STRUCTURAL NOTES)

POTENTIAL, BASED ON A MAP PUBLISHED BY THE COLORADO GEOLOGICAL SURVEY WHICH SHOWS THE LOCATION FOR THE FOUNDATION HAS LOW SWELL POTENTIAL BASED ON A MAP PUBLISHED BY THE COLORADO GEOLOGICAL SURVEY WHICH SHOWS THE LOCATION OF VARRIOUS TYPES OF SOILS IN THE FRONT RANGE URBAN CORRIDOR: THE DESIGN ENGINEERS EXPERIENCE INDICATES THAT CLAYEY/SILTY SAND SOILS EXIST AT THIS STIE. IT IS EXPECTED THAT THIS SOIL HAS LOW SWELL POTENTIAL AND IS CAPABLE OF SUPPORTING FOOTINGS, PADS, AND WALLS-ON-CGRADE IMPOSING A MAXIMUM BEARING PRESSURE OF 2,000 PSF, AS SPECIFIED IN IRC TABLE NO. 7401.4.1, "WITH A BALANCED DEAD LOAD OF 600 PSF TO MINIMIZE DIFFERENTIAL MOVEMENT. AN "OPEN HOLE" OBSERVATION SHALL BE PERFORMED AT THE OWNERS EXPENSE UNDER THE SUPERVISION OF MARK BENJAMIN P.E., AS SOON AS THE EXCAVATION WORK IS COMPLETED TO VERIFY THIS ASSUMPTION. IF THIS OPTION, AS AN ALTERNATIVE, A COPY OF THE SUILS REPORT MAY BE FORWARDED TO MARK BENJAMIN P.E., TO VERHYTH AS SHAPPING THE SUILDING OFFICIAL DEEMS THAT THIS IS THE SOIL THAT EXISTS AT THIS LOCATION, OR IF A COMPETENT INDIVIDUAL (CONTRACTOR OR EXCAVATOR) CAN MAKE THIS DETERMINATION TO THE BUILDING OFFICIAL'S SATISFACTION, FORWARD THIS INFORMATION TO MARK BENJAMIN P.E., FOR CONSTRUCTION OF A PERIMETER SUBDRAIN SYSTEM WITH A VISIBLE SUMP PIT OR DAYLIGHT DRAIN IN THE BASEMENT AREA IS REQUIRED.

3) PER RAG3.1, PLACE FOOTINGS ON UNDISTURBED SOIL BELOW LOCALLY MANDATED FROST DEPTH; IN CASE SOIL IS DISTURBED, RECOMPACT IT TO 95% STANDARD PROCTOR DENSITY OR PER GEOTECHNICAL REPORT. EXCEPTIONS FOR BEARING ON SOLID ROCK PER RAG3.1.4.1. DUNDATION CONCRETE SHALL DEVELOP 3000 PSI COMPRESSIVE STRENGTH IN 28 DAYS; THE AGGREGATE SHALL BE 1 INCH MAXIMUM SIZE. THE CEMENT CONCRETE IN CONTACT WITH EARTH SHALL BE TYPE II OR TYPE II MODIFIED UNLESS OTHERWISE NOTED. MAXIMUM SLUMP SHALL BE 4" IN WALLS, BEAMS OCTINGS, 3" IN STRUCTURAL SLABS, 5" IN SLABS-ON-GRADE, AND 8" IN DRILLED CONCRETE PIERS (AKA CAISSONS); WATER IN EXCESS OF 1.0 GALLONS UBIC YARD CANNOT BE ADDED AT THE SITE WITHOUT APPROVAL OF THE DESIGN ENGINEER, CAISSONS MUST HAVE 6" MINIMUM SLUMP, WHICH MAY SUPPLEMENTING THE MIX WITH ADDITIONAL CEMENT OR PLASTICIZER IN ORDER TO ACHIEVE THE 3000 PSI 28-DAY STRENGTH REQUIREMENT. SEE

