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4206 INTRACOASTAL DRIVE
PREPARED FOR:
DAVE & RANDI STANLEY

EXISTING CONDITIONS		
Date: 8/24/2020	Sheet : 1	of : 8
Proj No.: 19-0050		

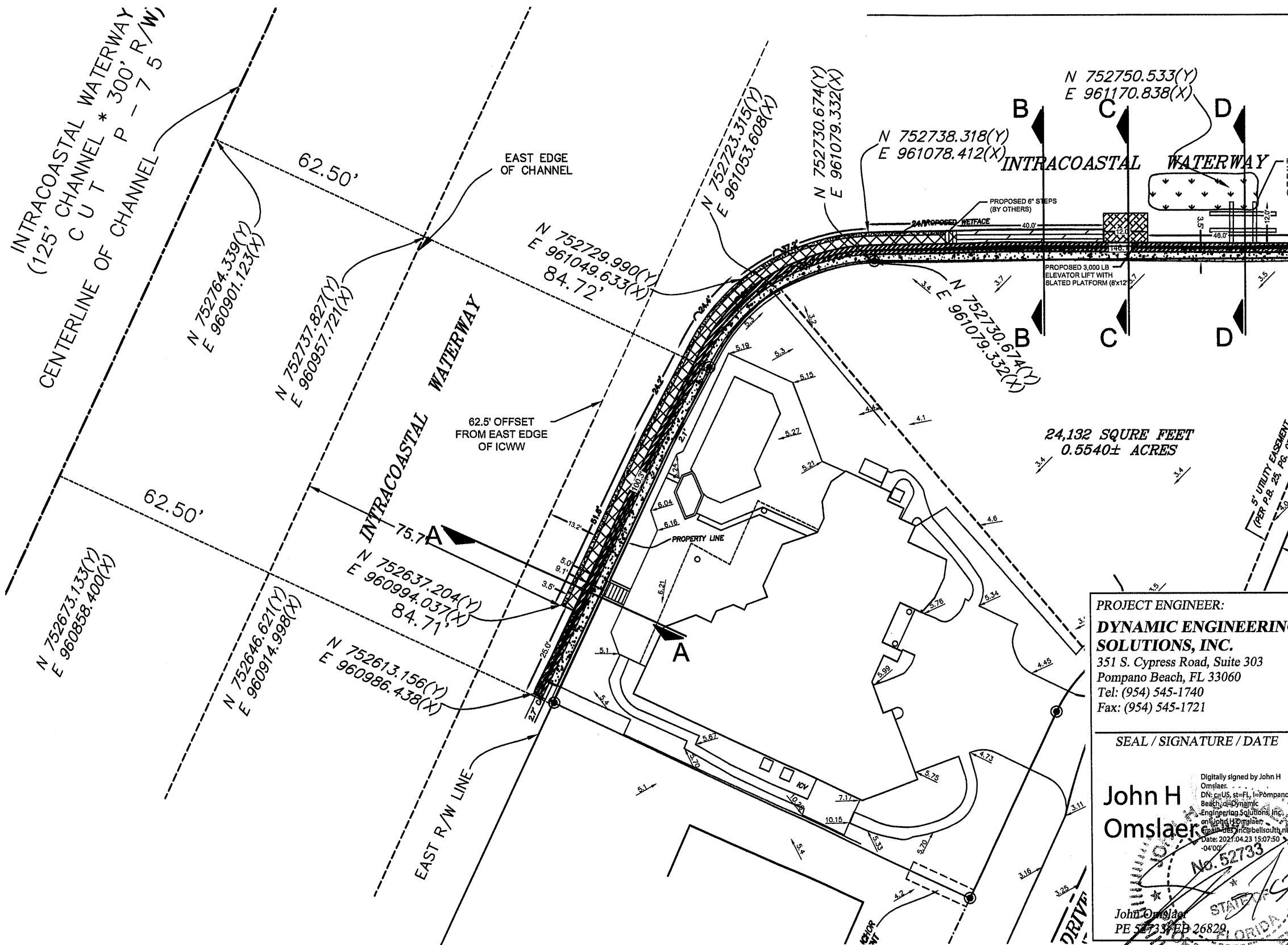
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HIGHLAND BEACH
BUILDING DEPARTMENT



INTRACOASTAL WATERWAY
(125' CHANNEL * 300' R/W)
C U T P - 7 5



N 752736.593(Y)
E 961224.718(X)

LEGEND

- SUBJECT SITE (0.55 AC)
- EXISTING CONCRETE SEAWALL TO REMAIN (±300.4 LN. FT.)
- EXISTING PADDLE GRASS (*Halophila decipiens*; ±1% COVERAGE; ±288 SQ. FT.)
- PREVIOUSLY AUTHORIZED CONCRETE SEAWALL W/ 3.0' CAP (±304.8 LN. FT.; ±457.5 SQ. FT. OVERWATER)*
- PROPOSED ±1.0' CONCRETE NOTCH ABOVE CAP/DOCK (±309.5 LN. FT.)
- PROPOSED CONCRETE DOCK (±564 SQ. FT.)
- PROPOSED WOOD DOCK (±140 SQ. FT.)
- PROPOSED JET SKI LIFT WITH 1/2 INCHED SPACED DECK BOARDS (±95 SQ. FT.)
- PROPOSED 33,000 LB BOAT LIFT

M.H.W. = 0.35' NAVD M.L.W. = (-)2.04' NAVD

NOTES:

1. SURVEY INFORMATION PROVIDED BY BASELINE LAND SURVEY LLC.
2. PROPOSED SEAWALL WAS PREVIOUSLY AUTHORIZED UNDER USACOE PERMIT No. SAJ-1995-02049(NWGP-JKA) AND FDEP ERP No. 50-0221786-003-004-EE.
3. TOTAL OVERWATER SQUARE FOOTAGE IS 1,387 SQ. FT.

PROJECT ENGINEER:
DYNAMIC ENGINEERING SOLUTIONS, INC.
351 S. Cypress Road, Suite 303
Pompano Beach, FL 33060
Tel: (954) 545-1740
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PE 52733
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-04'00'

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N 752750.533(Y)
E 961170.838(X)

B

C

D

752738.318(Y)
961078.412(X)

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N 752736.
E 961224.

INTRACOASTAL WATERWAY

PROPOSED
33,000 LB
ELEVATOR
BOATLIFT
(10'x25')

90' CANAL WIDTH

PROPOSED 6" STEPS
(BY OTHERS)

PROPOSED WETFACE

40.0'

12.0'

3.5'

46.0'

12.0'

2.5'

25.0'

PROPOSED 3,000 LB
ELEVATOR LIFT WITH
SLATED PLATFORM (8'x12')

3.7'

3.5'

3.3'

