

MEMORANDUM

TO: Mayor Miller, City Council Members

FROM: Matt Holmes, City Engineer

SUBJECT: <u>Jesse Elsmore, Jardine Builders, LLC</u> – To request site plan approval for two (2)

two-story mixed-use buildings located at 139 West Main Street & 127 West Main

Street, consisting of 0.96 acres.

DATE: May 9, 2025

CITY COUNCIL MEETING: May 15, 2025
CITY COUNCIL ROLE: Administrative
APPLICATION TYPE: Site Plan Approval

NATURE OF REQUEST:

Permitted Use: Mixed Use Commercial.

OVERVIEW:

On January 28, 2025, Hyrum City received an application for Site Plan Approval for The Market 1860 project which includes two identical mixed-use buildings consisting of first-level commercial space and second-level residential rental units. The footprint of each building is planned at 40-feet by 70-feet. The first-floor commercial space in each building will be divided into two (north and south) units. Parking and landscape areas will be constructed behind the buildings and open patio and outdoor dining areas will wrap the building perimeter. The second level in each building will consist of six and seven short-term hotel style residential units with single and double-bed units. Each building will also include a common laundry facility on the same floor.

On May 1, 2025, the City Council continued the application to coordinate addition Site Plan Approval Conditions. See responses attached and referenced herein as Exhibit "A".

Note: UDOT has stated support for the change of curbing and access driveway approaches.

ATTACHMENTS:

- 1. May 1, 2025 City Council Memorandum
- 2. Staff Evaluations March 25, 2025/February 13, 2025 with updated Applicant Submittals:
- 3. Storm Water Calculations: Received April 21, 2025

Friday, May 9, 2025 Hyrum City 60 West Main Street Hyrum, UT 84319

Subject: Market 1860 Site Plan Approval Condition Requests

Attn. Hyrum City Mayor and Council Members

Following are responses from the owners of the proposed Market 1860 project concerning the condition items given by the city at the City Council meeting, held May 1, 2025

1. Coordinate the building floor plans such that the kitchen hood does not exit the side of the building (applicable to Building #2, specifically).

Owner response:

If kitchens are built into the commercial spaces, all building codes and requirements for commercial kitchens will be met and kitchen fans will not exit the east side wall of building #2.

2. Address the building elevation drawings so that the street fronts fit with the neighborhood. ..discussed Section 17.08.100 (F.) "... harmonious and beneficial relation among the buildings and uses in the area..." and Section 17.45.080 Residential Entrances ...residential units in a mixed-use site, shall provide access to the residential units from the in true ground floor and shall lead from the main travelway or common area. Such entrances shall be designed with separate architectural features, such as varied facade depth and color, canopies, stairs, etc.

Owner response:

The buildings shown on the proposed plan are relative in height and size to surrounding homes and commercial buildings and are similar in design style.

All entrances to the buildings, including entrances to the residential upper floor, will have extra architectural features such as awnings, varied colors, varied facade, etc. These entrances will, as all areas of the project, meet fire code requirements.

- 3. Replace the deep gutter section on Main Street with a 6" curb and gutter.

 Owner response: We understand and agree with the benefit of lowering the curb and gutter

 After site plan approval, if the cost is reasonable, we are willing to work with the city and UDOT on this project.
- 4. Developer and Hyrum City to jointly petition UDOT for a reduced width necessary for the development without extending westward beyond the commercial driveway.
 Owner response: After site plan approval, we would appreciate the City's help to petition
 UDOT to reduce the width of the proposed driveway and separate the commercial and residential driveways. This is an important matter for a variety of reasons.

- **5.** Trailer parking is not to be allowed in the parking lot or overnight on Main Street. Owner response: All short term rental agreements include stipulations on several items including parking. Our agreements signed by guests, will include information about the parking spaces they are allowed to use and restrictions concerning trailer parking on the lot or on the street. This is not only important to our neighbors but it is also important to the business owners and the guests.
- 6. The condition was made that at the south end of the parking lot the hard surface at the south part of the snow removal area should be removed (would recommend 5 ft wide) to allow for landscape trees to be planted to screen the residential property.

 Owner response: The 5 foot landscape area including trees will be added to the landscape design plan on the south edge of the parking lot
- 7. Address the concern about loud parties, and loud music, etc. that was a concern with the westerly building based on the project narrative describing expected building use.

 Owner response: As a C2 mixed use space zone, the main floor commercial areas could be used by a variety of businesses as stated in the code. 17.45.020 Use Regulations

 The narrative was given as an example of some uses suggested by Hyrum residents as to what they would like to see in the downtown area and by business owners that may be interested in the space.

Lease agreements of these spaces cannot be pursued until after site plan approval and no decisions have been made at this point about any specific types of businesses that would be placed in specific areas of the buildings. The enclosed patio was placed as an option for uses such as a restaurant with outdoor dining, a cafe or book store with outdoor seating, an outdoor space for an art class, etc.

Any businesses owners doing business in the proposed commercial spaces will be required to adhere to all city noise ordinances and fire code occupancy requirements. All businesses will be required to close at a designated reasonable time not only for the benefit of the neighbors but also that of the guests. And all businesses will have staff on duty to monitor the business during business hours.

The Owner recognizes the concerns of the City with respect to the conditions, and has herewith provided the above written response, and respectfully requests that the Market 1860 Site Plan be approved upon approval of the response to the conditions noted at the next scheduled City Council Meeting.

Sincerely,

Andrea Nielsen

Auch Miles



MEMORANDUM

TO: Mayor Miller, City Council Members

FROM: Tony Ekins, City Planner

SUBJECT: <u>Jesse Elsmore, Jardine Builders, LLC</u> – To request site plan approval for two (2)

two-story mixed-use buildings located at 139 West Main Street & 127 West Main

Street, consisting of 0.96 acres.

DATE: April 28, 2025

CITY COUNCIL MEETING: May 1, 2025
CITY COUNCIL ROLE: Administrative
APPLICATION TYPE: Site Plan Approval

NATURE OF REQUEST:

Permitted Use: Mixed Use Commercial.

OVERVIEW:

On January 28, 2025, Hyrum City received an application for Site Plan Approval for The Market 1860 project which includes two identical mixed-use buildings consisting of first-level commercial space and second-level residential rental units. The footprint of each building is planned at 40-feet by 70-feet. The first-floor commercial space in each building will be divided into two (north and south) units. Parking and landscape areas will be constructed behind the buildings and open patio and outdoor dining areas will wrap the building perimeter. The second level in each building will consist of six and seven short-term hotel style residential units with single and double-bed units. Each building will also include a common laundry facility on the same floor.

On March 27, 2025, the Planning Commission recommended approval of the mixed-use to the City Council for the main level commercial space and second level short-term hotel style residential units, as the *Hotel or Motel* use is specifically defined and permitted in the Commercial Zone C-2:

HCC 17.04.020 Definitions

Hotel or motel - "Hotel" or "motel" means any building which is designed to offer temporary abiding space to individuals and family. To be classified as a hotel or motel, it shall contain individual guest rooms and provide services such as linens and furnishings.

HCC 17.45.020 Use Regulations

A. Permitted Uses:

4. Hotel or motel

Following the Planning Commission's recommendation on March 27, 2025, the Scope Narrative revised the second level use from "short-term hotel style residential units" to "hospitality boutique units". Hyrum City Code does not specifically describe "Hospitality Boutique" and staff recommends the City Council have a thorough discussion to determine if a "Hospitality Boutique" meets the qualifying Hotel or motel requirements as defined in HCC 17.04 Definitions and HCC 17.45 Commercial Zone C-2.

PLANNING COMMISSION RECOMMENDATION:

On February 13, 2025, the applicant presented to the Planning Commission a request for site plan approval. The Planning Commission recommended the requestor to respond to staff evaluation first review comments and schedule a second meeting. On March 27, 2025, the Planning Commission held a second meeting and made a motion to recommend approval to the City Council (4-0 Vote) for site improvements, first-level commercial, and second-level short-term hotel based on applicant submittals, staff evaluations, staff comments are addressed, and ADA parking requirements are accommodated.

CITY COUNCIL RESPONSIBILITY:

- 1. The City Council should have a thorough discussion of the site plan, staff comments, and specifying conditions and requirements for approval.
- 2. The City Council may approve, disapprove, approve with additional conditions and requirements, or require the requestor to return to the Planning Commission for significant revisions.

STAFF RECOMMENDATION:

- 1. Hyrum City Code does not specifically describe "Hospitality Boutique" and staff recommends the City Council have a thorough discussion to determine if a "Hospitality Boutique" meets the qualifying Hotel or motel requirements as defined in HCC 17.04 Definitions and HCC 17.45 Commercial Zone C-2.
- 2. Parcels 01-050-0023 and 01-050-0024 be combined and recorded to the Cache County Recorder's Office as one (1) parcel prior to building or sign permits issued.

STIPULATIONS:

- 1. Upon site plan approval, construction drawings will be required for any public works construction or improvement for approval by the City Engineer.
- 2. Approval of the site plan shall be effective for one (1) year from the date of final approval by the City Council. If construction has not begun during that period, or an extended by the City Council, the site plan approval is void and applicant shall be required to submit a new site plan for review and approval subject to the then existing provisions of this code.
- 3. Changes in use from those approved by this Site Plan Approval will require a site plan or site plan waiver as regulated by HCC 17.08 Administration and Enforcement.
- 4. The applicant will submit and obtain a building permit as regulated by HCC 15.08 Building and Construction.
- 5. The applicant will submit and obtain a sign permit as regulated by HCC 17.72 Signs.

FINDINGS OF FACT:

- 1. The property is located in the Commercial Zone C-2.
- 2. Mixed Commercial Uses are a permitted use in the Commercial Zone C-2.

ATTACHMENTS:

- 1. Staff Evaluations: March 25, 2025 and February 13, 2025
- 2. Storm Water Calculations: Received April 21, 2025



PLANNING COMMISSION STAFF EVALUATION – SECOND REVIEW

APPLICATION NO: 25-002A

APPLICANT: Jesse Elsmore, Jardine Builders, LLC

PROPERTY OWNER: Andrea Nielsen / John Kimball Jr & Virginia Francis **PROPERTY ADDRESS:** 139 West Main Street & 127 West Main Street

PARCEL NUMBER: 01-050-0023 & 01-050-0024

PARCEL AREA: 01-050-0023 = .33 Acres / 01-050-0024 = 0.63 Acres (0.96 Acres)

ZONE: Commercial Zone C-2

DATE: March 25, 2025

PLANNING COMMISSION MEETING: March 27, 2025 (Special Meeting)
PLANNING COMMISSION ROLE: Recommending Body to City Council

APPLICATION TYPE: Site Plan Approval

NATURE OF REQUEST:

Permitted Use: Mixed Use Commercial.

CURRENT ZONING DISTRICT:

Commercial Zone C-2: The C-2 Downtown Mixed-Use Zone is designed to preserve the mixed-use nature and feel of downtown Hyrum, providing for office, commercial, and residential uses within a mixed-use setting. A major objective of the C-2 Downtown Mixed-Use Zone is to create aesthetically pleasing streetscapes with landscaping that buffers sidewalks from major vehicular traffic ways, landscape features, recreational amenities, and social gathering areas that promote a walkable community.

OVERVIEW:

(updated). The applicant would like to develop the property with two identical mixed-use buildings consisting of first-level commercial space and second-level residential rental units. The first-floor commercial space in each building will be divided into two (north and south) units. Parking and landscape areas will be constructed behind the buildings and open patio and outdoor dining areas will wrap the building perimeter. See Applicant's Scope Narrative additional site improvements.

UTILITIES: Existing Power, Sewer, Water, Irrigation servicing 127 West Main Street are for a Single-Family Residential Home. Upgrades will need to be built/installed.

STAFF COMMENTS:

Requestor responses in red

Planning and Zoning:

1. The Planning Commission recommend site plan approval to the City Council based on the applicants compliance with staff evaluations and staff comments, applicant

submittals, and any additional Planning Commission recommendations. The City Council can require revisions as necessary.

- Understood
- 2. The dwelling unit(s) in each building engage the following code requirements to require a separate sewer lateral to each building for a total of two (2) sewer laterals:
 - a. Hyrum City Code 17.04.070 Definitions: "Dwelling unit" means one or more rooms in a dwelling, apartment hotel or apartment motel, designed to be occupied by one family for living and sleeping purposes.
 - b. Hyrum City General Requirements and Specifications for Sanitary Sewer Installations 5.2.1.C. Design Regulations: Each dwelling unit shall be served by an individual lateral.
 - c. (Reference Sewer Department Comment #1 in Staff Evaluation First Review)
 - Separate sewer laterals have been provided and a 3rd was added for the first floor commercial units in the East building where potential restaurant spaces may be built-out

Engineering:

- 1. See attached comments contained on Site Plan Second Review.
 - Comments and subsequent direction from the City have been incorporated in this update of the Site Plan
- 2. Provide stormwater calculations and data. Table A was not provided in the submittal, see Engineering comments on sheet: 2 of 10.
 - Storm water calculations are included with this Site Plan update
- 3. The utility plan is lacking information, see Engineering comments on sheet: 2 of 10.
 - Utility plan has been updated to include 8-inch water lateral and 4 water meters
- 4. Streetscape requirements are not being shown as required by Hyrum City Code 17.45.090 Streetscape Features.
 - Project consists of a 165-foot street frontage which requires 6 streetscape features (1 for every 30 feet). Site plan includes and calls out the following streetscape features: 2 trees, 2 park benches, and 2 planter areas, 1 existing street lamp

Sewer Department:

- 1. Each dwelling unit shall be served by an individual lateral per Construction Standard 5.2.1.C. Drawings show one shared lateral for two dwellings which is not to standard. (Reference Sewer Department Comment #1 in Staff Evaluation First Review)
 - Separate sewer laterals have been provided and a 3 rd was added for the first floor commercial units in the East building where potential restaurant spaces may be built-out.

Water / Irrigation Department:

- 1. If the original water meter and lateral is used as part of the total of four (4) new, the existing water meter and lateral will need to be updated to comply with Hyrum City Code 13.04 Water System; Hyrum City General Requirements and Specifications for Potable Water Mains, Service Lines and Secondary Pressure Irrigation Installations. (Reference Water / Irrigation Comment #1 in Staff Evaluation First Review)
 - Utility plan has been updated to include 4 new water meters, existing meter will be removed and lateral abandoned per city requirements
- 2. If the existing water meter and lateral is not used as part of the total of four (4) new, the existing water meter shall be removed, and the existing water lateral will be removed at main line and the main line be capped. (Reference Water / Irrigation Comment #1 in Staff Evaluation First Review)
 - Site plan shows existing water meter will be removed and lateral abandoned per city requirements.

STAFF EVALUATION FIRST REVIEW - Planning Commission: February 13, 2025

Requestor responses in red

Planning and Zoning:

- 1. Staff supports a Mixed Use of Commercial and Hotel as permitted in HCC 17.45.020 Use Regulations.
 - Yes!
- 2. Staff did not receive the required Lighting Plan submittal. HCC 17.45.120 requires that each site plan shall include a lighting plan.
 - Lighting Plan has since been completed and submitted with Site Plan documents
- 3. Staff does not support the west building crossing the east property line of property parcel: 01-050-0023 as proposed on the site plan and recommends combining the two property parcels. HCC 17.45.050 Yard Regulations Commercial Use may permit a zero-yard setback at a property line at a qualifying location; however, it does not qualify a building to encroach beyond a property parcel. If the applicant desires to relocate the said building to a conforming location on the parcel, Staff will request the following revisions to the site plan:
 - a. Each property parcel is serviced by separate power, sewer, water and water meters, fire line (if required), and irrigation connections to main lines; and
 - Parcel 0023 and 0024 are owned by individual Market 1860 partners; partners
 are in process of selling & transferring ownership to the Market 1860 entity which
 will result in single ownership. Applicant requests that Commission allow Site
 Plan Approval on condition that this is completed before issue of building permit
 - b. Cross access agreements need to be prepared and recorded to each property parcel for UDOT driveway approach interior parking.
 - Cross access agreement has been addressed with UDOT and per UDOT will not be signed until issue of building permit
- 4. The site plan proposes parking stalls to be a minimum of eight (8) feet in width and does not identify the proposed parking stall depth. In the C-2 Zone, HCC 17.45.210.C. Off-Street Parking Special Requirements requires the site plan to provide nine (9) feet by twenty (20) feet parking stalls.
 - Parking stall sizes have been revised to 9 feet wide by 20 feet deep
- 5. The applicant did not submit a total parking stall count on the site plan. Staff supports the required parking of one (1) space per each unit, room, or guest accommodation as regulated in HCC 17.45.200 Off Street Parking Specific Requirements which reserves a total of twelve (12) parking spaces for the Hotel Use.
 - Parking totals 54 stalls, includes 1 ADA stall & 1 ADA Van Accessible Stall
- 6. Without the applicant providing the exact square feet of proposed commercial uses (excluding storage areas, restrooms, office areas, etc.) Staff cannot verify the required off-street parking requirements regulated in HCC 17.45.200 Off-Street Parking Specific Requirements to conclude parking requirements.
 - Total commercial floor space is 4,050 SF (2,100 SF in West, 1,950 SF in East)
 - Breakdown of estimated commercial use is as follows:
 - Retail / Market = 2,100 SF (requires 1 per 250 SF = 8 stalls)
 - Single Office = 224 SF (requires 1 per employee = 1 stall)
 - Restaurant / Café = 1,275 (requires 1 per 40 SF = 32 stalls)
 - Hotel Space = 13 units (requires 1 per unit = 13 stalls)
 - Total stalls required = 54
- 7. The site plan shows the cedar fence terminating at the south portion of the paved parking lot. The parking lot is still facing adjoining neighbor properties. HCC 17.45.050 Yard Regulations Commercial Use requires the fence to continue south on both east and west property line and along the south property line enclosing the development.

- Cedar fence continues along west, south, and east property lines
- 8. The Trash Enclosure requirements in the C-2 Zone, HCC 17.45.055 Trash Enclosure Regulations Commercial Use requires that enclosures shall be located away from main traffic areas and sheltered from street sight as much as possible. Staff recommends relocating the Trash Enclosure from the current location within the main traffic area and street sight to a conforming location on the site.
 - Trash enclosure relocated to back of parking lot, hidden from street view behind west building, >50' from residential zoning. Have assumed doublesize dumpster enclosure in site plan
- 9. A building permit will be required for building structures as regulated by HCC Section 15.08 Building Permits.
 - Understood
- 10. All construction shall comply with Hyrum City Design Standards and Construction Specifications.
 - Understood

Engineering:

- 1. See comments contained on Site Plan.
 - Site Plan updated per comments

Fire Department:

- 1. Water flow, we need 1750 GPM at the hydrant for 2 hours. If it is less than that the entire building will need to be sprinklered.
 - Fire sprinklers are planned for each building, on both floor levels
- 2. The upstairs hotel/apartments is a R-1, it is required to have Sprinklers.
 - Fire sprinklers are planned for each building, on both floor levels
- 3. We need 26' of clearance on driveway and on all sides of parking for fire apparatus access lot see drawings.
 - 26' driveway clearance has been provided and extends to back side of buildings
- 4. What type of building construction?
 - Wood frame construction with structural steel elements
- 5. Each unit will need to be reviewed and inspected as built out.
 - Understood

Parks Department:

- 1. No comments or concerns.
 - Yes!

Power Department:

- 1. Staff requests the applicant contact the Power Department to schedule an initial onsite meeting to verify all existing electrical utilities.
 - Understood, this will happen immediately following site plan approval
- The applicant must complete and submit the required Commercial Structure Load Data Sheet to the Power Department. The Load Data Sheet is available in Section 7 General Requirements and Specifications for Electrical Installations in Hyrum City Design Standards and Construction Specifications.
 - Load data sheet has been completed and will be submitted immediately following site plan approval
- 3. All construction specific to electrical work shall verify compliance with Section 7 General Requirements and Specifications for Electrical Installations in Hyrum City Design Standards and Construction Specifications.
 - Understood, electrical systems will be designed by Sine Source Engineering
- 4. Applicant must maintain and clearance of 10 feet around the interior overhead service line on the property. At the applicant's request, the Power Department can provide visual ribbon indicators on the overhead line to help maintain visual clearance.
 - Understood, visual ribbon indicators will be requested during construction

Road / Stormwater Department:

- 1. An NOI is generally not required for disturbances less than one (1) acre that are not part of a larger common plan of development project, however, it is the responsibility of the applicant to confirm any and all exemptions pursuant to HCC 13.18.110 Notice of Intent (NOI) Exemptions. While an NOI is not typically required, applicant is responsible to control Stormwater and Erosion & Pollution on and from the site.
 - If required, contractor will file an NOI; regardless; BMPs will be installed and maintain during construction until all permanent landscaping and stormwater systems are completed
- 2. All public rights of way permits, construction and improvements, and traffic control on Main Street are the Powers and Duties of Utah Department of Transportation.
 - Understood, applicant has already engaged in permits and discussions with UDOT regarding this project and will continue to do so
- 3. Hyrum City may enforce provisions and all other ordinances relating to the maintenance and use of streets, culverts, drains, ditches, waterways, curbs, gutters, sidewalks and other public ways; and the repair or cause to be repaired, all defects coming to the Hyrum City Department of Streets attention and make reasonable precautions to protect the public from injuries due to such defects pending their repair pursuant to HCC 2.36.030. Powers and Duties.
 - Understood

Sewer Department:

- Staff recommends the engineer verify the existing sewer lateral and design a new lateral for peak flow for both buildings, and demonstrate on the plans the material, quality and specifications as regulated by HCC 13.12.200 Service and Other Pipes – Material, Quality and Specifications – Alteration or Inspection.
 - Understood, sewer line size shown on Site Plan, existing laterals to be abandoned
- For any proposed use or future use that will introduce or cause to be introduced into the Publicly Owned Treatment Works (POTW) or any pollutant or wastewater which causes to pass through or interference, the applicant must comply with HCC Section 13.13 Wastewater Pretreatment, whether or not the source is subject to categorical Pretreatment Standards or any other National, State or Local Pretreatment Standards for requirements.
 - Understood, additionally it is anticipated that a grease trap will be provided for restaurant spaces
- 3. All construction specific to sanitary sewer shall verify compliance with Section 5 General Requirements and Specifications for Sanitary Sewer Installations in Hyrum City Design Standards and Construction Specifications.
 - Understood

Water / Irrigation Department:

- 1. HCC 13.04.180 Separate Connections Required for Each User regulates that each service user cannot be supplied from the same service pipe, connection or water meter unless special permission for such combination usage has been granted by the governing body. Staff recommends that each user have its own water service (tentatively 2 meters for main level commercial uses, and 1 meter for upper-level hotel uses for each building).
 - Understood, owner would like to request a single meter for each floor-level in each building (4 meters total)
- 2. Staff recommends that all meters be installed in a meter vault for multiple meters for 3/4" to 1-1/2" meters as approved in Section 6 of the Hyrum City General Requirements and Specifications for Potable Water Mains, Service Lines, and Secondary Pressure Irrigation Installations.

- Understood
- 3. Staff recommends that each service to commercial uses be a minimum of 1-1/2" to ensure adequate future flows.
 - Understood
- 4. If the mixed commercial use and hotel use requires a fire sprinkler system required by the International Fire Code and requires an additional fire line connection to the water main, the fire line connections shall comply with Section 6 Hyrum City General Requirements and Specifications for Potable Water Mains, Service Lines, and Secondary Pressure Irrigation Installations.
 - Understood
- 5. All construction specific to sanitary sewers shall verify compliance with Section 5 General Requirements and Specifications for Potable Water Mains, Service Lines, and Secondary Pressure Irrigation Installations in Hyrum City Design Standards and Construction Specifications.
 - Understood

PLANNING COMMISSION RESPONSIBILITY:

- 1. Site plan approval is a function of the Planning Commission which has a wide latitude in specifying conditions and requirements for approval.
- 2. The Planning Commission should have a thorough discussion of the site plan, staff comments, and specifying conditions and requirements for approval.
- 3. The Planning Commission is a recommending body to the City Council and should be specific in their motion to the City Council.