CONCRETE FOR EXTERIOR FLATWORK (SEE BELOW FOR INTERIOR FLATWORK) PER TABLE R402.2 SHALL DEVELOP 3500 PSI COMPRESSIVE STRENGTH IN 28 DJ 4.5%; SUBSTITUTE MIXES CONTAINING FLY ASH AND "POZZOLITH" ARE ACCEPTABLE AS LONG AS THEY MEET THE 3500 PSI STRENGTH. "JITTERBUGGING" DUR PLACEMENT IS NOT RECOMMENDED. SAWCUT OR TOOLED CONTROL JOINTS SHALL BE PLACED AT A MAXIMUM SPACING OF 8' TO 10'. APPLICATION SPRAY MEMBRANE CURING COMPOUND AT A RATE OF NO MORE THAN 200 S.F. PER GALLON AFTER FINISHING IS STRONGLY RECOMMENDED. WHERE SLE BUTT UP AGAINST CONCRETE OR MASONRY WALLS, PROVIDE TWO (2) 1/4" WIDE STRIPS OF TEMPERED MASONITE WITH SILICONE LUBRICART BETWEEN SMOOTH FACES AS A PERMETER SLIP JOINT UNLESS EXPANSION JOINTS ARE SHOWN ON THE DRAWINGS. PROVIDE SAWCUT OR SCORED CONTROL JOINT ALL INTERIOR "FLOATING" SLABS-ON-GRADE AT A 14 MAXIMUM SPACING IN ANY DIRECTION AND 3' PARALLEL TO THE FOUNDATION WALLS, WITH THER DE EQUAL TO ONE-FOURTH OF THE SLAB THICKNESS, UNLESS THERE IS A GEOTECHNICAL REPORT WITH A MORE STRINGENT REQUIREMENT. THE OWNER ASSULT ALL THE RISKS AND LIABILITY FOR POSSIBLE FUTURE DAMAGE TO CONCRETE SLABS-ON-GRADE EVEN IF CONSTRUCTION OF THE SLABS ADHERES TO ENGINEERING RECOMMENDATIONS AND DESIGNS. PER R506.2.2 CONCRETE SLAB BASE SHALL BE 4" THICK BASE OF CLEAN GRADED SAND. GRAVEL, CRUSHED STONE, CRUSHED CONCRETE OR CRUSHED BLAST-FURNACE SLAG PASSING A 2" SIEVE SHALL BE PLACED ON THE PREPARED SUBGRADE WHERE THE SLAB IS BELOW GRADE, PER R506.2.3 VAPOR RETARDER SHALL BE A 6-MIL POLYETHYLENE OR APPROVED VAPOR RETARDER WITH JOINTS LAPPED NOT LESS THAN 6" SHALL BE PLACED BETWEEN THE CONCRETE FLOOR SLAB AND THE BASE COURSE, RADON PROTECTION SHALL BE PER APPENDIX AF103.1.

ON FOUNDATION PLAN REFERS TO FOOTING FORMS BY BIGFOOT SYSTEMS (<u>www.bigfootsystems.com</u>) OR Equivalent, nn refers to Size (8528 is 28" (eter), install appropriate size construction tube (i.e. sonotube) on top to height required, place footing form with construction in hole and backfill prior to filling with concrete.

ALL REINFORCING STEEL BARS SHALL CONFORM TO ASTM A615 GRADE 60, EXCEPT FOR COLUMN TIES, BEAM STIRRUPS AND EMBEDDED PLATE ANCHORS, WHICH SHALL BE GRADE 40. CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE IN ACCORDANCE WITH IRC SECTION R403.1.5.3. MAKE ALL BARS CONTINUOUS AROUND CORNERS OR PROVIDE CORNER BARS OF EQUAL SIZE AND SPACING, PLACE 2 - #5 GRADE 40 OR 2 - #4 GRADE 60 BARS (I EACH FACE) WITH 2'-0" PROJECTION AROUND ALL OPENINGS IN THE CONCRETE. IN GRADE BEAMS, BOTTOM SPLICES SHALL BE AT THE DRILLED PIERS AND TOP SPLICES SHALL BE AT THE DRILLED PIERS AND TOP

