	134	193	49	249	104
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	309	1618	96	232	1659	86	216	227	327	227	241	101
Arrive On Green	0.04	0.47	0.47	0.05	0.48	0.48	0.09	0.33	0.33	0.19	0.19	0.19
Sat Flow, veh/h	1781	3408	203	1781	3437	178	1781	693	998	1053	1253	523
Grp Volume(v), veh/h	88	595	615	112	449	464	188	0	327	49	0	353
Grp Sat Flow(s),veh/h/ln	1781	1777	1834	1781	1777	1838	1781	0	1691	1053	0	1776
Q Serve(g_s), s	3.5	37.0	37.1	4.5	24.5	24.5	11.6	0.0	22.6	5.7	0.0	26.9
Cycle Q Clear(g_c), s	3.5	37.0	37.1	4.5	24.5	24.5	11.6	0.0	22.6	9.3	0.0	26.9
Prop In Lane	1.00		0.11	1.00		0.10	1.00		0.59	1.00		0.29
Lane Grp Cap(c), veh/h	309	843	871	232	858	888	216	0	554	227	0	341
V/C Ratio(X)	0.29	0.71	0.71	0.48	0.52	0.52	0.87	0.00	0.59	0.22	0.00	1.03
Avail Cap(c_a), veh/h	348	843	871	320	858	888	216	0	554	227	0	341
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	19.7	29.0	29.1	23.6	25.0	25.0	41.5	0.0	39.2	51.0	0.0	56.6
Incr Delay (d2), s/veh	0.5	4.9	4.8	1.6	2.3	2.2	29.9	0.0	1.7	0.5	0.0	57.9
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	2.7	23.6	24.2	3.6	16.3	16.7	11.2	0.0	14.8	2.8	0.0	24.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	20.2	34.0	33.9	25.2	27.3	27.2	71.4	0.0	40.9	51.5	0.0	114.4
LnGrp LOS	C	C	C	C	C	C	E		D	D		F
Approach Vol, veh/h		1298			1025			515			402	
Approach Delay, s/veh		33.0			27.1			52.0			106.7	
Approach LOS		C			C			D			F	

Timer - Assigned Phs	1	2		4	5	6	7	8
Phs Duration (G+Y+Rc), s	12.9	75.1		52.0	14.0	74.0	19.0	33.0
Change Period (Y+Rc), s	7.5	7.5		6.1	7.5	7.5	6.1	6.1
Max Green Setting (Gmax), s	8.5	64.5		45.9	13.5	59.5	12.9	26.9
Max Q Clear Time (g_c+I1), s	5.5	26.5		24.6	6.5	39.1	13.6	28.9
Green Ext Time (p_c), s	0.0	7.1		2.1	0.1	8.6	0.0	0.0

