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# **TRAFFIC IMPACT ANALYSIS**

## **LEO Haywood Cottages Hendersonville, North Carolina**

**NOVEMBER 12, 2025**

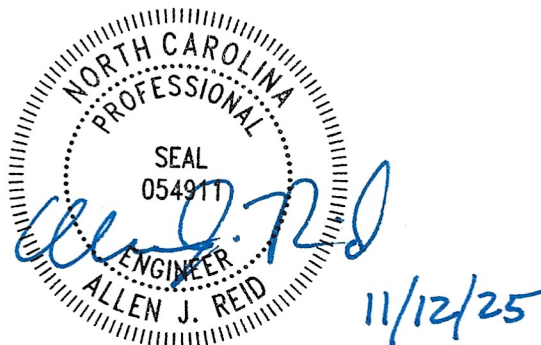
**IMPACT DESIGNS, INC.**

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# TRAFFIC IMPACT ANALYSIS

## *LEO Haywood Cottages*

*HENDERSONVILLE, NORTH CAROLINA*



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## Table of Contents

<b>EXECUTIVE SUMMARY .....</b>	<b>iii</b>
<b>1. INTRODUCTION .....</b>	<b>1</b>
1.1. Project Summary .....	1
1.2. Existing Roadway Conditions.....	2
1.3. Driveway Location.....	2
<b>2. TRAFFIC VOLUME DEVELOPMENT.....</b>	<b>6</b>
2.1. Existing Traffic Volumes.....	6
2.2. Projected Traffic Volumes .....	6
2.3. Proposed Development Traffic Volumes .....	6
2.4. Future Build Traffic Volumes .....	7
<b>3. TRAFFIC IMPACT ANALYSIS .....</b>	<b>13</b>
3.1. Turn Lane Analysis .....	13
3.2. Intersection LOS Analysis.....	13
3.3. Mitigation Requirements .....	14
3.4. Capacity Analysis.....	15
3.5. Queuing Analysis .....	17
<b>4. PEDESTRIAN LOS ANALYSIS.....</b>	<b>19</b>
<b>5. SUMMARY OF FINDINGS AND RECOMMENDATIONS.....</b>	<b>22</b>

## Tables

Table 1 – Study Area Summary .....	2
Table 2 – Trip Generation.....	6
Table 3 – <i>HCM 6<sup>th</sup> Edition</i> LOS Criteria for Signalized & Unsignalized Intersections .....	13
Table 4 – Intersection Capacity Analysis Results.....	15
Table 5 – Queuing Analysis.....	17
Table 6 – Pedestrian LOS Analysis .....	20

## Figures

Figure 1 – Project Location.....	3
Figure 2 – Conceptual Site Plan.....	4
Figure 3 – Existing Lane Configuration .....	5
Figure 4 – Existing (2024) Traffic Volumes.....	8
Figure 5 – No-Build (2028) Traffic Volumes .....	9
Figure 6 – Site Trip Distribution.....	10
Figure 7 – Site Trip Assignments .....	11
Figure 8 – Build (2028) Traffic Volumes .....	12
Figure 9 – Recommended Lane Configurations .....	23

## Appendices

- A) NCDOT TIA Scoping Checklist
- B) Traffic Count Data
- C) Turn Lane Warrant Analysis
- D) Synchro Capacity Analysis Reports
- E) SimTraffic Analysis Reports
- F) Pedestrian Analysis Reports
- G) Signal Information

## EXECUTIVE SUMMARY

A traffic impact study was conducted for the proposed LEO Haywood Cottages development in accordance with NCDOT guidelines. The proposed development is located on the south side of Haywood Road (NC 191), east of Blythe Street, in Hendersonville, North Carolina. The development is expected to consist of 180 single family attached homes and would be completed by the end of 2028. Access to the site is to be provided via a full movement access on Haywood Road.

The study was determined through coordination with NCDOT and the City of Hendersonville and consists of the following intersections:

- Asheville Highway (US 25 Business) and Haywood Road (NC 191)
- Haywood Road (NC 191) and Blythe Street
- Brevard Road/6<sup>th</sup> Avenue West (US 64) and Blythe Street
- Haywood Road (NC 191) and Ewbank Drive
- Haywood Road (NC 191) and Morris Lane
- Haywood Road (NC 191) and Ridgewood Boulevard / Whitmire Circle
- Haywood Road (NC 191) and Orleans Avenue
- Haywood Road (NC 191) and N. Justice Street
- Haywood Road (NC 191) and Site Access

For the purpose of this analysis, the study intersections listed above were analyzed under the following scenarios:

- Existing (2024) Conditions
- No-Build (2028) Conditions
- Build (2028) Conditions

Traffic operations during the AM and PM peak hours were modeled for each scenario. The results of each scenario were compared to determine impacts from background traffic growth and the proposed development.

### Recommendations:

- Construct a westbound left turn lane on Haywood Road at the site access with at 100 feet of full width storage. Final design to be coordinated with NCDOT during permitting.
- Construct an eastbound right turn lane on Haywood Road at the site access with 75 feet of full width storage. Final design to be coordinated with NCDOT during permitting.

# 1. INTRODUCTION

The purpose of this report is to summarize the traffic impact analysis that was completed for the proposed LEO Haywood Cottages development in Hendersonville, North Carolina. The study was developed in accordance with NCDOT guidelines. The purpose of the study is to determine the potential impact to the surrounding transportation system caused by the traffic generated by the development. This report summarizes the procedures and findings of the traffic impact study.

## 1.1. Project Summary

The proposed development is located on the south side of Haywood Road, east of Blythe Street, in Hendersonville, North Carolina. The development is expected to consist of 180 single family attached units and would be completed by the end of 2028. This traffic impact study analyzes the effects of the additional traffic associated with the proposed development during the weekday AM (7:00 AM - 9:00 AM) and the weekday PM (4:00 PM - 6:00 PM) peak periods. The study area for the purpose of the analysis includes:

- Asheville Highway (US 25 Business) and Haywood Road (NC 191)
- Haywood Road (NC 191) and Blythe Street
- Brevard Road/6<sup>th</sup> Avenue West (US 64) and Blythe Street
- Haywood Road (NC 191) and Ewbank Drive
- Haywood Road (NC 191) and Morris Lane
- Haywood Road (NC 191) and Ridgewood Boulevard / Whitmire Circle
- Haywood Road (NC 191) and Orleans Avenue
- Haywood Road (NC 191) and N. Justice Street
- Haywood Road (NC 191) and Site Access

Refer to Figures 1 and 2 for the site location and the conceptual site plan.

For the purpose of this analysis, the study intersections listed above were analyzed under the following scenarios:

- Existing (2024) Conditions
- No-Build (2028) Conditions
- Build (2028) Conditions

Refer to Appendix A for a copy of the NCDOT TIA Scoping Checklist Scoping Form.

## 1.2. Existing Roadway Conditions

The primary roadways within the study area are Asheville Highway, Haywood Road, US 64 and Blythe Street. A summary of the existing characteristics is shown in Table 1.