9) IF USED, NUDURA ICF WALLS SHALL HAVE THE FOLLOWING MINIMUM RENFORCEMENT AT ALL WALL OPENINGS: 2 - #4 HORIZONTAL REBAR APPROXIMATELY 24" ABOVE THE TOP OF THE OPENING AND EXTENDING 24" PAST THE OPENING ON EITHER SIDE: 2 - #4 HORIZONTAL REBAR APPROXIMATELY 2" ABOVE THETOP OF THE OPENING AND EXTENDING 24" PAST THE OPENING ON EITHER SIDE: 2 - #4 VERTICAL BARS ON EACH SIDE OF THE OPENING (FULL HEIGHT); 2 - #4 HORIZONTAL BARS APPROXIMATELY 2" BELOW THE BOTTOM OF THE OPENING AND EXTENDING 24" PAST THE OPENING ON EITHER SIDE. DECREASE THE STANDARD VERTICAL REBAR SPACING BETWEEN WALL OPENINGS BY 50% WHEN WALL OPENINGS ARE WITHIN 6" OF EACH OTHER, INSTALL 2 - #4 VERTICAL BARS AT ALL CORNERS (FULL HEIGHT), INSTALL 2 - #5 VERTICAL BARS DIRECTLY BENEATH ALL BEAM AND GIRDER POINT LOADS (FULL HEIGHT), INSTALL 2 - #4 HORIZONTAL BARS AT THE TOP AND BOTTOM OF EACH WALL POUR, MINIMUM REBAR LAP LENGTH SHALL BE 40 BAR DIAMETERS, CONSTRUCTION SHALL CONFORM TO ALL GUIDELINES PROVIDED BY THE ICF MANUFACTURER. SLOPE FINISHED SOIL GRADE AND ANY PAVING AWAY FROM THE BUILDING ON ALL SIDES TO PROVIDE POSITIVE SURFACE DRAINAGE AT THE RATE OF 3/4"
PER LINEAR FOOT, UNLESS THERE IS, A GEOTECHNICAL REPORT WITH MORE STRINGENT RECOMMENDATIONS, PROVIDE DRAINAGE CONDUITS AND
COLLECTION AS NEEDED FOR ROOF RUNOFF, LANDSCAPING ADJACENT TO THE FOUNDATION SHOULD CONSIST OF A 5-FOOT-WIDE STRIP OF INERT GROUND
COVER SUCH AS ROCK OR BARK WITH AN UNDERLYING LAYER OF IMPERVIOUS SOIL OR GEOTEXTILE MEMBRANE. IRRIGATION SHALL BE DESIGNED SO AS NOT
TO DISCHARGE WATER ON THIS STRIP, SEE R403.1.5.

STRUCTURAL STEEL SHALL CONFORM TO ASTM A36/A572 GRADE 50 OR A992 MATERIAL CONFORM TO ASTM A36/A572 GRADE 50 OR A992 MATERIAL CONFORM TO ASTM A53 GRADE 8 OR A36. COLUMNS SHALL BE FIXED LENGTH, UNO. ANY ADJU EXPOSED. BOLTS SHALL CONFORM TO ASTM DESIGNATION A307 UNLESS OTHERWISE NOTED. ALL F WITH AMERICAN INSTITUTE FOR STEEL CONSTRUCTION SPECIFICATIONS AND STANDARDS. IF COLINSTEAD OF SIZE, THE CONTRACTOR SHALL PROVIDE LOAD RATING INFORMATION ON THE PRODUCING ASTERMANDS.