Intersection Summary	
HCM 7th Control Delay, s/veh	43.3
HCM 7th LOS	D

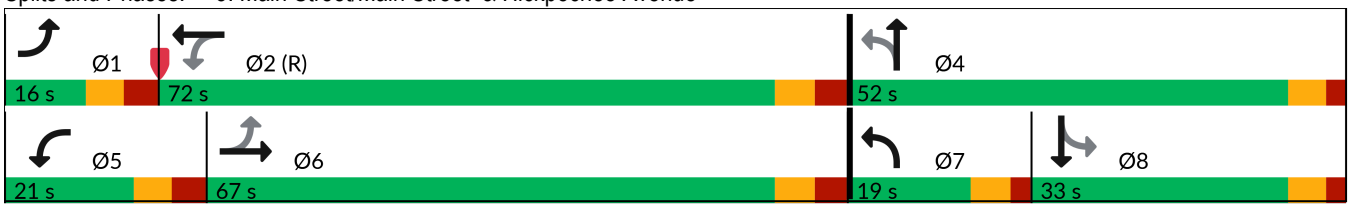
APPENDIX F

TOTAL TRAFFIC CONDITIONS SYNCHRO

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	76	638	138	824	155	153	105	233
Future Volume (vph)	76	638	138	824	155	153	105	233
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	NA
Protected Phases	1	6	5	2	7	4		8
Permitted Phases	6		2		4		8	
Detector Phase	1	6	5	2	7	4	8	8
Switch Phase								
Minimum Initial (s)	5.0	12.0	5.0	12.0	5.0	7.0	7.0	7.0
Minimum Split (s)	12.5	23.5	12.5	23.5	11.1	22.1	22.1	22.1
Total Split (s)	16.0	67.0	21.0	72.0	19.0	52.0	33.0	33.0
Total Split (%)	11.4%	47.9%	15.0%	51.4%	13.6%	37.1%	23.6%	23.6%
Yellow Time (s)	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1
All-Red Time (s)	3.4	3.4	3.4	3.4	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.5	7.5	7.5	7.5	6.1	6.1	6.1	6.1
Lead/Lag	Lead	Lag	Lead	Lag	Lead		Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes
Recall Mode	None	Max	None	C-Max	None	None	None	None
Act Effct Green (s)	70.0	62.3	76.0	65.2	45.9	45.9	27.1	27.1
Actuated g/C Ratio	0.50	0.45	0.54	0.47	0.33	0.33	0.19	0.19
v/c Ratio	0.29	0.48	0.39	0.58	0.77	0.41	0.51	1.03
Control Delay (s/veh)	16.7	28.6	17.1	29.0	58.5	36.0	59.9	106.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	16.7	28.6	17.1	29.0	58.5	36.0	59.9	106.9
LOS	B	C	B	C	E	D	E	F
Approach Delay (s/veh)		27.5		27.4		45.1		95.9
Approach LOS		C		C		D		F

Intersection Summary	
Cycle Length: 140	
Actuated Cycle Length: 140	
Offset: 0 (0%), Referenced to phase 2:WBTL, Start of Green	
Natural Cycle: 90	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 1.03	
Intersection Signal Delay (s/veh): 41.6	Intersection LOS: D
Intersection Capacity Utilization 79.9%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 3: Main Street/Main Street & Hickpochee Avenue



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	80	746	145	952	163	242	111	363
v/c Ratio	0.29	0.48	0.39	0.58	0.77	0.41	0.51	1.03
Control Delay (s/veh)	16.7	28.6	17.1	29.0	58.5	36.0	59.9	106.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	16.7	28.6	17.1	29.0	58.5	36.0	59.9	106.9
Queue Length 50th (ft)	31	245	59	327	110	157	92	~342
Queue Length 95th (ft)	57	315	94	397	#203	238	158	#547
Internal Link Dist (ft)		990		1144		978		957
Turn Bay Length (ft)								
Base Capacity (vph)	283	1555	406	1632	215	593	218	354
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.28	0.48	0.36	0.58	0.76	0.41	0.51	1.03

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	76	638	70	138	824	81	155	153	77	105	233	112
Future Volume (veh/h)	76	638	70	138	824	81	155	153	77	105	233	112
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	80	672	63	145	867	74	163	161	70	111	245	107
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	301	1546	145	395	1630	139	204	397	173	272	237	104
Arrive On Green	0.04	0.47	0.47	0.06	0.49	0.49	0.09	0.32	0.32	0.19	0.19	0.19
Sat Flow, veh/h	1781	3284	308	1781	3313	283	1781	1236	537	1149	1234	539
Grp Volume(v), veh/h	80	363	372	145	465	476	163	0	231	111	0	352
Grp Sat Flow(s),veh/h/ln	1781	1777	1815	1781	1777	1819	1781	0	1774	1149	0	1773
Q Serve(g_s), s	3.2	19.0	19.1	5.9	25.2	25.2	10.0	0.0	14.2	12.1	0.0	26.9
Cycle Q Clear(g_c), s	3.2	19.0	19.1	5.9	25.2	25.2	10.0	0.0	14.2	12.1	0.0	26.9
Prop In Lane	1.00		0.17	1.00		0.16	1.00		0.30	1.00		0.30
Lane Grp Cap(c), veh/h	301	836	854	395	874	895	204	0	570	272	0	341
V/C Ratio(X)	0.27	0.43	0.44	0.37	0.53	0.53	0.80	0.00	0.41	0.41	0.00	1.03
Avail Cap(c_a), veh/h	345	836	854	465	874	895	216	0	581	272	0	341
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	19.9	24.6	24.7	18.6	24.5	24.5	41.6	0.0	37.1	50.6	0.0	56.6
Incr Delay (d2), s/veh	0.5	1.6	1.6	0.6	2.3	2.3	18.0	0.0	0.5	1.0	0.0	57.6
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	2.5	13.2	13.5	4.5	16.7	17.0	9.3	0.0	10.4	6.4	0.0	24.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	20.3	26.3	26.3	19.2	26.8	26.8	59.6	0.0	37.5	51.5	0.0	114.1
LnGrp LOS	C	C	C	B	C	C	E		D	D		F
Approach Vol, veh/h		815			1086			394			463	
Approach Delay, s/veh		25.7			25.8			46.7			99.1	
Approach LOS		C			C			D			F	

