

TRAFFIC IMPACT ANALYSIS
RESIDENTIAL DEVELOPMENT
HOWEY-IN-THE-HILLS, FLORIDA



Prepared for:
Blue Sky Capital Group, LLC
103 Commerce Street, Suite 160
Lake Mary, Florida 32746

Prepared by:
Traffic Planning and Design, Inc.
535 Versailles Drive
Maitland, Florida 32751
407-628-9955

May 2022
Revised December 2022

TPD № 5659

PROFESSIONAL ENGINEERING CERTIFICATION

I hereby certify that I am a Professional Engineer properly registered in the State of Florida practicing with Traffic Planning & Design, Inc., a corporation authorized to operate as an engineering business, EB-3702, by the State of Florida Department of Professional Regulation, Board of Professional Engineers, and that I have prepared or approved the evaluations, findings, opinions, conclusions, or technical advice attached hereto for:

PROJECT: Residential Development
LOCATION: Howey-in-the-Hills, Lake County
CLIENT: Blue Sky Capital Group, LLC

I hereby acknowledge that the procedures and references used to develop the results contained in these computations are standard to the professional practice of Transportation Engineering as applied through professional judgment and experience.

NAME: Turgut Dervish
P.E. No.: 20400
DATE: December 21st, 2020
SIGNATURE:



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INTRODUCTION

This traffic analysis was performed to assess the impact of a proposed residential development in Howey-in-the-Hills, Lake County. The proposed development will consist of 180 single family units. The site, as depicted in **Figure 1**, is located to the north side of Number 2 Road approximately two-miles west of SR 19. Access to the site will be via a full access driveway on Number 2 Road. It is anticipated that the development will be completed in 2028. **Figure 2** depicts the site plan and the proposed access connection.

A Tier 2 Traffic Impact Analysis (TIA) is required as per the Lake Sumter MPO Traffic Impact Study Methodology and Guidelines which require a minimum of one mile impact area from the main access point plus all roadways which the project consumes 5% or more of the roadway capacity. This area includes segments of CR 48, SR 19 and Number 2 Road which provide external access to the site.

The analysis was conducted as per the study methodology submitted to the City and County. The study methodology and related correspondence are included in **Appendix A**. Reference data used in the analysis were obtained from the Florida Department of Transportation (FDOT) Annual Average Daily Traffic Report, Lake County CMP Database spreadsheets and trip generation data from the Institute of Transportation Engineers (ITE). Additionally, A.M./P.M. peak hour traffic data were collected at the intersections by TPD personnel for use in the analysis.

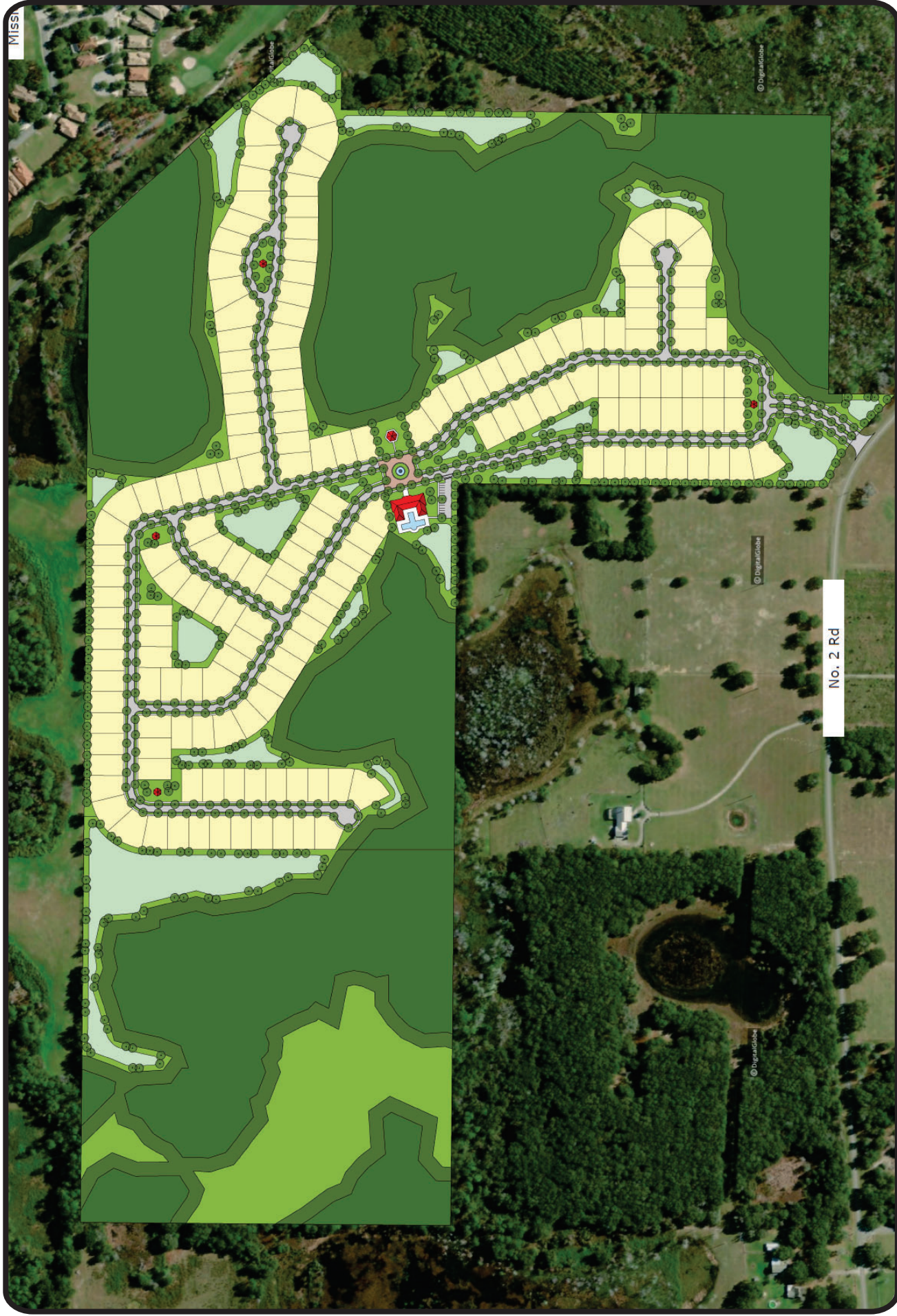




Site Location

Howey in the Hills
Project No 5659
Figure 1





EXISTING CONDITIONS ANALYSIS

Existing traffic conditions were analyzed using peak direction P.M. peak hour volumes for the study roadways and A.M./P.M. peak hour traffic volumes for the study intersections. The roadway analysis consisted of a generalized capacity analysis with the existing traffic volumes and the available capacity. The intersection analysis was conducted as per the procedures of the Highway Capacity Manual.

Roadway Segment Analysis

The roadway segments were analyzed by comparing their existing P.M. peak hour directional volumes with the corresponding peak hour directional capacities at the adopted Level of Service (LOS) standard. The existing P.M. peak hour volumes, LOS standard, and peak hour direction capacities were obtained from the Lake County's 2022 CMP Database. A summary of the existing roadway capacity analysis is presented in **Table 1**. This table shows that the segments in the vicinity of the site are operating at satisfactory Levels of Service. The Lake County's 2022 CMP Database is included in **Appendix B**.

Intersection Analysis

A capacity analysis was conducted for the study intersections using the *Highway Capacity Software (HCS)* in accordance with the procedures of the *Highway Capacity Manual (HCM 6E)*. The capacity analysis was performed using the existing intersection geometry, traffic volumes during the A.M./P.M. peak hours and traffic controls. Existing turning movement counts obtained by TPD in 2022 were adjusted as per FDOT seasonal factors for Lake County. The adjusted intersection volumes are displayed in **Figures 3a** and **3b**. The intersection counts, FDOT seasonal factors and signal timings are included in **Appendix C**.

The intersection capacity analysis is summarized in **Table 2**. This analysis indicates that the study intersections are currently operating at acceptable Levels of Service. Detailed *HCS* analysis worksheets are included in **Appendix D**.



Table 1
Existing Roadway Capacity Analysis

Roadway Segment	Segment ID	No. of Lanes	Adopted		P.M. Peak Hour*		v/c Ratio	LOS
			LOS	Capacity	Peak Direction	Volume		
CR 48								
US 27 to Lime Ave	1240	2	D	1,080	EB	469	0.43	B
Lime Ave to SR 19	1250	2	D	1,080	EB	409	0.38	B
SR 19								
Lane Park Rd to CR 48	3040	2	D	920	SB	652	0.71	C
CR 48 to Central Ave	3050	2	D	700	SB	415	0.59	C
Central Ave to CR 455	3060	2	D	1,200	SB	415	0.35	B
Number 2 Road**								
CR 48 to Bloomfield Ave	---	2	D	675***	WB	26	0.04	C
Bloomfield Ave to SR 19	---	2	D	675***	EB	52	0.08	C

* Based on FDOT and Lake County 2020 traffic counts

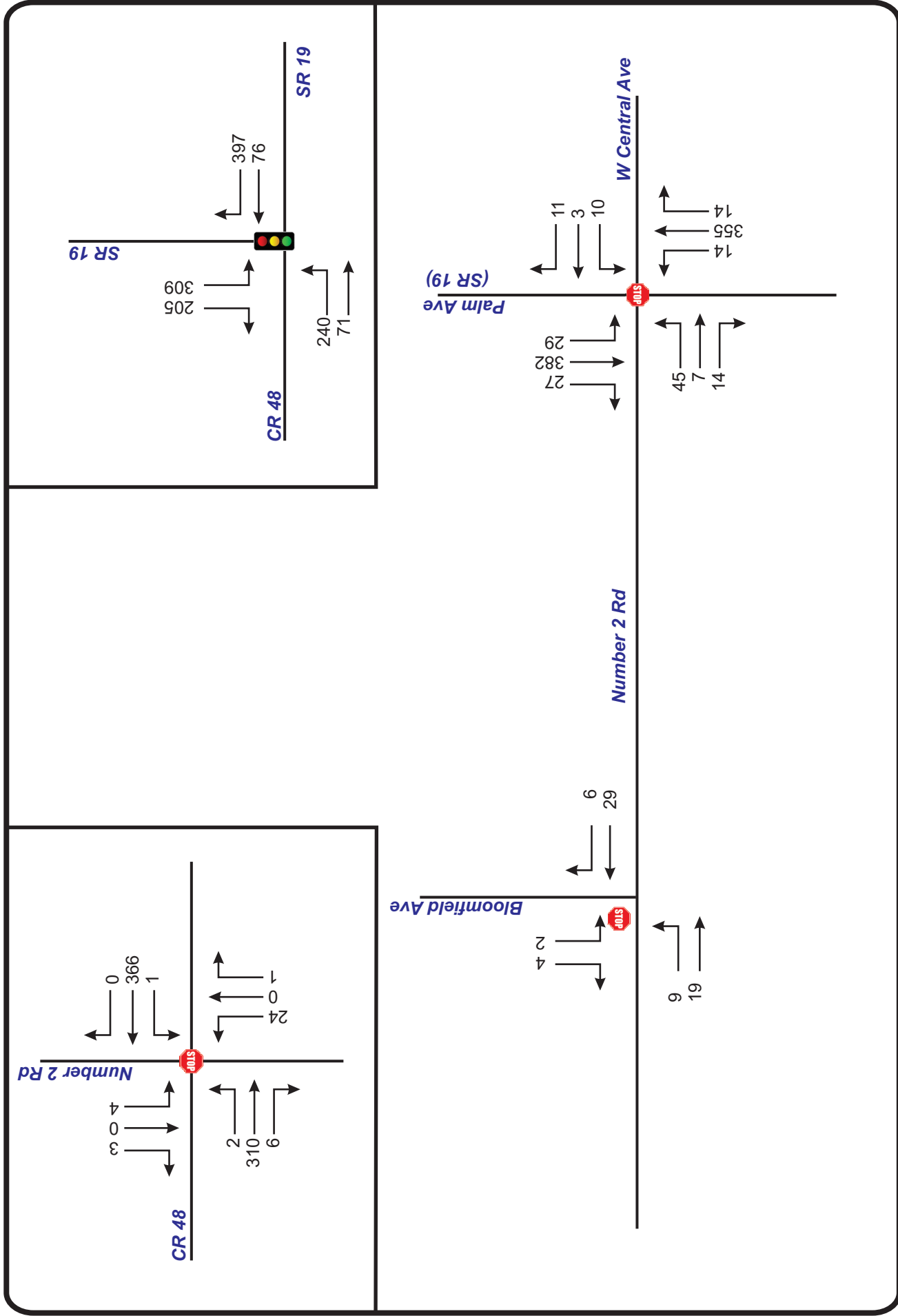
** Not included in the County's Database

***Obtained from FDOT Quality/LOS Handbook

Table 2
Existing P.M. Peak Hour Intersection Capacity Analysis

Intersection	Control	Period	EB		WB		NB		SB		Overall	
			Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Number 2 Rd & CR 48	Stop	A.M.	0.1	A	0.0	A	17.5	C	14.2	B	--	--
		P.M.	0.1	A	0.1	A	19.7	C	16.2	C	--	--
Number 2 Rd & Bloomfield Ave	Stop	A.M.	2.4	A	--	--	--	--	8.6	A	--	--
		P.M.	1.1	A	--	--	--	--	8.5	A	--	--
Palm Ave & Central Ave	Stop	A.M.	25.9	D	18.0	C	0.5	A	0.9	A	--	--
		P.M.	23.2	C	18.9	C	0.7	A	0.4	A	--	--
CR 48 & SR 19	Signal	A.M.	10.4	B	2.7	A	0.0	A	27.7	C	14.4	B
		P.M.	12.2	B	3.4	A	0.0	A	27.1	C	16.5	B

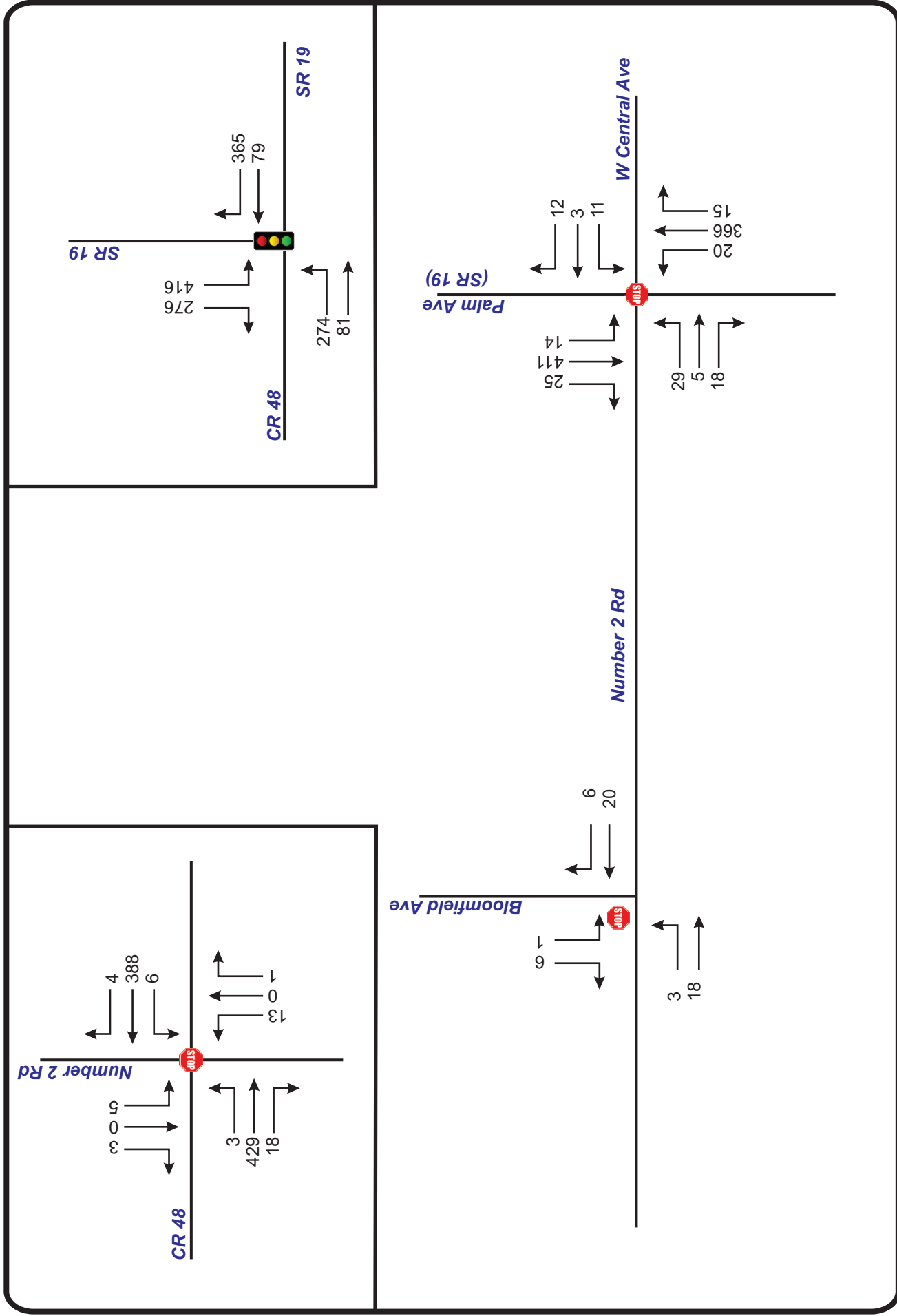




Existing AM Peak Hour Volumes

Howey in the Hills
Project No 5659
Figure 3a





PROPOSED DEVELOPMENT AND TRIP GENERATION

The proposed development consists of 180 single family homes anticipated to be completed in 2028. Access to the site will be via a full access driveway on Number 2 Road. To determine the impact of this development in the area, an analysis of its trip generation characteristics was conducted.

Trip Generation

The trip generation of the proposed development was calculated with the use of rates obtained from the 11th Edition of the Institute of Transportation Engineers (ITE) *Trip Generation Manual*. A summary of the trip generation calculation is shown in **Table 3**. The ITE trip generation sheets are included in the study methodology in Appendix A.

Table 3
Trip Generation Calculation Summary

ITE Code	Land Use	Size	Daily		A.M. Peak Hour				P.M. Peak Hour			
			Rate*	Trips	Rate*	Enter	Exit	Total	Rate*	Enter	Exit	Total
210	Single Family Residential	180 DU**	9.63	1,733	0.71	32	95	127	0.96	109	64	173
Total Trips				1,733	---	32	95	127	---	109	64	173

* Equation used, $R^2 > 0.75$

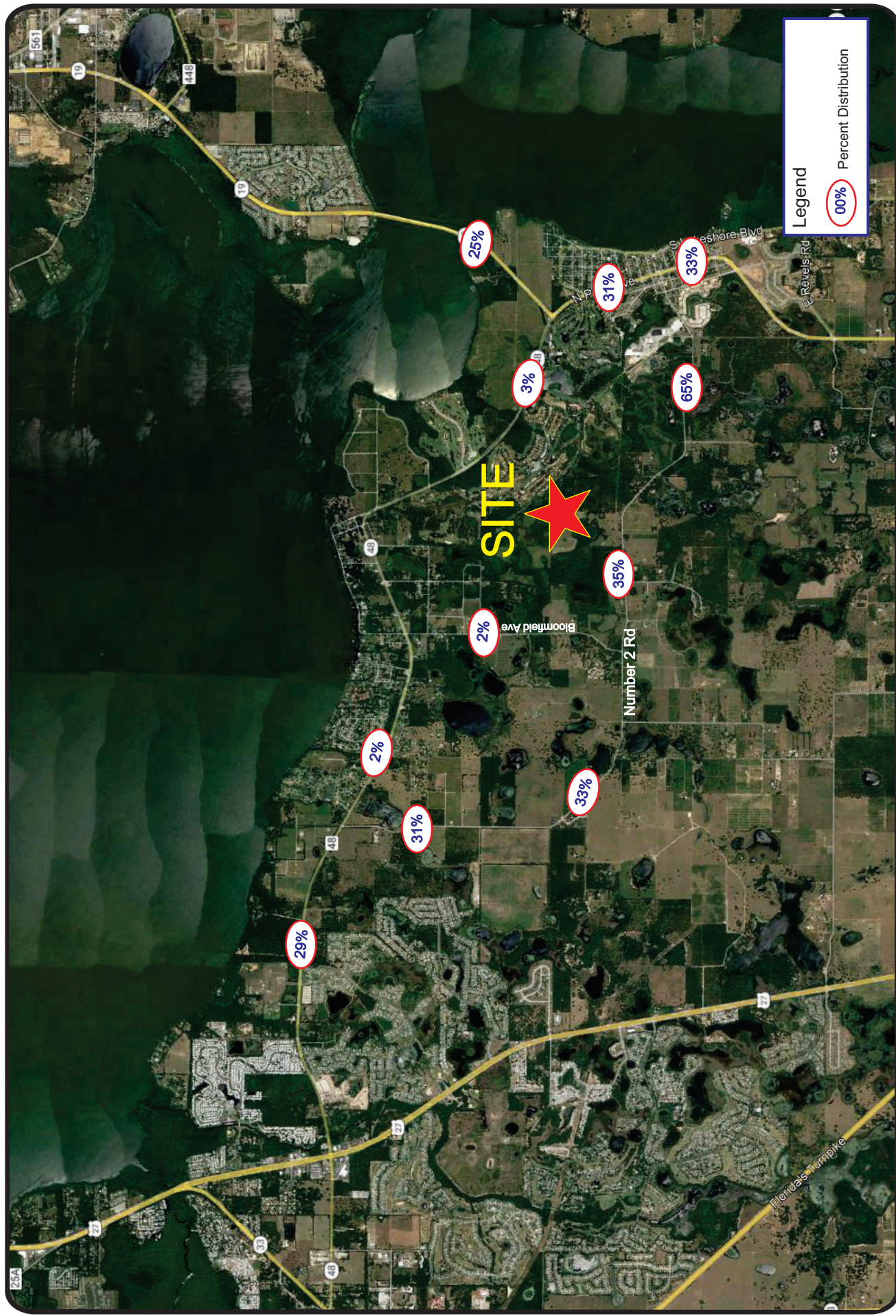
** DU = Dwelling Unit

The development is estimated to generate a total of 1,733 daily trips, of which 127 will occur during the A.M. peak hour and 173 will occur during the P.M. peak hour upon full development.

Trip Distribution and Assignment

A distribution pattern was estimated using the currently adopted CFRPM model with a Select Zone Analysis (SZA). The model trip distribution plot is included in the study methodology. The trip distribution thus determined is shown in **Figure 4**. This distribution pattern was used to distribute and assign the project trips to the study roadways and intersections.





Howey in the Hills
Project № 5659
Figure 4



Trip Distribution

PROJECTED TRAFFIC CONDITIONS

Projected traffic conditions were analyzed for the study roadway segments for the P.M. peak hour directional volumes and intersections for the A.M. and P.M. peak hours. Projected traffic volumes used in the analysis consisted of background traffic combined with site generated traffic. Background traffic volumes were determined by combining the existing traffic volumes with the approved trips provided by the City from the following developments:

- Whispering Hills
- The Reserve
- Simpson Howey-in-the-Hills
- Talichet PUD
- Mission Rise

The trip information provided by the City for the approved developments is included in **Appendix E**.

Roadway Segment Analysis

The projected roadway segment analysis was performed by comparing the projected traffic volume of each segment with the capacity of the segment at the adopted LOS standard. The analysis as summarized in **Table 4** shows the study segments along with their number of lanes, adopted LOS/capacity, projected traffic volumes and resultant Levels of Service. The roadway segments in the vicinity of the project will continue operate at satisfactory Levels of Service similar to the existing conditions, except for the segment of SR 19 from CR 48 to Central Avenue. This segment will become over-capacity and fail with the addition of the approved 439 peak hour trips alone, which are more than double the existing traffic on the segment. This segment will fail regardless of the addition of the project trips. The project is adding only 34 peak hour trips to this segment.



Table 4
Projected Roadway Capacity Analysis

Roadway Segment	No. of Lanes	Adopted		P.M. Peak Hour Peak Direction					
		LOS	Capacity	Direction	Volume	Approved Trips*	Project Trips*	Total	LOS
CR 48									
US 27 to Lime Ave	2	D	1,080	EB	469	123	32	624	C
Lime Ave to SR 19	2	D	1,080	EB	409	159	3	571	C
SR 19									
Lane Park Rd to CR 48	2	D	920	SB	652	182	27	861	D
CR 48 to Central Ave	2	D	700	SB	415	439	34	888	F
Central Ave to CR 455	2	D	1,200	SB	415	435	21	871	C
Number 2 Road									
CR 48 to Bloomfield Ave	2	D	675	WB	26	66	21	113	C
Bloomfield Ave to SR 19	2	D	675	EB	52	212	71	335	C

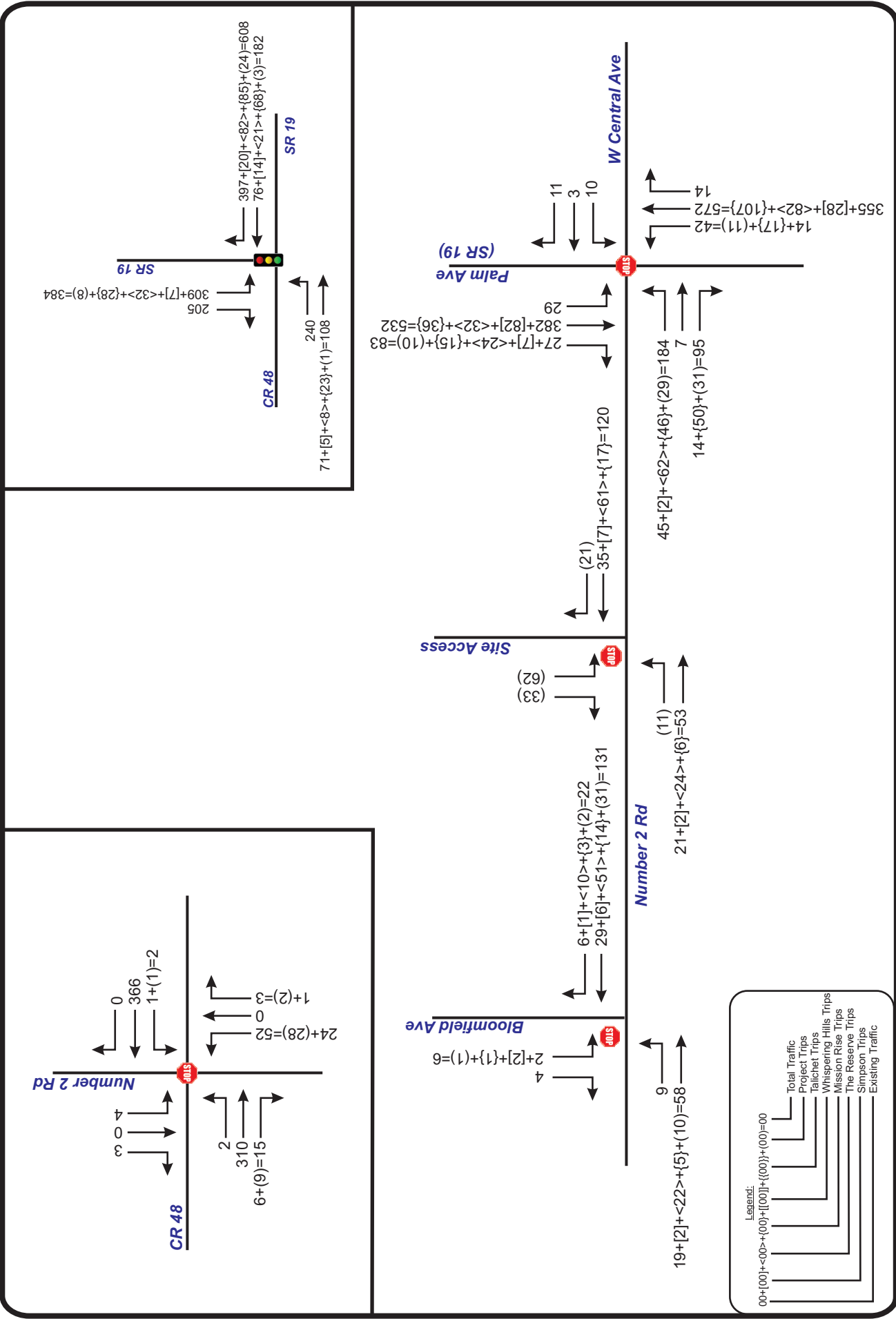
* Highest Trips on the Segment

Intersection Analysis

To assess the projected operating conditions at the study intersections, intersection capacity analyses were conducted using the *Highway Capacity Software (HCS)* in accordance with the procedures of the *Highway Capacity Manual*. **Figure 5a** and **5b** show the total traffic volumes with the project trips. The projected Levels of Service are summarized in **Table 5** and the *HCS* analysis worksheets are provided in **Appendix F**. The analysis shows that the study intersections will operate at overall satisfactory Levels of Service, except for the intersection of SR 19 and CR 48, and the intersection of Palm Avenue and Central Avenue.

The intersection of SR 19 and CR 48 will have a failing westbound approach in the P.M. peak hour, but will operate satisfactorily with signal timing optimization.