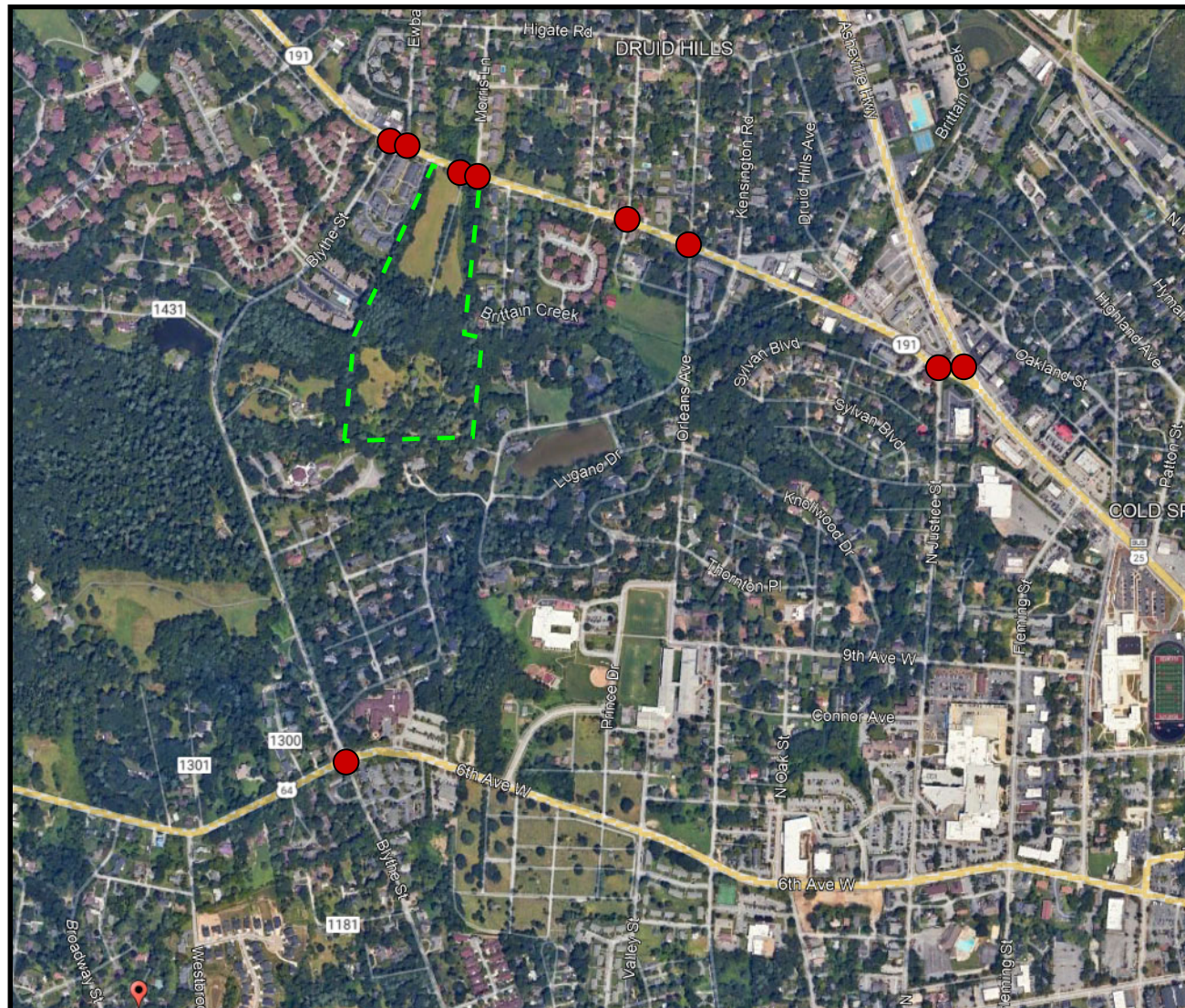
**Table 1 – Study Area Summary**

Facility Name	Route #	Typical Cross Section	Posted Speed Limit	Maintained By	AADT
Asheville Highway	US 25 Business	5-lane undivided	35 MPH	NCDOT	28,500 (2022)
Haywood Road	NC 191	2-lane undivided	35 MPH	NCDOT	11,000 (2022)
Brevard Road/ 6 <sup>th</sup> Avenue West	US 64	2-/3-lane undivided	35 MPH	NCDOT	14,000 (2022)
Blythe Street	SR 2162	2-lane undivided	35 MPH	NCDOT	6,200 (2022)

Refer to Figure 3 for an illustration of the existing lane geometry and traffic control at the study intersections.

## 1.3. Driveway Location

Direct access to the site is to be provided via a full movement access on Haywood Road.



### LEGEND

- - - Proposed Site Location
- Study Intersections

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Site Location Map

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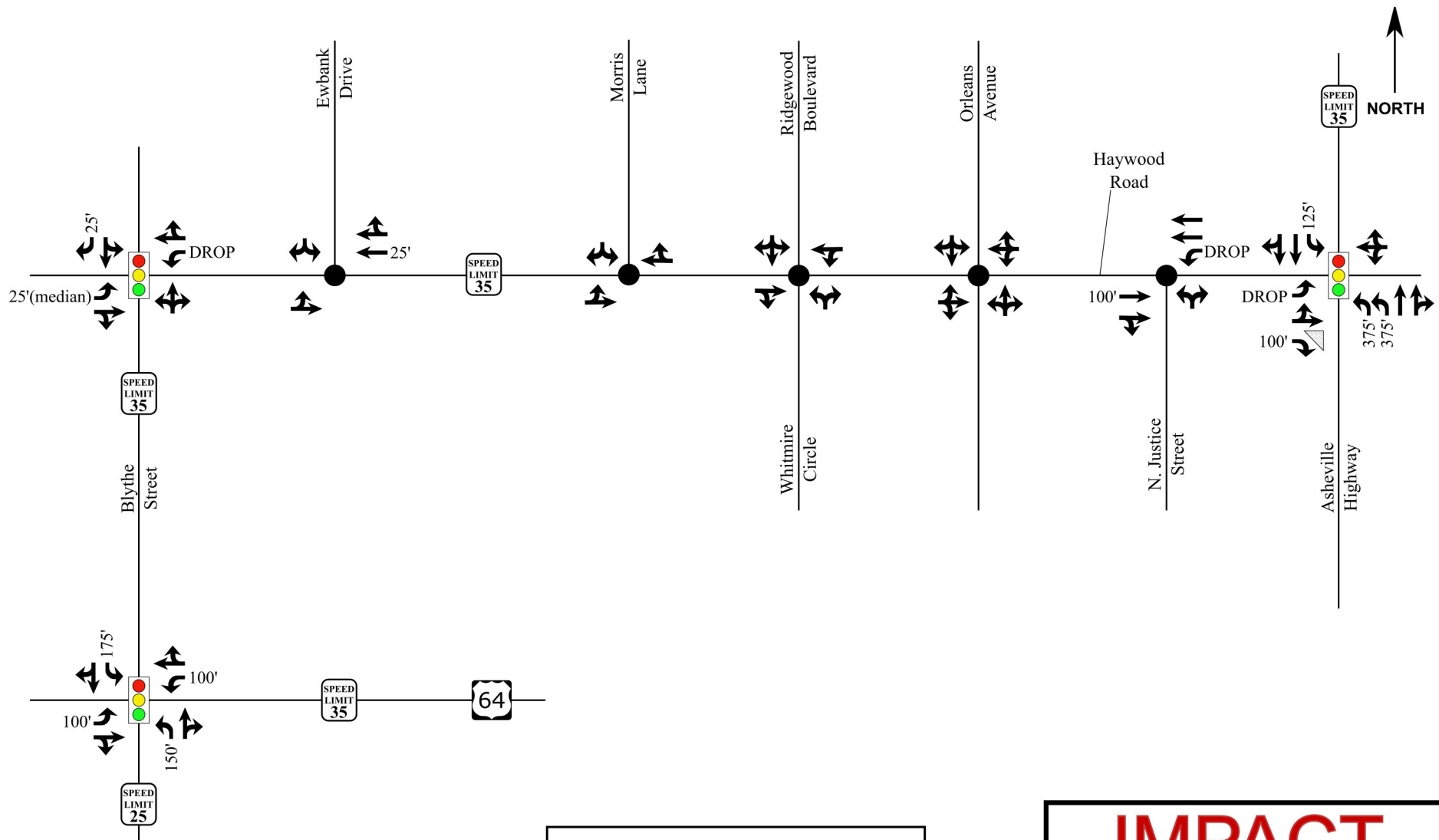
Figure