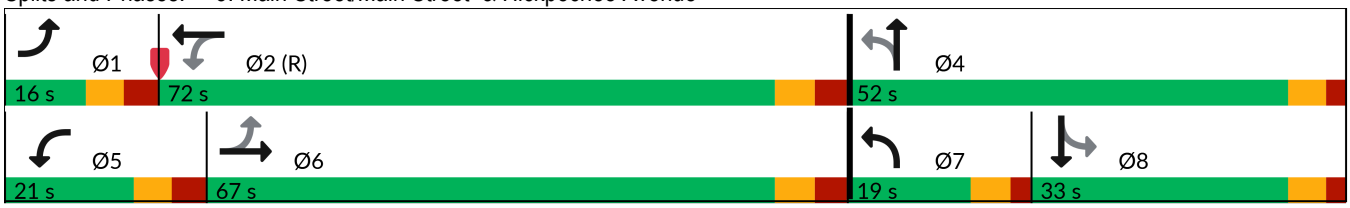
Timer - Assigned Phs	1	2		4	5	6	7	8
Phs Duration (G+Y+Rc), s	12.6	76.3		51.1	15.5	73.4	18.1	33.0
Change Period (Y+Rc), s	7.5	7.5		6.1	7.5	7.5	6.1	6.1
Max Green Setting (Gmax), s	8.5	64.5		45.9	13.5	59.5	12.9	26.9
Max Q Clear Time (g_c+I1), s	5.2	27.2		16.2	7.9	21.1	12.0	28.9
Green Ext Time (p_c), s	0.0	7.4		1.4	0.2	5.3	0.0	0.0

Intersection Summary	
HCM 7th Control Delay, s/veh	41.0
HCM 7th LOS	D

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	118	1085	106	839	193	147	80	270
Future Volume (vph)	118	1085	106	839	193	147	80	270
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	NA
Protected Phases	1	6	5	2	7	4		8
Permitted Phases	6		2		4		8	
Detector Phase	1	6	5	2	7	4	8	8
Switch Phase								
Minimum Initial (s)	5.0	12.0	5.0	12.0	5.0	7.0	7.0	7.0
Minimum Split (s)	12.5	23.5	12.5	23.5	11.1	22.1	22.1	22.1
Total Split (s)	16.0	67.0	21.0	72.0	19.0	52.0	33.0	33.0
Total Split (%)	11.4%	47.9%	15.0%	51.4%	13.6%	37.1%	23.6%	23.6%
Yellow Time (s)	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1
All-Red Time (s)	3.4	3.4	3.4	3.4	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.5	7.5	7.5	7.5	6.1	6.1	6.1	6.1
Lead/Lag	Lead	Lag	Lead	Lag	Lead		Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes
Recall Mode	None	Max	None	C-Max	None	None	None	None
Act Effct Green (s)	71.4	63.1	74.6	64.7	45.9	45.9	26.9	26.9
Actuated g/C Ratio	0.51	0.45	0.53	0.46	0.33	0.33	0.19	0.19
v/c Ratio	0.45	0.77	0.54	0.59	0.94	0.61	0.43	1.14
Control Delay (s/veh)	19.8	36.7	23.9	29.5	85.4	38.7	57.7	138.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	19.8	36.7	23.9	29.5	85.4	38.7	57.7	138.6
LOS	B	D	C	C	F	D	E	F
Approach Delay (s/veh)		35.1		28.9		55.6		124.5
Approach LOS		D		C		E		F

Intersection Summary	
Cycle Length: 140	
Actuated Cycle Length: 140	
Offset: 0 (0%), Referenced to phase 2:WBTL, Start of Green	
Natural Cycle: 90	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 1.14	
Intersection Signal Delay (s/veh): 49.0	Intersection LOS: D
Intersection Capacity Utilization 92.5%	ICU Level of Service F
Analysis Period (min) 15	

Splits and Phases: 3: Main Street/Main Street & Hickpochee Avenue



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	124	1221	112	960	203	359	84	400
v/c Ratio	0.45	0.77	0.54	0.59	0.94	0.61	0.43	1.14
Control Delay (s/veh)	19.8	36.7	23.9	29.5	85.4	38.7	57.7	138.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	19.8	36.7	23.9	29.5	85.4	38.7	57.7	138.6
Queue Length 50th (ft)	50	480	44	333	140	237	68	~414
Queue Length 95th (ft)	81	603	76	402	#294	347	125	#625
Internal Link Dist (ft)		990		1144		978		957
Turn Bay Length (ft)								
Base Capacity (vph)	276	1583	255	1621	216	592	195	352
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.45	0.77	0.44	0.59	0.94	0.61	0.43	1.14

