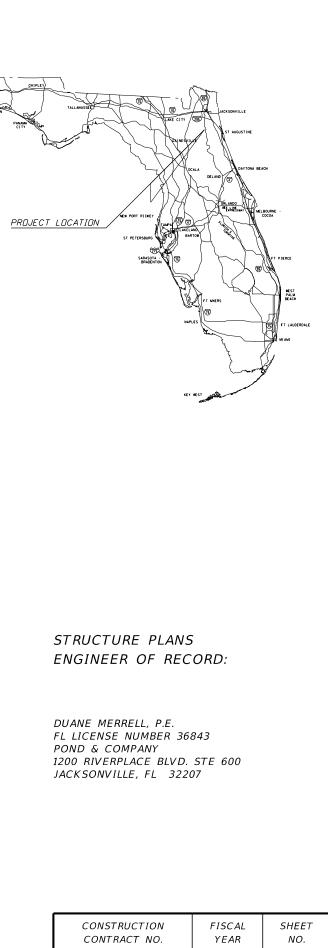
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B1-15 INTERMEDIATE B1-16 FRAMING PLAN B1-17 FINISH GRADE B1-18 FINISH GRADE B1-19 SUPERSTRUCTUR B1-20 SUPERSTRUCTUR B1-21 SUPERSTRUCTUR B1-22 SUPERSTRUCTUR B1-23 SUPERSTRUCTUR B1-24 SUPERSTRUCTUR B1-25 TABLE OF BEAM B1-26 TABLE OF BEAM B1-27 PRESTRESED B1-28 APPROACH SLAE B1-29 APPROACH SLAE B1-30 REINFORCING B. B1-31 REINFORCING B. B1-32 LOAD RATING SU B1-33 FIELD INSTRUM WALLS MSE WALL CONT BW-01 MSE WALL 1 PL	E BENT DETAILS (1 OF 3)	
B1-16 FRAMING PLAN B1-17 FINISH GRADE B1-18 FINISH GRADE B1-19 SUPERSTRUCTUR B1-20 SUPERSTRUCTUR B1-21 SUPERSTRUCTUR B1-22 SUPERSTRUCTUR B1-23 SUPERSTRUCTUR B1-24 SUPERSTRUCTUR B1-25 TABLE OF BEAM B1-26 TABLE OF BEAM B1-27 PRESTRESSED B1-28 APPROACH SLAE B1-29 APPROACH SLAE B1-30 REINFORCING B B1-31 REINFORCING B B1-33 FIELD INSTRUM WALLS MSE WALL CONT BW-01 MSE WALL 1 PL	E BENT DETAILS (2 OF 3)	EDGT STANDARD RIANS FOR BRIDGE CONSTRUCTION (EX 2021.22)
B1-17 FINISH GRADE B1-18 FINISH GRADE B1-19 SUPERSTRUCTUR B1-20 SUPERSTRUCTUR B1-21 SUPERSTRUCTUR B1-22 SUPERSTRUCTUR B1-23 SUPERSTRUCTUR B1-24 SUPERSTRUCTUR B1-25 TABLE OF BEAM B1-26 TABLE OF BEAM B1-27 PRESTRESSED B1-28 APPROACH SLAE B1-29 APPROACH SLAE B1-30 REINFORCING B B1-31 REINFORCING B B1-33 FIELD INSTRUM WALLS MSE WALL CONT BW-01 MSE WALL 1 PL	E BENT DETAILS (3 OF 3)	FDOT STANDARD PLANS FOR BRIDGE CONSTRUCTION (FY 2021-22)
B1-18 FINISH GRADE B1-19 SUPERSTRUCTUR B1-20 SUPERSTRUCTUR B1-21 SUPERSTRUCTUR B1-22 SUPERSTRUCTUR B1-23 SUPERSTRUCTUR B1-24 SUPERSTRUCTUR B1-25 TABLE OF BEAM B1-26 TABLE OF BEAM B1-27 PRESTRESSED B1-28 APPROACH SLAE B1-29 APPROACH SLAE B1-30 REINFORCING B B1-31 REINFORCING B B1-33 FIELD INSTRUM WALLS MSE WALL CONT BW-01 MSE WALL 1 PL	V	400–090 APPROACH SLABS (30 FT.) FLEXIBLE PAVEMENT APPROACHES
B1-19 SUPERSTRUCTUR B1-20 SUPERSTRUCTUR B1-21 SUPERSTRUCTUR B1-22 SUPERSTRUCTUR B1-23 SUPERSTRUCTUR B1-24 SUPERSTRUCTUR B1-25 TABLE OF BEAM B1-26 TABLE OF BEAM B1-27 PRESTRESSED B1-28 APPROACH SLAE B1-29 APPROACH SLAE B1-30 REINFORCING B B1-31 REINFORCING B B1-33 FIELD INSTRUM WALLS MSE WALL CONT BW-01 MSE WALL 1 PL	E ELEVATIONS (1 OF 2)	400–510 COMPOSITE ELASTOMERIC BEARING PADS – PRESTRESSED
B1-20 SUPERSTRUCTUR B1-21 SUPERSTRUCTUR B1-22 SUPERSTRUCTUR B1-23 SUPERSTRUCTUR B1-24 SUPERSTRUCTUR B1-25 TABLE OF BEAM B1-26 TABLE OF BEAM B1-27 PRESTRESSED B1-28 APPROACH SLAE B1-29 APPROACH SLAE B1-30 REINFORCING B B1-31 REINFORCING B B1-32 LOAD RATING SU B1-33 FIELD INSTRUM WALLS MSE WALL CONT BW-01 MSE WALL 1 PL	E ELEVATIONS (2 OF 2)	FLORIDA-I AND AASHTO TYPE II BEAMS
B1-21 SUPERSTRUCTUR B1-22 SUPERSTRUCTUR B1-23 SUPERSTRUCTUR B1-24 SUPERSTRUCTUR B1-25 TABLE OF BEAM B1-26 TABLE OF BEAM B1-27 PRESTRESSED B1-28 APPROACH SLAE B1-29 APPROACH SLAE B1-30 REINFORCING B B1-31 REINFORCING B B1-33 FIELD INSTRUM WALLS MSE WALL CONT BW-01 MSE WALL 1 PL	URE PLAN	415-001 BAR BENDING DETAILS (STEEL)
B1-22 SUPERSTRUCTUR B1-23 SUPERSTRUCTUR B1-24 SUPERSTRUCTUR B1-25 TABLE OF BEAM B1-26 TABLE OF BEAM B1-27 PRESTRESSED B1-28 APPROACH SLAE B1-29 APPROACH SLAE B1-30 REINFORCING B B1-31 REINFORCING B B1-33 FIELD INSTRUM WALLS MSE WALL CONT BW-01 MSE WALL 1 PL	URE REINFORCING DETAILS & NOTES	450–010 FLORIDA–I BEAM – TYPICAL DETAILS AND NOTES
B1-22 SUPERSTRUCTUR B1-23 SUPERSTRUCTUR B1-24 SUPERSTRUCTUR B1-25 TABLE OF BEAM B1-26 TABLE OF BEAM B1-27 PRESTRESSED B1-28 APPROACH SLAE B1-29 APPROACH SLAE B1-30 REINFORCING B B1-31 REINFORCING B B1-33 FIELD INSTRUM WALLS MSE WALL CONT BW-01 MSE WALL 1 PL	URE SECTIONS (1 OF 2)	450–036 FLORIDA-I 36 BEAM – STANDARD DETAILS
B1-23SUPERSTRUCTURB1-24SUPERSTRUCTURB1-25TABLE OF BEAMB1-26TABLE OF BEAMB1-27PRESTRESSEDB1-28APPROACH SLAERB1-29APPROACH SLAERB1-30REINFORCING BRB1-31REINFORCING BRB1-32LOAD RATING SUB1-33FIELD INSTRUMWALLSMSE WALL CONTBW-01MSE WALL 1 PL	URE SECTIONS (2 OF 2)	450–054 FLORIDA-I 54 BEAM – STANDARD DETAILS
B1-24SUPERSTRUCTURB1-25TABLE OF BEAMB1-26TABLE OF BEAMB1-27PRESTRESSEDB1-28APPROACH SLAEB1-29APPROACH SLAEB1-30REINFORCING BB1-31REINFORCING BB1-32LOAD RATING SUB1-33FIELD INSTRUMWALLSMSE WALL CONTBW-01MSE WALL 1 PL	URE DETAILS (1 OF 2)	450–199 PRESTRESSED I-BEAMS BUILD-UP & DEFLECTION DATA
B1-25TABLE OF BEAMB1-26TABLE OF BEAMB1-27PRESTRESSEDB1-28APPROACH SLAEB1-29APPROACH SLAEB1-30REINFORCING BB1-31REINFORCING BB1-32LOAD RATING SUB1-33FIELD INSTRUMWALLSBW-01BW-02MSE WALL CONT	URE DETAILS (2 OF 2)	450–511 BEARING PLATES (TYPE 1) – PRESTRESSED FLORIDA-I AND AASHTO TYPE II BE
B1-26TABLE OF BEAMB1-27PRESTRESSEDB1-28APPROACH SLAEB1-29APPROACH SLAEB1-30REINFORCING B.B1-31REINFORCING B.B1-32LOAD RATING SUB1-33FIELD INSTRUMWALLSBW-01BW-02MSE WALL CONT	AM VARIABLES (1 OF 2)	455-001 SQUARE PRESTRESSED CONCRETE PILES - TYPICAL DETAILS AND NOTES
B1-27PRESTRESSEDB1-28APPROACH SLAEB1-29APPROACH SLAEB1-30REINFORCING B.B1-31REINFORCING B.B1-32LOAD RATING SUB1-33FIELD INSTRUMWALLSBW-01BW-02MSE WALL CONT	AM VARIABLES (2 OF 2)	455-024 24" SQUARE PRESTRESSED CONCRETE FILES - THICKE DETAILS AND NOTES
B1-28APPROACH SLAEB1-29APPROACH SLAEB1-30REINFORCING B.B1-31REINFORCING B.B1-32LOAD RATING SUB1-33FIELD INSTRUMWALLSBW-01BW-02MSE WALL CONTBW-02MSE WALL 1 PL	D BEAM TEMPORARY BRACING DATA TABLE	455-024 24 SQUARE FRESTRESSED CONCRETE FILE 458-110 EXPANSION JOINT SYSTEM - POURED JOINT WITH BACKER ROD
B1-29APPROACH SLAEB1-30REINFORCING B.B1-31REINFORCING B.B1-32LOAD RATING SUB1-33FIELD INSTRUMWALLSBW-01BW-02MSE WALL CONTBW-02MSE WALL 1 PL		
B1-30REINFORCING B.B1-31REINFORCING B.B1-32LOAD RATING SUB1-33FIELD INSTRUMWALLSBW-01BW-02MSE WALL CONTBW-02MSE WALL 1 PL		520-020 TRAFFIC SEPARATOR (TYPE "F" CURB)
B1-31REINFORCING B.B1-32LOAD RATING SUB1-33FIELD INSTRUMWALLSBW-01BW-02MSE WALL CONTBW-02MSE WALL 1 PL		521-427 TRAFFIC RAILING - (36" SINGLE-SLOPE)
B1-32LOAD RATING SUB1-33FIELD INSTRUMWALLSBW-01BW-02MSE WALL CONTBW-02MSE WALL 1 PL		521-428 TRAFFIC RAILING - (42" SINGLE-SLOPE)
B1-33FIELD INSTRUMWALLSMSE WALL CONTBW-01MSE WALL CONTBW-02MSE WALL 1 PL	BAR LIST (2 OF 2)	521–660 LIGHT POLE PEDESTAL – BRIDGE
WALLSBW-01MSE WALL CONTBW-02MSE WALL 1 PL		521–825 PEDESTRIAN/BICYCLE RAILING (42" CONCRETE)
BW-01MSE WALL CONTBW-02MSE WALL 1 PL	IMENTATION LOCATION PLAN	548–020 MSE RETAINING WALL SYSTEMS (PERMANENT)
BW-01MSE WALL CONTBW-02MSE WALL 1 PL		550–013 BRIDGE FENCING (OVER RAILROAD)
BW-02 MSE WALL 1 PL		630-010 CONDUIT DETAILS - EMBEDDED
RW_03 MSE WALL 2A P	PLAN & ELEVATION	
DW-05 MJL WALL ZA T	PLAN & ELEVATION	
BW-04 MSE WALL (2B &	3 & 2C) PLAN & ELEVATION	
BW-05 MSE WALL DETA	TAILS	
BW-06 MSE WALL DATA	TA TABLES	
		RELEASED FOR CONSTRUCTION



