PROPOSED ADDITION FOR,
CLUB RITZ
301 W. 7TH ST.
KAUKAUNA, WI 54130

PROJECT INFORMATION

DESIGNERS OF RECORD

ARCHITECTURAL / STRUCTURAL
DANIEL J. MEISSNER AIA, LLC
1230 E. CALUMET ST.
APPLETON, WI 54915
920-428-0982

CODE ANALYSIS

USE AND OCCUPANCY

TYPE VB – WOOD FRAME
NON-SPRINKLED
A-2 ASSEMBLY – BAR
UNOCCUPIED BASEMENT

ALLOWABLE AREA

EXISTING BUILDING 2,000 S.F.
NEW ADDITION 1,840 S.F.
TOTAL BUILDING AREA 4,115 S.F.

IBC 2015 TABLE 506.2 – 6,000 S.F. ALLOWED

SHEET INDEX

SHEET	TITLE
T1.0	TITLE SHEET, SITE PLAN, NOTES
D1.0	EXISTING & DEMOLITION PLAN
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S1.1	EXTERIOR ELEVATIONS
S1.2	CROSS SECTION
S1.3	CROSS SECTION
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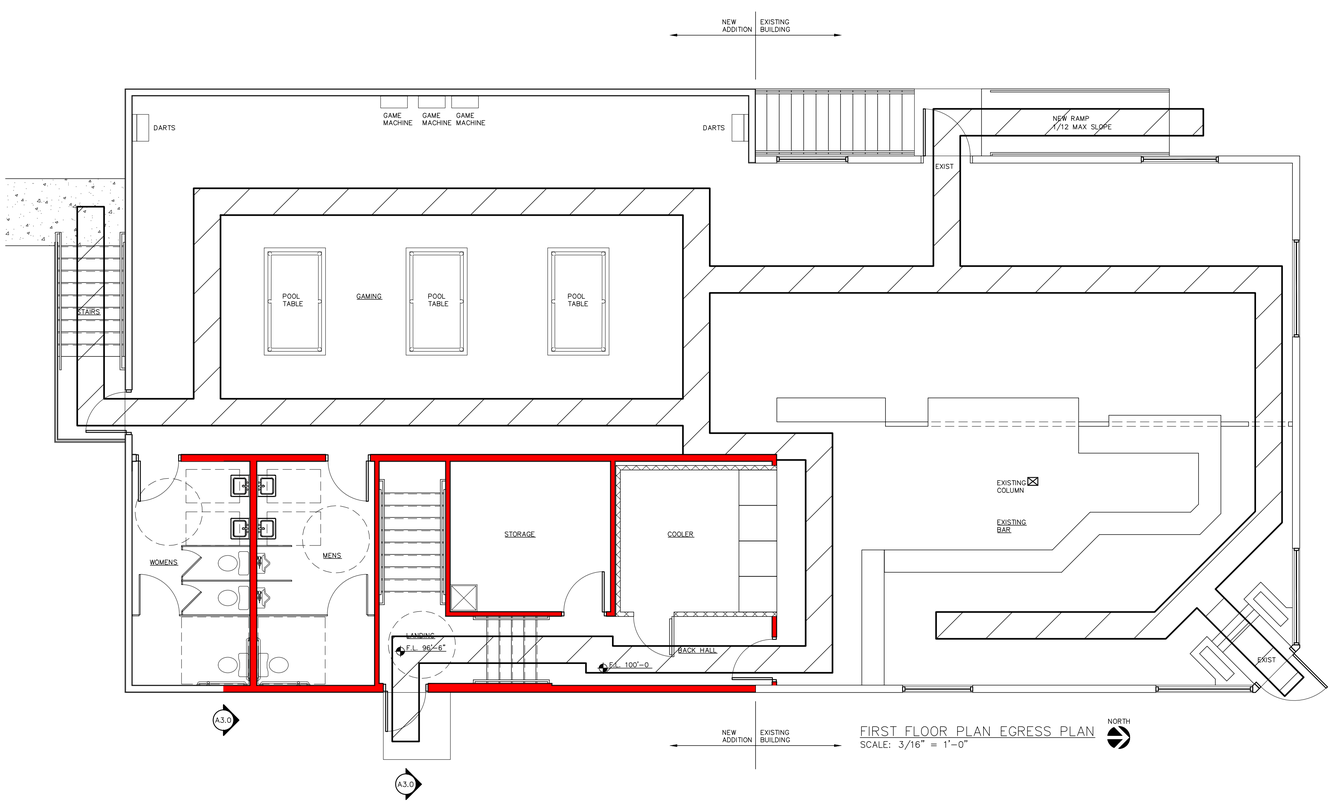
A1.0	FLOOR PLAN
A2.0	EXTERIOR ELEVATIONS
A3.0	CROSS SECTION
A4.0	ADA DETAILS
A5.0	DUMPSTER DETAILS

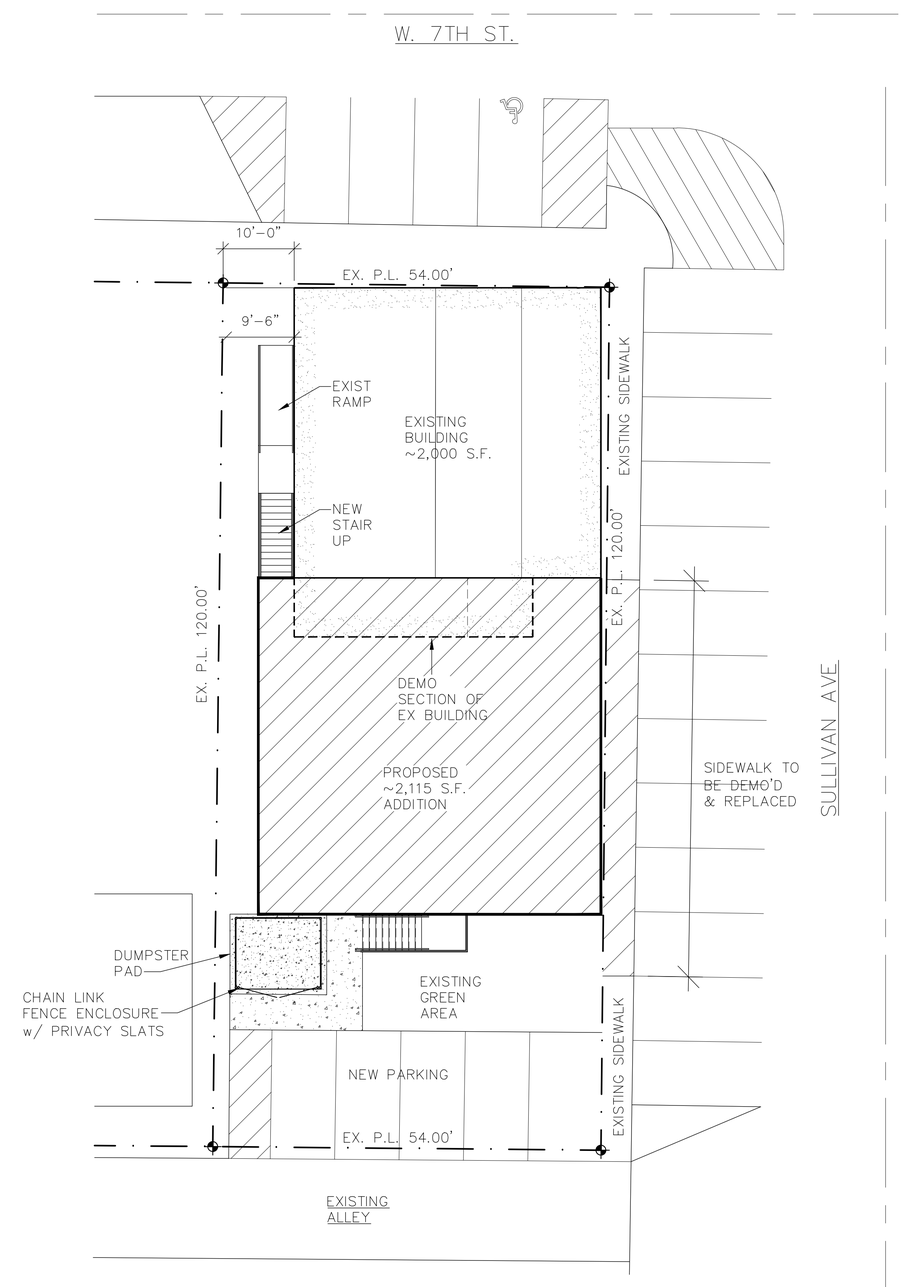
WISCONSIN DANIEL J. MEISSNER AIA 26009 APPLETON, WI ARCHITECT

5-30-25



FIRST FLOOR PLAN EGRESS PLAN
SCALE: 3/16" = 1'-0"