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	118	1085	75	106	839	73	193	147	194	80	270	110
Future Volume (veh/h)	118	1085	75	106	839	73	193	147	194	80	270	110
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	124	1142	68	112	883	66	203	155	193	84	284	105
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	307	1617	96	232	1580	118	216	248	309	211	250	93
Arrive On Green	0.05	0.47	0.47	0.05	0.47	0.47	0.09	0.33	0.33	0.19	0.19	0.19
Sat Flow, veh/h	1781	3408	203	1781	3352	251	1781	757	943	1033	1302	481
Grp Volume(v), veh/h	124	595	615	112	468	481	203	0	348	84	0	389
Grp Sat Flow(s),veh/h/ln	1781	1777	1834	1781	1777	1825	1781	0	1701	1033	0	1784
Q Serve(g_s), s	5.0	37.0	37.1	4.5	26.5	26.5	12.6	0.0	24.2	10.5	0.0	26.9
Cycle Q Clear(g_c), s	5.0	37.0	37.1	4.5	26.5	26.5	12.6	0.0	24.2	15.7	0.0	26.9
Prop In Lane	1.00		0.11	1.00		0.14	1.00		0.55	1.00		0.27
Lane Grp Cap(c), veh/h	307	843	870	232	838	861	216	0	558	211	0	343
V/C Ratio(X)	0.40	0.71	0.71	0.48	0.56	0.56	0.94	0.00	0.62	0.40	0.00	1.14
Avail Cap(c_a), veh/h	326	843	870	320	838	861	216	0	558	211	0	343
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	20.4	29.1	29.1	23.7	26.5	26.5	41.9	0.0	39.8	54.4	0.0	56.6
Incr Delay (d2), s/veh	0.9	4.9	4.8	1.6	2.7	2.6	45.1	0.0	2.2	1.2	0.0	90.4
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	3.9	23.6	24.2	3.6	17.5	17.9	13.0	0.0	15.8	5.1	0.0	30.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	21.3	34.0	33.9	25.2	29.2	29.2	86.9	0.0	41.9	55.6	0.0	147.0
LnGrp LOS	C	C	C	C	C	C	F		D	E		F
Approach Vol, veh/h		1334			1061			551			473	
Approach Delay, s/veh		32.8			28.8			58.5			130.8	
Approach LOS		C			C			E			F	

Timer - Assigned Phs	1	2		4	5	6	7	8
Phs Duration (G+Y+Rc), s	14.5	73.5		52.0	14.1	73.9	19.0	33.0
Change Period (Y+Rc), s	7.5	7.5		6.1	7.5	7.5	6.1	6.1
Max Green Setting (Gmax), s	8.5	64.5		45.9	13.5	59.5	12.9	26.9
Max Q Clear Time (g_c+I1), s	7.0	28.5		26.2	6.5	39.1	14.6	28.9
Green Ext Time (p_c), s	0.0	7.4		2.1	0.1	8.6	0.0	0.0

Intersection Summary		
HCM 7th Control Delay, s/veh		49.2
HCM 7th LOS		D

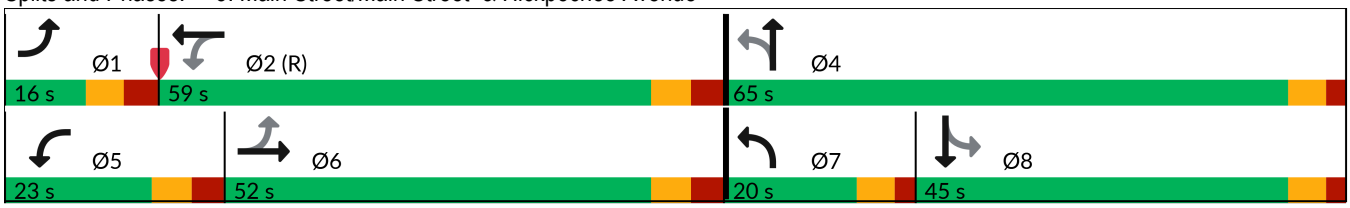
APPENDIX G

OPTIMIZED CONDITIONS SYNCHRO

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	76	638	138	824	155	153	105	233
Future Volume (vph)	76	638	138	824	155	153	105	233
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	NA
Protected Phases	1	6	5	2	7	4		8
Permitted Phases	6		2		4		8	
Detector Phase	1	6	5	2	7	4	8	8
Switch Phase								
Minimum Initial (s)	5.0	12.0	5.0	12.0	5.0	7.0	7.0	7.0
Minimum Split (s)	12.5	23.5	12.5	23.5	11.1	22.1	22.1	22.1
Total Split (s)	16.0	52.0	23.0	59.0	20.0	65.0	45.0	45.0
Total Split (%)	11.4%	37.1%	16.4%	42.1%	14.3%	46.4%	32.1%	32.1%
Yellow Time (s)	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1
All-Red Time (s)	3.4	3.4	3.4	3.4	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.5	7.5	7.5	7.5	6.1	6.1	6.1	6.1
Lead/Lag	Lead	Lag	Lead	Lag	Lead		Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes
Recall Mode	None	Max	None	C-Max	None	None	None	None
Act Effct Green (s)	63.6	55.5	70.4	58.9	51.9	51.9	32.4	32.4
Actuated g/C Ratio	0.45	0.40	0.50	0.42	0.37	0.37	0.23	0.23
v/c Ratio	0.33	0.54	0.43	0.65	0.65	0.36	0.43	0.86
Control Delay (s/veh)	22.3	35.6	22.3	35.9	41.7	29.5	49.7	68.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	22.3	35.6	22.3	35.9	41.7	29.5	49.7	68.5
LOS	C	D	C	D	D	C	D	E
Approach Delay (s/veh)		34.3		34.1		34.4		64.1
Approach LOS		C		C		C		E