D. R. HORTON



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6507 J 01/	PAY ITEM				QUANT I TY	TOT	ΓAL			
SECTION	NO.	PAY ITEM DESCRIPTION	LOCATION	UNIT	P F	Р				
			End Bent 1	LF	440		1			
	155 DA 5	Drectrosced Concrete Dilling 24" CO	Int. Bent 2	LF	360	1 5 2 0	1			
	455-54-5	Prestressed Concrete Piling, 24" SQ	Int. Bent 3	LF	360	1,520	1			
			End Bent 4	LF	360		1			
FOUNDAT I ON				End Bent 1	LF	70		1		
TOUNDATION	155 112 5	Test Piles-Prestressed Concrete 24" SQ	Int. Bent 2	LF	60	250	l l			
	455-145-5	rest rires-riestressed concrete 24 50	Int. Bent 3	LF	60	250	l I			
			End Bent 4	LF	60		1			
	459-71	Polyethylene Sheeting on Concrete Piles	End Bent 1	SY	60	120	1			
	459-71	Foryethyrene Sheeting on concrete Fires	End Bent 4	SY	60	120	1			
WALLS	548-12	Retaining Wall System, Permanent, Excluding Barrier	Wall 1	SF	7,138	28566	1			
WALLS	546-12	Recarifing warr System, Fermanent, Excluding Barrier	Wall 2	SF	21,428	28500	I			
			End Bent 1	CY	36.5		1			
	400-4-5	Concrete Class IV, Bridge Substructure	Int. Bent 2	CY	45.3	164	1			
	400-4-5	concrete erass iv, brrage substracture	Int. Bent 3	CY	45.3	104	l I			
JBSTRUCTURE			End Bent 4	CY	36.5		I			
DDJINOCIONE	415 - 1 - 5		End Bent 1	LB	2,591		l l			
		Reinforcing Steel - Bridge Substructure	Int. Bent 2	LB	17,303	39,870	1			
			Int. Bent 3	LB	17,303	,	l I			
				End Bent 4	LB	2,673		I		
	400-2-10	Concrete Class II, Approach Slabs	Approach Slab 1	CY	77.5	155	1			
PROACH SLABS			Approach Slab 2	СҮ	77.5		I			
	415-1-9	Reinforcing Steel - Approach Slabs	Approach Slab 1	LB	6,317	12,634	1			
			Approach Slab 2	LB	6,317		 			
		Concrete Class II, Bridge Superstructure	Spans 1 & 2	CY	408.5	408.5	 			
		Reinforcing Steel - Bridge Superstructure	Spans 1 & 2	LB	48,559	48559.0	I			
	400-9	Bridge Deck Grooving & Planing, Deck 8.5" and Greater	Bridge Deck & App. Slabs	SY	1393	1393.0	I			
	400 - 147	400 - 147	400-147	400-147	Composite Neoprene Pads	All Spans	CF	7.6	7.6	I
PERSTRUCTURE	450-2-36	Prestressed Beams, Florida I-Beam 36"	Span 1	LF	288	576	l I			
			Span 3	LF	288	1.150	I			
	450-2-54	Prestressed Beams, Florida I-Beam 54"	Span 2	LF	1152	1,152	I			
	458-1-11	Bridge Deck Expansion Joint, New Construction, F&I Poured Joint With Backer Rod	Begin Bridge	LF	27	54	l I			
		WILLI BALKET KOU	End Bridge	LF	27		 			
	531 5 13		Approach Slab 1	LF	30	252	l I			
	521-5-13	Concrete Traffic Railing - Bridge, 36" Single-Slope	Bridge Deck	LF	192	252	l I			
			Approach Slab 2	LF	30		 			
RAILING/	534 5 44		Approach Slab 1	LF	30	252	l I			
BARRIERS	521-5-14	Concrete Traffic Railing - Bridge, 42" Single-Slope	Bridge Deck	LF	192	252	1			
			Approach Slab 2	LF	30		 			
	531 6 13		Approach Slab 1	LF	30	252	l I			
	521-6-12	Concrete Parapet, Pedestrian/Bicycle, 42" Height	Bridge Deck	LF	192	252	1			
			Approach Slab 2	LF	30		I			
CONDUIT	630-2-16	Conduit F&I Embedded Concrete Barriers and Traffic Railings	36" Single-Slope Traffic Railing 42" Single-Slope Traffic Railing	LF	756	756	l			
NCTION DOVES	C 2 5 2 1 2	lunchion Dev 561 Enhadded	36" Single-Slope Traffic Railing	EA	6		i			
NCTION BOXES	635-3-13	Junction Box F&I Embedded	42" Single-Slope Traffic Railing	EA	o	6	L			

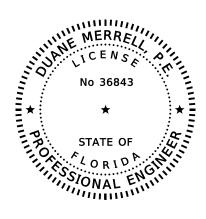
		REVI	SIONS				DRAWN BY		STATE OF FL	ORIDA	SHEET TITLE:	
DATE B	BY	DESCRIPTION	DATE	BY	DESCRIPTION	DUANE MERRELL, FL P.E. P.E. LICENSE NUMBER 36843	J.F. I-21 CHECKED BY: R.K. I-21	DEPA		ANSPORTATION		SUMMARY OF STRUCTURE QUANTITIES
						POND & COMPANY 1200 RIVERPLACE BLVD. STE 600 JACKSONVILLE, FL 32207	DESIGNED BY: D.M. I-21 CHECKED BY: R.K. I-21	ROAD NO.	CLAY	FINANCIAL PROJECT ID	PROJECT NAME:	PEARCE BLVD. BRIDGE OVER CSX RAILROAD
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DESIGN NOTES		CONSTRU REMAR	CTION RKS
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			1 40 -
	BRIDGE	: NO. 7	
			REF. DWG.

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SHEET NO.

BQ1-01



THIS DOCUMENT HAS BEEN DIGITALLY SIGNED AND SEALED BY:

Duane R Merrell 2022.06.28 10:24:07 -04'00'

ON THE DATE ADJACENT TO THE SEAL.

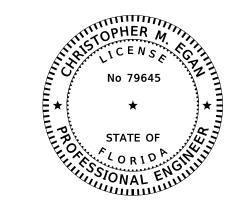
PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED. THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC DOCUMENTS.

POND & COMPANY 1200 RIVERPLACE BLVD., STE. 600 JACKSONVILLE, FL 32207 DUANE MERRELL, P.E. NO. 36843

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

STRUCTURE PLANS

<u>SHEET NO</u> .							CULE		
	<u>SHEET DESCRIPTION</u>	<u>Sheet no</u> .	SHEET DESCRIPTION				<u>SHEE</u>	<u>I NU</u> .	<u>Sheet i</u>
B-1	KEY SHEET	BW-01	MSE WALL CONTROL PLAN				B-2		SIGNATURE S
BQ1-01	SUMMARY OF STRUCTURE QUANTITIES (BRIDGE NO. XXXXXX)	BW-02	MSE WALL 1 PLAN & ELEVATION				B-8		REPORT OF
B-2	SIGNATURE SHEET	BW-02 BW-03	MSE WALL 2A PLAN & ELEVATION				B-9		REPORT OF
B-3	SPECIAL NOTES	BW-04	MSE WALL (2B & 2C) PLAN & ELEVATION	N			B-10		REPORT OF
Б-3 В-4	GENERAL NOTES	BW-05	MSE WALL (26 & 20) PLAN & ELEVATION MSE WALL DETAILS	v			B-11		REPORT OF
B-4 B-5	PROJECT LOCATION	BW-05 BW-06	MSE WALL DATA TABLES				B-12		REPORT OF
B-5 B-6		DW-00	MSE WALL DATA TABLES				B-13		REPORT OF
В-0 В-7	UTILITY ADJUSTMENT						B-14		REPORT OF
	RAILROAD CLEARANCE DIAGRAM						B-15		REPORT OF
B1-01	PLAN AND ELEVATION						B-16		REPORT OF
B1-02	TYPICAL SECTIONS						B1-33	3	FIELD INST
B1-03	FOUNDATION LAYOUT								
B1-04	PILE DATA TABLE								
B1-05	END BENT 1								
B1-06	END BENT 4								
B1-07	END BENT DETAILS (1 OF 4)								
B1-08	END BENT DETAILS (2 OF 4)								
B1-09	END BENT DETAILS (3 OF 4)								
B1-10	END BENT DETAILS (4 OF 4)								
B1-11	INTERMEDIATE BENT 2								
B1-12	INTERMEDIATE BENT 3								
B1-13	INTERMEDIATE BENT DETAILS (1 OF 3)								
B1-14	INTERMEDIATE BENT DETAILS (2 OF 3)								
B1-15	INTERMEDIATE BENT DETAILS (3 OF 3)								
B1-16	FRAMING PLAN								
B1-17	FINISH GRADE ELEVATIONS (1 OF 2)								
B1-18	FINISH GRADE ELEVATIONS (2 OF 2)								
B1-19	SUPERSTRUCTURE PLAN								
B1-20	SUPERSTRUCTURE REINFORCING DETAILS & NOTES								
B1-21	SUPERSTRUCTURE SECTIONS (1 OF 2)								
B1-22	SUPERSTRUCTURE SECTIONS (2 OF 2)								
B1-23	SUPERSTRUCTURE DETAILS (1 OF 2)								
B1-24	SUPERSTRUCTURE DETAILS (2 OF 2)								
B1-25	TABLE OF BEAM VARIABLES (1 OF 2)								
B1-26	TABLE OF BEAM VARIABLES (2 OF 2)								
B1-27	PRESTRESSED BEAM TEMPORARY BRACING DATA TABLE								
B1-28	APPROACH SLABS								
B1-29	APPROACH SLAB DETAIL								
B1-29 B1-30	REINFORCING BAR LIST (1 OF 2)								
B1-31	REINFORCING BAR LIST (2 OF 2)								
B1-32	LOAD RATING SUMMARY SHEET								
				DDAMALDV4				SHEET TITLE:	
	REVISIONS		DUANE MERRELL, FL P.E.	DRAWN BY: J.F. 1-21		STATE OF F		SHEET TITLE:	
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E BY	DESCRIPTION DATE BY DESCRIPT	ION	P.E. LICENSE NUMBER 36843	CHECKED BY: R.K. 1-21	DEPA	RTMENT OF TR	ANSPORTATION		
E BY	DESCRIPTION DATE BY DESCRIPT	ON	P.E. LICENSE NUMBER 36843 POND & COMPANY	R.K. I-21 DESIGNED BY:	DE PAI ROAD NO.	RTMENT OF TR	ANS PORTATION FINANCIAL PROJECT ID	PROJECT NAME:	
Е ВУ	DESCRIPTION DATE BY DESCRIPT	ION	P.E. LICENSE NUMBER 36843	R.K. I-21				PROJECT NAME:	PEARCE



NUTOPHER M.

STRUCTURE PLANS

THIS DOCUMENT HAS BEEN DIGITALLY SIGNED AND SEALED BY: Ovintegher M. Ager, Gate of Revie Brainwer, Janmar No. 79535

This bern has been digitally signed and sested by Overspher Eges on the date indicated here. Ninded captes of this document are not canobe upped and acalest and the captores must be

2022.06.28 11:38: ON THE DATE ADJACENT TO THE SEAL.

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED. THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC DOCUMENTS.

ECS FLORIDA, LLC 11554 DAVIS CREEK COURT JACKSONVILLE, FL 32256 CHRISTOPHER M. EGAN, P.E. NO. 79645

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

SHEET DESCRIPTION

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			BORING	(1	0F	9)	
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DRT	0F	CORE	BORING	(3	0F	9)	
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DRT	0F	CORE	BORING	(7	0F	9)	
DRT	0F	CORE	BORING	(8	0F	9)	
DRT	0F	CORE	BORING	(9	0F	9)	
DI	NST	RUMEI	NTATION	LO	CAT	ION	PLAN

BRIDGE NO. 7	14054
	REF. DWG. NO.
SIGNATURE SHEET	
	SHEET NO.
ARCE BLVD. BRIDGE OVER CSX RAILROAD	B - 2
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SPECIAL NOTES

- 1. All work on, over, under, or adjacent to CSX right-of-way shall be done in accordance with the CSX Special Provisions, which can be found within the CSX Public Projects Manual, available at: https://www.csx.com/index.cfm/about-us/property/.
- 2. No work shall take place within 50 feet of the centerline of the CSX track without a railroad flagman being present. Provide the CSX field representative with at least thirty (30) business days advance notice of beginning work within this area to allow for the scheduling of the railroad flagman.
- 3. Construction clearances shall be subject to approval by CSX. Typically, reduction in clearance for construction is not permitted.
- 4. Contractor shall maintain all ditches and drainage structures free of silt or other obstructions that may result from their operations. Contractor, upon completion of the Project, shall leave CSX Property in neat a condition, satisfactory to the CSX Representative.
- 5. The Contractor shall provide, install and maintain a geotextile fabric ballast protection system to prevent debris and fines from fouling the ballast. The ballast protection system shall extend 25' beyond the outer limits of the bridge on both sides.
- 6. The Contractor may not use CSX right-of-way for storage of materials or equipment during construction without prior approval from CSX.
- 7. CSX shall be furnished as-built drawings showing actual clearances as constructed prior to project completion and close-out.
- 8. The Contractor shall reference the CSX Construction Submission Criteria for construction related submittal requirements while working on, over, under or adjacent to CSX right-of-way. The Construction Submission Criteria can be found within the Public Project Manual. The Contractor(s) is required to submit a detailed work plan for review and approval by CSX, including but not limited to the below items: a) Foundation Installation b) Girder Erection and Stabilization c) Protective Fencing Detail
- 9. The Contractor shall notify and coordinate their work with the on-site CSX Representative.
- 10. "One Call" services do not locate buried railroad signal and communications lines. The contractor shall contact the railroad's representative two (2) days in advance of those places where excavation, pile driving, or heavy loads may damage railroad underground lines on railroad property. Upon request from the contractor or agency, railroad signal forces will locate and paint mark or flag railroad underground signal, communication, and power lines in the area to be disturbed for the contractor. The contractor shall avoid excavation or other disturbance of these lines which are critical to the safety of the railroad and the public. If disturbance or excavation is required near a buried railroad signal, communication, or power line, the line shall be potholed manually with careful hand excavation by the contractor and protected by the contractor during the course of the disturbance under the supervision and direction of a railroad signal representative.
- 11. All soils excavated within CSX's railroad right-of-way shall remain on CSX's right-of-way. For any excavated soil that requires off-site disposal, the licensee is required to use only CSX approved laboratories, transporters, and disposal facility that are in compliance with all applicable environmental laws and CSX's policies and procedures. Soil resulting from excavation outside of CSX's railroad right-of-way or railroad owned property shall not be brought onto CSX's property and therefore must be stored off CSX property. CSX shall not incur any costs related to the disposal of soils generated due to construction activity related to this project.