NORTH



SCHEMATIC SITE PLAN
SCALE: 1:20

NORTH

REV. NO.	DATE	DESCRIPTION

Architect:
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920.428.0982

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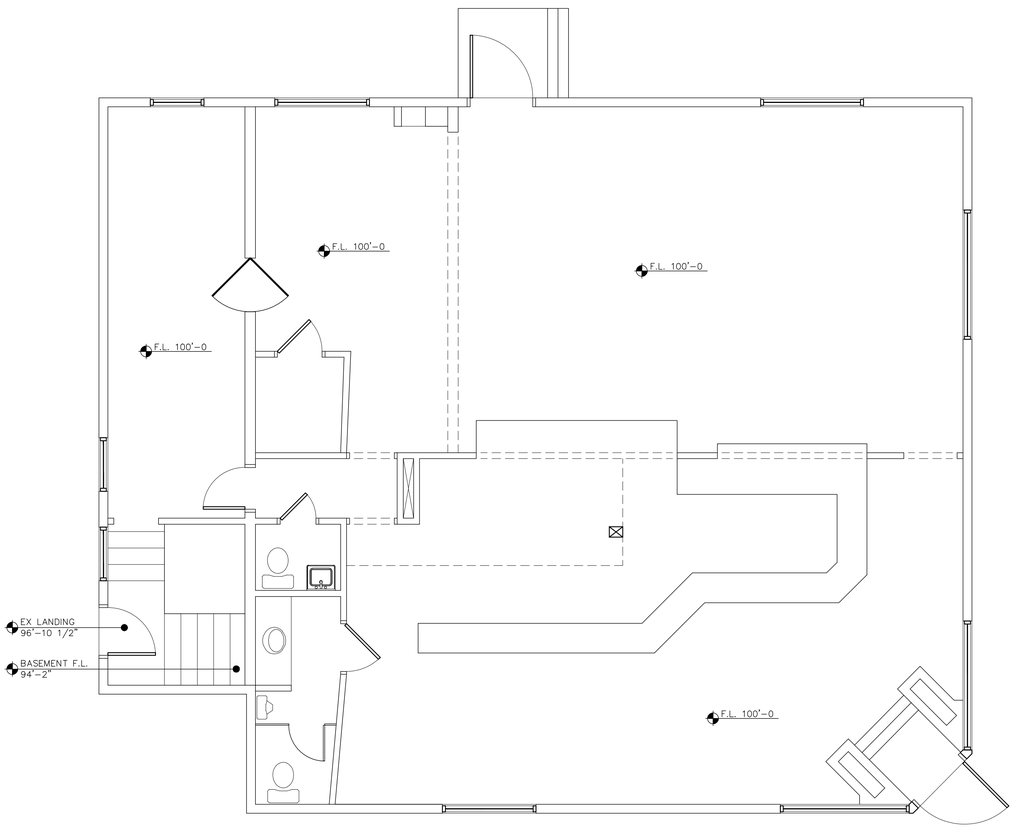
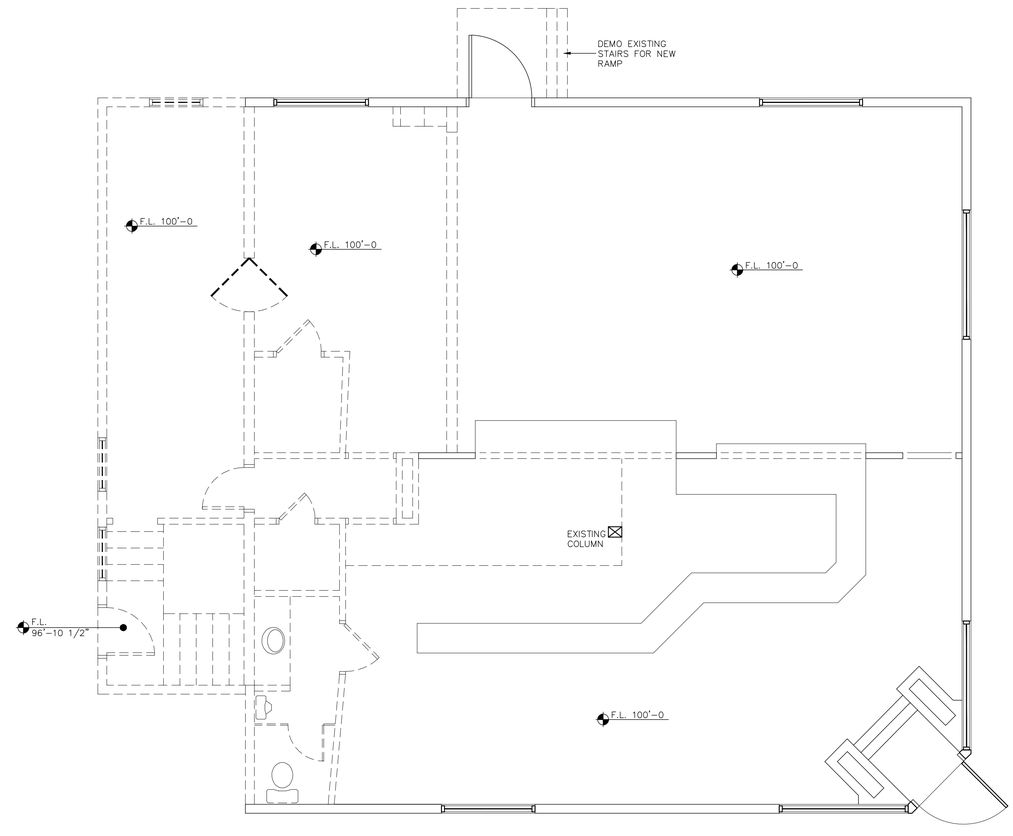
S. BURTON



EXISTING FIRST FLOOR PLAN
SCALE: 1/4" = 1'-0"

NORTH



FIRST FLOOR DEMOLITION PLAN
SCALE: 1/4" = 1'-0"

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GENERAL

1. ALL MATERIALS, WORKMANSHIP AND DETAILS SHALL CONFORM TO THE REQUIREMENTS OF THE 2015 EDITION OF THE INTERNATIONAL BUILDING CODE.
2. THE CONTRACTOR SHALL FAMILIARIZE HIMSELF WITH ALL ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS TO VERIFY THE LOCATION AND DIMENSIONS OF CHASES, INSERTS, OPENINGS, SLEEVES, REGLETS, DEPRESSIONS AND OTHER PROJECT REQUIREMENTS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
3. OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL NOT BE REVISED WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.
4. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS, ELEVATIONS AND CONDITIONS AND NOTIFY ARCHITECT OF ANY DISCREPANCIES.
5. THE TYPICAL DETAILS SHOWN ON THE DRAWINGS SHALL BE APPLICABLE TO ALL PARTS OF THE CONTRACT DRAWINGS UNLESS SPECIFICALLY NOTED OTHERWISE.
6. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR JOB SAFETY DURING CONSTRUCTION.

DESIGN LOADS

FLOOR LIVE LOADS:

1. OFFICES: 50 PSF (WITH 15 PSF ALLOWANCE FOR PARTITIONS)
2. CORRIDORS : 100 PSF
3. LOBBIES: 100 PSF

SNOW LOADS: GROUND SNOW LOAD: 40 PSF
EXPOSURE FACTOR Ce: 1.0
TEMP. FACTOR Ct: 1.1
IMPORTANCE FACTOR: 1.0
FLAT ROOF SNOW LOAD Pf: 30.8 PSF
DRIFTING: PER IBC, AS REQUIRED

WIND LOADS: ASCE 7–10
BASIC WIND SPEED: 115 MPH (ULTIMATE)

EXPOSURE: B
MAIN FORCE RESISTING SYSTEM WIND
WALLS
WINDWARD 14.0 PSF
LEEWARD –4.7 PSF
CORNERS
WINDWARD 19.4 PSF
LEEWARD –8.8 PSF
INTERNAL PRESSURE COEFFICIENT Gcpi= ±0.18

SEISMIC LOADS:
SEISMIC RISK CATEGORY: II
Sds 0.059
Sd1 0.056
SITE CLASS: D
SEISMIC DESIGN CATEGORY: A
BASIC SEISMIC FORCE RESISTING SYSTEM:
WOOD FRAMED SHEAR WALLS & DIAPHRAGM

FOUNDATIONS

1. FOUNDATION WORK FOR THIS PROJECT SHALL CONSIST OF SPREAD FOOTINGS, CONTINUOUS WALL FOOTINGS, FOUNDATION WALLS, AND SLABS–ON–GRADE.
2. FOUNDATIONS ARE DESIGNED TO BE SUPPORTED ON APPROVED EXISTING SUBGRADE OR APPROVED COMPACTED STRUCTURAL FILL HAVING A MINIMUM ALLOWABLE BEARING CAPACITY OF 2,000 PSF. (PRESUMED)
3. THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THE VALIDITY OF THE SUBSURFACE CONDITIONS DESCRIBED IN THE DRAWINGS, SPECIFICATIONS, TEST BORINGS OR GEOTECHNICAL REPORTS. THESE DATA ARE INCLUDED TO ASSIST THE CONTRACTOR DURING BIDDING AND SUBSEQUENT CONSTRUCTION AND TO REPRESENT CONDITIONS ONLY AT SPECIFIC LOCATIONS AT THE PARTICULAR TIME OBSERVATIONS WERE MADE.
4. ALL EXTERIOR FOUNDATIONS SHALL BEAR ON APPROVED SUBGRADE AT A MINIMUM DEPTH OF 4'–0" BELOW ADJACENT FINISH EXTERIOR GRADE.
5. FOOTING ELEVATIONS SHOWN ON THE DRAWINGS REPRESENT ESTIMATED DEPTHS AND ARE NOT TO BE CONSTRUED AS LIMITING THE AMOUNT OF EXCAVATION REQUIRED TO REACH SUITABLE BEARING MATERIAL.
6. CONTRACTOR SHALL PROVIDE TEMPORARY SUPPORTS AS REQUIRED TO PREVENT HORIZONTAL MOVEMENT OR VERTICAL SETTLEMENT WHICH WILL ENDANGER ADJACENT STRUCTURES, STREETS OR UTILITIES.
7. CONTRACTOR SHALL PROVIDE CONTROL OF SURFACE AND SUBSURFACE WATER PROMPTLY TO INSURE THAT ALL FOUNDATION WORK IS DONE IN THE DRY.
8. NO FOUNDATION(S) SHALL BE PLACED ON FROZEN SUBGRADE.
9. PROTECT IN–PLACE FOUNDATIONS AND SLABS–ON–GRADE FROM FROST PENETRATION UNTIL THE PROJECT IS COMPLETE.
10. FOUNDATION WALLS SHALL BE BRACED DURING BACKFILLING AND COMPACTION OPERATIONS. BRACING SHALL BE LEFT IN POSITION UNTIL PERMANENT STRUCTURAL SUPPORT SYSTEM IS INSTALLED AND APPROVED BY ARCHITECT.
11. BACKFILLING SHALL BE DONE SIMULTANEOUSLY ON BOTH SIDES OF WALL.

CONCRETE

1. CONCRETE WORK SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING STANDARDS.

   ACI 301 – "SPECIFICATIONS FOR STRUCTURAL CONCRETE"
   ACI MCP – "MANUAL OF CONCRETE PRACTICE"
   ACI 318 – "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"
   ACI 318.1 – "BUILDING CODE REQUIREMENTS FOR STRUCTURAL PLAIN CONCRETE"

2. CONCRETE SHALL HAVE A MINIMUM 28–DAY ULTIMATE COMPRESSIVE STRENGTH AS FOLLOWS:

   SLABS–ON–GRADE 4,000 PSI
   FOOTINGS AND FOUND. / FROST WALLS 3,000 PSI

3. CONCRETE MIX DESIGN (INCLUDING AGGREGATE SIZE, WATER/CEMENT RATIO, AIR ENTRAINMENT, ADMIXTURES AND SLUMP) SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF ANY WORK. MAXIMUM WATER/CEMENT RATIO PERMITTED AS NOTED:

   0.50 FOR SLABS ON GRADE
   0.54 FOR BELOW GRADE CONCRETE
   0.48 FOR EXPOSED CONCRETE

   USE OF ANY CONCRETE MIX ADDITIVES ARE REQUIRED TO BE APPROVED BY JOB SUPERVISORS PRIOR TO USE.