1









### LEGEND



Signalized Intersection



Unsignalized Intersection



Existing Lane



Storage (In Feet)



Posted Speed Limit

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Existing Lane Configurations  
and Traffic Control

Scale: Not to Scale

Figure

3

## 2. TRAFFIC VOLUME DEVELOPMENT

### 2.1. Existing Traffic Volumes

Existing turning movement counts were conducted at the intersections during the weekday AM (7:00 AM to 9:00 AM) and weekday PM (4:00 PM to 6:00 PM) peak periods in December of 2024 and in September of 2025. To be conservative the Existing (2024) traffic volumes assumed all counts to be collected in 2024. The Existing (2024) traffic volumes are illustrated in Figure 4. Refer to Appendix B for a copy of the raw traffic count data.

### 2.2. Projected Traffic Volumes

Based on coordination with NCDOT, a 1% annual growth was applied to the 2024 counts to project traffic volumes for the future year (2028). This growth rate was applied to account for all background growth in the area without any adjacent and/or the proposed developments. Refer to Figure 5 for an illustration of the No-Build (2028) traffic volumes.

### 2.3. Proposed Development Traffic Volumes

As mentioned previously, the proposed development is expected to consist of 180 single family attached homes and would be completed by the end of 2028. The trip generation potential for the development was estimated utilizing methodology contained within the ITE's *Trip Generation Manual*, 11<sup>th</sup> Edition. Utilizing ITE equations for ITE Code 215 traffic volumes were generated for the weekday daily, the weekday AM peak hour, and the weekday PM peak hour. Refer to Table 2 for a summary of the trip generation potential of the proposed development.

**Table 2 – Trip Generation**

ITE Land Use (Code)	Density	Independent Variable	Daily Traffic	AM Peak		PM Peak	
				Enter	Exit	Enter	Exit
Single Family Attached Housing (ITE Code 215)	180	Dwelling Units	1,321	22	66	61	43

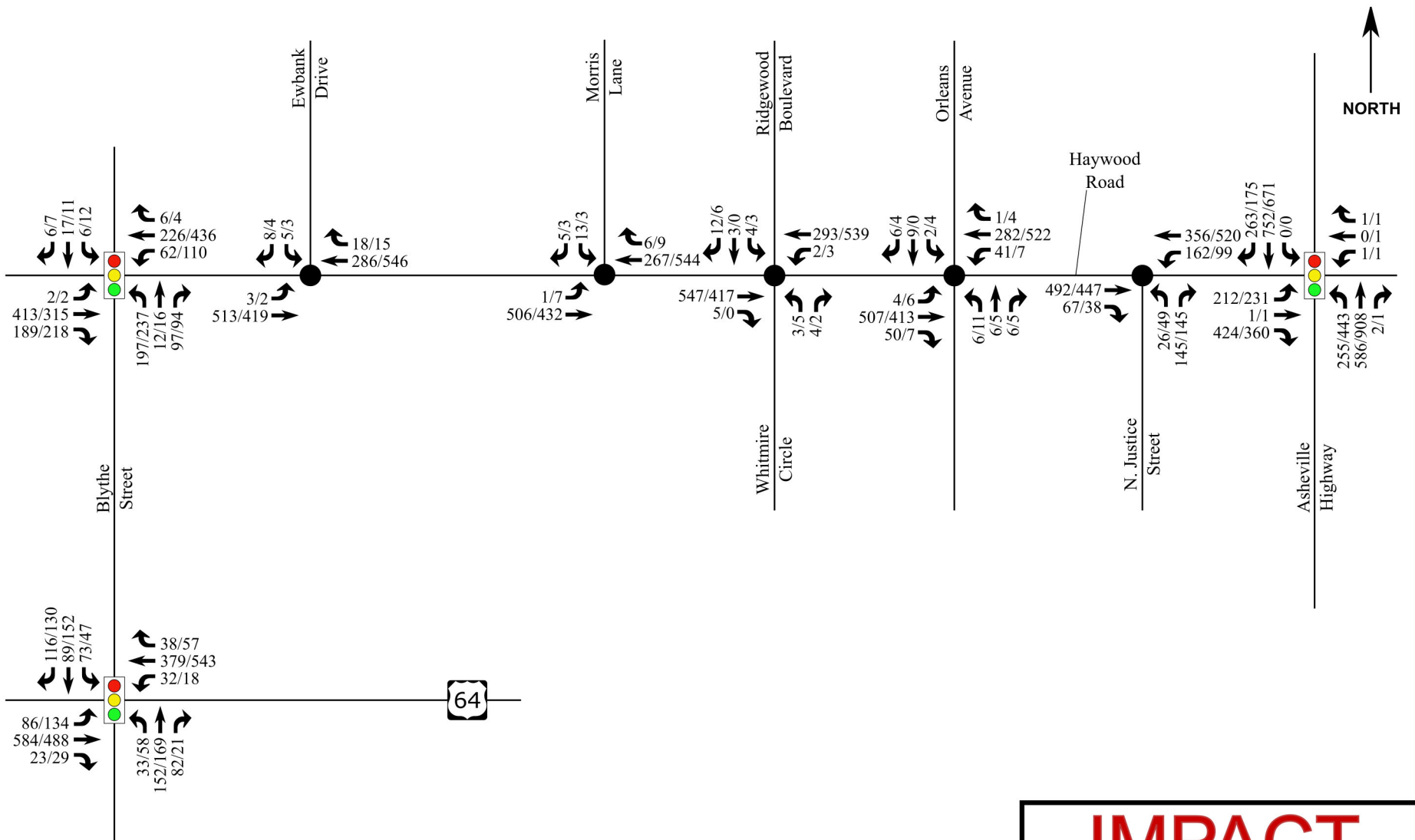
Site traffic associated with the proposed development was distributed and assigned to the roadway network based upon existing travel patterns and are summarized below:

- 20% to/from the north via Asheville Highway
- 25% to/from the south via Asheville Highway
- 10% to/from the west via Haywood Road
- 5% to/from the west via US 64
- 20% to & 30% from the east via US 64
- 5% to/from the south via Blythe Street
- 10% to the south via N. Justice Street

Refer to Figures 6 and 7 for illustrations of the site trip distributions and assignments.

## **2.4. Future Build Traffic Volumes**

The site generated traffic volumes were added to the No-Build traffic volumes to determine the Build traffic volumes. The Build (2028) volumes are illustrated in Figure 8.