	REVIS	SIONS				DRAWN BY		STATE OF I	LORIDA	SHEET TITLE:		REF. DWG. NO.
DATE	BY DESCRIPTION	DATE	BY DESCR	RIPTION	DUANE MERRELL, FL P.E. P.E. LICENSE NUMBER 36843	J.F. 1-21 CHECKED BY: B.K. 1-21	DEPAR		RANSPORTATION		SPECIAL NOTES	
					POND & COMPANY 1200 RIVERPLACE BLVD. STE 600	DESIGNED BY: D.M. I-21	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:		SHEET NO.
					JACKSONVILLE, FL 32207	CHECKED BY: R.K. I-21	N/A	CLAY	N/A		PEARCE BLVD. BRIDGE OVER CSX RAILROAD	В-3
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GENERAL NOTES

А.	Design Specific	cations:						3) Concrete Cover:	
	1) FDOT Struc	tures Manual dated Janu	uary 2022	2.				C.I.P. Superstructure (Top of Deck)	2½"
		5	,	Transportation Officials (,		istance Factor		C.I.P. Superstructure (Except Top of Deck)	2"
				ition and all subsequent in				C.I.P. Substructure (Cast Against Earth) C.I.P. Substructure (Formed Surfaces)	4" 3"
		,	,	nd Structures Design Bulle	etins.			C.I.P. Substructure (Formed Surraces) C.I.P. Substructure (Top of Beam Pedestals)	3 2"
Β.		dards and Construction .						Precast Prestressed Beams (Except Top Surface)	
	Florida Depar January 2022	tment of Transportation Standard Specifications	, FY 2022 ; for Road	2–23 Standard Plans and r d and Bridge Construction,	evised Index Drawings as amended by Contra	s as appended herein, and a sect Documents.		Top Surface of Beam Top Flange	
C.				rom the North American V					
D.	Environment:			Substructure				Concrete cover dimensions shown in the plans do not i "minimum cover". See Specification 415 for allowable t	
		Superstructure	Co	ncrete	Steel			reinforcing steel are to centerline of bar except wher	
		Slightly		lerately	Moderately			· · · · · · · · · · · · · · · · · · ·	
E.	Design Methodo	ology: Load and Resista	nce Facto	or Design (LRFD) method u	sing strength (extreme	e event), service and		Provide $^{3}\!$	erwise no
	fatigue limit st						Н.	Surface Finish: A Class 2 Surface Finish shall be app	lied to to
F.	Design Loading 1) Live Loads:							ends of intermediate bent caps as shown in the Finish	
	,	Dynamic Load Allowance	۵				I.	Plan Dimensions: All dimensions in these plans are me	acurod ir
	2) Dead Loads	,	-				1.	otherwise noted.	asureu n
	· · · · · · · · · · · · · · · · · · ·	 le Slope Traffic Railing:		430 plf					
		le Slope Traffic Railing:		580 plf			J.	Utilities: For plan locations of existing utilities, see P	lan and E
	-	Place Forms:		20 pcf				plans are approximate. For disposition of utilities, see	e the Util
	d. Reinforc	ed Concrete:		150 pcf			K	Bridge Name and Number: Place the following bridge n	amo and
	e. Future M	learing Surface:		Design does not include	e an allowance for fut	ure wearing surface	К.	Traffic Railing Design Standards:	ante anu
	f. The 8½"	deck thickness includes	a one- h	alf inch sacrificial thickne	ess included in the			ranne nannig Design Standards.	
				he section properties use	d for design.			Name: CSX RR	
	g. 4" Wide	Type "F" Traffic Separat	tor	292 plf				Number: 714054	
	h. Bridge F	encing (Curved Top):		40 plf			,	Consultan Deal. Consultation and for a fithe Deid	
	i. 42" Concr	rete Pedestrian/Bicycle	Railing:	320 plf			L.	Screeding Deck: Screed the riding surface of the Brid Elevations shown in the plans. Account for the theore	
	3) Constructio							deck forming systems, construction loads and temporal	
	5	g Machine Load:			7 kips			deek for ming systems, construction rouds and temporal	' , 51101111
	5		n beyond i	the edge of deck overhang			М.	Stay-In-Place Deck Forms: Design includes allowance	for 20 ps
	c. Construc	tion Live Load:			,	ver the entire bridge width gitudinal length centered achine.		for the unit weight of the metal forms and the concre not allowed at deck cantilevers.	te requir
	d. Removab	le Deck Cantilever Timbe	er Forms	with Overhang Brackets:	15 psf		N.	Joints In Concrete: Construction joints will be permitte	ed only at
	e. Live load	l at or near the outside	edge of	deck during deck casting	75 psf applied as a length of 20 fee	a moving load over t.		construction joints or alterations to those shown shall	
	f. Construc	tion Inactive Design Win	d Speed:		90 MPH				
	g. Velocity	Pressure Exposure Coel	fficient (k	(z):	1.14				
	h. Construc	tion Active Design Wind	Speed:		30 MPH				
	4) Vehicle Col	lision Force:			Not Applicable			┌── Class 2	
	5) Utilities: No	o allowance for utility lo	oads has	been included in the desig	ın.			Surface Finish	1
G.	Materials:								1
	1) Reinforcing	Steel: Grade 60 Carbon	n Steel pe	er Specification 931.					

2) Concrete:

	Min. 28–Day	
Class	Compressive Strength (psi)	Location
II	3400	Traffic Railing
II (Bridge Deck)	4500	Bridge Deck & Approach Slabs
IV	5500	C.I.P. Substructure
VI	8500	Prestressed Concrete Beams
V (Special)	6000	Prestressed Concrete Piles

END BENTS 1 & 4

FINISH DETAIL

	REVIS	SIONS				DRAWN BY		STATE OF FI	ORIDA	SHEET TITLE:		REF. DWG. NO.
DATE	BY DESCRIPTION	DATE	BY	DESCRIPTION	DUANE MERRELL, FL P.E. P.E. LICENSE NUMBER 36843	J.F. I-21 CHECKED BY:			ANSPORTATION		GENERAL NOTES	
					POND & COMPANY 1200 RIVERPLACE BLVD. STE 600	DESIGNED BY: D.M. I-21	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:		SHEET NO.
					JACKSONVILLE, FL 32207	CHECKED BY: R.K. I-21	N/A	CLAY	N/A		PEARCE BLVD. BRIDGE OVER CSX RAILROAD	B - 4
						MerrellD	6	6/27/2022 5:52	2:40 PM		X:\FY21\1210023\04.CAD\struct\B1Gene	eralNotes02.dg

Ide placement and fabrication tolerances unless shown as ances. All dimensions pertaining to the location of ear dimension is noted to face of concrete.

se noted.

to top and sides of all railings, bridge deck coping, and tail on this sheet.

red in feet either horizontally or vertically unless

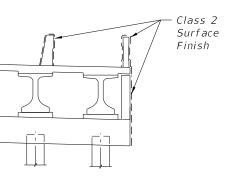
and Elevation sheet. Locations of utilities shown in the e Utility Adjustment sheet in the Roadway plans.

and number on the traffic railings in accordance with the

Deck and Approach slabs to achieve the Finish Grade I deflections due to self weight, deck casting sequence, horing, etc., as required.

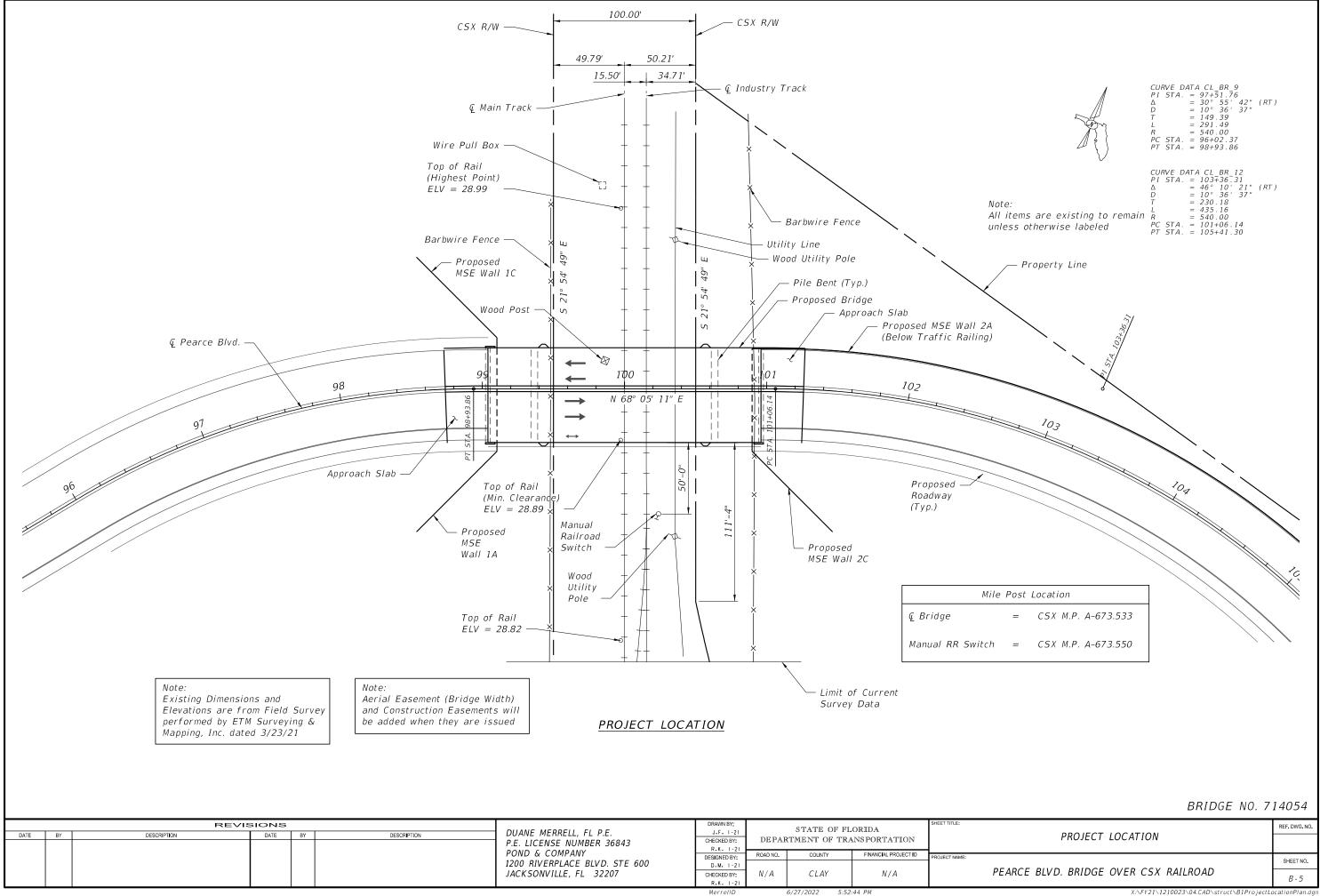
20 psf over the projected plan area of the metal forms equired to fill the form flutes. Stay-in-place forms are

only at the locations indicated in the plans. Additional quire approval of the Engineer.