4. CONCRETE TO BE EXPOSED TO THE WEATHER SHALL HAVE AIR–ENTRAINING ADMIXTURE AS REQUIRED TO PROVIDE 4–6% AIR ENTRAINMENT.
5. CONCRETE STRENGTH SHALL BE EVALUATED ACCORDING TO METHOD 1 OR METHOD 2 AS DESCRIBED IN ACI 301. THE RESULTS OF THESE ANALYSES SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO ANY WORK.
6. CONSTRUCTION JOINTS SHOWN ON THE CONTRACT DRAWINGS SHALL NOT BE ALTERED WITHOUT WRITTEN APPROVAL OF THE ARCHITECT.
7. DRAWINGS SHOWING THE LOCATION OF CONSTRUCTION JOINTS, CONTROL JOINTS AND PLACING SEQUENCE SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO THE PREPARATION OF REINFORCING SHOP DRAWINGS.
8. GROUT USED TO SET PLATES SHALL BE NON–SHRINK AND NON–METALLIC.
9. CONTRACTOR SHALL USE SMOOTH FORMS FOR EXPOSED CONCRETE SURFACES. BOARD FORMS MAY BE USED FOR UNEXPOSED CONCRETE SURFACES. EARTH FORMS ARE FORBIDDEN.
10. PROVIDE A MINIMUM OF 6" COMPACTED GRANULAR FILL UNDER ALL SLABS–ON–GRADE.

REINFORCEMENT

1. DETAILING, FABRICATION AND ERECTION OF REINFORCING STEEL SHALL CONFORM TO THE FOLLOWING:

   ACI 315 – "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT"
   ACI 318 – "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"
   MSP2 – "CRSI MANUAL OF STANDARD PRACTICE"
   AWS D1.4 – "STRUCTURAL WELDING CODE – REINFORCING STEEL"
   WRI – "WELDED WIRE FABRIC MANUAL OF STANDARD PRACTICE"

2. STEEL REINFORCING BARS SHALL CONFORM TO ASTM A615 (GRADE 60). DEFORMED. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
3. CLEARANCES FOR REINFORCEMENT: CONCRETE PLACED DIRECTLY ON EARTH (FOOTINGS, SLABS, ETC.) 3" FROM BOTTOM. ALL OTHER CONCRETE PROVIDE 2" CLEAR TO REINFORCING.
4. CONTRACTOR SHALL REFER TO TYPICAL DETAILS SHOWN ON CONTRACT DRAWINGS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS.
5. WHERE REINFORCEMENT IS REQUIRED IN SECTIONS, REINFORCEMENT IS CONSIDERED TYPICAL WHEREVER SECTION APPLIES.
6. WELDED WIRE MESH SHALL LAP A MINIMUM OF 6" AND BE TIED TOGETHER.

STRUCTURAL STEEL

1. STRUCTURAL STEEL WORK SHALL CONFORM TO THE FOLLOWING:

   AISC – "SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION OF STEEL FOR BUILDINGS".
   AISC – "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
   AWS D1.1 – "STRUCTURAL WELDING CODE – STEEL".
   AISC – "STRUCTURAL STEEL DETAILING MANUAL".

2. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWNG:

   HOT ROLLED WIDE–FLANGE – ASTM A992 (Fy=50 KSI)
   ALL OTHER STRUCTURAL SHAPES AND PLATES – ASTM A36 (Fy=36KSI)
   STRUCTURAL STEEL PIPE – ASTM A53 GRADE B (Fy=35 KSI).
   TUBULAR STEEL – ASTM A500 GRADE B (Fy=46 KSI)
   HIGH STRENGTH BOLTS – ASTM A325N (BEARING TYPE)
   ASTM A325F (FRICTION TYPE)
   ANCHOR BOLTS – ASTM A307 OR A36

3. PROVIDE 2 MIL. THICK RED OR GRAY OXIDE PRIMER ON ALL STEEL SURFACES UNLESS NOTED OTHERWISE.
4. ANCHOR BOLTS SHALL BE PRESET WITH TEMPLATES AT REQUIRED LOCATIONS.
5. LEVELING PLATES AND BEARING PLATES SHALL BE SET IN FULL BED OF NON–SHRINK GROUT.
6. CONNECTIONS MAY BE EITHER BOLTED OR WELDED AT THE FABRICATOR'S OPTION. BOLTED CONNECTIONS SHALL BE AS FOLLOWS:

   MINIMUM BOLT DIAMETER: 3/4"

   SIMPLE SHEAR CONNECTIONS SHALL BE CAPABLE OF END ROTATION PER AISC REQUIREMENTS FOR "UNRESTRAINED MEMBERS".

7. ALL BEAM CONNECTIONS NOT DETAILED, SHALL SUPPORT 1/2 THE TOTAL UNIFORM LOAD CAPACITY FOR THE GIVEN BEAM AND SPAN OR THE INDICATED REACTION, WHICHEVER IS GREATER. CONNECTIONS SHALL GENERALLY FOLLOW THE TYPES SHOWN IN THE "AISC MANUAL OF STEEL CONSTRUCTION," TABLE II, III, OR X.
8. WELDS SHALL FULLY DEVELOP STRENGTH OF THE MATERIALS BEING WELDED, UNLESS NOTED OTHERWISE, EXCEPT THAT FILLET WELDS SHALL BE A MINIMUM 3/16".
9. WELDED CONNECTIONS SHALL BE MADE BY APPROVED CERTIFIED WELDERS USING FILLER METAL CONFORMING TO E70XX.
10. CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SUPPORTS TO HOLD STRUCTURAL STEEL FRAMING SECURELY IN POSITION. TEMPORARY BRACING SHALL REMAIN UNTIL THE PERMANENT LATERAL BRACING HAS BEEN INSTALLED AND AND THE CONCRETE FOR FLOOR SLABS HAS ATTAINED 75% OF ITS REQUIRED STRENGTH.
11. STRUCTURAL STEEL FRAMING SHALL BE TRUE AND PLUMB BEFORE FINAL BOLTING OR WELDING OF CONNECTIONS.
12. CONTRACTOR SHALL NOT MODIFY OR CUT ANY STRUCTURAL STEEL WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.
13. CONTRACTOR SHALL FIELD TOUCH UP ALL ABRASIONS, BURNS AND SIMILAR DEFECTS IN PAINT OF THE STRUCTURAL STEEL, JOISTS AND STEEL DECK.
14. PROVIDE ACCESS FOR AN INDEPENDENT TESTING AGENCY, HIRED BY THE OWNER, TO MAKE THE FOLLOWING INSPECTIONS:
    - BOLT INSTALLATION
    - VISUAL INSPECTION OF FILLET WELDS.
    - ULTRASONIC TESTING OF FULL–PENETRATION WELDS (TEST REPORTS FROM ALL SHOP–FABRICATED, FULL PENETRATION WELDS SHALL BE SUBMITTED TO ENGINEER PRIOR TO RELEASE FOR SHIPPING).

STRUCTURAL WOOD CONSTRUCTION

1. WOOD MATERIALS AND CONSTRUCTION SHALL CONFORM TO THE FOLLOWING:

   NFPA – "NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION"
   NFPA – "DESIGN VALUES FOR WOOD CONSTRUCTION"
   AITC – "TIMBER CONSTRUCTION MANUAL, PART II, DESIGN SPECIFICATIONS"
   APA – "U.S. PRODUCT STANDARD PS 1–83 FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD"

2. STRUCTURAL WOOD SHALL BE VISUALLY GRADED IN ACCORDANCE WITH ASTM D245. WOOD SHALL BE IDENTIFIED BY A GRADE MARK OR CERTIFICATE OF INSPECTION ISSUED BY A RECOGNIZED INSPECTION AGENCY.
3. ALL WOOD SHALL HAVE A MAXIMUM MOISTURE CONTENT OF 15% PRIOR TO INSTALLATION.
4. NEW WOOD SHALL BE SPF #1 OR #2 OR BETTER
5. ALL JOISTS AND RAFTERS SHALL BE SUPPORTED BY DIRECT END BEARING ON BEAMS, PARTITIONS OR JOIST HANGERS.
6. ALL WOOD PERMANENTLY EXPOSED TO THE WEATHER, IN CONTACT WITH EXTERIOR CONTACT WITH THE GROUND SHALL HAVE A PRESERVATIVE TREATMENT EQUAL TO 0.25 P.C.F. RETENTION OF PRESSURE INJECTED PRESERVATIVE
7. NO WOOD MEMBER SHALL BE CUT, NOTCHED OR DRILLED WITHOUT THE SPECIFIC, WRITTEN APPROVAL OF THE ENGINEER.
8. DO NOT EMBED WOOD MEMBERS IN CONCRETE.
9. PLYWOOD SHALL BE LAID WITH FACE GRAIN PERPENDICULAR TO SUPPORTS. STAGGER ALL JOINTS.
10. PLYWOOD SHALL BE CAPABLE OF SUPPORTING DESIGN LOADS AT REQUIRED SUPPORT SPACING AND BEAR APPROPRIATE GRADING STAMP FROM AMERICAN PLYWOOD ASSOCIATION.
11. PLYWOOD SHALL BE FASTENED TO SUPPORTS WITH 8d NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS. (UNLESS NOTED OTHERWISE ON PLANS).
12. USE STANDARD WIRE NAILS UNLESS SPECIFICALLY NOTED OTHERWISE.
13. ALL BOLTS AND LAG SCREWS SHALL CONFORM TO ASTM A307. USE STEEL WASHER BETWEEN HEAD OF BOLT OR LAG SCREW AND WOOD. USE STEEL WASHER BETWEEN NUT AND WOOD.

STRUCTURAL WOOD CONSTRUCTION – CONTINUED

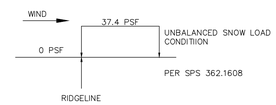
14. ALL FASTENERS USED FOR PRESERVATIVE TREATED WOOD SHALL BE STAINLESS OR GALVANIZED
15. JOIST HANGER FASTENERS MUST BE AS SUPPLIED BY THE JOIST HANGER MANUFACTURER.
16. LAMINATED VENEER LUMBER (LVL) BEAMS SHALL CONFORM TO TRUS JOIST MIRCROLAM 1.9E LVL SPECIFICATION OR EQUAL

PRE–FABRICATED WOOD TRUSSES

1. WOOD TRUSSES SHALL CONFORM TO THE LATEST EDITION OF THE "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES", PUBLISHED BY THE TRUSS PLATE INSTITUTE.
2. WOOD TRUSS FABRICATOR SHALL SUBMIT CALCULATIONS TO THE ENGINEER FOR REVIEW PRIOR TO INSTALLATION. THE CALCULATIONS MUST BE STAMPED BY A REGISTERED PROFESSIONAL ENGINEER.
3. WOOD TRUSSES MUST BE ERECTED AND BRACED ACCORDING TO THE PROCEDURES DESCRIBED IN "BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS", PUBLISHED BY THE TRUSS PLATE INSTITUTE.
4. WOOD TRUSSES SHALL BE DESIGNED TO SUPPORT THE FOLLOWNG LOADS UNLESS INDICATED OTHERWISE ON CONTRACT DRAWINGS:

   ROOF TRUSSES TOP CHORD LIVE LOAD 30.8 PSF (BALANCED SNOW LOAD CONDITION)
   DEAD LOAD 10 PSF
   BTM CHORD DEAD LOAD 10 PSF