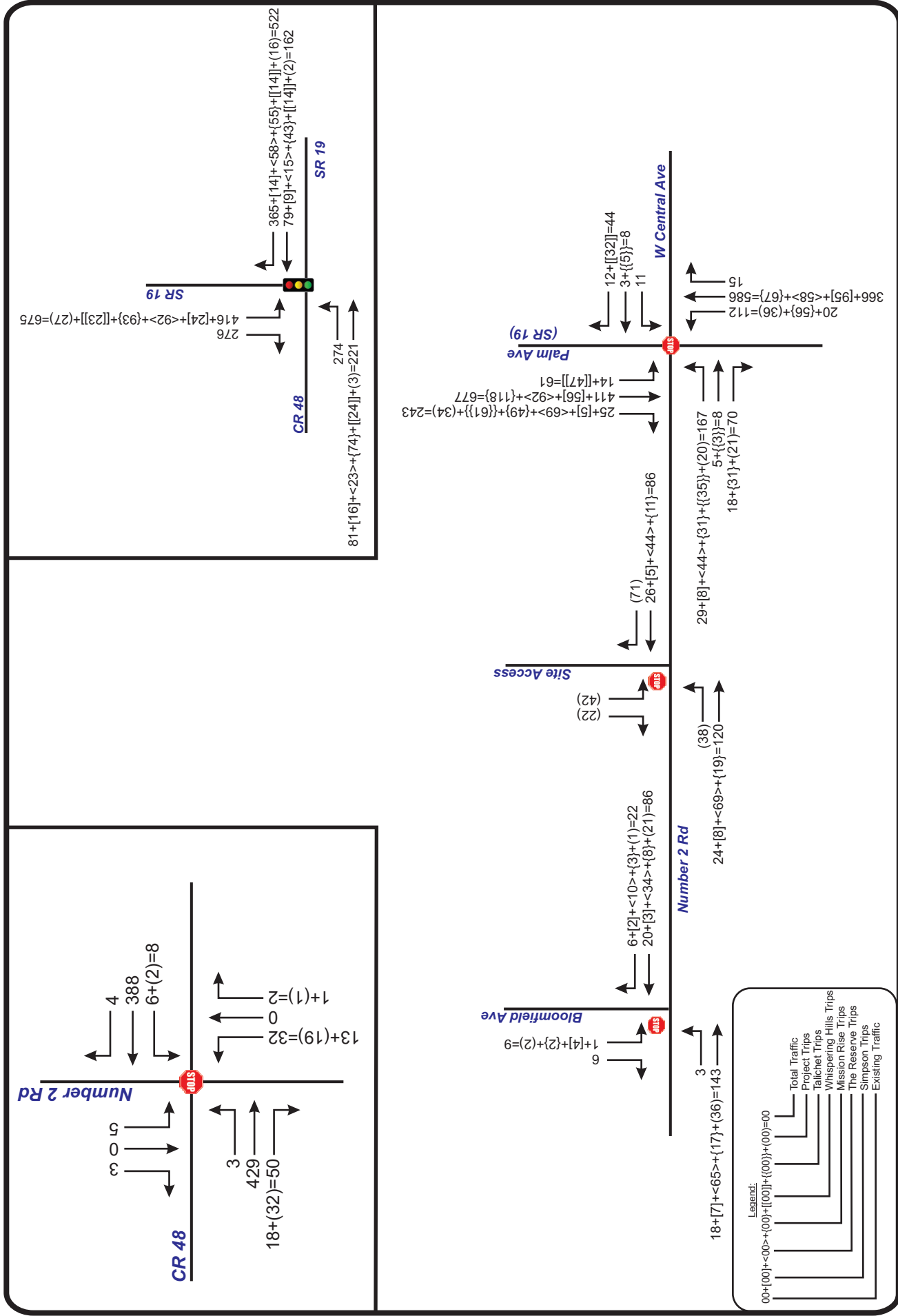


Table 5
Projected P.M. Peak Hour
Intersection Capacity Analysis

Intersection	Control	Period	EB		WB		NB		SB		Overall	
			Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Number 2 Rd & CR 48	Stop	A.M.	0.0	A	0.0	A	19.2	C	14.3	B	--	--
		P.M.	0.1	A	0.2	A	21.8	C	16.2	C	--	--
Number 2 Rd & Bloomfield Ave	Stop	A.M.	1.1	A	--	--	--	--	9.6	A	--	--
		P.M.	0.2	A	--	--	--	--	9.6	A	--	--
Palm Ave & Central Ave	Stop	A.M.	935.5	F	45.8	E	1.5	A	1.0	A	--	--
		P.M.	6309.4	F	352.9	F	6.0	A	2.5	A	--	--
CR 48 & SR 19	Signal	A.M.	12.0	B	4.5	A	--	--	51.9	D	22.2	C
		P.M.	13.2	B	5.1	A	--	--	138.8	F	66.7	E
CR 48 & SR 19 – Opt. Signal Timing	Signal	A.M.	13.2	B	5.0	A	--	--	22.2	C	12.5	B
		P.M.	20.3	C	6.7	A	--	--	32.7	C	21.5	C
Number 2 Rd & Site Access	Stop	A.M.	1.4	A	--	--	--	--	10.2	B	--	--
		P.M.	2.0	A	--	--	--	--	10.7	B	--	--

The intersection of Palm Avenue and Central Avenue was reanalyzed with the background traffic only by removing the project trips from the intersection. The results summarized below for both the A.M. and P.M. peak hours show that the intersection will fail without the addition of the project trips.

Table 6
Palm Ave & Central Ave
Analysis with Background Traffic Only

Intersection	Control	Period	EB		WB		NB		SB		Overall	
			Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
Palm Ave & Central Ave	Stop	A.M.	643.4	F	39.2	E	1.1	A	1.0	A	--	--
		P.M.	3438.3	F	155.9	F	3.9	A	2.4	A	--	--



Turn Lane Analysis

An analysis was conducted for the proposed driveway on Number 2 Road to determine if turn lanes would be warranted. Based upon the procedures of the NCHRP Report 457, Evaluating Intersection Improvements, right and left turns lanes are not warranted. The warrant charts are included in **Appendix G**. The site access driveway is located on a curve with sight distance concerns. Therefore, the Developer will consider the construction of turn lanes as follows:

Left turn lane length (TL) = Deceleration Distance (DD) + Queue Length (QL)

DD = 290 feet for 50 mph design speed (As per FDM 212)

QL = 0.1 vehicles (from HCS P.M. analysis, 25 feet per vehicle, minimum 1 vehicle)

TL = 290 + 25 = 315 feet

Right turn lane length (TL) = Deceleration Distance (DD) + Queue Length (QL)

DD = 290 feet for 50 mph design speed (As per FDM 212)

QL = 0 vehicles (from HCS P.M. analysis, 25 feet per vehicle)

TL = 290 + 0 = 290 feet



STUDY CONCLUSIONS

This traffic analysis was performed to assess the impact of a proposed residential development in Howey-in-the-Hills, Lake County. The proposed development consists of 180 single family homes to be completed by 2028. The results of the study as documented herein are summarized below:

- The development is expected to generate 1,733 new net daily trips, of which 127 will occur in the A.M. peak hour and 173 will occur during the P.M. peak hour.
- The study roadway segments currently operate at satisfactory Levels of Service in the existing conditions and will continue to do so upon completion of the project in 2028, except for the segment of SR 19 from CR 48 to Central Avenue. This segment will fail with the addition of approved project trips which are more than double the existing traffic on the segment.
- The study intersections currently operate at overall satisfactory Level of Service. In the projected conditions, the intersections will continue to operate at a satisfactory Level of Service with project trips added, except for the intersection of Palm Avenue and Central Avenue. This intersection will have a failing eastbound approach due to the existing stop control. This condition will continue to prevail until a signal becomes warranted and installed.
- The proposed development will be served by a full access driveway on Number 2 Road which is projected to operate satisfactorily. Based upon the procedures of the NCHRP Report 457, turn lanes are not warranted at the driveway. While not required, the Developer will consider the construction of turn lanes to address sight distance concerns.



APPENDICES

APPENDIX A

Study Methodology and Related Correspondence

Turgut Dervish

From: Tom Harowski <tom@tmhconsultinginc.com>
Sent: Friday, June 17, 2022 2:07 PM
To: Turgut Dervish
Cc: Don Griffey; Sean O'Keefe; John Brock
Subject: RE: TPD#5659

Okay. Let's go to work.

Thomas A. Harowski, AICP
President
Please make note of my NEW email address: Tom@TMHConsultingInc.com
(386) 316-8426

From: Turgut Dervish <turgut@tpdtraffic.com>
Sent: Tuesday, June 14, 2022 1:49 PM
To: Tom Harowski <tom@tmhconsultinginc.com>
Cc: Don Griffey <dag@griffeyengineering.com>; Sean O'Keefe <sokeefe@howey.org>; John Brock <jbrock@howey.org>
Subject: RE: TPD#5659

Tom,
Table 2 is the significance test showing the project's highest impacts on the roadway segments. You are correct that 100% of the trips end up on Number 2 Road but with 65% going east and 35% going west. Therefore, the project's highest percent impact on Number 2 Road is 65%. In the analysis, the segment was broken into two subsegments at the request of the county. As for the distribution map showing percentages, the trips go to different destinations (represented by traffic zones in the traffic model) along the way. Of the 35% of the trips going west on Number 2 Road, only 31% reach CR 48 with 4% having destinations along Number 2 Road, of which 2% was assigned to Bloomfield Avenue. Of the 65% of the trips going east on Number 2 Road, 64% reach SR 19 with 33% going south and 31% going north. The same happens on SR 19 between Number 2 Road and CR 48 where 4% is lost due to trip destinations along the segment. In the graphical presentation of the trips, we showed the highest trip percentage on each segment. Detailed percentages are shown on the model distribution plot.

Turgut Dervish, P.E., President
TRAFFIC PLANNING AND DESIGN, INC.
535 Versailles Drive
Maitland, Florida 32751
407-628-9955
turgut@tpdtraffic.com

From: Tom Harowski <tom@tmhconsultinginc.com>
Sent: Monday, June 13, 2022 3:30 PM
To: Turgut Dervish <turgut@tpdtraffic.com>
Cc: Don Griffey <dag@griffeyengineering.com>; Sean O'Keefe <sokeefe@howey.org>; John Brock <jbrock@howey.org>
Subject: RE: TPD#5659

Your proposal and response still has me confused. On Table 2 which you cite as correct has 65% of the trips impacting Number 2 Road between CR 48 and SR 19. Since Number 2 Road is the only access to the project, why is the impact not 100%? The distribution map shows the split on Number 2 road as 35% west and 65% east, but the Table 2 seems to contradict this assignment. It seems to me the full project volume could affect the significance of impact to some roads.

The distribution map also seems to leak traffic percentages, and I don't understand where these trips go. For example, the 35% westbound on Number 2 Road drops to 33% at Bloomfield Ave. where 2% of the trips are shown as taking Bloomfield Ave, but at CR 48 the total volume drops to 31% with 29% west on CR 48 and 2% east on CR 48. Where did the other two percent go? The same thing happens on Number 2 Road at SR 19 where 31% goes north and 33% of the 65% goes south. We lose 1% here. At SR 19 and CR 48 25% of the 31% goes east and 3% continues on CR 48, so another 3% of the volume is unaccounted. Are these trips stopping in Howey proper?

Thomas A. Harowski, AICP

President

Please make note of my NEW email address: Tom@TMHConsultingInc.com

(386) 316-8426

From: Turgut Dervish <turgut@tpdtraffic.com>

Sent: Tuesday, June 7, 2022 4:08 PM

To: Tom Harowski <tom@tmhconsultinginc.com>; Lewis, Sharon E <SELewis@lakecountyfl.gov>; tmhconsulting@cfl.rr.com

Cc: Rita Merhi <rita@tpdtraffic.com>; John Brock <jbrock@howey.org>; Sean O'Keefe <sokeefe@howey.org>; Don Griffey <dag@griffeyengineering.com>

Subject: RE: TPD#5659

Tom,

The trip distribution map shows the highest percent distribution on any segment. When a roadway segment has more than one distribution, the higher is depicted on the map for illustration purposes. For example, the segment of SR 19 between CR 48 and Central Avenue has a trip distribution ranging from 27% to 31%. In this instance the higher 31% used in impact assessment was shown on the map. The detailed distributions are shown in the model distribution plot included in the appendix of the TIA. For reference, attached are the trip distribution map and the model distribution plot. We also checked the tables and they are correct. If you have any further comments/questions, please do not hesitate to bring to our attention. We will be glad to provide clarification and/or response with additional information.

Turgut

Turgut Dervish, P.E., President
TRAFFIC PLANNING AND DESIGN, INC.
535 Versailles Drive
Maitland, Florida 32751
407-628-9955
turgut@tpdtraffic.com

From: Tom Harowski <tom@tmhconsultinginc.com>

Sent: Monday, June 6, 2022 1:36 PM

To: Turgut Dervish <turgut@tpdtraffic.com>; Lewis, Sharon E <SELewis@lakecountyfl.gov>; tmhconsulting@cfl.rr.com

Cc: Rita Merhi <rita@tpdtraffic.com>; John Brock <jbrock@howey.org>; Sean O'Keefe <sokeefe@howey.org>; Don Griffey <dag@griffeyengineering.com>

Subject: RE: TPD#5659

The distributions still don't add up properly and the tables in the text still need to be corrected. PLEASE HAVE SOMEONE PROOFREAD THE SUBMITTAL AND MAKE THE NECESSARY EDITS.

Thomas A. Harowski, AICP

President

Please make note of my NEW email address: Tom@TMHConsultingInc.com

(386) 316-8426

Rita Merhi

From: Lewis, Sharon E <SELewis@lakecountyfl.gov>
Sent: Wednesday, May 25, 2022 2:50 PM
To: Turgut Dervish; tmhconsulting@cfl.rr.com
Cc: Rita Merhi
Subject: RE: TPD#5659
Attachments: Markup 5659 Residential Development-Howey-in-the-Hills Methodology 052422.pdf

Dervish,

Please see comments in the attached . Thanks



SHARON E LEWIS, MSCTM
Traffic Project Engineer

PUBLIC WORKS
Engineering

A P.O Box 7800, Tavares, FL 32778
P 352-253-9050 | F 352-253-6016
E selewis@lakecountyfl.gov | W www.lakecountyfl.gov

*NOTE: Florida has a very broad public records law.
Your email communications may be subject to public disclosure.*

From: Turgut Dervish <turgut@tpdtraffic.com>
Sent: Tuesday, May 24, 2022 12:27 PM
To: Lewis, Sharon E <SELewis@lakecountyfl.gov>; tmhconsulting@cfl.rr.com
Cc: Rita Merhi <rita@tpdtraffic.com>
Subject: FW: TPD#5659

CAUTION: This email originated from outside of your organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.

Sharon/Tom,

Attached is our proposed traffic study methodology for a residential project located on Number 2 Road in Howey-in-the-Hills, Lake County for your review. Please call if you have questions.

Thanks,

Turgut

Turgut Dervish, P.E., President
TRAFFIC PLANNING AND DESIGN, INC.
535 Versailles Drive
Maitland, Florida 32751
407-628-9955
turgut@tpdtraffic.com

Table 2
Significance Analysis

Roadway Segment	# of Lns	Adopted			Existing		PHPD Trips Volume	%	Project Trips as % of Capacity	Significance*
		LOS	Capacity	Direction	Volume	Volume				
CR 48	US 27 to Lime Ave	2L	D	1,080	EB	460	26%	43	3.9%	No
	Lime Ave to SR 19	2L	D	1,080	EB	400	3%	4	0.4%	No
SR 19	Lime Park Rd to CR 48	2L	D	920	SB	662	25%	37	4.0%	No
	CR 48 to Central Ave	2L	D	700	SB	415	31%	46	6.6%	Yes
Member 2 Road	Central Ave to CR 455	2L	D	1,200	SB	4150	33%	49	4.1%	No
	CR 48 to SR 19	2L	D	675	WB	28	66%	66	14.2%	Yes

* Project as % of Capacity

** Not Included in the County's Database

5. Traffic Impact Assessment

a) Roadway

- Obtain existing traffic volumes on study roadway segment from Lake County for use in the traffic analysis.
- Determine background traffic with the use of an annual growth rate obtained from historical AADT in the vicinity of the project plus vested trips obtained from the City/County.
- Combine project traffic with background traffic to obtain total traffic volumes.
- Perform P.M. peak hour/peak direction roadway analyses utilizing City/County LOS standards.

b) Intersections

- Conduct intersection counts or use factors to adjust intersection data during the A.M./P.M. peak periods at the study intersections.
- Determine background traffic by expanding existing traffic counts to the project's buildout year plus committed trips provided by the County.
- Combine project traffic with background traffic to obtain total traffic.

3 comments



PAGE 5

1

selewis May 25

Split percentage does not add up

PAGE 6

2

selewis May 25

A High Plan will need to be conducted

selewis May 25

Please split the segment at Bloomfield Ave
(East of/West of)



TO: Sharon Lewis, MS
Lake County Public Works
Thomas A. Harowski
Howey-in-the-Hills Town Planner

FROM: Turgut Dervish, P.E.

DATE: May 24, 2022

RE: **Traffic Impact Analysis Methodology**
Residential Development, Howey-in-the-Hills, Florida
TPD No. 5659

The following is an outline of the proposed methodology for the Traffic Impact Study for a residential development in Howey-in-the Hills, Lake County. The project site is located on the north side of Number 2 Road approximately 2 miles west of the SR 19 (Palm Avenue). **Figure 1** depicts the site location and the area roadways.

1. Proposed Development

The proposed development will consist of 250 single family lots. The development is anticipated to be built by 2025. **Figure 2** depicts the conceptual site plan.

2. Trip Generation

Trip generation data from the 11th Edition of the Institute of Transportation Engineers (ITE) *Trip Generation Manual* will be used for the trip generation estimation of the development. **Table 1** provides a summary of the trip generation for the proposed development calculated with the ITE data. The project is expected to generate a total of 2,344 daily trips of which 172 will occur during the A.M. peak hour and 235 will occur during P.M. peak hour. The ITE trip generation worksheets are included in **Attachment A**.

Table 1
Trip Generation Calculation Summary

ITE Code	Land Use	Quantity	Daily		A.M. Peak Hour				P.M. Peak Hour			
			Rate*	Trips	Rate*	Enter	Exit	Total	Rate*	Enter	Exit	Total
210	Single-Family Detached	250 DU**	9.38	2,344	0.69	45	127	172	0.94	148	57	235
Totals				2,344	---	45	127	172	---	148	57	235

*ITE Equations Used, $R^2 > 0.75$.

**DU=Dwelling Units



Site Location

Howey in the Hills
Project № 5659
Figure 1





Howey in the Hills
Project No 5659
Figure 2



Conceptual Site Plan



3. Trip Distribution

The trip distribution pattern for the proposed project was estimated using the currently adopted Central Florida Regional Planning Model (CFRPM). A Select Zone Analysis (SZA) was conducted by modifying the 2030 interim year model network to include a Traffic Analysis Zone (TAZ) representing the proposed project and the model's socio-economic data updated to reflect the proposed project buildout. The trip distribution in the project vicinity is shown in **Figure 3**. The model distribution plot is included in **Attachment B**.

4. Impact Area

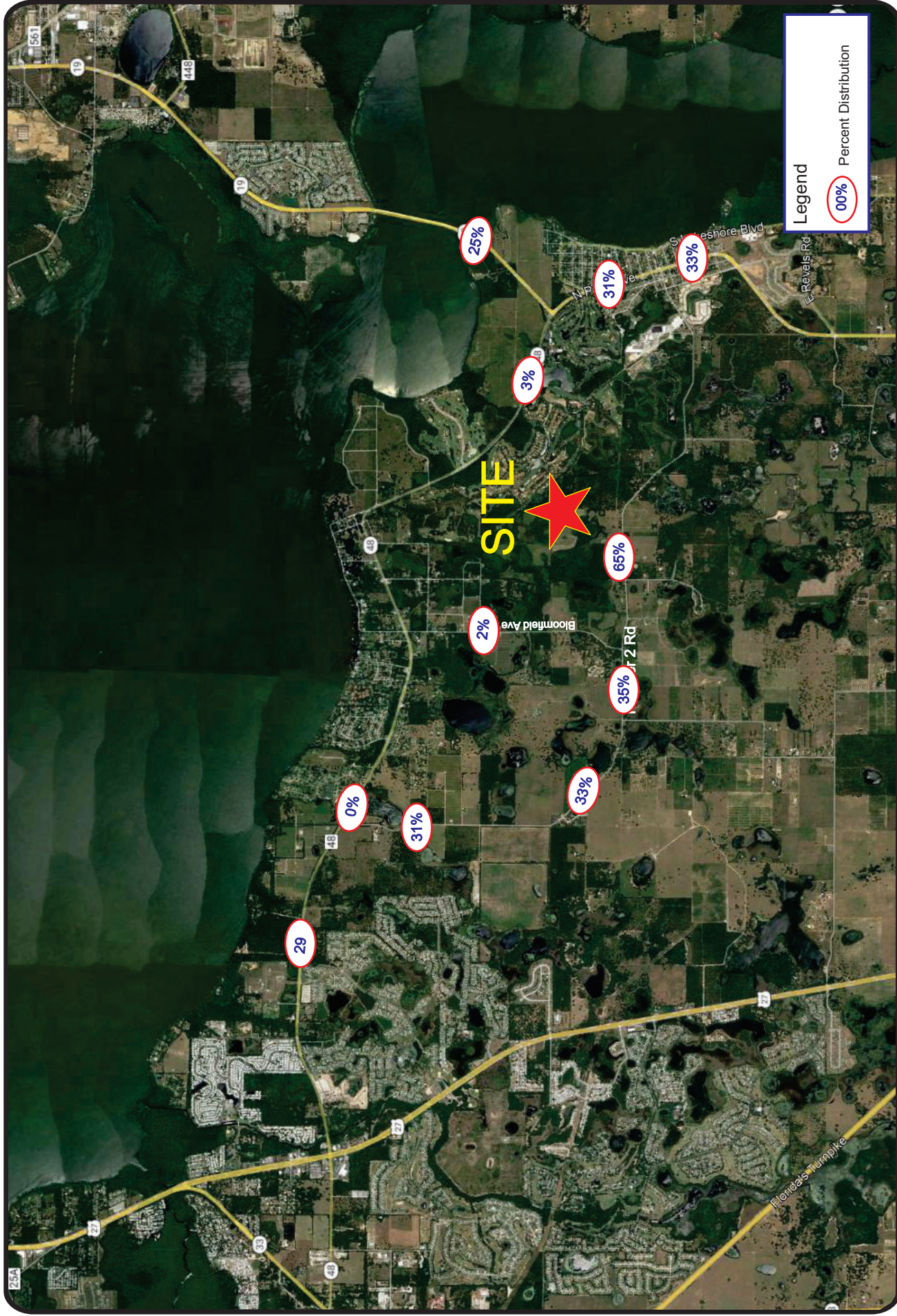
As per the Lake Sumter MPO TIS guidelines, a Tier 2 Traffic Impact Study (TIS) is required for this project. A minimum of one-mile impact area plus all roadway which the project trips consume 5% or more of the roadway capacity. Based upon the project's significance as per **Table 2**, the following roadway segments and intersections will be included in the analysis as a minimum:

The roadway segments to be included in the analysis:

- Number 2 Road
 - CR 48 to SR 19 (Palm Avenue)
- CR 48
 - US 27 to Lime Avenue
 - Lime Avenue to SR 19
- SR 19
 - Lane Park Road to CR 48
 - CR 48 to Central Avenue
 - Central Avenue to CR 455

The intersections to be included in the area analysis are:

- Number 2 Road and CR 48
- CR 48 and SR 19
- Number 2 Road (Central Avenue) and SR 19
- Site Entrance



Trip Distribution

Howey in the Hills
Project No 5659
Figure 3



Table 2
Significance Analysis

Roadway Segment	# of Lns	Adopted		Existing		PHPD Trips		Project Trips as % of Capacity	Significance*
		LOS	Capacity	Direction	Volume	%	Volume		
CR 48									
US 27 to Lime Ave	2L	D	1,080	EB	469	29%	43	3.9%	No
Lime Ave to SR 19	2L	D	1,080	EB	409	3%	4	0.4%	No
SR 19									
Lane Park Rd to CR 48	2L	D	920	SB	652	25%	37	4.0%	No
CR 48 to Central Ave	2L	D	700	SB	415	31%	46	6.6%	Yes
Central Ave to CR 455	2L	D	1,200	SB	4150	33%	49	4.1%	No
Number 2 Road**									
CR 48 to SR 19	2L	D	675	WB	28	65%	96	14.2%	Yes

* Project as % of Capacity

** Not Included in the County's Database

5. Traffic Impact Assessment

a) Roadway

- Obtain existing traffic volumes on study roadway segment from Lake County for use in the traffic analysis.
- Determine background traffic with the use of an annual growth rate obtained from historical AADT in the vicinity of the project plus vested trips obtained from the City/County.
- Combine project traffic with background traffic to obtain total traffic volumes.
- Perform P.M. peak hour/peak direction roadway analyses utilizing City/County LOS standards.

b) Intersections

- Conduct intersection counts or use factors to adjust intersection data during the A.M./P.M. peak periods at the study intersections.
- Determine background traffic by expanding existing traffic counts to the project's buildout year plus committed trips provided by the County.
- Combine project traffic with background traffic to obtain total traffic.

- Perform intersection capacity analysis utilizing the HCS operational analysis procedures for the A.M./P.M. peak hour.

6. Traffic Report

Prepare traffic report summarizing study procedures, analyses and recommendations. If you have any questions or concerns, please contact us at (407) 628-9955.