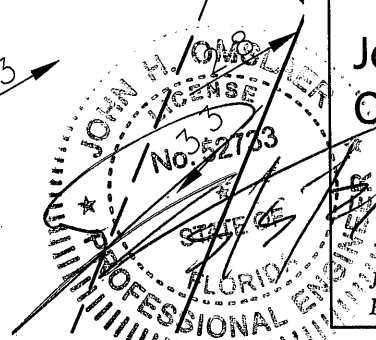
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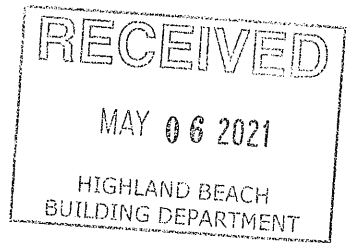
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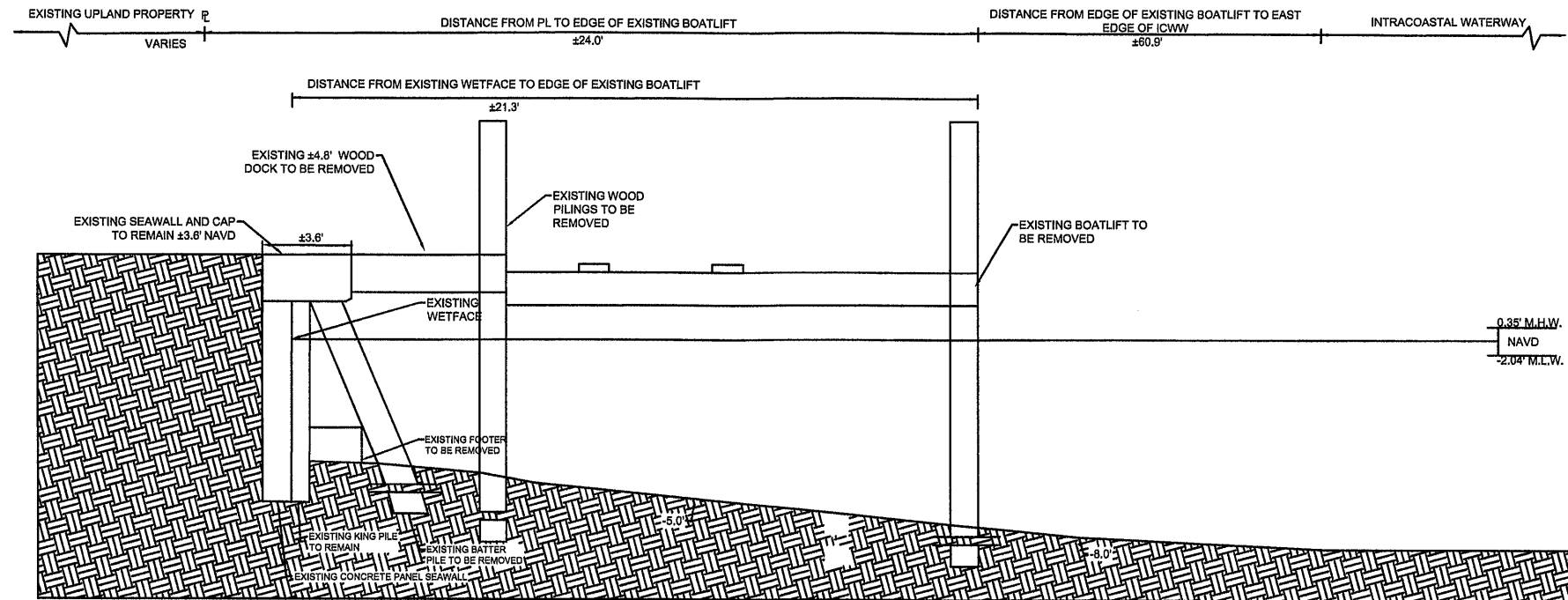
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DAVE & RANDI STANLEY

PROPOSED CONDITIONS

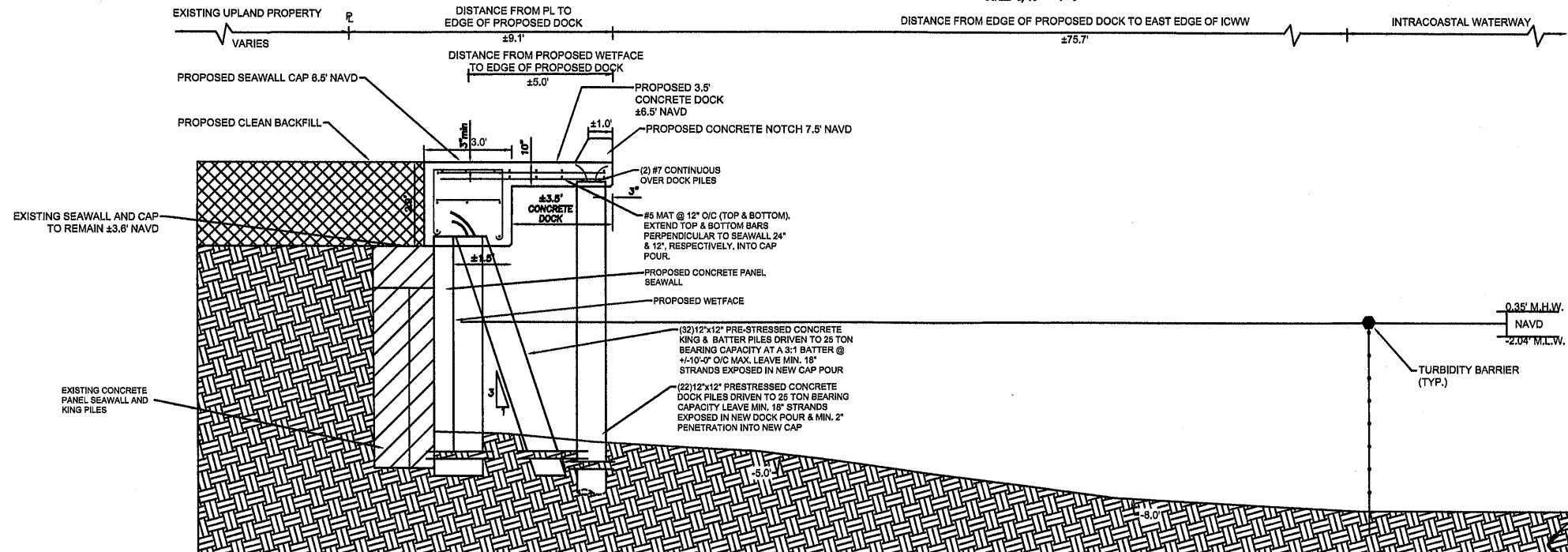
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Proj No.: 19-0050		



EXISTING CONDITIONS A-A (TYP.)
SCALE: 3/16" = 1'-0"



PROPOSED CONDITIONS A-A (TYP.)
SCALE: 3/16" = 1'-0"