STAFF RECOMMENDATION:

1. The Planning Commission recommend site plan approval to the City Council based on the applicants compliance with staff evaluations and staff comments, applicant submittals, and any additional Planning Commission recommendations. The City Council can require revisions as necessary.

STIPULATIONS:

- 1. The City Council may approve, disapprove, approve with additional conditions and requirements, or require the requestor to return to the Planning Commission with revisions; or require the applicant to return revisions to the City Council.
- 2. Final Approval must be obtained from the City Council. Following final approval, the requestor will provide final approval documents to staff for the issuance of a permitted use permit, issuance of Hyrum City Approval final approval documents, and issuance of notification to attend a pre-construction meeting.

FINDINGS OF FACT:

- 1. The property is located in the Commercial Zone C-2.
- Mixed Commercial Uses are a permitted use in the Commercial Zone C-2.

ATTACHMENTS:

- 1. Cache County Parcel and Zoning Viewer Aerial View
- 2. Hyrum Market 1860 Scope Narrative Updated
- 3. Hyrum Market 1860 Conceptual Building Exterior Elevation Updated
- 4. Hyrum Market 1860 Site Plan Submittal Updated
- 5. Hyrum Market 1860 Lighting Plan Updated

Cache County Parcel and Zoning Viewer – Aerial Image



Hyrum City 60 West Main Street Hyrum, UT 84319

Subject: Market 1860 Site Plan Approval

Attn: Hyrum City Council Members

Scope Narrative

The Market 1860 project includes two identical mixed-use buildings consisting of first-level commercial space and second-level residential rental units. The footprint of each building is planned at 40-feet by 70-feet with a conventional peaked roof line down the longitudinal center of the building. The first-floor commercial space in each building will be divided into two (north and south) units. Parking and landscape areas will be constructed behind the buildings and open patio and outdoor dining areas will wrap the building perimeter.

The two commercial spaces in the west building will consist of open classrooms, conference, and market space with a small demonstration kitchen, restrooms, storage and mechanical room. The two units will be constructed with an open hallway so that the spaces can be used simultaneously if needed. One purpose of these units is to provide a place for community events available for rent or use depending on the function or activity. It may be utilized by home-school groups, small training classrooms, farmer's or craft markets, non-profit organizations, and more.

The commercial space in the east building will consist of two café, bakery, or restaurant-type lease units. The owner's intent is to construct the two units as shell-space and allow tenants the opportunity to finish the space according to business needs.

The second level in each building will consist of six and seven (13 total) hospitality boutique units with single and double bed units. Each building will also include a common laundry facility on the same floor. The hospitality units will be accessible by two stairways and secured first-level exterior doors located on the back and shared patio side between buildings. An elevator will be provided in the East Building for ADA access to the second floor.

Landscaping Plan

Landscaping will include 15-foot-wide green space along the east and west perimeter of the site as well as small landscape areas near the buildings. Vegetation will include trees, lawn, native grass, flowers, and shrubs. Landscaping will be meticulously maintained to attract renters to the short-term rental units and commercial businesses.

Storm water will be managed on site by surface collection to catch basins and dispersed through underground infiltration basins.

Market 1860 -

Ownership

Annette Francis (Hyrum, UT)

Andrea Nielsen (Hyrum, UT)

Amy Knight (Park City, UT)

Design-Builder

Jardine Builders, LLC (Millville & Centerville, UT)

Architect & Engineers

Architect – Gary Hunt Architect, P.C. (Syracuse, UT)

Civil & Structural – Beyler Engineering (Lakewood, WA)

Mechanical – VBFA (Logan, UT)

Electrical – Sine Source Engineering (Logan, UT)

Geotechnical – Civil Solutions Group (Logan, UT)



East Building North Elevation

EXISTING UTILITIES

LEGEND

EX WATER LINE

EX METER

EX GAS LINE

EX U/G POWER LINE

EX LIGHT POLE

EX POWER POLE

EX SEWER PIPE

EX SANITARY MH/CO

EX PROPERTY BOUNDARY

EX MINOR CONTOURS

EX ASPHALT

EX CONCRETE

EX CURBING

EX SIGN

CONCRETE

BUILDING

WOOD FENCE

WHEEL STOP

POND BOTTOM

RIP RAP PAD

ROOF DOWNSPOUT

STORM CLEANOUT

AREA DRAIN / CATCH BASIN TYPE 1

FLOW ARROW/SLOPE ARROW

SANITARY SEWER LATERAL

SANITARY CLEANOUT

WATER SERVICE / FIRE LINE

IRRIGATION CONTROL VALVE

WATER METER

LIGHT POLE

STORM DRAINAGE PIPE

LANDSCAPING

EX WOOD FENCE

ASPHALT PAVEMENT

EX OVERHEAD POWER LINE

EX U/G COMMUNICATION LINE

SECTION 03, TOWNSHIP 10 N., RANGE 01 E., S.L.B. & M. **CIVIL ABBREVIATIONS**

HYRUM MARKET 1860

CIVIL IMPROVEMENT PLANS PARCEL #'s 01-050-0023 & 01-050-0024



1,060 SF (0.024 Ac)

29,653 SF (0.673 Ac)

12,410 SF (0.285 Ac)

30,983 SF (0.918 Ac)



IMPERVIOUS / PERVIOUS SITE AREAS

PROPOSED IMPERVIOUS ONSITE:	
BUILDING #1	3,400 SF (0.078 Ac)
BUILDING #2	2,800 SF (0.064 Ac)
ASPHALT ACCESS/PARKING LOT (PGHS)	23,065 SF (0.523 Ac)
CONCRETE (PGHS)	325 SF (0.007 Ac)
CONCRETE	63 SF (0.001 Ac)

PROPOSED PERVIOUS ONSITE LANDSCAPING

TOTAL PROPOSED IMPERVIOUS:

PROPOSED IMPERVIOUS OFFSITE ASPHALT ACCESS (PGHS) 49 SF (0.001 Ac) CONCRETE DRIVEWAY APPROACH (PGHS) 920 SF (0.021 Ac) CONCRETE CURBING (PGHS) 176 SF (0.004 Ac) CONCRETE SIDEWALK 140 SF (0.003 Ac) CONCRETE FOOT BRIDGES 45 SF (0.001 Ac) 1,330 SF (0.030 Ac)

TOTAL PROPOSED (PGHS): 24,535 SF (0.563 Ac) IS THE PLAN STILL TO PROVIDE SIDEWALKS IN BETWEEN THE BUILDINGS? THESE NEED TO BE SHOWN AND INCORPORATED INTO ORMWATER CALCULATIONS. DRAINAGE PATTERNS NEED TO B SHOWN FOR THESE AREAS. ADA PATHS NEED TO BE PROVIDED AT A

PERCOLATION TESTS BEEN DONE TO SHOW THAT THE WATER WILL **DRAIN DOWN WITHIN 72 HOURS?**

WILL THE PROXIMITY OF THESE BASINS CAUSE ANY ISSUES TO THE **NEARBY STRUCTURES ON THE OTHER LOTS?**

SHEET INDEX

1- COVER SHEET / SITE PLAN 2- STORM DRAINAGE AND UTILITY PLAN 3- NOTES AND DETAILS SHEET 1 4- NOTES AND DETAILS SHEET 2 5- NOTES AND DETAILS SHEET 3 6- NOTES AND DETAILS SHEET 4 7- NOTES AND DETAILS SHEET 5

> 9- NOTES AND DETAILS SHEET 7 10- NOTES AND DETAILS SHEET 8

6' ENCLOSURE AROUND NEW HVAC EQUIPMENT (TYF

2 - 40'X70' 2-STORY WOOD FRAME STRUCTURES MAIN FLOOR RETAIL, UPPER FLOOR SHORT TERM RENTAL UNITS 3 - 10'X20' STORAGE SHEDS 34' MAX BUILDING HEIGHT

SITE DATA

PIN 01-050-0023 ANDREA NIELSEN

PIN 01-050-0024 JOHN & VIRGINIA FRANCIS 450 W 2000 N MAPLETON, UT 84664

SITE ADDRESS: 127 WEST MAIN STREET, HYRUM, UT

ZONING – C-2 OVERLAY RESIDENTIAL NO SETBACKS REQUIRED.

STANDARD PARKING: **ADA PARKING**

I CERTIFY THAT I HAVE EXAMINED THIS PLAT AND FIND IT TO BE IN GENERAL COMPLIANCE TO THE CITY STANDARDS

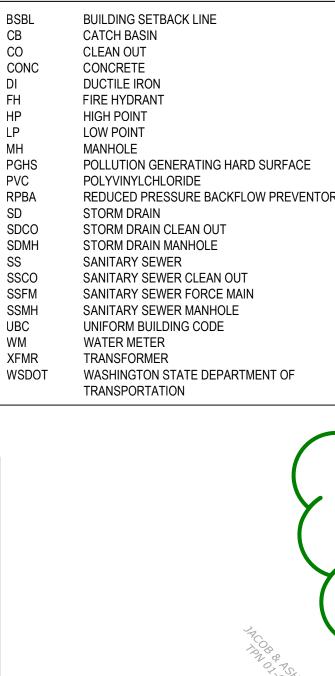
8- NOTES AND DETAILS SHEET 6

BUILDING INFORMATION

PARCEL NUMBER(S): 3788 ECKER HILL DR. PARK CITY, UT 84098

15 FT SIDE SETBACKS AGAINST

CITY ENGINEER



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O WASHINGTON STATE DEPARTMENT OF

24' WIDE X 11' DEEP -TRASH DUMPSTER. FENCE SCREENING PER CITY OF HYRUM

ORDINANCE 13-02

6' CEDAR FENCING (TYP.)

STALL (TYP

N02°00'25"E - 177.49'

01-050-002

ANDREA NIELSEN 3788 ECKER HILL DR. PARK CITY, UT 84908

01-050-0024 OHN & VIRGINIA FRANCIS 450 W 2000 N PROPOSED ADA MAPLETON, UT 84664

STALL/LOADING

S01°58'56"W - 332.17 PARKING LOT DOWN LIGHT (TYP.) PROPOSED MAINTENANCE SHED J

ALL SIGNS PER UDOT SN SERIES STANDARD DRAWINGS.

STORM RETENTION POND (TYP)

SEE SHEET 2 FOR STORM DRAINAGE PLAN)

STANDARD PARKING -

STALL (TYP)

UDOT UTILITY SPECIFICATIONS & NOTES

NOTES? CIRCLE WITH

GREY FILL

I. CONTRACTOR TO USE TRENCHLESS CONSTRUCTION UNLESS SUFFICIENT REASON IS GIVEN FOR OPEN TRENCH 2. ALL UTILITY TRENCHES TO BE CUT AT RIGHT ANGLES TO TRAVEL LANES. TEMPORARY PATCHES REQUIRE AT LEAST 2-INCH ROTOMILING APPROACHING AND LEAVING THE PATCH IN ALL TRAVEL LANES IMPACTED BEFORE FINAL

3. ALL FINAL PARALLEL SAWCUT LINES OR ROTOMILLING MUST BE LOCATED EITHER AT DESIGNED LANE LINES OR DESIGNED CENTER OF LANE. SAWCUTS MUST BE CLEANED AND TACK-COAT APPLIED BEFORE ASPHALT PLACEMENT

T-PATCH ASPHALT PLACEMENT PER APWA PLAN NO. 255.

4. ALL TRENCHES FOR LATERALS WITHIN A 100FT DISTANCE MUST HAVE 2 INCH MILL AND REPLACED AS A SINGLE PATCH. SINGLE LATERALS REQUIRE A 2 INCH MILL FOR 20FT EACH DIRECTION

ALL ASPHALT CONSTRUCTION WITHIN UDOT RIGHT-OF-WAY TO MATCH EXISTING. HOT MIX ASPHALT (HMA) SHALL BE PG-GRADE 64-34 ASPHALT BINDER, 1/2 INCH NOMINAL MAX, 7-75-115 GYRATION PER UDOT STANDARD SPECIFICATION 02741; OVER 6 INCHES UNTREATED BASE COURSE (UTBC) PER UDOT SPECIFICATION 02721; OVER 11 INCHES GRANULAR BORROW (GB) PER UDOT SPECIFICATION 02056 (WHICHEVER IS GREATER). PROVIDE DOCUMENTATION OF COMPACTION FROM A UDOT-QUALIFIED LABORATORY.

6. PAVEMENT SEALING - CHIP SEAL TYPE II WITH EMULSION LMCRS-2 PER UDOT STANDARD SPECIFICATION 02785

(ESTIMATED APPLICATION RATE OF 0.45 GAL/SQ YD) IS REQUIRED ON ALL NEW PAVEMENT WITHIN UDOT RIGHT-OF-WAY.

ALL CONSTRUCTION WITHIN THE UDOT RIGHT-OF-WAY SHALL CONFORM TO THE MOST CURRENT UDOT STANDARD (INCLUDING SUPPLEMENTAL) DRAWINGS AND SPECIFICATION. APPLICABLE UDOT STANDARD AND SUPLEMENTAL DRAWINGS ARE INCLUDED IN THIS PLAN SET

THE CONTRACTOR IS TO OBTAIN AN ECROACHMENT PERMIT FROM THE APPLICABLE UDOT REGION PERMIT OFFICE PRIOR TO COMMENCING WORK WITHIN THE UDOT RIGHT-OF-WAY. WORKING HOUR LIMITATIONS WILL BE LISTED IN THE LIMITATIONS SECTION OF THE ENCROACHMENT PERMIT.

3. UDOT RESERVES THE RIGHT, AS ITS OPTION, TO INSTALL A RAISED MEDIAN ISLAND OR RESTRICT THE ACCESS TO A RIGHT-IN OR RIGHT-OUT AT ANY TIME.

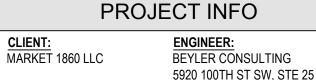
4. OWNER, DEVELOPER, AND CONTRACTOR ARE RESPONSIBLE FOR ANY DAMAGES DIRECTLY OR INDIRECTLY WITHIN THE UDOT RIGHT-OF-WAY AS A RESULT OF DEVELOPMENT ACTIVITIES.

ALL SIGNS INSTALLED ON THE UDOT RIGHT-OF WAY MUST BE HIGH INTENSITY GRADE (TYPE XI SHEETING) WITH A B3 SLIP BASE. INSTALL

COMPLY WITH THE REQUIREMENTS OF UTAH CODE 17-23-14 (DISTURBED CORNERS - COUNTY SURVEYOR TO BE NOTIFIED -COORDINATION WITH CERTAIN STATE AGENCIES).

UDOT RIGHT-OF-WAY (MAIN STREET) NOTES

OWNER, DEVELOPER, AND/OR CONTRACTOR IS REQUIRED TO HIRE AN INDEPENDENT COMPANY FOR ALL TESTING WITHIN THE UDOT



LAKEWOOD, WA 98499

CONTACT: LANDON BEYLER, P.E. TEL: 253-984-2900

HYRUM CITY CULINARY WATER **AUTHORITY**

UTILITIES

(IN FEET

1 inch = 30 ft.

CONSTRUCT PIPED

(BRIDGE STYLE) PER

APWA PLAN NO. 229

WITH CONCRETE TIE-IN

70'

40' X 70'=2,800 SF

PROPOSED SIGN-

- ADA CURB RAMP

40' X 70'=2,800 SF

BLDG #2 HEIGHT=34'

- DELIVERY UNLOADING ZONE

SEE SHEET 2 FOR UTILITY PLAN -

6" RAISED CURB (TYP)

LOCATION

BLDG #1 HEIGHT=34'

DRIVEWAY APPROACH

MANHOLE, OWNER TO

EX SSMH

RIM = 4695.84

(E+W)=4689.89

REMOVE EXISTING

Block 15, Plat "A"

EXISTING SCHOOL ZONE

APPROACH, REPLACE WITH

MATCHING CURB, GUTTER,

AND SIDEWALK PER UDOT

STANDARD DRAWINGS

REPLACE W/ NEW 5' WIDE

FOOT BRIDGE PER UDOT:

- CONSULT WITH TREE SURGEON

REGARDING HEALTH OF EXISTING

TREE. REPLACE IF NECESSARY.

REMOVE EXISTING FOOTBRIDGE AND

STANDARDS.

SIGN & WARNING LIGHTS

PARALLEL PARKING SIGN.

COORDINATE WITH UDOT

PROVIDE MAINTENANCE PLAN TO PREVENT

STORMWATER OVERFLOW UNDER DRIVE APPROACH.

GRAVITY HYRUM CITY SEWER AUTHORITY

HYRUM CITY POWER

52 STALLS 2 STALLS 54 STALLS CITY ENGINEER APPROVAL JOB NUMBER

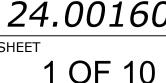
DATE

1 OF 10

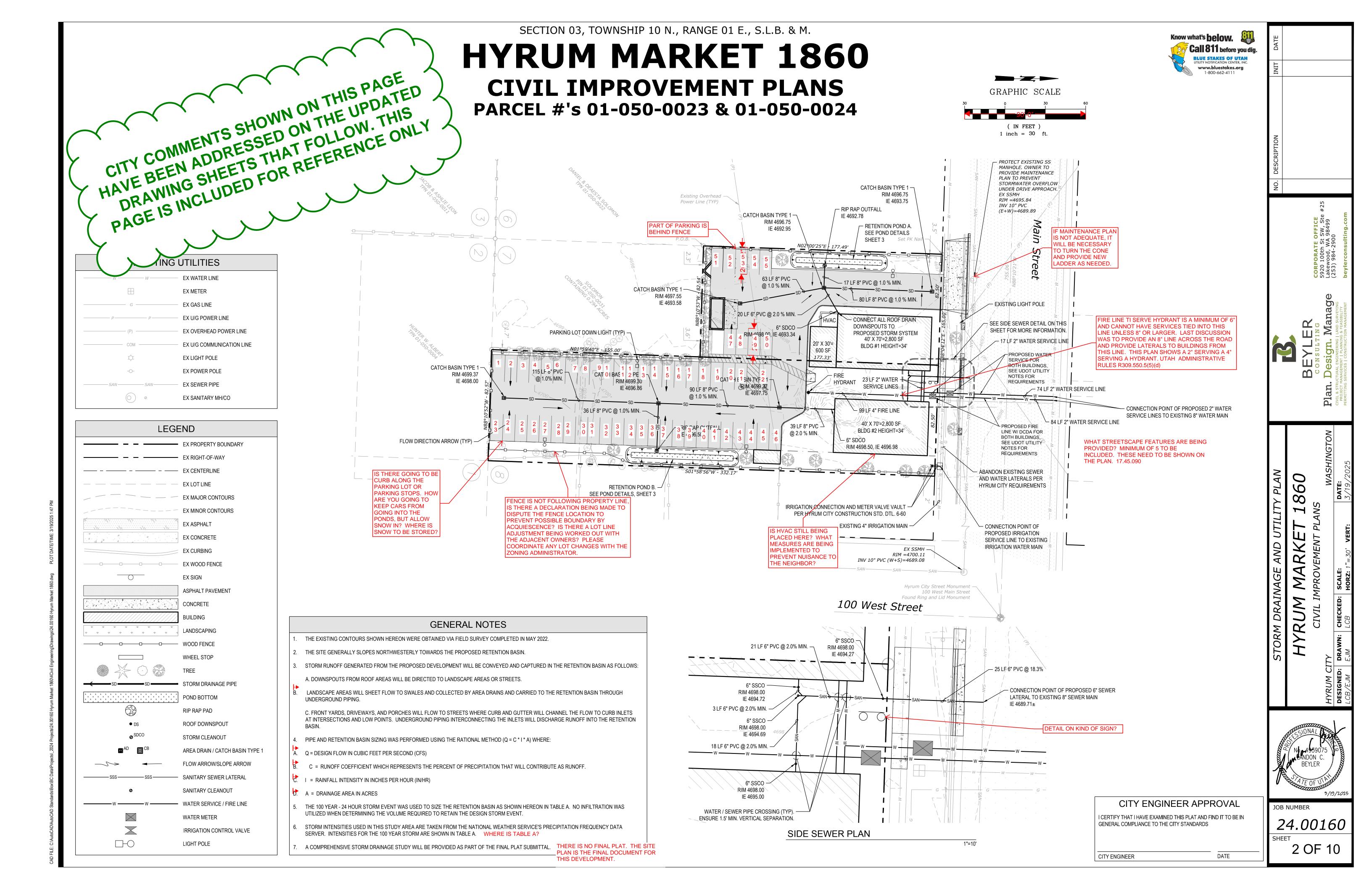
V V V V V V —— — BUILDING SETBACK SDCO

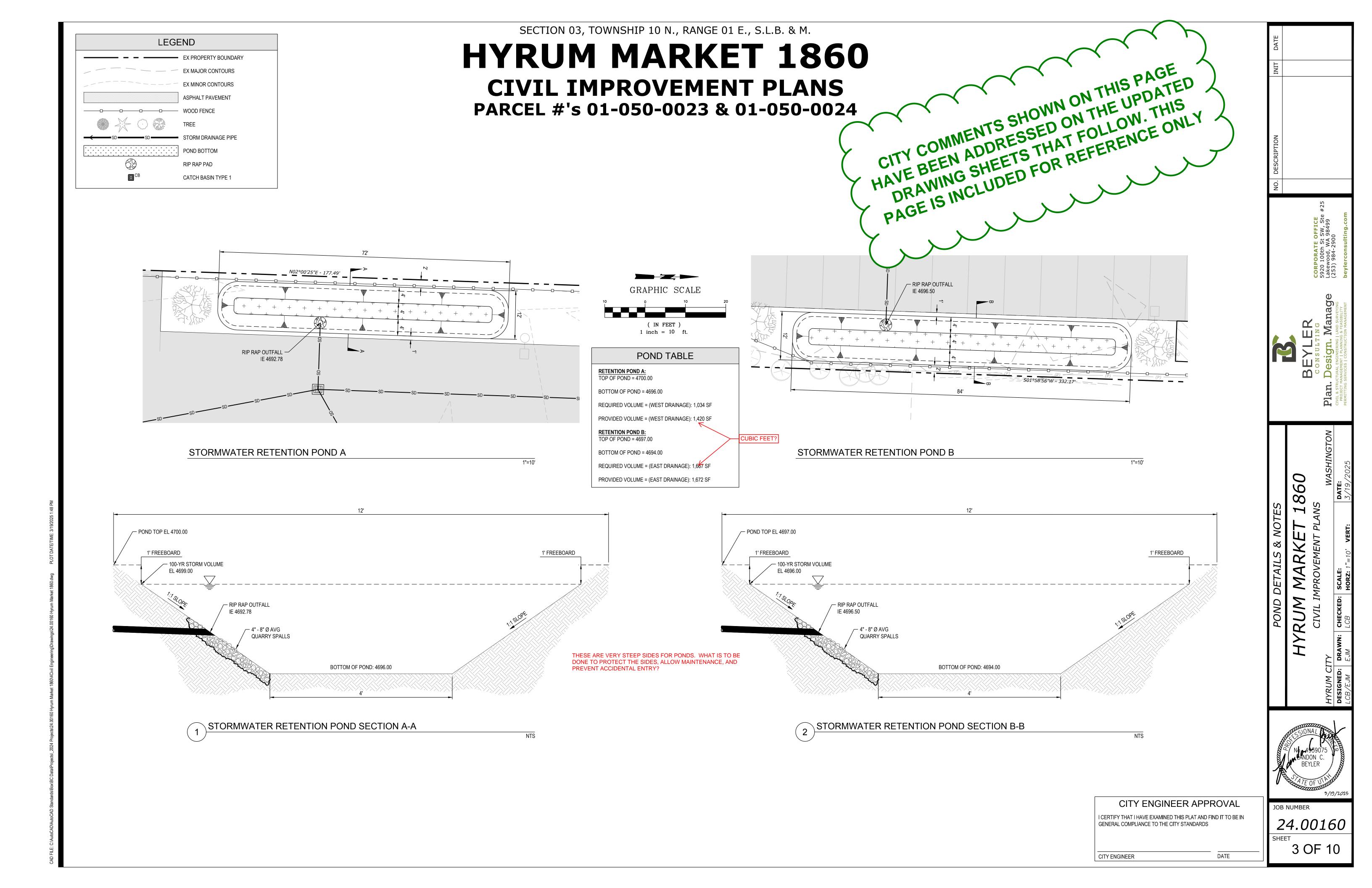






Y





EXISTING UTILITIES

EX WATER LINE

EX METER

EX GAS LINE

EX U/G POWER LINE

EX LIGHT POLE

EX POWER POLE

EX SEWER PIPE

EX SANITARY MH/CO

EX OVERHEAD POWER LINE

EX U/G COMMUNICATION LINE

CIVIL ABBREVIATIONS

STORM DRAIN MANHOLE

SANITARY SEWER CLEAN OUT

SANITARY SEWER MANHOLE

UNIFORM BUILDING CODE

SANITARY SEWER FORCE MAIN

WASHINGTON STATE DEPARTMENT OF

SANITARY SEWER

WATER METER

TRANSFORMER

TRANSPORTATION

BUILDING SETBACK LINE CATCH BASIN CLEAN OUT CONC CONCRETE DUCTILE IRON FIRE HYDRANT HIGH POINT LOW POINT MANHOLE **PGHS** POLLUTION GENERATING HARD SURFACE PVC POLYVINYLCHLORIDE REDUCED PRESSURE BACKFLOW PREVENTOR STORM DRAIN SDCO STORM DRAIN CLEAN OUT

SDMH

SSCO

SSFM

SSMH

UBC

WM

XFMR

HYRUM MARKET 1860

CIVIL IMPROVEMENT PLANS PARCEL #'s 01-050-0023 & 01-050-0024

PARKING LOT DOWN LIGHT. LIGHTING

GRADE TO LIGHT FIXTURE (TYP.)