Attachment A
ITE Trip Generation Sheets

Single-Family Detached Housing (210)

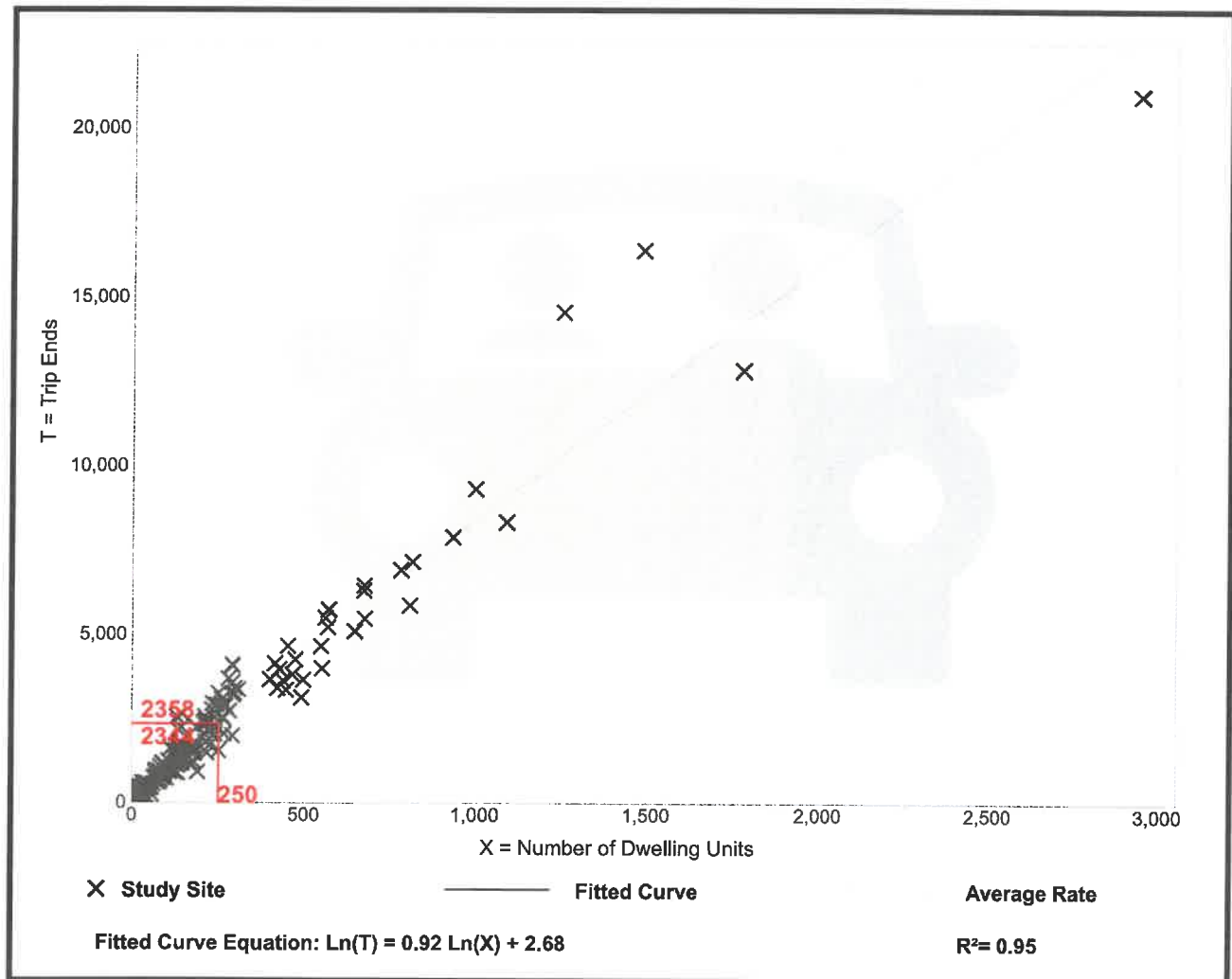
Vehicle Trip Ends vs: Dwelling Units
On a: Weekday

Setting/Location: General Urban/Suburban
Number of Studies: 174
Avg. Num. of Dwelling Units: 246
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
9.43	4.45 - 22.61	2.13

Data Plot and Equation



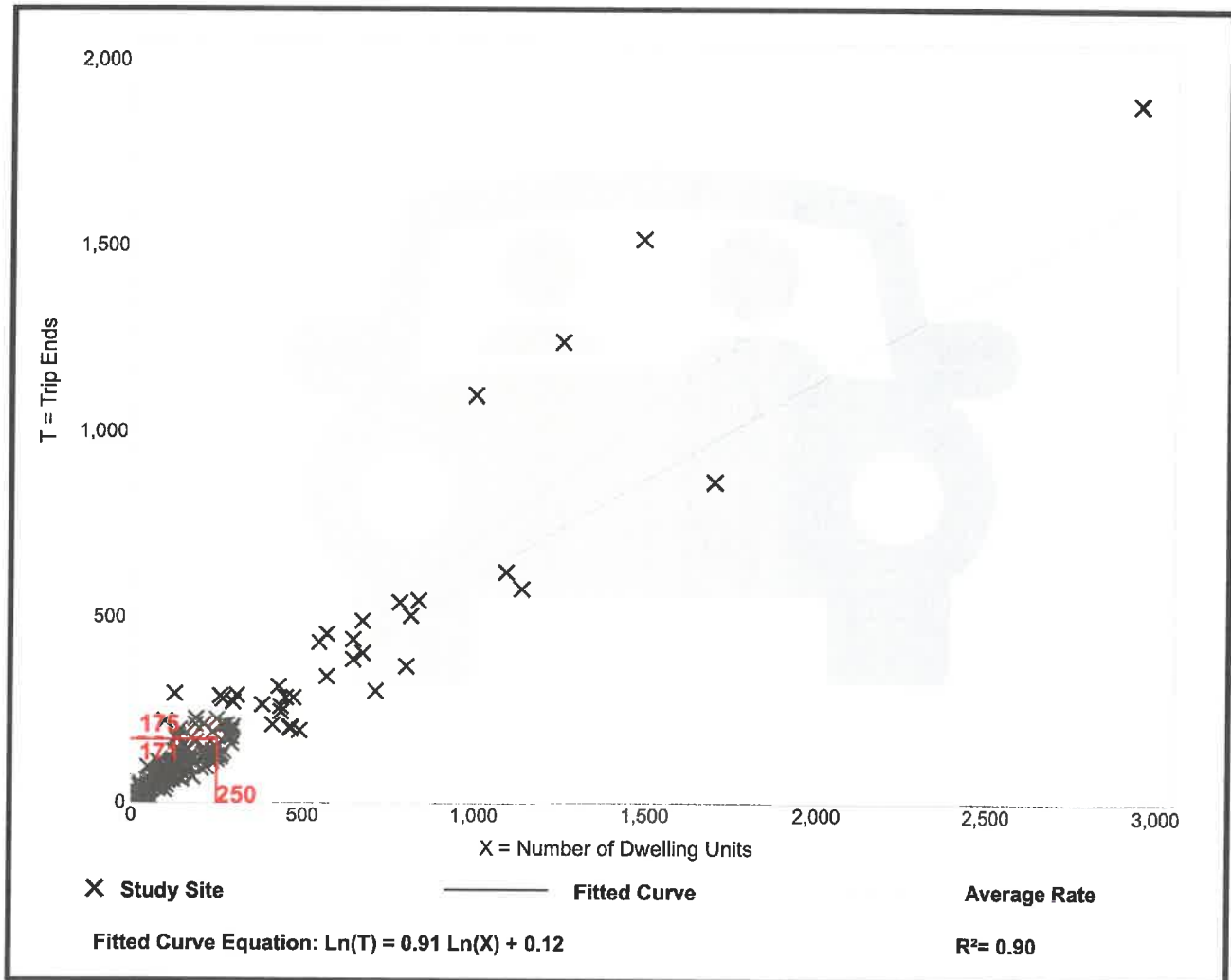
Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 7 and 9 a.m.
Setting/Location: General Urban/Suburban
Number of Studies: 192
Avg. Num. of Dwelling Units: 226
Directional Distribution: 26% entering, 74% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.70	0.27 - 2.27	0.24

Data Plot and Equation



Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 208

Avg. Num. of Dwelling Units: 248

Directional Distribution: 63% entering, 37% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate

0.94

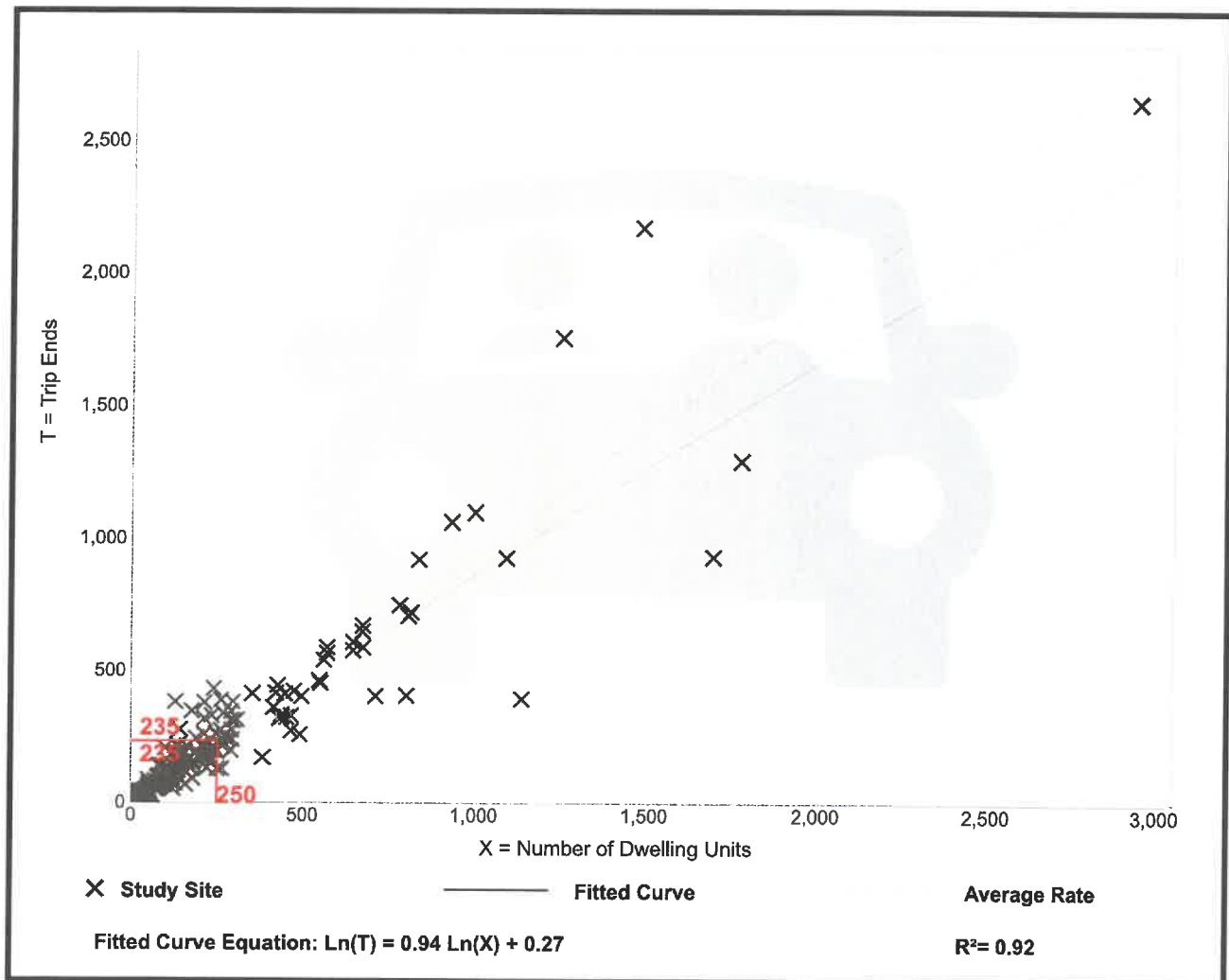
Range of Rates

0.35 - 2.98

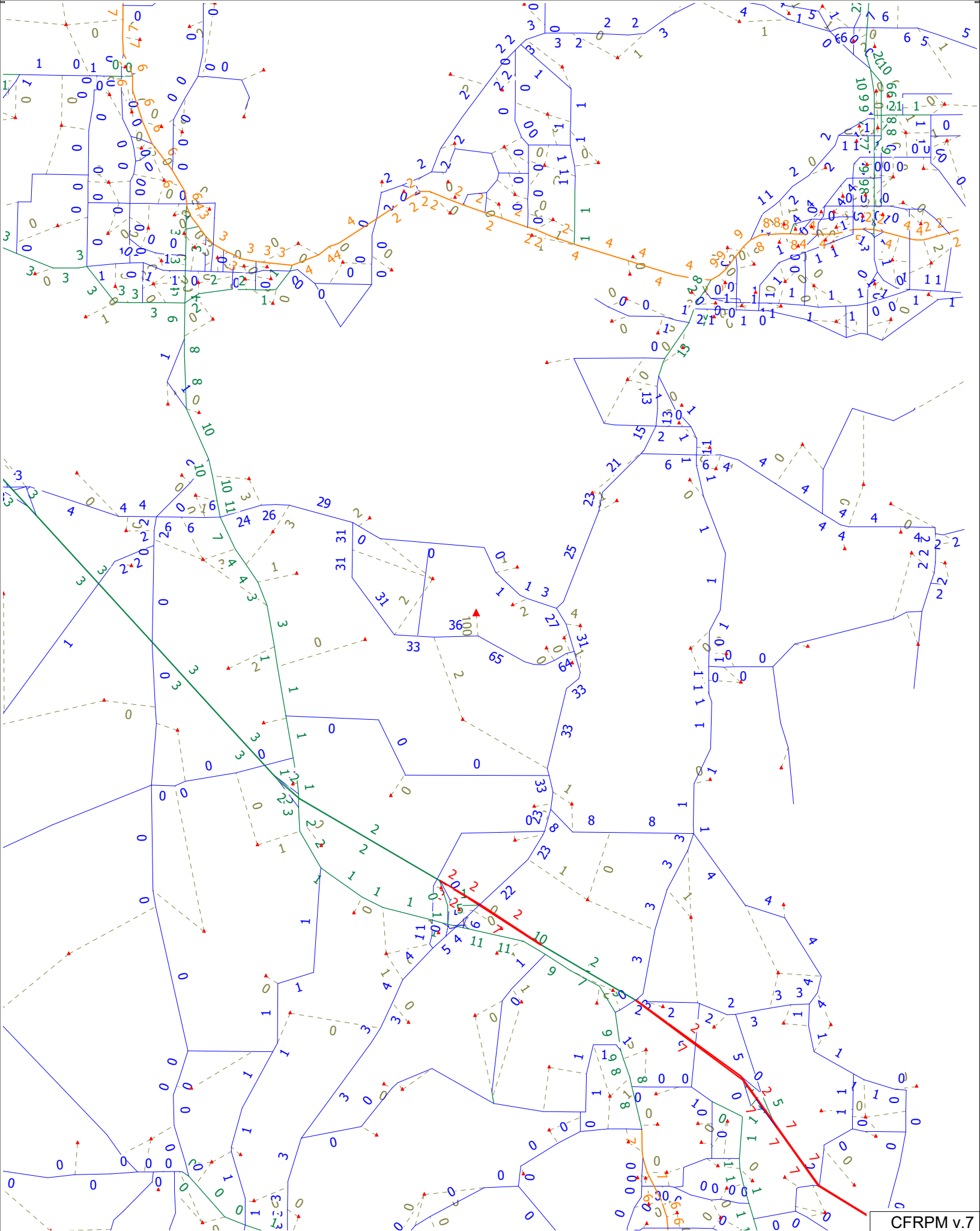
Standard Deviation

0.31

Data Plot and Equation



Attachment B
Model Distribution Plot



APPENDIX B

Lake County 2022 CMP Database

Segment ID	County Station	Foot Station	Data Source	Speed Limit	Segment Length (mi)	Road Name	From	To	Lanes (2022)	Lanes (2027)	Urban / Rural	Divided / Undivided	Maintaining Agency	Jurisdiction	Adopted LOS Standard	Daily Service Volume	2022 ADOT VC	2022 Daily LOS	2022 Peak Hour VC	2022 Peak Hour SBWB	2022 Peak Hour NBEB	2022 Peak Hour NBEB Volume	2027 ADOT VC	2027 Daily LOS	2027 Peak Hour Directional Service Volume (2027)	2027 Peak Hour NBEB Volume	2027 Peak Hour SBWB	2027 Peak Hour VC	2027 Peak Hour LOS			
1120	480		County	35	1.80	CR 488	PINE RIDGE DAIRY ROAD	GRiffin ROAD	2	2	URBAN	UNDIVIDED	COUNTY	FRUITLAND PARK	D	13,320	7,736	0.86	D	680	384	343	680	9,868	0.87	D	680	384	445	0.85	D	
1130	496		County	45	1.13	CR 488	GRiffin ROAD	SR 44	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	12,320	9,173	0.74	C	620	404	440	620	10,005	0.81	C	620	440	440	0.77	C	
1145	612		County	55	3.65	CR 488 REALIGNMENT	SR 44	SR 46	2	2	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	12,320	16,576	2.14	E	620	857	863	620	19,687	2.54	E	620	857	1,018	2.48	E	
1150	267		County	55	0.54	CR 470	SUMMIT COUNTY LINE	FLORIDA TURNPIKE	2	4	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	13,300	11,303	0.85	D	690	530	530	690	16,986	0.89	C	1,500	788	566	0.83	C	
1155	266		County	55	2.39	CR 470	FLORIDA TURNPIKE	BAY AVENUE	2	2	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	12,600	8,626	0.70	D	660	436	436	660	9,276	0.74	D	660	436	292	0.69	D	
1160	266		ADJACENT	55	0.54	CR 470	BAY AVENUE	CR 44	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	12,600	8,626	0.71	C	660	436	436	660	9,276	0.75	D	660	436	292	0.74	C	
1170	489		County	35	2.89	CR 473	FOUNTAIN LAKE BOULEVARD	FOUNTAIN LAKE BOULEVARD	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	12,600	6,887	0.49	D	710	422	442	710	14,000	0.52	D	710	422	338	0.25	C	
1180	443		County	40	1.03	CR 473	FOUNTAIN LAKE BOULEVARD	US 441	4	4	URBAN	DIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	35,820	14,713	0.41	C	1,600	811	461	1,600	16,444	0.44	C	1,600	852	485	0.47	C	
1190	4		County	55	5.21	CR 474	GREEN SWAMP ROAD	GREEN SWAMP ROAD	2	2	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	C	17,400	5,962	0.77	C	410	240	240	410	9,745	0.67	C	410	240	171	0.22	C	
1200	5		County	55	3.35	CR 474	GREEN SWAMP ROAD	US 27 / US 411	2	2	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	C	7,740	5,436	0.70	C	410	240	240	410	9,745	0.74	C	410	240	171	0.22	C	
1210	262		County	45	0.78	CR 478	WATER LANE RD	US 27 / US 411	2	2	RURAL	UNDIVIDED	COUNTY	CITY OF FLEESBURG	D	17,400	5,436	0.70	C	410	240	240	410	9,745	0.74	C	410	240	171	0.22	C	
1220	246		County	55	3.37	CR 48	CLEARWATER LAKE RD	CLEARWATER LAKE RD	2	2	RURAL	UNDIVIDED	COUNTY	CITY OF FLEESBURG	D	17,400	5,436	0.70	C	410	240	240	410	9,745	0.74	C	410	240	171	0.22	C	
1225	246		County	55	2.41	CR 48	CLEARWATER LAKE RD	US 27	2	2	RURAL	UNDIVIDED	COUNTY	CITY OF FLEESBURG	D	7,740	3,324	0.43	B	410	206	206	410	7,629	0.47	C	410	206	134	0.24	C	
1230	263		County	45	0.46	CR 48	HAYWOOD WORM FARM RD	US 27	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	15,920	8,626	0.55	C	790	307	297	790	15,920	0.64	C	790	307	424	0.54	C	
1235	262		County	45	0.68	CR 48	HAYWOOD WORM FARM RD	US 27	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	15,920	8,626	0.54	C	840	401	375	840	16,820	0.57	C	840	424	384	0.50	C	
1240	264		County	40	4.89	CR 48	LINE AVENUE	LINE AVENUE	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	21,760	9,821	0.45	B	1,080	400	380	1,080	11,949	0.55	C	1,080	421	384	0.47	B	
1250	295		County	40	2.04	CR 48	SR 19	SR 19	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	21,760	9,821	0.46	B	1,080	420	380	1,080	11,949	0.55	C	1,080	462	435	0.43	B	
1260	253		County	40	1.14	CR 48	RANCH ROAD	RANCH ROAD	2	2	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	16,820	6,515	0.39	C	840	310	292	840	16,820	0.41	C	840	326	307	0.39	C	
1270	253		ADJACENT	40	3.17	CR 48	CR 418A	CR 418A	2	2	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	7,740	6,515	0.84	C	520	262	262	520	6,647	0.86	C	520	326	307	0.80	C	
1280	217		County	30	0.71	CR 50 (SUNSET AVENUE)	SR 50	SR 50	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF MASCOETTE	D	10,360	1,592	0.15	C	520	95	95	520	10,360	1.76	0.17	C	520	72	164	0.20	C
1290	210		County	45	1.74	CR 50	N HANCOCK ROAD	CR 50	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF MINNEOLA	D	16,820	6,881	0.42	C	840	285	346	840	16,820	0.37	C	840	299	363	0.43	C	
1300	202		County	45	2.47	CR 50	N HANCOCK ROAD	CR 50	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF MINNEOLA	D	21,760	6,881	0.41	C	1,080	228	491	1,080	20,420	1.38	F	1,080	251	542	0.50	C	
1310	42		County	45	1.92	CR 50	ORANGE COUNTY LINE	ORANGE COUNTY LINE	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	16,820	6,828	0.32	B	840	195	457	840	16,820	1.76	0.43	C	840	205	585	0.70	C
1320	417		County	35	1.08	CR 50A OLD 441	SR 19	SR 19	2	2	URBAN	DIVIDED	COUNTY	CITY OF FAVAIRES	D	8,360	9,807	1.18	F	870	387	450	870	8,360	10.412	1.24	F	870	386	473	0.54	D
1330	413	115084	County	35	1.08	CR 50A OLD 441	DORA AVENUE	DORA AVENUE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF FAVAIRES	D	8,360	9,807	1.18	F	870	387	450	870	8,360	10.412	1.24	F	870	386	473	0.54	D
1340	420		County	35	1.84	CR 50A OLD 441	BAY ROAD	BAY ROAD	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF FAVAIRES	D	16,820	9,568	0.87	C	840	498	424	840	16,820	10.045	0.60	C	840	514	446	0.61	C
1350	421		County	35	1.08	CR 50A OLD 441	CR 440 / EUDORA AVENUE	CR 440 / EUDORA AVENUE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF MOUNT DORA	D	10,360	9,817	0.86	D	530	465	458	530	10,360	11.220	1.08	F	530	526	518	0.89	D
1360	415		County	35	0.79	CR 50A OLD 441	LAKESHORE DRIVE	LAKESHORE DRIVE	2	2	URBAN	DIVIDED	COUNTY	CITY OF MOUNT DORA	D	14,760	16,591	1.12	F	750	725	761	750	14,760	20.420	1.38	F	750	893	937	1.25	F
1370	415		ADJACENT	35	0.63	CR 50A OLD 441	LAKESHORE DRIVE	STAVENUE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF MOUNT DORA	D	10,360	11,207	1.08	F	530	499	505	530	10,360	13.3	F	530	577	621	1.17	F	
1380	605		ADJACENT	35	0.63	CR 50A OLD 441	N HIGHLAND STREET	N HIGHLAND STREET	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF MOUNT DORA	D	10,360	2,702	1.08	F	530	499	505	530	10,360	13.3	F	530	577	621	1.17	F	
1390	602	115004	County	35	0.75	CR 50A OLD 441	ORANGE COUNTY LINE	ORANGE COUNTY LINE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF MOUNT DORA	D	10,360	2,702	1.08	F	530	499	505	530	10,360	13.3	F	530	577	621	1.17	F	
1400	252		County	45	1.82	CR 50A OLD 441	CR 48	CR 48	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF MOUNT DORA	D	10,360	2,702	1.08	F	530	499	505	530	10,360	13.3	F	530	577	621	1.17	F	
1410	252		County	45	1.82	CR 50A OLD 441	CR 48	CR 48	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF MOUNT DORA	D	10,360	2,702	1.08	F	530	499	505	530	10,360	13.3	F	530	577	621	1.17	F	
1420	252		ADJACENT	40	0.63	CR 50A OLD 441	CR 48	CR 48	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF MOUNT DORA	D	10,360	2,702	1.08	F	530	499	505	530	10,360	13.3	F	530	577	621	1.17	F	
1430	252		ADJACENT	40	2.49	CR 50A OLD 441	SOUTH ASTATULA CITY LIMIT	SOUTH ASTATULA CITY LIMIT	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	12,360	11,947	0.95	D	1,080	570	558	1,080	12,360	12.556	1.01	E	1,080	599	586	0.97	D
1440	248		County	35	1.74	CR 50A OLD 441	HONEY CROSS ROAD	HONEY CROSS ROAD	2	2	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	21,760	7,697	0.85	C	470	369	364	470	9,030	0.90	C	470	367	402	0.82	C	
1450	237		County	40	1.77	CR 50A OLD 441	TUNPIKE ROAD / CR 50A	TUNPIKE ROAD / CR 50A	2	2	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	12,360	8,115	0.86	C	640	386	386	640	12,360	9.529	0.70	C	640	345	405	0.63	C
1460	235		County	45	0.46	CR 50A OLD 441	US 27	US 27	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	12,360	9,075	0.73	C	620	403	385	620	12,360	9.529	0.73	C	620	345	405	0.63	C
1470	214		County	30	1.78	EAST AVELANE, MINNEOLA DRAINAGE	US 27	EAST AVELANE	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	14,060	2,151	0.15	C	710	108	124	710	14,060	2.555	0.18	C	710	126	147	0.21	C
1480	214		ADJACENT	30	1.05	8TH STOSCEGLIA STATH STICARROL STIGROS	W MINNEOLA AVENUE	W MINNEOLA AVENUE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF CLEMMONT	D	10,360	2,151	0.21	C	520	108	124	520	10,360	2.555	0.25	C	520	126	147	0.28	C
1490	115065		State	-	0.42	CR 50A OLD 441	CR 50A	CR 50A	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF CLEMMONT	D	10,360	1,085	0.09	C	620	179	186	620	1,140	0.09	C	620	188	195	0.31	C	
1500	203		ADJACENT	35	0.23	CR 50A	SR 50	SR 50	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF CLEMMONT	D	14,060	5,175	0.37	C	710	278	278	710	14,060	7.060	0.50	D	710	381	290	0.54	D
1510	45		County	25	4.31	CR 50A	LOG HOUSE ROAD	LOG HOUSE ROAD	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF CLEMMONT	D	14,060	6,597	0.47	C	710	326	276	710	14,060	6.534	0.48	C	710	342	290	0.48	C
1520	10		County	55	1.56	CR 50A	FLORIDA BOYS RANCH ROAD	FLORIDA BOYS RANCH ROAD	2	2	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	16,820	3,767	0.22	B	840	159	156	840	16,820	4.189	0.25	C	840	175	172	0.21	C
1530	6		County	55	5.87	CR 50A	FLORIDA BOYS RANCH ROAD	FLORIDA BOYS RANCH ROAD	2	2	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	7,740	2,228	0.29	B	410	106	100	410	7,740	2.491	0.32	B	410	116	112	0.29	B
1540	234		County	55	1.16	CR 50A	SCHUBERT LANE	SCHUBERT LANE	2	2	URBAN	UNDIVIDED	COUNTY																			