) ALL PRESSURE-TREATED FOUNDATION DIMENSION LUMBER (INCLUDING PORTIONS OF POLE BARN POLES IN THE GROUND) SHALL BE NO. 2 GRADE HEM-FIR OR BETTER. ALL PRESSURE-TREATED MATERIALS SHALL BEAR "FDN" GRADE MARKS OF A RECOGNIZED GRADING AUTHORITY, AND SHALL BE INSTALLED IN ACCORDANCE WITH BOTH IRC. SECTION R404.2.1 THROUGH R404.2.6 AND THE LATEST EDITION OF THE MANUAL ENTITLED "ALL-WEATHER WOOD FOUNDATION SYSTEM DESIGN, FABRICATION, AND INSTALLATION", PUBLISHED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION, WASHINGTON, D.C. ALL LUMBER MATERIAL SHALL BE NO. 2 GRADE HEM-FIR OR BETTER, WITH A BASE MINIMUM ALLOWABLE EXTREME FIBER BENDING STRESS FOR MEMBERS (FB) 850 PSI EXCLUDING ADJUSTMENT FACTORS FOR USE. SIZE, LOAD DURATION, ENVIRONMENT, ETC... UNLESS OTHERWISE NOTED, WHERE THE TERM "DOUG-FIR" "PARS ON THE DRAWINGS, THIS DENOTES NO. 2 GRADE DOUGLAS FIR OR BETTER, HAVING AN FB EQUAL TO 875 PSI, TOP PLATES MUST BE NO. 2 GRADE UGLAS FIR OR BETTER, OR NO. 2 GRADE HEM-FIR IF THE FRAMING MEMBERS BEARING ON WALLS ARE STACKED ABOVE THE STUDS, STUDS SHALL BE NOARD GRADE OR BETTER. WITH A 50,000 PSI YIELD STRESS. PIPE COLUMNS SHALL HAVE MAXIMUM 3" OF THREAD ABRICATION AND ERECTION SHALL BE IN ACCORDANCE MANS ARE SPECIFIED IN TERMS OF REQUIRED CAPACITY THAT IS PLANNED TO BEUSED PRIOR TO INSTALLATION.

B. FLOOR JOISTS SHOULD NOT RUN "WILD" MORE THAN 3" PAST THE CENTER LINE OF ANY BEAM. DOUBLE-UP ALL FLOOR JOISTS WHICH ARE PARALLEL TO PARTITIONS ABOVE UNLESS OTHERWISE NOTED, FLOOR SHEATHING SHALL BE 3/4" T&G SECURED WITH 8D NAILS AT A 6" SPACING AT PANEL AND DIAPHRAGM EDGES AND AT A 12" SPACING AT PANEL FIELD OR 2" 16 GAGE STAPLES AT A 4" SPACING AT PANEL AND DIAPHRAGM EDGES AND AT AN 8" SPACING AT PANEL FIELD, EDGES UNBLOCKED A. STANDARD FRAMED WALL HEADERS, UNO, SHALL BE 2 - 2 X 8'S BEARING ON SINGLE TRIMMER STUDS FOR OPENINGS LESS THAN BEARING ON DOUBLE TRIMMER STUDS (UNLESS NOTED OTHERWISE) FOR OPENINGS 4 FEET TO 6 FEET WIDE: ALL OPENINGS OVER ENGINEERED HEADERS, IF 3 OR MORE STUDS ARE INDICATED AS A COLUMN, THE STUDS SHALL BE BUNDLED AND NAILED TOGETHER VERTICAL FOOT, AND THE TOP AND BOTTOM PLATES SHALL BE INTERRUPTED SO THAT THE BEAMS CAN BEAR DIRECTLY ON STEPLATES OR BUNDLED WOOD COLUMNS, AND SO THAT THE COLUMNS CAN BEAR DIRECTLY ON A BEAM OR THE FOUNDATION WILLIAMS.

15) ALL PRE-MANUFACTURED CONNECTORS SPECIFIED SHALL BE PROVIDED BY SIMPS SHOP DRAWINGS AND SPECIFICATIONS FOR REVIEW AND APPROVAL TO MARK BENJ NN STRONG-TIE COMPANY, FOR MIN P.E., 7 DAYS PRIOR TO ORD