5. WOOD TRUSSES SHALL BE SUPPORTED BY DIRECT END BEARING ON WALLS, BEAMS, COLUMNS OR APPROPRIATE STEEL HANGERS.



REINFORCEMENT DEVELOPMENT AND SPLICE LENGTH SCHEDULE

Fy = 60 KSI f'c = 3000 PSI

BAR SIZE		#3	#4	#5	#6	#7	#8	#9	#10	#11
CLASS A SPLICE LENGTH	TOP BARS	22	29	36	43	63	72	81	91	101
	OTHERS	17	22	28	33	48	55	62	70	78
CLASS B SPLICE LENGTH	TOP BARS	28	37	47	56	81	93	105	118	131
	OTHERS	22	29	36	43	63	72	81	91	101

NOTES

1. ALL SPLICE LENGTHS SHALL BE CLASS B UNLESS NOTED OTHERWISE.
2. TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL WEIGHT CONCRETE.
3. TENSION DEVELOPMENT LENGTHS AND TENSION LAP SPLICE LENGTHS ARE CALCULATED PER ACI 318–95, SECTIONS 12.2.2 AND 12.15, RESPECTIVELY. TABULATED VALUES FOR BEAMS AND COLUMNS ARE BASED ON TRANSVERSE REINFORCEMENT AND CONCRETE COVER MEETING MIN. CODE REQUIREMENTS. LENGTHS ARE IN INCHES.
4. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" OF CONCRETE CAST BELOW THE BARS.
5. SPLICE AND DEVELOPMENT LENGTHS IN THIS SCHEDULE ARE BASED ON CASE 1 PER CRSI (1996):

   BEAMS OR COLUMNS: COVER AT LEAST 1.0 BARø AND C.–C. SPACING AT LEAST 2.0 BARø

   ALL OTHERS: COVER AT LEAST 1.0 BARø AND C.–C. SPACING AT LEAST 3.0 BARø

WOOD FRAMING DESIGN STRESSES

MEMBER	SPECIES & GRADE	Fb Single Use (psi)	Fb Repet Use (psi)	Fv (psi)	Fc (psi) PERP–TO GRAIN	Fc (psi) PARALLEL TO GRAIN	E (psi)
JOISTS, BEAMS, AND HEADERS	SPF #1/#2 U.N.O.	875	1,005	135	425	1,150	1,400,000
LOAD BRG. STUDS	SPF #1/#2 U.N.O.	875	1,005	135	425	1,150	1,400,000
NON– BRG. STUDS	SPF STUD	675	776	135	425	725	1,200,000

TYPICAL NAILING SCHEDULE, U.N.O.

CONNECTION TYPE	NAILING (COMMON NAILS– UNLESS OTHER CONNECTION IS REQUIRED)	NAILING (STRIP NAILS– UNLESS OTHER CONNECTION IS REQUIRED)
DOUBLE TOP PLATES, FACE NAIL DOUBLE TOP PLATES, LAP SPLICE, FACE NAIL TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	16d COMMON (3 1/2"x0.162") @ 16" OC 8–16d COMMON (3 1/2"x0.162") 4–16d COMMON (3 1/2"x0.162")	3" x 0.131" @ 12" O.C. 12 – 3" x 0.131" 4 – 3" x 0.131"
TOP PLATE TO STUD, END NAIL HEADER TO KING STUD, EACH PLY TREATED BOTTOM PLATE TO STUD	2–16d COMMON (3 1/2"x0.162") 3–16d COMMON (3 1/2"x0.162") 4–8d COMMON (2 1/2"x0.131"), TOENAIL OR 2–16d COMMON (3 1/2"x0.162"), END NAIL	3 – 3" x 0.131" 3 – 3 1/4" x 0.131" (HOT DIP GALVANIZED)
BUILT–UP STUD COLUMNS, FACE NAIL BUILT–UP CORNER STUDS & SUPPORT STUDS	10d COMMON (3"x0.148") @ 16" OC STAGGERED 10d COMMON (3"x0.148") @ 16" OC STAGGERED	3"x 0.131" @ 12" OC STAGGERED 3"x 0.131" @ 12" OC STAGGERED
CONTINUOUS HEADER, FACE NAIL	16d COMMON (3 1/2"x0.162") @ 12" OC ALONG EACH SIDE	-----
5/8" PLYWOOD OR OSB ROOF SHEATHING (APA RATED) UNLESS NOTED OTHERWISE	8d COMMON (2 1/2"x0.131") @ 6" OC AT SUPPORTED PANEL EDGES 8d COMMON (2 1/2"x0.131") @ 12" OC AT INTERMEDIATE MEMBERS IN FIELD OF PANELS	2 1/2"x0.113" @ 6" OC AT SUPPORTED PANEL EDGES 2 1/2"x0.113" @ 12" OC AT INTERMEDIATE MEMBERS IN FIELD OF PANELS
7/16" OSB WALL SHEATHING (APA RATED) UNLESS NOTED OTHERWISE – SEE SHEAR WALL SCHEDULE	SEE SHEARWALL NOTES ON S3.0	SEE SHEARWALL NOTES ON S3.0

HURRICANE TIE FASTENERS AT ALL TRUSS BEARING LOCATIONS

CONNECTION TYPE	HURRICANE TIE	ATTACHMENT TO TRUSS	ATTACHMENT TO SUPPORTING STRUCTURE
TYPICAL TRUSS (111'–6" BRG. ELEV.)	SIMPSON H10A	9–10d (1 1/2"x0.148")	9–10d (1 1/2"x0.148")
2–PLY TRUSS	SIMPSON LGT2	16–16d SINKERS	14–16d SINKERS
3–PLY TRUSS	SIMPSON LGT3–SDS2.5	12–SDS 1/4" x 2 1/2"	26–16d SINKERS
4–PLY TRUSS	SIMPSON LGT4–SDS3	16–SDS 1/4" x 3"	30–16d SINKERS
JACK TRUSS	SIMPSON H1	6–8D 1 1/2" x 0.131"	4–8D 2 1/2" x 0.131"
HIP CORNER TRUSS	SIMPSON HCP2	6–10D 1 1/2" x 0.148"	6–10D 1 1/2" x 0.148"

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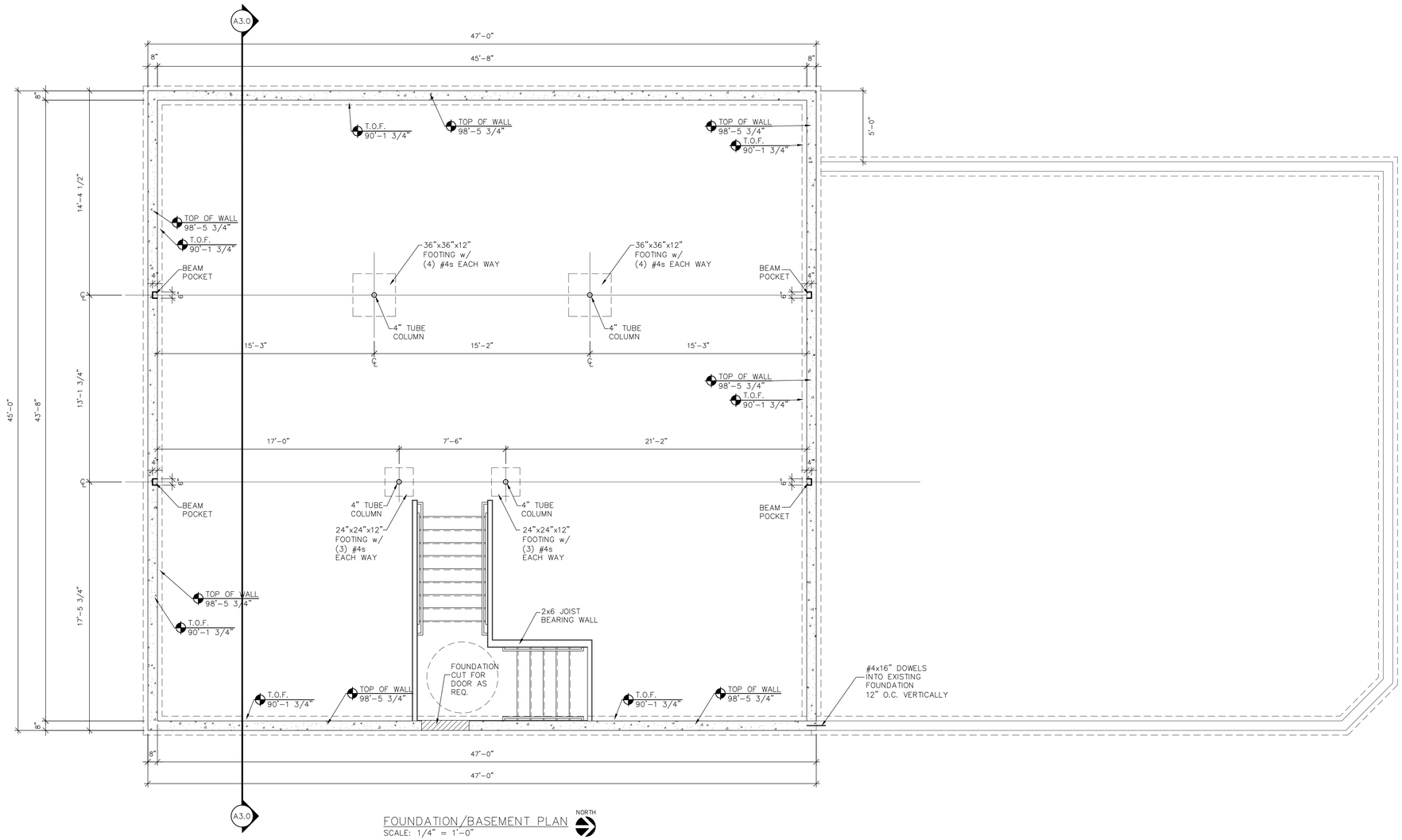

FOUNDATION/BASEMENT PLAN

SCALE: 1/4" = 1'-0"

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FLOOR JOIST FRAMING PLAN

SCALE: 1/4" = 1'-0"