Lake County CMP Database

Segment ID	County Station	Foot Station	Data Source	Speed Limit	Segment Length (mi)	Road Name	From	To	Lanes (2022)	Lanes (2027)	Urban / Rural	Divided / Undivided	Maintaining Agency	Jurisdiction	Adopted LOS Standard	2022 ADOT Volume	2022 ADOT VC	2022 Daily VC	2022 Peak Hour Volume	2022 Peak Hour SBWB Volume	2022 Peak Hour VC	2022 Peak Hour LOS	Growth Rate	Daily Service Volume (24hr)	2027 ADOT VC	2027 Daily LOS	Peak Hour Directional Service Volume (2027)	2027 Peak Hour SBWB Volume	2027 Peak Hour VC	2027 Peak Hour LOS				
2070	39		County	45	1.50	N. HANCOCK ROAD	SR 90	SR 50	4	4	URBAN	DIVIDED	COUNTY	CITY OF CLEMONT	D	37,810	17,618	0.47	C	1,900	860	0.43	C	1.00%	37,810	16,516	0.48	C	1,800	715	0.45	C		
2080	35		County	45	0.25	HOKES STREET	SR 90	HOKES STREET	4	4	URBAN	DIVIDED	COUNTY	CITY OF CLEMONT	D	35,820	22,918	0.64	C	1,800	734	0.99	0.95	2.25%	35,820	26,615	0.72	C	1,800	821	1.16	0.62	C	
2085	35		ADJACENT	45	1.23	S. HANCOCK ROAD	JOHNS LAKE ROAD	JOHNS LAKE ROAD	2	2	URBAN	DIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	17,660	22,918	1.30	F	880	840	0.99	1.13	2.25%	17,660	26,615	1.45	F	880	821	1.16	1.27	F	
2090	17		County	45	1.75	HARTWOOD MARSH ROAD	JOHNS LAKE ROAD	HARTWOOD MARSH ROAD	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF CLEMONT	D	16,820	12,491	0.74	C	840	710	0.87	0.86	1.50%	16,820	13,456	0.80	F	840	641	1.18	0.83	C	
2100	15		County	40	0.70	HARTWOOD MARSH ROAD	US 27	HARTWOOD MARSH ROAD	2	4	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	15,920	17,664	1.11	F	790	960	0.73	0.98	2.75%	35,820	13,456	0.80	F	1,800	641	999	0.96	C	
2104	AVG (146.150)		AVERAGE	40	1.41	HARTWOOD MARSH ROAD	US 27	HARTWOOD MARSH ROAD	2	4	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	16,820	17,664	1.11	F	840	410	0.87	0.86	N/A	35,820	16,110	0.83	C	1,500	494	854	0.45	C	
2110	20		County	40	2.47	HARTWOOD MARSH ROAD	N. 90 DEGREE BEND	ORANGE COUNTY LINE	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	21,760	11,408	0.52	C	1,080	330	0.74	0.18	1.00%	21,760	11,389	0.55	C	1,080	347	708	0.98	C	
2120	465		County	30	0.75	LAKEVIEW AVENUE	LAKEVIEW AVENUE	LAKEVIEW AVENUE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF EUSTIS	D	10,360	1,646	0.16	C	530	80	0.94	0.18	1.00%	10,360	1,720	0.17	C	530	84	99	0.19	C	
2130	605		County	25	1.01	HASELTON STREET	LIMIT AVENUE	LIMIT AVENUE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF MOUNT DORA	D	10,360	2,752	0.27	C	530	179	1.27	0.34	3.50%	10,360	3,316	0.32	C	530	213	150	0.40	C	
2140	32		County	35	0.35	HOKES STREET	LAKESHORE DRIVE	LAKESHORE DRIVE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF CLEMONT	D	10,360	2,752	0.27	C	530	281	0.86	0.86	1.00%	10,360	3,316	0.32	D	530	355	367	0.69	D	
2150	31		County	35	0.37	HOKES STREET	LAKEVIEW AVENUE	LAKEVIEW AVENUE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF CLEMONT	D	10,360	2,752	0.27	C	530	281	0.86	0.86	1.00%	10,360	3,316	0.32	D	530	355	367	0.69	D	
2155	34		County	35	1.05	HOKES STREET	OTIS BLISS DRIVE	OTIS BLISS DRIVE	4	4	URBAN	DIVIDED	COUNTY	CITY OF CLEMONT	D	20,720	11,413	0.52	C	1,080	340	0.87	0.18	1.00%	20,720	11,389	0.55	C	1,080	347	708	0.98	C	
2160	456		County	35	0.59	HUFFSTETLER DRIVE	OTIS BLISS DRIVE	OTIS BLISS DRIVE	4	4	URBAN	DIVIDED	COUNTY	CITY OF CLEMONT	D	20,720	11,413	0.52	C	1,080	340	0.87	0.18	1.00%	20,720	11,389	0.55	C	1,080	347	708	0.98	C	
2170	224		County	35	0.35	JAWARAY ROAD	DAVID WALKER DRIVE	DAVID WALKER DRIVE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF EUSTIS	D	10,360	1,169	0.11	C	530	73	0.53	0.15	1.00%	10,360	1,229	0.12	C	530	83	56	0.16	C	
2180	26		County	35	1.57	JOHNS LAKE ROAD	CR 479A	CR 479A	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	10,360	6,226	0.60	D	530	273	2.22	0.92	8.50%	10,360	9,362	0.90	D	530	431	333	0.78	D	
2190	473		County	35	0.25	KURT STREET	W. LAKEVIEW AVENUE	W. LAKEVIEW AVENUE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF CLEMONT	D	14,060	8,972	0.84	D	710	566	4.03	0.84	2.75%	10,360	10,276	0.73	D	710	663	461	0.96	D	
2200	469		County	35	0.50	KURT STREET	DAVID WALKER DRIVE	DAVID WALKER DRIVE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF EUSTIS	D	10,360	8,710	0.84	D	530	546	2.92	1.03	1.00%	10,360	9,154	0.88	D	530	574	307	1.08	F	
2205	455		County	35	0.42	KURT STREET	MT HOMER ROAD / W. ARDICE AVENUE	MT HOMER ROAD / W. ARDICE AVENUE	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF EUSTIS	D	14,060	4,334	0.39	C	710	179	246	0.35	1.00%	14,060	4,555	0.42	C	710	186	259	0.36	C	
2210	520		County	25	0.45	W. LADY LAKE BOULEVARD	WEST TERMINI	US 271/US441	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF EUSTIS	D	10,360	5,512	0.39	C	530	261	281	0.40	1.00%	10,360	5,793	0.41	C	530	275	295	0.42	C	
2220	521		County	25	0.86	E LADY LAKE BOULEVARD	US 271/US41	BERCHFIELD ROAD	2	2	URBAN	UNDIVIDED	COUNTY	TOWN OF LADY LAKE	D	10,360	1,298	0.13	C	530	66	39	0.12	1.00%	10,360	1,364	0.13	C	530	69	41	0.13	C	
2230	408		County	35	0.56	FAIRVIEW AVENUE	US 271/US41	E LADY LAKE BOULEVARD	2	2	URBAN	UNDIVIDED	COUNTY	TOWN OF LADY LAKE	D	10,360	618	0.06	C	530	38	38	0.21	1.00%	10,360	650	0.06	C	530	40	22	0.08	C	
2240	0		NO COUNTY	40	0.64	LAKE DRIVE	LAKE DRIVE	COUNTRY ROAD	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	10,360	680	0.07	C	530	43	24	0.08	1.00%	10,360	715	0.07	C	530	45	25	0.08	C	
2250	509		County	35	0.50	LAKE ELIA ROAD	SUMTER COUNTY LINE	LAKE ELIA ROAD	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	9,030	-	-	-	470	-	-	-	N/A	9,030	-	-	-	-	-	-	-	-	-
2254	511		ADJACENT	35	0.51	LAKE ELIA ROAD	MICRO RACETRACK ROAD	MICRO RACETRACK ROAD	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	10,360	2,817	0.17	C	530	175	101	0.33	6.50%	10,360	3,859	0.37	C	530	239	138	0.45	C	
2260	511		County	45	1.81	LAKE ELIA ROAD	ROLLING ACRES ROAD	ROLLING ACRES ROAD	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	16,820	1,652	0.10	C	840	66	71	0.88	1.00%	16,820	1,737	0.17	C	840	70	75	0.14	C	
2265	7		County	35	5.01	LAKE ERIE ROAD	LAKE ERIE ROAD	LAKE ERIE ROAD	2	2	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	7,740	884	0.11	B	410	23	47	0.12	5.00%	7,740	1,128	0.10	B	410	29	61	0.15	B	
2270	448		County	35	1.59	LAKE EUSTIS DRIVE	CLAY BOULEVARD	CLAY BOULEVARD	2	2	URBAN	UNDIVIDED	COUNTY	EUSTIS/FAVARES	D	14,060	7,262	0.52	D	1,080	330	285	0.46	3.75%	14,060	7,600	0.56	D	1,080	360	311	0.51	D	
2280	19		County	40	2.57	LAKE LOUISA ROAD	VISTA DEL LAZO BOULEVARD	VISTA DEL LAZO BOULEVARD	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	21,760	5,071	0.23	B	1,080	266	545	0.50	3.75%	21,760	6,086	0.28	B	1,080	320	655	0.81	C	
2290	9		County	35	1.13	LAKE LOUISA DRIVE	US 27	LAKE LOUISA ROAD	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	14,060	5,377	0.38	C	710	512	710	0.72	4.50%	14,060	6,701	0.48	C	710	639	338	0.90	D	
2300	802		County	25	1.10	LAKE MAX DRIVE	CR 42	ANOTHER ANNA ROAD	2	2	RURAL	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	9,030	1,515	0.17	C	470	45	104	0.22	1.00%	9,030	1,592	0.18	C	470	109	0.23	C		
2310	435		County	25	0.20	LAKE STREET	LAKE STREET	LAKE STREET	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF LEESBURG	D	10,360	3,074	0.30	C	530	115	140	0.26	1.00%	10,360	3,231	0.31	C	530	121	147	0.28	C	
2320	428		County	45	0.54	LAKE STREET (CLER)	LAKE STREET	LAKE STREET	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF LEESBURG	D	10,360	3,074	0.30	C	530	115	140	0.26	1.00%	10,360	3,231	0.31	C	530	121	147	0.28	C	
2330	428		County	45	0.54	LAKE STREET (CLER)	LAKE STREET	LAKE STREET	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF LEESBURG	D	10,360	3,074	0.30	C	530	115	140	0.26	1.00%	10,360	3,231	0.31	C	530	121	147	0.28	C	
2340	14		County	45	1.62	LAKESHORE DRIVE (CLER)	OSWALT ROAD	OSWALT ROAD	2	2	TRANS.	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	15,360	11,219	0.78	C	790	242	142	0.29	1.00%	15,360	12,560	0.81	C	790	149	144	0.27	C	
2350	22		County	40	0.87	LAKESHORE DRIVE (CLER)	HARGER ROAD	LAKE LOUISA ROAD	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	15,360	11,219	0.78	C	790	242	142	0.29	1.00%	15,360	12,560	0.81	C	790	149	144	0.27	C	
2360	23		County	35	0.75	LAKESHORE DRIVE (CLER)	LAKE LOUISA ROAD	LAKE LOUISA ROAD	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	15,360	16,407	0.98	D	840	514	632	1.11	1.00%	15,360	17,244	1.03	F	840	543	580	1.17	F	
2380	484		County	35	1.65	LAKESHORE DRIVE (EUSTIS)	ANDERSON HILL ROAD	ANDERSON HILL ROAD	2	2	URBAN	UNDIVIDED	COUNTY	UNINCORPORATED LAKE COUNTY	D	13,320	6,987	0.52	D	680	433	296	0.64	2.75%	10,360	7,453	0.55	D	680	455	216	0.67	D	
2370	477		County	35	0.43	W. LAKEVIEW AVENUE	SR 19	SR 19	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF EUSTIS	D	10,360	7,701	0.74	D	530	356	294	0.67	2.75%	10,360	8,196	0.79	D	530	407	337	0.77	D	
2380	477		County	30	0.65	E. LAKEVIEW AVENUE	CR 468A	CR 468A	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF EUSTIS	D	10,360	2,689	0.26	C	530	180	206	0.39	1.00%	10,360	3,392	0.33	C	530	159	216	0.41	C	
2394	477		ADJACENT	30	0.34	E. LAKEVIEW AVENUE	JASMINE STREET / CROOKED LAKE COURT	JASMINE STREET / CROOKED LAKE COURT	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF EUSTIS	D	10,360	2,689	0.26	C	530	180	206	0.39	1.00%	10,360	3,392	0.33	C	530	159	216	0.41	C	
2390	271		County	35	0.62	LAKE PARK CUTOFF	SR 19A	HASELTON STREET	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF FAVARES	D	13,320	2,071	0.16	C	660	212	129	0.34	1.00%	13,320	2,177	0.16	C	660	223	133	0.36	C	
2400	441		County	25	0.74	LEE STREET	GRiffin ROAD	GRiffin ROAD	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF LEESBURG	D	10,360	2,244	0.22	C	530	232	129	0.34	1.00%	10,360	2,358	0.23	C	530	232	129	0.36	C	
2410	438		County	25	0.50	LEE STREET	MAIN STREET	MAIN STREET	2	2	URBAN	UNDIVIDED	COUNTY	CITY OF LEESBURG	D	10,360	2,533	0.24	C	530	-	-	-	-	1.00%	10,360	2,662	0.26	C	530	-	-	-	-
2420	239		County	40	0.35																													

APPENDIX C

Intersection Counts/FDOT Seasonal Factors/Signal Timings

15 MINUTE TURNING MOVEMENT COUNTS

(Cars and Trucks)

DATE: May 24, 2022 (Tuesday)

CITY: Howie in the Hills

LATITUDE: 0

LOCATION: Number 2 & CR 48

COUNTY: Lake County

LONGITUDE: 0

Number 2						Number 2						CR 48						CR 48							
TIME BEGIN	NORTHBOUND					SOUTHBOUND					N/S TOTAL	EASTBOUND					WESTBOUND					E/W TOTAL	GRAND TOTAL		
	L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL		L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL				
07:00 AM	3	0	0	0	3	2	0	2	0	4	7	1	58	2	0	61	0	92	0	0	92	153	160		
07:15 AM	4	0	1	0	5	1	0	1	0	2	7	0	82	2	0	84	0	90	0	0	90	174	181		
07:30 AM	7	0	0	0	7	0	0	0	0	0	7	0	70	0	0	70	0	92	0	0	92	162	169		
07:45 AM	10	0	0	0	10	1	0	0	0	1	11	1	97	2	0	100	1	88	0	0	89	189	200		
TOTAL	24	0	1	0	25	4	0	3	0	7	32	2	307	6	0	315	1	362	0	0	363	678	710		
08:00 AM	3	0	3	0	6	0	0	0	0	0	6	2	62	5	0	69	2	80	0	0	82	151	157		
08:15 AM	4	0	0	0	4	0	0	0	0	0	4	0	74	7	0	81	0	91	0	0	91	172	176		
08:30 AM	5	0	1	0	6	1	1	0	0	2	8	2	68	1	0	71	1	70	0	0	71	142	150		
08:45 AM	1	0	1	0	2	0	0	1	0	1	3	0	85	1	0	86	0	75	1	0	76	162	165		
TOTAL	13	0	5	0	18	1	1	1	0	3	21	4	289	14	0	307	3	316	1	0	320	627	648		
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5	5	5		
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
09:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
09:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5	5	5		
04:00 PM	0	0	0	0	0	1	0	0	0	1	1	0	75	1	0	76	1	91	0	0	92	168	169		
04:15 PM	3	0	0	0	3	1	0	1	0	2	5	0	91	3	0	94	0	85	0	0	85	179	184		
04:30 PM	4	0	1	0	5	1	0	2	0	3	8	1	108	1	0	110	2	106	2	0	110	220	228		
04:45 PM	3	0	0	0	3	1	0	1	0	2	5	0	106	4	0	110	2	92	0	0	94	204	209		
TOTAL	10	0	1	0	11	4	0	4	0	8	19	1	380	9	0	390	5	374	2	0	381	771	790		
05:00 PM	3	0	0	0	3	1	0	0	0	1	4	1	95	8	0	104	2	85	1	0	88	192	196		
05:15 PM	3	0	0	0	3	2	0	0	0	2	5	1	116	5	0	122	0	101	1	0	102	224	229		
05:30 PM	6	0	3	0	9	0	0	0	0	0	9	1	73	6	0	80	3	89	1	0	93	173	182		
05:45 PM	5	0	0	0	5	0	0	2	0	2	7	2	114	6	0	122	0	78	0	0	78	200	207		
TOTAL	17	0	3	0	20	3	0	2	0	5	25	5	398	25	0	428	5	353	3	0	361	789	814		

AM Peak

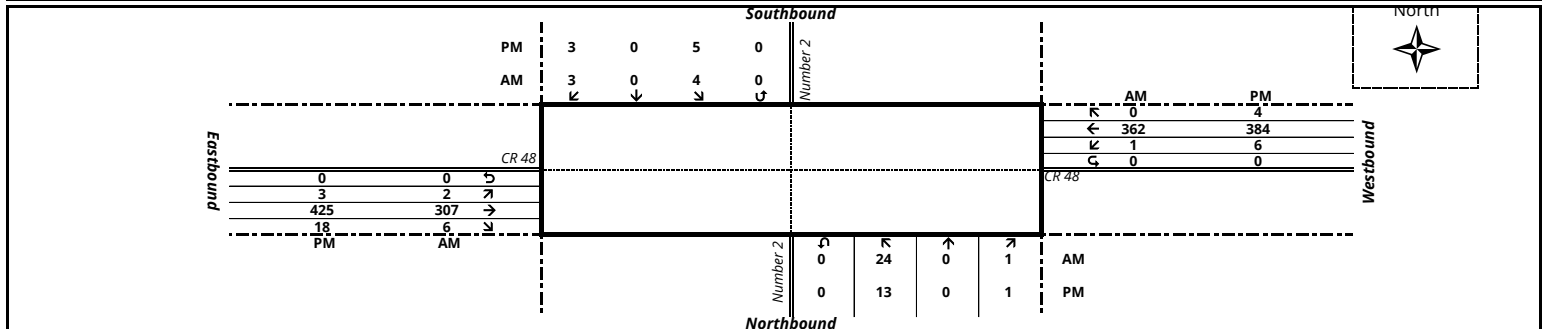
Peak Hour Factor: 0.888

07:00 AM to 08:00 AM	24	0	1	0	25	4	0	3	0	7	32	2	307	6	0	315	1	362	0	0	363	678	710
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PM Peak

Peak Hour Factor: 0.941

04:30 PM to 05:30 PM	13	0	1	0	14	5	0	3	0	8	22	3	425	18	0	446	6	384	4	0	394	840	862
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15 MINUTE TURNING MOVEMENT COUNTS

(Trucks Only)

DATE: May 24, 2022 (Tuesday)

CITY: Howie in the Hills LATITUDE: 0

LOCATION: Number 2 & CR 48

COUNTY: Lake County LONGITUDE: 0

Number 2						Number 2						CR 48					CR 48						
TIME BEGIN	NORTHBOUND					SOUTHBOUND					N/S TOTAL	EASTBOUND					WESTBOUND					E/W TOTAL	GRAND TOTAL
	L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL		L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL		
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5	0	8	0	0	8	13	13
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	6	0	0	6	0	8	0	0	8	14	14
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	11	0	0	11	0	4	0	0	4	15	15
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	12	0	0	12	0	6	0	0	6	18	18
TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	34	0	0	34	0	26	0	0	26	60	60
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	4	1	0	5	0	9	0	0	9	14	14
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	7	0	0	7	0	12	0	0	12	19	19
08:30 AM	1	0	0	0	1	0	0	0	0	0	1	0	5	1	0	6	0	5	0	0	5	11	12
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3	0	7	0	0	7	10	10
TOTAL	1	0	0	0	1	0	0	0	0	0	1	0	19	2	0	21	0	33	0	0	33	54	55
09:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
09:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	0	3	0	0	3	5	5
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	0	2	0	0	2	4	4
04:30 PM	1	0	0	0	1	0	0	0	0	0	1	0	5	0	0	5	0	4	0	0	4	9	10
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	5	0	0	5	0	5	0	0	5	10	10
TOTAL	1	0	0	0	1	0	0	0	0	0	1	0	14	0	0	14	0	14	0	0	14	28	29
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	2	0	4	0	0	4	6	6
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	0	4	0	0	4	6	6
05:30 PM	1	0	0	0	1	0	0	0	0	0	1	0	1	0	0	1	0	0	0	0	0	1	2
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	1	1
TOTAL	1	0	0	0	1	0	0	0	0	0	1	0	5	1	0	6	0	8	0	0	8	14	15
AM Peak																							
07:00 AM to 08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	34	0	0	34	0	26	0	0	26	60	60
PM Peak																							
04:30 PM to 05:30 PM	1	0	0	0	1	0	0	0	0	0	1	0	13	1	0	14	0	17	0	0	17	31	32

← CB-48

1/2 mi Stone Mountain Rd →

← Number 2 Rd

↑
N

5A-48

1/2 mi

15 MINUTE TURNING MOVEMENT COUNTS

(Cars and Trucks)

DATE: May 19, 2022 (Thursday)

CITY: Howie in the Hills

LATITUDE: 0

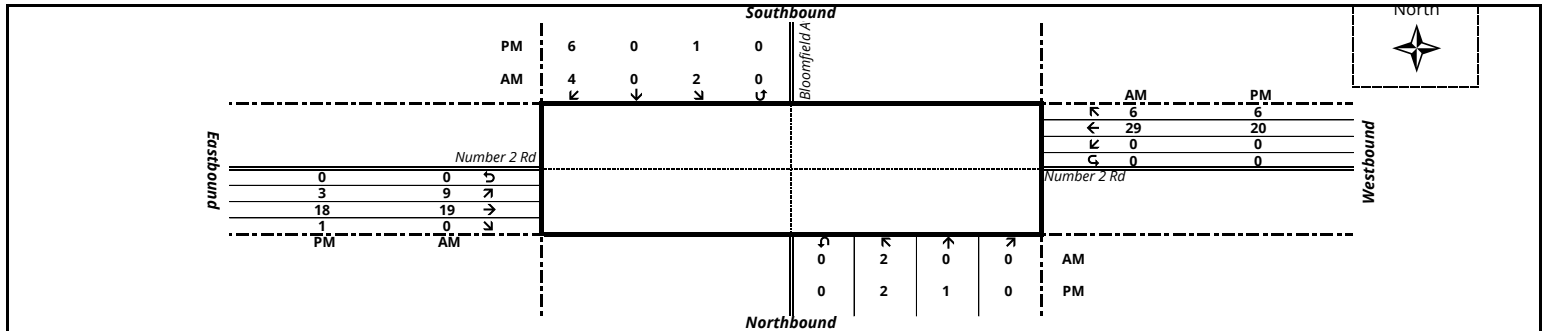
LOCATION: Bloomfield Av & Number 2 Rd

COUNTY: Lake County

LONGITUDE: 0

						Bloomfield Av					Number 2 Rd					Number 2 Rd									
TIME BEGIN	NORTHBOUND					SOUTHBOUND					N/S	EASTBOUND					WESTBOUND					E/W TOTAL	GRAND TOTAL		
	L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL		L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL				
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2	0	6	2	0	8	10	10		
07:15 AM	1	0	0	0	1	0	0	1	0	1	2	2	3	0	0	5	0	2	0	0	2	7	9		
07:30 AM	0	0	0	0	0	1	0	0	0	1	1	0	7	0	0	7	0	4	1	0	5	12	13		
07:45 AM	0	0	0	0	0	0	0	1	0	1	1	1	2	0	0	3	0	3	0	0	3	6	7		
TOTAL	1	0	0	0	1	1	0	2	0	3	4	5	12	0	0	17	0	15	3	0	18	35	39		
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	2	0	3	1	0	4	6	6		
08:15 AM	0	0	0	0	0	0	0	1	0	1	1	2	1	0	0	3	0	6	2	0	8	11	12		
08:30 AM	1	0	0	0	1	1	0	1	0	2	3	1	5	0	0	6	0	5	0	0	5	11	14		
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	2	2	0	0	4	0	2	0	0	2	6	6		
TOTAL	1	0	0	0	1	1	0	2	0	3	4	6	9	0	0	15	0	16	3	0	19	34	38		
04:00 PM	0	0	0	0	0	1	0	0	0	1	1	0	4	0	0	4	0	4	0	0	4	8	9		
04:15 PM	0	0	0	0	0	0	0	1	0	1	1	1	1	1	0	3	0	4	0	0	4	7	8		
04:30 PM	1	0	0	0	1	0	0	1	0	1	2	1	5	0	0	6	0	5	2	0	7	13	15		
04:45 PM	1	0	0	0	1	0	0	2	0	2	3	1	2	0	0	3	0	0	3	0	3	6	9		
TOTAL	2	0	0	0	2	1	0	4	0	5	7	3	12	1	0	16	0	13	5	0	18	34	41		
05:00 PM	0	0	0	0	0	0	0	1	0	1	1	0	4	0	0	4	0	2	1	0	3	7	8		
05:15 PM	0	1	0	0	1	0	0	1	0	1	2	0	2	0	0	2	0	5	0	0	5	7	9		
05:30 PM	0	0	0	0	0	2	0	1	1	4	4	1	4	1	0	6	0	1	0	0	1	7	11		
05:45 PM	0	0	0	0	0	0	0	1	0	1	1	2	4	0	0	6	0	2	0	0	2	8	9		
TOTAL	0	1	0	0	1	2	0	4	1	7	8	3	14	1	0	18	0	10	1	0	11	29	37		

AM Peak																			Peak Hour Factor: 1.268					
07:00 AM to 08:00 AM		2	0	0	0	2	2	0	4	0	6	8	9	19	0	0	28	0	29	6	0	35	63	71
PM Peak																			Peak Hour Factor: 0.967					
04:00 PM to 05:00 PM		2	1	0	0	3	1	0	6	0	7	10	3	18	1	0	22	0	20	6	0	26	48	58



15 MINUTE TURNING MOVEMENT COUNTS

(Trucks Only)

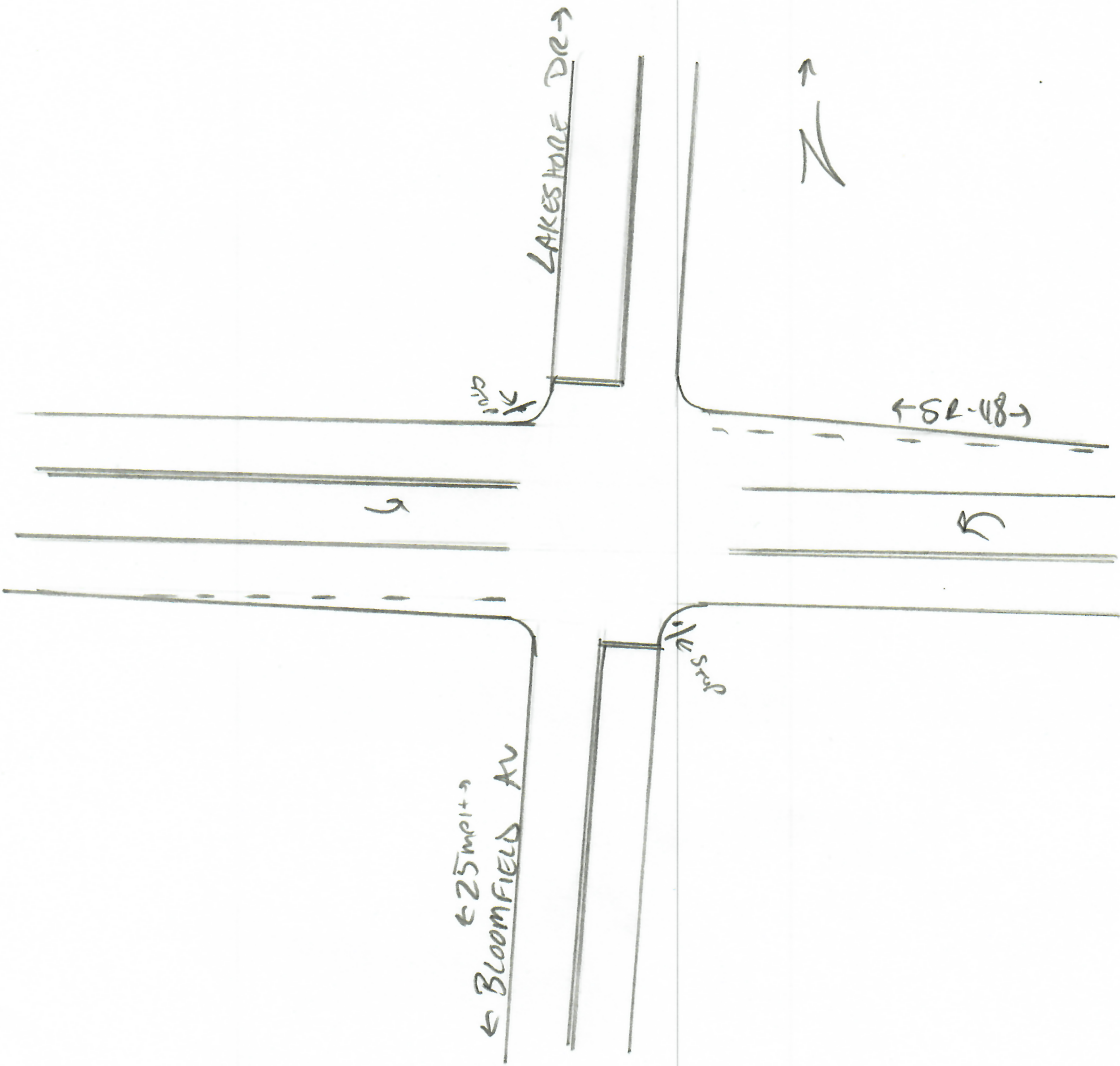
DATE: May 19, 2022 (Thursday)

CITY: Howie in the Hills LATITUDE: 0

LOCATION: Bloomfield Av & Number 2 Rd

COUNTY: Lake County LONGITUDE: 0

						Bloomfield Av						Number 2 Rd						Number 2 Rd							
TIME BEGIN	NORTHBOUND					SOUTHBOUND					N/S TOTAL	EASTBOUND					WESTBOUND					E/W TOTAL	GRAND TOTAL		
	L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL		L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL				
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
07:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
07:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	1	1		
TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	1	1		
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1	1		
08:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1	1		
TOTAL	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	1	0	0	1	2	2		
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
04:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1	1		
TOTAL	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1	1		
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
05:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
05:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
AM Peak																									
07:00 AM to 08:00 AM	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	2	0	0	0	0	0	2	2		
PM Peak																									
04:00 PM to 05:00 PM	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1	1		



15 MINUTE TURNING MOVEMENT COUNTS

(Cars and Trucks)

DATE: May 19, 2022 (Thursday)

CITY: Howie in the Hills

LATITUDE: 0

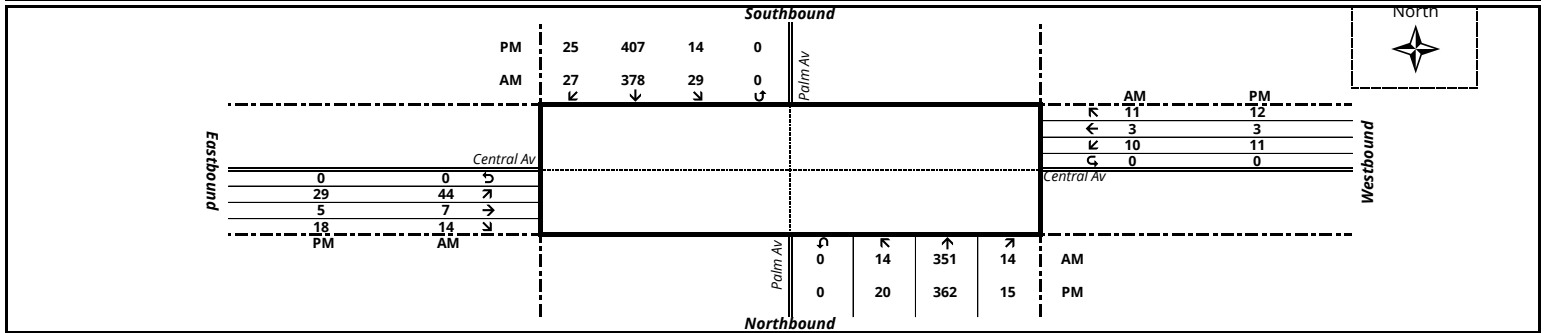
LOCATION: Palm Av & Central Av

COUNTY: Lake County

LONGITUDE: 0

	Palm Av					Palm Av					N/S	Central Av					Central Av					E/W	GRAND
TIME BEGIN	NORTHBOUND					SOUTHBOUND					TOTAL	EASTBOUND					WESTBOUND					TOTAL	TOTAL
	L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL		L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL		
07:00 AM	2	92	1	0	95	7	103	3	0	113	208	17	3	8	0	28	2	1	2	0	5	33	241
07:15 AM	3	84	2	0	89	4	74	6	0	84	173	8	2	1	0	11	4	1	3	0	8	19	192
07:30 AM	2	105	5	0	112	3	122	8	0	133	245	8	1	1	0	10	1	0	3	0	4	14	259
07:45 AM	7	70	6	0	83	15	79	10	0	104	187	11	1	4	0	16	3	1	3	0	7	23	210
TOTAL	14	351	14	0	379	29	378	27	0	434	813	44	7	14	0	65	10	3	11	0	24	89	902
08:00 AM	3	79	4	0	86	6	93	9	0	108	194	5	1	2	0	8	1	1	4	0	6	14	208
08:15 AM	2	75	6	0	83	9	66	6	0	81	164	2	0	1	0	3	5	1	6	0	12	15	179
08:30 AM	2	84	4	0	90	3	62	5	0	70	160	5	0	5	0	10	1	0	8	0	9	19	179
08:45 AM	2	77	5	0	84	4	58	6	0	68	152	4	1	4	0	9	3	1	7	0	11	20	172
TOTAL	9	315	19	0	343	22	279	26	0	327	670	16	2	12	0	30	10	3	25	0	38	68	738
04:00 PM	4	97	4	0	105	6	98	6	0	110	215	4	2	3	0	9	3	1	6	0	10	19	234
04:15 PM	3	81	5	0	89	2	88	9	0	99	188	13	0	1	0	14	5	0	3	0	8	22	210
04:30 PM	7	90	0	0	97	3	96	8	0	107	204	4	2	2	0	8	2	0	1	0	3	11	215
04:45 PM	2	99	4	0	105	6	86	7	0	99	204	10	0	2	0	12	3	2	3	0	8	20	224
TOTAL	16	367	13	0	396	17	368	30	0	415	811	31	4	8	0	43	13	3	13	0	29	72	883
05:00 PM	5	79	0	0	84	3	92	6	0	101	185	6	2	10	0	18	3	0	7	0	10	28	213
05:15 PM	6	94	11	0	111	2	133	4	0	139	250	9	1	4	0	14	3	1	1	0	5	19	269
05:30 PM	1	69	5	0	75	5	74	13	0	92	167	3	2	1	0	6	2	1	1	0	4	10	177
05:45 PM	2	88	2	0	92	1	85	8	0	94	186	6	0	1	0	7	2	1	1	0	4	11	197
TOTAL	14	330	18	0	362	11	384	31	0	426	788	24	5	16	0	45	10	3	10	0	23	68	856

AM Peak																					Peak Hour Factor: 0.871		
07:00 AM to 08:00 AM	14	351	14	0	379	29	378	27	0	434	813	44	7	14	0	65	10	3	11	0	24	89	902
PM Peak																					Peak Hour Factor: 0.856		
04:30 PM to 05:30 PM	20	362	15	0	397	14	407	25	0	446	843	29	5	18	0	52	11	3	12	0	26	78	921