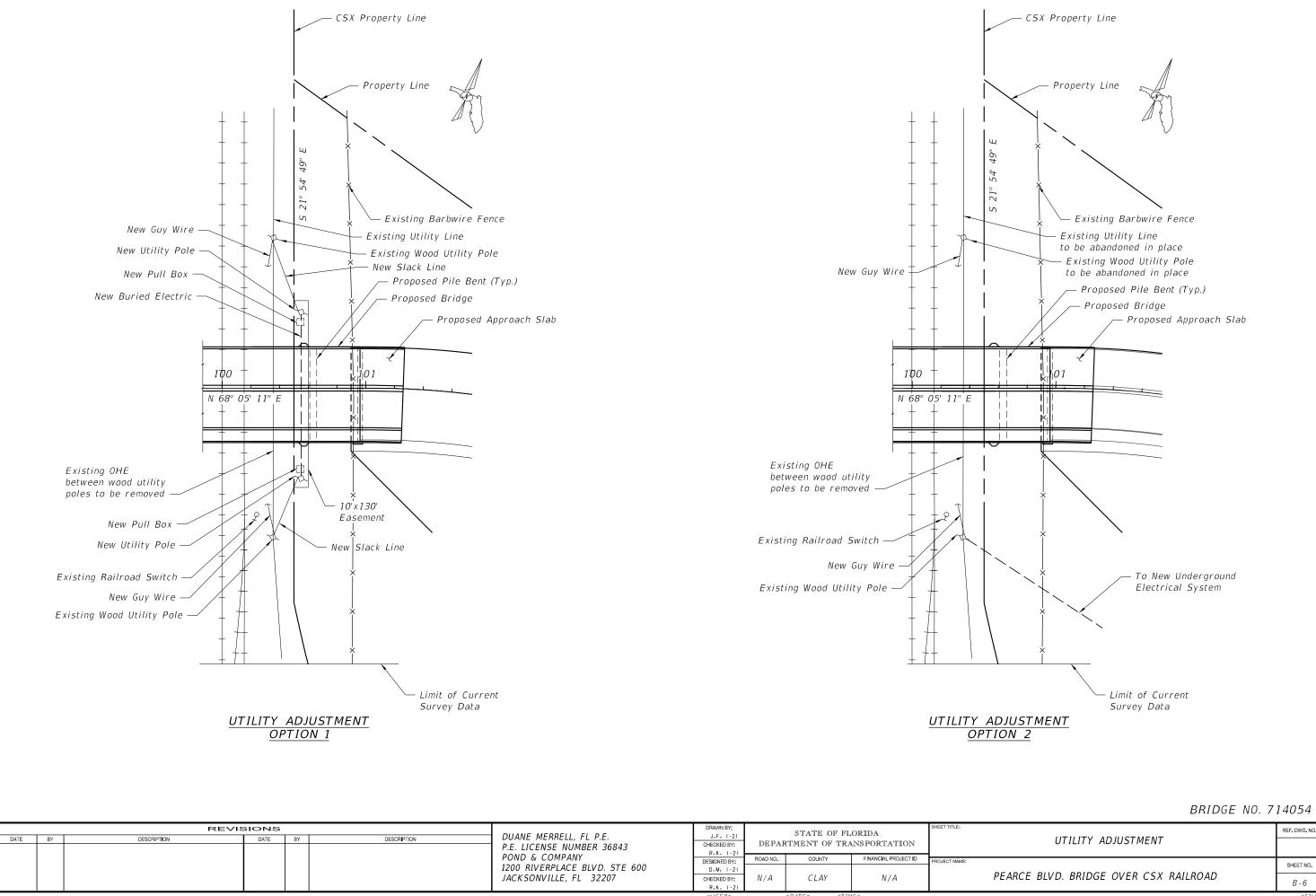


INTERMEDIATE BENTS 2 & 3

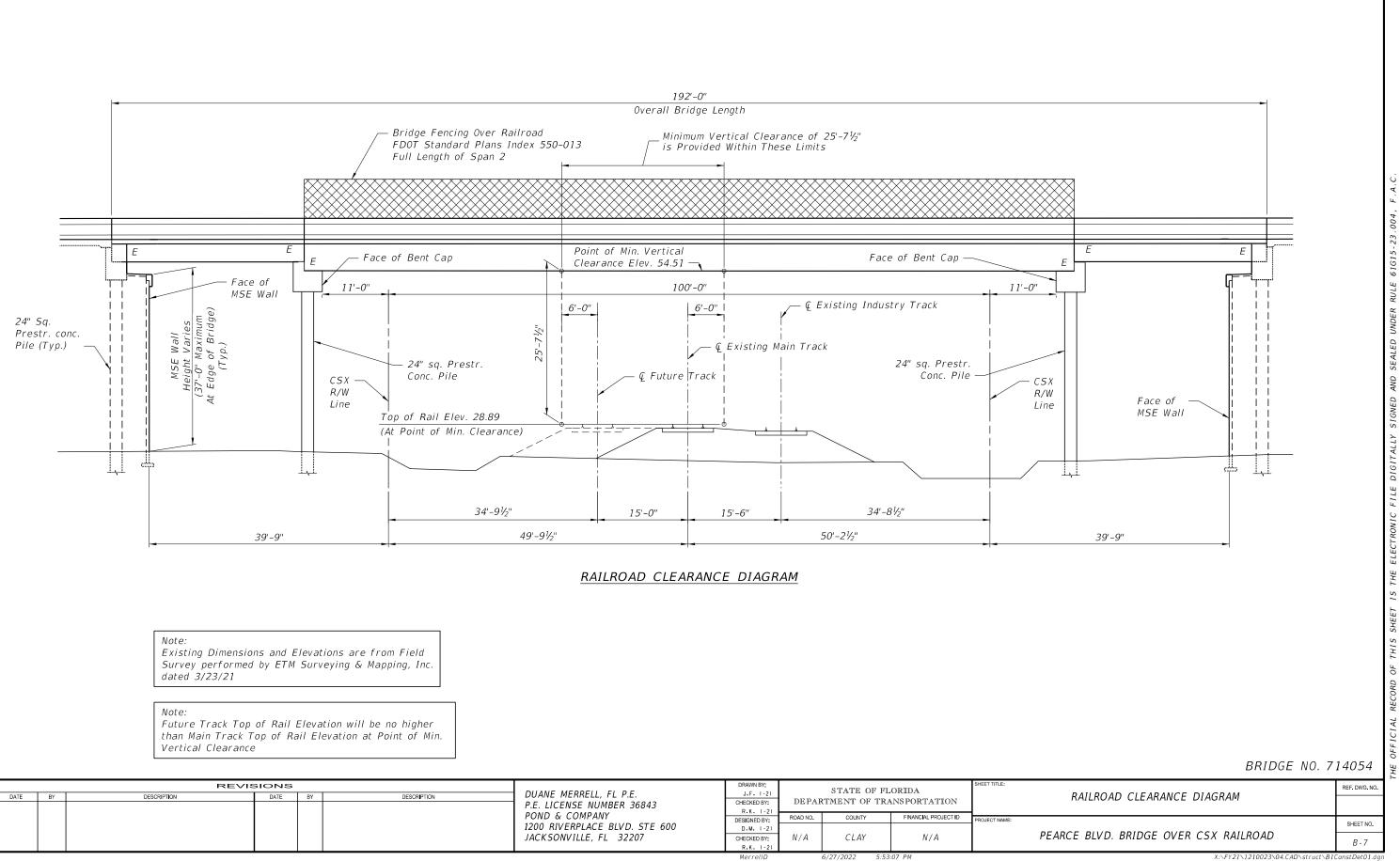
BRIDGE NO. 714054

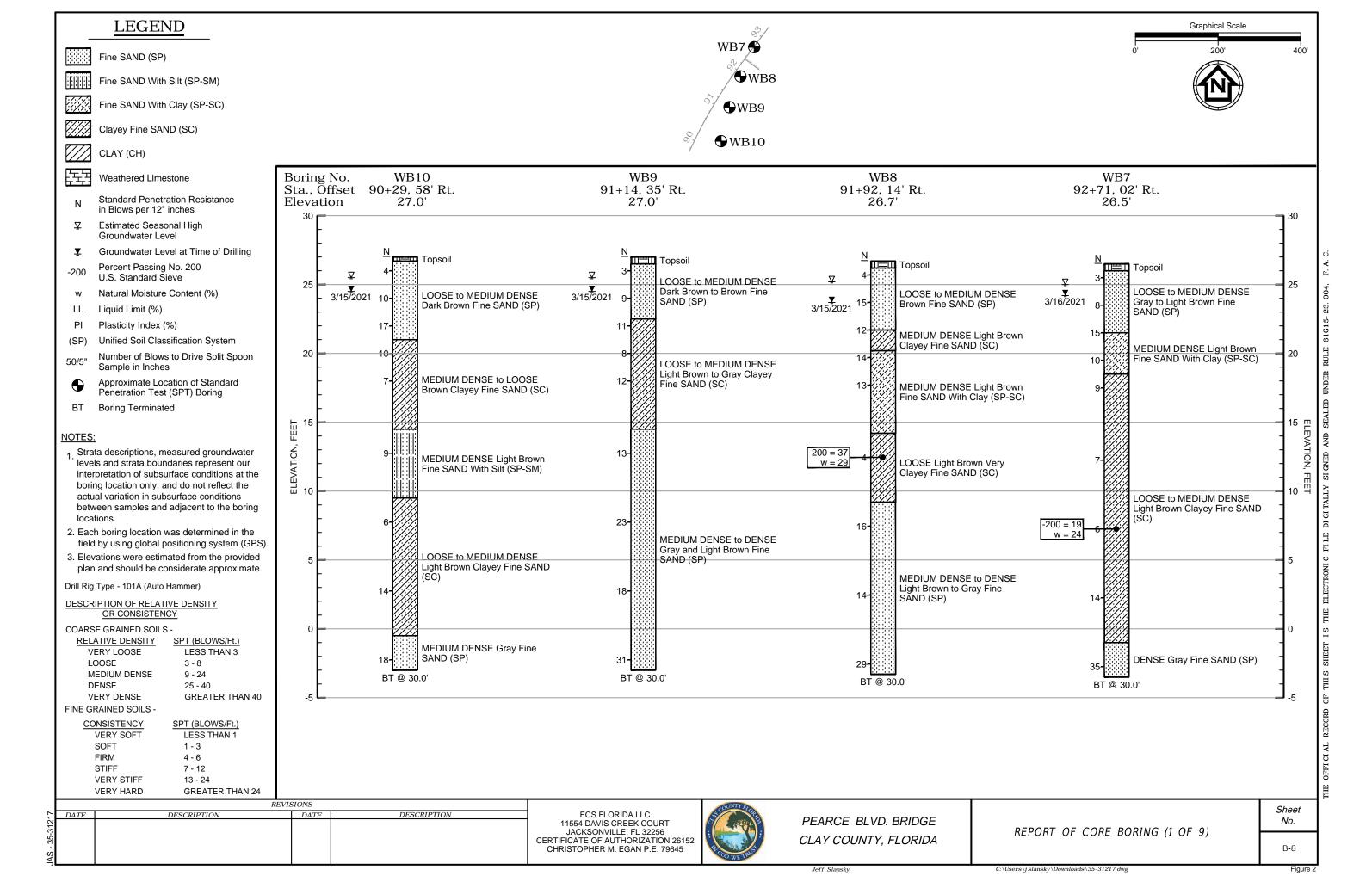


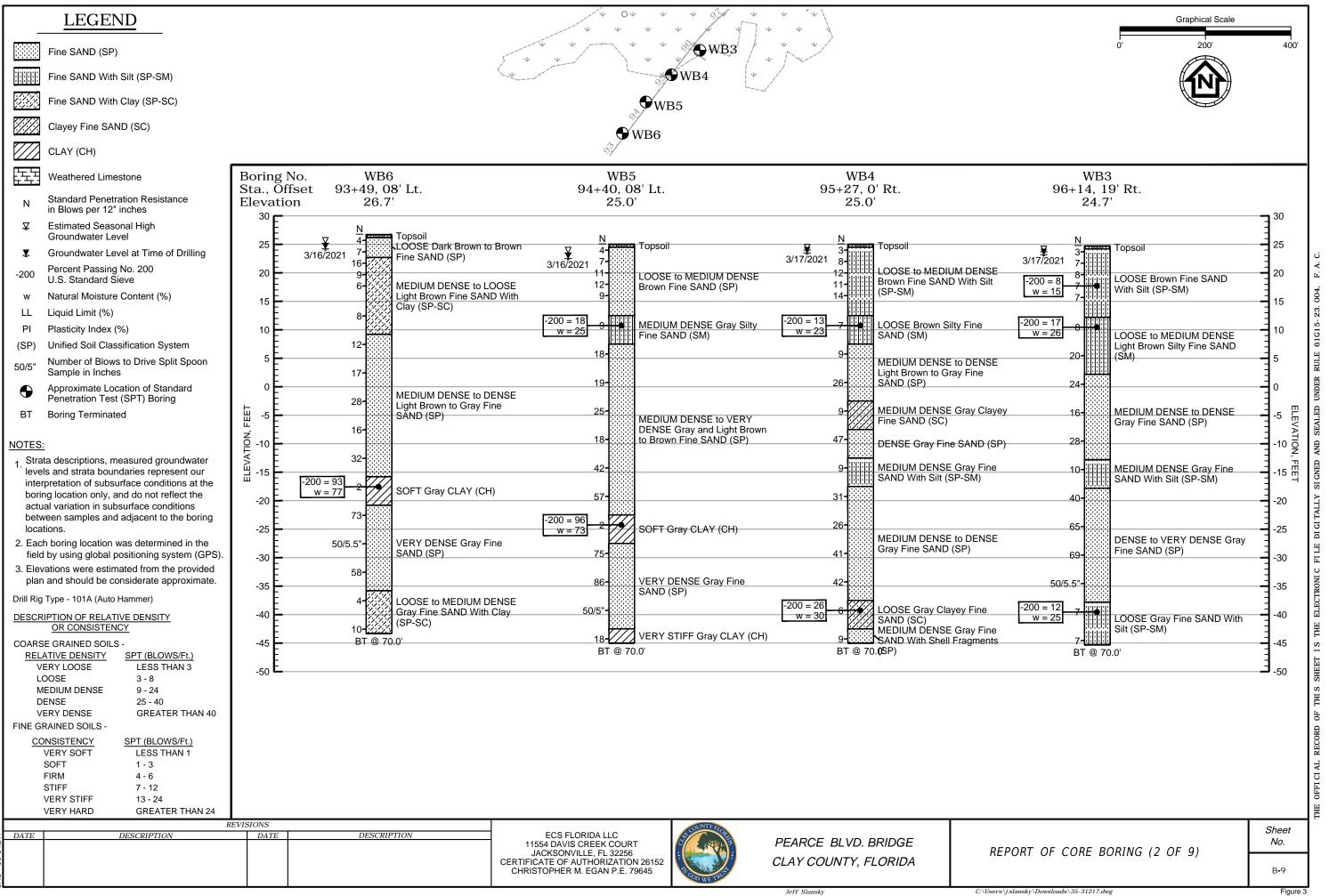
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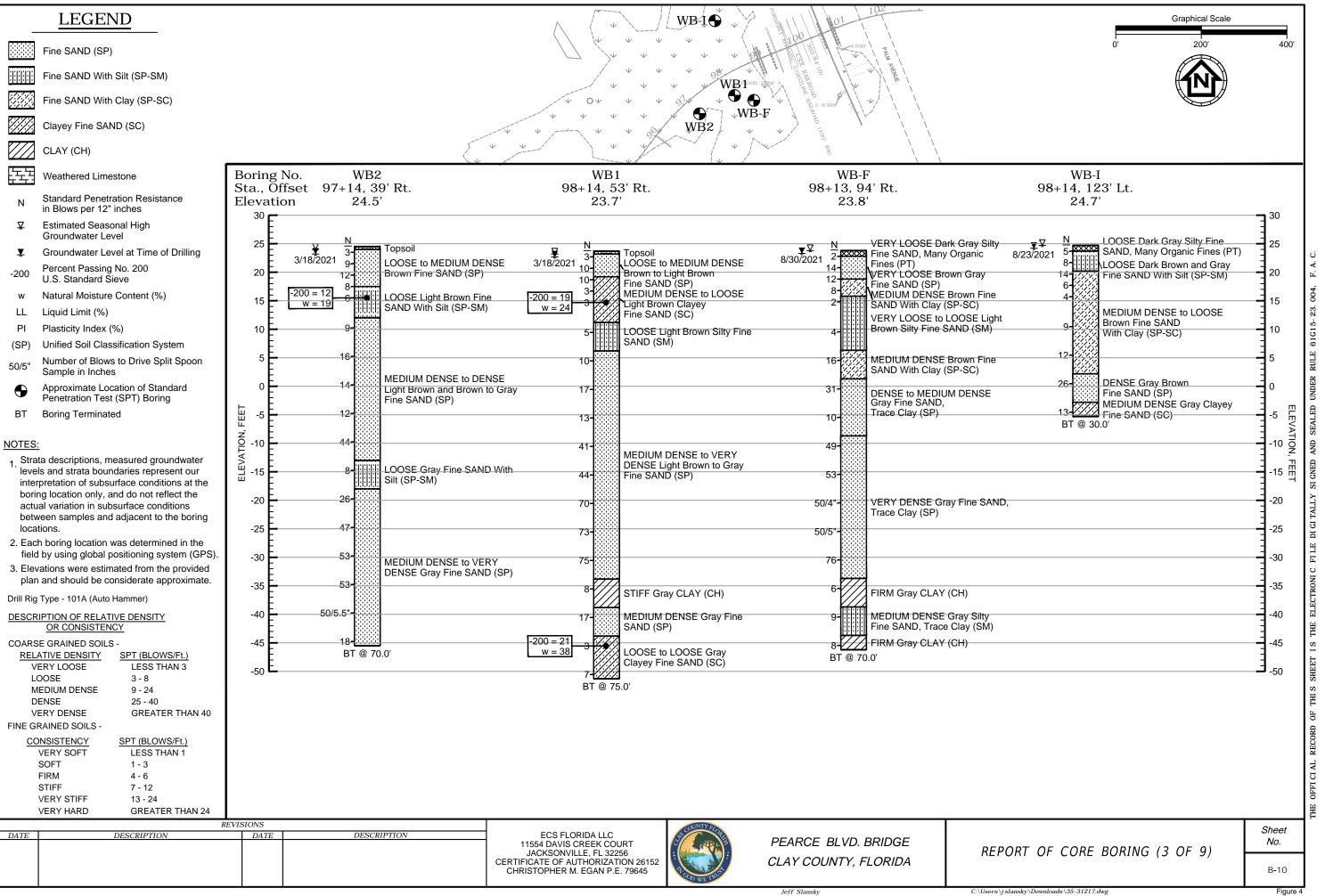