NOTE PROPOSED WETFACE TO BE NO GREATER THAN 18" WATERWARD OF EXISTING WETFACE

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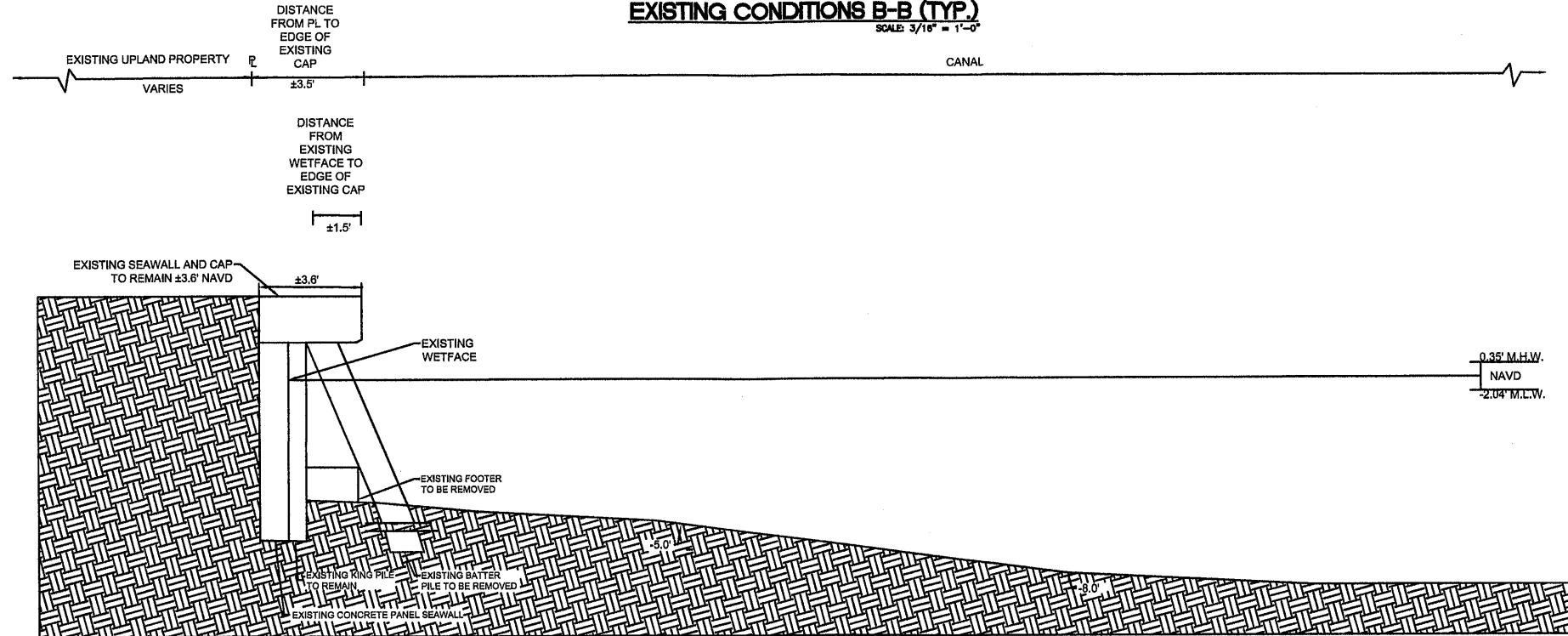
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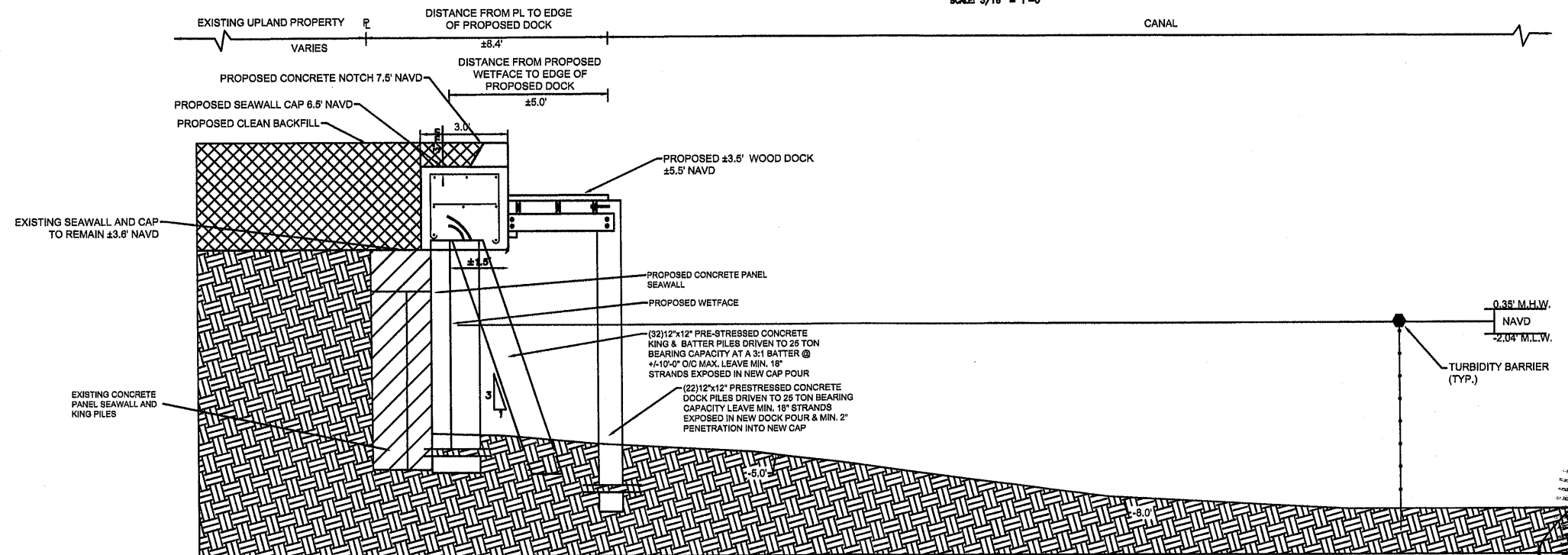
EXISTING CONDITIONS B-B (TYP.)

SCALE: 3/16" = 1'-0"



PROPOSED CONDITIONS B-B (TYP.)

SCALE: 3/16" = 1'-0"