15 MINUTE TURNING MOVEMENT COUNTS

(Trucks Only)

DATE: May 19, 2022 (Thursday)

CITY: Howie in the Hills

LATITUDE: 0

LOCATION: Palm Av & Central Av

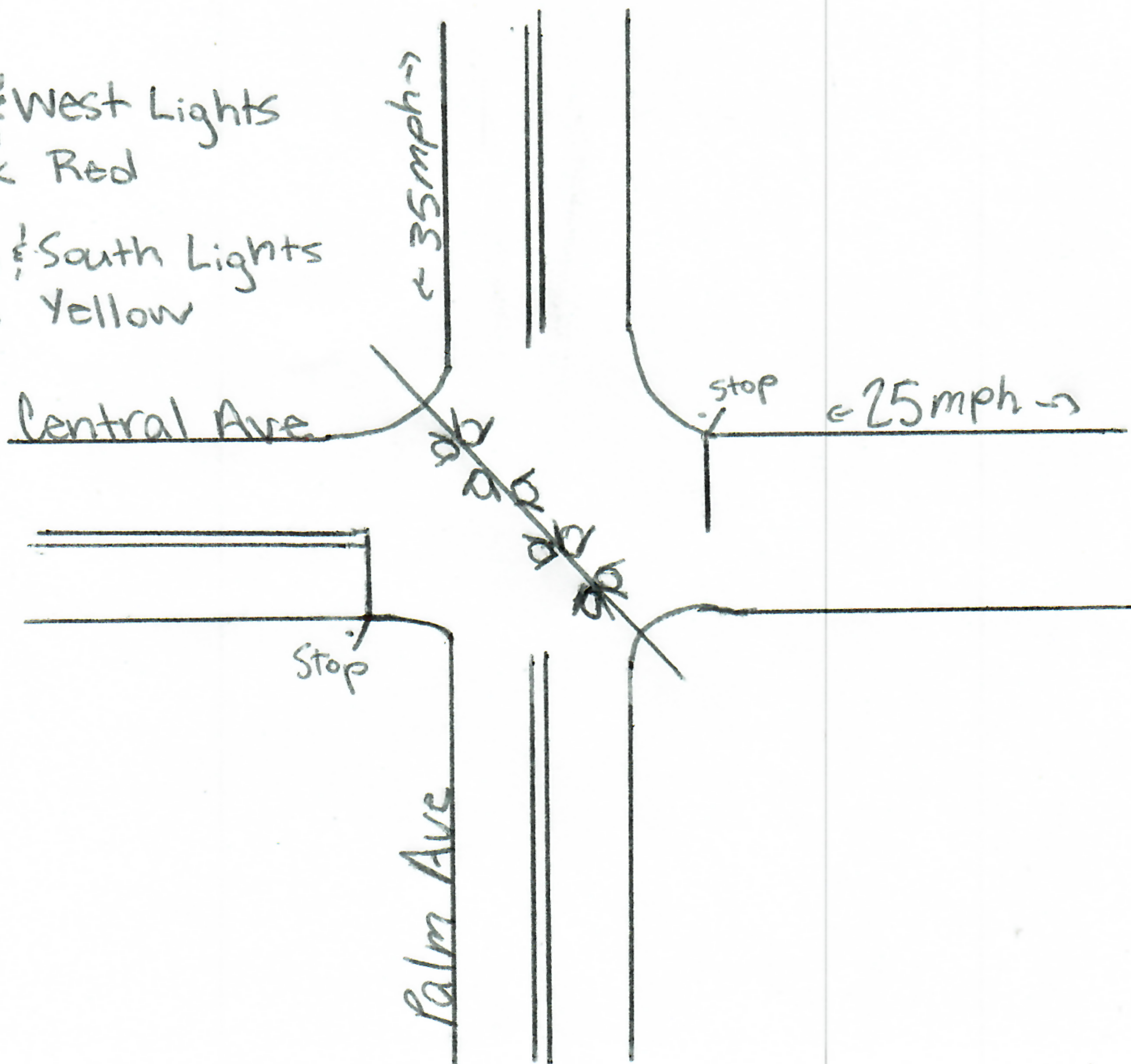
COUNTY: Lake County

LONGITUDE: 0

Palm Av						Palm Av						Central Av						Central Av						E/W TOTAL	GRAND TOTAL
TIME BEGIN	NORTHBOUND					SOUTHBOUND					N/S TOTAL	EASTBOUND					WESTBOUND								
	L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL		L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL				
07:00 AM	0	6	0	0	6	0	10	0	0	10	16	0	0	2	0	2	0	0	0	0	0	2	18		
07:15 AM	1	4	1	0	6	0	5	0	0	5	11	0	0	0	0	0	0	0	0	0	0	0	11		
07:30 AM	0	10	0	0	10	1	5	1	0	7	17	0	0	0	0	0	0	0	0	0	0	0	17		
07:45 AM	2	7	0	0	9	1	2	1	0	4	13	0	0	0	0	0	0	0	0	0	0	0	13		
TOTAL	3	27	1	0	31	2	22	2	0	26	57	0	0	2	0	2	0	0	0	0	0	2	59		
08:00 AM	0	8	0	0	8	3	6	1	0	10	18	0	0	1	0	1	0	0	0	0	0	1	19		
08:15 AM	0	8	0	0	8	1	5	0	0	6	14	0	0	0	0	0	0	0	2	0	2	2	16		
08:30 AM	0	7	0	0	7	0	5	0	0	5	12	0	0	1	0	1	0	0	3	0	3	4	16		
08:45 AM	0	5	0	0	5	0	6	0	0	6	11	0	0	1	0	1	0	0	1	0	1	2	13		
TOTAL	0	28	0	0	28	4	22	1	0	27	55	0	0	3	0	3	0	0	6	0	6	9	64		
04:00 PM	1	6	0	0	7	0	2	0	0	2	9	1	1	0	0	2	0	0	0	0	0	2	11		
04:15 PM	0	3	0	0	3	0	3	0	0	3	6	0	0	1	0	1	0	0	0	0	0	1	7		
04:30 PM	4	4	0	0	8	0	7	0	0	7	15	0	0	0	0	0	0	0	0	0	0	0	15		
04:45 PM	0	2	0	0	2	0	2	1	0	3	5	0	0	1	0	1	0	0	0	0	0	1	6		
TOTAL	5	15	0	0	20	0	14	1	0	15	35	1	1	2	0	4	0	0	0	0	0	4	39		
05:00 PM	2	1	0	0	3	0	2	0	0	2	5	1	0	4	0	5	0	0	0	0	0	5	10		
05:15 PM	1	3	0	0	4	0	1	0	0	1	5	0	0	0	0	0	0	0	0	0	0	0	5		
05:30 PM	0	1	0	0	1	0	2	0	0	2	3	0	0	1	0	1	0	0	0	0	0	1	4		
05:45 PM	1	3	0	0	4	0	8	0	0	8	12	1	0	0	0	1	0	0	0	0	0	1	13		
TOTAL	4	8	0	0	12	0	13	0	0	13	25	2	0	5	0	7	0	0	0	0	0	7	32		
AM Peak																									
07:00 AM to 08:00 AM		3	27	1	0	31	2	22	2	0	26	57	0	0	2	0	2	0	0	0	0	2	59		
PM Peak																									
04:30 PM to 05:30 PM		7	10	0	0	17	0	12	1	0	13	30	1	0	5	0	6	0	0	0	0	6	36		



East & West Lights
Blink Red
North & South Lights
Blink Yellow



15 MINUTE TURNING MOVEMENT COUNTS

(Cars and Trucks)

DATE: January 13, 2022 (Thursday)

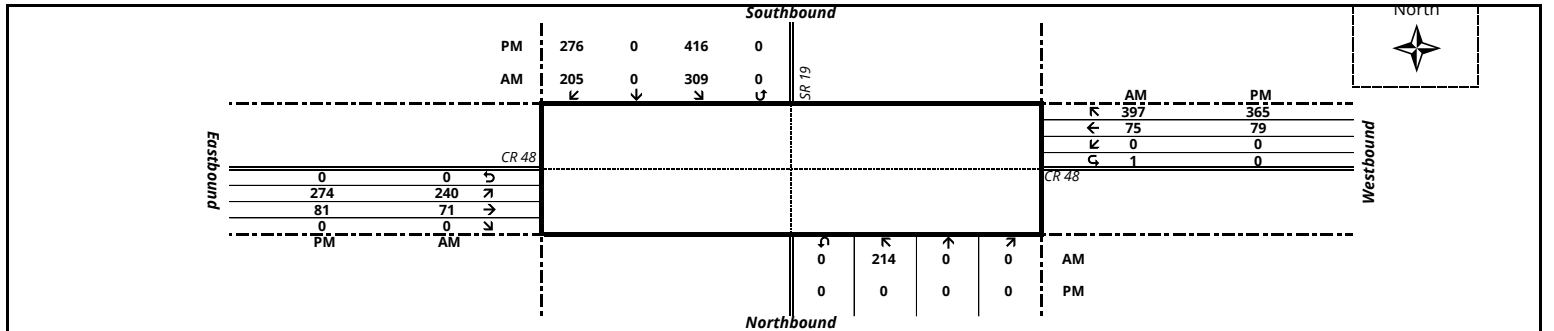
CITY: Howie in the Hills LATITUDE: 0

LOCATION: SR 19 & CR 48

COUNTY: Lake County LONGITUDE: 0

	SR 19										CR 48					CR 48							
TIME BEGIN	NORTHBOUND					SOUTHBOUND					N/S TOTAL	EASTBOUND					WESTBOUND					E/W TOTAL	GRAND TOTAL
	L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL		L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL		
07:00 AM	47	0	0	0	47	62	0	34	0	96	143	67	16	0	0	83	0	8	62	1	71	154	297
07:15 AM	70	0	0	0	70	83	0	49	0	132	202	78	17	0	0	95	0	15	110	1	126	221	423
07:30 AM	53	0	0	0	53	94	0	44	0	138	191	59	16	0	0	75	0	25	108	0	133	208	399
07:45 AM	48	0	0	0	48	62	0	55	0	117	165	53	18	0	0	71	0	19	87	0	106	177	342
TOTAL	218	0	0	0	218	301	0	182	0	483	701	257	67	0	0	324	0	67	367	2	436	760	1,461
08:00 AM	43	0	0	0	43	70	0	57	0	127	170	50	20	0	0	70	0	16	92	0	108	178	348
08:15 AM	55	0	0	0	55	69	0	67	0	136	191	58	16	0	0	74	0	31	82	0	113	187	378
08:30 AM	66	0	0	0	66	54	0	54	0	108	174	71	15	0	0	86	0	24	84	1	109	195	369
08:45 AM	50	0	0	0	50	56	0	66	0	122	172	56	17	0	0	73	0	16	63	0	79	152	324
TOTAL	214	0	0	0	214	249	0	244	0	493	707	235	68	0	0	303	0	87	321	1	409	712	1,419
04:00 PM	0	0	0	0	0	106	0	68	0	174	174	81	13	0	0	94	0	19	86	0	105	199	373
04:15 PM	0	0	0	0	0	83	0	76	0	159	159	72	22	0	0	94	0	16	88	0	104	198	357
04:30 PM	0	0	0	0	0	91	0	61	0	152	152	76	22	0	0	98	0	19	84	0	103	201	353
04:45 PM	0	0	0	0	0	101	0	64	0	165	165	70	19	0	0	89	0	24	99	0	123	212	377
TOTAL	0	0	0	0	0	381	0	269	0	650	650	299	76	0	0	375	0	78	357	0	435	810	1,460
05:00 PM	0	0	0	0	0	108	0	77	0	185	185	74	20	0	0	94	0	12	84	0	96	190	375
05:15 PM	0	0	0	0	0	96	0	74	0	170	170	75	16	0	0	91	0	26	98	0	124	215	385
05:30 PM	0	0	0	0	0	111	0	61	0	172	172	55	26	0	0	81	0	17	84	0	101	182	354
05:45 PM	0	0	0	0	0	85	0	65	0	150	150	79	20	0	0	99	0	15	66	0	81	180	330
TOTAL	0	0	0	0	0	400	0	277	0	677	677	283	82	0	0	365	0	70	332	0	402	767	1,444

AM Peak																			Peak Hour Factor: 0.894					
07:15 AM to 08:15 AM		214	0	0	0	214	309	0	205	0	514	728	240	71	0	0	311	0	75	397	1	473	784	1,512
PM Peak																			Peak Hour Factor: 0.968					
04:45 PM to 05:45 PM		0	0	0	0	0	416	0	276	0	692	692	274	81	0	0	355	0	79	365	0	444	799	1,491



15 MINUTE TURNING MOVEMENT COUNTS

(Trucks Only)

DATE: January 13, 2022 (Thursday)

CITY: Howie in the Hills

LATITUDE: 0

LOCATION: SR 19 & CR 48

COUNTY: Lake County

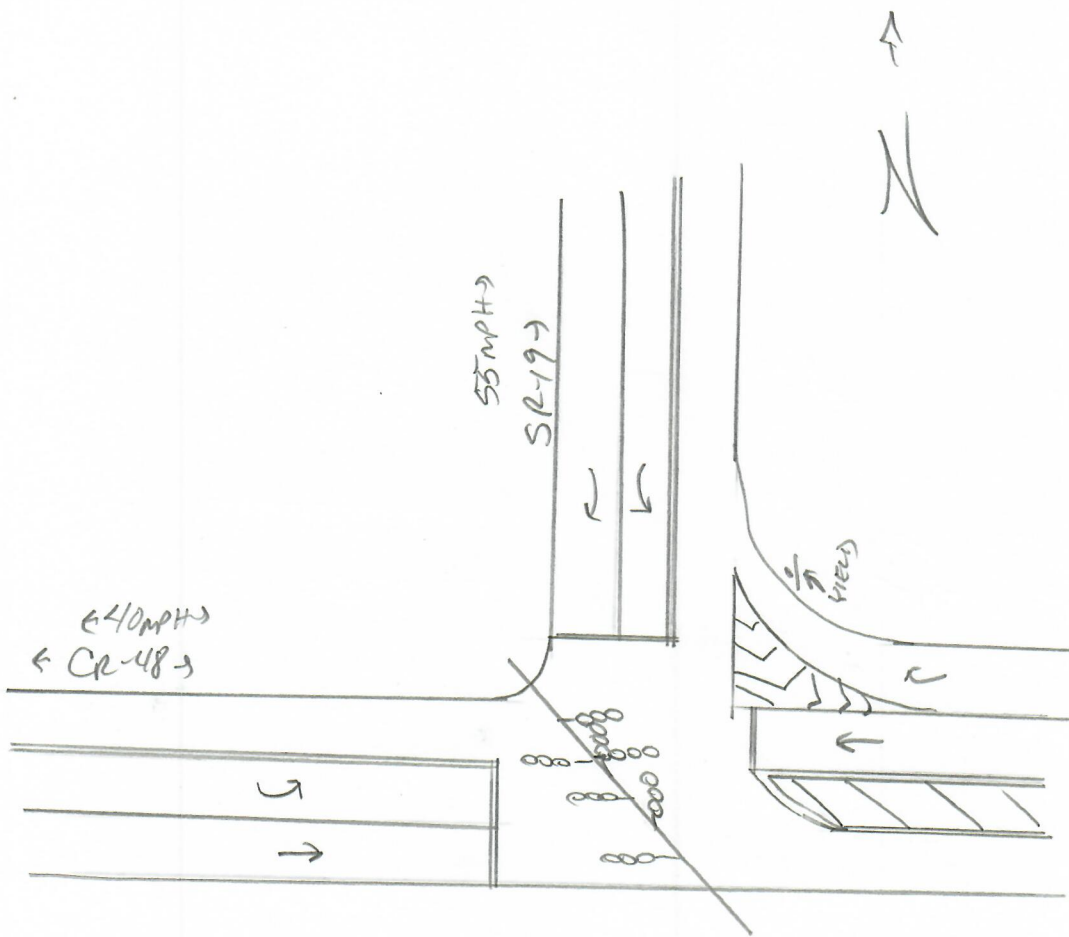
LONGITUDE: 0

						SR 19						CR 48						CR 48							
TIME BEGIN	NORTHBOUND					SOUTHBOUND					N/S TOTAL	EASTBOUND					WESTBOUND					E/W TOTAL	GRAND TOTAL		
	L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL		L	T	R	U-turn	TOTAL	L	T	R	U-turn	TOTAL				
07:00 AM	0	0	0	0	0	4	0	2	0	6	6	6	2	0	0	8	0	0	3	0	3	11	17		
07:15 AM	0	0	0	0	0	4	0	2	0	6	6	8	0	0	0	8	0	0	2	0	2	10	16		
07:30 AM	0	0	0	0	0	5	0	4	0	9	9	6	2	0	0	8	0	0	5	0	5	13	22		
07:45 AM	0	0	0	0	0	6	0	3	0	9	9	5	0	0	0	5	0	0	4	0	4	9	18		
TOTAL	0	0	0	0	0	19	0	11	0	30	30	25	4	0	0	29	0	0	14	0	14	43	73		
08:00 AM	0	0	0	0	0	4	0	7	0	11	11	7	6	0	0	13	0	2	4	0	6	19	30		
08:15 AM	0	0	0	0	0	11	0	11	0	22	22	3	1	0	0	4	0	4	6	0	10	14	36		
08:30 AM	0	0	0	0	0	6	0	8	0	14	14	5	0	0	0	5	0	2	7	0	9	14	28		
08:45 AM	0	0	0	0	0	7	0	7	0	14	14	6	0	0	0	6	0	1	5	0	6	12	26		
TOTAL	0	0	0	0	0	28	0	33	0	61	61	21	7	0	0	28	0	9	22	0	31	59	120		
04:00 PM	0	0	0	0	0	2	0	1	0	3	3	5	0	0	0	5	0	0	7	0	7	12	15		
04:15 PM	0	0	0	0	0	5	0	3	0	8	8	4	1	0	0	5	0	0	3	0	3	8	16		
04:30 PM	0	0	0	0	0	2	0	3	0	5	5	6	0	0	0	6	0	0	0	0	0	6	11		
04:45 PM	0	0	0	0	0	1	0	2	0	3	3	2	0	0	0	2	0	1	1	0	2	4	7		
TOTAL	0	0	0	0	0	10	0	9	0	19	19	17	1	0	0	18	0	1	11	0	12	30	49		
05:00 PM	0	0	0	0	0	3	0	0	0	3	3	0	0	0	0	0	0	0	2	0	2	2	5		
05:15 PM	0	0	0	0	0	9	0	2	0	11	11	1	0	0	0	1	0	1	3	0	4	5	16		
05:30 PM	0	0	0	0	0	2	0	2	0	4	4	2	0	0	0	2	0	0	1	0	1	3	7		
05:45 PM	0	0	0	0	0	4	0	1	0	5	5	2	0	0	0	2	0	0	3	0	3	5	10		
TOTAL	0	0	0	0	0	18	0	5	0	23	23	5	0	0	0	5	0	1	9	0	10	15	38		
AM Peak																									
07:15 AM to 08:15 AM	0	0	0	0	0	19	0	16	0	35	35	26	8	0	0	34	0	2	15	0	17	51	86		
PM Peak																									
04:45 PM to 05:45 PM	0	0	0	0	0	15	0	6	0	21	21	5	0	0	0	5	0	2	7	0	9	14	35		

(BANK 2 Only)

COUNTY: Lake County **LONGITUDE:** 0

[illegible]



2019 PEAK SEASON FACTOR CATEGORY REPORT - REPORT TYPE: ALL
 CATEGORY: 1100 LAKE COUNTYWIDE

WEEK	DATES	SF	MOCF: 0.94 PSCF
1	01/01/2019 - 01/05/2019	1.01	1.07
2	01/06/2019 - 01/12/2019	1.01	1.07
3	01/13/2019 - 01/19/2019	1.01	1.07
4	01/20/2019 - 01/26/2019	0.99	1.05
* 5	01/27/2019 - 02/02/2019	0.97	1.03
* 6	02/03/2019 - 02/09/2019	0.96	1.02
* 7	02/10/2019 - 02/16/2019	0.94	1.00
* 8	02/17/2019 - 02/23/2019	0.94	1.00
* 9	02/24/2019 - 03/02/2019	0.93	0.99
* 10	03/03/2019 - 03/09/2019	0.93	0.99
* 11	03/10/2019 - 03/16/2019	0.92	0.98
* 12	03/17/2019 - 03/23/2019	0.93	0.99
* 13	03/24/2019 - 03/30/2019	0.93	0.99
* 14	03/31/2019 - 04/06/2019	0.94	1.00
* 15	04/07/2019 - 04/13/2019	0.94	1.00
* 16	04/14/2019 - 04/20/2019	0.95	1.01
* 17	04/21/2019 - 04/27/2019	0.96	1.02
18	04/28/2019 - 05/04/2019	0.98	1.04
19	05/05/2019 - 05/11/2019	0.99	1.05
20	05/12/2019 - 05/18/2019	1.00	1.06
21	05/19/2019 - 05/25/2019	1.01	1.07
22	05/26/2019 - 06/01/2019	1.03	1.10
23	06/02/2019 - 06/08/2019	1.04	1.11
24	06/09/2019 - 06/15/2019	1.05	1.12
25	06/16/2019 - 06/22/2019	1.05	1.12
26	06/23/2019 - 06/29/2019	1.06	1.13
27	06/30/2019 - 07/06/2019	1.06	1.13
28	07/07/2019 - 07/13/2019	1.07	1.14
29	07/14/2019 - 07/20/2019	1.07	1.14
30	07/21/2019 - 07/27/2019	1.06	1.13
31	07/28/2019 - 08/03/2019	1.05	1.12
32	08/04/2019 - 08/10/2019	1.04	1.11
33	08/11/2019 - 08/17/2019	1.03	1.10
34	08/18/2019 - 08/24/2019	1.03	1.10
35	08/25/2019 - 08/31/2019	1.04	1.11
36	09/01/2019 - 09/07/2019	1.05	1.12
37	09/08/2019 - 09/14/2019	1.06	1.13
38	09/15/2019 - 09/21/2019	1.07	1.14
39	09/22/2019 - 09/28/2019	1.05	1.12
40	09/29/2019 - 10/05/2019	1.04	1.11
41	10/06/2019 - 10/12/2019	1.02	1.09
42	10/13/2019 - 10/19/2019	1.00	1.06
43	10/20/2019 - 10/26/2019	1.00	1.06
44	10/27/2019 - 11/02/2019	1.00	1.06
45	11/03/2019 - 11/09/2019	1.00	1.06
46	11/10/2019 - 11/16/2019	1.00	1.06
47	11/17/2019 - 11/23/2019	1.00	1.06
48	11/24/2019 - 11/30/2019	1.00	1.06
49	12/01/2019 - 12/07/2019	1.00	1.06
50	12/08/2019 - 12/14/2019	1.01	1.07
51	12/15/2019 - 12/21/2019	1.01	1.07
52	12/22/2019 - 12/28/2019	1.01	1.07
53	12/29/2019 - 12/31/2019	1.01	1.07

* PEAK SEASON

14-FEB-2020 15:39:28

830UPD

5_1100_PKSEASON.TXT

LAKE COUNTY - TRAFFIC SIGNAL OPERATIONS

CARTEGRAPH ID: LC-S-043**DATE: 05/15/2015****INTERSECTION NAME AND ID#: SR 19 & CR 48 076**

PHASE	1	2	3	4	5	6	7	8
	EBL	WB		SB		EB		
INITIAL	8	15		8		15		
PASSAGE	3	3		3		3		
YELLOW	4.4	4.4		4.8		4.4		
RED CLEAR	2.1	2.0		2.5		2.0		
MAX 1	25	45		30		45		
MAX 2								
WALK								
DON'T WALK								
RECALL				SOFT				
DET. FUNC.	L	L		L		L		

SYSTEM TIMING

	CYCLE	OFFSET	COORDINATED		BASE DAY 1		BASE DAY 2	
PATTERN	Sec.	Sec.	Phase	Sequence	Mon.- Fri.		Sat.- Sun.	

SPLIT ALLOCATION - Sec.

PHASE	1	2	3	4	5	6	7	8

NOTES: Naztec 980

APPENDIX D

Existing Capacity Analysis Worksheets

HCS7 Two-Way Stop-Control Report

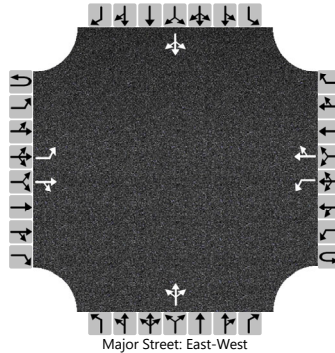
General Information

Analyst	SS
Agency/Co.	TPD, Inc.
Date Performed	5/26/2022
Analysis Year	2022
Time Analyzed	Existing AM
Intersection Orientation	East-West
Project Description	5659

Site Information

Intersection	CR 48 and Number 2 Rd
Jurisdiction	Lake County
East/West Street	CR 48
North/South Street	Number 2 Rd
Peak Hour Factor	0.88
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0
Configuration		L		TR		L		TR			LTR				LTR	
Volume (veh/h)		2	310	6		1	366	0		24	0	1		4	0	3
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)									0				0			
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33

Delay, Queue Length, and Level of Service

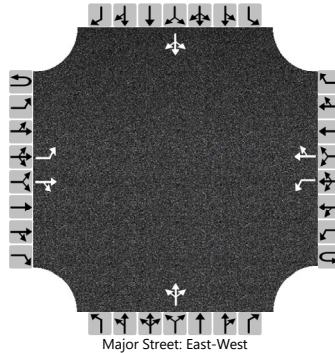
Flow Rate, v (veh/h)		2				1					28				8	
Capacity, c (veh/h)		1138				1194					316				398	
v/c Ratio		0.00				0.00					0.09				0.02	
95% Queue Length, Q ₉₅ (veh)		0.0				0.0					0.3				0.1	
Control Delay (s/veh)		8.2				8.0					17.5				14.2	
Level of Service (LOS)		A				A					C				B	
Approach Delay (s/veh)	0.1				0.0				17.5				14.2			
Approach LOS									C				B			

HCS7 Two-Way Stop-Control Report

General Information

Analyst	SS	Intersection	CR 48 and Number 2 Rd
Agency/Co.	TPD, Inc.	Jurisdiction	Lake County
Date Performed	5/26/2022	East/West Street	CR 48
Analysis Year	2022	North/South Street	Number 2 Rd
Time Analyzed	Existing PM	Peak Hour Factor	0.94
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	5659		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0
Configuration		L		TR		L		TR			LTR				LTR	
Volume (veh/h)		3	429	18		6	388	4		13	0	1		5	0	3
Percent Heavy Vehicles (%)		3				3				8	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)									0				0			
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.18	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.57	4.03	3.33		3.53	4.03	3.33

Delay, Queue Length, and Level of Service

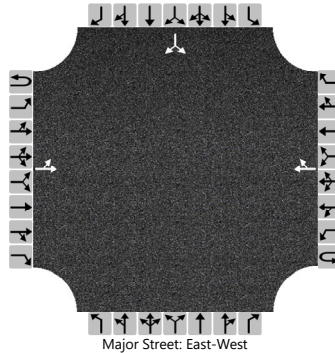
Flow Rate, v (veh/h)		3				6					15				9	
Capacity, c (veh/h)		1137				1081					260				330	
v/c Ratio		0.00				0.01					0.06				0.03	
95% Queue Length, Q ₉₅ (veh)		0.0				0.0					0.2				0.1	
Control Delay (s/veh)		8.2				8.3					19.7				16.2	
Level of Service (LOS)		A				A					C				C	
Approach Delay (s/veh)	0.1				0.1				19.7				16.2			
Approach LOS									C				C			

HCS7 Two-Way Stop-Control Report

General Information

Analyst	SS	Intersection	Number 2 Rd & Bloomfield
Agency/Co.	TPD, Inc.	Jurisdiction	Lake County
Date Performed	5/26/2022	East/West Street	Number 2 Rd
Analysis Year	2022	North/South Street	Bloomfield Ave
Time Analyzed	Existing AM	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	5659		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration		LT						TR							LR	
Volume (veh/h)		9	19				29	6						2		4
Percent Heavy Vehicles (%)		11												3		3
Proportion Time Blocked																
Percent Grade (%)													0			
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		4.1												7.1		6.2
Critical Headway (sec)		4.21												6.43		6.23
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.30												3.53		3.33

Delay, Queue Length, and Level of Service

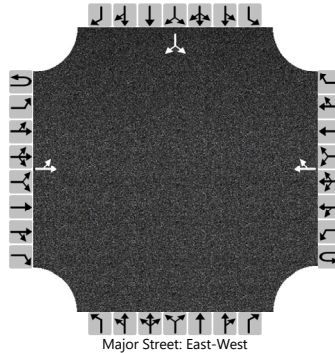
Flow Rate, v (veh/h)		10													7	
Capacity, c (veh/h)		1516													994	
v/c Ratio		0.01													0.01	
95% Queue Length, Q ₉₅ (veh)		0.0													0.0	
Control Delay (s/veh)		7.4													8.6	
Level of Service (LOS)		A													A	
Approach Delay (s/veh)	2.4												8.6			
Approach LOS													A			

HCS7 Two-Way Stop-Control Report

General Information

Analyst	SS	Intersection	Number 2 Rd & Bloomfield
Agency/Co.	TPD, Inc.	Jurisdiction	Lake County
Date Performed	5/26/2022	East/West Street	Number 2 Rd
Analysis Year	2022	North/South Street	Bloomfield Ave
Time Analyzed	Existing PM	Peak Hour Factor	0.97
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	5659		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration		LT						TR							LR	
Volume (veh/h)		3	18				20	6						1		6
Percent Heavy Vehicles (%)		33												3		3
Proportion Time Blocked																
Percent Grade (%)													0			
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		4.1												7.1		6.2
Critical Headway (sec)		4.43												6.43		6.23
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.50												3.53		3.33