**LEGEND**

- Signalized Intersection
- Unsignalized Intersection
- X / Y → AM / PM Peak Hour Traffic

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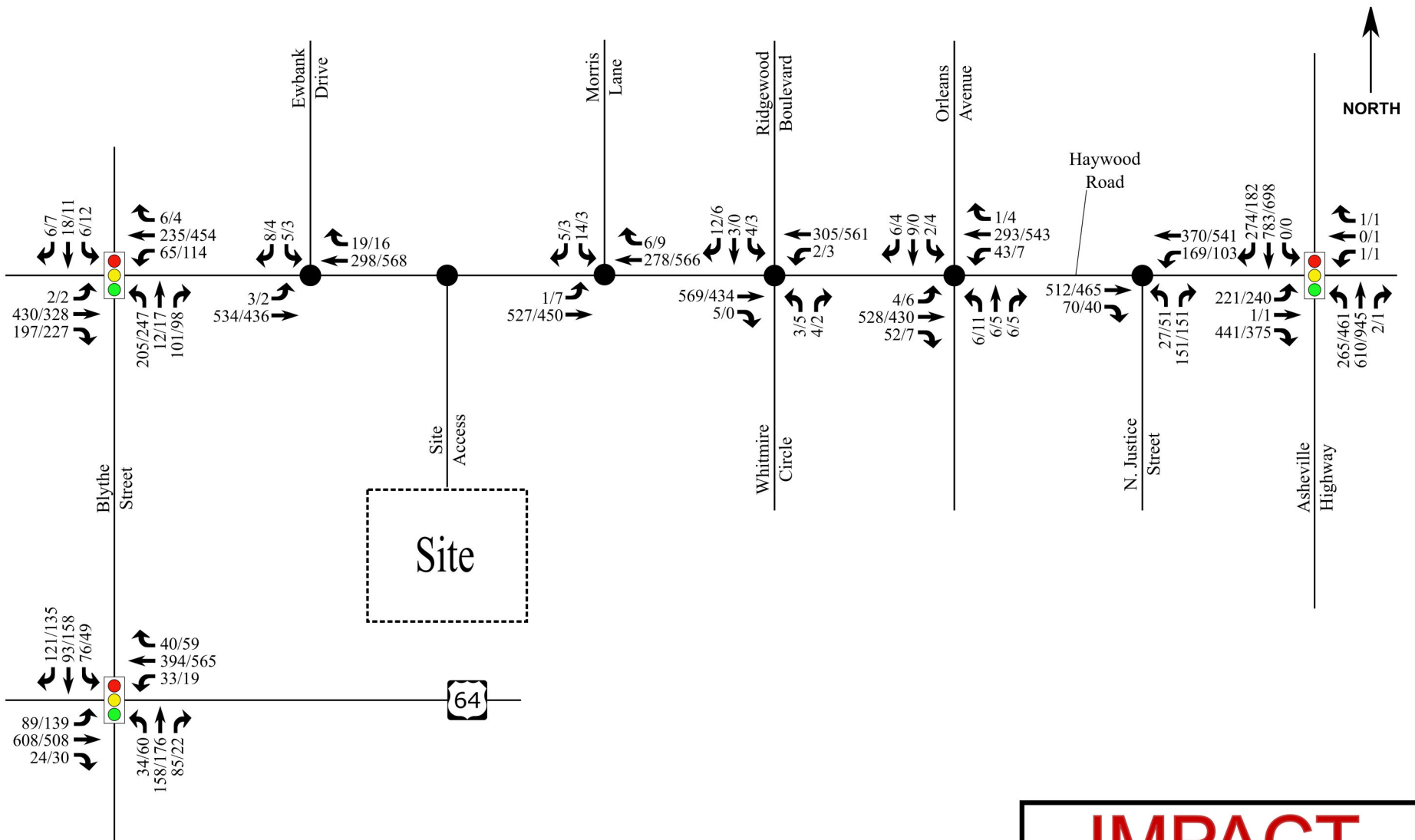
Existing (2024)

Traffic Volumes



Scale: Not to Scale

Figure

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**LEGEND**

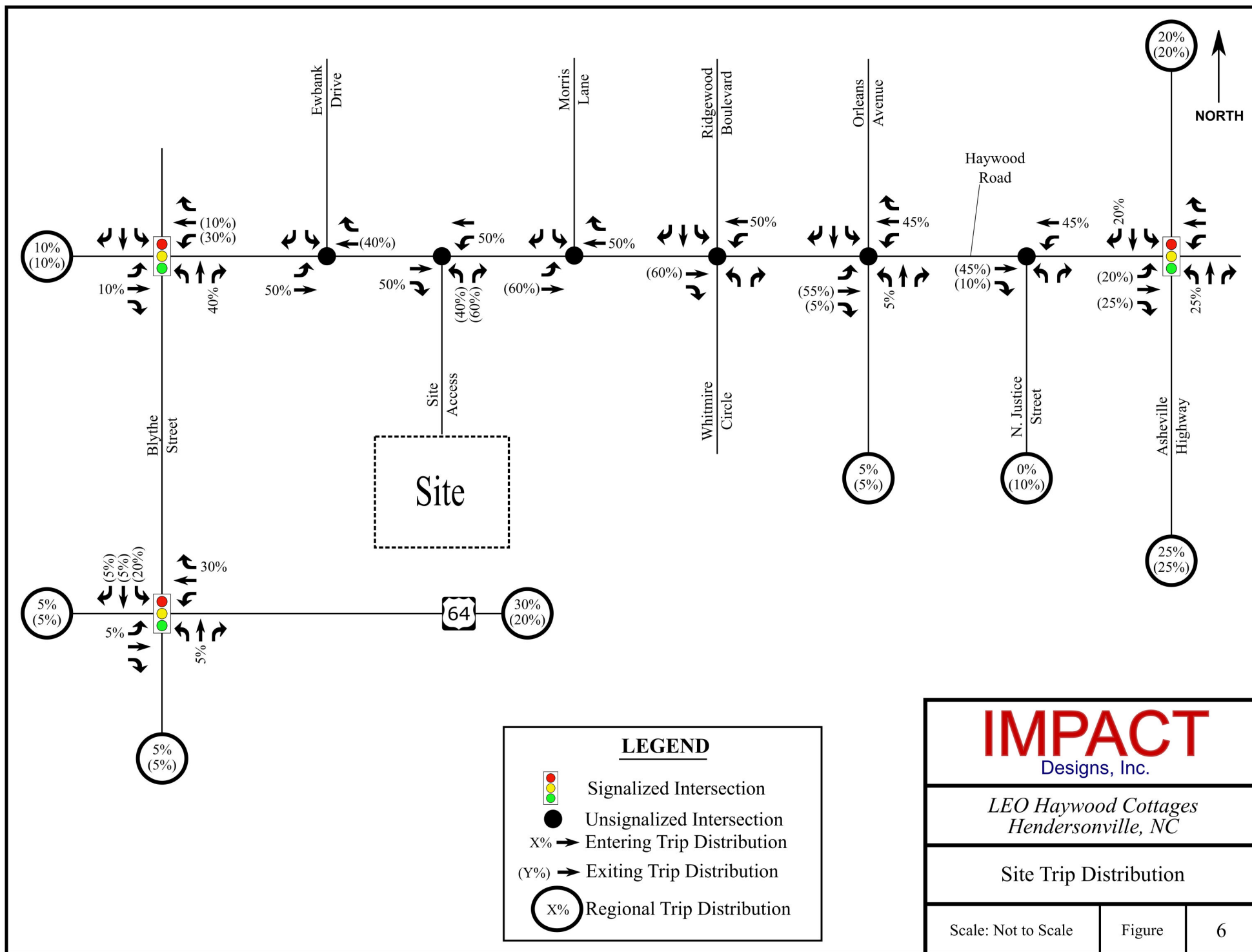
 Signalized Intersection  
 Unsignalized Intersection  
 X / Y → AM / PM Peak Hour Traffic