WHERE BEAMS ARE INDICATED ON THE PLANS WITH THE LETTERS "LVL", 1-3/4" WIDE LAMINATED STRESS OF 2600 PSI, A MODULUS OF ELASTICITY OR "E-VALUE" OF 1,800,000 PSI, AND A MINIMUM HAPPROVED PRODUCTS ARE "MICRO-LAMS" OR "PARALLAMS" MANUFACTURED BY TRUS JOIST-NFOREST PRODUCTS, "GANG-LAMS" MANUFACTURED BY LOUISIANA-PACIFIC, "TEC-LAMS" MANUFACTURED BY BOISE-CASCADE. THE PLANS WILL INDICATE THE QUANTITY NEEDED TO BUILD MANUFACTURED BY BOISE-CASCADE. THE PLANS WILL INDICATE THE QUANTITY NEEDED TO BUILD NEEDED, PER RS02.8.2, MODIFICATIONS TO ENGINEERED WOOD PRODUCTS SHALL BE PER MANUF HERE BEAMS ARE INDICATED ON THE PLANS WITH THE LETTERS "LSL", OR WHERE ENGINEERED I-JOISTS ARE INSTALLED, 1-1/4" OR 3-1/2" WIDE LAMINATED UNDIG A MINIMUM FLEXURAL STRESS OF 1700 PSI, A MODULUS OF ELASTICITY OR "E-VALUE" OF 1,300,000 PSI (800,000 PSI FOR RIM USE ONLY), AND A MINIMUM HORIZONTAL SHEAR STRESS OF 285 PSI SHALL BE INSTALLED. THE ONLY APPROVED PRODUCTS ARE "TIMBERSTRAND" JFACTURED BY TRUS JOIST MACMILLAN, "RIGIDRIM" MANUFACTURED BY ROSEBURG FOREST PRODUCTS, AND "INNERSEAL" MANUFACTURED BY INAM-PACIFIC. THE PLANS WILL INDICATE THE QUANTITY NEEDED TO BUILD UP A SINGLE BEAM AND THE MINIMUM STANDARD DEPTH NEEDED, PER JAMA-PACIFIC. THE PLANS WILL INDICATE THE QUANTITY NEEDED TO BUILD UP A SINGLE BEAM AND THE MINIMUM STANDARD DEPTH NEEDED, PER JAMA-PACIFIC. THE PLANS WILL INDICATE THE QUANTITY NEEDED TO BUILD UP A SINGLE BEAM AND THE MINIMUM STANDARD DEPTH NEEDED. PER MANUFACUTRER RECOMMENDATION OR APPROVED BY ENGINEER.

(19) PRESERVATIVE TREATED WOOD USED IN DECKS, HIGH MOISTURE CONTENT GREENHOUSES OR SIMILAR MOISTURE LADEN PROJECTS SHALL HAVE CONNECTORS AND FASTENERS THAT ARE COMPATIBLE WITH THE CHEMICALS USED PER R507.2.3. CONTACT THE PRESSURE TREATED WOOD SUPPLIER TO OBTAIN THE SPECIFICATIONS AND CHOOSE CONNECTORS AND FASTENERS ACCORDINGLY. BOTH CONNECTORS AND FASTENERS SHALL BE MATCHED (I.E. IF STAINLESS STEEL CONNECTORS ARE REQUIRED, STAINLESS STEEL FASTENERS OF THE SAME GRADE MUST BE USED.)

20) PROVIDE DECK LATERAL LOAD CONNECTIONS IN ACCORDANCE WITH IRC SECTION R507.9.2 INSTAIL (2) HORIZONTAL 1500 LB CAPACITY DTT72 HOLD DOWNS PER FIGURE R507.9.2(1) OR (4) 750 LB CAPACITY DTT72 SECURED WITH MIN. 3/8" LAG WITH 3" PENETRATION TO SOLID LUMBER PER FIGURE R507.9.2(2)(DOES NOT REQUIRE CONNECTION THROUGH RIM JOIST INTO FLOOR JOISTS). EXAMPLE FIGURES CAN BE FOUND HERE: HTTPS://CODES.ICCSAFE.ORG/CONTENT/IRC2021/CHAPTER-S-FLOORS#IRC2021_PT03_CH05_SECR807.2 ALL DECK LUMBER MATERIAL SHALL BE NO. 1 GRADE DOUG-FIR, OR SOUTHERN YELLOW PINE, OR BETTER, WITH A BASE MINIMUM ALLOWABLE EXTREME FIBER BENDING STRESS FOR MEMBERS (FB) OF 1000 PSI EXCLUDING ADJUSTMENT FACTORS FOR USE, SIZE, LOAD DURATION, ENVIRONMENT, ETC... UNLESS OTHERWISE NOTED, PRESERVATIVE TREATED LUMBER SHALL BE USED PER R317.1.8.