Delay, Queue Length, and Level of Service

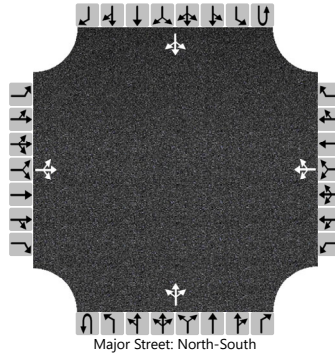
Flow Rate, v (veh/h)		3													7	
Capacity, c (veh/h)		1408													1035	
v/c Ratio		0.00													0.01	
95% Queue Length, Q ₉₅ (veh)		0.0													0.0	
Control Delay (s/veh)		7.6													8.5	
Level of Service (LOS)		A													A	
Approach Delay (s/veh)	1.1												8.5			
Approach LOS													A			

HCS7 Two-Way Stop-Control Report

General Information

Analyst	SS	Intersection	Palm Ave and Central Ave
Agency/Co.	TPD, Inc.	Jurisdiction	Lake County
Date Performed	5/26/2022	East/West Street	Number 2 Rd/W Central Ave
Analysis Year	2022	North/South Street	Palm Ave/SR 19
Time Analyzed	Existing AM	Peak Hour Factor	0.87
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	5659		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	0	1	0	0	0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		45	7	14		10	3	11		14	355	14		29	382	27
Percent Heavy Vehicles (%)		3	3	14		3	3	3		21				7		
Proportion Time Blocked																
Percent Grade (%)	0				0											
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		7.1	6.5	6.2		7.1	6.5	6.2		4.1				4.1		
Critical Headway (sec)		7.13	6.53	6.34		7.13	6.53	6.23		4.31				4.17		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.43		3.53	4.03	3.33		2.39				2.26		

Delay, Queue Length, and Level of Service

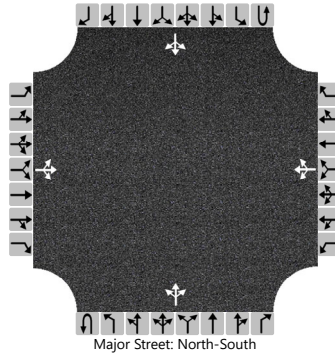
Flow Rate, v (veh/h)			76				28			16				33		
Capacity, c (veh/h)			247				304			999				1109		
v/c Ratio			0.31				0.09			0.02				0.03		
95% Queue Length, Q ₉₅ (veh)			1.3				0.3			0.0				0.1		
Control Delay (s/veh)			25.9				18.0			8.7				8.3		
Level of Service (LOS)			D				C			A				A		
Approach Delay (s/veh)	25.9				18.0				0.5				0.9			
Approach LOS	D				C											

HCS7 Two-Way Stop-Control Report

General Information

Analyst	SS	Intersection	Palm Ave and Central Ave
Agency/Co.	TPD, Inc.	Jurisdiction	Lake County
Date Performed	5/26/2022	East/West Street	Number 2 Rd/W Central Ave
Analysis Year	2022	North/South Street	Palm Ave/SR 19
Time Analyzed	Existing PM	Peak Hour Factor	0.86
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	5659		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	0	1	0	0	0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		29	5	18		11	3	12		20	366	15		14	411	25
Percent Heavy Vehicles (%)		3	3	28		3	3	3		35				3		
Proportion Time Blocked																
Percent Grade (%)	0				0											
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		7.1	6.5	6.2		7.1	6.5	6.2		4.1				4.1		
Critical Headway (sec)		7.13	6.53	6.48		7.13	6.53	6.23		4.45				4.13		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.55		3.53	4.03	3.33		2.52				2.23		

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			60				30			23				16		
Capacity, c (veh/h)			258				290			908				1112		
v/c Ratio			0.23				0.10			0.03				0.01		
95% Queue Length, Q ₉₅ (veh)			0.9				0.3			0.1				0.0		
Control Delay (s/veh)			23.2				18.9			9.1				8.3		
Level of Service (LOS)			C				C			A				A		
Approach Delay (s/veh)	23.2				18.9				0.7				0.4			
Approach LOS	C				C											

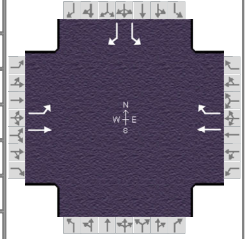
HCS7 Signalized Intersection Results Summary

General Information

Agency	TPD, Inc.		
Analyst	SS	Analysis Date	5/26/2022
Jurisdiction	Lake County	Time Period	Existing AM
Urban Street	CR 48	Analysis Year	2022
Intersection	SR 19	File Name	5659 - CR 48 and
Project Description	5659		

Intersection Information

Duration, h	0.250
Area Type	Other
PHF	0.89
Analysis Period	1> 7:15
SR 19 - Existing AM.xus	



Demand Information

	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	240	71			76	397				309		205

Signal Information

Cycle, s	71.2	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	Yes	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									

Timer Results

	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase	1	6		2				4
Case Number	1.0	4.0		7.3				9.0
Phase Duration, s	16.3	46.3		30.0				24.9
Change Period, (Y+R _c), s	6.5	6.4		6.4				7.3
Max Allow Headway (MAH), s	4.1	4.2		4.2				4.2
Queue Clearance Time (g _s), s	9.0	3.5		21.1				15.5
Green Extension Time (g _e), s	0.8	2.6		2.5				2.0
Phase Call Probability	1.00	1.00		1.00				1.00
Max Out Probability	0.00	0.00		0.00				0.03

Movement Group Results

	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	1	6			2	12				7		14
Adjusted Flow Rate (v), veh/h	270	80			85	446				347		230
Adjusted Saturation Flow Rate (s), veh/h/ln	1654	1737			1856					1725		1510
Queue Service Time (g _s), s	7.0	1.5			2.3					13.5		9.7
Cycle Queue Clearance Time (g _c), s	7.0	1.5			2.3					13.5		9.7
Green Ratio (g/C)	0.50	0.56			0.33					0.25		0.25
Capacity (c), veh/h	696	974			616					427		373
Volume-to-Capacity Ratio (X)	0.387	0.082			0.139					0.814		0.617
Back of Queue (Q), ft/ln (95 th percentile)	107.1	22.4			41.6					248.1		161.6
Back of Queue (Q), veh/ln (95 th percentile)	3.9	0.8			1.6					9.5		6.1
Queue Storage Ratio (RQ) (95 th percentile)	0.00	0.00			0.00					0.00		0.00
Uniform Delay (d ₁), s/veh	10.9	7.2			16.7					25.3		23.8
Incremental Delay (d ₂), s/veh	0.4	0.0			0.1					3.8		1.7
Initial Queue Delay (d ₃), s/veh	0.0	0.0			0.0					0.0		0.0
Control Delay (d), s/veh	11.3	7.3			16.8	0.0				29.1		25.5
Level of Service (LOS)	B	A			B	A				C		C
Approach Delay, s/veh / LOS	10.4	B		2.7	A		0.0			27.7		C
Intersection Delay, s/veh / LOS	14.4						B					

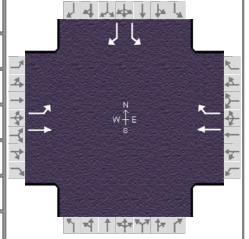
Multimodal Results

	EB			WB			NB			SB		
Pedestrian LOS Score / LOS	0.68	A		1.91	B		1.95	B		1.95		B
Bicycle LOS Score / LOS	1.06	A		1.36	A							F

HCS7 Signalized Intersection Results Summary

General Information

Agency	TPD, Inc.		
Analyst	SS	Analysis Date	5/26/2022
Jurisdiction	Lake County	Time Period	Existing PM
Urban Street	CR 48	Analysis Year	2022
Intersection	SR 19	File Name	5659 - CR 48 and
Project Description	5659		



Demand Information

	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	274	81			79	365				416		276

Signal Information

Cycle, s	71.4	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	Yes	Simult. Gap E/W	On	Green	10.2	20.2	20.8	0.0	0.0	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.4	4.4	4.8	0.0	0.0	0.0		
				Red	2.1	2.0	2.5	0.0	0.0	0.0		

Timer Results

	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase	1	6		2				4
Case Number	1.0	4.0		7.3				9.0
Phase Duration, s	16.7	43.3		26.6				28.1
Change Period, ($Y+R_c$), s	6.5	6.4		6.4				7.3
Max Allow Headway (MAH), s	4.1	4.2		4.2				4.2
Queue Clearance Time (g_s), s	9.4	3.6		18.0				18.4
Green Extension Time (g_e), s	0.8	2.2		2.1				2.3
Phase Call Probability	1.00	1.00		1.00				1.00
Max Out Probability	0.00	0.00		0.00				0.14

Movement Group Results

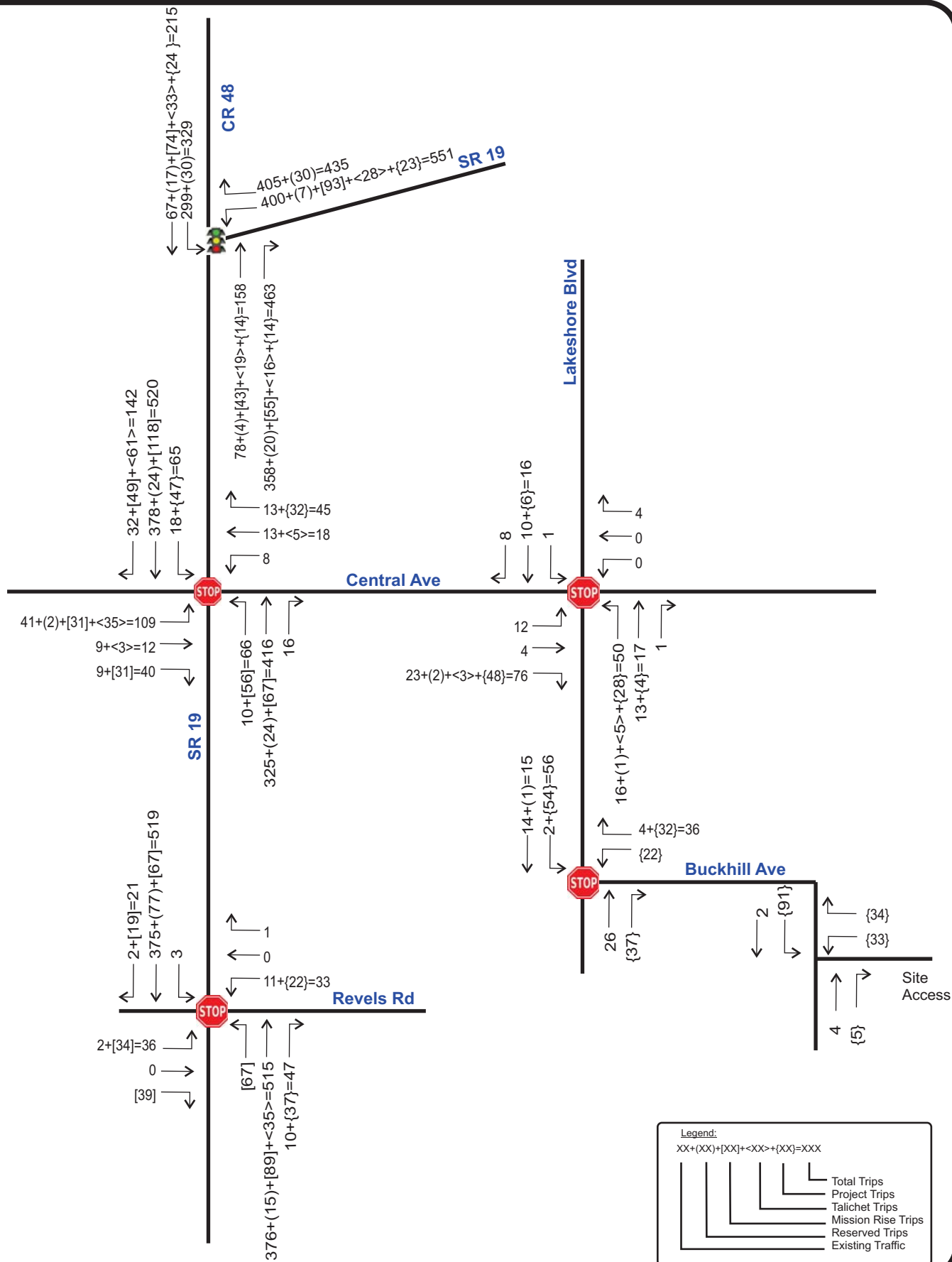
	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	1	6			2	12				7		14
Adjusted Flow Rate (v), veh/h	282	84			81	376				429		285
Adjusted Saturation Flow Rate (s), veh/h/ln	1781	1900			1856					1753		1585
Queue Service Time (g_s), s	7.4	1.6			2.4					16.4		11.1
Cycle Queue Clearance Time (g_c), s	7.4	1.6			2.4					16.4		11.1
Green Ratio (g/C)	0.45	0.52			0.28					0.29		0.29
Capacity (c), veh/h	687	982			525					512		463
Volume-to-Capacity Ratio (X)	0.411	0.085			0.155					0.838		0.615
Back of Queue (Q), ft/ln (95 th percentile)	118.2	25			43.8					294.3		181.4
Back of Queue (Q), veh/ln (95 th percentile)	4.7	1.0			1.7					11.4		7.1
Queue Storage Ratio (RQ) (95 th percentile)	0.00	0.00			0.00					0.00		0.00
Uniform Delay (d_1), s/veh	12.8	8.7			19.2					23.8		21.9
Incremental Delay (d_2), s/veh	0.4	0.0			0.1					5.9		1.3
Initial Queue Delay (d_3), s/veh	0.0	0.0			0.0					0.0		0.0
Control Delay (d), s/veh	13.2	8.8			19.4	0.0				29.6		23.2
Level of Service (LOS)	B	A			B	A				C		C
Approach Delay, s/veh / LOS	12.2	B		3.4	A		0.0			27.1		C
Intersection Delay, s/veh / LOS	16.5						B					

Multimodal Results

	EB			WB			NB			SB		
Pedestrian LOS Score / LOS	0.68	A		1.91	B		1.95	B		1.95	B	
Bicycle LOS Score / LOS	1.09	A		1.24	A						F	

APPENDIX E

Approved Project Trips



Whispering Hills
 Project No 5199
Figure 5

Projected P.M. Peak Hour Intersection Volumes



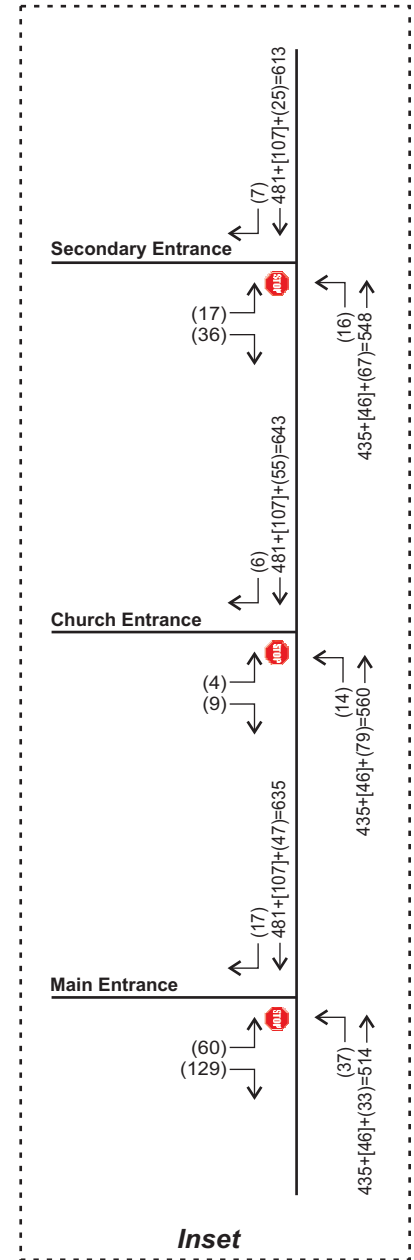
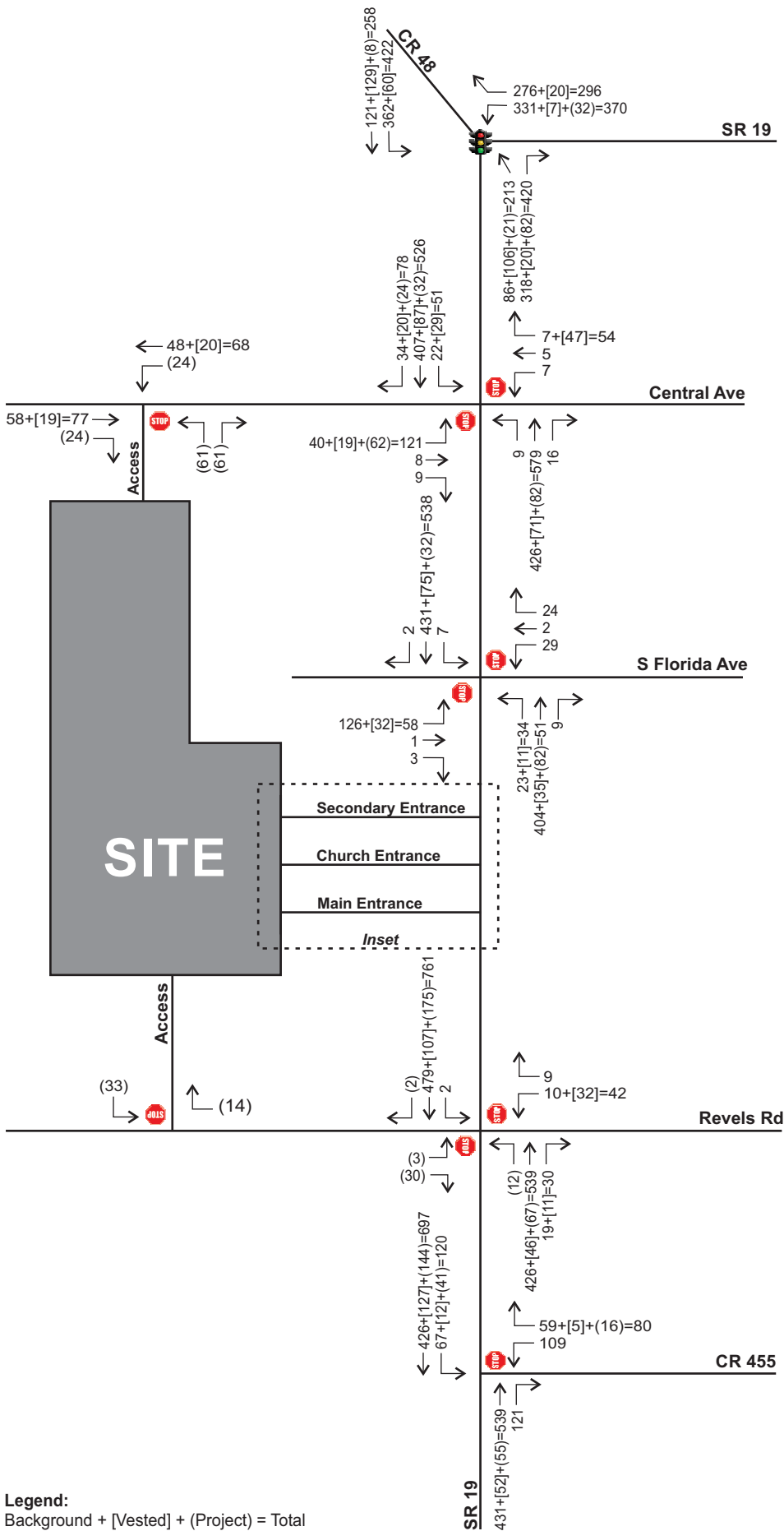
Table 4
Projected P.M. Peak Hour Roadway Analysis

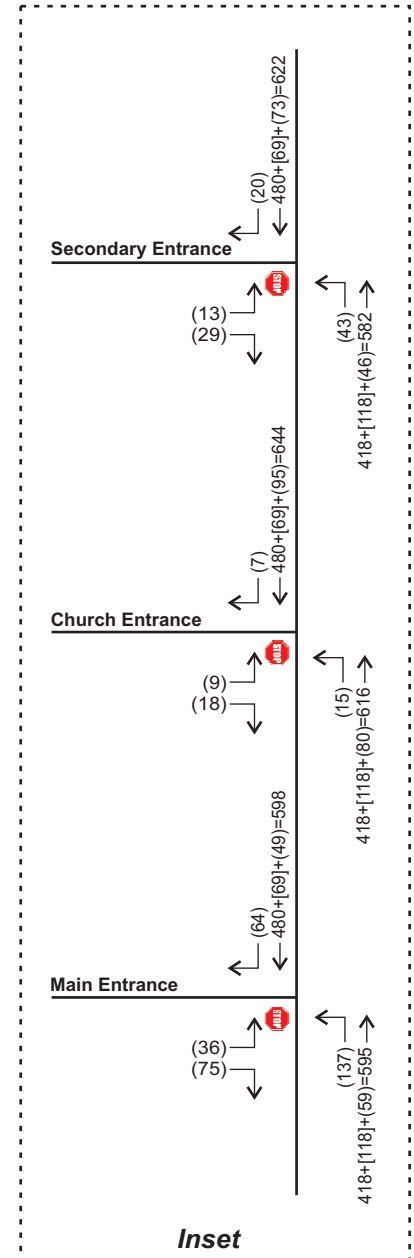
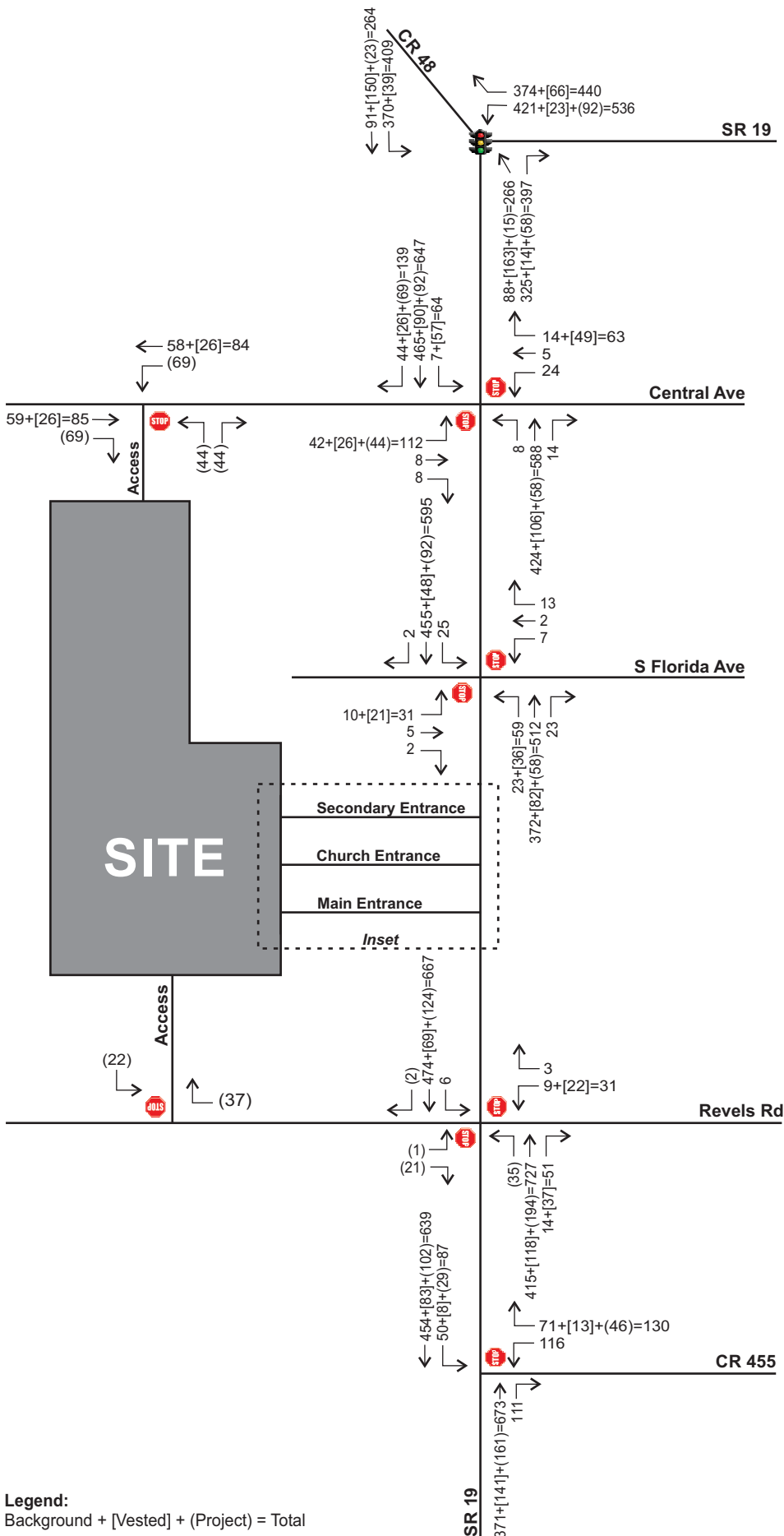
Roadway Segment	# of Lns	F/Class	LOS		Peak Hour/Peak Direction							V/C Ratio	LOS
			Std	Capacity	Direction	Existing	Reserved*			Project**	Total		
							1	2	3				
SR 19													
CR 561 to Lake Harris N. End	2	ART. 1	D	1,190	NB	595	50	93	14	14	766	0.64	C
Lake Harris N. End to CR 48	2	ART. 1	C	850	NB	595	50	93	14	14	766	0.90	C
CR 48 to Central Ave	2	ART. 1	C	710	NB	362	24	167	32	28	613	0.86	C
Central Ave to CR 455	2	ART. 1	C	850	SB	446	77	171	29	37	760	0.89	C
CR 455 to US 27	2	ART. 1	C	850	SB	446	77	130	11	10	674	0.79	C
CR 48													
US 27 to Lime Ave	2	Maj. Coll.	D	792	WB	576	65	93	12	14	760	0.96	C
Lime Ave to SR 19	2	Maj. Coll.	D	792	WB	390	50	74	16	14	544	0.69	C
CR 561 to Ranch Rd	2	Maj. Coll.	D	792	WB	307	23	22	3	9	364	0.46	C
Ranch Rd to CR 448A	2	Coll.	C	670	WB	258	28	19	3	9	317	0.47	C
CR 561													
CR 448 to CR 48	2	Maj. Coll.	D	792	SB	449	52	5	2	1	509	0.64	C
CR 48 to S. Astatula City Limits	2	Maj. Coll.	D	720	SB	449	52	5	7	1	514	0.71	D
S. Astatula City Limits to CR 455	2	Maj. Coll.	D	720	SB	534	57	22	6	11	630	0.88	D
CR 455 to Howey Cross Rd	2	Maj. Coll.	D	720	NB	357	26	0	3	11	397	0.55	D
Howey Cross Rd to Turnpike Rd/CR 561A	2	Maj. Coll.	D	720	SB	542	153	7	1	11	714	0.99	E
CR 455													
SR 19 to CR 561	2	Maj. Coll.	D	1,200	EB	165	28	33	8	14	248	0.21	B
CR 561 to CR 561A	2	Maj. Coll.	D	1,200	WB	99	12	19	2	1	133	0.11	B

* 1 = MPO Database, 2 = Mission Rise, 3 = Talichet

** Highest trips on the segment







Projected PM Peak Intersection Volumes

The Reserve at Howey in the Hills
21082

Figure

6

Table 5
Projected Roadway Segment Capacity Analysis

Roadway Segment	No Lns	LOS Std	PH Dir Capacity	Dir	Exist Vol	Growth Rate	2028 Backg'd	Vested Trips	Trip Distr	Project Volume	Total Volume	LOS
Number 2 Rd												
CR 48 to N Mare Ave	2	D	612	NB/EB SB/WB	51 50	2.00%	58 57	26 26	15%	69 44	153 127	C C
W Central Ave												
N Mare Ave to SR 19	2	D	612	NB/EB SB/WB	51 50	2.00%	58 57	26 26	15%	69 44	153 127	C C
CR 455												
SR 19 to NF 552	2	C	740	NB/EB SB/WB	146 151	5.75%	205 212	8 13	10%	46 29	259 254	B B
SR 19												
Lane Park Rd to CR 48	4	D	1,200	NB/EB SB/WB	625 676	3.75%	789 853	229 189	5%	15 23	1,033 1,065	D D
CR 48 to Central Ave	2	D	800	NB/EB SB/WB	387 423	2.00%	441 482	177 173	30%	87 138	705 793	C D
Central Ave to CR 455	2	D	1,200	NB/EB SB/WB	387 423	2.00%	441 482	82 48	50%	231 145	754 675	C C
CR 455 to US 27/ SR 25	2	C	850	NB/EB SB/WB	419 453	2.00%	478 516	141 83	35%	161 102	780 701	C C
US 27/ SR 25 to CR 478	2	C	850	NB/EB SB/WB	481 405	2.00%	548 462	141 83	20%	92 58	781 603	C C

Source: 2020 Lake County Annual Traffic Counts



MEMORANDUM

May 16, 2022

Re: Simpson Howey-In-The-Hills
Tier 2 Traffic Impact Analysis (TIA) Methodology
Town of Howey-In-The-Hills, Florida
Project № 22105

This methodology outlines the Traffic Impact Analysis (TIA) for the above referenced project. This methodology is consistent with the requirements of the Town of Howey-In-The-Hills, Lake County, and the Lake-Sumter Metropolitan Planning Organization (LSMPO) for a Tier 2 TIA.