Intersection Summary	
Cycle Length: 140	
Actuated Cycle Length: 140	
Offset: 0 (0%), Referenced to phase 2:WBTL, Start of Green	
Natural Cycle: 90	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.86	
Intersection Signal Delay (s/veh): 39.3	Intersection LOS: D
Intersection Capacity Utilization 79.9%	ICU Level of Service D
Analysis Period (min) 15	

Splits and Phases: 3: Main Street/Main Street & Hickpochee Avenue



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	80	746	145	952	163	242	111	363
v/c Ratio	0.33	0.54	0.43	0.65	0.65	0.36	0.43	0.86
Control Delay (s/veh)	22.3	35.6	22.3	35.9	41.7	29.5	49.7	68.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	22.3	35.6	22.3	35.9	41.7	29.5	49.7	68.5
Queue Length 50th (ft)	36	274	67	366	100	141	86	303
Queue Length 95th (ft)	70	378	117	472	146	200	140	407
Internal Link Dist (ft)		990		1144		978		957
Turn Bay Length (ft)								
Base Capacity (vph)	251	1387	387	1473	256	757	314	504
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.32	0.54	0.37	0.65	0.64	0.32	0.35	0.72

Intersection Summary

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	76	638	70	138	824	81	155	153	77	105	233	112
Future Volume (veh/h)	76	638	70	138	824	81	155	153	77	105	233	112
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	80	672	63	145	867	74	163	161	70	111	245	107
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	281	1451	136	373	1538	131	229	430	187	305	272	119
Arrive On Green	0.04	0.44	0.44	0.06	0.46	0.46	0.08	0.35	0.35	0.22	0.22	0.22
Sat Flow, veh/h	1781	3284	308	1781	3313	283	1781	1236	537	1149	1234	539
Grp Volume(v), veh/h	80	363	372	145	465	476	163	0	231	111	0	352
Grp Sat Flow(s),veh/h/ln	1781	1777	1815	1781	1777	1819	1781	0	1774	1149	0	1773
Q Serve(g_s), s	3.4	20.1	20.1	6.2	26.6	26.6	9.6	0.0	13.7	11.7	0.0	27.0
Cycle Q Clear(g_c), s	3.4	20.1	20.1	6.2	26.6	26.6	9.6	0.0	13.7	11.7	0.0	27.0
Prop In Lane	1.00		0.17	1.00		0.16	1.00		0.30	1.00		0.30
Lane Grp Cap(c), veh/h	281	785	802	373	825	845	229	0	617	305	0	391
V/C Ratio(X)	0.28	0.46	0.46	0.39	0.56	0.56	0.71	0.00	0.37	0.36	0.00	0.90
Avail Cap(c_a), veh/h	323	785	802	464	825	845	257	0	746	371	0	493
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	22.1	27.4	27.4	20.7	27.2	27.2	39.3	0.0	34.2	47.1	0.0	53.0
Incr Delay (d2), s/veh	0.5	2.0	1.9	0.7	2.8	2.7	7.9	0.0	0.4	0.7	0.0	16.6
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	2.7	14.0	14.2	4.8	17.6	18.0	8.3	0.0	10.1	6.2	0.0	20.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	22.7	29.4	29.3	21.4	30.0	29.9	47.2	0.0	34.6	47.8	0.0	69.7
LnGrp LOS	C	C	C	C	C	C	D		C	D		E
Approach Vol, veh/h		815			1086			394			463	
Approach Delay, s/veh		28.7			28.8			39.8			64.4	
Approach LOS		C			C			D			E	