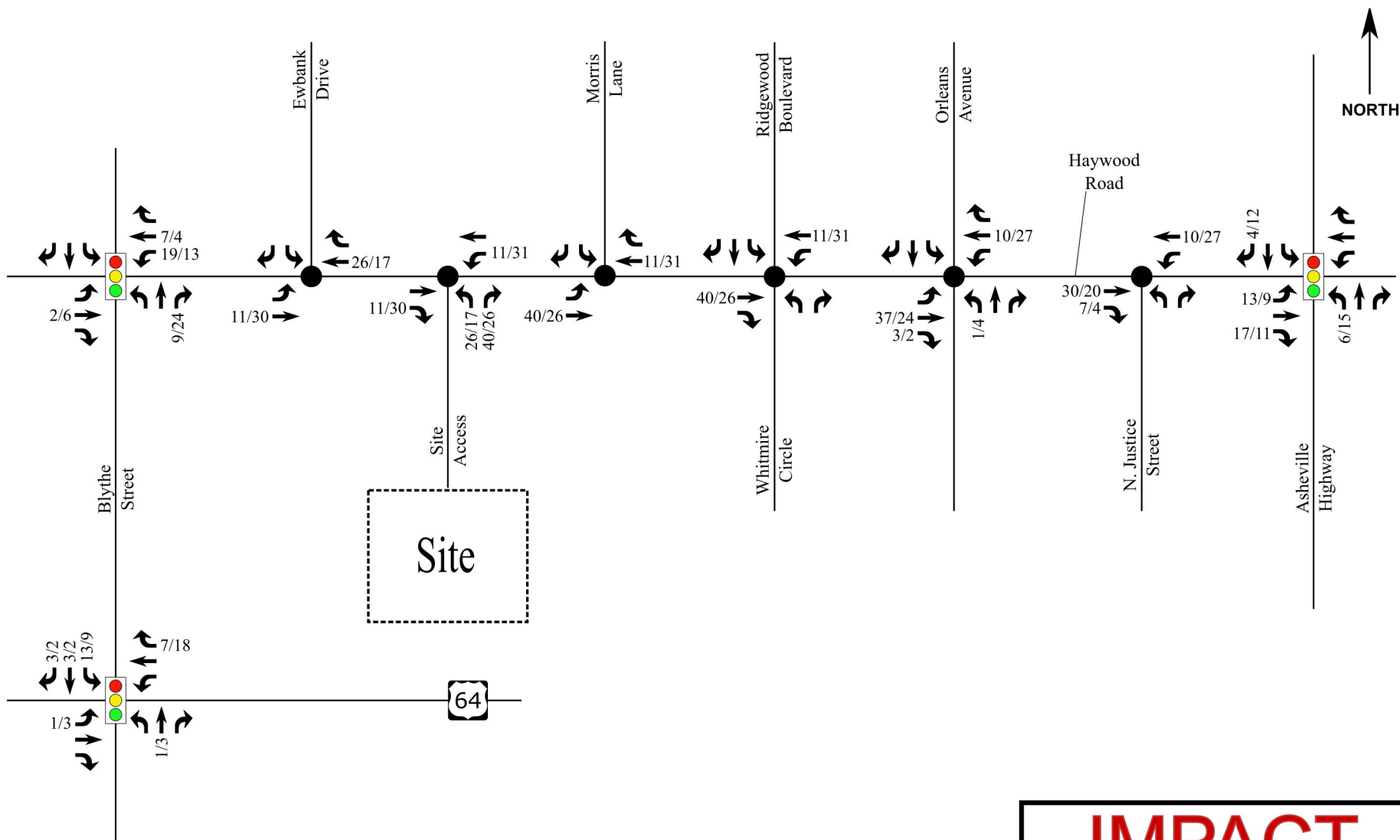
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

No-Build (2028)  
 Traffic Volumes

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### LEGEND

-  Signalized Intersection
-  Unsignalized Intersection
- X / Y → AM / PM Peak Hour Trips

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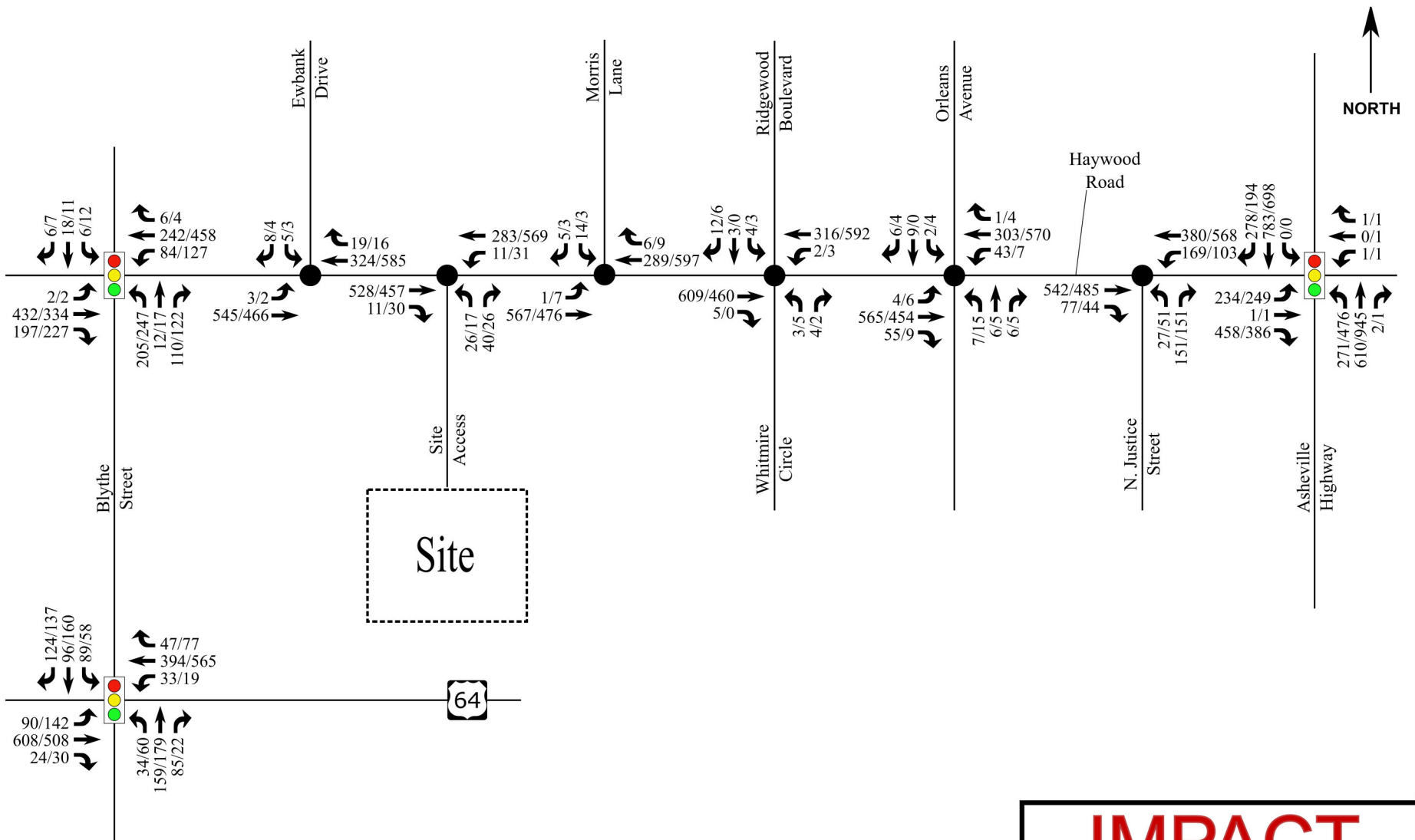
Trip Assignments

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Figure

7





### LEGEND



Signalized Intersection



Unsignalized Intersection

X / Y → AM / PM Peak Hour Traffic

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**Build (2028)  
Traffic Volumes**

Scale: Not to Scale

Figure

8

### 3. TRAFFIC IMPACT ANALYSIS

#### 3.1. Turn Lane Analysis

A turn lane analysis was conducted for the site access utilizing the Build (2028) volumes. Based on build-out volumes, both a left turn lane and a right turn lane are warranted on Haywood Road at the site access.