PROPOSED MAINTENANCE SHED J

TO BE 15' MAX. HEIGHT FROM FINISHED

ALL SIGNS PER UDOT SN SERIES STANDARD DRAWINGS.



IMPERVIOUS / PERVIOUS SITE AREAS

1,060 SF (0.024 Ac) PROPOSED IMPERVIOUS ONSITE **BUILDING #1** 3,400 SF (0.078 Ac) BUILDING #2 MAINTENANCE SHED 200 SF (0.005 Ac) TRASH ENCLOSURE 264 SF (0.006 Ac) ASPHALT ACCESS/PARKING LOT (PGHS) GRAVEL (PGHS) CONCRETE (PGHS) 325 SF (0.007 Ac) CONCRETE WALKWAY / HARDSCAPE 3,232 SF (0.074 Ac)

23,167 SF (0.532 Ac) TOTAL PROPOSED (PGHS):

9- NOTES AND DETAILS SHEET 6

10- NOTES AND DETAILS SHEET 7

2,800 SF (0.064 Ac) 19,958 SF (0.458 Ac) 1,191 SF (0.027 Ac) 31,370 SF (0.720 Ac) TOTAL (PGHS) ONSITE: 21,474 SF (0.493 Ac) LANDSCAPING 10,693 SF (0.245 Ac) PROPOSED IMPERVIOUS OFFSITE ASPHALT ACCESS (PGHS) 37 SF (0.001 Ac) ASPHALT UTILITY TRENCHING (PGHS) 371 SF (0.008 Ac) CONCRETE DRIVEWAY APPROACH (PGHS) 1,056 SF (0.024 Ac) **CONCRETE CURBING (PGHS)** 229 SF (0.005 Ac) **CONCRETE SIDEWALK** 197 SF (0.004 Ac) **CONCRETE FOOT BRIDGES** 60 SF (0.001 Ac) CONCRETE WALKWAY / HARDSCAPE 31 SF (0.001 Ac) 1,981 SF (0.045 Ac) TOTAL (PGHS) OFFSITE: 1,693 SF (0.039 Ac) PROPOSED PERVIOUS OFFSITE 212 SF (0.005 Ac) TOTAL PROPOSED IMPERVIOUS: 33,351 SF (0.765 Ac)

SHEET INDEX

1- COVER SHEET / SITE PLAN 2- STORM DRAINAGE AND UTILITY PLAN 3- STORM INFILTRATION TRENCH PLAN, DETAILS/NOTES 4- NOTES AND DETAILS SHEET 1 5- NOTES AND DETAILS SHEET 2 6- NOTES AND DETAILS SHEET 3 7- NOTES AND DETAILS SHEET 4 8- NOTES AND DETAILS SHEET 5

34' MAX BUILDING HEIGHT

BUILDING INFORMATION 2 - 40'X70' 2-STORY WOOD FRAME STRUCTURES MAIN FLOOR RETAIL, UPPER FLOOR SHORT TERM RENTAL UNITS 1 - 10'X20' MAINTENANCE SHEDS

PROPERTY AREA: PIN 01-050-0023 = 0.34 Ac (14,640 SF) 0.97 Ac (42,063 SF)

C-2 OVERLAY RESIDENTIAL NO SETBACKS REQUIRED.

ADA PARKING:

CITY ENGINEER APPROVAL

I CERTIFY THAT I HAVE EXAMINED THIS PLAT AND FIND IT TO BE IN

CITY ENGINEER

GENERAL COMPLIANCE TO THE CITY STANDARDS

DATE

SLOPE 6' FENCE DOWN TO 4' HEIGHT -CONTINUED TOWARD DRIVE APPROACH & AROUND CORNER TO DRIVEWAY CONSTRUCT PIPED DRIVEWAY APPROACH (BRIDGE STYLE) PER PARCELS 0023 AND 0024 IN -APWA PLAN NO. 229 Existing Overhead -PROCESS OF MERGING PER NEW WITH CONCRETE TIE-IN Power Line (TYP) OWNERSHIP ENTITY. DIVIDING PROPERTY LINE TO BE ELIMINATED BEFORE ISSUE OF BUILDING PERMIT. FLUSH CURB / MOWSTRIP -N02°00'25"E - 177.49' PROPOSED TREES FOR RESIDENTIAL/COMMERCIAL LANDSCAPE SCREENING (TYP) 24' WIDE X 11" DEEP -PROPOSED SHRUB OR COLUMNAR TREES FOR — ANDREA NIELSEN TRASH DUMPSTER. 3788 ECKER HILL DR. COMMERCIAL/RESIDENTIAL LANDSCAPE SCREENING (TYP) FENCÉ SCREENING PER PARK CITY, UT 84908 CITY OF HYRUM - DELIVERY UNLOADING ZONE **ORDINANCE 13-02** PROPOSED WHEEL STOP (TYP) -6' CEDAR FENCING (TYP.) -ALONG PROPERTY LINE PROPOSED PARKING 40' X 70'=2,800 SF BLDG #1 HEIGHT=34' NUMBER OF PARKING -PROPOSED SIGN LOCATION -STALLS PER ROW (TYP) CROSSWALK / PATH OF TRAVEL JOHN & VIRGINIA FRANCIS 450 W 2000 N V PROPOSED ADA - ADA CURB RAMP MAPLETON, UT 84664 STALL/LOADING

UDOT UTILITY SPECIFICATIONS & NOTES

STORM INFILTRATION TRENCH (TYP) —

PROPOSED TREES RECOMMENDED EVERY —

30' FOR RESIDENTIAL/COMMERCIAL

LANDSCAPE SCREENING (TYP)

SEE SHEET 2 FOR STORM DRAINAGE PLAN)

PROPOSED GRAVELED AREA /

SNOW STORAGE AREA

- 1. CONTRACTOR TO USE TRENCHLESS CONSTRUCTION UNLESS SUFFICIENT REASON IS GIVEN FOR OPEN TRENCH
- 2. ALL UTILITY TRENCHES TO BE CUT AT RIGHT ANGLES TO TRAVEL LANES. TEMPORARY PATCHES REQUIRE AT LEAST 2-INCH ROTOMILING APPROACHING AND LEAVING THE PATCH IN ALL TRAVEL LANES IMPACTED BEFORE FINAL T-PATCH ASPHALT PLACEMENT PER APWA PLAN NO. 255.
- 3. ALL FINAL PARALLEL SAWCUT LINES OR ROTOMILLING MUST BE LOCATED EITHER AT DESIGNED LANE LINES OR DESIGNED CENTER OF LANE. SAWCUTS MUST BE CLEANED AND TACK-COAT APPLIED BEFORE ASPHALT PLACEMENT
- 4. ALL TRENCHES FOR LATERALS WITHIN A 100FT DISTANCE MUST HAVE 2 INCH MILL AND REPLACED AS A SINGLE PATCH. SINGLE LATERALS REQUIRE A 2 INCH MILL FOR 20FT EACH DIRECTION
- 5. ALL ASPHALT CONSTRUCTION WITHIN UDOT RIGHT-OF-WAY TO MATCH EXISTING. HOT MIX ASPHALT (HMA) SHALL BE PG-GRADE 64-34 ASPHALT BINDER, 1/2 INCH NOMINAL MAX, 7-75-115 GYRATION PER UDOT STANDARD SPECIFICATION 02741; OVER 6 INCHES UNTREATED BASE COURSE (UTBC) PER UDOT SPECIFICATION 02721; OVER 11 INCHES GRANULAR BORROW (GB) PER UDOT SPECIFICATION 02056 (WHICHEVER IS GREATER). PROVIDE DOCUMENTATION OF COMPACTION FROM A UDOT-QUALIFIED LABORATORY.
- 6. PAVEMENT SEALING CHIP SEAL TYPE II WITH EMULSION LMCRS-2 PER UDOT STANDARD SPECIFICATION 02785 (ESTIMATED APPLICATION RATE OF 0.45 GAL/SQ YD) IS REQUIRED ON ALL NEW PAVEMENT WITHIN UDOT RIGHT-OF-WAY.

UDOT RIGHT-OF-WAY (MAIN STREET) NOTES

WAS THIS SUPPOSED

TO BE MOVED FROM

THE LANDSCAPED

40' X 70'=2,800 SF

SEE SHEET 2 FOR UTILITY PLAN -

IS THIS AN

EASEMENT? WHAT

UTILITY OR USE IS

A LANDSCAPING

BUFFER ONLY?

THIS FOR? OR IS THIS

6' ENCLOSURE AROUND NEW —

- 6" RAISED CURB (TYP)

HVAC EQUIPMENT (TYP)

BLDG #2 HEIGHT=34'

ALL CONSTRUCTION WITHIN THE UDOT RIGHT-OF-WAY SHALL CONFORM TO THE MOST CURRENT UDOT STANDARD (INCLUDING SUPPLEMENTAL) DRAWINGS AND SPECIFICATION. APPLICABLE UDOT STANDARD AND SUPLEMENTAL DRAWINGS ARE INCLUDED IN THIS PLAN SET

BUFFER?

- THE CONTRACTOR IS TO OBTAIN AN ECROACHMENT PERMIT FROM THE APPLICABLE UDOT REGION PERMIT OFFICE PRIOR TO COMMENCING WORK WITHIN THE UDOT RIGHT-OF-WAY. WORKING HOUR LIMITATIONS WILL BE LISTED IN THE LIMITATIONS SECTION OF THE
- UDOT RESERVES THE RIGHT, AS ITS OPTION, TO INSTALL A RAISED MEDIAN ISLAND OR RESTRICT THE ACCESS TO A RIGHT-IN OR RIGHT-OUT AT ANY TIME.
- OWNER, DEVELOPER, AND CONTRACTOR ARE RESPONSIBLE FOR ANY DAMAGES DIRECTLY OR INDIRECTLY WITHIN THE UDOT RIGHT-OF-WAY AS A RESULT OF DEVELOPMENT ACTIVITIES.
- OWNER, DEVELOPER, AND/OR CONTRACTOR IS REQUIRED TO HIRE AN INDEPENDENT COMPANY FOR ALL TESTING WITHIN THE UDOT

ALL SIGNS INSTALLED ON THE UDOT RIGHT-OF WAY MUST BE HIGH INTENSITY GRADE (TYPE XI SHEETING) WITH A B3 SLIP BASE. INSTALL

COMPLY WITH THE REQUIREMENTS OF UTAH CODE 17-23-14 (DISTURBED CORNERS - COUNTY SURVEYOR TO BE NOTIFIED -COORDINATION WITH CERTAIN STATE AGENCIES).

PROJECT INFO

BEYLER CONSULTING MARKET 1860 LLC 5920 100TH ST SW, STE 25

LAKEWOOD, WA 98499 CONTACT: LANDON BEYLER, P.E. TEL: 253-984-2900

UTILITIES

(IN FEET

1 inch = 30 ft.

- RELOCATE EXISTING

CROSSWALK SIGN.

COORDINATE WITH UDOT.

REMOVE EXISTING

FEATURE

NEW PLANTER &

Block 15, Plat "A"

- EXISTING SCHOOL ZONE

SIGN & WARNING LIGHTS

BENCHES / STREETSCAPE

– PROPOSED PARK

APPROACH, REPLACE WITH

MATCHING CURB, GUTTER,

AND SIDEWALK PER UDOT

STREETSCAPE FEATURE

REPLACE W/ NEW 5' WIDE

FOOT BRIDGE PER UDOT

- EXISTING TREE / STREETSCAPE

FEATURE CONSULT WITH TREE

SURGEON REGARDING HEALTH

OF EXISTING TREE. REPLACE IF

STANDARD DRAWINGS.

- NEW PLANTER &

REMOVE EXISTING

FOOTBRIDGE AND

STANDARDS.

NECESSARY.

PARALLEL PARKING SIGN.

COORDINATE WITH UDOT

NEW TREE & STREETSCAPE

STREETSCAPE FEATURE

HYRUM CITY CULINARY WATER **AUTHORITY**

GRAVITY HYRUM CITY SEWER AUTHORITY

HYRUM CITY POWER

SITE DATA PARCEL NUMBER(S): PIN 01-050-0023 ANDREA NIELSEN 3788 ECKER HILL DR. PARK CITY, UT 84098 PIN 01-050-0024 JOHN & VIRGINIA FRANCIS 450 W 2000 N MAPLETON, UT 84664

SITE ADDRESS: 127 WEST MAIN STREET, HYRUM, UT

PIN 01-050-0024 = 0.63 Ac (27,423 SF)

15 FT SIDE SETBACKS AGAINST

STANDARD PARKING: 46 STALLS 3 STALLS 49 STALLS

JOB NUMBER 24.00160

Y

1 OF 10

LEGEND EX PROPERTY BOUNDARY — EX RIGHT-OF-WAY EX MAJOR CONTOURS EX MINOR CONTOURS EX ASPHALT EX CONCRETE **EX CURBING** EX WOOD FENCE EX SIGN ASPHALT PAVEMENT CONCRETE BUILDING LANDSCAPING —— — BUILDING SETBACK — WOOD FENCE WHEEL STOP **ROOF DOWNSPOUT** STORM CLEANOUT AREA DRAIN / CATCH BASIN TYPE 1 FLOW ARROW/SLOPE ARROW SANITARY SEWER LATERAL

SANITARY CLEANOUT WATER SERVICE / FIRE LINE

WATER METER IRRIGATION CONTROL VALVE

LIGHT POLE

1 FRONT SIGN DETAIL

EX WATER LINE

EX METER

EX GAS LINE

EX U/G POWER LINE

EX OVERHEAD POWER LINE

EX U/G COMMUNICATION LINE

EX LIGHT POLE

EX POWER POLE

EX SEWER PIPE

EX SANITARY MH/CO

EXISTING UTILITIES

LEGEND

EX MAJOR CONTOURS EX MINOR CONTOURS EX ASPHALT EX CONCRETE EX CURBING EX WOOD FENCE EX SIGN ASPHALT PAVEMENT CONCRETE BUILDING LANDSCAPING WOOD FENCE WHEEL STOP STORM DRAINAGE PIPE INFILTRATION TRENCH **ROOF DOWNSPOUT** STORM CLEANOUT AREA DRAIN / CATCH BASIN TYPE 1 FLOW ARROW/SLOPE ARROW SANITARY SEWER LATERAL SANITARY CLEANOUT WATER SERVICE / FIRE LINE WATER METER IRRIGATION CONTROL VALVE

LIGHT POLE

SECTION 03, TOWNSHIP 10 N., RANGE 01 E., S.L.B. & M.

HYRUM MARKET 1860

CIVIL IMPROVEMENT PLANS PARCEL #'s 01-050-0023 & 01-050-0024

CATCH BASIN TYPE 1

CATCH BASIN TYPE 1

20 LF 8" PVC @ 1.0% MIN.

SEDIMENT CONTROL STRUCTURE -

ETERMINED? WHAT WAS THE TOTAL VOLUME TO BE RETAINED?

WHAT ARE THE INFILTRATION RATES?
THIS IS THE FINAL SITE PLAN SUBMITTAL

W/ SOLID LOCKING LID

RIM 4699.70

RIM 4699.30

RIM 4697.55

IE 4693.58

INFILTRATION TRENCH

RIM 4696.75

(SEE SHEET 3 FOR DETAILS)

W/ SOLID LOCKING LID

SEDIMENT CONTROL STRUCTURE -

21 LF 8" PVC @ 1.0 % MIN.

RIM 4696.75

IE 4692.54

55 LF 8" PVC @ 1.0 % MIN.

CATCH BASIN TYPE 1

33 LF 6" PVC @ 2.0 % MIN.

CATCH BASIN TYPE 1

RIM 4699.37

IE 4696.80

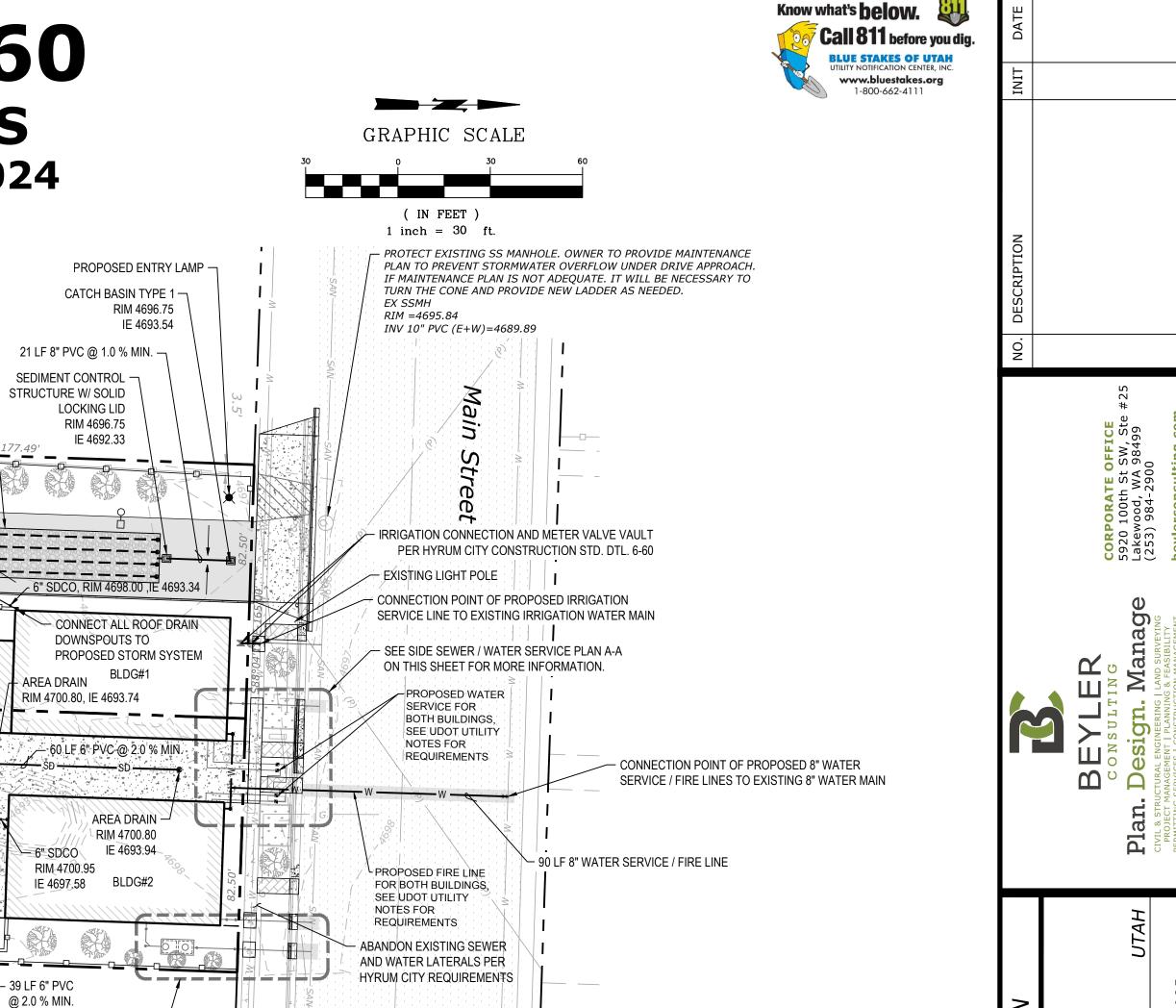
@ 1.0 % MIN.

RIM 4697 55, IE 4693.08

CATCH BASIN TYPE 1

31 LF 8" PVC -

IE 4692.30



GENERAL NOTES

THE EXISTING CONTOURS SHOWN HEREON WERE OBTAINED VIA FIELD SURVEY COMPLETED IN MAY 2022.

CATCH BASIN TYPE

FLOW DIRECTION ARROW (TYP)

RIM 4699.36

IE 4696.86

PARKING LOT DOWN LIGHT (TYP) —

93' L x 16' W x 3' H —

IE 4695.68

INFILTRATION TRENCH

(SEE SHEET 3 FOR DETAILS)

- THE SITE GENERALLY SLOPES NORTHWESTERLY TOWARDS THE PROPOSED RETENTION BASIN.
- STORM RUNOFF GENERATED FROM THE PROPOSED DEVELOPMENT WILL BE CONVEYED AND CAPTURED IN THE RETENTION BASIN AS FOLLOWS:
- A. DOWNSPOUTS FROM ROOF AREAS WILL BE DIRECTED TO LANDSCAPE AREAS OR STREETS.
- B. LANDSCAPE AREAS WILL SHEET FLOW TO SWALES AND COLLECTED BY AREA DRAINS AND CARRIED TO THE RETENTION BASIN THROUGH UNDERGROUND PIPING.
- C. FRONT YARDS, DRIVEWAYS, AND PORCHES WILL FLOW TO STREETS WHERE CURB AND GUTTER WILL CHANNEL THE FLOW TO CURB INLETS AT INTERSECTIONS AND LOW POINTS.