21) PART-TIME MONITORING OF THE STEEL QUONSET BUILDING INSTALLATION COULD BE REQUIRED BY THE AHJ. IN WHICH INITIAL OBSERVATIONS WILL BE MADE BY A REPRESENTATIVE OF CROWN JADE DESIGN AND ENGINEERING AT THE GENERAL CONTRACTOR'S EXPENSE FOR THE PURPOSE OF DETERMINING THAT PROPER MATERIALS AND METHODS WERE UTILIZED. REVIEW THIS THOROUGHLY PRIOR TO INSTALLATION. FOR ANY QUESTIONS, CONTACT CJDE, SEE 2021 BC SECTION 1705.2. BUILDING DEPARTMENT REQUIRED ENGINEER INSPECTIONS/OBSERVATIONS AS NOTED IN THE CONTRACT MEANS WHEN THE AUTHORITY HAVING JURISDICTION (AHJ OR BUILDING DEPARTMENT] REQUIRES THE ENGINEER TO PERFORM CERTAIN OBSERVATIONS TO VERIFY CONFORMANCE WITH THE DRAWINGS (I.E. OEO, FOOTING, FOUNDATION, SPECIAL INSPECTIONS, ETC.), ALLOW AMPLE TIME FOR CJDE TO SCHEDULE SAID OBSERVATIONS. SEVERAL DAYS' NOTICE IS NOT SUFFICIENT AND DOES NOT REQUIRE CJDE TO MEET THAT SCHEDULE.

23) THIS FOUNDATION DESIGN IS BASED ON DRAWINGS/LOADS PREPARED BY DUROSPAN, DATED 9/6/2024, THEIR MODEL NO. Q35-17. IT SHALL BE THE RESPONSIBILITY OF THE OWNER AND HIS FOUNDATION CONTRACTOR TO MAKE SURE THAT THIS FOUNDATION PLAN FITS THE ANCHOR BOLT PLAN PROVIDED BY DUROSPAN. ns shown on structural d intended to locate struct ay devation in excess of t ges, in no way can the des HED BY ARCHITECTS, CONTRACTORS, OR THE DESIGN ENGINEER MUST BE NOTIFIED BY PONENTS, WHICH MIGHT BE AFFECTED BY N OR PLACEMENT OF ANY COMPONENT.

AND THE STRUCTURAL P.E. DESIGN AND CONSTRUCTION PLANS ARE COMPLEX. THE STRUCTURAL P.E. AND HS CONSULTANTS HAVE PERFORMED THE SERVICES WITH DUE CARE AND DILIGENCE, ALTHOUGH EVERY DETAIL CANNOT BE ANTICIPATED AND IS DEPENDENT ON MANY FACTORS WHICH MAY REQUIRE CHANGES TO THE PLANS... ANY AMBIGUITY OR DISCREPANCY DISCOVERED BY THE USE OF THE PLANS SHALL BE REPORTED IMMEDIATELY TO THE STRUCTURAL P.E. OF ANY ISSUE IN WRITING SHALL RELIEVE THE STRUCTURAL P.E. FROM RESPONSIBILITY FOR ADDITIONAL COSTS, DAMAGES, OR LIABILITY THEREFROM. CHANGES MADE FROM THE PLANS WITHOUT THE CONSENT OF THE STRUCTURAL P.E. ARE UNAUTHORIZED, AND SHALL BELIEVE THE STRUCTURAL P.E. ARE UNAUTHORIZED, AND SHALL RELIEVE THE STRUCTURAL P.E. OF RESPONSIBILITY FOR ALL CONSEQUENCES ARISING OUT OF SUCH CHANGES.

BASED ON A 130 MPH V-ULTIMATE WIND SPEED AT I.B.C. EXPOSURE "C", AS SUBJECT TO THE CURRENT ASCE 7 REQUIREMENTS VITON (#) FACTOR KE OF 0.78; REFER TO CODE TABLES FOR OTHER ZONE LOAD VALUES. WITH APPROPRIATE FACTORS APP JFACTURER; REFER TO SPECIFICATIONS, NOTES, AND LOADS ON BUILDING MANUFACTURER'S DRAWINGS.