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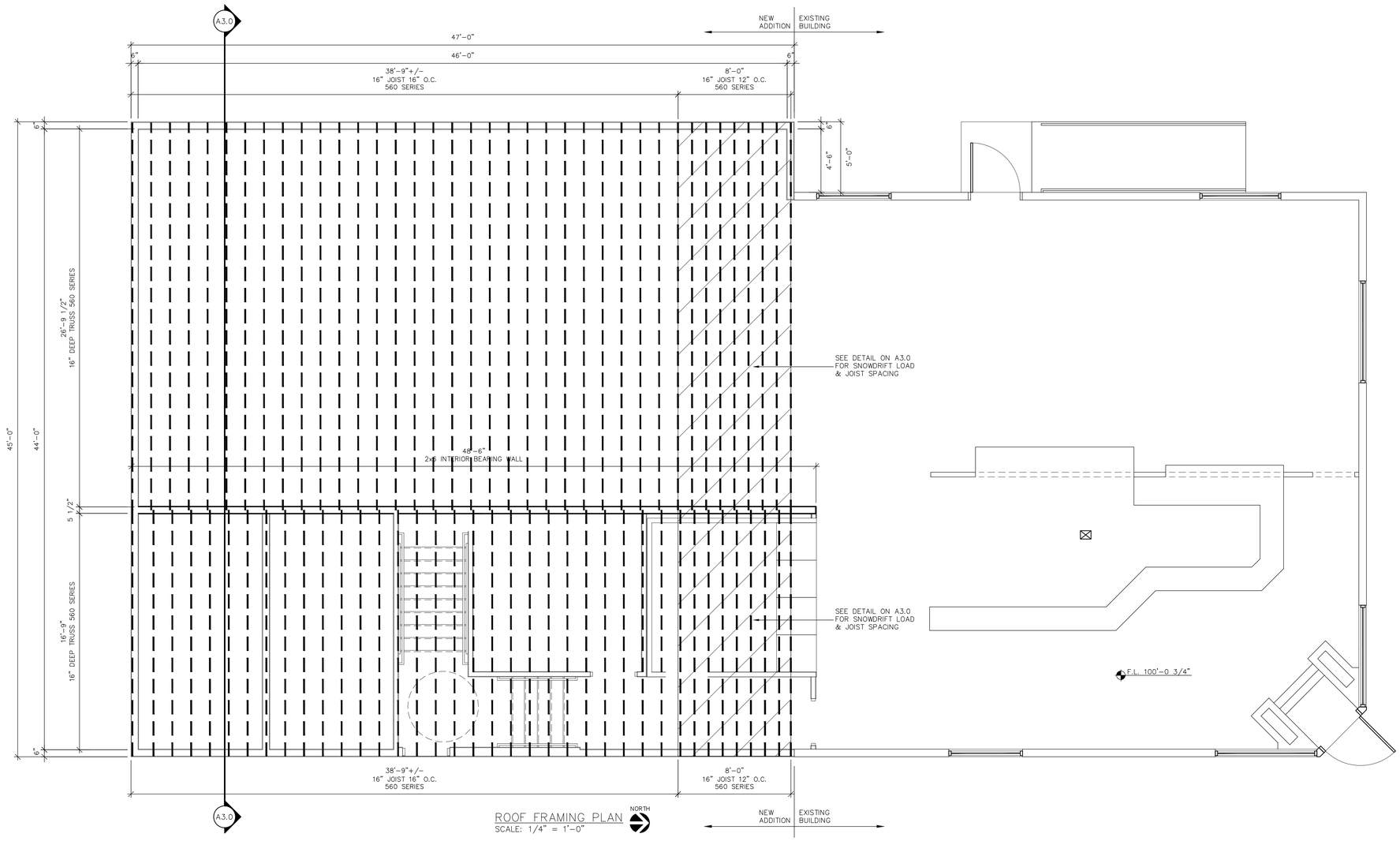
CLUB RITZ
301 W. 7TH ST.
KAUKAUNA, WI 54130

DATE
5/29/2025

SHEET
S1.2

DRAWN BY:
S. BURTON



ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"

REV. NO.	DATE	DESCRIPTION

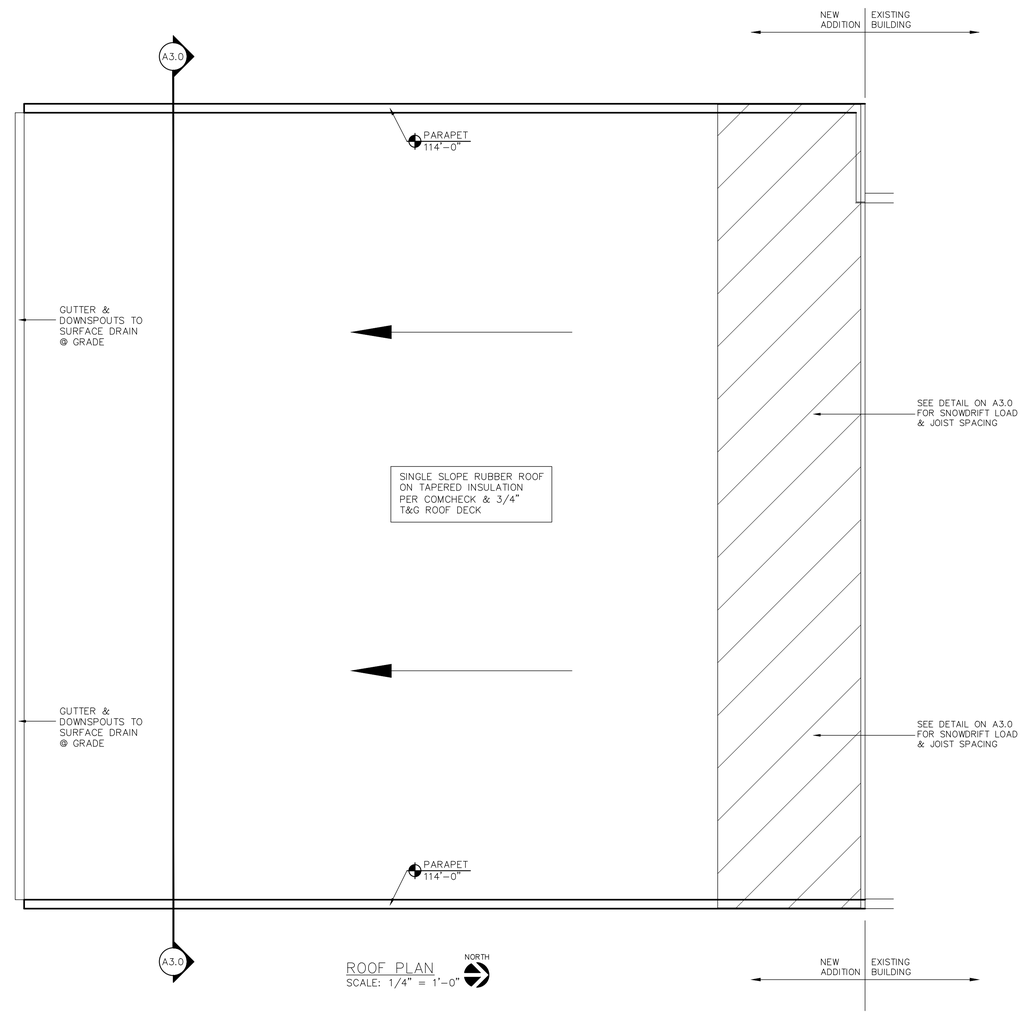
Architect:
Daniel J. Meissner
AIA, LLC
1230 E. Calumet Street Appleton, WI
920.428.0982

PROPOSED ADDITION FOR,
CLUB RITZ
301 W. 7TH ST.
KAUKAUNA, WI 54130

DATE
5/29/2025

SHEET
S1.3

DRAWN BY:
S. BURTON



ROOF PLAN
SCALE: 1/4" = 1'-0"

REV. NO.	DATE	DESCRIPTION

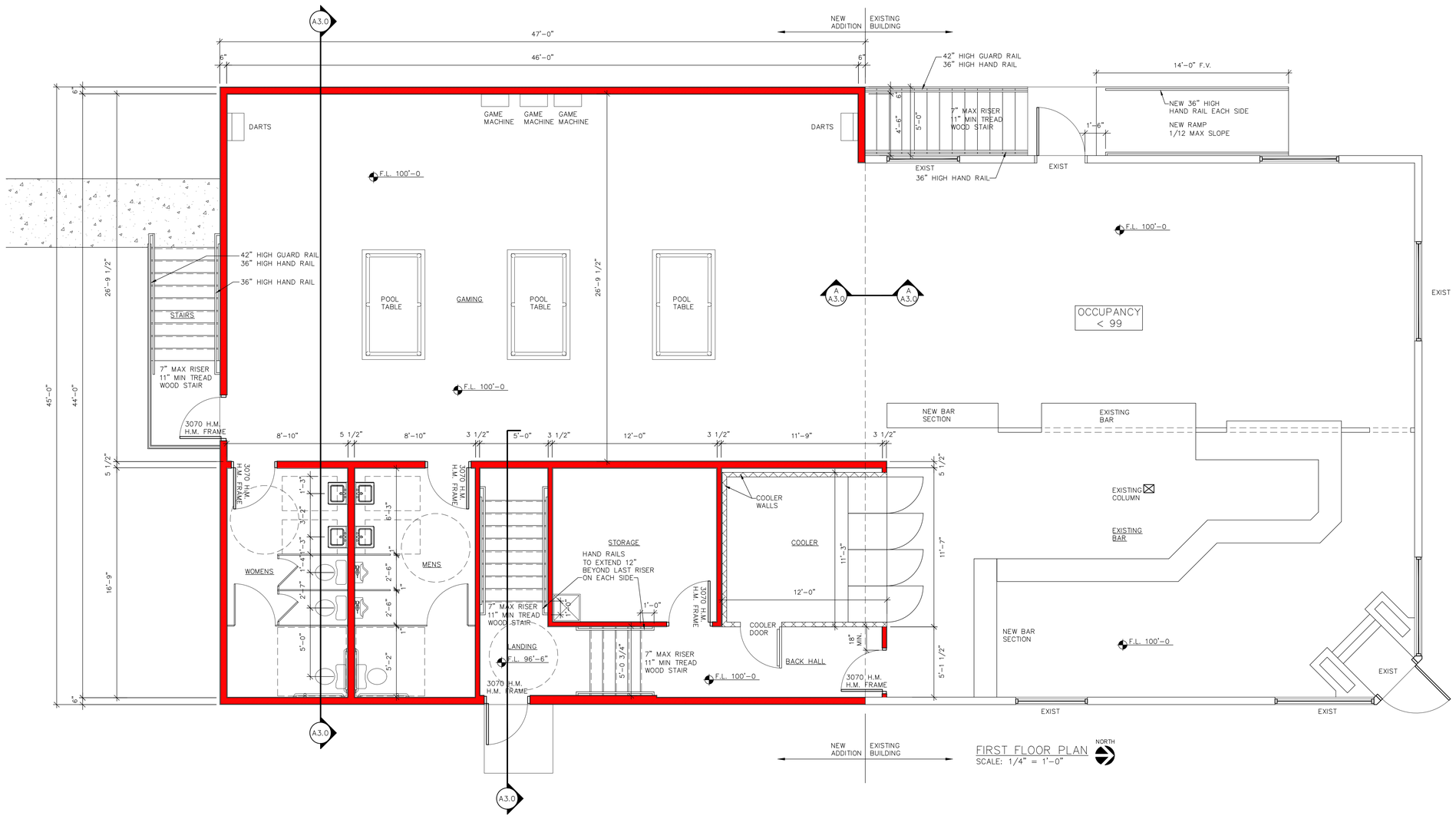
Architect:
Daniel J. Meissner
AIA, LLC
1230 E. Calumet Street Appleton, WI
920.428.0982