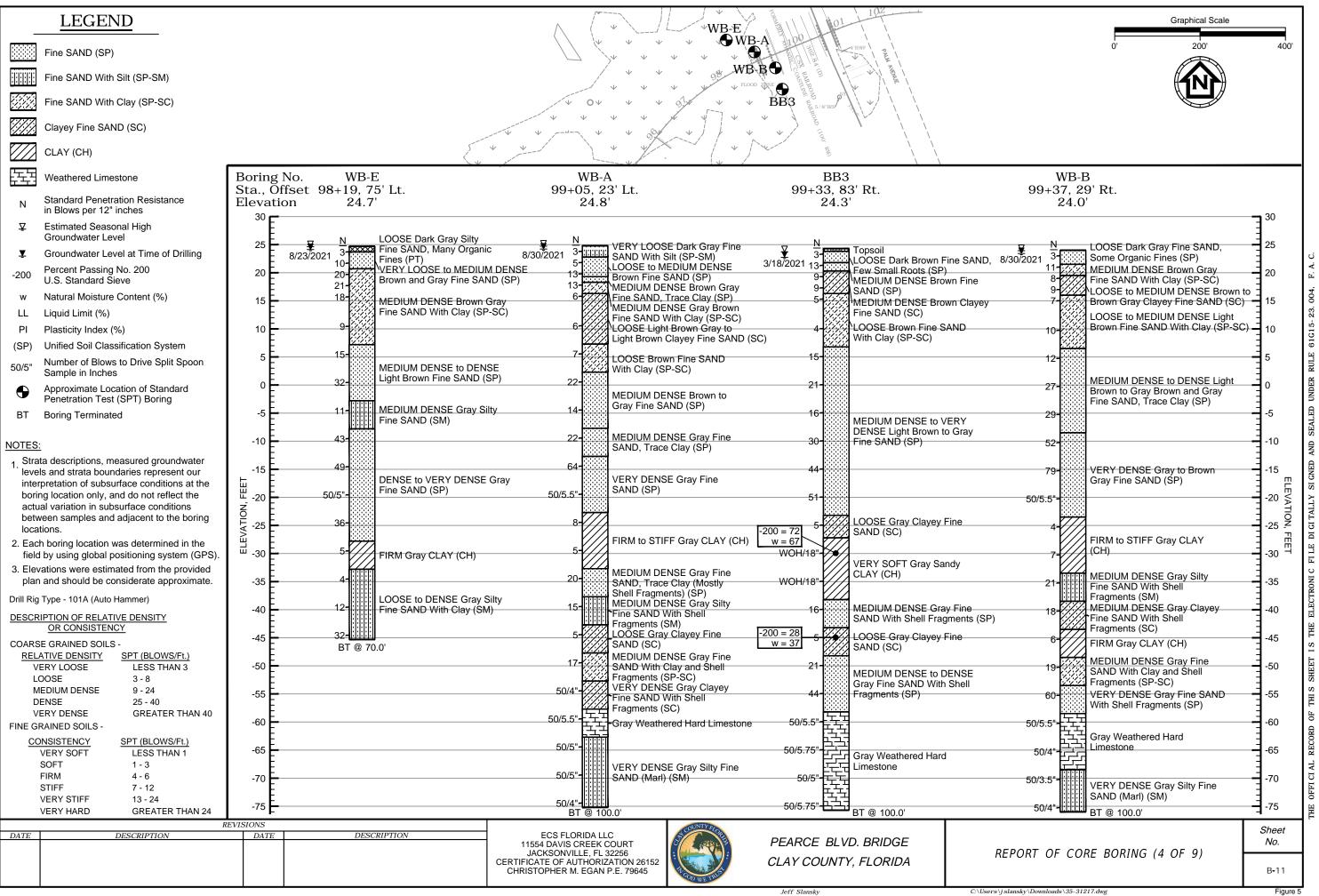
\$FILE

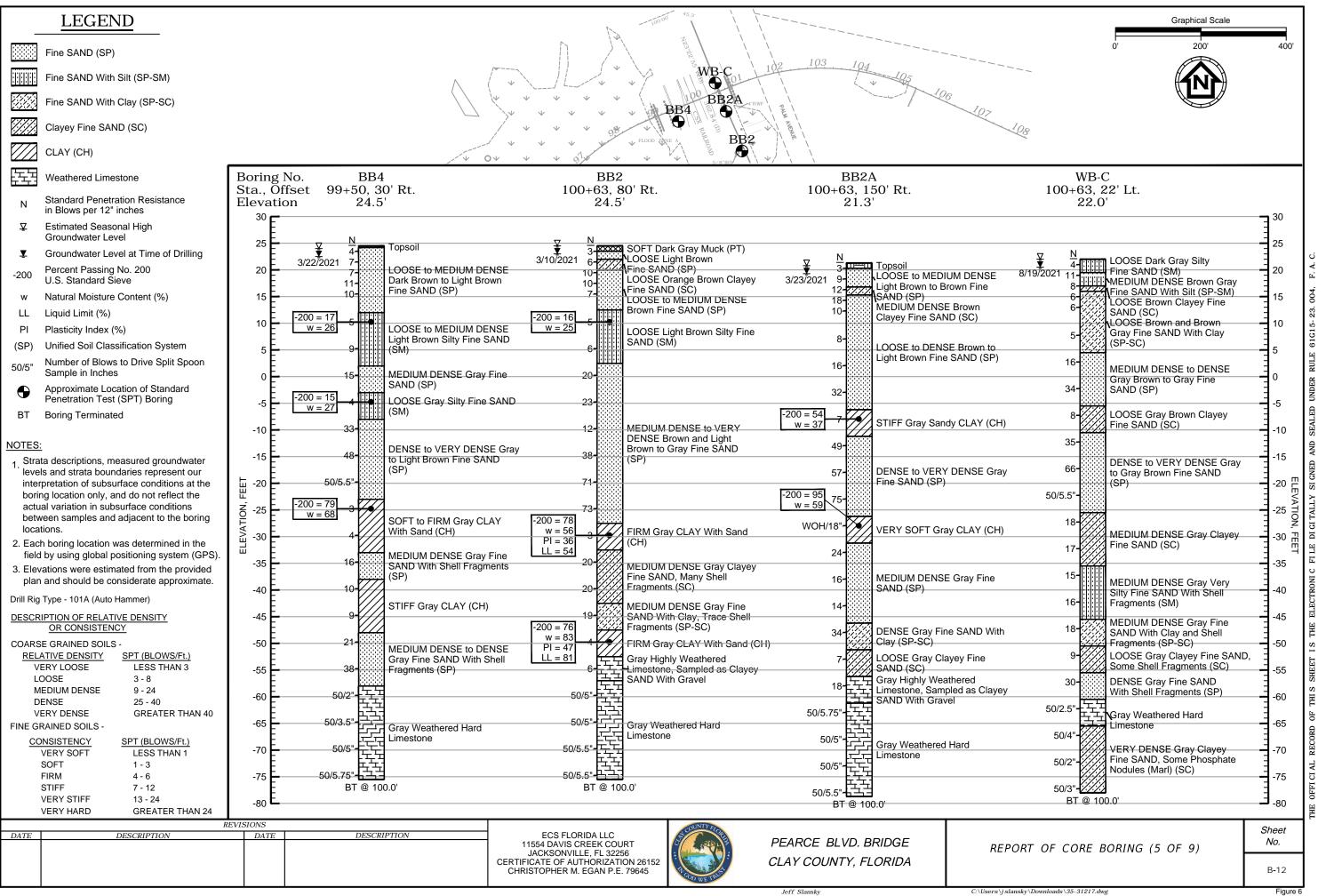


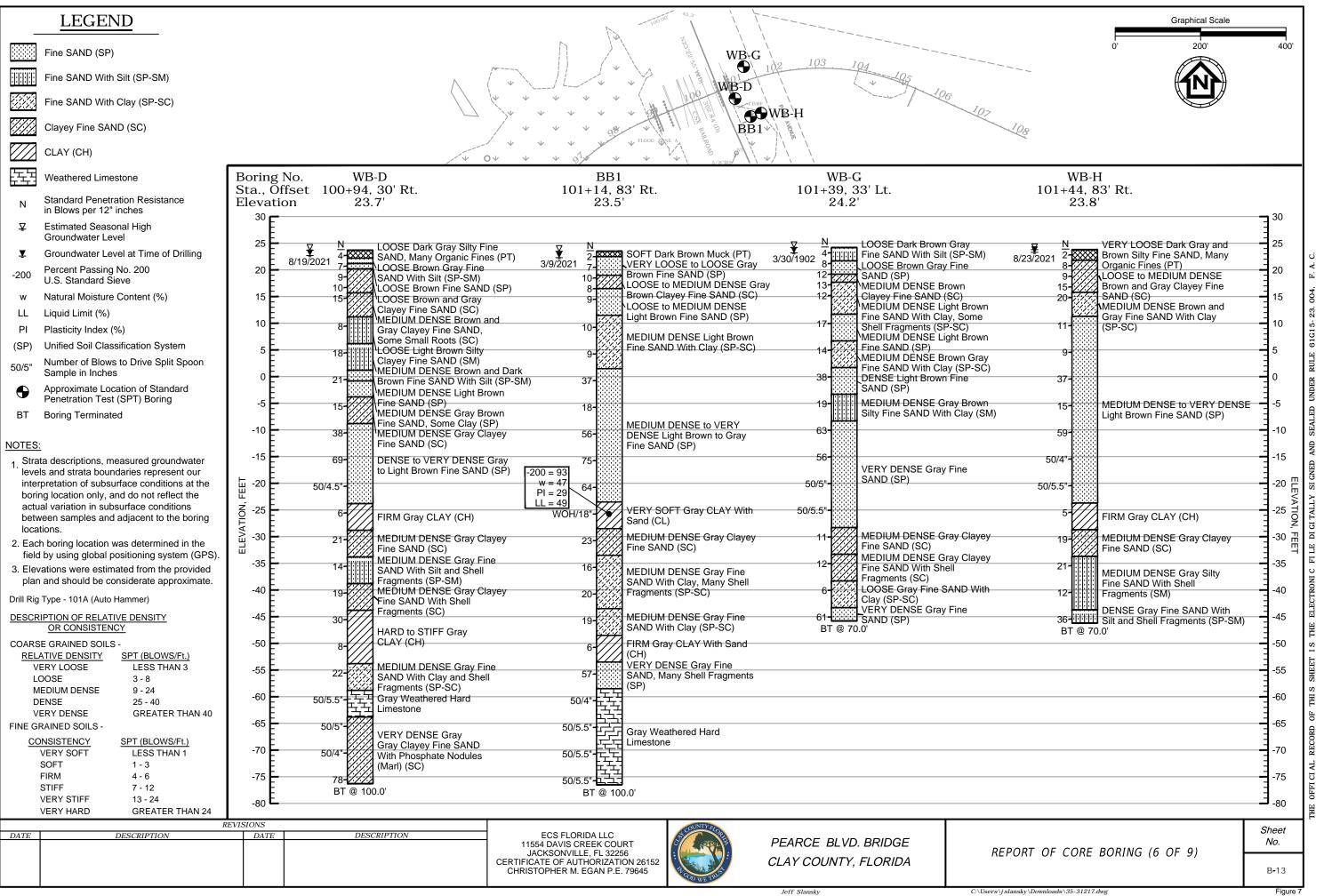




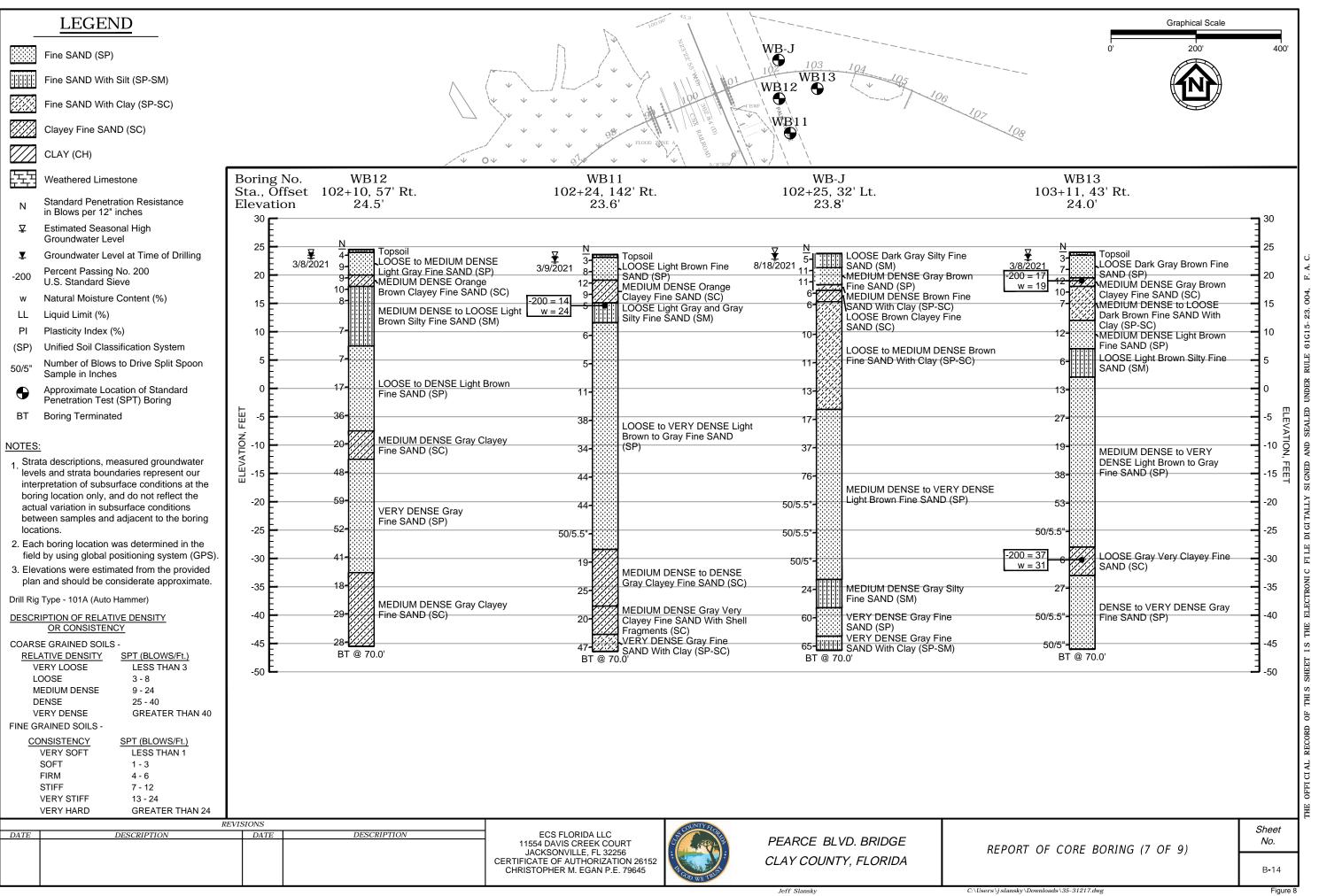