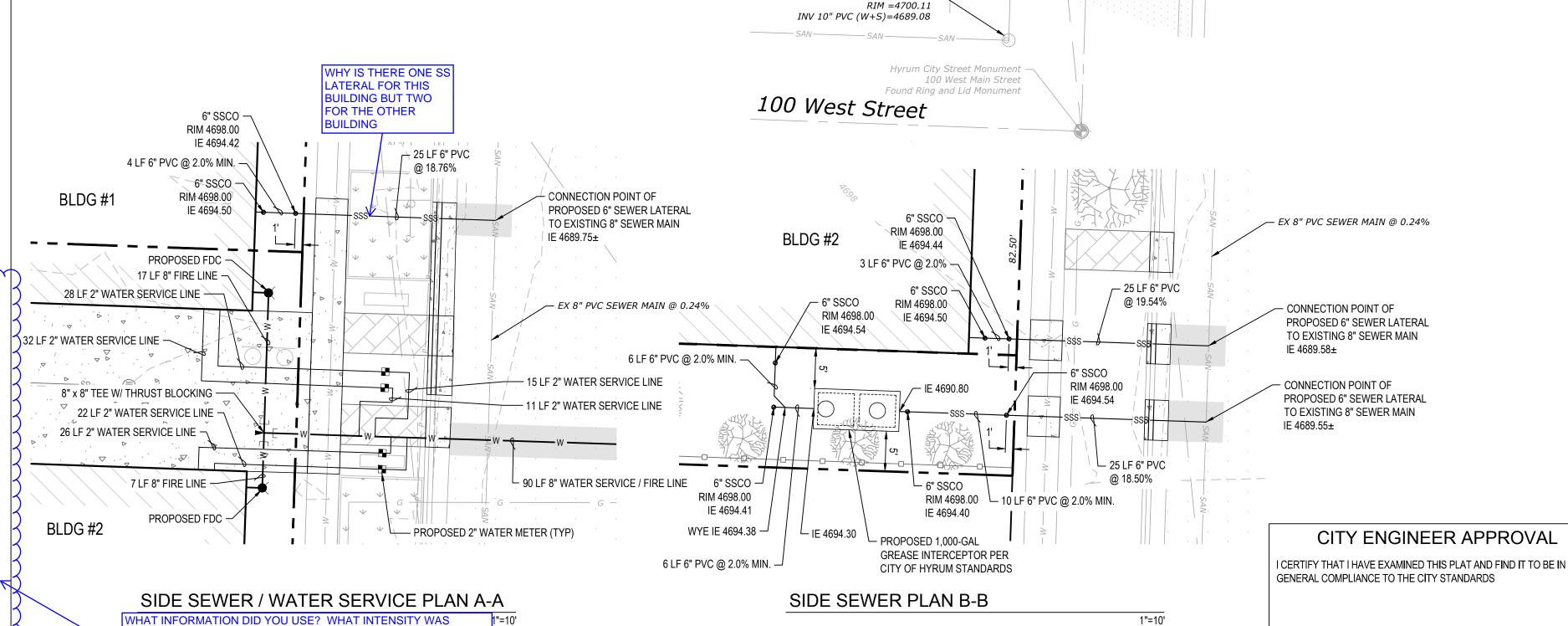
 UNDERGROUND PIPING INTERCONNECTING THE INLETS WILL

 PISCHARGE RUNOFF, INTO THE RETENTION BASIN.

PIPE AND RETENTION BASIN SIZING WAS PERFORMED USING THE

- RATIONAL METHOD (Q = C * I * A) WHERE:

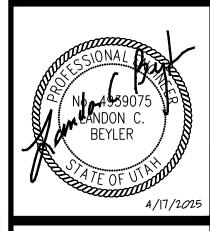
 A. Q = DESIGN FLOW IN CUBIC FEET PER SECOND (CFS)
- A. Q DESIGN FLOW IN CODIC FEET FER SECOND (CFS)
- B. C = RUNOFF COEFFICIENT WHICH REPRESENTS THE PERCENT OF PRECIPITATION THAT WILL CONTRIBUTE AS RUNOFF.
- C. I = RAINFALL INTENSITY IN INCHES PER HOUR (IN/HR)
- D. A = DRAINAGE AREA IN ACRES
- 5. THE 100 YEAR 24 HOUR STORM EVENT WAS USED TO SIZE THE RETENTION BASIN AS SHOWN HEREON IN TABLE A. NO INFILTRATION WAS UTILIZED WHEN DETERMINING THE VOLUME REQUIRED TO RETAIN THE DESIGN STORM EVENT.
- 6. STORM INTENSITIES USED IN THIS STUDY AREA ARE TAKEN FROM THE NATIONAL WEATHER SERVICE'S PRECIPITATION FREQUENCY DATA SERVER. INTENSITIES FOR THE 100 YEAR STORM ARE SHOWN IN THE DRAINAGE REPORT ASSOCIATED WITH THIS PROJECT.
- 7. A COMPREHENSIVE STORM DRAINAGE STUDY WILL BE PROVIDED AS PART OF THE FINAL SITE PLAN SUBMITTAL.



SEE SIDE SEWER SERVICE PLAN -

B-B ON THIS SHEET FOR MORE

EXISTING 4" IRRIGATION MAIN —



JOB NUMBER **24.00160**

2 OF 10

DATE

CITY ENGINEER

vianat 1000 4-Civil Englineaning Drawnigs 24-.00 100 Tyrun market 1000-uwg

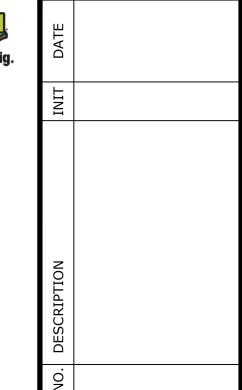
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LEGEND CATCH BASIN TYPE 1

HYRUM MARKET 1860

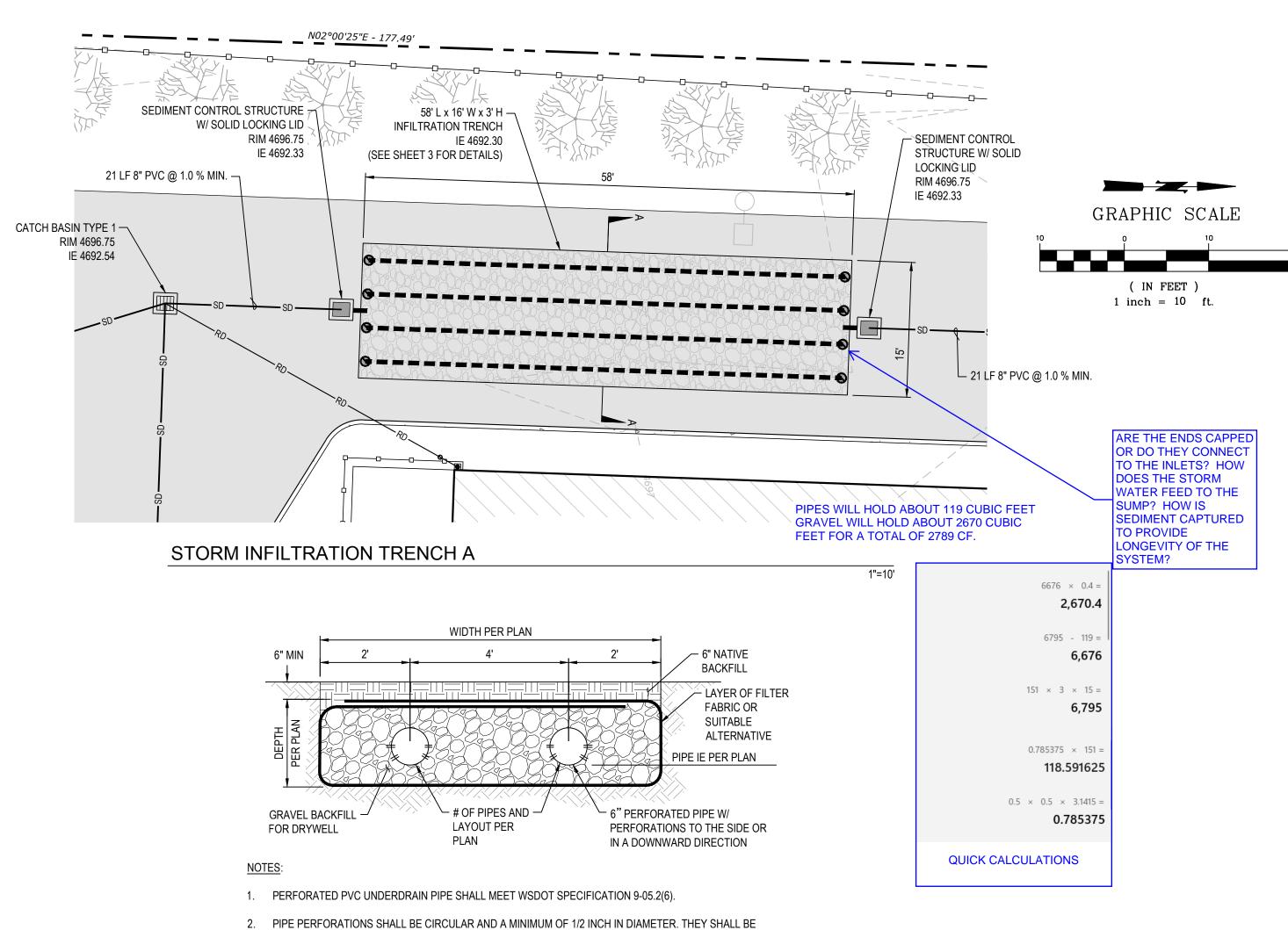
CIVIL IMPROVEMENT PLANS PARCEL #'s 01-050-0023 & 01-050-0024





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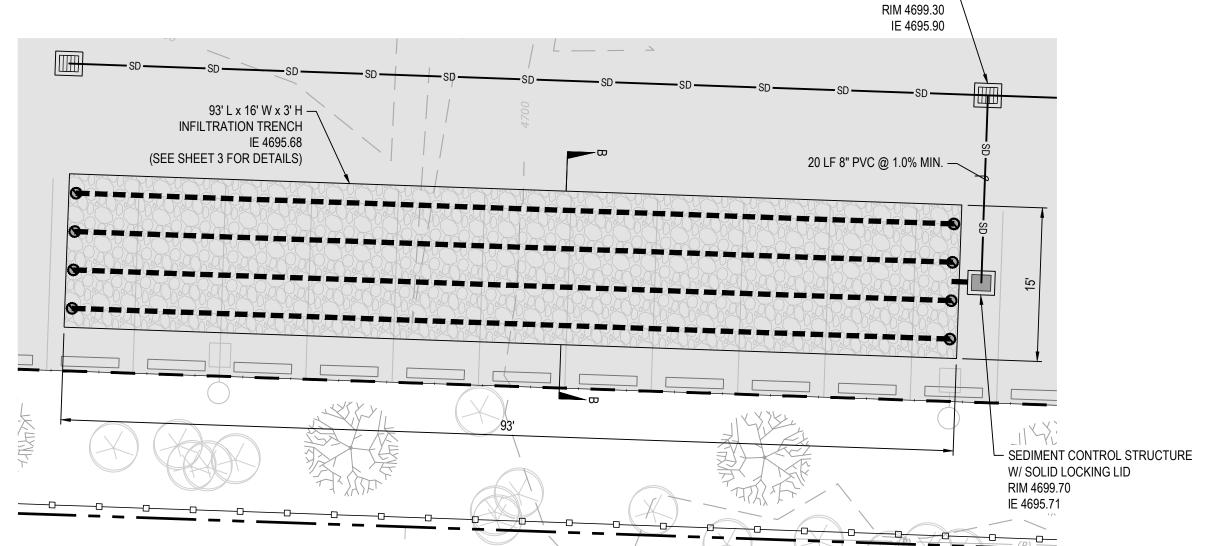
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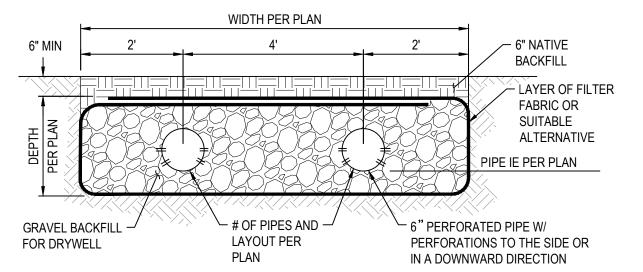
CLEANLY CUT AND BE SMOOTH AND UNIFORM. THERE SHALL BE A MINIMUM OF 7 SETS OF PERFORATIONS

WITH 2 HOLES PER SET OF PERFORATIONS FOR EACH 3-1/2 FEET OF PIPE LENGTH.

STORM INFILTRATION TRENCH SECTION A-A



STORMWATER RETENTION POND B



- 1. PERFORATED PVC UNDERDRAIN PIPE SHALL MEET WSDOT SPECIFICATION 9-05.2(6).
- 2. PIPE PERFORATIONS SHALL BE CIRCULAR AND A MINIMUM OF 1/2 INCH IN DIAMETER. THEY SHALL BE CLEANLY CUT AND BE SMOOTH AND UNIFORM. THERE SHALL BE A MINIMUM OF 7 SETS OF PERFORATIONS WITH 2 HOLES PER SET OF PERFORATIONS FOR EACH 3-1/2 FEET OF PIPE LENGTH.

STORM INFILTRATION TRENCH SECTION B-B

CITY ENGINEER

CATCH BASIN TYPE 1 -

CITY ENGINEER APPROVAL

I CERTIFY THAT I HAVE EXAMINED THIS PLAT AND FIND IT TO BE IN GENERAL COMPLIANCE TO THE CITY STANDARDS

DATE

CIVIL IMPROVEMENT PLANS

PARCEL #'s 01-050-0023 & 01-050-0024

ALL CONSTRUCTION AND MATERIALS SHELL BE IN

ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

CITY OF HYRUM STANDARDS, STATE OF UTAH AND

CONTROLLING AGENCY. CONTRACTOR AND

THE STANDARDS.

OCCUR.

CONSTRUCTION.

CONDITIONS.

December 2005

ANY OTHER APPLICABLE STANDARDS ISSUED BY THE

DEVELOPER ARE TO FAMILIARIZE THEMSELVES WITH

CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS

BETWEEN CONSTRUCTION DOCUMENTS AND FIELD

CONDITIONS SHALL IMMEDIATELY BE BROUGHT TO

THE ATTENTION OF THE OWNER. ANY WORK DONE

CONTRACTOR SHALL REPAIR AND/OR REPLACE ANY

CONTRACTOR SHALL PROVIDE SMOOTH TRANSITION

WITHOUT VERIFICATION IS DONE SO AT THE

CONTRACTORS RISK AND EXPENSE IF ERRORS

AREAS AND/OR MATERIALS DAMAGED DURING

CONTRACTOR SHALL MAINTAIN ALL ADJACENT

PROPERTY (PUBLIC AND PRIVATE) FROM ALL

FROM ALL NEW CONSTRUCTION TO EXISTING

BEFORE CONSTRUCTION. ANY DISCREPANCIES

Know what's **below.** Call 811 before you dig. **BLUE STAKES OF UTAH** www.bluestakes.org

PIPED DRIVEWAY APPROACH

1. UNTREATED BASE COURSE: Provide material specified in APWA Section 32 11 23. A. Do not use gravel as a substitute for untreated base course without ENGINEER's

B. Place material per APWA Section 32 05 10.

C. Compact per APWA Section 31 23 26 to a modified proctor density of 95 percent or greater. Maximum lift thickness before compaction is 8 inches when using riding

compaction equipment or 6 inches when using hand held compaction equipment. 2. CONCRETE: Class 4000 per APWA Section 03 30 04.

A. If necessary, provide concrete that achieves design strength in less than 7 days. Use caution; however, as concrete crazing (spider cracks) may develop if air temperature exceeds 90 degrees F.

B. Place concrete per APWA Section 03 30 10.

C. Provide 1/2 inch radius on concrete edges exposed to public view.

D. Cure concrete per APWA Section 03 39 00 with type ID Class A or B (clear with fugitive dye) membrane forming compound unless specified otherwise.

3. EXPANSION JOINT: Make expansion joints vertical, full depth 1/2 inch wide with type F1

filler material per APWA Section 32 13 73. Set top of filler flush with surface of concrete.

4. CONTRACTION JOINT: Make contraction joints vertical. A. 1/8 inch wide and 2 inches deep or 1/4 slab thickness if slab is greater than 8 inches thick.

B. Maximum length to width ratio for non-square panels is 1.5 to 1. C. Maximum panel length (in feet) is .2.5 times the slab thickness (in inches) to a maximum of

5. REINFORCEMENT: ASTM A 615, grade 60, galvanized or epoxy coated deformed steel. See APWA Section 03 20 00 requirements . Not required if driveway ramp is constructed without a

6. FIELD CHANGES TO SLOPE REQUIREMENTS: The following design parameters are to be used as a guide. Specific uses or site conditions may require profile design submittal for review

A. As a rule, driveway grades may have a 6 percent change in slope over a 11 feet wheel base run for both crest or sag vertical curves.

B. Where heavy truck use and fire truck access applies, or to improve design speed, design grades should be cut in half.

C. Grades subject to roadway crown and gutter span to be reviewed by ENGINEER for high centering and vehicle approach speed.

7. FINISH: Broomed.

8 PROTECTION AND REPAIR:

A. Fill flow-line with water. Repair construction that doesn't drain

B. Protect concrete from deicing chemicals during cure period.

PIPED DRIVEWAY APPROACH

1. ASPHALT CONCRETE: As specified in APWA Section 32 12 05. Compaction to be

within range of 92 to 96 percent relative to ASTM D 2041 (Rice

2. CONCRETE: Class 4000 per APWA Section 03 30 04. A. If necessary, provide concrete that achieves design strength in less than 7 days.

Use caution; however, as concrete crazing (spider cracks) may develop if air

temperature exceeds 90 degrees F.

B (clear with

B. Place concrete per APWA Section 03 30 10. C. Provide 1/2 inch radius on concrete edges exposed to public view. D. Cure concrete per APWA Section 03 39 00 with type ID Class A or

fugitive dve) membrane forming compound unless specified otherwise.

3. REINFORCEMENT: ASTM A 615, grade 60, galvanized or epoxy

steel. See APWA Section 03 20 00 requirements. CONSTRUCTION DEBRIS.

GENERAL NOTES (APPLICABLE TO ALL CIVIL SHEETS)

6. CONTRACTOR SHALL PROVIDE ALL NECESSARY AUTOMOBILE AND PEDESTRIAN TRAFFIC CONTROL DEVICES REQUIRED BY LOCAL, STATE AND FEDERAL CODES AND ORDINANCES.

CONTRACTOR SHALL REPLACE SURVEY MONUMENTS DAMAGED DURING CONSTRUCTION. SURVEY MONUMENTS TO BE REPLACED BY A REGISTERED, LICENSED LAND SURVEYOR.

CONTRACTOR TO LOCATE ALL EXISTING UTILITIES, INCLUDING FIBER OPTIC. ANY DAMAGES TO EXISTING UTILITIES WILL BE REPAIRED AT CONTRACTORS EXPENSE.

DIMENSIONS SHOWN ARE TO THE CENTER OF THE PIPELINE UNLESS OTHERWISE NOTED.

10. DISTANCES SHOWN ALONG PIPELINES ARE HORIZONTAL DISTANCE AND NOT ACTUAL PIPE LENGTHS. MORE PIPE MAY BE REQUIRED TO COMPLETE CONSTRUCTION THAN IS DIMENSIONED IN

11. THRUST BLOCKS SHALL BE PLACED ON WATERLINES AT ALL DIRECTIONAL CHANGES, FITTINGS, BENDS, ELBOWS. FIRE HYDRANTS AND GATE VALVES AS SHOWN IN THE PROJECT PLANS.

ON THE SITE AT ALL TIMES. ANY WORK COMPLETED WITHOUT A SET PRESENT IS DONE SO AT THE CONTRACTORS RISK AND EXPENSE IF ERRORS

13. CONTRACTOR IS RESPONSIBLE FOR PROVIDING WATER NECESSARY FOR DUST ABATEMENT, COMPACTION, ETC. THIS MAY BE COORDINATED WITH HYRUM WATER DEPARTMENT.

14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SECURING SOURCES FOR GRANULAR MATERIALS, WATER, WASTE SITES, AND ANY OTHER MATERIALS SOURCES AS REQUIRED FOR PROJECT COMPLETION.

15. ANY WORK DONE WITHIN A PUBLIC RIGHT-OF-WAY SHALL BE COORDINATED WITH THE APPROPRIATE TRANSPORTATION AGENCY AND SHALL MEET THAT AGENCY AND THE REQUIREMENTS OF ANY RIGHT-OF-WAY OR SPECIAL USE PERMITS.

16. THE CONTRACTOR SHALL COORDINATE ALL LIVE TAPS AND ANY OTHER WORK OR MANIPULATION OF THE EXISTING WATER SYSTEM WITH THE CITY.

12. CONTRACTOR IS REQUIRED TO HAVE A SET OF PLANS 17. ON SLOPING AREAS THE CONTRACTOR SHALL TAKE PRECAUTIONS TO MITIGATE ANY POSSIBLE EROSION PROBLEMS IN THE TRENCHES DUE TO STORM WATER THAT MIGHT OCCUR DURING OR AFTER CONSTRUCTION AS DIRECTED OR APPROVED BY

> 18. THE CONTRACTOR SHALL INSTALL AND MAINTAIN ALL EROSION CONTROL MEASURES AS DETAILED IN THE PROJECT PLANS UNTIL FINAL ACCEPTANCE OF THIS

19. THE CONTRACTOR IS REQUIRED TO TAKE ALL PRECAUTIONS NECESSARY TO INSURE THAT NO DEBRIS ARE RELEASED FROM THE SITE. ANY THE CONTRACTOR'S EXPENSE.

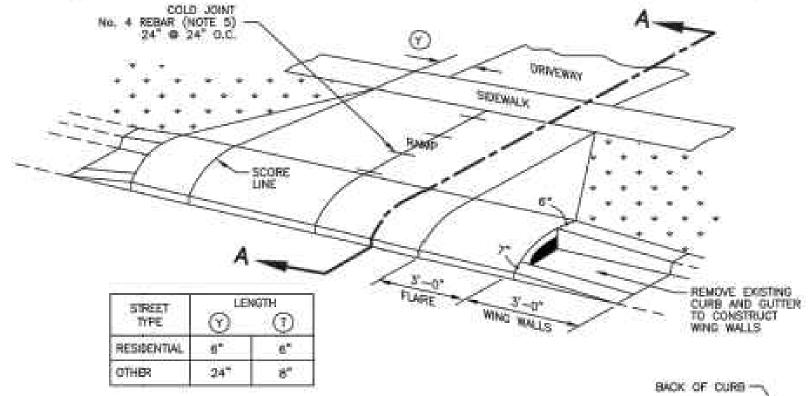
OR CROSSES AND THEIR ASSOCIATED REDUCERS AS SHOWN ON THE PROJECT PLANS.

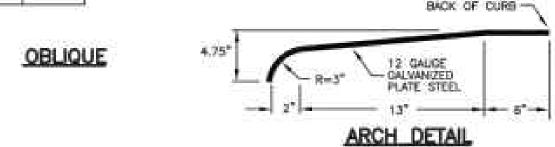
FITTINGS. HARDWARE, LABOR, ETC TO CONSTRUCT VERTICAL AND HORIZONTAL BENDS IN PIPE AS NEEDED TO MEET THE REQUIRED GRADES,

24. ALL WATER SYSTEM COMPONENTS SHALL BE INSTALLED, PRESSURE TESTED, AND CHLORINATED PRIOR TO COMPLETING ANY ROADWAY

25. ONE MYLAR AND ONE PAPER SET OF AS-BUILTS SHALL BE SUBMITTED TO THE CITY UPON COMPLETION OF PUBLIC IMPROVEMENTS. A DIGITAL COPY OF THE DRAWINGS WILL ALSO BE REQUIRED FOR GIS LINEWORK. AS BUILT PLANS WILL BE REQUIRED TO BE SUBMITTED TO THE CITY BEFORE WARRANTY BONDS SHALL BE RELEASED.