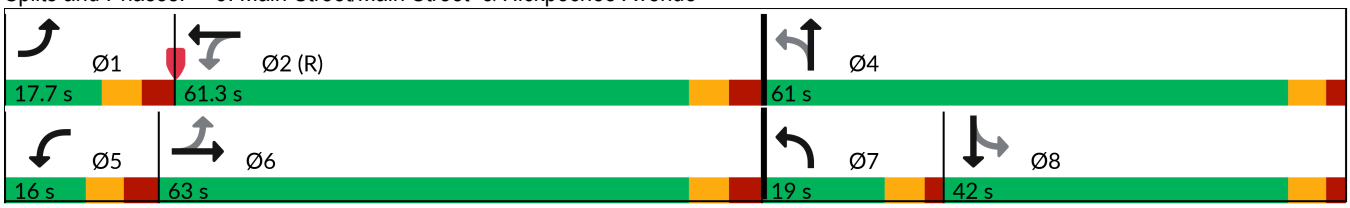
Timer - Assigned Phs	1	2		4	5	6	7	8
Phs Duration (G+Y+Rc), s	12.7	72.5		54.8	15.9	69.4	17.8	37.0
Change Period (Y+Rc), s	7.5	7.5		6.1	7.5	7.5	6.1	6.1
Max Green Setting (Gmax), s	8.5	51.5		58.9	15.5	44.5	13.9	38.9
Max Q Clear Time (g_c+I1), s	5.4	28.6		15.7	8.2	22.1	11.6	29.0
Green Ext Time (p_c), s	0.0	6.5		1.5	0.2	4.8	0.1	1.9

Intersection Summary		
HCM 7th Control Delay, s/veh		36.3
HCM 7th LOS		D

Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Configurations								
Traffic Volume (vph)	118	1085	106	839	193	147	80	270
Future Volume (vph)	118	1085	106	839	193	147	80	270
Turn Type	pm+pt	NA	pm+pt	NA	pm+pt	NA	Perm	NA
Protected Phases	1	6	5	2	7	4		8
Permitted Phases	6		2		4		8	
Detector Phase	1	6	5	2	7	4	8	8
Switch Phase								
Minimum Initial (s)	5.0	12.0	5.0	12.0	5.0	7.0	7.0	7.0
Minimum Split (s)	12.5	23.5	12.5	23.5	11.1	22.1	22.1	22.1
Total Split (s)	17.7	63.0	16.0	61.3	19.0	61.0	42.0	42.0
Total Split (%)	12.6%	45.0%	11.4%	43.8%	13.6%	43.6%	30.0%	30.0%
Yellow Time (s)	4.1	4.1	4.1	4.1	4.1	4.1	4.1	4.1
All-Red Time (s)	3.4	3.4	3.4	3.4	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	7.5	7.5	7.5	7.5	6.1	6.1	6.1	6.1
Lead/Lag	Lead	Lag	Lead	Lag	Lead		Lag	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes
Recall Mode	None	Max	None	C-Max	None	None	None	None
Act Effct Green (s)	67.4	58.0	65.4	57.0	52.5	52.5	33.5	33.5
Actuated g/C Ratio	0.48	0.41	0.47	0.41	0.38	0.38	0.24	0.24
v/c Ratio	0.50	0.84	0.67	0.67	0.89	0.53	0.35	0.92
Control Delay (s/veh)	25.2	43.5	43.3	37.1	70.2	31.3	47.8	76.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	25.2	43.5	43.3	37.1	70.2	31.3	47.8	76.4
LOS	C	D	D	D	E	C	D	E
Approach Delay (s/veh)		41.8		37.8		45.3		71.4
Approach LOS		D		D		D		E

Intersection Summary	
Cycle Length: 140	
Actuated Cycle Length: 140	
Offset: 0 (0%), Referenced to phase 2:WBTL, Start of Green	
Natural Cycle: 90	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.92	
Intersection Signal Delay (s/veh): 45.3	Intersection LOS: D
Intersection Capacity Utilization 92.5%	ICU Level of Service F
Analysis Period (min) 15	

Splits and Phases: 3: Main Street/Main Street & Hickpochee Avenue



Lane Group	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Lane Group Flow (vph)	124	1221	112	960	203	359	84	400
v/c Ratio	0.50	0.84	0.67	0.67	0.89	0.53	0.35	0.92
Control Delay (s/veh)	25.2	43.5	43.3	37.1	70.2	31.3	47.8	76.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	25.2	43.5	43.3	37.1	70.2	31.3	47.8	76.4
Queue Length 50th (ft)	58	534	52	384	126	208	62	338
Queue Length 95th (ft)	95	635	#128	464	#265	306	114	#513
Internal Link Dist (ft)		990		1144		978		957
Turn Bay Length (ft)								
Base Capacity (vph)	256	1455	170	1428	227	702	261	467
Starvation Cap Reductn	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.48	0.84	0.66	0.67	0.89	0.51	0.32	0.86