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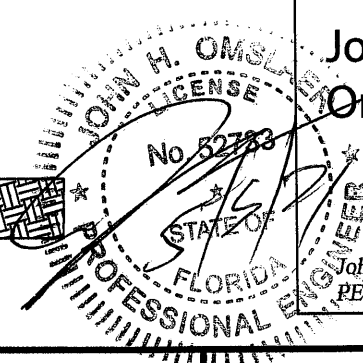
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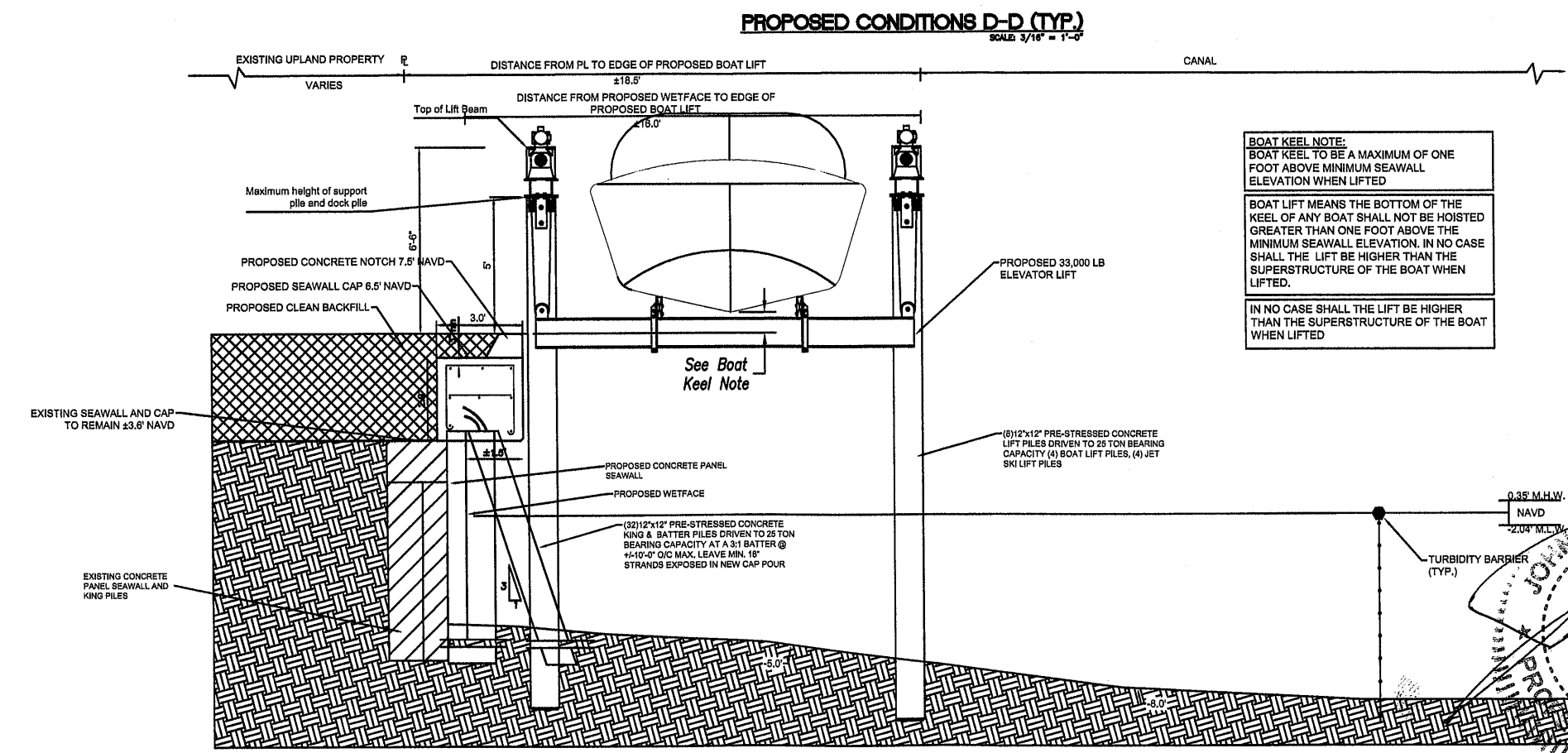
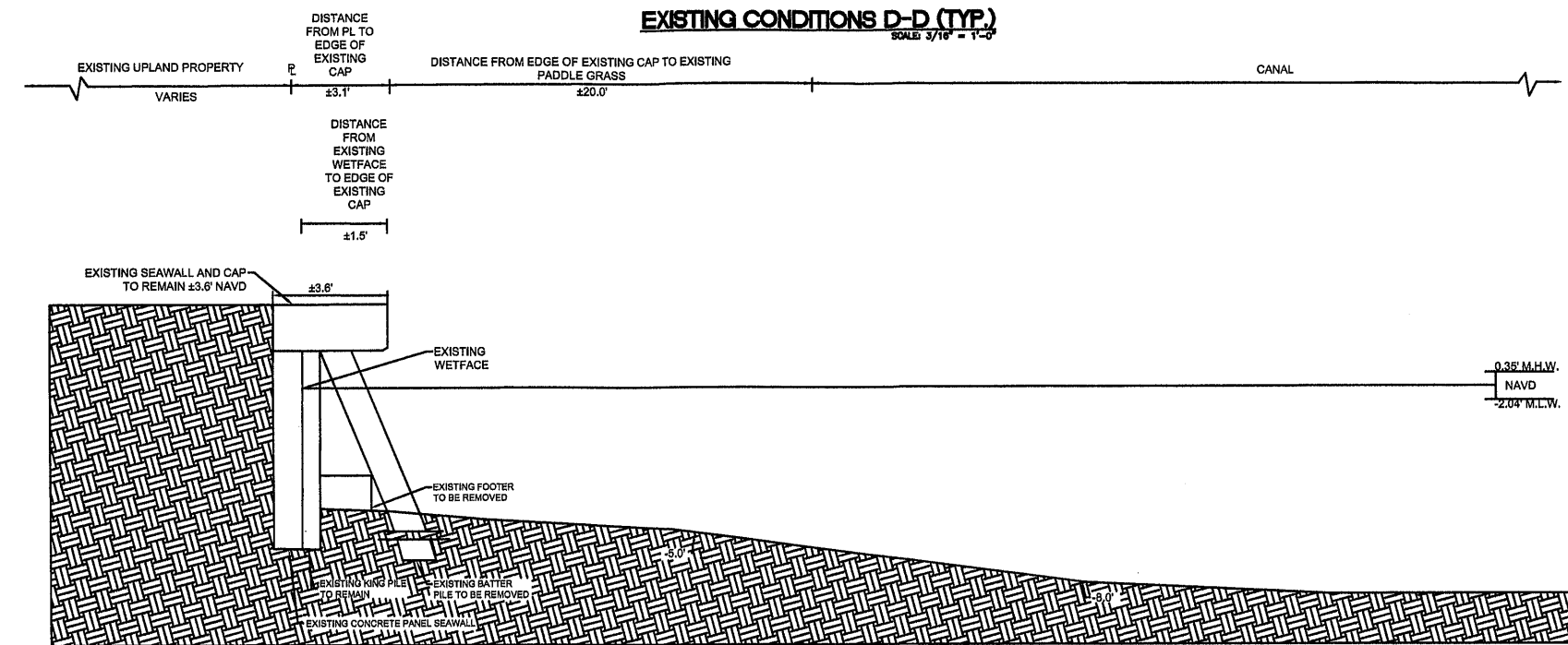
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PREPARED FOR:
DAVE & RANDI STANLEY

SECTIONS

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BOAT KEEL NOTE:
BOAT KEEL TO BE A MAXIMUM OF ONE FOOT ABOVE MINIMUM SEAWALL ELEVATION WHEN LIFTED

BOAT LIFT MEANS THE BOTTOM OF THE KEEL OF ANY BOAT SHALL NOT BE HOISTED GREATER THAN ONE FOOT ABOVE THE MINIMUM SEAWALL ELEVATION. IN NO CASE SHALL THE LIFT BE HIGHER THAN THE SUPERSTRUCTURE OF THE BOAT WHEN LIFTED.