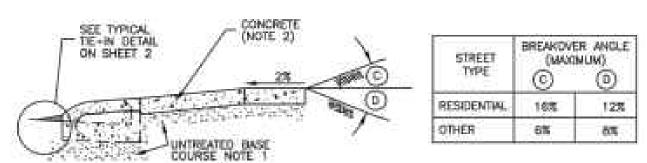
DO NOT CONSTRUCT THIS APPROACH WITHOUT ENGINEER'S WRITTEN APPROVAL





SEE ARCH DETAIL BREAKOVER ANGLE (MAXIMUM) 3.5" MIN ~ 12% RESIDENTIAL 16% OTHER 1"-0" MIN. (RESIDENTIAL) 25'-0" MIN. (OTHER) INSTALL ANCHOR NAIL TO PREVENT MOVEMENT OF ARCH

SECTION A-A - APPROACH REQUIRING SERVICE TRUCK ACCESS



SECTION A-A - TYPICAL DRIVEWAY APPROACH

Piped driveway approach

Drawing 1 of 2

January 2005

HOURS PRIOR TO INSPECTION.

STORM WATER/SEDIMENT AND/OR CONSTRUCTION RELEASES SHALL BE CLEANED AND MITIGATED AT

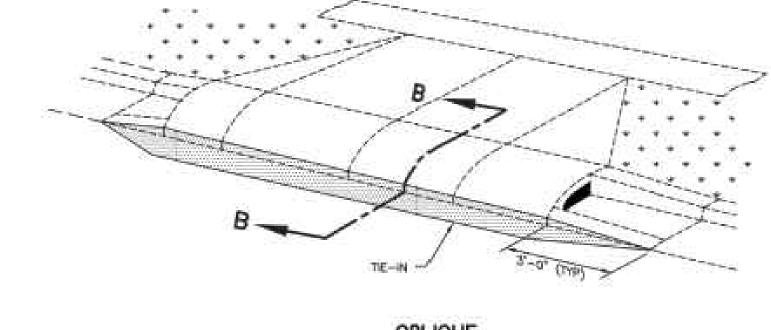
20. CONTRACTOR SHALL COORDINATE ALL CONSTRUCTION ACCESS AND RELATED TRAFFIC CONTROL WITH THE COUNTY, CITY AND STATE ROADWAY DEPARTMENTS. THE ENGINEER SHALL REVIEW ALL TRAFFIC CONTROL PLANS.

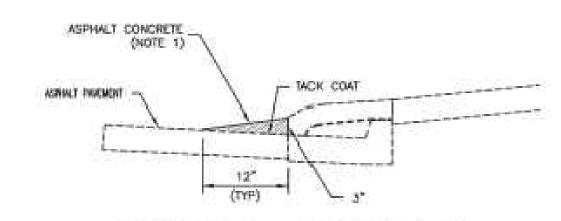
21. ALL GATE VALVES SHALL BE LOCATED NEAR TO TEES

22. CONTRACTOR SHALL PROVIDE ALL NECESSARY ALIGNMENTS AND COVER REQUIREMENTS.

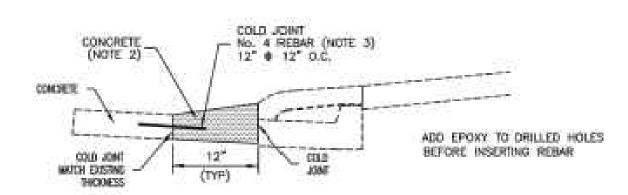
23. THE CONTRACTOR SHALL COORDINATE WITH THE CITY OF HYRUM FOR ALL UTILITY INSPECTIONS PRIOR TO BACKFILLING. NOTICE MUST BE GIVEN TO CITY 48

CONSTRUCTION.





SECTION B-B - ASPHALT TIE-IN



SECTION B-B - CONCRETE TIE-IN

Piped driveway approach

Plon No. 229 Drawing 2 of 2

CITY ENGINEER APPROVAL

I CERTIFY THAT I HAVE EXAMINED THIS PLAT AND FIND IT TO BE IN GENERAL COMPLIANCE TO THE CITY STANDARDS

DATE CITY ENGINEER

9

X

JOB NUMBER 24.00160 4 OF 10

UNTREATED BASE COURSE

TYPE B2 CURB & GUTTER

AREA = 2.059 SQ FT

NOTES:

PRECAST CURBS:

INCLUDE ADEQUATE REINFORCING STEEL TO

MEASURE CURB HEIGHT VERTICALLY FROM THE FLOW LINE OF THE GUTTER TO TOP BACK OF CURB.

4. REFER TO STD DWG GW 2B FOR CURB AND GUTTER AT ADA ACCESSES.

USE $\frac{3}{4}$ INCH DEFORMED DOWELS ON 5 FT MAXIMUM CENTERS.

MINIMUM OF 10 FT IN LENGTH.

DOWELS AT A MINIMUM OF 3 PER 10 FT LENGTH.

WITHSTAND HANDLING STRESSES.

CIVIL IMPROVEMENT PLANS

PARCEL #'s 01-050-0023 & 01-050-0024

ASPHALT CONCRETE T-PATCH

feature is within 2 feet of the second saw-cut.

B. Place material per APWA Section 32 05 10.

permission.

steel 24 inches on center.

within the patch.

and inlay.

FULL DEPTH \

JOINT REPAIR -

SECOND CUT_

FULL DEPTH

(NOTE B)

ACCRECATE BASE

ALL SIDES

COMPACT BACKFILL TO THIS

LEVEL BEFORE MAKING THE

SECOND PAVEMENT CUT AND

SECOND SURFACE REMOVAL

COMPACT THIS AREA

JOINT REPAIR

(NOTE 8)

AGGREGATE BASI

COMPACT THIS AREA

COMPACT BACKFILL TO THIS

LEVEL BEFORE MAKING THE -SECOND PAVEMENT CUT AND

SECOND SURFACE REMOVAL

Repair option: Crack seal.

SAW CUT

substituted for concrete substrate.

1. ADDITIONAL PAVEMENT REMOVAL: Remove additional pavement to a painted lane stripe,

a lip of gutter, a curb, an existing pavement patch, or an edge of the pavement if such street

2. UNTREATED BASE COURSE: Provide material specified in APWA Section 32 11 23.

A. Do not use gravel as a substitute for untreated base course without ENGINEER's

C. Compact per APWA Section 31 23 26 to a modified proctor density of 95 percent or

3. FLOWABLE FILL: Provide 28 day 60 psi controlled low strength material as specified in

APWA Section 31 05 15. Use fill material which flows easily and vibration is not required.

Cure to initial set before placing aggregate base or asphalt pavement. Use flowable fill in

4. TACK COAT: APWA Section 32 12 14. Full tack coat coverage on all vertical surfaces.

6. REINFORCEMENT: ASTM A 615, Grade 60, No. 5 galvanized or epoxy coated deformed

B. Not required if (1) existing concrete is less than 6 inches thick, (2) existing concrete is

7. CONCRETE SUBSTRATE: Class 4000 per APWA Section 03 30 04. Place concrete per

8. JOINT REPAIR: If a crack occurs at the "T" patch connection to existing pavement or at any

9. PATCH REPAIR: Repair the asphalt pavement patch if any of the following conditions occur

A. Pavement surface distortion exceeds 1/4 inch deviation in 10 feet. Repair option: Plane

B. Cracks at least 1-foot long and 1/4 inch wide occur more often than 1 in 10 square feet.

C. Asphalt raveling is greater than 1 square foot per 100 square feet. Repair option: Mill

(4" MINIMUM)

MATCH EXISTING THICKNESS + 1"

(NOTE 4) -

EXCAVATION BACKFILL

PLAN No. 381)

MATCH EXISTING THICKNESS USING

ASPHALT PAVEMENT OR CONCRETE SUBSTRATE

(NOTE 1)

UNTREATED BASE COURSE

UTILITY TRENCHING NOTE: 2" MILL AND SINGLE PATCH REQUIRED

SEE SPECIFICATIONS AND NOTES SHEET 1 FOR MORE INFORMATION.

WITHIN 100' OF EACH OTHER PER UDOT STANDARDS.

ADDITIONAL REMOVAL

(NOTE 1)

UNTREATED BASE COURSE

OR FLOWABLE FILL

EXCAVATION BACKFILL

(SEE DRAWINGS OR

PLAN No. 381)

OR FLOWABLE FILL

DEEP EXCAVATION

(MORE THAN 48 INCHES FROM PAVEMENT SURFACE TO BOTTOM OF EXCAVATION)

8 MIN

(ASPHALT RESTORATION)

-NATCH EXISTING THICKNESS

8" MIN

off surface distortions. Coat planed surfaces with a cationic or anionic emulsion that

APWA Section 03 30 10. Cure to initial set before placing new asphalt concrete patch.

5. ASPHALT PAVEMENT: Use asphalt concrete specified in APWA Section 33 05 25.

B. Compact to 94 percent of ASTM D 2041 (Rice Method) plus or minus 2 percent.

deteriorating, (3) excavation is less than 3 feet square, (4) asphalt pavement is

greater. Maximum lift thickness before compaction is 8 inches when using riding

compaction equipment or 6 inches when using hand held compaction equipment.

excavations that are too narrow to receive compaction equipment.

A. Install in lifts no greater than 3 inches after compaction.

street fixture, seal the crack per APWA Section 32 01 17.

complies with APWA Section 32 12 03 and provide sand blotter.

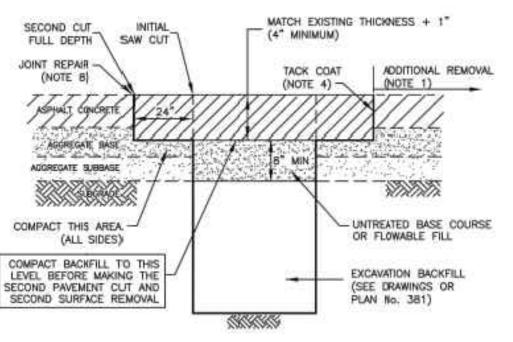
A. Required if existing concrete thickness is 6 inches or greater.

ASPHALT CONCRETE T-PATCH

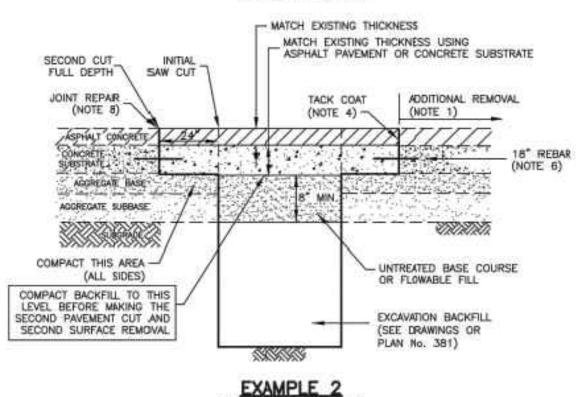
- 1. ADDITIONAL PAVEMENT REMOVAL: Remove additional pavement to a painted lane stripe, a lip of gutter, a curb, an existing pavement patch, or an edge of the pavement if such street feature is within 2 feet of the second saw-cut.
- 2. UNTREATED BASE COURSE: Provide material specified in APWA Section 32 11 23. A. Do not use gravel as a substitute for untreated base course without ENGINEER's permission.
- B. Place material per APWA Section 32 05 10.
- C. Compact per APWA Section 31 23 26 to a modified proctor density of 95 percent or greater. Maximum lift thickness before compaction is 8 inches when using riding compaction equipment or 6 inches when using hand held compaction equipment.
- 3. FLOWABLE FILL: Provide 28 day 60 psi controlled low strength material as specified in APWA Section 31 05 15. Use fill material which flows easily and vibration is not required. Cure to initial set before placing aggregate base or asphalt pavement. Use flowable fill in
- excavations that are too narrow to receive compaction equipment. 4. TACK COAT: APWA Section 32 12 14. Full tack coat coverage on all vertical surfaces. 5. ASPHALT PAVEMENT: Use asphalt concrete specified in APWA Section 33 05 25.
- A. Install in lifts no greater than 3 inches after compaction. B. Compact to 94 percent of ASTM D 2041 (Rice Method) plus or minus 2 percent. 6. REINFORCEMENT: ASTM A 615, Grade 60, No. 5 galvanized or epoxy coated deformed
- steel 12 inches on center. A. Required if existing concrete thickness is 6 inches or greater.
- B. Not required if (1) existing concrete is less than 6 inches thick, (2) existing concrete is deteriorating, (3) excavation is less than 3 feet square, (4) asphalt pavement is
- substituted for concrete substrate. 7. CONCRETE SUBSTRATE: Class 4000 per APWA Section 03 30 04. Place concrete per
- APWA Section 03 30 10. Cure to initial set before placing new asphalt concrete patch. 8. JOINT REPAIR: If a crack occurs at the "T" patch connection to existing pavement or at any street fixture, seal the crack per APWA Section 32 01 17.
- 9. PATCH REPAIR: Repair the asphalt pavement patch if any of the following conditions within the patch occur.
- A. Pavement surface distortion exceeds 1/4 inch deviation in 10 feet. Repair option: Plane off surface distortions. Coat planed surfaces with a cationic or anionic emulsion that complies with APWA Section 32 12 03 and provide sand blotter.
- B. Cracks at least 1-foot long and 1/4 inch wide occur more often than 1 in 10 square feet.
- C. Asphalt raveling is greater than 1 square foot per 100 square feet. Repair option: Mill

SHALLOW EXCAVATION

(LESS THAN 48 INCHES FROM PAVEMENT SURFACE TO BOTTOM OF EXCAVATION)



EXAMPLE 1 (ASPHALT RESTORATION)



Asphalt concrete "T" patch

March 2006

Plan No.

SECTION 02056

EMBANKMENT, BORROW, AND BACKFILL

1.1 SECTION INCLUDES

Embankment, backfill, and bridge approach embankments.

1.2 RELATED SECTIONS

GENERAL

PART 1

A. Section 02721: Untreated Base Course (UTBC)

B. Section 03575: Flowable Fill

1.3 REFERENCES

A. AASHTO M 145: Classification of Soils and Soil-Aggregate Mixtures for Highway Construction Purposes

AASHTO T 11: Materials Finer than 75 µm (No. 200) Sieve in Mineral Aggregates by Washing

C. AASHTO T 27: Sieve Analysis of Fine and Coarse Aggregates

AASHTO T 99: Moisture-Density Relations of Soils Using a 2.5 kg (5.5-lb) Rammer and a 305 mm (12 inch) Drop

AASHTO T180: Moisture-Density Relations of Soils Using a 4.54 kg (10lb) Rammer and a 457 mm (18 inch) Drop

F. UDOT Materials Manual of Instruction

G UDOT Minimum Sampling and Testing Requirements

1.4 DEFINITIONS

A. Borrow material – imported material for use in a constructed fill or backfill.

Embankment material - suitable material from project roadway excavation or other excavation for use in a constructed fill or backfill.

> Embankment, Borrow, and Backfill 02056 - Page 1 of 8

> > 2025 Standard Specifications Latest Revision: September 14, 2023

PART 2 PRODUCTS

2.1 GENERAL

Provide materials free of contamination from chemical or petroleum products for embankment, borrow, and backfill placements. Materials may include recycled Portland Cement Concrete

Do not include asphalt pavement materials.

2.2 MATERIALS

Classifications A-1-a through A-4. Refer to AASHTO M 145.

Granular Borrow

Classification A-1-a. Refer to AASHTO M 145.

Non-plastic. Meet the gradation requirements of Table 1

Granular Borrow Gradation Percent Passing Sieve Size 3 inch 90 - 100 1 inch 60 - 100 1/2 inch 30 - 80 25 - 65 No. 4 No. 10 0 - 50No. 40 0 - 30No. 200

UTBC meeting the requirements of Section 02721, may be used, at no additional cost to the Department, upon authorization of the Engineer.

Granular Backfill Borrow

Classification A-1-a. Refer to AASHTO M 145. Well-graded, 2 inch maximum.

Free-Draining Granular Backfill

Meet the gradation requirements of Table 2:

Embankment, Borrow, and Backfill 02056 - Page 3 of 8

2025 Standard Specifications Latest Revision: September 14, 2023 Well-graded material - Material having an even distribution of different particle sizes. This even distribution of particles of different sizes results in a dense mass upon compaction.

Know what's **below.**

Call 811 before you dig. **BLUE STAKES OF UTAH** www.bluestakes.org

1.5 SUBMITTALS

Provide the following for information before delivering material to the

Supplier and source of materials. Gradation analysis. Refer to AASHTO T 27 and T 11.

Soil classification when applicable. Refer to AASHTO M 145. Maximum Dry Density and Optimum Moisture Determination a. Use AASHTO T 180 Method D for A-1 soils and AASHTO T 99 Method D for all other soils.

Requests, for review, to use Untreated Base Course (UTBC) instead of granular borrow.

Engineering proposals for review for alternate materials or trench configurations for drainage pipe bedding and pipe backfill as outlined in this Section, 2.2 G. Include all of the following:

Stamped drawings and specifications signed and sealed by a Professional Engineer licensed in the state of Utah.

Evaluation of site specific conditions and surrounding soils.

including potential for migration of fines. A structural evaluation of the pipe support system for the proposed

pipe that includes the pipe structural capacity and the depth of fill. Complete bedding or backfill source information including

gradation, soil classification, and laboratory testing reports.

Proposals, for review, to place an initial layer of granular material as a working platform.

1.6 ACCEPTANCE

Acceptance sampling and testing is according to UDOT Minimum Sampling and Testing Requirements.

The Engineer reserves the right to select and test material from any location at the construction site. The Engineer will establish the limits of nonconforming material

Remove nonconforming material and replace with acceptable material.

sampled non-randomly.

Embankment, Borrow, and Backfill 02056 - Page 2 of 8

2025 Standard Specifications Latest Revision: September 14, 2023

Free Draining Granular Backfill Gradation Percent Passing 0-15 3/4 inch

Embankment for Bridge

Classification A-1. Refer to AASHTO M 145. 3 inch maximum.

Embankment Material

Roadway excavation and other excavation material. Do not include unsuitable materials such as organic, frozen.

or contaminated soils.

Do not use rock or broken concrete materials with any dimension over 1 ft.

Borrow may be substituted for embankment material.

Drainage Pipe Bedding and Drainage Pipe Backfill Classification A-1. Refer to AASHTO M145

a. Well-graded material.

Maximum aggregate size is 11/2 inches for plastic pipe, 2

Use only for drainage pipe backfill.

inches for all other pipes. Flowable fill. Refer to Section 03575

Other materials or trench configurations for drainage pipe bedding and backfill may be used when authorized.

 Native materials or uniformly graded materials enclosed in an appropriate drainage geotextile may be proposed.

PART 3 EXECUTION

3.1 GENERAL

 Complete clearing, grubbing, stripping, and stockpiling topsoil, and any necessary excavation before placing material.

Requirements when placing material during freezing or snowy conditions: Do not place embankment, borrow, or backfill material on frozen or snow-covered areas.

> Embankment, Borrow, and Backfill 02056 - Page 4 of 8

CITY ENGINEER

2025 Standard Specifications Latest Revision: September 14, 2023

DATE

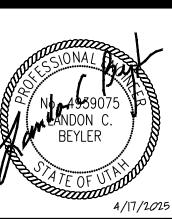
CITY ENGINEER APPROVAL

I CERTIFY THAT I HAVE EXAMINED THIS PLAT AND FIND IT TO BE IN

GENERAL COMPLIANCE TO THE CITY STANDARDS

age

Õ 8 Y



JOB NUMBER 24.00160 SHEET 5 OF 10

Plan No. Asphalt concrete "T" patch 255 Drawing 1 of 2 Merch 2006 Drawing 2 of 2

EXAMPLE B

(COMPOSITE RESTORATION)

CIVIL IMPROVEMENT PLANS PARCEL #'s 01-050-0023 & 01-050-0024

Free-Draining Granular Backfill

soft, wet ground when authorized by the Engineer.

placed above the working platform.

3.2 EMBANKMENT MATERIAL AND BORROW PLACEMENT

embankment height is 6 ft or less.

section.

excavation shown.

Maintain Drainage

concrete.

and under-drains.

3.2 PROCEDURE - ASPHALT SURFACES

full depth of the surface.

propagate.

broken vertical edge.

appropriate. Refer to Section 02748.

Compact each lift to 100 percent of the developed field

Place an initial layer of granular material to act as a working platform over

Density requirements do not apply to the working platform except

Meet density requirements for embankment, borrow, or backfill

Place embankment material or borrow or both in the embankment section with the highest quality material in the top portion of the embankment

foundation to at least 90 percent of maximum laboratory density when the

Remove other pavement surfaces that are not portland cement

Grade and maintain the roadway to provide adequate drainage.

Maintain drainage pipes and drainage ditches or provide temporary

facilities when interrupting items such as irrigation systems, sewers,

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Scarify and compact the top eight inches of the working platform or

Break and scarify all underlying concrete pavement surfaces so that

pieces do not exceed 1 ft² before placing material over an existing

concrete pavement surface that is outside the limits of removal or

Embankment, Borrow, and Backfill

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existing surface is cracked, broken, or deteriorated.

Do not allow traffic or construction equipment to cross the cut edge.

END OF SECTION

Concrete and Asphalt Cutting

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D. Apply a tack coat to the cut edge before placing asphalt pavement when

Use any method that provides a vertical cut in a straight line through the

Saw cut if the method of cutting does not produce a smooth, non-

Make cuts so the defective surface can be removed where the edge of the

Verify that the entire deficient areas are removed and will not

Do not place initial layer of embankment, borrow, or backfill until the

Engineer inspects and verifies the working platform or foundation.

Instruction Section 989.

as specified in this Section, Paragraph 3.2 B.

The Department develops a field density compaction

curve according to UDOT Materials Manual of

- Remove snow and frozen material and furnish specified materials that can be compacted to the specified density. 1) Measure removed material and provide quantities to
 - the Engineer. The Department does not pay for removed material or material replacement when it would otherwise meet specification requirements if unfrozen.
- Do not deliver or use frozen material.
- Use appropriate compaction equipment adjacent to pipes, abutments, back walls, approach slabs, wing walls, retaining walls, and other
- Expand the width of the trench to accommodate necessary
- compaction equipment. Compact by hand areas where compaction equipment cannot compact the soil.
- Compaction Requirements
 - Borrow, Drainage Pipe Bedding, Embankment Material, Embankment for Bridge, Granular Backfill Borrow and Granular Borrow
 - Compact each lift to a minimum average of 96 percent of maximum laboratory density with no single determination
 - lower than 92 percent. 1) Use AASHTO T 180 Method D for A-1 soils and AASHTO T 99 Method D for all other soils to establish maximum laboratory density.
 - Maintain appropriate moisture for compaction during
 - Drainage Pipe Backfill
 - Compact each lift to a minimum average of 92 percent maximum laboratory density with no single determination less than 90 percent.
 - Use AASHTO T 180 Method D for A-1 soils.
 - Maintain appropriate moisture for compaction during
 - Meet the pavement section material density requirement for pipes that encroach into the pavement section or use
 - Material with more than 30 percent retained on the 34 inch sieve Compact each lift to 100 percent of the developed field
 - The Department develops a field density compaction curve according to UDOT Materials Manual of Instruction Section 989.