PROPOSED ADDITION FOR,
CLUB RITZ
301 W. 7TH ST.
KAUKAUNA, WI 54130

DATE
5/29/2025

SHEET
S1.3

DRAWN BY:
S. BURTON



FIRST FLOOR PLAN
SCALE: 1/4" = 1'-0"

REV. NO.	DATE	DESCRIPTION

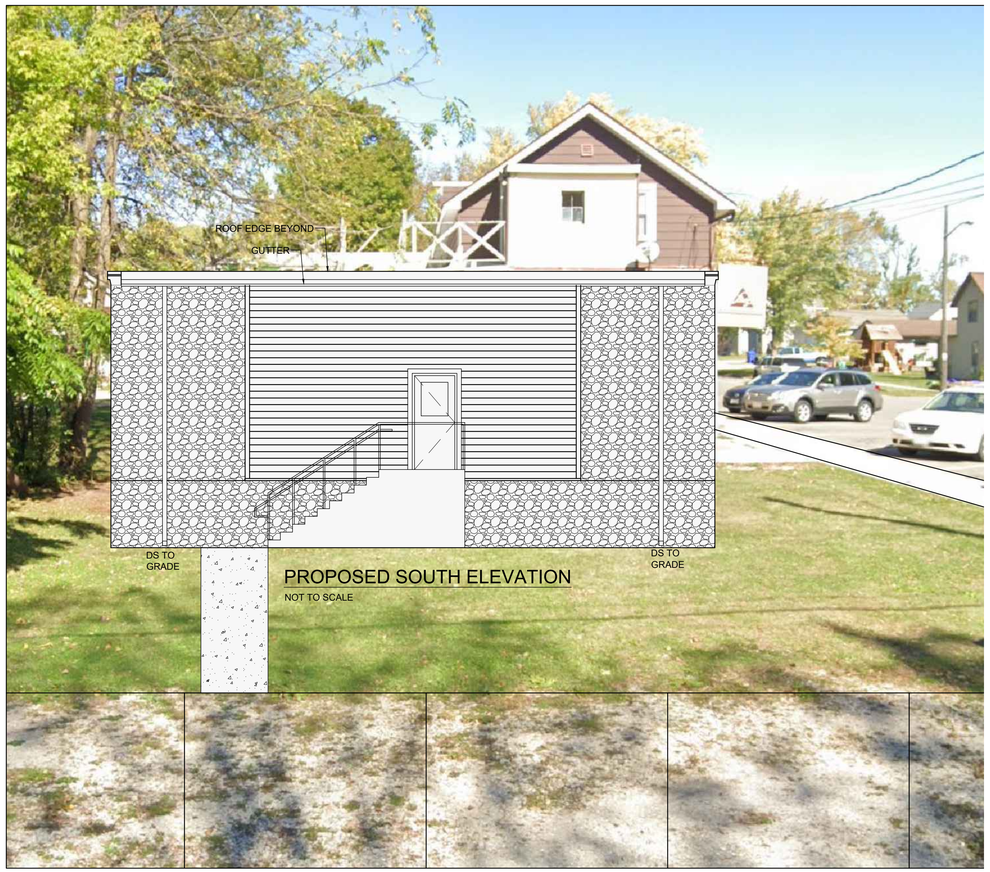
Architect:
Daniel J. Meissner
AIA, LLC
1230 E. Calumet Street Appleton, WI
920.428.0982

PROPOSED ADDITION FOR,
CLUB RITZ
301 W. 7TH ST.
KAUKAUNA, WI 54130

DATE
5/29/2025

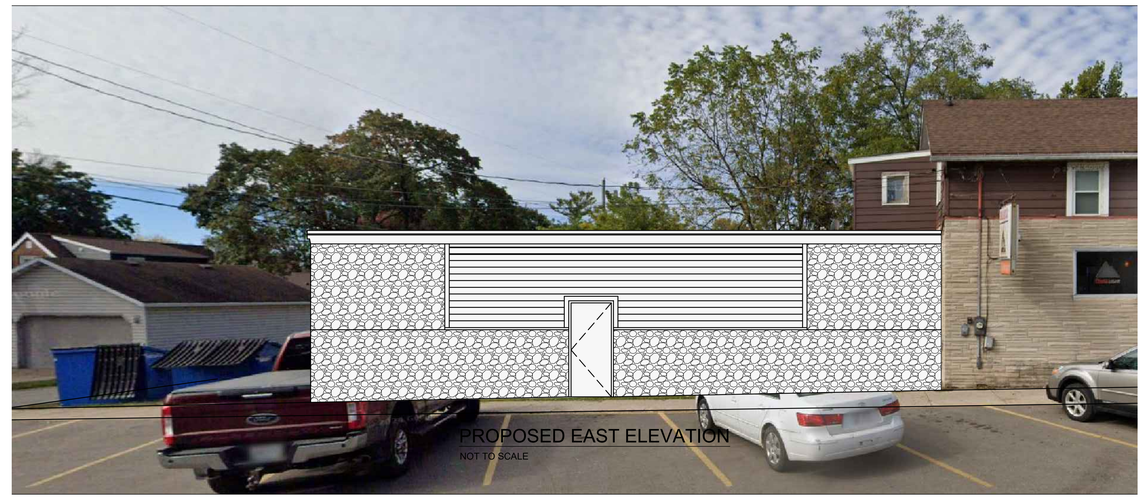
SHEET
A1.0

DRAWN BY:
S. BURTON



PROPOSED SOUTH ELEVATION

NOT TO SCALE



PROPOSED EAST ELEVATION

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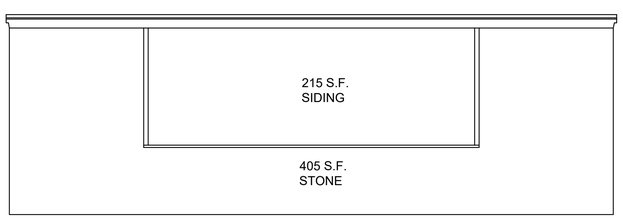
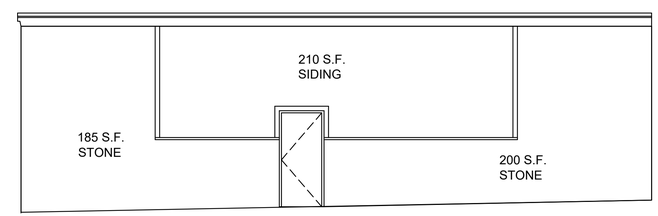


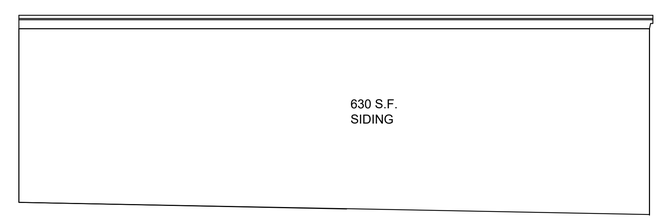
PROPOSED SOUTH ELEVATION

SCALE: 3/16 = 1'-0"



PROPOSED EAST ELEVATION

SCALE: 3/16 = 1'-0"



PROPOSED WEST ELEVATION

SCALE: 3/16 = 1'-0"

REV. NO.	DATE	DESCRIPTION

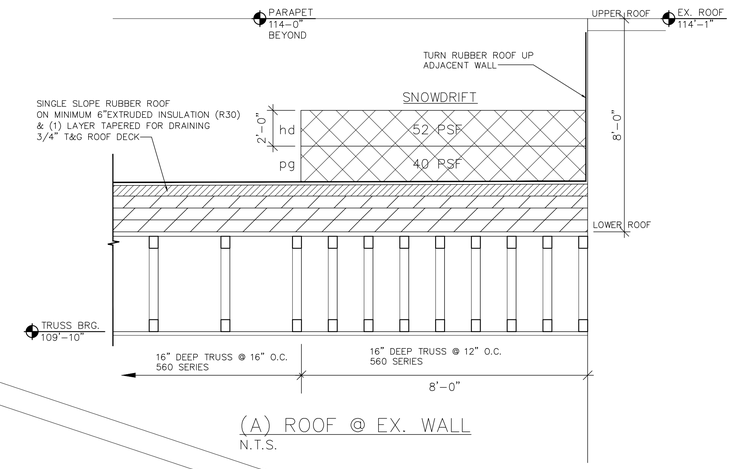
Architect:
Daniel J. Meissner
AIA, LLC
1230 E. Calumet Street Appleton, WI
920.428.0982