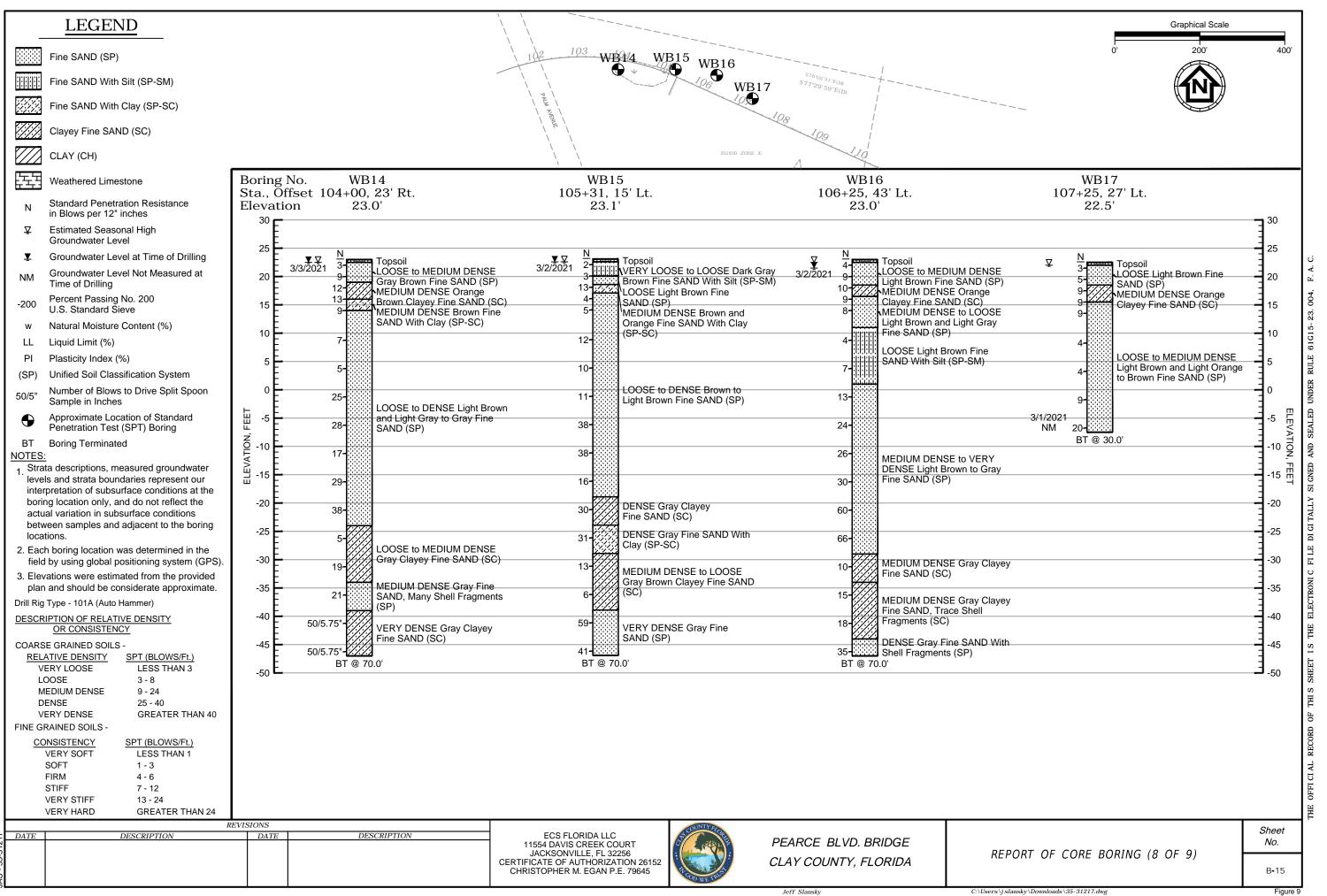


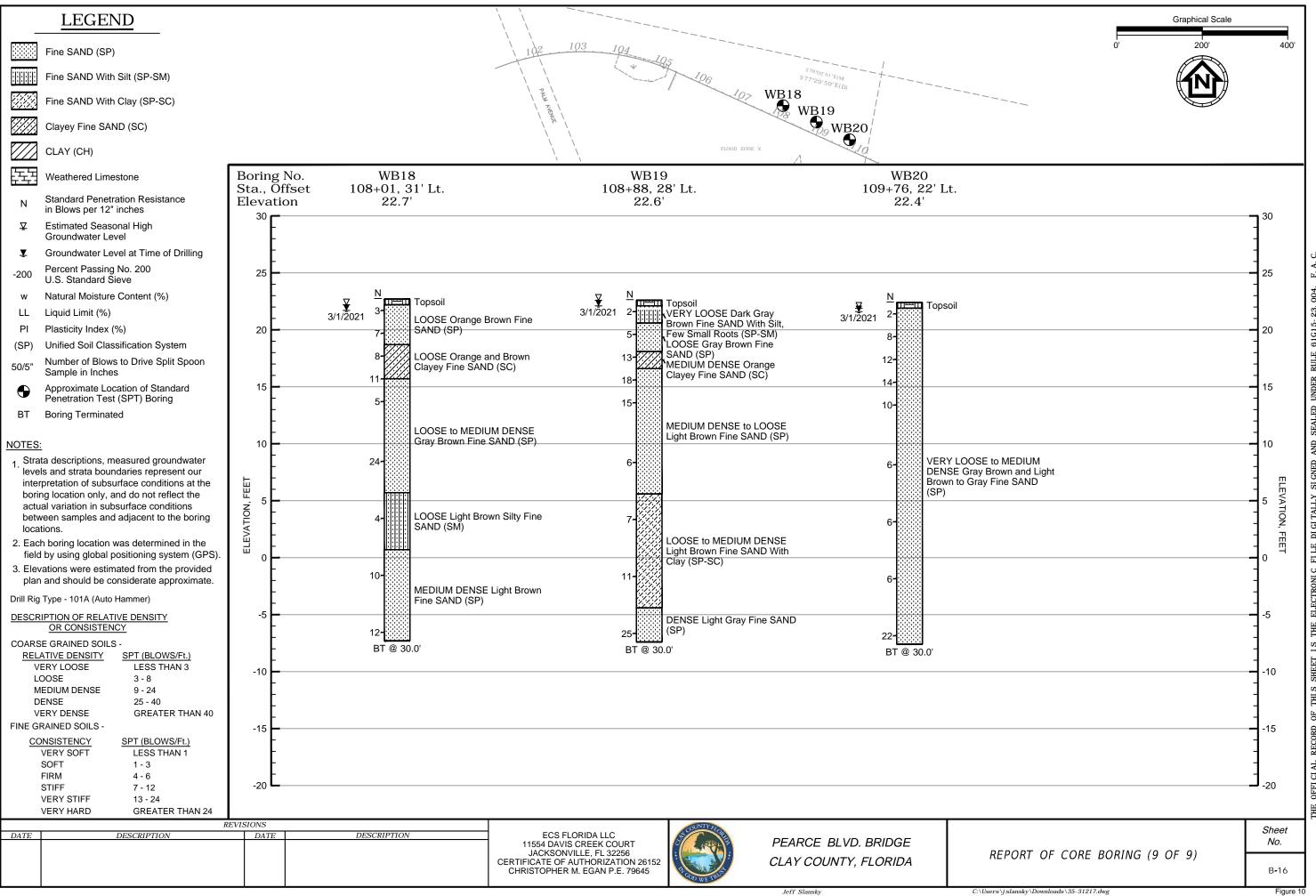




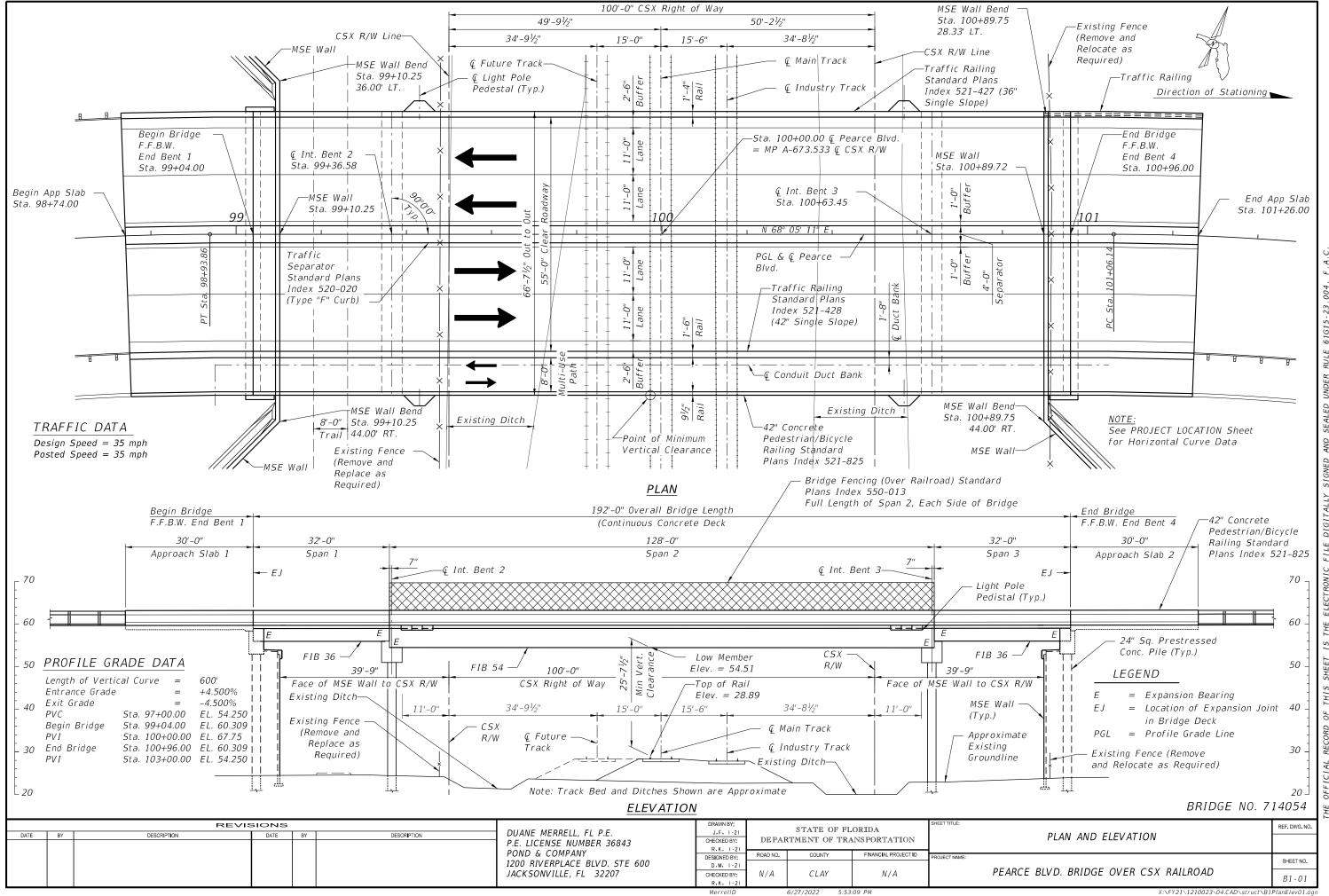
C:\Users\islanskv\Downloads\35-31217.dwg