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SECTION 02705

CONCRETE AND ASPHALT CUTTING

PART 1 GENERAL

- 1.1 SECTION INCLUDES
 - A. Saw or cut existing pavements, curb and gutter, sidewalk, and any appurtenances as required to provide a smooth surface to match.
 - Does not apply to new Portland cement concrete pavement (PCCP) joint sawing. Refer to Section 02752.
- 1.2 RELATED SECTIONS
- A. Section 02748: Prime Coat/Tack Coat
- Section 02752: Portland Cement Concrete Pavement

Not Used

- 1.3 REFERENCES Not Used 1.4 DEFINITIONS Not Used
- PART 2 PRODUCTS Not Used

PART 3 EXECUTION

1.5 SUBMITTALS

- 3.1 PROCEDURE CONCRETE SURFACES
 - Saw cut vertically in a straight line through the full depth of the surface.
 - Make cuts so the defective surface can be removed where the edge of the existing surface is cracked, broken, or deteriorated. Verify that the entire deficient areas are removed and will not
 - Do not allow traffic or construction equipment to cross the cut edge.

Concrete and Asphalt Cutting 02705 - Page 1 of 2

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unsatisfactory density. Distribute larger particles so space exists for placing and compacting remaining material.

depth) and compact to the density requirements.

Do not place rocks or broken concrete larger than 4 inches within 1 ft of the subgrade surface.

Reduce the lift thickness or modify operations if tests show

Spread material uniformly in layers not exceeding 1 ft (uncompacted

- F. Finish subgrade surface within ±0.2 ft of line and grade.
- Do not use compacting equipment that causes shear failure in the constructed fill or backfill.
- 3.3 GRANULAR BORROW, GRANULAR BACKFILL BORROW, AND BACKFILL
 - Compact material in maximum 6 inch layers (uncompacted depth) to the density requirement.
 - Finish surface within ± 0.1 ft of line and grade.
 - Backfill catch basins, cleanout boxes, manholes, drainage boxes, and diversion boxes with Granular Backfill Borrow unless otherwise specified

3.4 DRAINAGE PIPE FOUNDATION, BEDDING, AND BACKFILL PLACEMENT

- Place in 6 inch layers (uncompacted depth) and compact to the density
- Place uniform layers of drainage pipe backfill on both sides of the pipe and compact to the density requirement before placing successive lifts.
- Fully compact the haunch areas.

3.5 EMBANKMENT FOR BRIDGE PLACEMENT

- Construct bridge approach embankments from the existing ground up with the specified material to the limits defined in this Section and according to GW Series Standard Drawings. Approach Embankments
 - Place embankment for bridge beneath the bridge except riprap or other described materials used for MSE walls.

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SECTION 02721

UNTREATED BASE COURSE (UTBC)

GENERAL

- 1.1 SECTION INCLUDES
 - Production, construction, and compaction of UTBC used for pavements, shoulders, and incidental construction.

1.2 RELATED SECTIONS

A. Section 01572: Dust Control and Watering

1.3 REFERENCES

- AASHTO T 11: Materials Finer than 75-um (No. 200) Sieve in Mineral Aggregates by Washing
- AASHTO T 19: Bulk Density ("Unit Weight") and Voids in Aggregate
- AASHTO T 27: Sieve Analysis of Fine and Coarse Aggregates
- AASHTO T 89: Determining the Liquid Limit of Soils
- AASHTO T 90: Determining the Plastic Limit and Plasticity Index of Soils
- AASHTO T 96: Resistance to Degradation of Small-Sized Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine
- AASHTO T 180: Moisture-Density Relations of Soils Using a 4.54 kg (10 lb) Rammer and 457 mm (18 in) Drop
- AASHTO T 193: The California Bearing Ratio
- AASHTO T 255: Total Evaporable Moisture Content of Aggregate by
- AASHTO T 335: Determining the Percent of Fracture in Coarse Aggregate

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- Place embankment for bridge to extend at least 150 ft from the centerline of the bridge abutment as measured along the approach roadway alignment and on the inside of abutments.
- Use the described material throughout the length of the walls where retaining walls are located beyond this delineation.
- Intersecting Roadway Embankments Place embankment for bridge along the intersecting roadway alignment(s) at least 150 ft from the abutment centerline station as measured along the approach and intersecting
- Spread embankment for bridge uniformly in layers not exceeding 1 ft (uncompacted depth) and compact to the specified density requirements before placing the next layer.
- Reduce the lift thickness if tests show unsatisfactory density.

3.6 FREE-DRAINING GRANULAR BACKFILL PLACEMENT

Finish surface within ±0.2 ft of line and grade.

- Compact material in 1 ft maximum layers.
- Finish surface within ±0.2 ft of line and grade

END OF SECTION

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1.4 DEFINITIONS Not Used

- 1.5 SUBMITTALS
 - Written report for approval for each aggregate class and source, a minimum of five working days before placement. Include the following:
 - Aggregate suitability. Refer to this Section, Part 2. Name of supplier and location of source.
 - Maximum Dry Density and Optimum Moisture Content and
 - associated test result data. Refer to AASHTO T 180, Method D.
 - Job mix gradation including single values for each sieve size, No. 4 and finer. The target values must be within the gradation limits of Table 2.
 - Job-mix gradation changes Refer to this Section, Article 3.2.

1.6 ACCEPTANCE

- A. Type I Placement Pavement Section Use Class A aggregate, Table 1.
 - The Engineer takes random samples from the grade and tests for moisture, gradation, and laboratory density and performs in-place density determinations.
 - Meet gradation limits and applicable tolerances of Table 2 for each
 - Evaluate each sublot separately and do not average with other sublots. Meet minimum density test average of 97 percent of maximum
 - laboratory density with no test less than 94 percent. Type II Placement - Incidental includes placement for Curb, Curb and Gutter, Driveways, Pedestrian Access Ramps, Sidewalk, Waterways,

Flatwork, and other items of work in the contract to which UTBC is

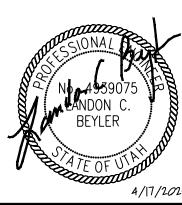
- included and not measured or paid for separately. Use Class A aggregate, Table 1. The Engineer takes random samples from the grade and tests for moisture, gradation, and laboratory density and performs in-place
- density determinations. Meet gradation limits and applicable tolerances of Table 2 for each
- Each sublot will be evaluated separately and not averaged with other sublots.
- Meet minimum density test average of 95 percent of maximum laboratory density with no test less than 92 percent.

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CITY ENGINEER APPROVAL I CERTIFY THAT I HAVE EXAMINED THIS PLAT AND FIND IT TO BE IN

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DATE CITY ENGINEER

Use Class A or B aggregate, Table 1. Adjust moisture content before compaction

Material not meeting the gradation requirements may be allowed to remain in-place at the discretion of the Engineer provided density requirements

Additional lots may not be placed until the deficiencies are addressed and corrected.

Correct material that does not meet the specified criteria by scarifying, placing additional material, re-mixing, reshaping, and re-compacting when determined by the Engineer.

Do not place additional material on any unaccepted layer.

PART 2 PRODUCTS

2.1 AGGREGATES

Well-graded, clean, hard, tough, durable, and sound mineral aggregates consisting of crushed stone, crushed gravel, or crushed slag, free of organic matter and contamination from chemical or petroleum products, according to Table 1.

	Table 1		
Aggre	gate Propert	ies	
= 88	Aggreg	ate Class	
	Α	В	
Dry Rodded Unit Weight	Not less that	n 75 lb/ft ³	AASHTO T 19
Liquid Limit/Plastic Index	Non-plastic	PI≤6	AASHTO T 89 AASHTO T 90
Aggregate Wear	Not to excee	ed 50 percent	AASHTO T 96
Gradation	Table 2		AASHTO T 11 AASHTO T 27
CBR with a 10 lb surcharge measured at 0.20 inch penetration	70% Minimum	N/A	AASHTO T 193
Two Fractured Faces	50% Min	N/A	AASHTO T 335

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AASHTO T 90: Determining the Plastic Limit and Plasticity Index of Soils

AASHTO T 96: Resistance to Degradation of Small-Size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine

AASHTO T 104: Soundness of Aggregate by Use of Sodium Sulfate or

J. AASHTO T 112: Clay Lumps and Friable Particles in Aggregate

AASHTO T 176: Plastic Fines in Graded Aggregates and Soils by Use of the Sand Equivalent Test

AASHTO T 195: Determining Degree of Particle Coating of Asphalt

AASHTO T 209: Theoretical Maximum Specific Gravity and Density of

AASHTO T 255: Total Evaporable Moisture Content of Aggregate by

AASHTO T 304: Uncompacted Void Content of Fine Aggregate

AASHTO T 335: Determining the Percentage of Fracture in Coarse

Q. UDOT Materials Manual of Instruction

UDOT Minimum Sampling and Testing Requirements

S. UDOT Quality Management Plans

1.4 DEFINITIONS

Longitudinal Joint – Any new asphalt lift abutting an existing paving lift. This includes joints created by echelon paving and new asphalt placed against a milled asphalt edge.

Lot - The amount of Asphalt Mix placed in a single Production Day.

Minor Target Change – A change from the verified mix design gradation target on a maximum of two sieves with the following limitations. The maximum change from the verified target gradation on the No. 8 or any coarser sieve is limited to 3 percent passing per sieve.

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Establish the job mix (target) gradation for the 1/4 inch sieve and finer within the gradation limits.

The Job Mix Gradation Tolerance is the allowable deviation from the job mix (target) gradation on the applicable sieves.

All other percents passing will be within the gradation limits. Refer to AASHTO T 11 and AASHTO T 27.

Gradation Limits						
Sieve Size	Job Mix Gradation Target Band	Job Mix Gradation Tolerance				
1½ inch	100					
1 inch	90 - 100	±9.0				
¾ inch	70 - 85	±9.0				
1/2 inch	65 - 80	±9.0				
% inch	55 - 75	±9.0				
No. 4	40 - 65	±7.0				
No. 16	25 - 40	±5.0				
No. 200	7 - 11	±3.0				

aggregate with approximately the same bulk specific gravities.

PART 3 EXECUTION

3.1 PREPARATION

Remove vegetation before Type III placement. Refer to Section 02231. Protect existing delineators in place.

3.2 INSTALLATION

 Provide moisture content of ± 2 percent of optimum at the time of placement. Refer to AASHTO T 180, Method D and AASHTO T 255.

Procedures for Changing the Job-Mix Gradation Submit changes in writing 24 hours before placement for approval by the Engineer.

 Place in layers of uniform thickness and compact each layer to a thickness not to exceed a 6 inch depth. Do not place on any frozen surface. Refer to Section 01572.

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The maximum change from the verified target gradation on the No. 16 or No. 50 sieves is 2 percent passing per sieve.

The maximum change from the verified target gradation on the No. 200 sieve is 0.5 percent passing.

No target change may violate the mix design requirements in this

Overband – an 8 inch protective asphalt coating sealing the longitudinal joint of final riding surface, as proposed by the contractor and approved by the Engineer

E. Production Day – A 24 hour period in which Asphalt Mix is being placed.

RAP - Recycled Asphalt Pavement. Crushed or milled asphalt materials that have been removed from pavements for recycling.

G. Thin Overlay Pavement - New Asphalt Mix design thickness less than 2

Lane-Leveling - Variable depth paving to correct minor rutting and longitudinal variations in the roadway. Depth varies from the maximum aggregate size to the depth needed to correct variations.

Profile leveling - Variable depth paving to correct minor profile variations in the roadway. Depth varies from the maximum aggregate size to the depth needed to correct variations.

1.5 SUBMITTALS

Mix design for verification and approval before paving according to UDOT Materials Manual of Instruction Section 960.

Changes in job mix design

Submit a written request for any proposed change in the job-mix

 Allow at least 12 hours for approval before incorporating a minor target change into production. Allow at least six working days for verification and approval

of any other change. Include documentation supporting correlation between suggested target changes and mix design volumetric requirements.

 Acceptable documentation may include Department or Contractor testing data. Submit samples according to the UDOT Materials Manual of Instruction 960 for a volumetric mix design verification for anything

other than approved minor target changes. Asphalt Mix 02741 - 3 of 22 2025 Standard Specifications

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D. Finish to a uniform line and grade with surface deviations no more than \(\frac{\pi}{2} \) inch in 10 ft in any direction.

Correct any profile deviations greater than % inch. Rework minimum of 4 inch lift to achieve homogeneous

Determine limits of correction based on extent of deviation.

Continue finishing until existing deviation is less than \(\frac{1}{2} \) inch.

 E. Maintain optimum moisture content ± 2 percent during compaction. Use appropriate compaction equipment adjacent to abutments, backwalls, approach slabs, wing walls, retaining walls, and other

Use a minimum of two passes with a roller for Type III placement or as directed by the Engineer.

END OF SECTION

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Corrective action plan for approval according to this Section, Article 3.3, paragraph C2 and Article 3.4, paragraph A4b.

Refer to this Section, Article 3.4 for laboratory correlation submittals for

Mat joint layout plan to the Engineer for review before placement.

1.6 ACCEPTANCE

Acceptance sampling and testing of material is according to UDOT Minimum Sampling and Testing Requirements.

Gradation and asphalt binder content

The Engineer evaluates a lot on the test results of four or more samples, except when only three samples can be taken.

Evaluate the lot using the number of tests "n" in Table 3. The Engineer informs the Contractor of the time and place of sampling not more than 15 minutes before sampling.

Increase sample sizes to accommodate validation or third-party testing as required.

Density and Thickness

Obtain cores from the mat and longitudinal joint within two calendar days after the pavement is placed and according to UDOT Materials Manual of Instruction, Section 984. The Engineer marks coring location for in-place mat density

and longitudinal joint density cores. Fill core holes with Asphalt Mix, SMA or high-asphalt-content

cold mix and compact in thin lifts within 24 hours and before returning to traffic. The Department witnesses the coring operation, takes

possession of the cores immediately, and begins testing the cores within 24 hours for density acceptance.

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SECTION 02741

ASPHALT MIX

PART 1 GENERAL

1.1 SECTION INCLUDES

Flexible pavement consisting of one or more layers of an asphalt mixture comprised of aggregate, asphalt binder, hydrated lime, and other

An option to incorporate Reclaimed Asphalt Pavement (RAP) materials into Asphalt Mix.

1.2 RELATED SECTIONS

A. Section 01456: Materials Dispute Resolution

B. Section 02701: Pavement Smoothness

C. Section 02742S: Project Specific Surfacing Requirements

D. Section 02745: Asphalt Material

E. Section 02746: Hydrated Lime

F. Section 02748: Prime Coat/Tack Coat

1.3 REFERENCES

A. AASHTO M 323: Superpave Volumetric Mix Design

AASHTO R 35: Superpave Volumetric Design for Asphalt Mixtures

AASHTO T 11: Materials Finer Than 75 µm (No. 200) Sieve in Mineral Aggregates by Washing

D. AASHTO T 19: Bulk Density ("Unit Weight") and Voids in Aggregate

E. AASHTO T 27: Sieve Analysis of Fine and Coarse Aggregates

F. AASHTO T89: Determining the Liquid Limit of Soils

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pavements.

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Density Requirements

The target for in-place density for the mat is 93.5 percent of Theoretical Maximum Specific Gravity except for thin overlay

b. The target for in-place density for the longitudinal joint is 91.5 percent of the Theoretical Maximum Specific Gravity

The target for in-place density is 92.5 percent of theoretical maximum specific gravity for thin overlay pavements. Do not take longitudinal joint cores for thin overlay

Thickness is evaluated with mat density cores. The thickness requirement may be waived when matching up to existing pavement, curb and gutter for Pavement in or next to intersections.

 The Department accepts a lot for thickness when: The average thickness is not more than ½ inch greater or 1/4 inch less than the total design thickness

No individual sublot shows a deficient thickness of

1) The Department pays for 50 percent of the mix for

more than % inch. Excess Thickness - The Engineer may allow excess thickness to remain in place or may order its removal.

material in excess of the +1/2 inch tolerance when excess thickness is allowed to remain in place. Deficient Thickness - Place additional material where lots or sublots are deficient in thickness.

The Department pays for material necessary to reach specified thickness.

The Department pays for 50 percent of the mix for additional material over specified thickness necessary to achieve minimum lift thickness. Minimum compacted lift is 3 times the nominal

maximum aggregate size. Thickness tolerances established above do not apply to leveling courses.

 Check final surfaces in staged construction. Check thickness regularly with a depth probe during placement and take corrective action as necessary.

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> > > CITY ENGINEER

CITY ENGINEER APPROVAL I CERTIFY THAT I HAVE EXAMINED THIS PLAT AND FIND IT TO BE IN

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JOB NUMBER 24.00160 SHEET

CIVIL IMPROVEMENT PLANS PARCEL #'s 01-050-0023 & 01-050-0024

- Longitudinal Joint
 - The edge of a new asphalt mat may be removed for the purpose of meeting longitudinal joint density requirements.
 - The material wasted is still included in the payment. Up to 3 inches for a confined edge is allowed.
 - Up to 6 inches for an unconfined edge is allowed.
- The Department applies one Incentive/Disincentive for the lowest dollar value for Gradation/Asphalt Content, one Incentive/Disincentive for In-Place Mat Density, and one Incentive/Disincentive for Longitudinal Joint Density. The Engineer computes Incentives/Disincentives as follows for each lot
- Compute incentive/disincentive for Gradation/Asphalt Binder and In-place Mat Density and Longitudinal Joint Density according to
- Base the incentive/disincentive on Percent within Limit (PT) computation using Tables 2, 3, and 4.
- Use lowest single PT value combined for gradation (each of the sieves) and asphalt binder content for calculating the
- gradation/asphalt binder content incentive/disincentive. Use Tables 2, 3, and 4 to determine PT for in-place Mat Density
- and Longitudinal Joint Density. Meet PT of 88 or greater for in-place mat density or the Department does not pay incentives on joint density or gradation/asphalt binder
- content except for lane-leveling material. The Department pays or assesses the longitudinal joint density incentive/disincentive per ton of Asphalt Mix placed adjacent to, and on the hot side of the longitudinal joint for each lift:
 - The incentive/disincentive will be calculated from the core densities taken from all abutting joints if the Asphalt Mix mat has a longitudinal joint on more than one side.
- The Department applies incentive/disincentive for smoothness according
- Refer to Section 02701 for smoothness requirements.
- F. The Department rejects lots:
- If the PT for any individual gradation measurement is less than 52 percent as shown in Table 1.
- If the PT for asphalt binder content or mat density measurement is less than 60 percent as shown in Table 1.

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	Qualit		Index Values (QU or QL) for Estimating Pe				Percent	Within			
PU or PL	n=3	n=4	n=5	n=6	n=7	n=8	n=10	n≃12	n=15	n=20	
100	1.16	1.50	1.75	1.91	2.06	2.15	2.29	2.35	2.47	2.56	
99	1.16	1.47	1.68	1.79	1.89	1.95	2.04	2.09	2.14	2.19	
98	1.15	1.44	1.61	1.70	1.77	1.80	1.86	1.89	1.93	1.97	
97	1.15	1.41	1.55	1.62	1.67	1.69	1.74	1.77	1.80	1.82	
96	1.15	1.38	1.49	1.55	1.59	1.61	1.64	1.66	1.69	1.70	
95	1.14	1.35	1.45	1.49	1.52	1.54	1.56	1.57	1.59	1.61	
94	1.13	1.32	1.40	1.44	1.46	1.47	1.49	1.50	1.51	1.53	
93	1.12	1.29	1.36	1.38	1.40	1.41	1.43	1.43	1.44	1.46	
92	1.11	1.26	1.31	1.33	1.35	1.36	1.37	1.37	1.38	1.39	
91	1.10	1.23	1.27	1.29	1.30	1.31	1.32	1.32	1.32	1.33	
90	1.09	1.20	1.23	1.24	1.25	1.25	1.26	1.26	1.27	1.27	
89	1.08	1.17	1:20	1.21	1.21	1.21	1.21	1.21	1.22	1.22	
88	1.07	1.14	1.16	1.17	1.17	1.17	1.17	1.17	1.17	1.17	
87	1.06	1.11	1.12.	1.12	1.12	1.13	1.13	1.13	1.13	1.13	
86	1.05	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	1.08	
85	1.03	1.05	1.05	1.05	1.05	1.04	1.04	1.04	1.04	1.04	
84	1.02	1.02	1.02	1.01	1.01	1.01	1.00	1.00	1.00	1.00	
83	1.00	0.99	0.98	0.97	0.97	0.96	0.96	0.96	0.96	0.96	
82	0.98	0.96	0.95	0.94	0.94	0.93	0.93	0.92	0.92	0.92	
81	0.96	0.93	0.92	0.91	0.90	0.90	0.89	0.89	0.89	0.88	
80	0.94	0.90	0.88	0.87	0.86	0.86	0.85	0.85	0.85	0.85	
79	0.92	0.87	0.85	0.84	0.83	0.83	0.82	0.82	0.82	0.81	
78	0.89	0.84	0.82	0.81	0.80	0.79	0.79	0.78	0.78	0.78	
77	0.87	0.81	0.79	.0.78	0.77	0.76	0.76	0.75	0.75	0.75	
76	0.84	0.78	0.76	0.75	0.74	0.73	0.72	0.72	0.72	0.72	
75	0.82	0.75	0.73	0.72	0.71	0.70	0.69	0.69	0.69	0.68	
74	0.79	0.72	0.70	0.68	0.67	0.67	0.66	0.66	0.66	0.65	
73	0.77	0.69	0.67	0.65	0.64	0.64	0.62	0.62	0.62	0.62	
72	0.74	0.66	0.64	0.62	0.61	0.61	0.60	0.59	0.59	0.59	
71	0.71	0.63	0.60	0.59	0.58	0.58	0.57	0.56	0.56	0.56	
70	0.68	0.60	0.58	0.56	0.55	0.55	0.54	0.54	0.54	0.53	
69	0.65	0.57	0.55	0.54	0.53	0.52	0.51	0.51	0.51	0.50	
68	0.62	0.54	0.52	0.51	0.50	0.50	0.48	0.48	0.48	0.48	
67	0.59	0.51	0.49	0.48	0.47	0.47	0.46	0.45	0.45	0.45	
66	0.56	0.48	0.46	0.45	0.44	0.44	0.43	0.42	0.42	0.42	
65	0.53	0.45	0.43	0.42	0.41	0.41	0.40	0.40	0.40	0.39	
64	0.49	0.42	0.40	0.39	0.38	0.38	0.37	0.37	0.37	0.37	
63	0.46	0.39	0.37	0.36	0.35	0.35	0.35	0.34	0.34	0.34	
62	0.43	0.36	0.34	0.33	0.33	0.33	0.32	0.31	0.31	0.31	
61	0.39	0.33	0.31	0.30	0.30	0.30	0.29	0.29	0.29	0.28	
60	0.36	0.30	0.28	0.27	0.26	0.26	0.25	0.25	0.25	0.25	
59	0.32	0.27	0.25	0.25	0.24	0.24	0.24	0.23	0.23	0.23	

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The Engineer may accept a reject or non-conforming lot. Refer to Section 01456.

a. A price reduction of 35 percent of the pay item or \$20 per ton, whichever is greater, will be assessed. The lot will not be eligible for any incentive.

- The Engineer may elect to accept material on visual inspection according to the Minimum Sampling and Testing Requirements. Incentives/Disincentives are not applied to material accepted
 - The Engineer reserves the option of conducting any acceptance tests necessary to determine that the material and workmanship meets the project requirements.
- Meet production control requirements of Table 9. Material placed within the Cease Production Limit in Table 9 is not eligible for incentives.

1.7 DISPUTE RESOLUTION

Refer to Section 01456 when disputing the validity of the Department's acceptance tests.