PROPOSED ADDITION FOR,
CLUB RITZ
301 W. 7TH ST.
KAUKAUNA, WI 54130

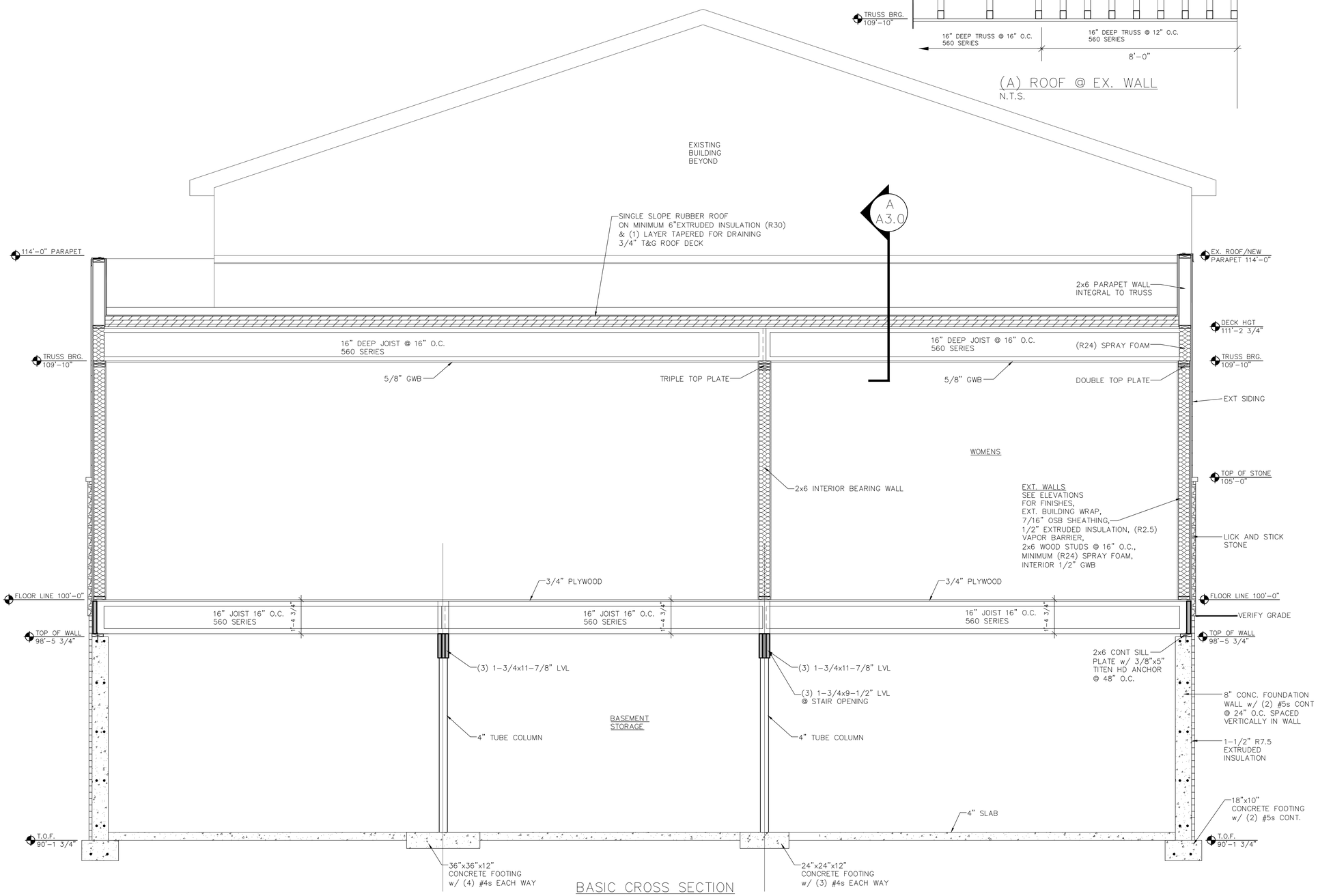
DATE
5/29/2025

SHEET
A2.0

DRAWN BY:
S. BURTON



(A) ROOF @ EX. WALL
N.T.S.



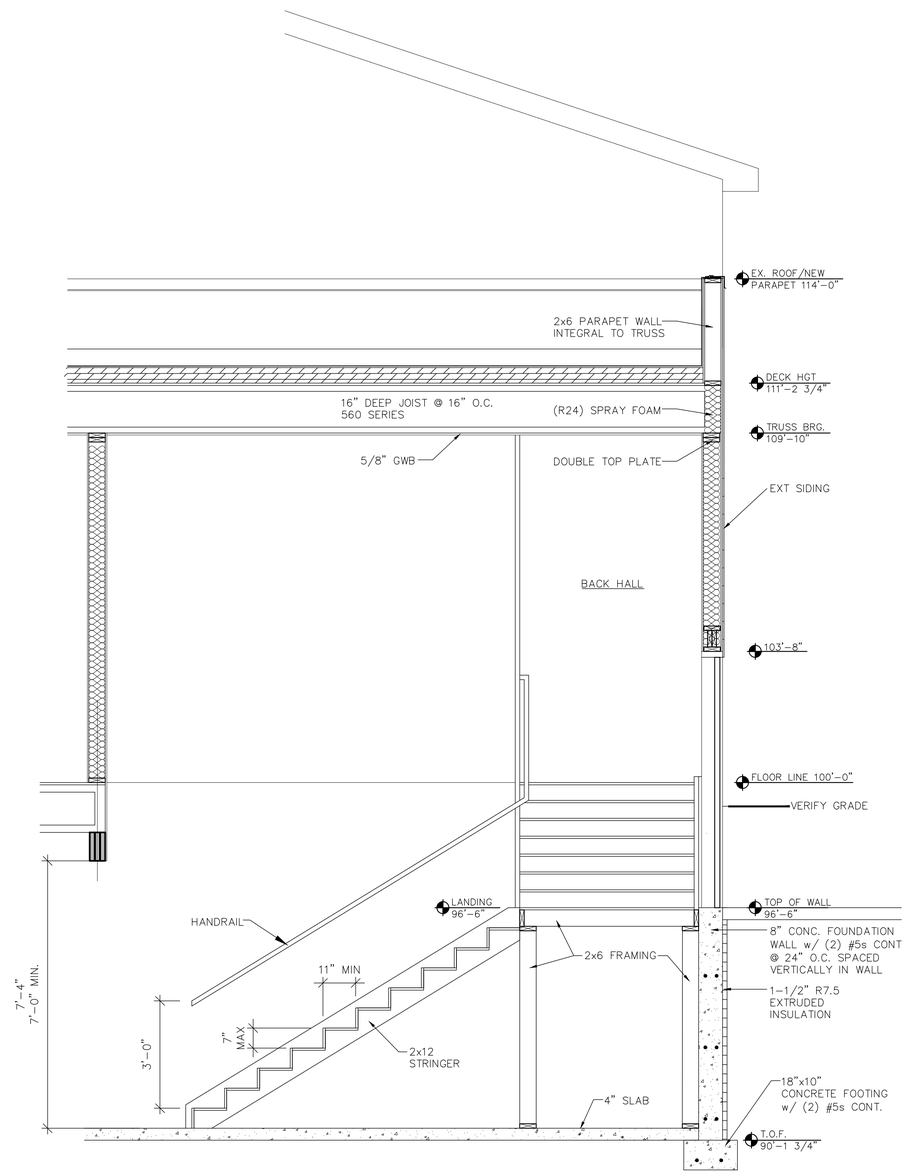
BASIC CROSS SECTION
SCALE: 1/2" = 1'-0"

REV. NO.	DATE	DESCRIPTION

Architect:
Daniel J. Meissner
AIA, LLC
1230 E. Calumet Street Appleton, WI
920.428.0982

PROPOSED ADDITION FOR,
CLUB RITZ
301 W. 7TH ST.
KAUKAUNA, WI 54130

DATE
5/29/2025
SHEET
A3.0
DRAWN BY:
S. BURTON



STAIR CROSS SECTION
SCALE: 1/2" = 1'-0"

REV. NO.	DATE	DESCRIPTION

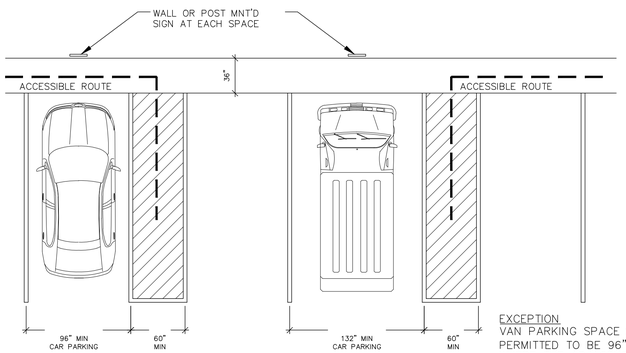
Architect:
Daniel J. Meissner
AIA, LLC
1230 E. Calumet Street Appleton, WI
920.428.0982

PROPOSED ADDITION FOR,
CLUB RITZ
301 W. 7TH ST.
KAUKAUNA, WI 54130

DATE
5/29/2025

SHEET
A3.1

DRAWN BY:
S. BURTON



EXCEPTION
VAN PARKING SPACE SHALL BE PERMITTED TO BE 96" MIN WIDTH WHERE THE ADJACENT ACCESS AISLE IS ALSO 96" MIN WIDTH

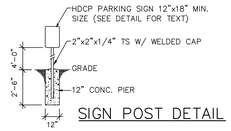
VEHICLE PARKING SPACE SIZE & ACCESS AISLE

THE MAXIMUM SLOPE AT PARKING SPACES IS 1:50 (4.6.2)

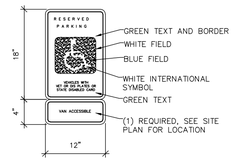
IT SHALL BE THE RESPONSIBILITY OF THE OWNER TO INSTALL (WHERE SHOWN HERON). BARRIER FREE PARKING SIGNS IN CONFORMITY TO WISCONSIN ADMINISTRATIVE CODE: TRANS #200.07

INTERNATIONAL SYMBOL OF ACCESSIBILITY

NO SCALE



SIGN POST DETAIL



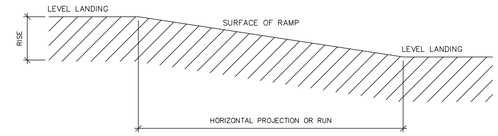
PARKING SIGN DETAIL

4.8.1* GENERAL
ANY PART OF AN ACCESSIBLE ROUTE WITH A SLOPE GREATER THAN 1:20 SHALL BE CONSIDERED A RAMP AND SHALL COMPLY WITH 4.8.

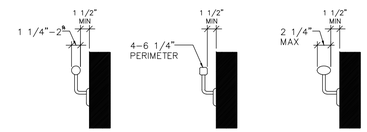
4.8.2* SLOPE AND RISE
THE LEAST POSSIBLE SLOPE SHALL BE USED FOR ANY RAMP. THE MAXIMUM SLOPE OF A RAMP IN NEW CONSTRUCTION SHALL BE 1:12. THE MAXIMUM RISE FOR ANY RUN SHALL BE 30" (SEE FIGURE 16). CURB RAMPS AND RAMPS TO BE CONSTRUCTED ON EXISTING SITES OR IN EXISTING BUILDINGS OF FACILITIES MUST HAVE SLOPES AND RISES AS ALLOWED IN 4.1.6(3)(A). IF SPACE LIMITATIONS PROHIBIT THE USE OF A 1:12 SLOPE OR LESS.