Project Description

The proposed project is a residential development consisting of 265 single-family units. The project buildout is anticipated to be in 2027. The ±87.17-acre site includes parcels 35-20-25-0150-000-01200, 02-21-25-0001-000-03700 and 35-20-25-0150-000-02600. A preliminary site plan is included in the **Attachments**. The site is located on the southeast corner of the SR 19 and Revels Road intersection, in the Town of Howey-In-The-Hills, Florida, as shown in **Figure 1**. The project proposes two (2) full access driveways along Revels Road.

Trip Generation

A trip generation analysis was performed for the development using the trip generation information from the Institute of Transportation Engineers (ITE) *Trip Generation Manual, 11th Edition*. The ITE information sheets are included in the **Attachments**. The trip generation for the proposed development is summarized in **Table 1**.

Table 1
Trip Generation Calculations

ITE Code	Land Use	Size	Daily		AM Peak Hour				PM Peak Hour			
			Rate	Trips	Rate	Total	Enter	Exit	Rate	Total	Enter	Exit
210	Single-Family Detached	269 DU	9.32	2,508	0.68	183	47	136	0.94	252	159	93

Source: ITE Trip Generation Manual, 11th Edition

The proposed development is projected to generate 2,508 new daily trips of which 183 trips occur during the AM peak hour, and 252 trips occur during the PM peak hour.

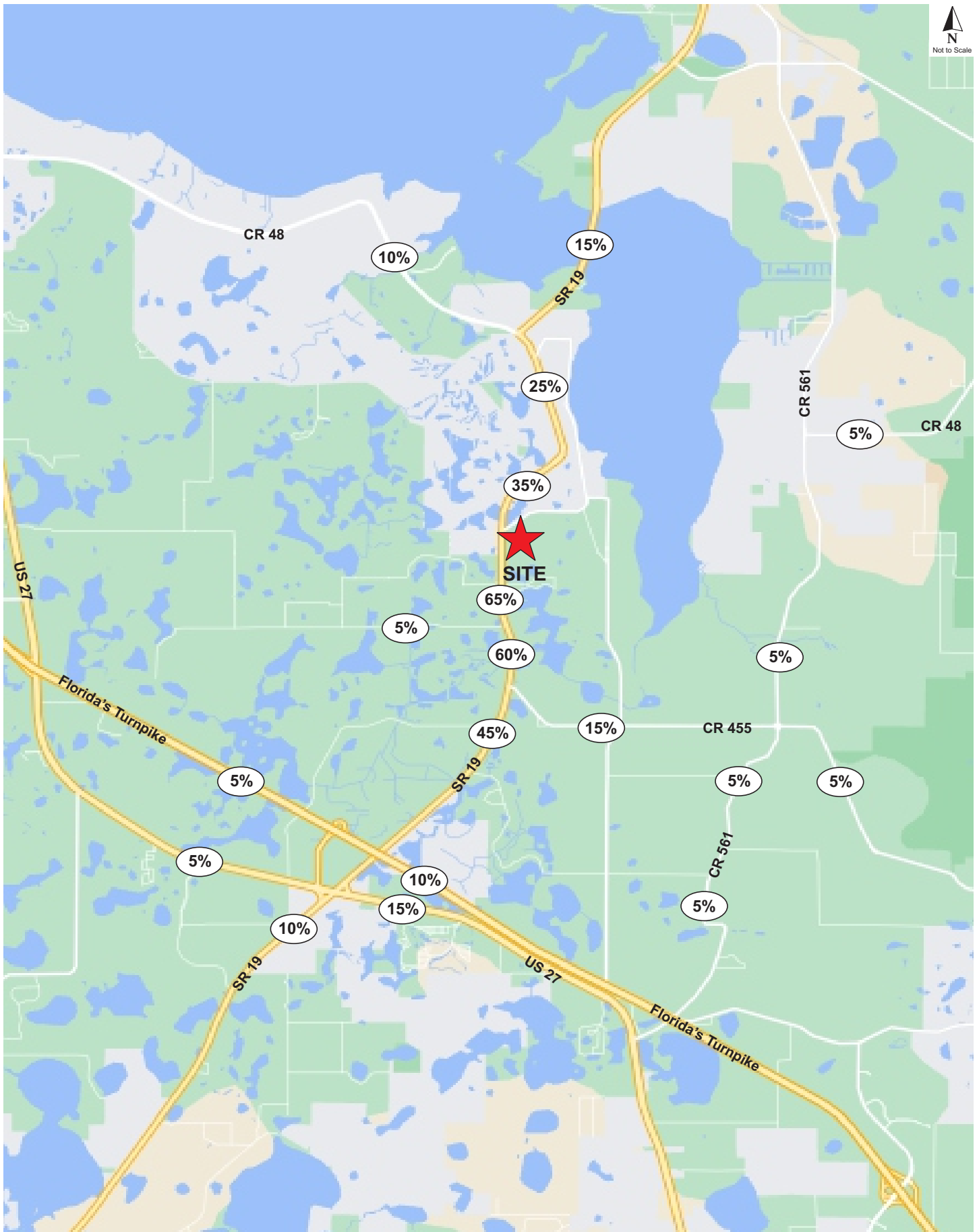


Table 2
Study Area Significance Analysis

Road Name	From	To	# LNS	A T	LOS Std	LOS Cap	Project Trips			Within 1.0 miles?	%	Signif?	In Study?
							% Dist	NB/EB	SB/WB				
CR 455	SR 19	CR 561	2	R	C	740	15%	14	24	NO	3.2%	NO	NO
CR 455	CR 561	CR 561A	2	R	C	410	5%	5	8	NO	2.0%	NO	NO
CR 48	US 27	Lime Ave	2	U	D	1,080	10%	16	9	NO	1.5%	NO	NO
CR 48	Lime Ave	SR 19	2	U	D	1,080	10%	16	9	NO	1.5%	NO	NO
CR 48	CR 561	Ranch Rd	2	U	D	840	5%	5	8	NO	1.0%	NO	NO
CR 48	Ranch Rd	CR 488A	2	R	C	410	5%	5	8	NO	2.0%	NO	NO
CR 561	CR 448	CR 48	2	U	D	1,080	0%	0	0	NO	0.0%	NO	NO
CR 561	CR 48	S Astatula City Limits	2	U	D	620	5%	5	8	NO	1.3%	NO	NO
CR 561	S Astatula City Limits	CR 455	2	U	D	1,080	5%	5	8	NO	0.7%	NO	NO
CR 561	CR 455	Howey Cross Rd	2	R	C	470	5%	8	5	NO	1.7%	NO	NO
CR 561	Howey Cross Rd	Turnpike Rd/CR 561A	2	R	C	640	5%	8	5	NO	1.3%	NO	NO
Revels Rd*	SR 19	6th Ave	2	R	C	600	100%	93	159	YES	26.5%	YES	YES
SR 19	Lane Park Rd	CR 48	2	U	D	920	15%	14	24	NO	2.6%	NO	NO
SR 19	CR 48	Central Ave	2	U	D	700	25%	23	40	NO	5.7%	YES	YES
SR 19	Central Ave	CR 455	2	R	C	1,200	65%	103	61	YES	8.6%	YES	YES
SR 19	CR 455	US 27/SR 25	2	R	C	450	45%	72	41	NO	16.0%	YES	YES
SR 19	US 27/SR 25	CR 478	2	R	C	450	10%	16	9	NO	3.6%	NO	NO
SR 91 (Florida Turnpike)	US 27/SR 25	US 27/SR 25/SR 19	4	U	B	2,230	5%	5	8	NO	0.4%	NO	NO
SR 91 (Florida Turnpike)	US 27/SR 25/SR 19	Orange County Line	4	U	C	3,100	10%	16	9	NO	0.5%	NO	NO
US 27/SR 25	Florida Turnpike	SR 19	4	U	D	2,100	5%	5	8	NO	0.4%	NO	NO
US 27/SR 25	SR 19	CR 561	4	U	D	3,280	15%	24	14	NO	0.7%	NO	NO

Source: 2021 Lake County CMP Database

* LOS Capacity Estimated Using FDOT 2020 QLOS Handbook Table 9

Based on the study area analysis, the following roadway segments will be analyzed for the PM peak hour:

- Revels Road
 - SR 19 to 6th Ave
- SR 19
 - CR 48 to Central Avenue
 - Central Avenue to CR 455
 - CR 455 to US 27/SR 25

The following intersections will be analyzed for the AM and PM peak hours:

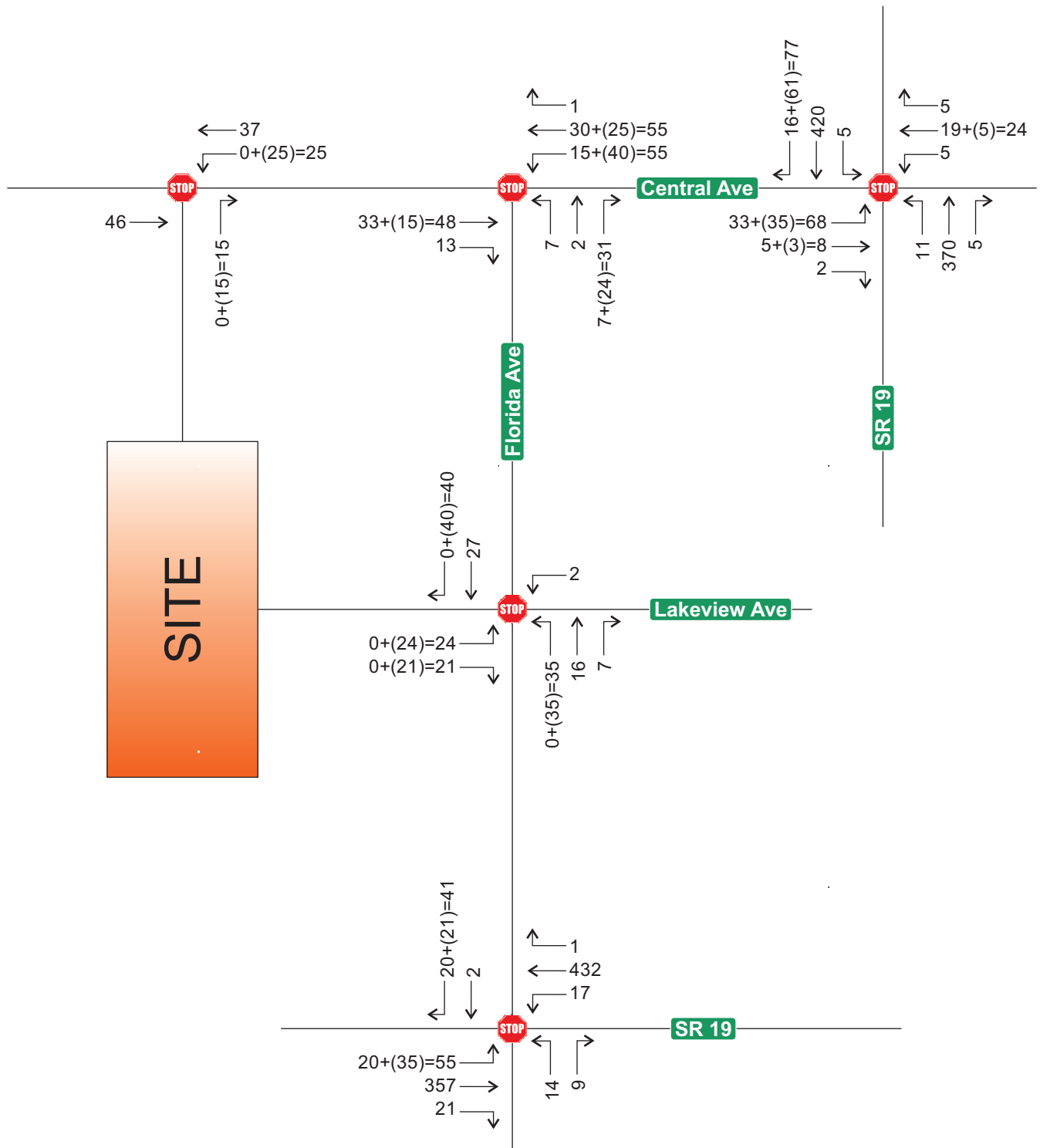
- SR 19 and CR 48 (signalized)
- SR 19 and CR 448 (signalized)
- SR 19 and Central Avenue (unsignalized)
- SR 19 and Revels Road (unsignalized)
- SR 19 and CR 455 (unsignalized)
- Revels Road and Site Access Driveway (proposed)

Table 4
Projected Roadway Capacity Analysis

Seg ID	Roadway	Segment	Lanes	LOS Stnd	PH Dir Capacity	Dir	Backg'd Vol	Trip Dist	Project Vol	% Sig.	Total Vol	Projected LOS
3030	SR 19	CR 561 to LAKE HARRIS NORTH END	2	D	1,190	NB/EB	1,189	28.0%	17	1.43%	1,206	E
						SB/WB	1,136		28	2.35%	1,164	D
3040	SR 19	LAKE HARRIS NORTH END to CR 48	2	C	850	NB/EB	479	28.0%	17	2.00%	496	C
						SB/WB	528		28	3.29%	556	C
3050	SR 19	CR 48 to CENTRAL AVENUE	2	C	710	NB/EB	349	65.0%	38	5.35%	387	C
						SB/WB	304		66	9.30%	370	C
3060	SR 19	CENTRAL AVENUE to CR 455	2	C	850	NB/EB	349	35.0%	35	4.12%	384	B
						SB/WB	304		21	2.47%	325	B
3070	SR 19	CR 455 to US 27 / SR 25	2	C	850	NB/EB	331	22.0%	22	2.59%	353	B
						SB/WB	376		13	1.53%	389	B
1250	C.R. 48	LIME AVENUE to SR 19	2	D	792	NB/EB	395	32.0%	32	4.04%	427	C
						SB/WB	337		19	2.40%	356	C

Note: Background volumes = Existing Volumes x 1 + (Annual Growth Rate x 5 Years of growth from 2016 to 2021)





Legend:

XX+(XX)=XXX

— Total Traffic

— Project Trips

— Background Traffic

*Schematic drawing. Not to scale.
 ** Any +/- 1 project trip discrepancies is due to rounding

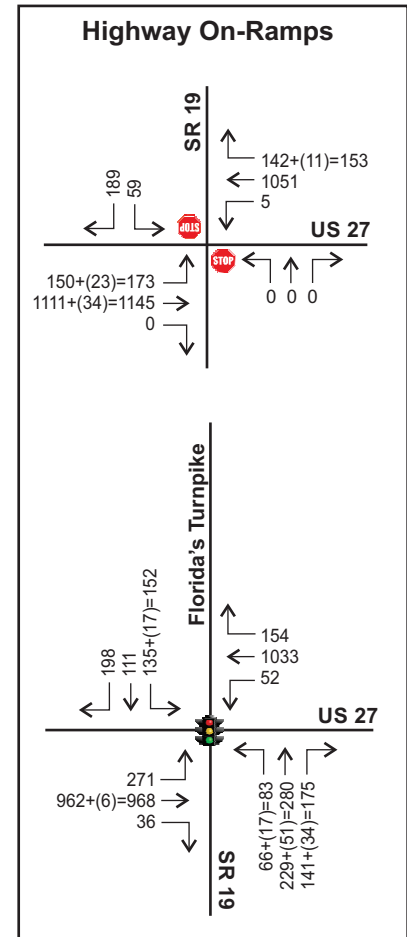
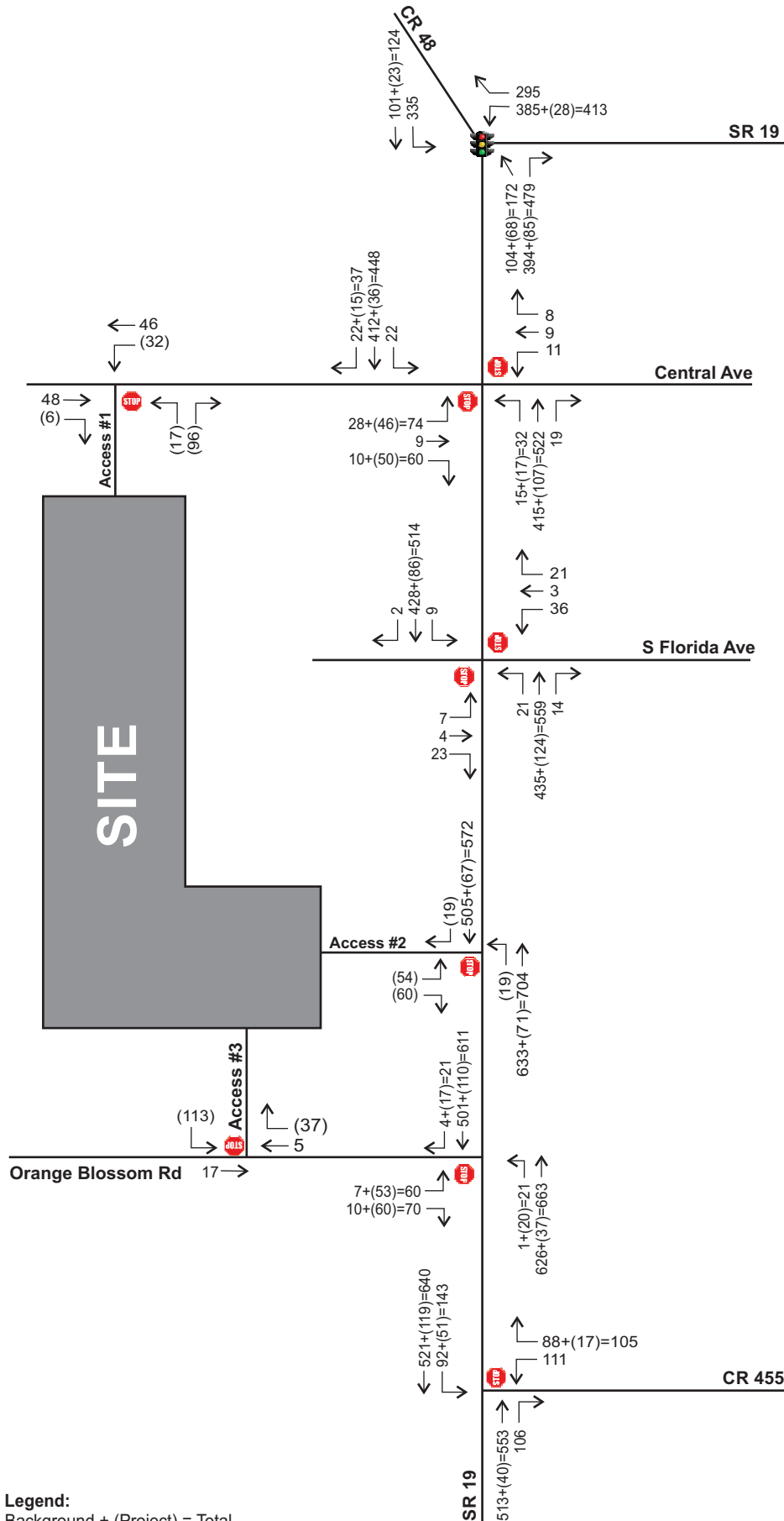
Table 5
Projected Roadway Segment Capacity Analysis

Roadway Segment	Seg ID	No Lns	LOS Std	PH Dir Capacity	Dir	Exist Vol	Growt h Rate	2024 Backg'd	Trip Distr	Proj Dir	Project Volume	Total Volume	LOS
SR 19													
CR 561 to Lake Harris North End	NA	4	D	1,190	NB SB	657 805	4.24%	796 976	25%	OUT IN	55 93	851 1,069	D D
Lake Harris North End to CR 48	NA	2	D	1,200	NB SB	657 805	4.24%	796 976	25%	OUT IN	55 93	851 1,069	D D
CR 48 to Central Ave	NA	2	D	800	NB SB	432 436	3.29%	503 508	45%	OUT IN	99 167	602 675	C C
Central Ave to Taylor Memorial Cemetery	NA	2	D	800	NB SB	432 436	3.29%	503 508	45%	OUT IN	99 167	602 675	C C
Taylor Memorial Cemetery to CR 455 (1)	NA	2	C	900	NB SB	508 503	8.65%	728 721	46%	OUT IN	101 171	829 892	C C
CR 455 to US 27/SR 25 (1)	NA	2	C	900	NB SB	526 541	8.65%	753 775	35%	IN OUT	130 77	883 852	C C
CR 48 (2)													
US 27 to Lime Ave	16	2	D	792	EB WB	366 483	5.62%	469 619	25%	IN OUT	93 55	562 674	C C
Lime Ave to SR 19	16	2	D	792	EB WB	366 483	5.62%	469 619	20%	IN OUT	74 44	543 663	C C
Orange Blossom Rd (2)													
Revels Rd to SR 19	NA	2	D	612	EB WB	8 13	2.00%	9 14	33%	OUT IN	72 123	81 137	C C
Number 2 Rd (2)													
Blue Sink Rd to Mare Ave	NA	2	D	612	EB WB	59 50	2.00%	65 55	5%	OUT IN	11 19	76 74	C C
Central Ave (2)													
Mare Ave to SR 19	NA	2	D	612	EB WB	59 50	2.00%	65 55	28%	OUT IN	61 104	126 159	C C
CR 561													
South Astatula City Limit to CR 455	16	2	D	720	EB WB	520 534	5.33%	659 676	10%	IN OUT	37 22	696 698	D D

Source: 2018 Lake County Annual Traffic Counts

(1) FDOT QLOS HIGHPLAN Analysis for these segments of SR 19

(2) Volumes Obtained from PM Peak Turning Movement Counts



Legend:
Background + (Project) = Total

APPENDIX F

Projected Capacity Analysis Worksheets

HCS Two-Way Stop-Control Report

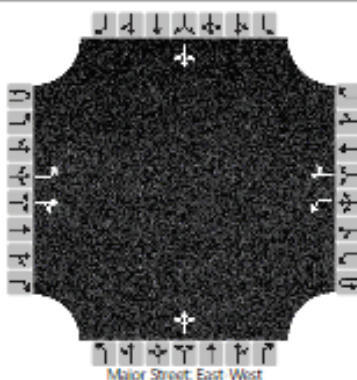
General Information

Analyst	SS
Agency/Co.	TPD, Inc.
Date Performed	12/22/2022
Analysis Year	2028
Time Analyzed	Projected AM
Intersection Orientation	East-West
Project Description	5659

Site Information

Intersection	CR 48 and Number 2 Rd
Jurisdiction	Lake County
East/West Street	CR 48
North/South Street	Number 2 Rd
Peak Hour Factor	0.88
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0
Configuration		L		TR		L		TR			LTR				LTR	
Volume (veh/h)		2	310	15		2	366	0		52	0	3		4	0	3
Percent Heavy Vehicles (%)		3				3				3	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)										0				0		
Right Turn Channelized																
Median Type Storage																

Critical and Follow-up Headways

Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.13	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.53	4.03	3.33		3.53	4.03	3.33

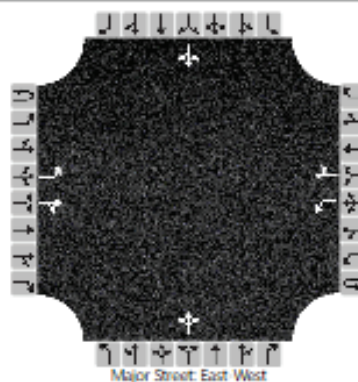
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		2				2					63				8	
Capacity, c (veh/h)		1138				1184					315				397	
v/c Ratio		0.00				0.00					0.20				0.02	
95% Queue Length, Q ₉₅ (veh)		0.0				0.0					0.7				0.1	
Control Delay (s/veh)		8.2				8.0					19.2				14.3	
Level of Service (LOS)		A				A					C				B	
Approach Delay (s/veh)		0.0				0.0				19.2				14.3		
Approach LOS		A				A				C				B		

HCS Two-Way Stop-Control Report

General Information		Site Information	
Analyst	SS	Intersection	CR 48 and Number 2 Rd
Agency/Co.	TPD, Inc.	Jurisdiction	Lake County
Date Performed	12/22/2022	East/West Street	CR 48
Analysis Year	2028	North/South Street	Number 2 Rd
Time Analyzed	Projected PM	Peak Hour Factor	0.94
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	5659		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	1	0	0	1	1	0		0	1	0		0	1	0
Configuration		L		TR		L		TR			LTR				LTR	
Volume (veh/h)		3	429	50		8	388	4		32	0	2		5	0	3
Percent Heavy Vehicles (%)		3				3				8	3	3		3	3	3
Proportion Time Blocked																
Percent Grade (%)									0				0			
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		4.1				4.1				7.1	6.5	6.2		7.1	6.5	6.2
Critical Headway (sec)		4.13				4.13				7.18	6.53	6.23		7.13	6.53	6.23
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.23				2.23				3.57	4.03	3.33		3.53	4.03	3.33

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		3				9					36				9	
Capacity, c (veh/h)		1137				1050					250				331	
v/c Ratio		0.00				0.01					0.14				0.03	
95% Queue Length, Q ₉₅ (veh)		0.0				0.0					0.5				0.1	
Control Delay (s/veh)		8.2				8.5					21.8				16.2	
Level of Service (LOS)		A				A					C				C	
Approach Delay (s/veh)	0.1				0.2				21.8				16.2			
Approach LOS	A				A				C				C			

HCS Two-Way Stop-Control Report

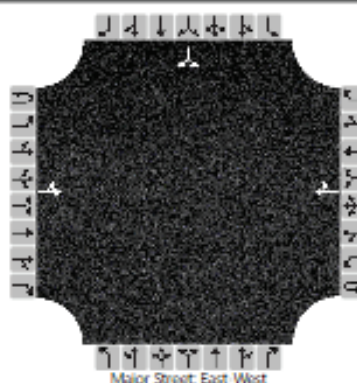
General Information

Analyst	SS
Agency/Co.	TPD, Inc.
Date Performed	12/22/2022
Analysis Year	2028
Time Analyzed	Projected AM
Intersection Orientation	East-West
Project Description	5659

Site Information

Intersection	Number 2 Rd & Bloomfield
Jurisdiction	Lake County
East/West Street	Number 2 Rd
North/South Street	Bloomfield Ave
Peak Hour Factor	0.92
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration		LT						TR							LR	
Volume (veh/h)		9	58				131	22						6		4
Percent Heavy Vehicles (%)		11												3		3
Proportion Time Blocked																
Percent Grade (%)														0		
Right Turn Channelized																
Median Type Storage																

Critical and Follow-up Headways

Base Critical Headway (sec)		4.1												7.1		6.2
Critical Headway (sec)		4.21												6.43		6.23
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.30												3.53		3.33

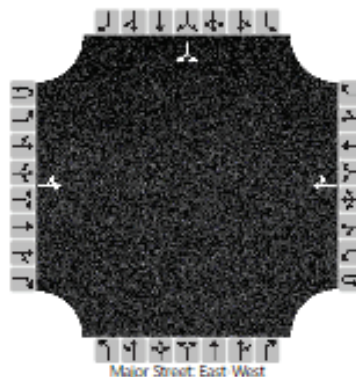
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		10													11	
Capacity, c (veh/h)		1359													796	
v/c Ratio		0.01													0.01	
95% Queue Length, Q ₉₅ (veh)		0.0													0.0	
Control Delay (s/veh)		7.7	0.1												9.6	
Level of Service (LOS)		A	A												A	
Approach Delay (s/veh)		1.1													9.6	
Approach LOS		A													A	

HCS Two-Way Stop-Control Report

General Information		Site Information	
Analyst	SS	Intersection	Number 2 Rd & Bloomfield
Agency/Co.	TPD, Inc.	Jurisdiction	Lake County
Date Performed	12/22/2022	East/West Street	Number 2 Rd
Analysis Year	2028	North/South Street	Bloomfield Ave
Time Analyzed	Projected PM	Peak Hour Factor	0.97
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	5659		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration		LT						TR							LR	
Volume (veh/h)		3	143				86	22						9		6
Percent Heavy Vehicles (%)		33												3		3
Proportion Time Blocked																
Percent Grade (%)													0			
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		4.1												7.1		6.2
Critical Headway (sec)		4.43												6.43		6.23
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.50												3.53		3.33

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		3													15	
Capacity, c (veh/h)		1306													806	
v/c Ratio		0.00													0.02	
95% Queue Length, Q ₉₅ (veh)		0.0													0.1	
Control Delay (s/veh)		7.8	0.0												9.6	
Level of Service (LOS)		A	A												A	
Approach Delay (s/veh)	0.2												9.6			
Approach LOS	A												A			

HCS Two-Way Stop-Control Report

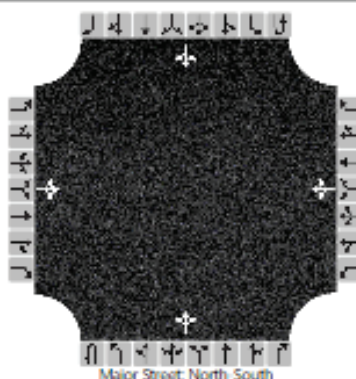
General Information

Analyst	SS
Agency/Co.	TPD, Inc.
Date Performed	12/22/2022
Analysis Year	2028
Time Analyzed	Bkgd AM
Intersection Orientation	North-South
Project Description	5659

Site Information

Intersection	Palm Ave and Central Ave
Jurisdiction	Lake County
East/West Street	Number 2 Rd/W Central Ave
North/South Street	Palm Ave/SR 19
Peak Hour Factor	0.87
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	0	1	0	0	0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		155	7	64		10	3	11		31	572	14		29	532	73
Percent Heavy Vehicles (%)		3	3	14		3	3	3		21				7		
Proportion Time Blocked																
Percent Grade (%)	0				0											
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		7.1	6.5	6.2		7.1	6.5	6.2		4.1				4.1		
Critical Headway (sec)		7.13	6.53	6.34		7.13	6.53	6.23		4.31				4.17		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.43		3.53	4.03	3.33		2.39				2.26		