Incentive/Disincentive for Aspha	It Binder Content, and Mat Density
PT Based on Min. Four Samples	Incentive/Disincentive (Dollars/Ton)
>99	2.00
96-99	1.50
92-95	1.00
88-91	0.00
84-87	-0.26
80-83	-0.60
76-79	-0.93
07070777	5.555
72-75	-1.27
68-71	-1.60
64-67	-1.93
60-63	-2,27
<60	Reject
Incentive/Disince	entive for Gradation
PT Based on Min. Four Samples	Incentive/Disincentive (Dollars/Ton)
>99	2.00
96-99	1.50
92-95	1.00
88-91	0.00
84-87	-0.26
80-83	-0.60
76-79	-0.93
671 5871 B	
72-75	-1.27
68-71	-1.60
64-67	-1.93
60-63	-2.27
56-59	-5.00
52-55	-10.00
<52	Reject
Incentive/Disincentive for	Longitudinal Joint Density
PT Based on Min Four Samples	Incentive/Disincentive (Dollars/Ton)
>99	2.00
96-99	1.50
92-95	1.00
88-91	0.00
84-87	-0.26
80-83	-0.60
76-79	-0.60
AND DOM:	
72-75	-1.27
68-71	-1.60
64-67	-1.93
60-63	-2.27
56-59	-2.60
52-55	-5.00
<52	Apply \$5 penalty and Overband Longitudinal Joint if Final Surface Lift

Table 1

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				Tabl	e 3 Conti	nued				
PU/PL	n=3	n=4	n=5	n=6	n=7	n=8	n=10	n=12	n=15	n=20
58	0.29	0.24	0.23	0.22	0.21	0.21	0.21	0.21	0.21	0.20
57	0.25	0.21	0.20	0.19	0.19	0.19	0.18	0.18	0.18	0.18
56	0.22	0.18	0.17	0.16	0.16	0.16	0.16	0.16	0.15	0.15
55	0.18	0.15	0.14	0.14	0.13	0.13	0.13	0.13	0.13	0.13
54	0.14	0.12	0.11	0.11	0.11	0.11	0.10	0.10	0.10	0.10
53	0.11	0.09	0.08	0.08	0.08	0.08	0.08	0.08	0.08	0.08
52	0.07	0.06	0.06	0.05	0.05	0.05	0.05	0.05	0.05	0.05
51	0.04	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03
50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Asphalt Mix

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Table 4

Definitions, A	bbreviations, and Formulas for Acceptance
Term	Explanation
arget Value (TV)	The target values for gradation and asphalt binder content are given in the Contractor's volumetric mix design. See this Section, Article 1.6 for density target values.
verage (AVE)	The sum of the lot's test results for a measured characteristic divided by the number of test results—the arithmetic mean.
tandard Deviations (s)	The square root of the value formed by summing the squared difference between the individual test results of a measured characteristic and AVE, divided by the number of test results minus one.
pper Limit (UL)	The value above the TV of each measured characteristic that defines the upper limit of acceptable production. (Table 2)
ower Limit (LL)	The value below the TV of each measured characteristic that defines the lower limit of acceptable production. (Table 2)
pper Quality Index (QU)	QU = (UL - AVE)/s
ower Quality Index (QL)	QL = (AVE - LL)/s
ercentage of Lot Within L (PU)	Determined by entering Table 3 with QU.
ercentage of Lot Within . (PL)	Determined by entering Table 3 with QL.
otal Percentage of Lot lithin UL and LL (PT)	PT = (PU + PL) - 100
centive/Disincentive	Determined by entering Table 1 with PT or PL.

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PART 2 PRODUCTS

- 2.1 ASPHALT BINDER
 - A. Project Specific Surfacing Requirements Refer to Section 02742S.
 - Asphalt Material Refer to Section 02745 and Quality Management Plan 509: Asphalt Binder.

2.2 AGGREGATE

- Crusher produced virgin aggregate material consisting of crushed stone,
- Refer to Table 5 to determine the suitability of the aggregate. Coarse aggregates
 - Retained on No. 4 sieve, AASHTO T 27 Fine aggregates
 - Clean, hard grained, and angular Passing the No. 4 sieve, AASHTO T 27
- C. Meet the gradation requirements in Table 6. (AASHTO T 11, AASHTO T

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Parameter	UL and LL
% inch sieve for % inch Asphalt Mix No. 4 sieve for % inch Asphalt Mix	Target Value ± 6.0%
No. 8 sieve	Target Value ± 5.0%
No.50 sieve	Target Value ± 3.0%
No. 200 sieve	Target Value ± 2.0%
Asphalt Binder Content	Target Value ± 0.35%
Mat Density	Lower Limit Target Value - 2.0% Upper Limit Target Value + 4.0%
Longitudinal Joint Density	Lower Limit Target Value - 2.0% Upper Limit Target Value + 6.0%

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		Table 5	
	Aggregate	Properties - Asphalt	Mix
Test Method	Test No.	75 Design Gyrations and Greater	Less Than 75 Design Gyrations
One Fractured Face	AASHTO T 335	95% minimum	90% minimum
Two Fractured Faces	AASHTO T 335	90% minimum	90% minimum
Fine Aggregate Angularity	AASHTO T 304	45 minimum	45 minimum
Flakiness Index	UDOT MOI 933 (Based on % inch sieve and above)	17% maximum	17% maximum
L.A. Wear	AASHTO T 96	35% maximum	40% maximum
Sand Equivalent	AASHTO T 176, alternate method 2, pre-wet method (test the sample in the wet condition).	60 minimum	45 minimum
Plasticity Index	AASHTO T 89 and T 90	0	0
Unit Weight	AASHTO T 19	minimum 75 lb/ft ³	minimum 75 lb/ ft ³
Soundness (sodium sulfate)	AASHTO T 104	16% maximum loss with five cycles	16% maximum loss with five cycles
Clay Lumps and Friable Particles	AASHTO T 112	2% maximum	2% maximum
Natural Fines	N/A	0%	10% maximum

Table 6

Aggregate Gra	dations (Percen	t Passing by Dry Weig	ght of Aggregate)
Sieve	Size	½ inch	¾ inch
Control Sieves	¾ inch	100.0	
	1/2 inch	90.0 - 100.0	100.0
	¾ inch	< 90	90.0 - 100.0
	No. 4		< 90
	No. 8	28.0 - 58.0	32.0 - 67.0
	No. 200	2.0 - 10.0	2.0 - 10.0

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JOB NUMBER

CIVIL IMPROVEMENT PLANS PARCEL #'s 01-050-0023 & 01-050-0024

SECTION 02785

CHIP SEAL COAT

1.1 SECTION INCLUDES

GENERAL

PART 1

Materials and procedures for applying emulsified asphalt, followed with an application of, either a standard chip seal cover material or lightweight chip seal cover material and bituminous flush coat.

1.2 RELATED SECTIONS

- A. Section 02742S: Project Specific Surfacing Requirements
- Section 02745: Asphalt Material
- C. Section 02748: Prime Coat/Tack Coat

1.3 REFERENCES

- A. AASHTO T 11: Materials Finer Than 75 µm (No. 200) Sieve in Mineral Aggregates by Washing
- AASHTO T 19: Bulk Density (Unit Weight) and Voids in Aggregate
- C. AASHTO T 27: Sieve Analysis of Fine and Coarse Aggregates
- AASHTO T 96: Resistance to Degradation of Small Size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine
- AASHTO T 104: Soundness of Aggregates by Use of Sodium Sulfate or Magnesium Sulfate
- AASHTO T 278: Surface Frictional Properties Using the British Pendulum
- AASHTO T 279: Accelerated Polishing of Aggregates Using the British
- AASHTO T 335: Determining the Percentage of Fracture in Coarse

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2025 Standard Specifications Latest Revision: February 15, 2024

- Use at least three articulating type pneumatic rollers for rolling operations. Use rollers weighing between 8 tons minimum and 12 tons
- maximum with a minimum width of 6 ft. Use rollers with pneumatic tires of equal size diameter and having treads satisfactory to the Engineer.
- Inflate tires so that the entire roller width area is compacted by the rear-axle tires and the front-axle tires. Inflate tires to 90 lb/in², or lower with permission from the
- Engineer. Maintain tire pressure within 5 lb/in².
- Sweeping Equipment
 - Use rotary brooms with nylon or steel bristles or pickup or vacuum brooms for pavement cleaning or brooming operations.
- Blotter Material Equipment
 - Apply blotter material using a truck mounted spinner broadcast spreader.

PART 3 EXECUTION

3.1 PREPARATION

- Clean the road surface of all dirt, sand, dust, and other objectionable material to the satisfaction of the Engineer.
- Protect structures including but not limited to guardrail, guideposts, concrete barriers, drains, and parapets.
- Protect manholes, valve boxes, drop inlets, and other service utility entrances before placing any chip seal coat.
- Stockpile blotter material with a quantity of at least 0.25 lb/yd2 for the Blotter material must be ready to be spread within 20 minutes of a
 - road section being chip sealed. Use blotter material, as needed to cover up oil if it bleeds through
- 3.2 LIMITATIONS

Complete all work between May 15, and August 31.

the new chip seal.

Do not place chip seal coat if surface moisture is present. Chip Seal Coat 02785 - Page 5 of 9

UDOT Materials Manual of Instruction (MMOI)

1.4 DEFINITIONS Not Used

1.5 SUBMITTALS

- Test reports for information that the cover material and emulsion meets requirements of this Section, Part 2.
- Equipment Calibration information including verifying asphalt application rates and chip application for information.
- Documentation verifying daily asphalt application rates and chip application for information.
- Vendor's bill of lading upon delivery for each emulsion used on the project

This bill of lading should certify if the emulsion was diluted or not according to this Section, Part 2.

PART 2 PRODUCTS

2.1 CATIONIC EMULSIONS

- CRS-2A according to Section 02745.
- B. CRS-2P according to Section 02745.
- C. LMCRS-2 according to Section 02745.

2.2 HIGH FLOAT EMULSIONS

- A. HFRS-2P according to Section 02745.
- B. HFMS-2 according to Section 02745.
- C. HFMS-2P according to Section 02745.

2.3 FLUSH COAT

 Use the emulsion as specified in Special Provision 02742S, diluted two parts concentrate to one part water by the manufacturer.

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- Chip seal coat application:
 - Place when the pavement temperature is between 70 and 136
 - Place when the air temperature is between 50 and 110 degrees F.
- Do not apply after 6:00 pm if the temperature is expected to be below 50 F during the night.
- Place when the forecasted temperature is not expected to be below 40 degrees F within 3 days after placement.
- Do not open to traffic the same day chip seal coat is placed on Interstate
 - Sweep chip seal to remove unbound aggregates prior to opening to
- Allow at least 48 hours after completing application of cover material before applying bituminous flush coat material.
 - Apply bituminous flush coat material when the air temperature in the shade is at least 50 degrees F and the pavement temperature is at least 70 degrees F.
 - Do not apply bituminous flush coat material during fog, rain, or other adverse conditions.

3.3 COVER MATERIAL STOCKPILE

- Construct individual 500 ton stockpiles for aggregates.
 - Construct on a clean base to minimize contamination. Construct to facilitate uniform dampening.
 - Avoid excess moisture.
 - Combining, altering, or moving accepted stockpiles may require retesting by the Engineer before use.
- order for the initial stockpiles to be sampled and tested for acceptance.
- area. Identify stockpiles that will be reworked.

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2025 Standard Specifications Latest Revision: February 15, 2024

2.4 COVER MATERIAL

- Meet the requirements of Table 1
- Use crusher processed virgin aggregate consisting of natural stone, gravel, or slag for standard chips.
- Use crusher-processed rotary-kiln lightweight expanded shale chips for lightweight chips.

Table 1

	The state of the s	Material Properties	
Test	Test Method	Standard Chip Seal Type I & II	Lightweight Chip Seal Type I &II
*Unit Weight	AASHTO T 19	100 lb/ft3, max	60 lb/ft3, max
One Fractured Face	AASHTO T 335	95% minimum	N/A
Two Fractured Faces	AASHTO T 335	90% minimum	N/A
*LA wear	AASHTO T 96	30% maximum	30% maximum
*Soundness	AASHTO T 104	10% maximum	10% maximum
Flakiness Index	Materials MOI 933	17 maximum	25 maximum
*Stripping	Materials MOI 945	10% maximum	10% maximum
*Polishing	AASHTO T 278, T 279	31 minimum	31 minimum

Meet gradation limits in Table 2. Refer to AASHTO T 27 and T 11.

Table 2

01	Percent Passing							
Sieve -	Standard	Aggregate	Lightweig	ht Aggregate				
3120	Type I	Type II	Type I	Type II				
1/2 in		100 - 98	100	100 - 90				
36 in	100	69 - 91	80 - 100	55 - 80				
No. 4	0 - 15	0 - 11	5 - 40	0 - 10				
No. 8		0-6	0 - 20	0-3				
No. 16		0.00-45	0 - 10					
No. 200	0 - 1	0 - 1.5		0-2				

2.5 BLOTTER MATERIAL

A. Refer to Section 02748.

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3.4 ASPHALT MATERIAL/COVER MATERIAL APPLICATION

- Apply asphalt material at a rate sufficient to obtain 50 percent chip embedment before the rolling operation and 70 percent chip embedment after rolling operation.
 - Adjust application rates throughout the project depending on existing conditions.
- Apply the asphalt emulsion at a minimum temperature of 145 degrees F.
- Do not apply asphalt material if material does not spray through the distributor in a uniform way and remain in place on the roadway.
- Place building paper adjacent to the transverse construction joint before starting each spraying operation. Maintain the control valve to act instantaneously both at start-up and cut- off.
- Locate longitudinal joints within 6 inches of the traffic lane line location. Construct meet lines with no skip or voids between adjacent
 - Do not place a double thickness of cover material.
- Calibrate the spreader at the beginning of each day and as often as necessary to comply with Table 3. Maintain a distance of less than 150 ft between the

distributor and the chip spreader.

Maintain the chip spreader speed so that chips do not bounce or roll during application.

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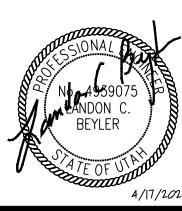
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3.8 LIMITATIONS

2.6 EQUIPMENT

50 degrees F.

Do not place Asphalt Mix on frozen base or subbase or during adverse

Place Asphalt Mix from April 15 through October 15, and when the air

outside these dates and temperature limits.

END OF SECTION

Asphalt Mix

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Tachometer, pressure gauges, accurate volume measuring devices

Insulated tanks capable of storing the binder at temperatures that

allow the binder to remain consistent with the appropriate viscosity

Use tanks equipped with baffles to prevent pressure surges

Use trucks equipped with devices to provide for accurate

Constant volume circulation pumps and heaters to maintain a

pressurized system so binder will be uniformly heated.

binder on the pavement surface.

all distributor operations from the cab to include:

of ± 0.03 gal/yd2.

individual spray bar sections.

aggregate plus or minus 1 lb/yd2.

Spinner broadcast type of aggregate spreader not allowed.

Chip Seal Coat

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the unit ground speed.

travel plus 1 ft/pass.

adjustable discharge gates.

length of the spray bar for each application.

control of the amount of bituminous material being applied.

Circulation pump must spray a constant volume for the entire

binder across the application width and triple lapping of the

Use a fully circulating spray bar with a positive shutoff valve.

automatic rate control adjustment to the unit ground speed.

Hydrostatic system capable of maintaining a tolerance

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Spray bar and nozzles designed to provide an appropriate fan width

Adjust the spray bar height to provide uniform distribution of

Computerized rate control system allowing the operator to control

Spray bar height and width adjustment and shut off of

aggregate across the full width of the binder and adjust output to

Spreading hopper with a minimum capacity to cover a full lane of

Pressure regulation of the material application and

Use a self-propelled aggregate (chip) spreader specifically designed and

Use gates adjustable to drop the correct amount of

Variable width spreader with hydraulic control extension and

manufactured for chip seal operations, equipped with the following:

Computerized controls that will apply a uniform, even layer of

to provide uniform transverse distribution without corrugation or

resulting from the asphalt sloshing in the tank when starting

or a calibrated tank, and a thermometer for measuring

Use distributor trucks with the following:

for proper application rates.

temperatures of the tank contents.

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climatic conditions such as precipitation or when roadway surface is icy or

the Engineer for all equipment and hand tools used to mix, haul, and place

temperature in the shade and the roadway surface temperature are above

The Department determines if it is feasible to place Asphalt Mix

Obtain authorization from the Engineer before paving outside these

Use a release agent that does not dissolve asphalt and is satisfactory to

2025 Standard Specifications Latest Revision: February 15, 2024 Notify the Engineer at least seven calendar days before placement in

Obtain the Engineer's acceptance of a stockpile before use.

Rework or remove material not meeting specifications from the stockpile

CIVIL IMPROVEMENT PLANS





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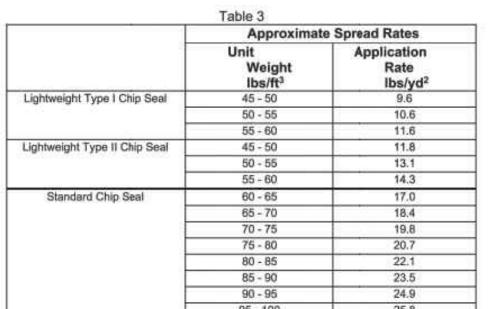
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3.5 SURFACE ROLLING

- A. Use at least three pneumatic-tire rollers in a longitudinal direction to roll surface after the cover material has been spread.
- Set the roller speed to prevent bouncing or skidding.
- Do not exceed 5 mph.
- E. Synchronize the speed of the distributor and chip spreader with that of the
 - immediately behind the chip spreader.
 - Begin secondary rolling, consisting of second and third coverage,

Chip Seal Coat 02785 - Page 8 of 9

2025 Standard Specifications Latest Revision: February 15, 2024 Sweep excess cover material off the roadway after the emulsion has set. Remove excess cover material to the satisfaction of the Engineer

- before opening the roadway to traffic. Keep downward pressure of broom to a minimum.
- Use water as requested by the Engineer if excessive dust is generated during sweeping operations.
- Use pickup or vacuum sweepers in urban areas where aggregate accumulates in gutters or where removal is required from the edge of the shoulder.
- Do not dislodge embedded aggregate when brooming chip sealed
- G. Repair all damage to the seal coat before opening the roadway to traffic.

3.6 BITUMINOUS FLUSH COAT APPLICATION

- Clean the surface of all dirt, sand, dust, loose chips, and other objectionable material to the satisfaction of the Engineer before applying
- B. Apply the bituminous flush coat at a rate of 0.11, ± 0.01 gal/yd². Keep traffic off the flushed surface until the bituminous material has set sufficiently to prevent tracking or pick-up.

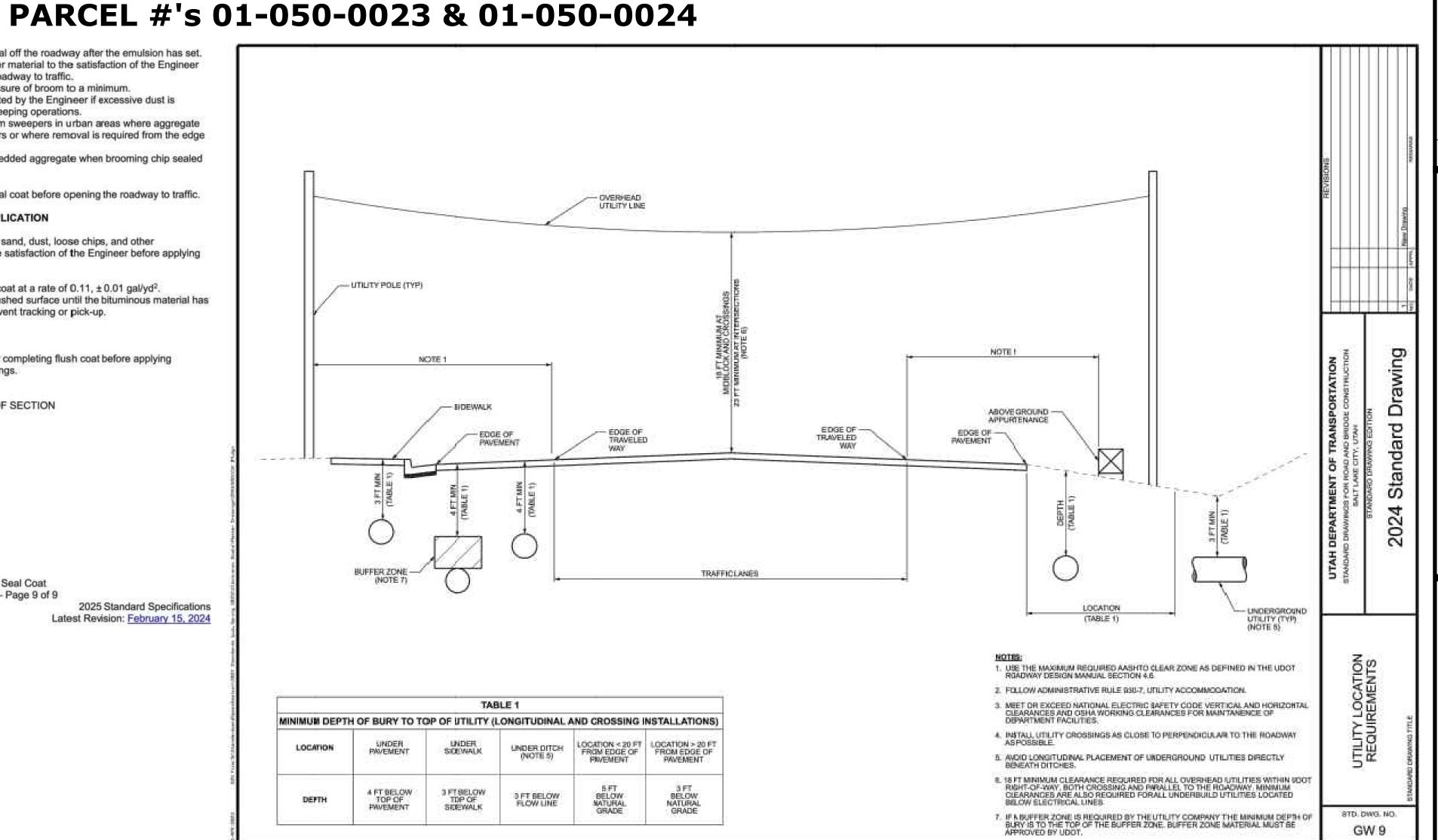
3.7 PAVEMENT MARKING PAINT

 A. Allow at least 24 hours after completing flush coat before applying permanent pavement markings.

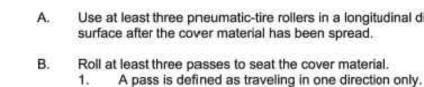
END OF SECTION

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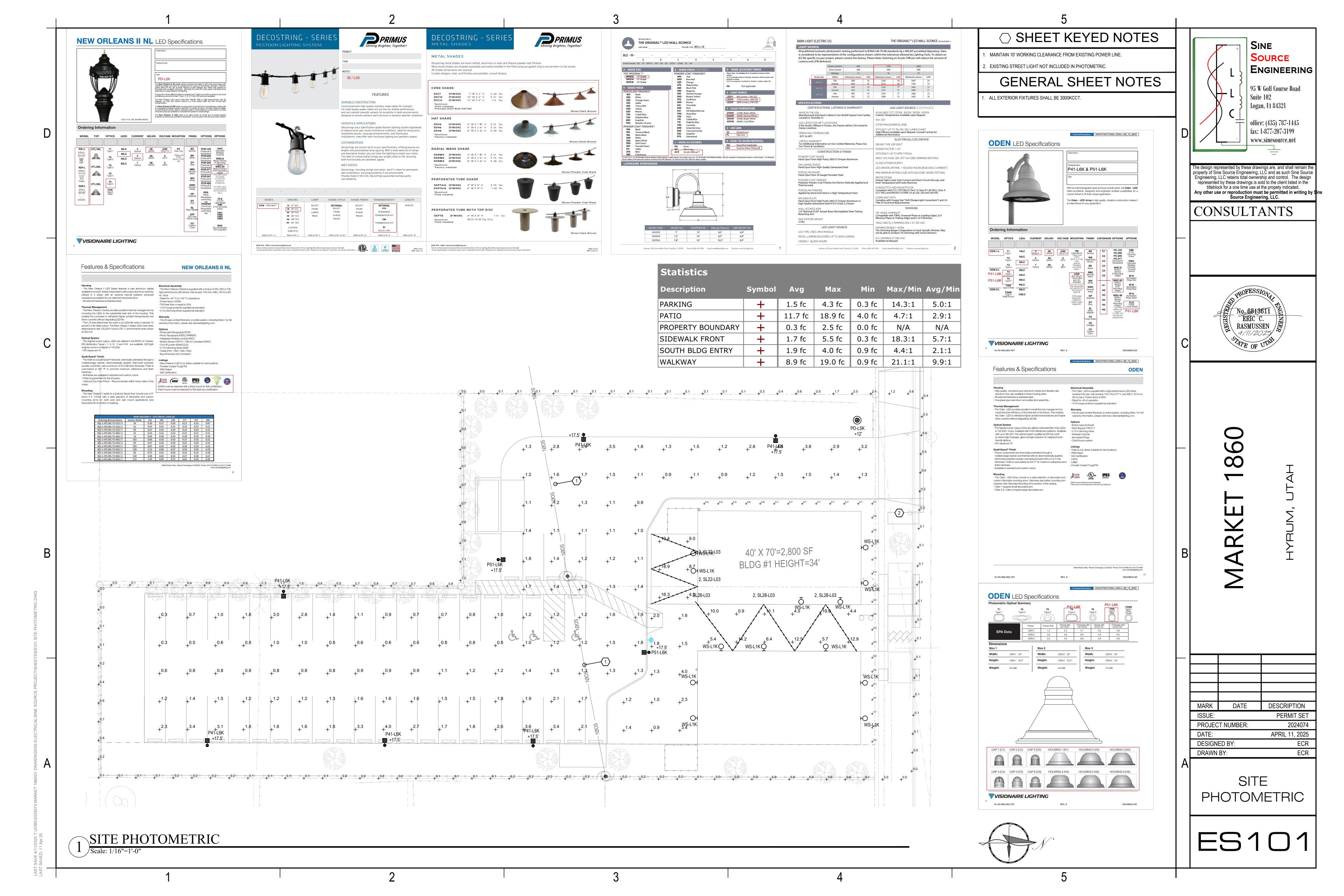


Control bleeding with blotter material and as determined by the Engineer.