SINGLE RAMP RUN

SCALE: 1/4" = 1'-0"

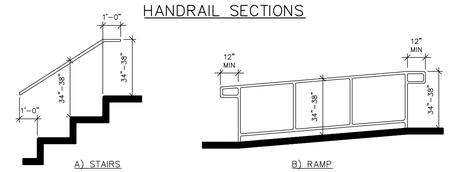


SLOPE	MAXIMUM RISE IN.	MAXIMUM HORIZONTAL PROJECTION FT.
1:12 TO 1:16	30	30
1:16 TO 1:20	30	40



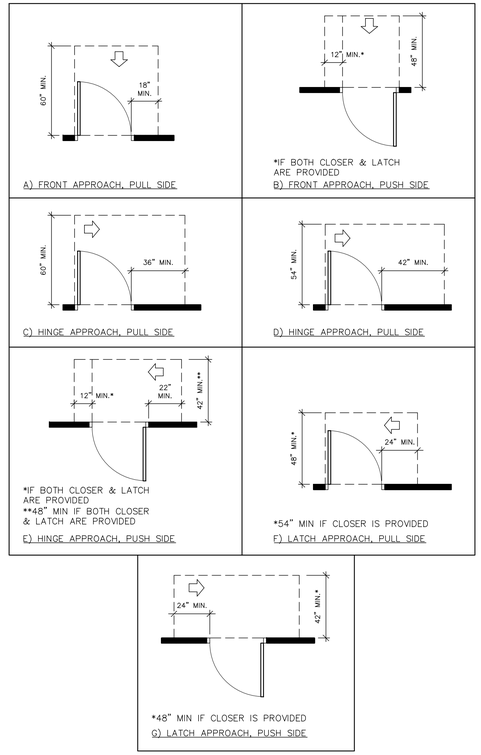
CIRCULAR — NON-CIRCULAR — NON-CIRCULAR

HANDRAIL SECTIONS

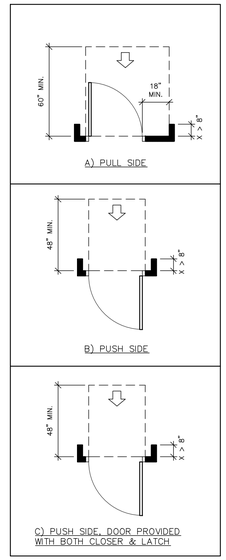


A) STAIRS — B) RAMP

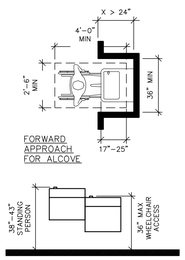
HANDRAIL EXTENSION & HEIGHTS



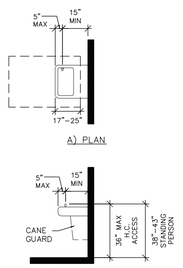
CLEARANCE AT MANUAL SWING DOORS



CLEARANCE AT RECESSED DOORS

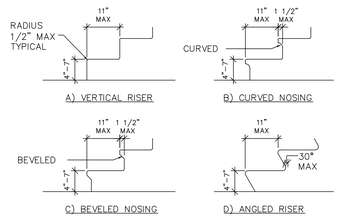


IF ONLY (1) EWC IS PROVIDED IT SHALL BE THE LOWER ONE AND WALL MOUNTED. CUPS TO BE PROVIDED.

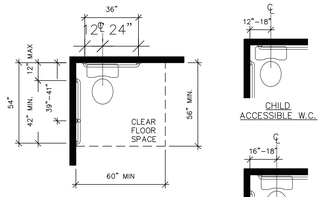


B) ELEVATION

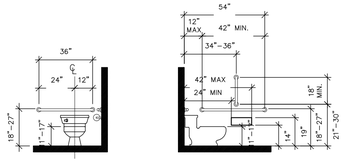
ELECTRIC WATER COOLER CLEAR SPACE

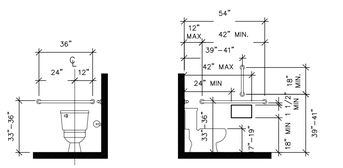


TREADS/RISERS & STAIR NOSINGS

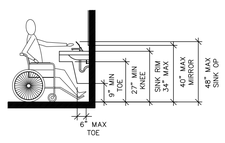


CLEARANCE & GRAB BARS @ WATER CLOSET

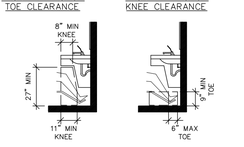




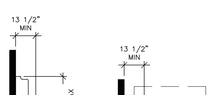
FLOOR MOUNT TOILET & GRAB BARS



LAVATORY KNEE/TOE CLEARANCE & HEIGHTS



KNEE & TOE CLEARANCES

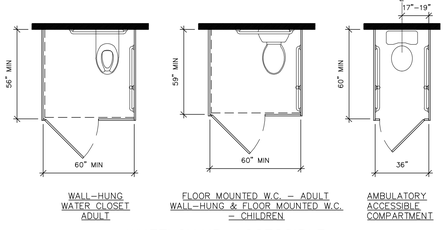


ELEVATION — PLAN

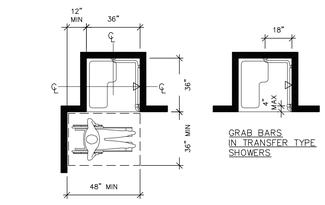
WALL MOUNTED URINAL

DINING SURFACES, WORK SURFACES, CHECK WRITING SURFACE THE TOPS SHALL BE 28" MIN & 34" MAX IN HEIGHT — SALES & SERVICE COUNTERS

SURFACE HEIGHTS

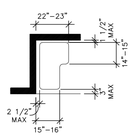


WHEELCHAIR ACCESSIBLE TOILET COMPARTMENTS

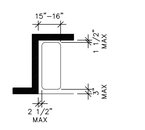


TRANSFER TYPE SHOWER

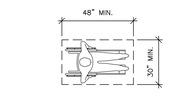
GRAB BARS IN TRANSFER TYPE SHOWERS



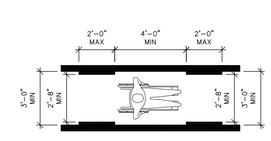
L-SHAPED SHOWER COMPARTMENT SEAT



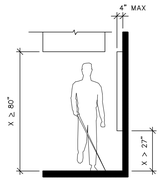
RECTANGLE SHOWER COMPARTMENT SEAT



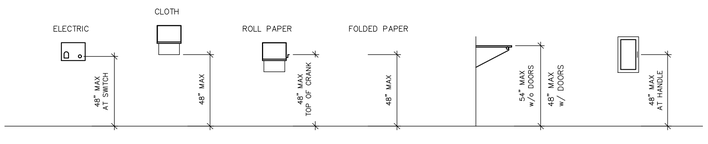
CLEAR FLOOR SPACE



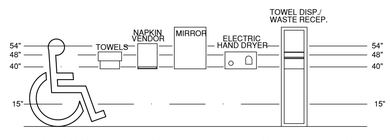
CLEAR WIDTH OF AN ACCESSIBLE ROUTE



LIMIT OF PROTRUDING OBJECTS



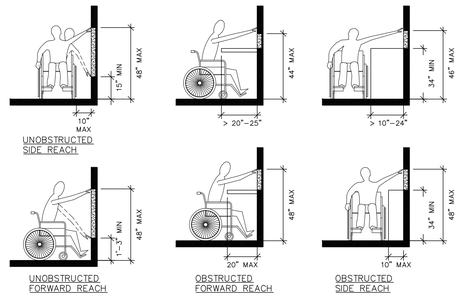
HAND DRYERS — SHELF & POLE — FIRE EXT. CABINET

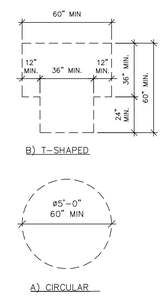


ACCESSIBLE INSTALLATION GUIDELINES

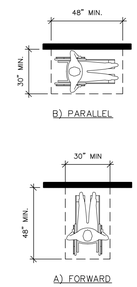
(BE SURE TO REFERENCE WITH ANSI CODE)

ACCESSORIES
THE DIAGRAM ABOVE SHOWS RECOMMENDED MOUNTING HEIGHTS FOR MANY WASHROOM ACCESSORIES. A MINIMUM 30x48" CLEAR FLOOR SPACE IS REQUIRED IN FRONT OF ALL ACCESSIBLE FIXTURES & ACCESSORIES. SEE OTHER DIAGRAMS ON THIS SHEET

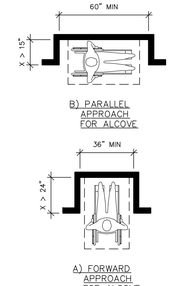




TURNING SPACES



CLEAR FLOOR SPACE POSITIONS



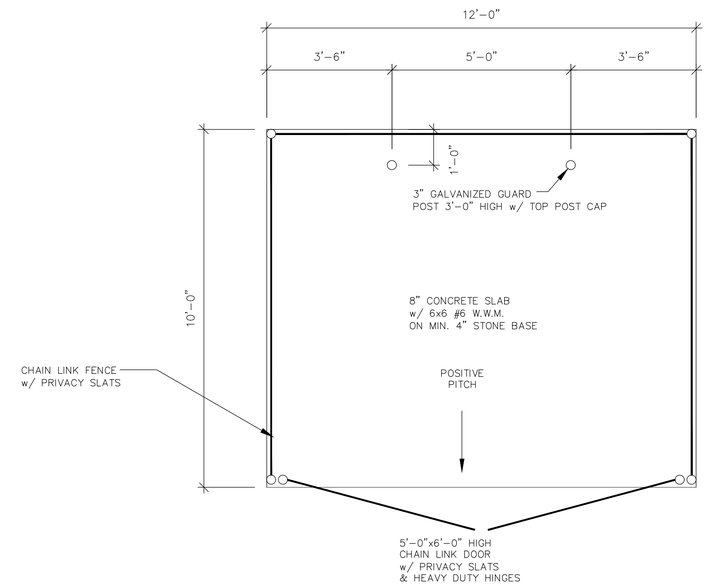
CLEAR FLOOR SPACE POSITIONS

REV. NO.	DATE	DESCRIPTION

Architect:
Daniel J. Meissner
AIA, LLC
1230 E. Calumet Street Appleton, WI
920.428.0982

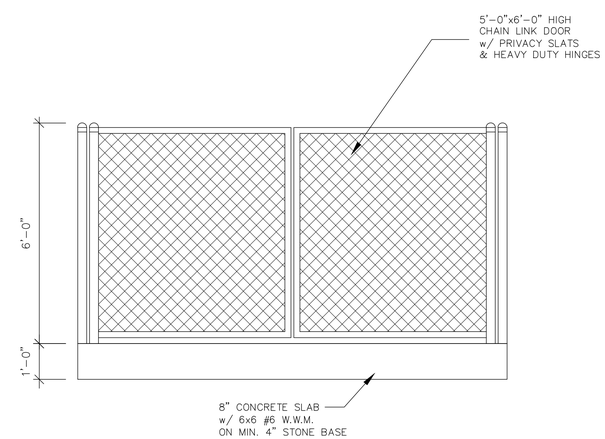
PROPOSED ADDITION FOR,
CLUB RITZ
301 W. 7TH ST.
KAUKAUNA, WI 54130

DATE 5/29/2025
SHEET A4.0
DRAWN BY: S. BURTON



DUMPSTER PLAN

SCALE: 1/2"=1'-0"



DUMPSTER ELEVATION

SCALE: 1/2"=1'-0"

REV. NO.	DATE	DESCRIPTION

Architect:
Daniel J. Meissner
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1230 E. Calumet Street Appleton, WI
920.428.0982

PROPOSED ADDITION FOR,
CLUB RITZ
301 W. 7TH ST.
KAUKAUNA, WI 54130

DATE
5/29/2025

SHEET
A5.0

DRAWN BY:
S. BURTON