IN NO CASE SHALL THE LIFT BE HIGHER THAN THE SUPERSTRUCTURE OF THE BOAT WHEN LIFTED

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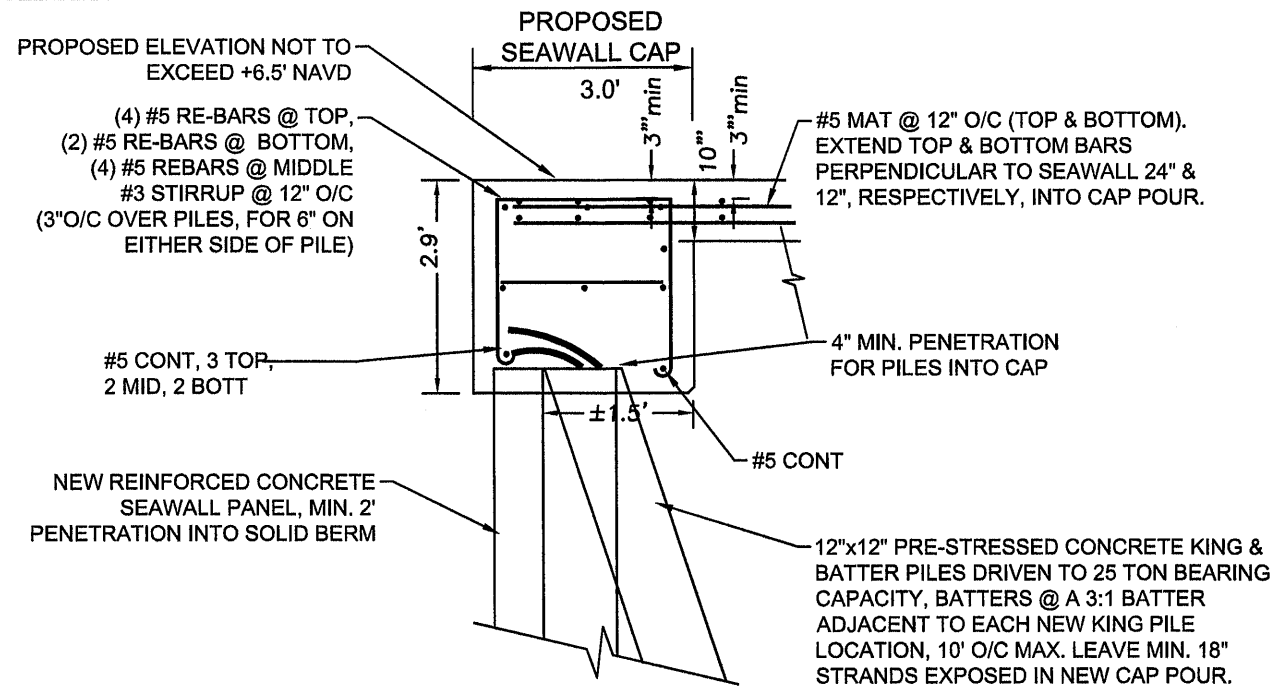
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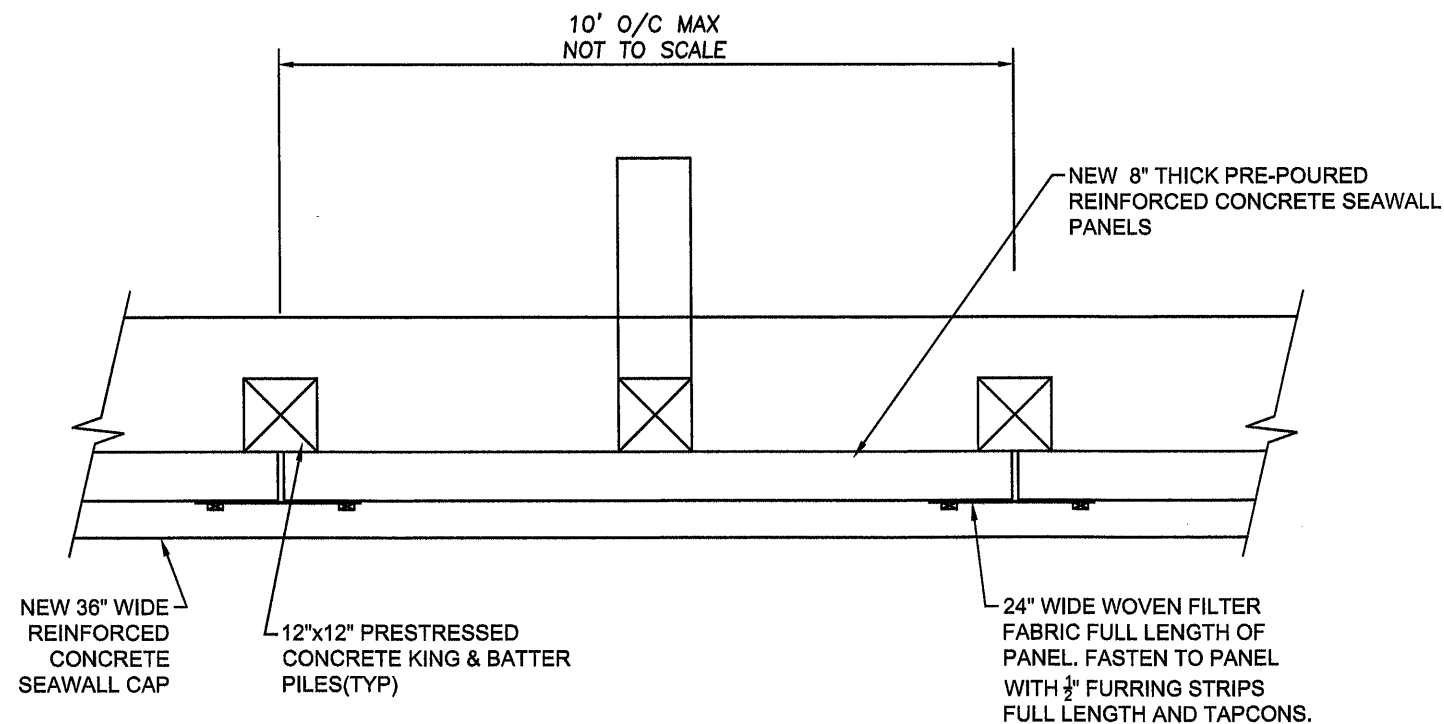
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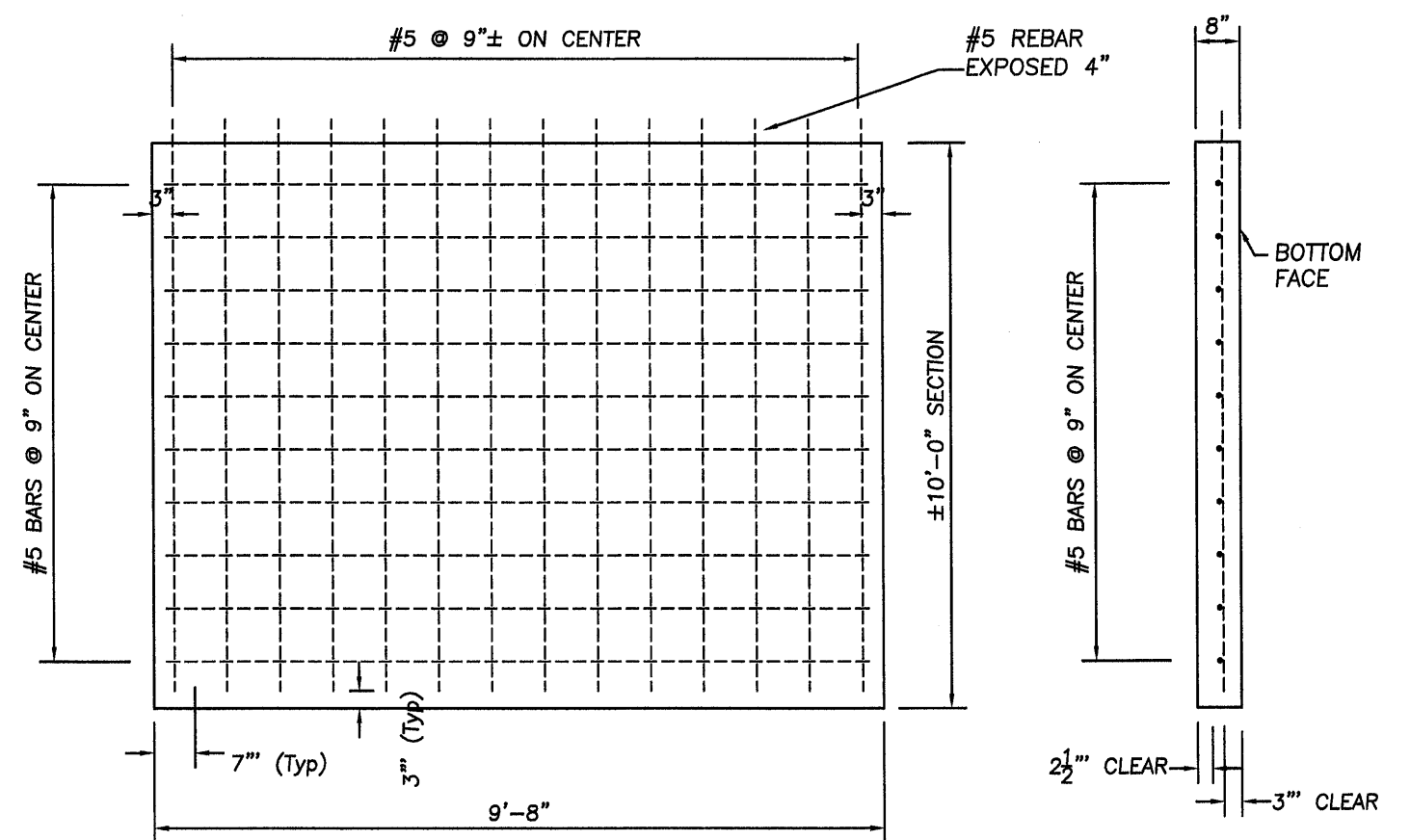
PROPOSED SEAWALL CAP/DOCK SECTION