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			260				28			36				33		
Capacity, c (veh/h)			116				133			818				894		
v/c Ratio			2.24				0.21			0.04				0.04		
95% Queue Length, Q ₉₅ (veh)			22.3				0.7			0.1				0.1		
Control Delay (s/veh)			643.4				39.2			9.6	0.7	0.7		9.2	0.6	0.6
Level of Service (LOS)			F				E			A	A	A		A	A	A
Approach Delay (s/veh)	643.4			39.2				1.1				1.0				
Approach LOS	F			E				A				A				

HCS Two-Way Stop-Control Report

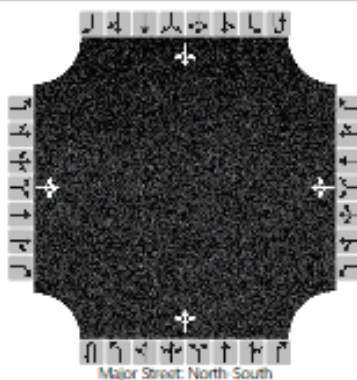
General Information

Analyst	SS
Agency/Co.	TPD, Inc.
Date Performed	12/22/2022
Analysis Year	2028
Time Analyzed	Bkgd PM
Intersection Orientation	North-South
Project Description	5659

Site Information

Intersection	Palm Ave and Central Ave
Jurisdiction	Lake County
East/West Street	Number 2 Rd/W Central Ave
North/South Street	Palm Ave/SR 19
Peak Hour Factor	0.86
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	0	1	0	0	0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		147	8	49		11	8	44		76	586	15		61	677	209
Percent Heavy Vehicles (%)		3	3	14		3	3	3		21				7		
Proportion Time Blocked																
Percent Grade (%)	0				0											
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		7.1	6.5	6.2		7.1	6.5	6.2		4.1				4.1		
Critical Headway (sec)		7.13	6.53	6.34		7.13	6.53	6.23		4.31				4.17		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.43		3.53	4.03	3.33		2.39				2.26		

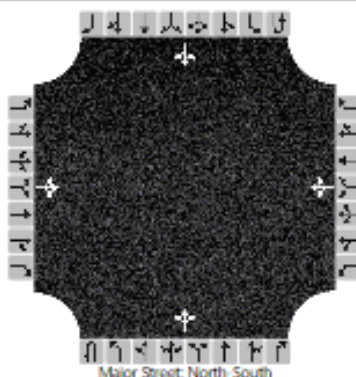
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			237				73				88				71		
Capacity, c (veh/h)			29				83				606				875		
v/c Ratio			8.06				0.88				0.15				0.08		
95% Queue Length, Q ₉₅ (veh)			29.0				4.7				0.5				0.3		
Control Delay (s/veh)			3438.3				155.9				12.0	2.9	2.9		9.5	1.9	1.9
Level of Service (LOS)			F				F				B	A	A		A	A	A
Approach Delay (s/veh)	3438.3			155.9				3.9				2.4					
Approach LOS	F			F				A				A					

HCS Two-Way Stop-Control Report

General Information		Site Information	
Analyst	SS	Intersection	Palm Ave and Central Ave
Agency/Co.	TPD, Inc.	Jurisdiction	Lake County
Date Performed	12/22/2022	East/West Street	Number 2 Rd/W Central Ave
Analysis Year	2028	North/South Street	Palm Ave/SR 19
Time Analyzed	Projected AM	Peak Hour Factor	0.87
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	5659		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	0	1	0	0	0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		184	7	95		10	3	11		42	572	14		29	532	83
Percent Heavy Vehicles (%)		3	3	14		3	3	3		21				7		
Proportion Time Blocked																
Percent Grade (%)	0				0											
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		7.1	6.5	6.2		7.1	6.5	6.2		4.1				4.1		
Critical Headway (sec)		7.13	6.53	6.34		7.13	6.53	6.23		4.31				4.17		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.43		3.53	4.03	3.33		2.39				2.26		

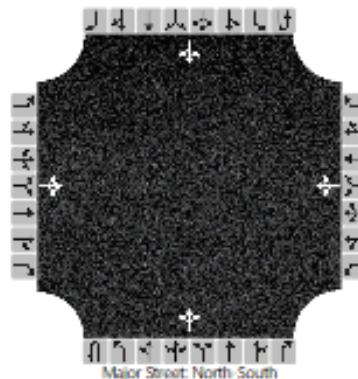
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			329				28			48				33		
Capacity, c (veh/h)			114				115			810				894		
v/c Ratio			2.90				0.24			0.06				0.04		
95% Queue Length, Q ₉₅ (veh)			30.9				0.9			0.2				0.1		
Control Delay (s/veh)			935.5				45.8			9.7	0.9	0.9		9.2	0.6	0.6
Level of Service (LOS)			F				E			A	A	A		A	A	A
Approach Delay (s/veh)	935.5				45.8				1.5				1.0			
Approach LOS	F				E				A				A			

HCS Two-Way Stop-Control Report

General Information		Site Information	
Analyst	SS	Intersection	Palm Ave and Central Ave
Agency/Co.	TPD, Inc.	Jurisdiction	Lake County
Date Performed	12/22/2022	East/West Street	Number 2 Rd/W Central Ave
Analysis Year	2028	North/South Street	Palm Ave/SR 19
Time Analyzed	Projected PM	Peak Hour Factor	0.86
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	5659		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	0	1	0	0	0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume (veh/h)		167	8	70		11	8	44		112	586	15		61	677	243
Percent Heavy Vehicles (%)		3	3	14		3	3	3		21				7		
Proportion Time Blocked																
Percent Grade (%)	0				0											
Right Turn Channelized																
Median Type Storage	Undivided															

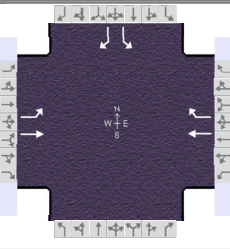
Critical and Follow-up Headways

Base Critical Headway (sec)		7.1	6.5	6.2		7.1	6.5	6.2		4.1				4.1		
Critical Headway (sec)		7.13	6.53	6.34		7.13	6.53	6.23		4.31				4.17		
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2				2.2		
Follow-Up Headway (sec)		3.53	4.03	3.43		3.53	4.03	3.33		2.39				2.26		

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			285				73			130				71		
Capacity, c (veh/h)			20				55			585				875		
v/c Ratio			14.20				1.32			0.22				0.08		
95% Queue Length, Q ₉₅ (veh)			36.1				6.5			0.8				0.3		
Control Delay (s/veh)			6309.4				352.9			12.9	4.7	4.7		9.5	2.1	2.1
Level of Service (LOS)			F				F			B	A	A		A	A	A
Approach Delay (s/veh)	6309.4				352.9				6.0				2.5			
Approach LOS	F				F				A				A			

HCS Signalized Intersection Results Summary

General Information				Intersection Information		
Agency	TPD, Inc.			Duration, h	0.250	
Analyst	SS	Analysis Date	Dec 22, 2022	Area Type	Other	
Jurisdiction	Lake County	Time Period	Projected AM	PHF	0.89	
Urban Street	CR 48	Analysis Year	2028	Analysis Period	1> 7:15	
Intersection	SR 19	File Name	CR 48 and SR 19 - Projected AM.xus			
Project Description	5659					

Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	240	108			182	608				384		205

Signal Information											
Cycle, s	105.5	Reference Phase	2								
Offset, s	0	Reference Point	End	Green	11.8	45.0	28.4	0.0	0.0	0.0	
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	4.4	4.4	4.8	0.0	0.0	0.0	
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.1	2.0	2.5	0.0	0.0	0.0	

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase	1	6		2				4
Case Number	1.0	4.0		7.3				9.0
Phase Duration, s	18.3	69.7		51.4				35.7
Change Period, ($Y+R_c$), s	6.5	6.4		6.4				7.3
Max Allow Headway (MAH), s	4.1	4.2		4.2				4.2
Queue Clearance Time (g_s), s	11.1	5.2		47.0				27.7
Green Extension Time (g_e), s	0.8	4.8		0.0				0.7
Phase Call Probability	1.00	1.00		1.00				1.00
Max Out Probability	0.00	0.00		1.00				1.00

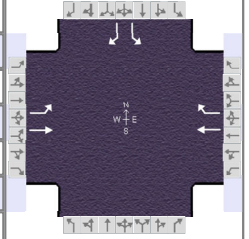
Movement Group Results	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	1	6		2	12					7		14
Adjusted Flow Rate (v), veh/h	270	121		204	683					431		230
Adjusted Saturation Flow Rate (s), veh/h/ln	1654	1737		1856						1725		1510
Queue Service Time (g_s), s	9.1	3.2		7.5						25.7		13.9
Cycle Queue Clearance Time (g_c), s	9.1	3.2		7.5						25.7		13.9
Green Ratio (g/C)	0.56	0.60		0.43						0.27		0.27
Capacity (c), veh/h	643	1043		792						465		407
Volume-to-Capacity Ratio (X)	0.420	0.116		0.258						0.928		0.566
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)	5.7	2.0		5.6						19.6		8.9
Queue Storage Ratio (RQ) (95 th percentile)	0.00	0.00		0.00						0.00		0.00
Uniform Delay (d_1), s/veh	12.9	9.0		19.5						37.5		33.2
Incremental Delay (d_2), s/veh	0.4	0.0		0.2						23.5		1.6
Initial Queue Delay (d_3), s/veh	0.0	0.0		0.0						0.0		0.0
Control Delay (d), s/veh	13.3	9.1		19.7	0.0					61.1		34.8
Level of Service (LOS)	B	A		B	A					E		C
Approach Delay, s/veh / LOS	12.0	B		4.5	A		0.0			51.9		D
Intersection Delay, s/veh / LOS	22.2						C					

Multimodal Results	EB			WB			NB			SB		
Pedestrian LOS Score / LOS	0.69	A		1.91	B		1.96	B		1.96	B	
Bicycle LOS Score / LOS	1.13	A		1.95	B							F

HCS Signalized Intersection Results Summary

General Information

Agency	TPD, Inc.			Duration, h	0.250
Analyst	SS	Analysis Date	Dec 22, 2022	Area Type	Other
Jurisdiction	Lake County	Time Period	Projected PM	PHF	0.97
Urban Street	CR 48	Analysis Year	2028	Analysis Period	1> 16:45
Intersection	SR 19	File Name	CR 48 and SR 19 - Projected PM.xus		
Project Description	5659				



Demand Information

	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	274	221			162	522				675		276

Signal Information

Cycle, s	98.3	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	Yes	Simult. Gap E/W	On	Green	11.9	36.2	30.0	0.0	0.0	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.4	4.4	4.8	0.0	0.0	0.0		
				Red	2.1	2.0	2.5	0.0	0.0	0.0		

Timer Results

	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase	1	6		2				4
Case Number	1.0	4.0		7.3				9.0
Phase Duration, s	18.4	61.0		42.6				37.3
Change Period, ($Y+R_c$), s	6.5	6.4		6.4				7.3
Max Allow Headway (MAH), s	4.1	4.2		4.2				4.2
Queue Clearance Time (g_s), s	11.1	8.0		33.9				32.0
Green Extension Time (g_e), s	0.8	4.0		2.3				0.0
Phase Call Probability	1.00	1.00		1.00				1.00
Max Out Probability	0.00	0.00		0.24				1.00

Movement Group Results

	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	1	6			2	12				7		14
Adjusted Flow Rate (v), veh/h	282	228			167	538				696		285
Adjusted Saturation Flow Rate (s), veh/h/ln	1781	1900			1856					1753		1585
Queue Service Time (g_s), s	9.1	6.0			6.1					30.0		15.0
Cycle Queue Clearance Time (g_c), s	9.1	6.0			6.1					30.0		15.0
Green Ratio (g/C)	0.51	0.56			0.37					0.31		0.31
Capacity (c), veh/h	662	1056			684					535		484
Volume-to-Capacity Ratio (X)	0.427	0.216			0.244					1.301		0.588
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)	6.2	4.2			4.6					51.0		9.7
Queue Storage Ratio (RQ) (95 th percentile)	0.00	0.00			0.00					0.00		0.00
Uniform Delay (d_1), s/veh	14.5	11.0			21.5					34.2		28.9
Incremental Delay (d_2), s/veh	0.4	0.1			0.2					148.8		1.9
Initial Queue Delay (d_3), s/veh	0.0	0.0			0.0					0.0		0.0
Control Delay (d), s/veh	14.9	11.1			21.7	0.0				182.9		30.8
Level of Service (LOS)	B	B			C	A				F		C
Approach Delay, s/veh / LOS	13.2	B		5.1	A		0.0			138.8		F
Intersection Delay, s/veh / LOS	66.7						E					

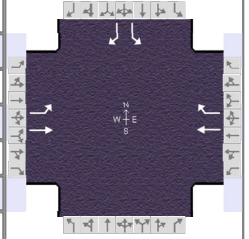
Multimodal Results

	EB			WB			NB			SB		
Pedestrian LOS Score / LOS	0.69	A		1.92	B		1.95	B		1.95	B	
Bicycle LOS Score / LOS	1.33	A		1.65	B						F	

HCS Signalized Intersection Results Summary

General Information

Agency	TPD, Inc.			Duration, h	0.250
Analyst	SS	Analysis Date	Dec 22, 2022	Area Type	Other
Jurisdiction	Lake County	Time Period	Projected AM	PHF	0.89
Urban Street	CR 48	Analysis Year	2028	Analysis Period	1> 7:15
Intersection	SR 19	File Name	CR 48 and SR 19 - Projected AM Optimized Sign...		
Project Description	5659 - Optimized Signal Timing				



Demand Information

	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	240	108			182	608				384		205

Signal Information

Cycle, s	63.4	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	Yes	Simult. Gap E/W	On	Green	9.0	15.0	19.2	0.0	0.0	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.4	4.4	4.8	0.0	0.0	0.0		
				Red	2.1	2.0	2.5	0.0	0.0	0.0		

Timer Results

	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase	1	6		2				4
Case Number	1.0	4.0		7.3				9.0
Phase Duration, s	15.5	36.9		21.4				26.5
Change Period, (Y+R _c), s	6.5	6.4		6.4				7.3
Max Allow Headway (MAH), s	4.1	4.2		4.2				4.2
Queue Clearance Time (g _s), s	9.3	4.5		17.0				16.7
Green Extension Time (g _e), s	0.0	3.4		0.0				2.5
Phase Call Probability	0.99	1.00		1.00				1.00
Max Out Probability	1.00	0.33		1.00				0.02

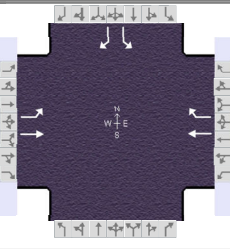
Movement Group Results

	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	1	6			2	12				7		14
Adjusted Flow Rate (v), veh/h	270	121			204	683				431		230
Adjusted Saturation Flow Rate (s), veh/h/ln	1654	1737			1856					1725		1510
Queue Service Time (g _s), s	7.3	2.5			6.0					14.7		8.0
Cycle Queue Clearance Time (g _c), s	7.3	2.5			6.0					14.7		8.0
Green Ratio (g/C)	0.41	0.48			0.24					0.30		0.30
Capacity (c), veh/h	503	835			439					523		457
Volume-to-Capacity Ratio (X)	0.536	0.145			0.466					0.826		0.504
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)	4.4	1.4			4.4					9.7		4.7
Queue Storage Ratio (RQ) (95 th percentile)	0.00	0.00			0.00					0.00		0.00
Uniform Delay (d ₁), s/veh	13.9	9.2			20.8					20.5		18.2
Incremental Delay (d ₂), s/veh	1.1	0.1			0.8					3.4		0.9
Initial Queue Delay (d ₃), s/veh	0.0	0.0			0.0					0.0		0.0
Control Delay (d), s/veh	15.0	9.3			21.5	0.0				23.9		19.0
Level of Service (LOS)	B	A			C	A				C		B
Approach Delay, s/veh / LOS	13.2	B		5.0	A		0.0			22.2		C
Intersection Delay, s/veh / LOS	12.5						B					

Multimodal Results

	EB			WB			NB			SB		
Pedestrian LOS Score / LOS	0.69	A		1.91	B		1.94	B		1.94		B
Bicycle LOS Score / LOS	1.13	A		1.95	B							F

HCS Signalized Intersection Results Summary

General Information				Intersection Information		
Agency	TPD, Inc.			Duration, h	0.250	
Analyst	SS	Analysis Date	Dec 22, 2022	Area Type	Other	
Jurisdiction	Lake County	Time Period	Projected PM	PHF	0.97	
Urban Street	CR 48	Analysis Year	2028	Analysis Period	1> 16:45	
Intersection	SR 19	File Name	CR 48 and SR 19 - Projected PM Optimized Sign...			
Project Description	5659 - Optimized Signal Timing					

Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	274	221			162	522				675		276

Signal Information											
Cycle, s	76.6	Reference Phase	2								
Offset, s	0	Reference Point	End								
Uncoordinated	Yes	Simult. Gap E/W	On								
Force Mode	Fixed	Simult. Gap N/S	On								

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase	1	6		2				4
Case Number	1.0	4.0		7.3				9.0
Phase Duration, s	15.5	36.9		21.4				39.7
Change Period, ($Y+R_c$), s	6.5	6.4		6.4				7.3
Max Allow Headway (MAH), s	4.1	4.2		4.2				4.2
Queue Clearance Time (g_s), s	11.0	8.3		17.0				31.1
Green Extension Time (g_e), s	0.0	2.3		0.0				1.3
Phase Call Probability	1.00	1.00		1.00				1.00
Max Out Probability	1.00	0.60		1.00				1.00

Movement Group Results	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	1	6		2	12					7		14
Adjusted Flow Rate (v), veh/h	282	228		167	538					696		285
Adjusted Saturation Flow Rate (s), veh/h/ln	1781	1900		1856						1753		1585
Queue Service Time (g_s), s	9.0	6.3		6.1						29.1		9.7
Cycle Queue Clearance Time (g_c), s	9.0	6.3		6.1						29.1		9.7
Green Ratio (g/C)	0.34	0.40		0.20						0.42		0.42
Capacity (c), veh/h	445	756		363						742		671
Volume-to-Capacity Ratio (X)	0.635	0.301		0.460						0.938		0.424
Back of Queue (Q), ft/ln (95 th percentile)												
Back of Queue (Q), veh/ln (95 th percentile)	7.1	4.5		4.7						20.6		5.9
Queue Storage Ratio (RQ) (95 th percentile)	0.00	0.00		0.00						0.00		0.00
Uniform Delay (d_1), s/veh	20.8	15.8		27.2						21.1		15.5
Incremental Delay (d_2), s/veh	3.0	0.2		0.9						18.5		0.4
Initial Queue Delay (d_3), s/veh	0.0	0.0		0.0						0.0		0.0
Control Delay (d), s/veh	23.8	16.0		28.1	0.0					39.6		16.0
Level of Service (LOS)	C	B		C	A					D		B
Approach Delay, s/veh / LOS	20.3	C		6.7	A		0.0			32.7	C	
Intersection Delay, s/veh / LOS	21.5						C					

Multimodal Results	EB			WB			NB			SB		
Pedestrian LOS Score / LOS	0.71	A		1.93	B		1.94	B		1.94	B	
Bicycle LOS Score / LOS	1.33	A		1.65	B							F

HCS Two-Way Stop-Control Report

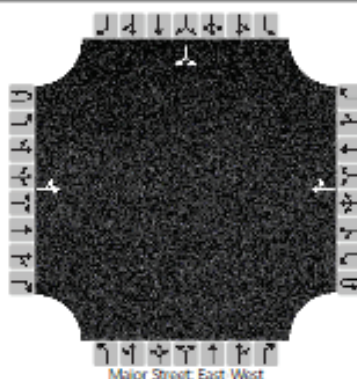
General Information

Analyst	SS
Agency/Co.	TPD, Inc.
Date Performed	12/22/2022
Analysis Year	2028
Time Analyzed	Projected AM
Intersection Orientation	East-West
Project Description	5659

Site Information

Intersection	Number 2 Rd Site Access
Jurisdiction	Lake County
East/West Street	Number 2 Rd
North/South Street	Site Access
Peak Hour Factor	0.92
Analysis Time Period (hrs)	0.25

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration		LT						TR							LR	
Volume (veh/h)		11	53				120	21						62		33
Percent Heavy Vehicles (%)		3												3		3
Proportion Time Blocked																
Percent Grade (%)													0			
Right Turn Channelized																
Median Type Storage					Undivided											

Critical and Follow-up Headways

Base Critical Headway (sec)		4.1												7.1		6.2
Critical Headway (sec)		4.13												6.43		6.23
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.23												3.53		3.33

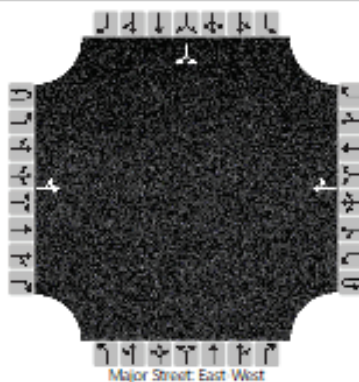
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		12													103	
Capacity, c (veh/h)		1421													801	
v/c Ratio		0.01													0.13	
95% Queue Length, Q ₉₅ (veh)		0.0													0.4	
Control Delay (s/veh)		7.6	0.1												10.2	
Level of Service (LOS)		A	A												B	
Approach Delay (s/veh)	1.4												10.2			
Approach LOS	A												B			

HCS Two-Way Stop-Control Report

General Information		Site Information	
Analyst	SS	Intersection	Number 2 Rd Site Access
Agency/Co.	TPD, Inc.	Jurisdiction	Lake County
Date Performed	12/22/2022	East/West Street	Number 2 Rd
Analysis Year	2028	North/South Street	Site Access
Time Analyzed	Projected PM	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	5659		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration		LT						TR							LR	
Volume (veh/h)		38	120				86	71						42		22
Percent Heavy Vehicles (%)		3												3		3
Proportion Time Blocked																
Percent Grade (%)													0			
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)		4.1												7.1		6.2
Critical Headway (sec)		4.13												6.43		6.23
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.23												3.53		3.33

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		41													70	
Capacity, c (veh/h)		1400													704	
v/c Ratio		0.03													0.10	
95% Queue Length, Q ₉₅ (veh)		0.1													0.3	
Control Delay (s/veh)		7.6	0.2												10.7	
Level of Service (LOS)		A	A												B	
Approach Delay (s/veh)	2.0												10.7			
Approach LOS	A												B			

APPENDIX G

Right and Left Turn Lanes Warrant Charts

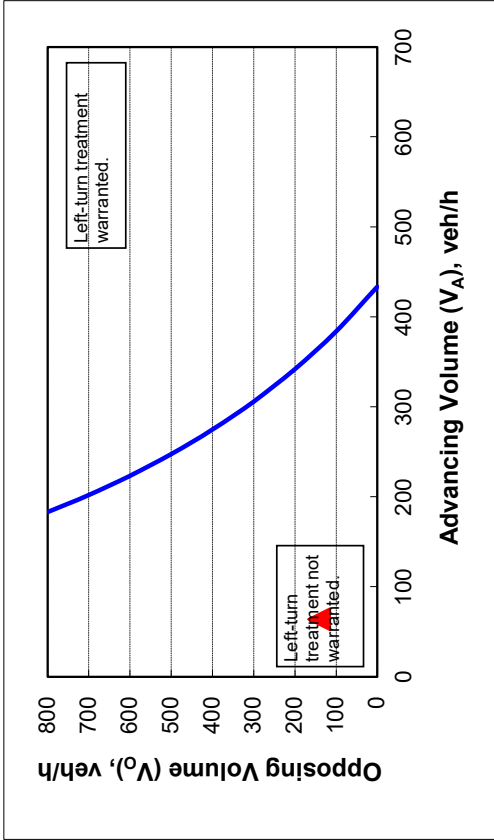
Figure 2 - 5. Guideline for determining the need for a major-road left-turn bay at a two-way stop-controlled intersection.

2-lane roadway (English)

INPUT	
Variable	Value
85 th percentile speed, mph:	45
Percent of left-turns in advancing volume (V_A), %:	17%
Advancing volume (V_A), veh/h:	64
Opposing volume (V_O), veh/h:	141

OUTPUT

Variable	Value
Limiting advancing volume (V_A), veh/h:	366
Guidance for determining the need for a major-road left-turn bay: Left-turn treatment NOT warranted.	



CALIBRATION CONSTANTS

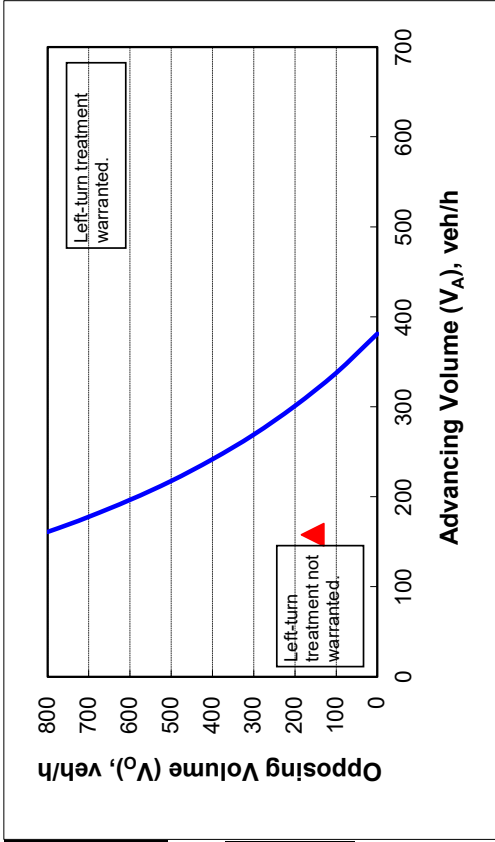
Variable	Value
Average time for making left-turn, s:	3.0
Critical headway, s:	5.0
Average time for left-turn vehicle to clear the advancing lane, s:	1.9

Figure 2 - 5. Guideline for determining the need for a major-road left-turn bay at a two-way stop-controlled intersection.

2-lane roadway (English)

INPUT	
Variable	Value
85 th percentile speed, mph:	45
Percent of left-turns in advancing volume (V_A), %:	24%
Advancing volume (V_A), veh/h:	158
Opposing volume (V_O), veh/h:	157

OUTPUT	
Variable	Value
Limiting advancing volume (V_A), veh/h:	316
Guidance for determining the need for a major-road left-turn bay: Left-turn treatment NOT warranted.	



CALIBRATION CONSTANTS	
Variable	Value
Average time for making left-turn, s:	3.0
Critical headway, s:	5.0
Average time for left-turn vehicle to clear the advancing lane, s:	1.9

Figure 2 - 6. Guideline for determining the need for a major-road right-turn bay at a two-way stop-controlled intersection.

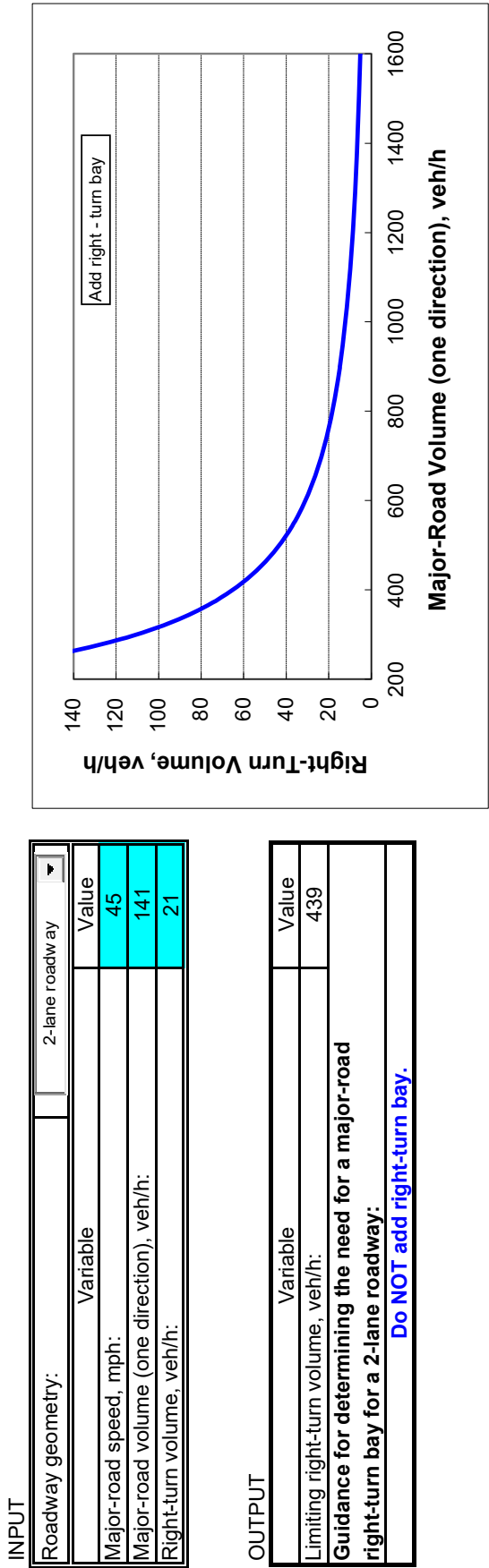


Figure 2 - 6. Guideline for determining the need for a major-road right-turn bay at a two-way stop-controlled intersection.