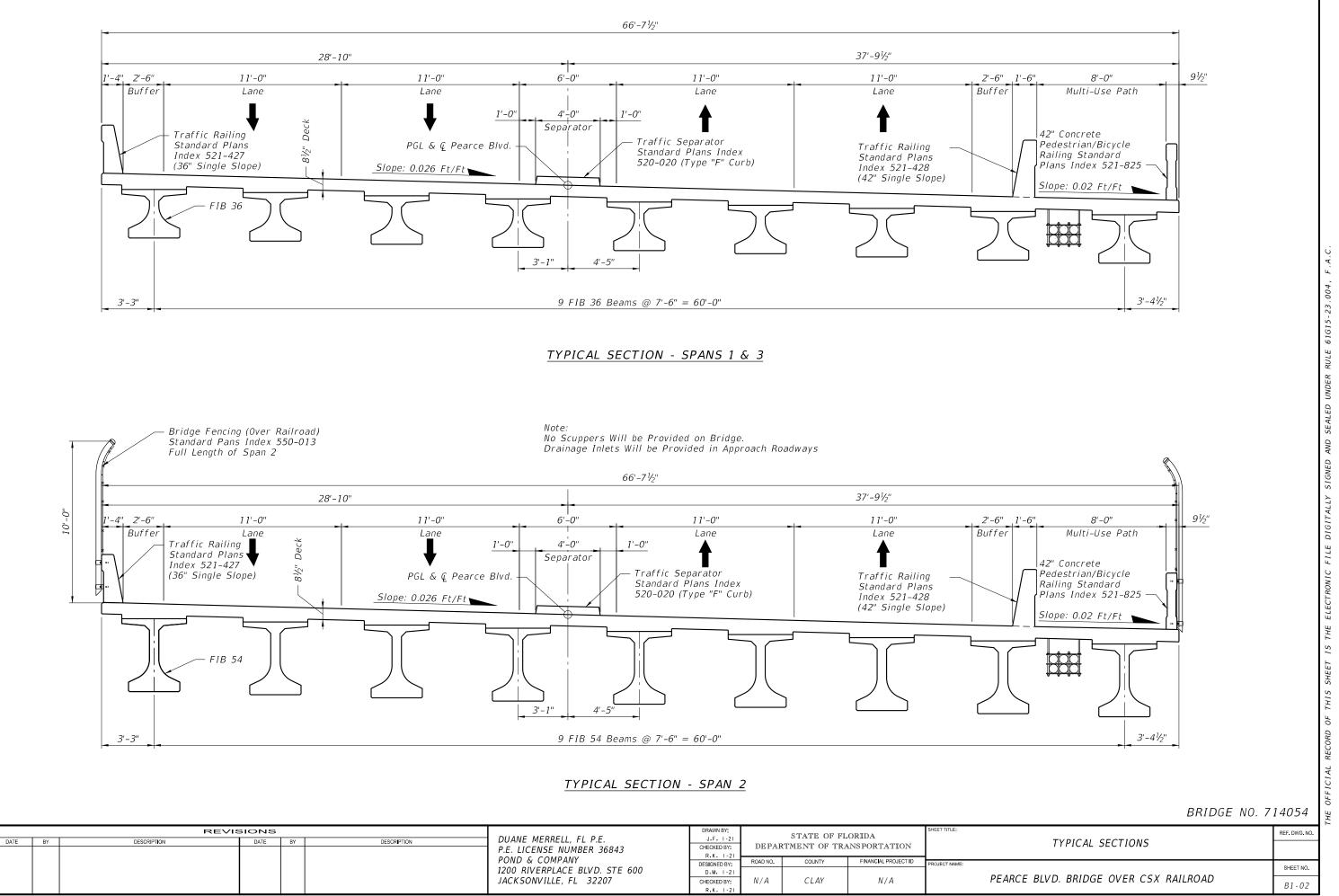


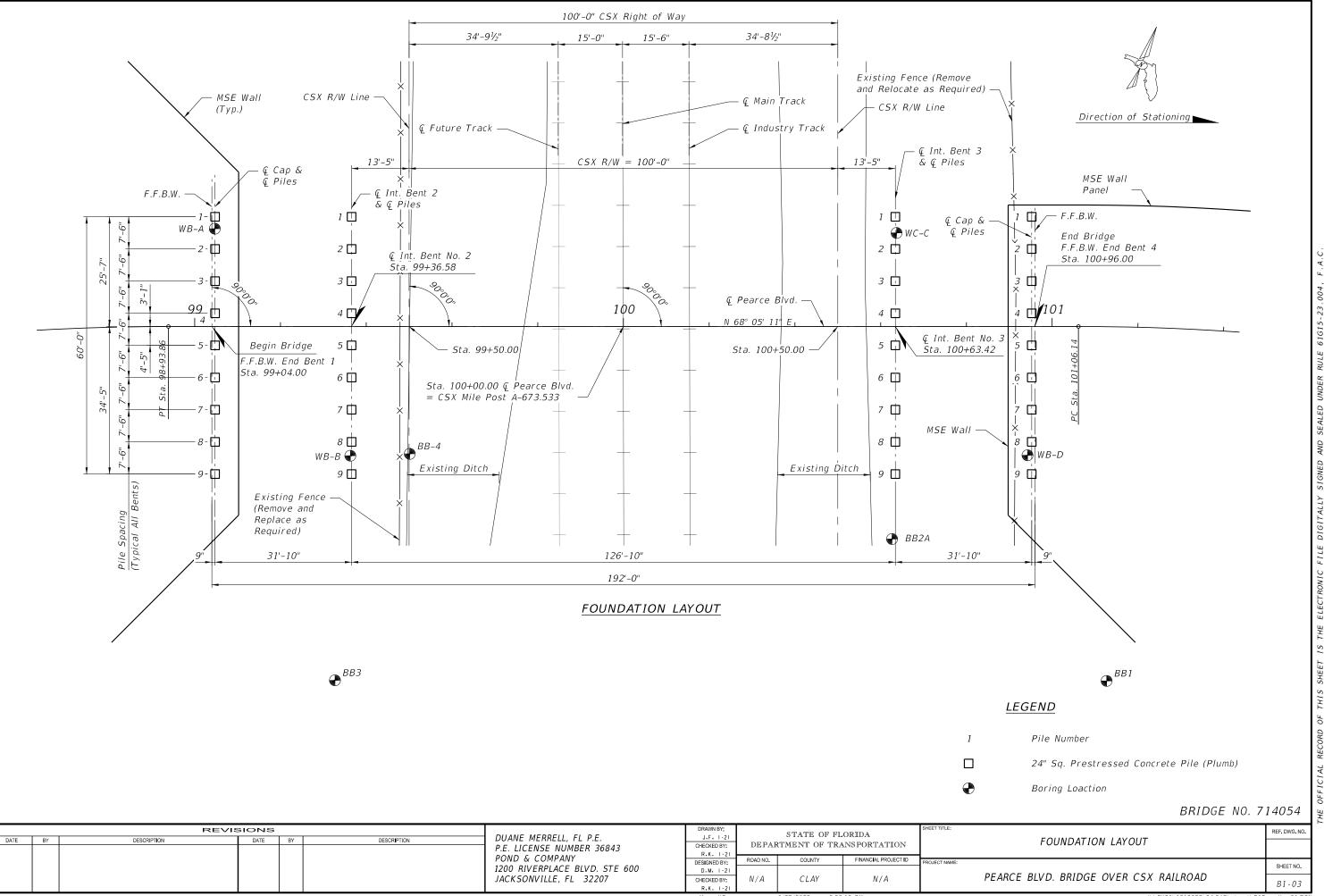




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							P	ILE DATA	TABL	E												Table Dat	e 01/01/
	I	NSTALLATI	ON CRITE	RIA					Ľ	DESIGN CR	ITERIA						PILI	E CUT-	OFF EL	EVATI	ONS		
PILE SIZE (in.)	NOMINAL BEARING RESISTANCE (tons)	NOMINAL UPLIFT RESIST ANCE (tons)	MINIMUM TIP ELEVATION (ft.)	TEST PILE LENGTH (ft.)	REQUIRED JET ELEVATION (ft.)	REQUIRED PREFORM ELEVATION (ft.)	FACTORED DESIGN LOAD (tons)		DOWN	TOTAL SCOUR RESISTANCE (tons)	NET SCOUR RESISTANCE (tons)	100-YEAR SCOUR ELEVATION (ft.)	Ø COMPRESSION	Ø NPLIFT	PILE 1	PILE 2	PILE 3	PILE 4	PILE 5	PILE 6	PILE 7	PILE 8	PILE
24	182	N/A	N/A	114	N/A	N/A	118	N/A	N/A	N/A	N/A	N/A	0.65	5 N/A	54.4	54.2	54.0	53.8	53.6	53.4	53.3	53.1	52.9
24	290	N/A	- 59	118	N/A	N/A	188	N/A	N/A	N/A	N/A	N/A	0.65	5 N/A	53.3	53.1	52.9	52.7	52.5	52.3	52.1	51.9	51.7
24	290	N/A	- 59	118	N/A	N/A	188	N/A	N/A	N/A	N/A	N/A	0.65	5 N/A	53.3	53.1	52.9	52.7	52.5	52.3	52.1	51.9	51.7
24	182	N/A	N/A	114	N/A	N/A	118	N/A	N/A	N/A	N/A	N/A	0.65	5 N/A	54.4	54.2	54.0	53.8	53.6	53.4	53.2	53.0	52.8
	SIZE (in.) 24 24 24	PILE SIZE (in.)NOMINAL BEARING RESISTANCE (tons)241822429024290	PILE SIZE (in.)NOMINAL BEARING RESISTANCE (tons)NOMINAL UPLIFT RESISTANCE (tons)24182N/A24290N/A24290N/A	PILE SIZE (in.)NOMINAL BEARING RESISTANCE (tons)NOMINAL UPLIFT RESISTANCE (tons)MINIMUM TIP ELEVATION (tf.)24182N/AN/A24290N/A-5924290N/A-59	PILE SIZE (in.)BEARING RESISTANCE (tons)UPLIFT RESISTANCE (tons)TIP ELEVATION (ft.)PILE LENGTH (ft.)24182N/AN/A11424290N/A-5911824290N/A-59118	PILE SIZE (in.)NOMINAL BEARING RESISTANCE (tons)NOMINAL UPLIFT 	PILE SIZZ (in.)NOMINAL BEARING ESISTANCE (tons)NOMINAL UPLIFT RESISTANCE (tons)MINIMUM TIP ELEVATION (ft.)TEST PILE LENGTH (ft.)REQUIRED JET ELEVATION (ft.)REQUIRED PREFORM ELEVATION (ft.)REQUIRED PREFORM ELEVATION (ft.)24182N/AN/A114N/AN/A24290N/A-59118N/AN/A24290N/A-59118N/AN/A	NOMINAL BEARING SIZZ (in)NOMINAL UPLIFT ESISTANCEMINIMUM TIP ELEVATION (ft.)TEST PILE LENGTH (ft.)REQUIRED JET ELEVATION (ft.)REQUIRED DESIGN LOAD (ft.)REQUIRED DESIGN LOAD (ft.)REQUIRED DESIGN LOAD (ft.)REQUIRED DESIGN LOAD (ft.)REQUIRED DESIGN LOAD (ft.)REQUIRED DESIGN LOAD (ft.)REQUIRED DESIGN LOAD (ft.)REQUIRED DESIGN LOAD (ft.)REQUIRED DESIGN LOAD (ft.)REQUIRED DESIGN LOAD (ft.)REQUIRED DESIGN DESIGN LOAD (ft.)REQUIRED DESIGN DESIGN LOAD (ft.)REQUIRED DESIGN DESI	INSTALLATION CRITERIA PILE SIZE (in.) 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Factored Design Load + Net Scour Resistance + Down Drag

Ø

≤ Nominal Bearing Resistance

UPLIFT RESISTANCE - The ultimate side friction capacity that must be obtained below the 100 year scour elevation to resist pullout of the pile (Specify only when design requires uplift capacity). TOTAL SCOUR RESISTANCE - An estimate of the ultimate static side friction resistance provided by the scourable soil. NET SCOUR RESISTANCE - An estimate of the ultimate static side friction resistance provided by the soil from the

required preformed or jetting elevation to the scour elevation.

100-YEAR SCOUR ELEVATION - Estimated elevation of scour due to the 100 year storm event.

PILE INSTALLATION NOTES [Notes Date 7-01-13]:

Contractor to verify location of all utilities prior to any pile installation activities.

Minimum Tip Elevation is required for vertical stability.

When a required jetting elevation is shown, the jet shall be lowered to the elevation and continue to operate at this elevation until the pile driving is completed. If jetting or preforming elevations differ from those shown on the table, the Engineer shall be responsible for determination of the required driving resistance.

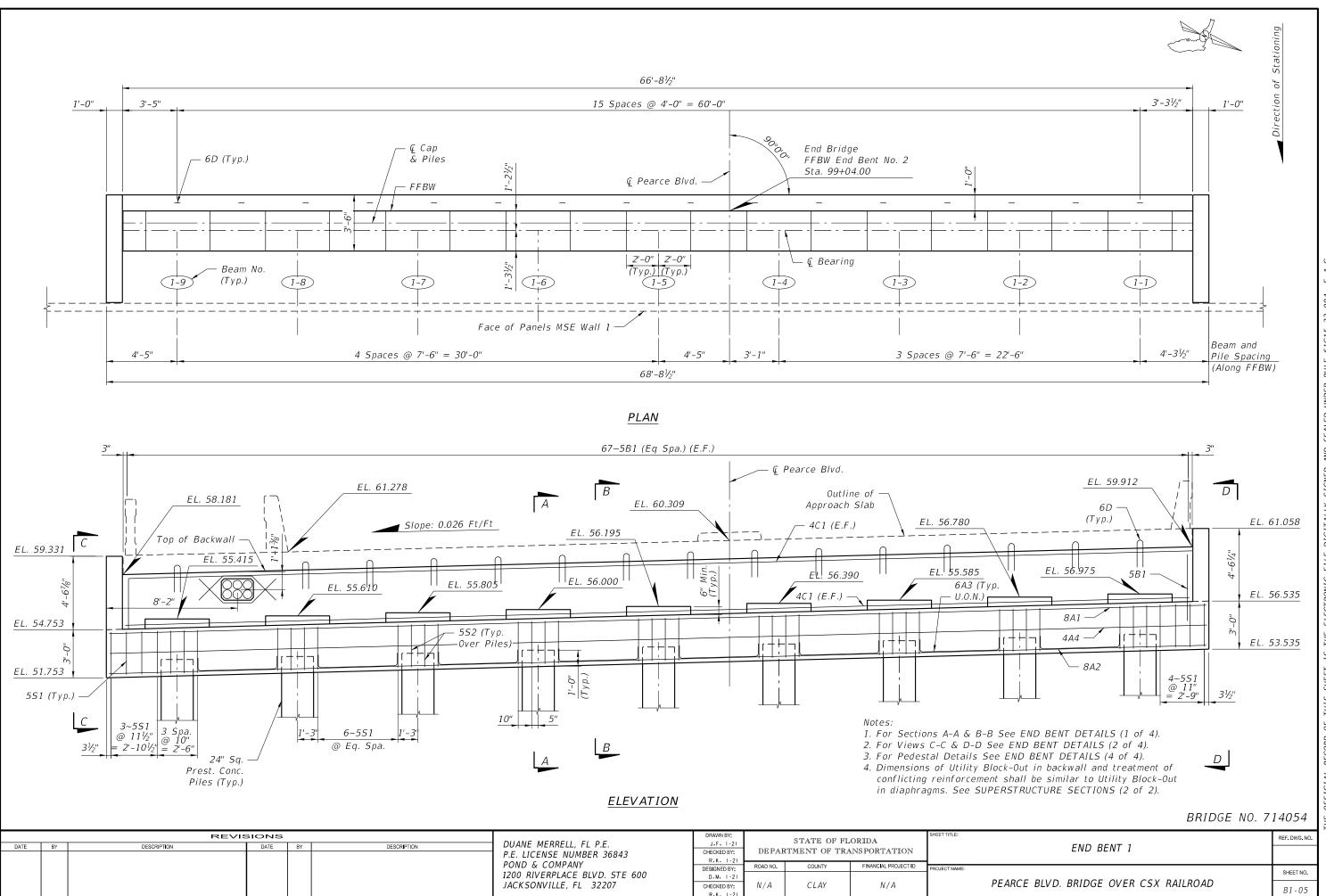
No jetting will be allowed without the approval of the Engineer.