Based on the NCDOT nomograph, the left turn lane requires at least 50 feet of storage; however, NCDOT is requiring 100 feet of full width storage.

Similarly, the right turn lane requires at least 50 feet of storage per the NCDOT nomograph. Based on feedback from NCDOT, 75 feet of full width storage will be required.

With the proximity of adjacent streets, constructing these turn lanes could present some geometric challenges. Final design of the improvements should be coordinated with NCDOT during permitting. Refer to Appendix C for the turn lane warrant charts with the volumes graphed.

#### 3.2. Intersection LOS Analysis

Using the existing, no-build, and build traffic volumes, intersection analyses were conducted for the study intersections under Existing (2024) conditions, No-Build (2028) conditions, and Build (2028) conditions. This analysis was conducted using the Transportation Research Board's *Highway Capacity Manual 6<sup>th</sup> Edition* (HCM 6<sup>th</sup> Edition) methodologies of the *Synchro*, Version 11 software.

Intersection level of service (LOS) grades range from LOS A to LOS F, which are directly related to the level of control delay at the intersection and characterize the operational conditions of the intersection traffic flow. LOS A operations typically represent ideal, free-flow conditions where vehicles experience little to no delays, and LOS F operations typically represent poor, forced-flow (bumper-to-bumper) conditions with high vehicular delays, and are generally considered undesirable. Table 3 summarizes the HCM 6<sup>th</sup> Edition control delay thresholds associated with each LOS grade for signalized and unsignalized intersections.

**Table 3 – HCM 6<sup>th</sup> Edition LOS Criteria for Signalized & Unsignalized Intersections**

Signalized Intersections		Unsignalized Intersections	
LOS	Control Delay per Vehicle (seconds)	LOS	Control Delay per Vehicle (seconds)
A	$\leq 10$	A	$\leq 10$
B	$> 10$ and $\leq 20$	B	$> 10$ and $\leq 15$
C	$> 20$ and $\leq 35$	C	$> 15$ and $\leq 25$
D	$> 35$ and $\leq 55$	D	$> 25$ and $\leq 35$
E	$> 55$ and $\leq 80$	E	$> 35$ and $\leq 50$
F	$> 85$	F	$> 50$

A PHF of 0.90 was applied and a heavy vehicle percentage of 2% was utilized for the purpose of this analysis. Additionally, a conservative approach was taken in which no right turns on red were permitted, although right turns on red are permitted on all intersections in the field.

### **3.3. Mitigation Requirements**

NCDOT typically requires mitigation to be identified when developments are expected to impact the traffic operations as described below:

- Overall intersection or intersection approach delay increases by 25%.
- LOS degrades by at least one level.
- LOS is F.
- Synchro 95<sup>th</sup> or SimTraffic maximum queue results are greater than the existing turn lane storage length.

### 3.4. Capacity Analysis

The results of the capacity analysis for the study intersections are summarized below in Table 4. Refer to Appendix D for the detailed capacity analysis reports.

**Table 4 – Intersection Capacity Analysis Results**

Intersections	Approach	LOS (Delay in seconds per vehicle)					
		Existing (2024)		No-Build (2028)		Build (2028)	
		AM	PM	AM	PM	AM	PM
Asheville Highway & Haywood Road	EB	C (20.8)	C (24.7)	C (21.6)	C (24.4)	C (21.6)	C (24.7)
	WB	D (53.5)	D (53.5)	D (53.5)	D (53.5)	D (53.5)	D (53.5)
	NB	C (23.0)	C (23.7)	C (23.0)	C (24.5)	C (22.8)	C (24.7)
	SB	B (19.3)	C (23.6)	B (19.6)	C (24.4)	C (21.1)	C (25.4)
	Overall	C (21.1)	C (24.0)	C (21.4)	C (24.6)	C (21.9)	C (25.0)
Haywood Road & Blythe Street	EB	D (39.1)	D (43.5)	D (42.9)	D (47.0)	D (45.6)	D (53.5)
	WB	B (18.0)	C (28.2)	B (18.7)	C (31.0)	B (19.8)	C (32.0)
	NB	E (58.7)	E (63.9)	E (61.4)	E (69.8)	E (72.9)	E (70.2)
	SB	C (25.7)	C (23.5)	C (25.8)	C (23.6)	C (26.6)	C (24.6)
	Overall	D (38.6)	D (42.2)	D (41.3)	D (45.9)	D (45.5)	D (49.0)
US 64 & Blythe Street	EB	D (37.9)	D (42.6)	D (39.8)	D (44.3)	D (40.1)	D (46.7)
	WB	C (34.7)	D (46.1)	D (35.7)	D (47.4)	D (36.3)	D (54.7)
	NB	E (72.4)	F (81.7)	E (75.1)	F (90.9)	E (75.5)	F (93.4)
	SB	E (71.9)	E (74.4)	E (74.6)	F (86.4)	E (75.7)	E (78.7)
	Overall	D (48.1)	D (54.7)	D (50.0)	E (59.1)	D (50.8)	E (61.4)
Haywood Road & Ewbank Drive	EB	A (8.0)	A (8.8)	A (8.0)	A (8.9)	A (8.1)	A (8.9)
	WB	-	-	-	-	-	-
	SB	B (12.6)	C (16.0)	B (12.9)	C (16.6)	B (13.3)	C (17.4)
Haywood Road & Morris Lane	EB	A (7.9)	A (8.8)	A (7.9)	A (8.8)	A (7.9)	A (9.0)
	WB	-	-	-	-	-	-
	SB	C (15.0)	C (16.8)	C (15.7)	C (17.5)	C (16.4)	C (18.4)
Haywood Road & Ridgewood Boulevard / Whitemire Circle	EB	-	-	-	-	-	-
	WB	A (8.7)	A (8.3)	A (8.8)	A (8.4)	A (9.0)	A (8.4)
	NB	C (16.8)	C (18.7)	C (17.5)	C (19.6)	C (18.5)	C (21.1)
	SB	B (14.3)	C (18.8)	B (14.7)	C (19.7)	C (15.4)	C (21.0)
Haywood Road & Orleans Avenue	EB	A (7.9)	A (8.7)	A (7.9)	A (8.7)	A (8.0)	A (8.8)
	WB	A (8.9)	A (8.3)	A (9.0)	A (8.4)	A (9.2)	A (8.8)
	NB	C (19.9)	C (22.2)	C (20.9)	C (23.4)	C (22.9)	D (31.2)
	SB	C (19.3)	C (20.2)	C (20.3)	C (21.1)	C (21.6)	D (25.3)
Haywood Road & N. Justice Street	EB	-	-	-	-	-	-
	WB	A (9.6)	A (8.9)	A (9.8)	A (9.0)	B (10.0)	A (9.1)
	NB	C (18.5)	C (20.6)	C (20.2)	C (22.6)	C (21.6)	C (24.4)
Haywood Road & Site Access	EB	<i>Analyzed under Build conditions only.</i>				-	-
	WB					A (8.7)	A (8.6)
	NB					C (16.3)	C (18.4)

The capacity analysis indicates that the operations at the signalized intersections are expected to be similar under Build conditions as compared to No-Build conditions. All approaches are expected to maintain their levels of service with one exception. In the AM peak hour, the Asheville Highway southbound approach at Haywood Road is expected to drop from LOS B under No-Build conditions to LOS C under Build conditions. However, because the delay is expected to increase by only 8% and would still operate acceptably, mitigation is not recommended.