Scale: 3/8" = 1'



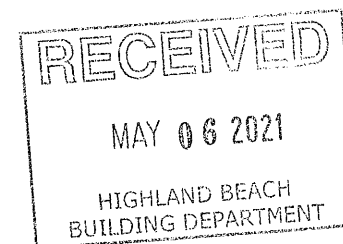
PROPOSED SEAWALL CAP

Scale: 3/8" = 1'



PANEL DETAIL

Scale: NOT TO SCALE



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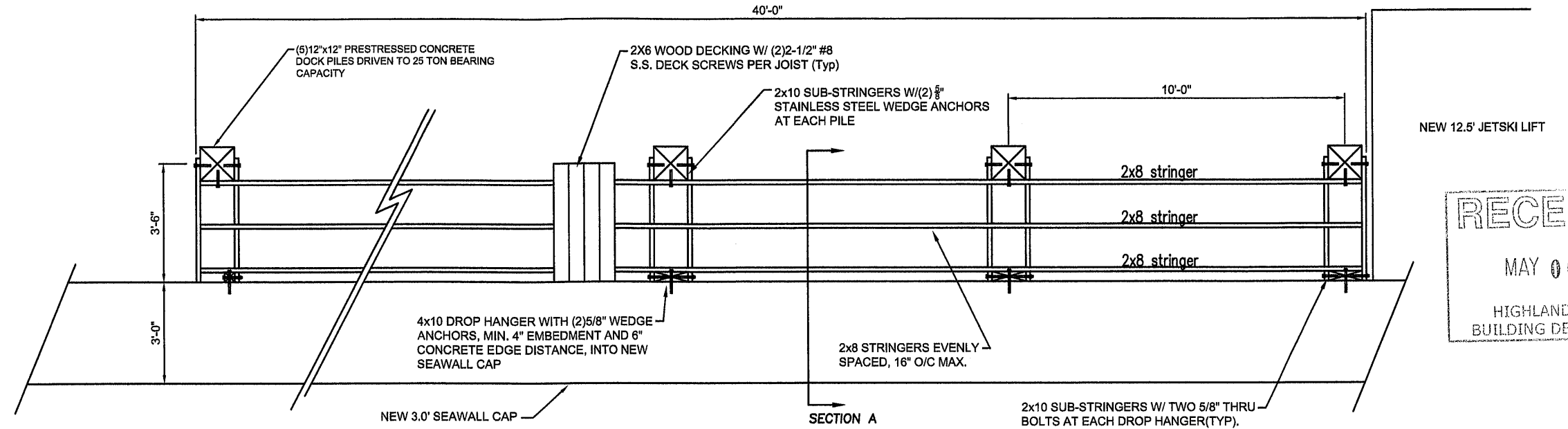
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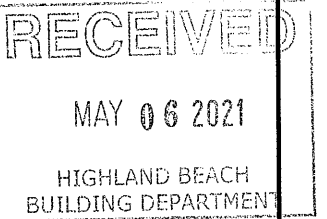
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DETAILS

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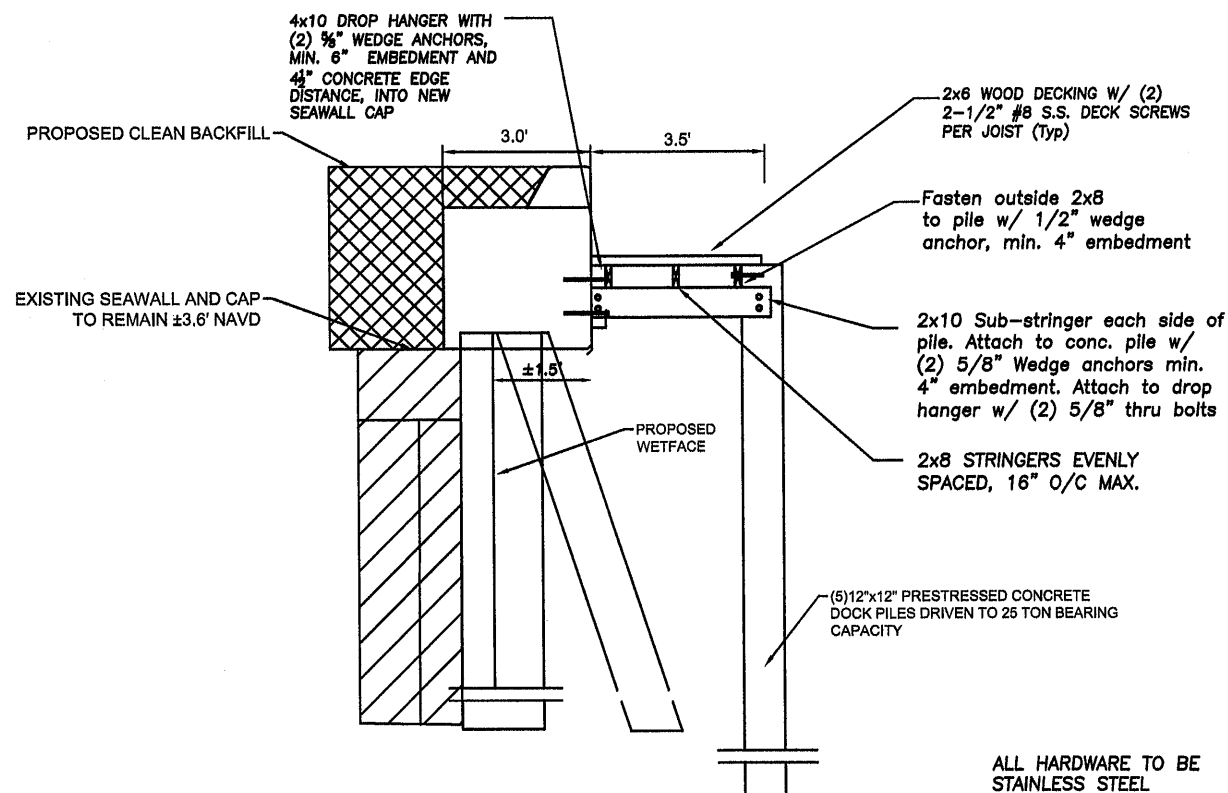
NEW 12.5' JETSKI LIFT



Framing Plan

Scale: 1/4" = 1'-0"