Reduce roller speeds during directional changes to prevent surface

Begin initial rolling, consisting of one complete coverage,

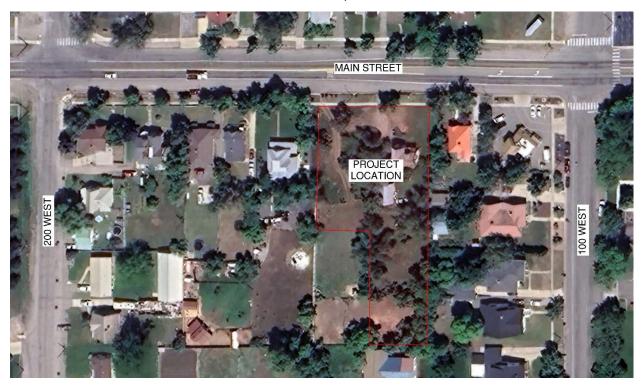
immediately after completing initial rolling. Synchronize all operations to keep rolling operations within 2,500 feet of the ongoing chip seal application.



PRELIMINARY DRAINAGE REPORT FOR:

HYRUM MARKET 1860

HYRUM CITY, UTAH





Preliminary Drainage Plan

1. GENERAL DESCRIPTION

The Hyrum Market 1860 is a 0.965 acre property located in Hyrum, Cache County, Utah and is situated west at 127 West Main Street. This property will be developed into 2 commercial buildings and consist of parking area, buildings, and landscaping.

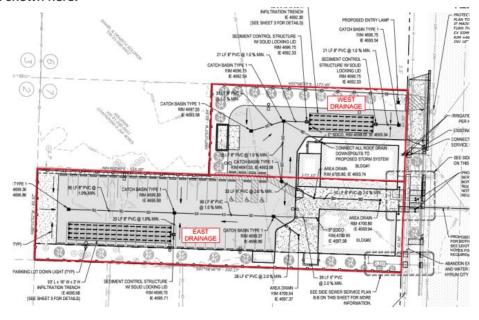
The current parcels consists of houses, detached garage/barn, small sheds, and an unfinished concrete structure. The remaining area appears to be mainly used as pastureland. All building on the site will be demolished as part of this development.

2. PROPOSED ON-SITE DRAINAGE

Surface water from precipitation generally drains towards the northwest corner of the property. Due to roadways that border to the north, it is anticipated that upstream flows will be conveyed past the property through the roadways and not impact the project.

Based on a zero release rate from the project, an analysis was performed for this site. The site is divided into two drainage areas, each with their own retention basin, designated as the west drainage and east drainage. The location is based on topography and the layout of the proposed roadways and buildings. The retention facility will capture the 100-year 24-hour storm event with one foot of freeboard.

Storm drain piping will be installed within the Project to capture runoff generated from buildings, roadways, and landscape areas. Water will generally sheet flow to roadways, flow down gutters to inlets and be conveyed in pipes to the retention basin. The proposed development for this site consists of 2 commercial buildings and consist of parking area, buildings, and landscaping. Runoff from this project will be collected within the project by inlet boxes and conveyed through underground piping to a retention basin. The retention basin will be located and described above and as shown on the Drainage Basin Map shown here.



3. CALCULATIONS

Runoff calculations are provided below. The 100 year, 24 hour storm event was used for the sizing of the retention basin.

3.1 DESIGN STORM CRITERIA

Storm intensities used in this study area are taken from the NOAA Atlas 14, Volume 1, Version 5 for Latitude 41.6339, Longitude -111.8589, Elevation 4698. Storm intensity and depth tables are attached for reference.

3.2 STORM DRAINAGE GENERATION

The storm drain analysis is completed using the rational method, using the rational formula:

Q = C * i * A

Where:

Q = Design flow in cubic feet per second (cfs)

C = Runoff coefficient. Represents percent of

precipitation that will contribute as runoff.

i = Rainfall intensity in inches per hour (in/hr)

A = Drainage area in acres (Ac)

A unique C value is assigned to each type of land cover such as impervious and pervious surfaces. Examples of impervious surfaces are the asphalt street, concrete driveways and sidewalks. Examples of pervious surfaces are lawns, gardens and open areas. The C value for some land cover types is also affected by the drainage configuration of that cover. For example, house roofs are essentially impervious surfaces, but because of the nature of roof rain gutter systems and because the down spouts from the rain gutters typically drain onto open lawns, the percentage of water contributed to the Subdivision storm drainage system is usually much lower than other impervious surfaces.

For this analysis, the drainage areas are divided into three surface cover types, namely hardscape, buildings, and landscaped areas. The streets include park strips and sidewalks.

The assigned C value for each surface cover is as follows:

Hardscape C = 0.90

Buildings C = 0.85

Landscaping C = 0.15

For each drainage sub-basin the amount of impermeable and permeable surface area is measured to calculate a composite C value. The composite C value is calculated as follows.

Where;

Ci = Runoff coefficient for each type of land cover.

Ai = Drainage area of each type of land cover.

At = Total drainage area of all types of land cover in the sub-basin.

3.3 DRAINAGE BASIN DELINEATION

For the purposes of this preliminary study, areas were calculated for each of the surface cover types in order to calculate the volume of runoff the retention basin should capture. For final design, delineation will be performed so piping can be sized appropriately. The attached Table A shows the volume calculation.

TABLE 1 WEST RETENTION BASIN

Rain Fall Intenstiy from No	OAA Website for Hyru	um, Utah, 41.6339° N, 1	11.8589° W								
Infiltration Design Rate [ft/sec]		80th Percentile Storm Land Disturbance [ac] Impervious Area [ac] Imperviousness R_v	Disturbance [ac] vious Area [ac] viousness			Drainage Characteristics Contributing Area [ac] C (Weighted Average) C*A		0.3 0.7 0.2			
				Minimum Retention Volume [cf] LID Depth [ft]		417 0.92		Calculated Orifice Diameter [in] Rounded Orifice Diameter [in]		0.0 0.0	
A	В	С	D	E	F	G	Н	I	J	K (C-F-I)	
Time [min]	100 yr intensity [in/hr]	Storm Volume [cf]	Estimated Max Stage [ft]	Infiltration Area [sf]	Infiltration [cf]	Outlet Time [min]	Outlet Rate [cfs]	Outlet Volume During the Storm [cf]	Outlet Volume After the Storm [cf]	Accumulated Storag	
5	5.78	417	0.89	669.28	16.73	-	-	-	-	400.5	
10	4.40	635	1.18	801.33	40.07	-	-	-	-	595.4	
15	3.64	788	1.35	878.75	65.91	-	-	-	-	721.9	
30	2.44	1,056	1.60	992.59	148.89	-	-	-	-	907.3	
60	1.52	1,316	1.60	992.59	297.78	-	-	-	-	1,018.2	
120	0.86	1,489	1.60	992.59	595.55	-	-	-	-	893.6	
180 360	0.61 0.36	1,576 1,853	1.60 1.60	992.59 992.59	893.33 1,786.66	-	-	-	-	682.4 66.1	
720	0.36	2,286	1.60	992.59	3,573.32	-	-	-	-	00.1	
1,440	0.13	2,788	1.60	992.59	7,146.64	-	-	- -	-	-	
Basin Characteristics Volume [cf]	1,033.73		mergency Overflow ate [cfs]	0.37	8,000				1		
Depth [ft]	1,033.73		eight [ft]	2.00	7,000						
Top Area [sf]	1,028.16		ength [ft]	0.04							
F []	-,		8 []		6,000						
					5,000						
Orifice Diameter [in]	-				4,000						
Height of Outlet	0' 11"				3,000						
					2,000						
Volume Provi	dad [ft³]	1,034	1029/	of required	1,000						
volume 110vi	מבנו []ו]	1,054	10270	oj requireu	- 📮		400 600	800 1,000	0 1,200	1,400 1,600	
					(1,000)	200	400 600		0 1,200	1,400 1,600	
								Time [min]			
						—■—Storm Volun	ne [cf]	─ In	nfiltration [cf]		
						Outlet Volum	ne During the Storm	[cf] — A	ccumulated Storage	[cf]	

TABLE 1EAST RETENTION BASIN

Rain Fall Intenstiy from No Infiltration Design Rate [ft/sec]	IOAA Website for Hyrum, Utah, 41.6339° N, 111.8589° W 80th Percentile Storm [in] 8.33E-05 Land Disturbance [ac] Impervious Area [ac] Imperviousness R v					0.50 0.63 0.40 63.49% 0.557		Drainage Characteristics Contributing Area [ac] C (Weighted Average) C*A		0.63 0.62 0.39	
				Minimum Retention Volume [cf] LID Depth [ft]		637 0.92		Calculated Orifice Diameter [in] Rounded Orifice Diameter [in]		0.00 0.00	
A	В	С	D	Е	F	G	Н	I	J	K (C-F-I)	
Time [min]	100 yr intensity [in/hr]	Storm Volume [cf]	Estimated Max Stage [ft]	Infiltration Area [sf]	Infiltration [cf]	Outlet Time [min]	Outlet Rate [cfs]	Outlet Volume During the Storm [cf]	Outlet Volume After the Storm [cf]	Accumulated Storage [cf]	
5	5.78	679	0.88	1,095.18	27.38	-	-	-	-	651.95	
10	4.40	1,034	1.18	1,317.17	65.86	-	-	-	-	968.64	
15	3.64	1,283	1.35	1,442.96	108.22	-	-	-	-	1,174.33	
30	2.44	1,719	1.60	1,627.96	244.19	-	-	-	-	1,475.27	
60	1.52	2,142	1.60	1,627.96	488.39	-	-	-	-	1,653.90	
120	0.86	2,424	1.60	1,627.96	976.77	-	-	-	-	1,447.39	
180 360	0.61 0.36	2,565 3,016	1.60 1.60	1,627.96 1,627.96	1,465.16 2,930.32	-	-	-	-	1,099.95 85.79	
720	0.36	3,721	1.60	1,627.96	2,930.32 5,860.64	-	-	-	-	85.79	
1,440	0.13	4,538	1.60	1,627.96	11,721.29	- -	-	-	-	-	
D : Ol						Hydrograph					
Basin Characteristics Volume [cf]	1,667.33		mergency Overflow ate [cfs]	0.60	14,000						
Depth [ft]	1.60		leight [ft]	2.00	12,000						
Top Area [sf]	1,640.16		ength [ft]	0.06	12,000						
[]	-,		8 []		10,000						
					8,000						
Orifice Diameter [in]	-										
Height of Outlet	0' 11"				6,000						
					4,000						
					2,000						
Volume Provi	ided [ft³]	1,667	1019/	of required	2,000						
roume 1 rove	men [ji]	1,007	101 /0	oj requireu		200 4	00 600	800 1,000	1,200	1,400 1,600	
					(2,000)	270			, 1,200	1,000	
								Time [min]			
						Storm Volume	[cf]	—— In	filtration [cf]		
						Outlet Volume	During the Storm	[cf] ——A	ccumulated Storage	[cf]	



NOAA Atlas 14, Volume 1, Version 5 Location name: Hyrum, Utah, USA* Latitude: 41.6339°, Longitude: -111.8589° Elevation: 4698 ft**

* source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sarah Dietz, Sarah Heim, Lillian Hiner, Kazungu Maitaria, Deborah Martin, Sandra Pavlovic, Ishani Roy, Carl Trypaluk, Dale Unruh, Fenglin Yan, Michael Yekta, Tan Zhao, Geoffrey Bonnin, Daniel Brewer, Li-Chuan Chen, Tye Parzybok, John Yarchoan

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.114 (0.100-0.130)	0.144 (0.128-0.166)	0.199 (0.175-0.228)	0.248 (0.216-0.284)	0.327 (0.279-0.376)	0.398 (0.330-0.460)	0.482 (0.390-0.563)	0.580 (0.453-0.687)	0.737 (0.549-0.893)	0.877 (0.627-1.08)
10-min	0.173 (0.152-0.197)	0.219 (0.195-0.252)	0.302 (0.265-0.347)	0.377 (0.328-0.432)	0.498 (0.424-0.573)	0.605 (0.503-0.700)	0.734 (0.593-0.857)	0.883 (0.689-1.05)	1.12 (0.835-1.36)	1.34 (0.954-1.65)
15-min	0.215 (0.188-0.244)	0.272 (0.241-0.312)	0.375 (0.329-0.430)	0.468 (0.406-0.535)	0.616 (0.526-0.709)	0.750 (0.624-0.868)	0.910 (0.735-1.06)	1.09 (0.855-1.30)	1.39 (1.04-1.68)	1.66 (1.18-2.05)
30-min	0.289 (0.254-0.329)	0.366 (0.324-0.420)	0.505 (0.443-0.578)	0.630 (0.547-0.721)	0.830 (0.707-0.955)	1.01 (0.840-1.17)	1.22 (0.990-1.43)	1.47 (1.15-1.75)	1.87 (1.39-2.27)	2.23 (1.59-2.76)
60-min	0.357 (0.314-0.407)	0.453 (0.401-0.520)	0.624 (0.548-0.716)	0.779 (0.677-0.892)	1.03 (0.876-1.18)	1.25 (1.04-1.45)	1.52 (1.22-1.77)	1.82 (1.42-2.16)	2.32 (1.73-2.81)	2.76 (1.97-3.41)
2-hr	0.463 (0.415-0.520)	0.581 (0.519-0.651)	0.762 (0.677-0.853)	0.929 (0.817-1.04)	1.20 (1.03-1.35)	1.44 (1.21-1.63)	1.72 (1.41-1.97)	2.05 (1.63-2.39)	2.57 (1.94-3.08)	3.04 (2.21-3.72)
3-hr	0.545 (0.496-0.610)	0.680 (0.618-0.763)	0.864 (0.782-0.965)	1.04 (0.929-1.16)	1.30 (1.15-1.46)	1.54 (1.33-1.74)	1.82 (1.53-2.08)	2.14 (1.75-2.49)	2.66 (2.08-3.17)	3.11 (2.35-3.79)
6-hr	0.746 (0.683-0.822)	0.921 (0.841-1.02)	1.14 (1.04-1.26)	1.34 (1.21-1.48)	1.63 (1.45-1.81)	1.87 (1.64-2.09)	2.14 (1.85-2.41)	2.45 (2.07-2.80)	2.97 (2.44-3.47)	3.44 (2.74-4.08)
12-hr	0.982 (0.900-1.08)	1.21 (1.11-1.34)	1.48 (1.35-1.64)	1.72 (1.56-1.90)	2.07 (1.85-2.29)	2.34 (2.08-2.61)	2.64 (2.30-2.98)	2.96 (2.53-3.37)	3.45 (2.87-3.99)	3.85 (3.12-4.52)
24-hr	1.27 (1.15-1.40)	1.56 (1.42-1.72)	1.90 (1.72-2.10)	2.18 (1.97-2.40)	2.58 (2.32-2.84)	2.89 (2.60-3.18)	3.22 (2.87-3.55)	3.56 (3.16-3.92)	4.03 (3.53-4.45)	4.39 (3.83-4.87)
2-day	1.50 (1.36-1.68)	1.85 (1.67-2.06)	2.24 (2.02-2.50)	2.56 (2.31-2.86)	3.02 (2.70-3.36)	3.38 (3.02-3.76)	3.76 (3.33-4.18)	4.15 (3.66-4.62)	4.68 (4.09-5.23)	5.11 (4.43-5.72)
3-day	1.66 (1.51-1.86)	2.04 (1.85-2.28)	2.48 (2.24-2.76)	2.84 (2.56-3.17)	3.35 (3.00-3.73)	3.75 (3.35-4.18)	4.17 (3.70-4.64)	4.61 (4.06-5.13)	5.21 (4.55-5.81)	5.68 (4.92-6.35)
4-day	1.82 (1.65-2.03)	2.24 (2.03-2.50)	2.72 (2.46-3.03)	3.12 (2.82-3.48)	3.68 (3.31-4.10)	4.12 (3.69-4.59)	4.59 (4.07-5.10)	5.07 (4.46-5.64)	5.73 (5.00-6.38)	6.25 (5.40-6.97)
7-day	2.20 (1.99-2.47)	2.72 (2.46-3.05)	3.30 (2.99-3.71)	3.79 (3.42-4.25)	4.46 (4.01-5.00)	4.99 (4.46-5.59)	5.54 (4.93-6.21)	6.11 (5.40-6.85)	6.88 (6.02-7.74)	7.48 (6.50-8.44)
10-day	2.49 (2.26-2.77)	3.07 (2.79-3.42)	3.73 (3.38-4.15)	4.27 (3.87-4.75)	5.00 (4.51-5.56)	5.56 (5.00-6.18)	6.15 (5.50-6.84)	6.74 (5.99-7.50)	7.53 (6.64-8.41)	8.14 (7.12-9.11)
20-day	3.22 (2.96-3.50)	3.97 (3.65-4.32)	4.77 (4.38-5.18)	5.39 (4.95-5.86)	6.22 (5.69-6.75)	6.82 (6.24-7.41)	7.43 (6.77-8.09)	8.02 (7.27-8.74)	8.78 (7.92-9.60)	9.35 (8.39-10.2)
30-day	3.89 (3.60-4.24)	4.79 (4.43-5.21)	5.73 (5.29-6.23)	6.47 (5.97-7.04)	7.46 (6.87-8.11)	8.20 (7.53-8.92)	8.94 (8.19-9.73)	9.66 (8.81-10.5)	10.6 (9.61-11.6)	11.3 (10.2-12.4)
45-day	4.84 (4.48-5.23)	5.96 (5.50-6.43)	7.06 (6.52-7.61)	7.90 (7.29-8.52)	8.99 (8.28-9.69)	9.78 (8.98-10.5)	10.6 (9.67-11.4)	11.3 (10.3-12.2)	12.2 (11.1-13.2)	12.9 (11.7-14.0)
60-day	5.73 (5.30-6.19)	7.03 (6.50-7.60)	8.26 (7.64-8.93)	9.19 (8.49-9.94)	10.4 (9.56-11.2)	11.2 (10.3-12.1)	12.0 (11.0-13.0)	12.7 (11.7-13.8)	13.6 (12.5-14.9)	14.3 (13.0-15.6)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

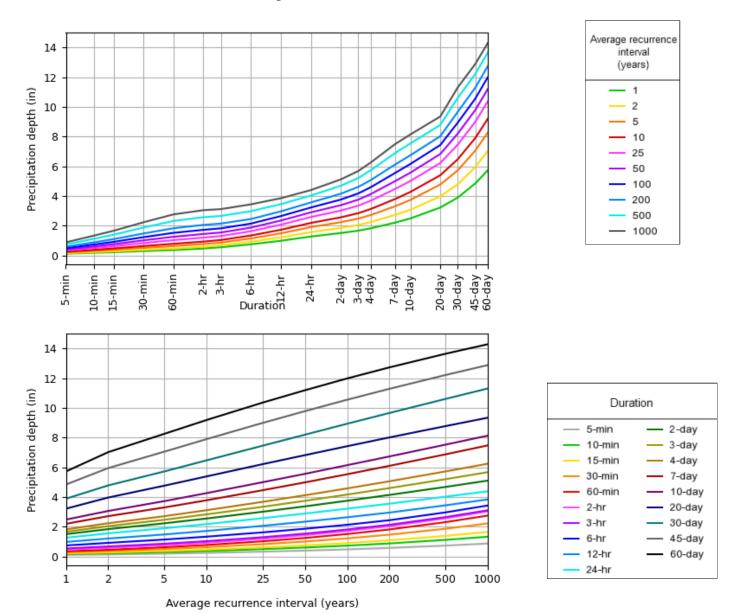
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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PF graphical

PDS-based depth-duration-frequency (DDF) curves Latitude: 41.6339°, Longitude: -111.8589°



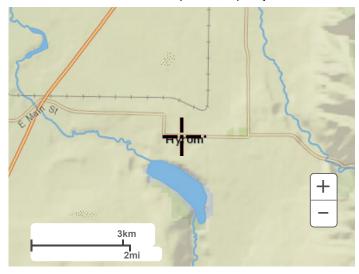
NOAA Atlas 14, Volume 1, Version 5

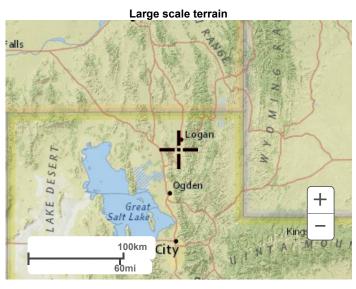
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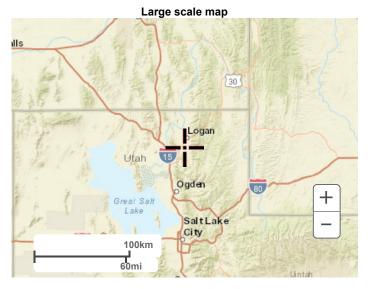
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Maps & aerials

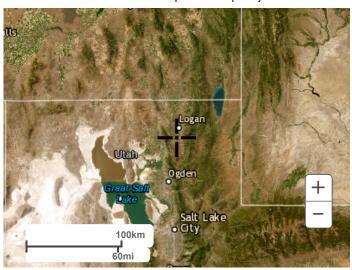
Small scale terrain







Large scale aerial



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