At the intersection of Haywood Road and Orleans Avenue, the side street approaches are expected to drop from LOS C to LOS D in the PM peak hour. The delay is still expected to be reasonable for an unsignalized intersection during the peak hour. Some of the additional delay could be mitigated by an additional lane on each side street approach, but such an improvement would adversely impact the residences on the corners and would thus not be considered reasonable in this case.

The site egress at Haywood Road is anticipated to operate at LOS C in both peak hours. It should be noted that the site access intersection was modeled with the warranted westbound left turn lane. No additional mitigation is recommended.

### 3.5. Queuing Analysis

A queuing analysis was also completed for all No-Build and Build Improved traffic. Reported in Table 5 are the SimTraffic maximum queue for each turn lane at study intersections. Additional storage is recommended where the site traffic introduced in the Build scenario extends the queue beyond the available storage. Refer to Appendix D for detailed Synchro capacity analysis reports and Appendix E for detailed SimTraffic reports.

**Table 5 – Queuing Analysis**

Intersections	Lane Group	Storage (feet)	Max Queue (feet)			
			AM Peak Hour		PM Peak Hour	
			No-Build	Build Improved	No-Build	Build Improved
Asheville Highway & Haywood Road	EB-L	200	165	210	243	201
	EB-LT	Full	280	391	332	287
	EB-R	100	145	150	142	147
	WB-LTR	Full	57	60	56	56
	NB-L	375	360	353	425	425
	NB-L	375	369	367	475	475
	SB-L	125	68	68	48	123
Haywood Road & Blythe Street	EB-L	25	73	110	93	91
	WB-L	100	124	146	193	217
	NB-LTR	Full	375	397	416	469
	SB-LT	Full	52	52	45	53
	SB-R	25	38	33	31	36
US 64 & Blythe Street	EB-L	100	200	200	255	263
	WB-L	100	184	187	185	187
	NB-L	150	249	214	250	250
	SB-L	175	248	274	275	275
Haywood Road & Ewbank Drive	EB-LT	Full	32	30	41	42
	SB-LR	Full	36	35	35	30
Haywood Road & Morris Lane	EB-LT	Full	6	23	49	55
	SB-LR	Full	42	43	33	34
Haywood Road & Ridgewood Boulevard/Whitemire Circle	WB-LT	Full	35	21	30	22
	NB-LR	Full	32	31	32	36
	SB-LTR	Full	36	42	35	33
Haywood Road & Orleans Avenue	EB-LTR	Full	22	18	32	43
	WB-LTR	Full	82	113	42	36
	NB-LTR	Full	45	34	34	47
	SB-LTR	Full	43	40	32	36
Haywood Road & N. Justice Street	WB-L	Full	73	73	71	71
	NB-LR	Full	434	617	770	666
Haywood Road & Site Access	WB-[L]	[50]	-	31	-	40
	NB-[LR]	[Full]	-	84	-	59

The queuing analysis indicates that the addition of site traffic in the Build scenario does not create a new queuing issue. In the Build scenarios, the maximum observed queues at the signalized intersections are not expected to increase significantly over the No-Build condition. No mitigation is recommended based on the queuing analysis.

#### **4. PEDESTRIAN LOS ANALYSIS**

At the request of NCDOT, an additional analysis was performed at the study intersections to include a pedestrian level of service. A description of the pedestrian-related characteristics of each intersection is provided below, along with the pedestrian level of service, in Table 6. Detailed reports are provided in Appendix F.



Table 6 – Pedestrian LOS Analysis

Unsignalized Intersections										
Intersection	Sidewalk	Pedestrian Crossing Features	Total Peak Hour Pedestrians		Approach	Haywood Road Crossing Length (feet)*	Pedestrian Approach LOS (Delay [sec])			
			AM	PM			No-Build		Build	
							AM	PM	AM	PM
Haywood Road & Ewbank Drive	South side of Haywood Road	None	0	0	EB	36	F (75.64)	F (128.92)	F (84.92)	F (148.94)
					WB	36	F (75.64)	F (128.92)	F (84.92)	F (148.94)
Haywood Road & Morris Lane	South side of Haywood Road; NE quadrant of intersection	None	2	0	EB	NB: 27; BLD: 36	E (33.90)	F (58.63)	F (81.54)	F (159.34)
					WB	NB: 24; BLD: 36	D (26.20)	E (43.82)	F (81.54)	F (159.34)
Haywood Road & Ridgewood Boulevard/ Whitmire Cicle	South side of Haywood Road	Marked crosswalk across Whitmire Circle	3	4	EB	26	E (37.25)	F (50.56)	E (42.42)	F (58.22)
					WB	25	E (34.07)	F (45.94)	E (38.69)	F (52.74)
Haywood Road & Orleans Avenue	South side of Haywood Road	Faded marked crosswalk across NB approach	4	6	EB	29	E (41.84)	F (63.16)	F (47.60)	F (91.92)
					WB	28	E (38.50)	F (57.64)	E (43.69)	F (83.19)
Haywood Road & N. Justice Street	North side of Haywood Road; both SW and SE quadrants of intersection	Marked crosswalk across N. Justice Street	0	2	EB	48	C (18.89)	D (22.82)	D (20.28)	D (24.55)
					WB	73	F (1,382.28)	F (2,784.70)	F (1,730.75)	F (3,410.10)
Haywood Road & Site Access	SW and SE quadrants intersection	None	-	-	EB	24	-	-	B (6.19)	B (8.23)
					WB	36	-	-	F (70.81)	F (137.94)

Signalized Intersections										
Intersection	Sidewalk	Pedestrian Crossing Features	Total Peak Hour Pedestrians		Approach	Haywood Road Crossing Length (feet)*	Pedestrian Crosswalk LOS (Score)			
			AM	PM			No-Build		Build	
							AM	PM	AM	PM
Asheville Highway & Haywood Road	All four quadrants of intersection	Marked crosswalks and pedestrians signals for all four approaches	2	0	EB	61.9	C (2.63)	C (2.64)	C (2.64)	C (2.66)
					WB	35.6	B (1.75)	B (1.75)	B (1.75)	B (1.75)
					NB	72.3	C (2.92)	C (3.00)	C (2.92)	C (3.01)
					SB	73.4	C (2.80)	C (2.85)	C (2.80)	C (2.85)
Haywood Road & Blythe Street	South side of Haywood Road; east side of Blythe Street	Marked crosswalk across Blythe Street; ped signals on NB approach	1	1	EB	42.4	B (2.40)	B (2.48)	B (2.41)	B (2.49)
					WB	36.2	B (2.31)	B (2.38)	B (2.32)	B (2.40)
					NB	36.0	B (2.10)	B (2.18)	B (2.12)	B (2.20)
					SB	54.1	B (1.97)	B (1.97)	B (1.97)	B (1.97)
US 64 & Blythe Street	Both sides of US 64 to the east of Blythe Street	Marked crosswalk and ped signals on WB approach	0	3	EB	36.0	B (2.50)	C (2.57)	B (2.50)	C (2.57)
					WB	36.0	B (2.48)	B (2.48)	B (2.49)	B (2.49)
					NB	35.9	B (2.09)	B (2.10)	B (2.09)	B (2.10)
					SB	36.2	B (2.20)	B (2.26)	B (2.22)	B (2.28)

\*From Synchro for purposes of LOS calculation. Not actual crossing distances.