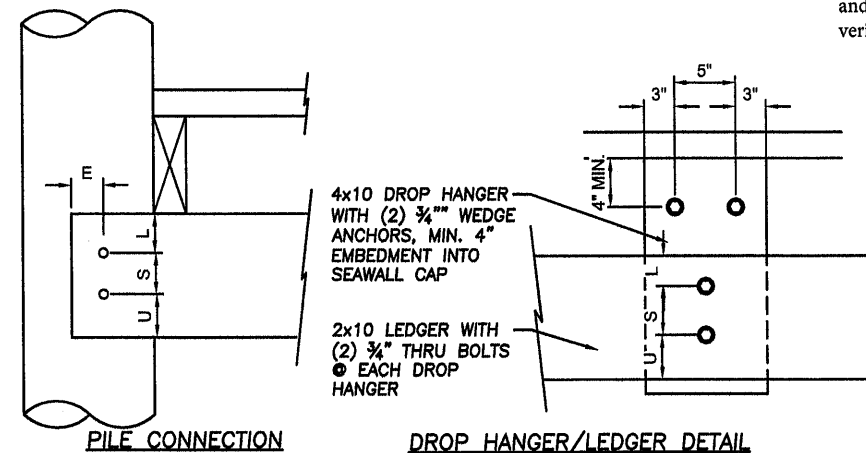
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Wood Dock Section A-A

Scale: 1/4" = 1'-0"

ALL HARDWARE TO BE STAINLESS STEEL



Bolt Diameter	1/2"	5/8"	3/4"	1"
E = End Dist = 4D	2	2 1/2"	3	4
L = Edge Dist, Loaded = 4D	2	2 1/2"	3	4
S = Spacing = 4D	2	2 1/2"	3	4
U = End Dist, Unloaded = 1.5D	3/4	15/16	1 1/8	1 1/2"

NDS Bolt Spacing Requirements

not to scale

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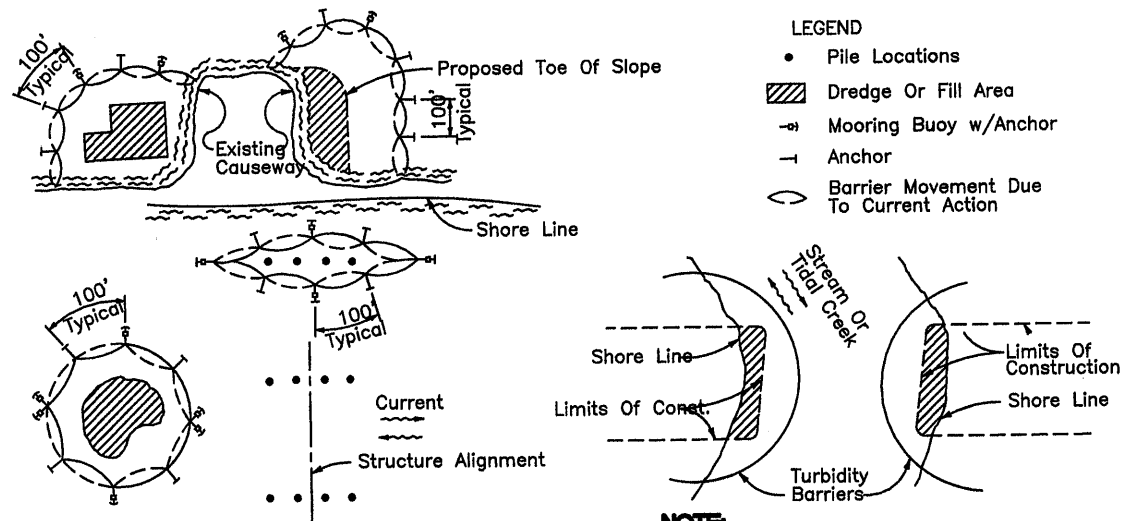
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PREPARED FOR:
DAVE & RANDI STANLEY

DETAILS

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NOTES

1. Turbidity barriers are to be used in all permanent bodies of water regardless of water depth.
2. Number and spacing of anchors dependant on current velocities.
3. Deployment of barrier around pile location may vary to accommodate construction operations.
4. Navigation may require segmenting barrier during construction operations.
5. For additional information see Section 104 of the Standard Specifications.

NOTE

Turbidity barriers for flowing streams and tidal creeks may be either floating, or staked types, or any combinations of types that will suit site conditions and meet erosion control and water quality requirements. The barrier type(s) will be at the Contractor's option unless otherwise specified in the plans. However, payment will be under the pay item(s) established in the plans for Floating Turbidity Barrier and /or Staked Turbidity Barrier. Posts in staked turbidity barriers to be installed in vertical position unless otherwise directed by the Engineer.

Turbidity Barrier

N.T.S.

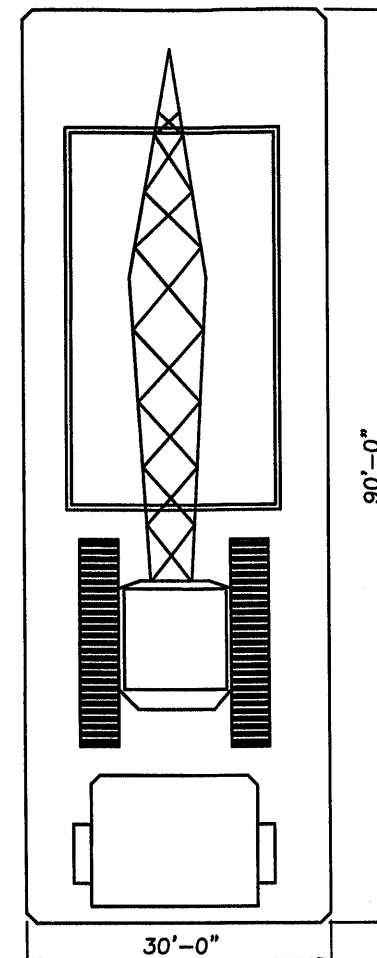
PILE NOTE:

CONCRETE COMPRESSIVE STRENGTH:
3000 PSI @ RELEASE
5000 PSI @ DRIVING

SPIRAL TO BE ASTM A-82 TIGHT HARD-DRAWN WIRE

STRANDS - USE FOUR 7/16" (270K) LOW RELAXATION ASTM 416-87 UNCOATED 7-WIRE STRANDS TENSIONED TO 2.33 KIPS EACH

1-POINT PICKUP-MAXIMUM L = 50'
2-POINT PICKUP-MAXIMUM L = 62'



Construction Barge (typ)

N.T.S.

PILE DRIVING NOTES

1. Pile driving operations shall be observed by a special inspector, including test piles sufficient to determine the approximate length required to meet design capacity.
2. Piles shall be driven using an approved cushion block consisting of material so arranged so as to provide the transmission of the hammer energy.
3. Piles shall be driven to required capacity a minimum of 6 feet into rock or a minimum of 10 feet into yielding material.
4. Piles shall be driven with a drop hammer or gravity hammer provided the weight of the hammer is no less than 3000 pounds, and the fall of the hammer shall not exceed 6 feet.
5. Piles shall be driven with a variation of not more than 1/4" per foot from the vertical, or from the batter line indicated, with a maximum variation of the head of the pile from the position shown on the plans of not more than 3 inches.
6. Where piling must penetrate strata offering high resistance to driving, the structural engineer of record or special inspector may require that the piles be set in pre-drilled or punched holes. The piles shall reach their final penetration by driving.