REGARDING GROUND

FOUNDATIO

FOUNDATION WIDTH
INDUSTRIAL BASE CONNECTOR
TABLE 1
DURO SPAN HAND WELDED BASEPLATE
MANUAL FOR ERECTION OF STEEL
PAGE 5

37-15 38-14	35-17	35-14	34-13	33-15	32-13	30-14	28-12	27-13	25-12	25-10	23-11	21-10	19-09	MODEL
37'-3 1/2" 38'-3 1/2"	35'-3"	35'-3 1/2"	34'-3 1/2"	33'-3"	32'-3 1/2"	30'-3"	28'-3 1/2"	27'-3"	25'-3"	25'-3 1/2"	23'-3"	21'-3"	19'-3"	WIDTH

FOUNDATION LENGTH
INDUSTRIAL BASE CONNECTOR
TABLE 2
DURO SPAN HAND WELDED BASEPLATE
MANUAL FOR ERECTION OF STEEL

50	40	48	47	46	4.	4	43	42	4	40	39	38	37	36	35	34	33	32	51	30	20	70	28	27	26	25	24	23	22	21	20	19	18	17	16	15	14	13	12	J===	10	9	ARCHES	OF	NUMBER
	100	99'.0"	96'-11 5"	94'-11"	92'-10.5"	90'-10"	881-9.5"	86'-9"	841-8.5"	82'-8"	80'-7.5"	781-711	76'-6.5"	74'-6"	72'-5.5"	70'-5"	68'-4.5"	66'-4"	04-3.3	02-3	601 211	60'-2 5"	581-211	56'-1.5"	54'-1"	52'-0.5"	50'-0"	47'-11.5"	45'-11"	43'-10.5"	41'-10"	39'-9.5"	37"-9"	35'-8.5"	33"-8"	31'-7.5"	29'-7"	27'-6.5"	25'-6"	23'-5.5"	21'-5"	19"-4.5"		PACKAGE 0	