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

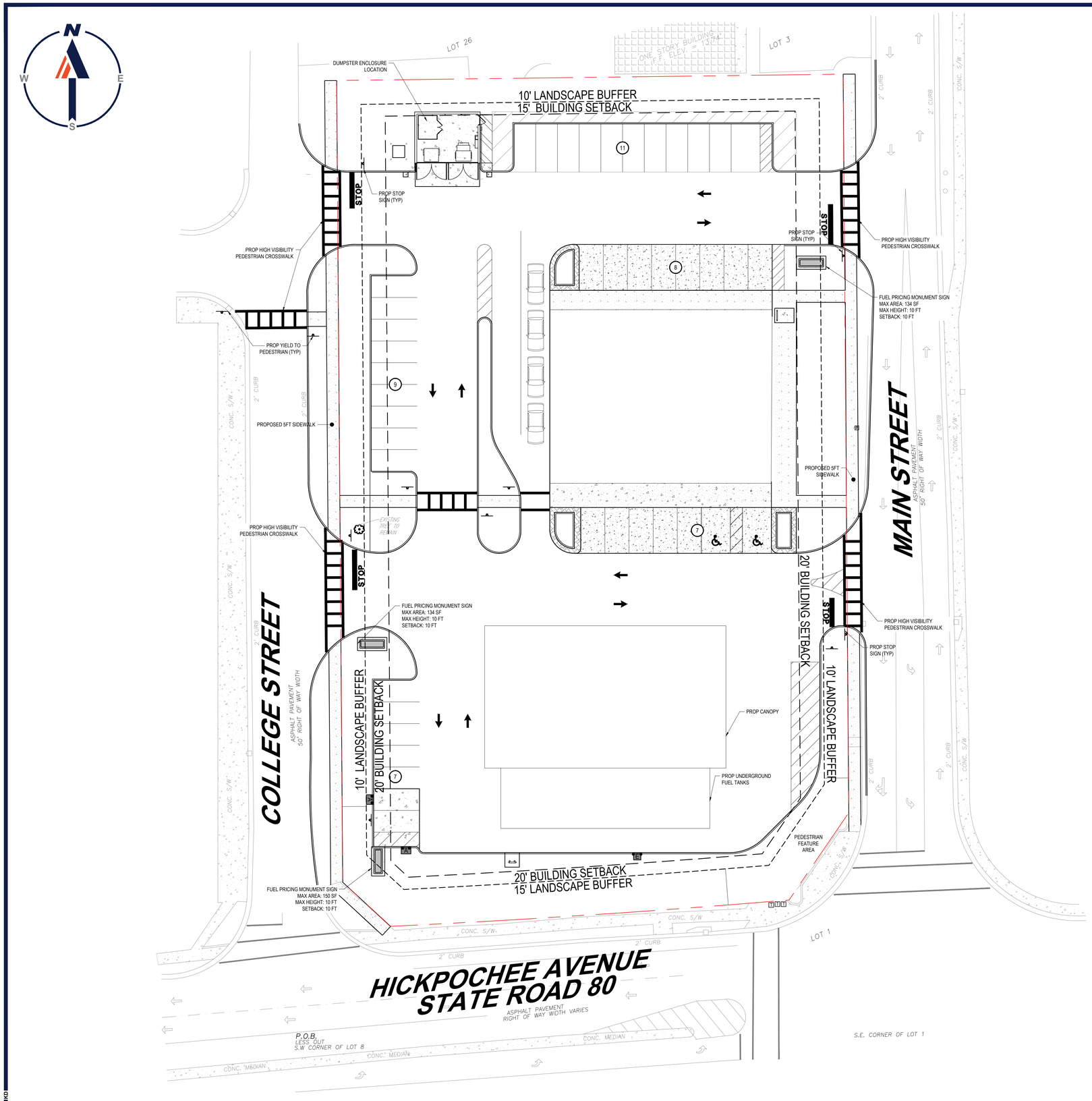
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	118	1085	75	106	839	73	193	147	194	80	270	110
Future Volume (veh/h)	118	1085	75	106	839	73	193	147	194	80	270	110
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width Adj.	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	124	1142	68	112	883	66	203	155	193	84	284	105
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	276	1463	87	204	1424	106	237	281	350	267	306	113
Arrive On Green	0.05	0.43	0.43	0.05	0.43	0.43	0.09	0.37	0.37	0.24	0.24	0.24
Sat Flow, veh/h	1781	3408	203	1781	3352	251	1781	757	943	1033	1302	481
Grp Volume(v), veh/h	124	595	615	112	468	481	203	0	348	84	0	389
Grp Sat Flow(s),veh/h/ln	1781	1777	1834	1781	1777	1825	1781	0	1701	1033	0	1784
Q Serve(g_s), s	5.5	40.2	40.3	4.9	28.8	28.8	11.9	0.0	22.7	9.8	0.0	29.9
Cycle Q Clear(g_c), s	5.5	40.2	40.3	4.9	28.8	28.8	11.9	0.0	22.7	13.5	0.0	29.9
Prop In Lane	1.00		0.11	1.00		0.14	1.00		0.55	1.00		0.27
Lane Grp Cap(c), veh/h	276	763	787	204	755	776	237	0	631	267	0	420
V/C Ratio(X)	0.45	0.78	0.78	0.55	0.62	0.62	0.86	0.00	0.55	0.31	0.00	0.93
Avail Cap(c_a), veh/h	311	763	787	225	755	776	237	0	667	289	0	457
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.2	34.3	34.3	28.1	31.4	31.4	38.4	0.0	34.8	47.7	0.0	52.4
Incr Delay (d2), s/veh	1.1	7.8	7.6	2.3	3.8	3.7	25.0	0.0	0.9	0.7	0.0	24.1
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(95%),veh/ln	4.3	26.1	26.8	4.0	19.1	19.5	11.1	0.0	14.7	4.7	0.0	22.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	25.4	42.1	41.9	30.4	35.2	35.1	63.5	0.0	35.7	48.3	0.0	76.5
LnGrp LOS	C	D	D	C	D	D	E		D	D		E
Approach Vol, veh/h		1334			1061			551			473	
Approach Delay, s/veh		40.4			34.7			45.9			71.5	
Approach LOS		D			C			D			E	