CONCRETE NOTES

1. Concrete shall conform to ACI 318 (latest edition) and shall be regular weight, sulfate resistant, with a design strength of 5000 psi at 28 days with a maximum water-cementitious materials ratio, by weight, normal aggregate concrete of 0.40.
2. Owner shall employ and pay for testing services from an independent testing laboratory for concrete sampling and testing in accordance with ASTM.
3. Licensed contractor is responsible for the adequacy of forms and shoring and for safe practice in their use and removal.
4. Concrete cover shall be 3" unless otherwise noted on approved drawings.
5. Reinforcing steel shall be in conformance with the latest version of ASTM A615 Grade 60 specifications. All reinforcement shall be placed in accordance with ACI 315 and ACI Manual of Standard Practice.
6. Splices in reinforcing bars shall not be less than 48 bar diameters and reinforcing shall be continuous around all corners and changes in direction. Continuity shall be provided at corners and changes in direction by bending the longitudinal steel around the corner 48 bar diameters.
7. For repair of defective, cracked, or loose concrete, the areas must be cut out and the rebar must be cleaned by sandblasting, coated with zinc, and repaired with at least three inches of epoxy/concrete mix of gunnite concrete with sulfate-resistant cement cover.
8. A 1" deep control joint shall be sawcut in seawall cap at every other pile location, or maximum 20' O/C spacing.
9. Per section 2003.8.4.4 of the 2010 Florida Building Code, all aluminum in contact with concrete shall be protected with alkali-resistant coatings, such as heavy bodied bituminous paint or water-white methacrylate lacquer.

STRUCTURAL TIMBER NOTES

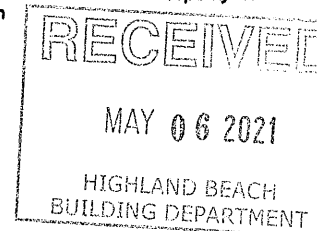
1. All structural timber framing shall be #2 Grade, pressure treated Southern Pine unless otherwise noted.
2. All decking shall be #1 Grade, pressure treated Southern Pine unless otherwise noted.

GENERAL NOTES

1. Construction to follow 2017 Florida Building code, 6th Edition and amendments as applicable and all local, state, and federal laws.
2. Contractor shall verify the existing conditions prior to commencement of the work. Any conflicts or omissions between existing conditions or the various elements of the working drawings shall be brought to the attention of the Engineer prior to the commencement of the work. Contractor and all subcontractors are responsible for all lines, elevations, and measurements in connection with their work.
3. Do not scale drawings for dimensions.
4. Contractor or customer to pay for all permit fees, inspections, and testing required.
5. Contractor to verify location of existing utilities prior to commencing work.
6. Contractor is responsible for all means, methods, and procedures of work.
7. Contractor to obtain all permits as necessary from all local, state, and federal agencies.
8. Contractor to properly fence and secure area with barricades.
9. Any deviation and/or substitution from the information provided herein shall be submitted to the Engineer for approval prior to commencement of work.
10. All unanticipated or unforeseen demolition and/or new construction conditions which require deviation from the plans and notes herein shall be reported to the Engineer prior to commencement of work.
11. All new materials and/or patchwork shall be provided to match existing materials and/or adjoining work where practical except as specifically noted herein.
12. Licensed contractor shall use all possible care to protect all existing materials, surfaces, and furnishings from damage during all phases of construction.
13. The licensed contractor to install and remove all shoring and bracing as required for the proper execution of the work.
14. All new work and/or materials shall conform to all requirements of each administrative body having jurisdiction in each pertaining circumstance.
15. Licensed contractor to verify location of existing utilities prior to commencing work.
16. All elevations shown refer to national geodetic vertical datum (NGVD) of 1929.
17. Turbidity Barriers are to surround all in water construction areas during all construction activities.

CONCRETE PILE NOTES

1. Concrete pile concrete shall attain 8000 psi compressive strength in 28 days.
2. Concrete piles shall be reinforced with (4) 7/8" lo-lax strands, 270 kips, and #5 spiral ties.
3. Concrete piles shall be 12"x12" square, 20 feet minimum length.
4. Concrete piles shall be driven to a minimum bearing capacity of 25 tons.



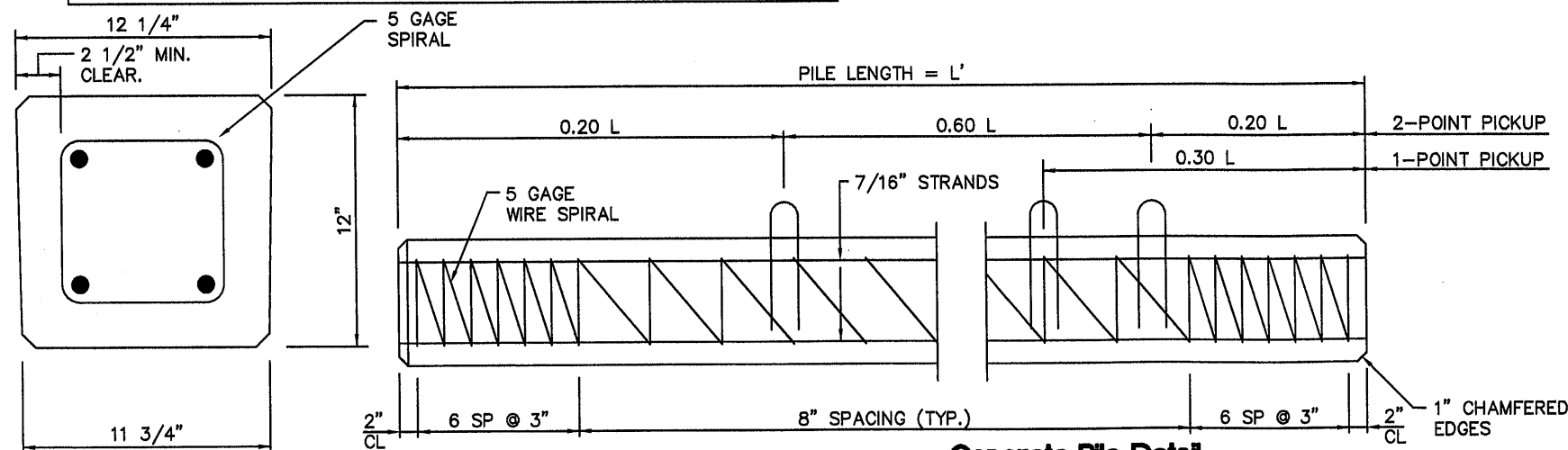
PROJECT ENGINEER:
DYNAMIC ENGINEERING SOLUTIONS, INC.
351 S. Cypress Road, Suite 303
Pompano Beach, FL 33060
Tel: (954) 545-1740
Fax: (954) 545-1721

SEAL / SIGNATURE / DATE

John H Omslae

Digitally signed by John H Omslae
DN: c=US, st=FL, l=Pompano Beach, o=Dynamic Engineering Solutions, Inc., email=John.H.Omslae@dynamiceng.com, cn=John H Omslae, email=John.H.Omslae@dynamiceng.com, Date: 2021.04.23 15:15:25 -0400

This document has been electronically signed and sealed by John H. Omslae, P.E. on date noted using a SHA-1 authentication code. Printed copies are not considered signed and sealed and the SHA-1 authentication code must be verified on any electronic copies per Rule 62-309.000, F.A.C.



Concrete Pile Detail

N.T.S.

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- Environmental Consultants
- Marina & Wetland Permitting
- Mitigation Design & Monitoring
- T&E Species Surveys

4206 INTRACOASTAL DRIVE

PREPARED FOR:
DAVE & RANDI STANLEY

DETAILS

Date: 8/24/2020	Sheet: 8	of: 8
Proj No.: 19-0050		