The pedestrian analysis indicates that most approaches along Haywood Road are expected to be LOS E or F during the peak hours. The count data shows little pedestrian traffic in this corridor, but most of the pedestrian activity was focused at the Ridgewood Boulevard and the Orleans Avenue intersections. Based on conversations with NCDOT and the City of Hendersonville, project HS-2414F will install a rapid rectangular flashing beacon crossing at this intersection to allow pedestrians to access the Haywood Road sidewalk from Ridgewood Boulevard. That project is expected to be completed in 2025.

The signalized intersections receive better LOS grades. All approaches with sidewalk crossings currently have pedestrian signals, although few pedestrians were counted at these locations.

## 5. SUMMARY OF FINDINGS AND RECOMMENDATIONS

A traffic impact study was conducted for the proposed LEO Haywood Cottages development in accordance with NCDOT guidelines. The proposed development is located on the south side of Haywood Road (NC 191), east of Blythe Street, in Hendersonville, North Carolina. The development is expected to consist of 180 single family attached homes and would be completed by the end of 2028. Access to the site is to be provided via a full movement access on Haywood Road.

The study was determined through coordination with NCDOT and the City of Hendersonville and consists of the following intersections:

- Asheville Highway (US 25 Business) and Haywood Road (NC 191)
- Haywood Road (NC 191) and Blythe Street
- Brevard Road/6<sup>th</sup> Avenue West (US 64) and Blythe Street
- Haywood Road (NC 191) and Ewbank Drive
- Haywood Road (NC 191) and Morris Lane
- Haywood Road (NC 191) and Ridgewood Boulevard / Whitmire Circle
- Haywood Road (NC 191) and Orleans Avenue
- Haywood Road (NC 191) and N. Justice Street
- Haywood Road (NC 191) and Site Access

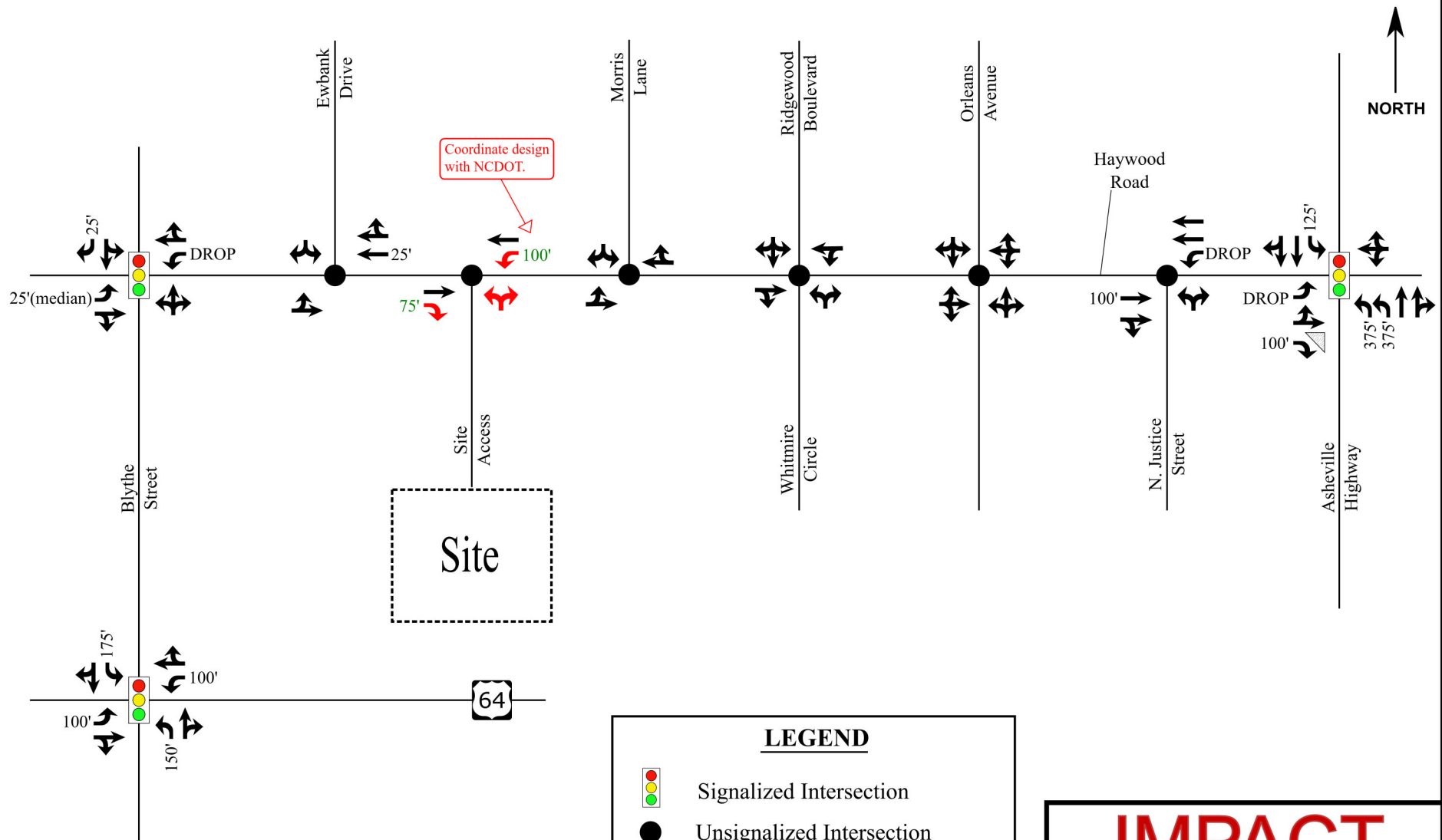
For the purpose of this analysis, the study intersections listed above were analyzed under the following scenarios:

- Existing (2024) Conditions
- No-Build (2028) Conditions
- Build (2028) Conditions






Traffic operations during the AM and PM peak hours were modeled for each scenario. The results of each scenario were compared to determine impacts from background traffic growth and the proposed development.

### Recommendations:

- Construct a westbound left turn lane on Haywood Road at the site access with at 100 feet of full width storage. Final design to be coordinated with NCDOT during permitting.
- Construct an eastbound right turn lane on Haywood Road at the site access with 75 feet of full width storage. Final design to be coordinated with NCDOT during permitting.



### LEGEND

-  Signalized Intersection
-  Unsignalized Intersection
-  Existing Lane
-  Recommended Improvement
-  NCDOT Required Improvement
- X' Storage (feet)
- X' Recommended Storage (feet)
- X' NCDOT Required Storage (feet)

**IMPACT**  
Designs, Inc.

*LEO Haywood Cottages  
Hendersonville, NC*

**Proposed Lane Configurations  
and Traffic Control**

Scale: Not to Scale

Figure

9