Timer - Assigned Phs	1	2		4	5	6	7	8
Phs Duration (G+Y+Rc), s	15.0	67.0		58.0	14.4	67.6	19.0	39.0
Change Period (Y+Rc), s	7.5	7.5		6.1	7.5	7.5	6.1	6.1
Max Green Setting (Gmax), s	10.2	53.8		54.9	8.5	55.5	12.9	35.9
Max Q Clear Time (g_c+I1), s	7.5	30.8		24.7	6.9	42.3	13.9	31.9
Green Ext Time (p_c), s	0.1	6.6		2.4	0.0	6.7	0.0	1.1

Intersection Summary		
HCM 7th Control Delay, s/veh		43.8
HCM 7th LOS		D

APPENDIX H

SITE PLAN



LOCATION MAP
SCALE: 1" = 300'
SOURCE: MICROSOFT BING

SITE DATA TABLE

JURISDICTION	CITY OF LABELLE
CURRENT ZONING	PUD
LAND USE DESIGNATION	COMMERCIAL
PARCEL AREA	± 1.58 AC
BUILDING AREA	7,000 SF
CANOPY AREA	5,900 SF
IMPERVIOUS AREA	MAXIMUM 63%
FRONT SETBACK (ROW)	20 FT
SIDE SETBACK	20 FT
REAR SETBACK	15 FT
FRONT/SIDE/REAR YARD BUFFER	15 FT / 10 FT / 10 FT
OPEN SPACE	20%
PARKING COUNT	42 STALLS
BUILDING HEIGHT	30 FT
CANOPY HEIGHT	26 FT

THIS PLAN TO BE UTILIZED FOR SITE LAYOUT PURPOSES ONLY

20 10 5 0 20
SCALE: 1" = 20'

BOHLER™

SITE CIVIL AND CONSULTING ENGINEERING
LAND SURVEYING
PROGRAM MANAGEMENT
LANDSCAPE ARCHITECTURE
SUSTAINABLE DESIGN
PERMITTING SERVICES
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REV	DATE	COMMENT	DRAWN BY / CHECKED BY

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Call 811 or visit sunshine811.com two full business days before digging to have buried facilities located and marked.
Check positive response codes before you dig!

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PROJECT No.:	FLC240030.00-0A
DRAWN BY:	LB
CHECKED BY:	TP
DATE:	01/30/2025
CAD I.D.:	P-CIVL-SITE

PROJECT:

LAND DEVELOPMENT PLANS

FOR

WAWA

PROPOSED COMMERCIAL DEVELOPMENT

10 W HICKPOCHEE AVE
LABELLE, FL 33935
CITY OF LABELLE
HENDRY COUNTY

BOHLER
1 SE 3rd AVENUE
SUITE 2700
MIAMI, FLORIDA 33131
Phone: (786) 681-0800
FLORIDA BUSINESS CERT. OF AUTH. No. 30780

SHEET TITLE:

CONCEPTUAL SITE PLAN

SHEET NUMBER:

C-301

ORG. DATE - 01/30/2025

H:\2024\FLC240030.00\CAD\DRAWINGS\PLAN SETS\CIVIL SITE PLANS\P-CIVL-SITE-FLC240030.00-0A----->LAYOUT: C-301 SITE