11-7"	AT DECK BEAM FW3 AND LVL RIM ICFVL. AND BETWEEN, INSTALL (2) #4 AT 2" MAX. ABOVE AND BELOW BEAM POCKET/ICFVL. SEE 52 FOR DETAIL.	*			34'	60'	10'-11 1/2"	4-7 1/2"	10'-5"	
	TE FOOTINGS, PER I	11'-6 1/2" TALL ICF FOUNDATION WALL W/ 6" CONCRETE THICKNESS AND 2-5/8" FOAM EACH SIDE W/ 16" WIDE X 8" THICK		13-8 1/2" TALL ICF FOUNDATION WALL W/6" CONCRETE THICKNESS AND 2-5/8" FOAM EACH SIDE, TAPERED AT TOP OF OUTSIDE EDGE W/ 24" WIDE X 8" THICK CONCRETE FOOTINGS, PER DETAIL ICF1	INTERIOR EDGE OF 10" PLATE TO BE FLUSH WITH INTERIOR EDGE OF ICF WALL. SEE DETAIL THIS PAGE.	24" WIDE FOOTING 24" WIDE FOOTING	HEADERS FOR 2'-5" WIDE WINDOWS IN ICF PER SCHEDULE, typ.	7-11"		
	CONCRETE FOOTINGS, PER DETAIL ICF1. VERIFY HEIGHT TO SUPPORT DECK BEAMS. DEC WITH 16-00	11'-6 1/2" TALL ICF FOUNDATION WALL W/ 6" CONCRETE THICKNESS AND 2-5/8" FOAM EACH SIDE w/ 16" WIDE X 8" THICK	PROST DEPTH ICF FOUNDATION WALL W/ 6" CONCRETE THICKNESS AND 2-5/8" FOAM EACH SIDE W/ 24" WIDE X 8" THICK CONCRETE FOOTINGS, PER DETAIL ICF2 24" WIDE FOOTING	13:-8 1/2"TALL ICF FOUNDATION WALL W/6" CONCRETE THICKNESS AND 2-5/8" FOAM EACH SIDE, TAPERED AT TOP OF OUTSIDE EDGE W/ 24" WIDE X 8" THICK CONCRETE FOOTINGS, PER DETAIL ICF1		PADS PER PDI, typ. 24" WIDE FOOTING	DE THICKENED R 8 x 16" STRIP OTING UNDER SLAB PER SL2 25"-10" 38"-1"	40" SQ. PAD W/ (5) #4 EA. WAY	13-8 1/2" TALL ICF FOUNDATION WALL W/ 6" CONCRETE THICKNESS AND 2-5/8" FOAM BACH SIDE W/ 16" WIDE X 8" THICK CONCRETE FOOTINGS, PER DETAIL ICF1 25/9	NOTE: DIMENSIONS ARE TO DUTSIDE EDGE OF ICF WALL FOAM
AT 2" MAX. ABOVE AND BELOW BEAM POCKET/ICFVL. SEE S2 FOR DETAIL.	TOP ICF WALL DETAIL SCALE: \frac{1}{2}" = 1"	* PLAN VIEW OUONSET PLATE AT TAPERED	3 1/2" - \(\frac{1}{1/2} \) \(\frac{1}{1/2} \	3/8" x 4" TITEN HD, typ. NEW HOLES MAY BE DRILLED INTO THE 14 GA. BASE PLATE AS REQUIRED TO MISS FOAM AND/OR ICF TIES WITH THE TITEN HD ANCHORS. MAINTAIN MAX. 12-1/4" SPACING SHOWN (MAY BE SHORTER DISTANCES).	EXTERIOR EDGE OF 10" PLATE TO BE 1-1/4" AWAY FROM EXTERIOR EDGE OF ICF TAPERED TOP INTERIOR EDGE OF 10" PLATE TO BE 10" PLATE TO BE HD, typ.			6" CONC (11" ICF) (1) #4 - 2" UP (1) #4 NONE REQUIRED (1) #4 - 8" UP (1) #4 (1) #4 WITHIN 12" UP (1) #4 - 10"-12" UP (1) #4 - 10"-12" UP (1) #4 - 8" UP (1) #4 - 10"-12" UP (1) #4 - 8" UP (1) #4 - 10"-12" UP (1) #4 - 8" UP (1) #4 - 10"-12" UP	ICF WINDOW HEADER SCHEDULE ALL THORNESS BOTTOM REBAR WITHIN 8" OF TOP OF OPENING OONC (13" ICF) (2) #4 (1) #4 ICF WINDOW HEADER SCHEDULE TOP REBAR OONINLOUS WITH OF OPENING (1) #4	RELEASE OF THE PLANS COMPLETED BY THE STRUCTURAL P.E. CONTEMPLATES FURTHER COOPERATION AMONG THE OWNER, HIS CONTRACTOR, AND THE STRUCTURAL P.E. AND HIS CONSTRUCTION ARE COMPLEX. ALTHOUGH THE STRUCTURAL P.E. AND HIS CONSULTANTS HAVE PERFORMED THEIR SERVICES WITH DUE CARE DILIGENCE, THEY CANNOT GUARANTEE PERFECTION. COMMUNICATION IS IMPERFECT, AND EVERY CONTINGENCY CANNOT BE ANTICIPATED. ANY AMBIGUITY OR DISCREPANCY DISCOVERED BY THE USE OF THESE PLANS SHALL BE REPORTED IMMEDIATELY TO THE STRUCTURAL P.E. A FAILURE TO COOPERATE BY A SIMPLE NOTICE TO THE STRUCTURAL P.E. SHALL RELIEVE THE STRUCTURAL P.E. FROM RESPONSIBILITY FOR ANY ADDITIONAL COSTS, DAMAGES OR LIABILITY THE CONSENT OF THE STRUCTURAL P.E. ARE UNAUTHORIZED, AND SHALL RELIEVE THE STRUCTURAL P.E. STRUCTURAL P.E. ARE UNAUTHORIZED, AND SHALL RELIEVE THE STRUCTURAL P.E. OF RESPONSIBILITY FOR ALL CONSEQUENCES ARISING OUT OF SUCH CHANGES.

Mark by Mark Benjamin Date: 2024/30.15
Benjamin Date: 2024/30.15 10/15/2024

ENGINEERS JOB#

HGND1-24179 SHEET

SC

ALE:

3/16"

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EVAN CHEADLE
ENGINEERING REVIEW:
MARK BENJAMIN / DOUG INGERSOL
/ SETH HOOVER
ENGINEER OF RECORD:
MARK BENJAMIN, P.E.

MARK BENJAMIN, P.E.

PHI-D&E Inc. dba Crown Jade Designand Engineering

SCALE: 1/4" = 1"U.N.O. SHEET SIZE: D (24X36)

FOUNDATION

PLAN &

FINAL NOTES

10/15/2024