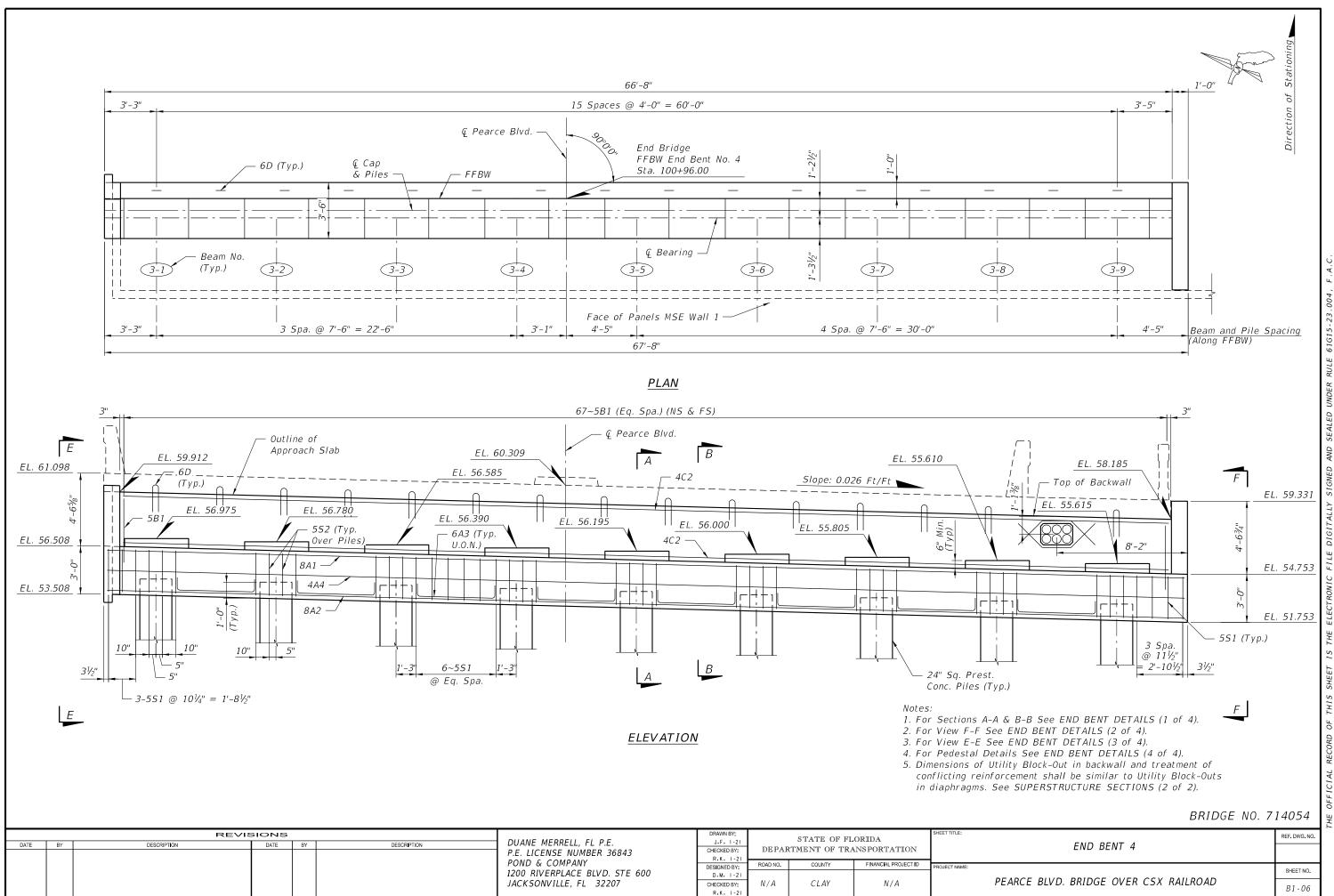
The Contractor should not anticipate being allowed to jet piles below the 100-year scour elevation or required jet elevation, whichever is deeper.

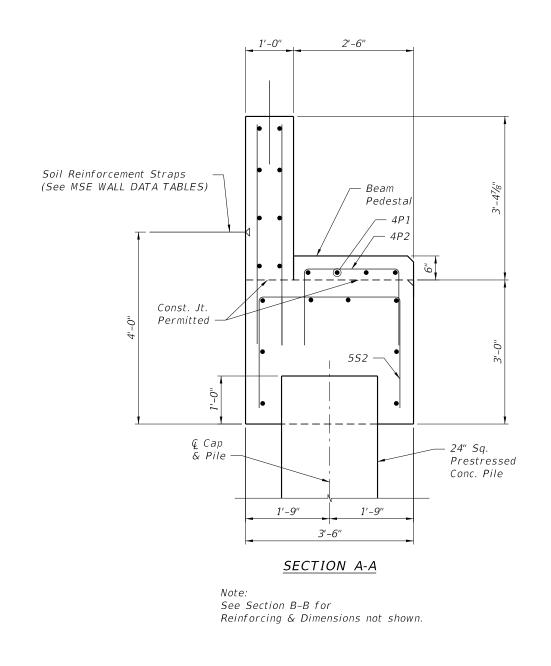
At each Bent, pile driving is to commence at the center of the Bent and proceed outward.

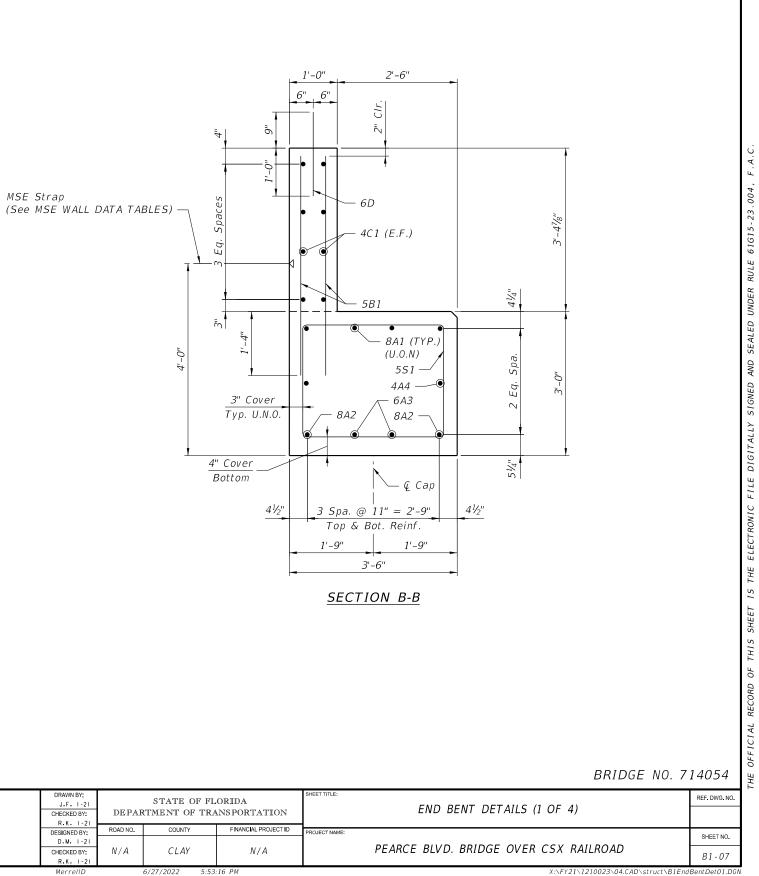
	REV	ISIONS				DRAWN BY		STATE OF FI	ORIDA	SHEET TITLE:	
BY	DESCRIPTION	DATE	BY	DESCRIPTION			DEPAF				
$ \wedge$	Undated Table, added note					R.K. I-21					
	opuliced runne, daded note					DESIGNED BY:	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:	
					JACKSONVILLE, FL 32207	CHECKED BY:	N/A	CLAY	N/A		PEARCE E
								6/27/2022 5:53			
	BY <u> </u> <u> </u>		BY DESCRIPTION DATE		BY DESCRIPTION DATE BY DESCRIPTION	BY DESCRIPTION DATE BY DESCRIPTION 1 Updated Table, added note BY DESCRIPTION DUANE MERRELL, FL P.E. 1 Updated Table, added note DUANE MERRELL POND & COMPANY 1200 RIVERPLACE BLVD. STE 600	BY DESCRIPTION DATE BY DESCRIPTION Image: A state of the state o	BY DESCRIPTION DATE BY DESCRIPTION Image: Discription in the image: Discripti	BY DESCRIPTION DATE BY DESCRIPTION Image: Discription in the image: Discripti	BY DESCRIPTION DATE BY DESCRIPTION Image: Discription in the state of the sta	BY DESCRIPTION DATE BY DESCRIPTION Image: Discription discription Date BY DESCRIPTION Date BY DESCRIPTION DIANE MERRELL, FL P.E. J.F. 1-21 STATE OF FLORIDA Image: Discription discription Date BY DESCRIPTION DESCRIPTION R.K. 1-21 CHECKED BY: DEPARTMENT OF TRANSPORTATION Image: Discription discription Image: Discription discription Image: Discription discription R.K. 1-21 DESCRIPTION DESCRIPTION Image: Discription discription discription Image: Discription discription Image: Discription discription R.K. 1-21 Image: Discription discription PROJECT NAME: Image: Discription discription Image: Discription discription Image: Discription discription Image: Discription discription PROJECT NAME: PROJECT NAME: Image: Discription discription Image: Discription discription Image: Discription discription Image: Discription discription PROJECT NAME: PROJECT NAME: Image: Discription discription discription Image: Discription discription Image: Discription discription Image: Discription discription Image: Discription discription Image: Discription Image: Discription discrinternation discription Imag

BRIDGE NO. 714054 REF. DWG. NO. PILE DATA TABLE SHEET NO. BLVD. BRIDGE OVER CSX RAILROAD B1-04

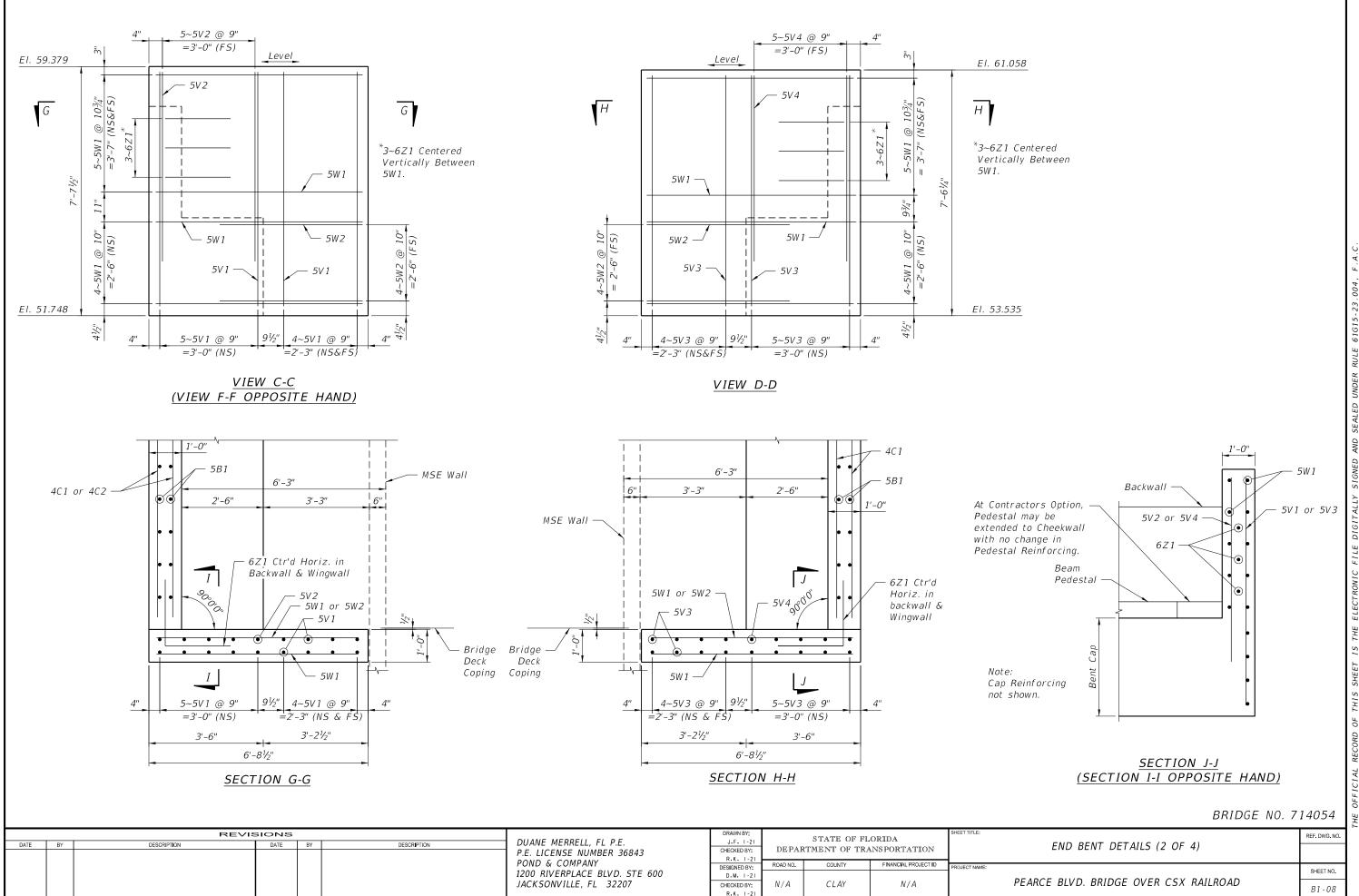


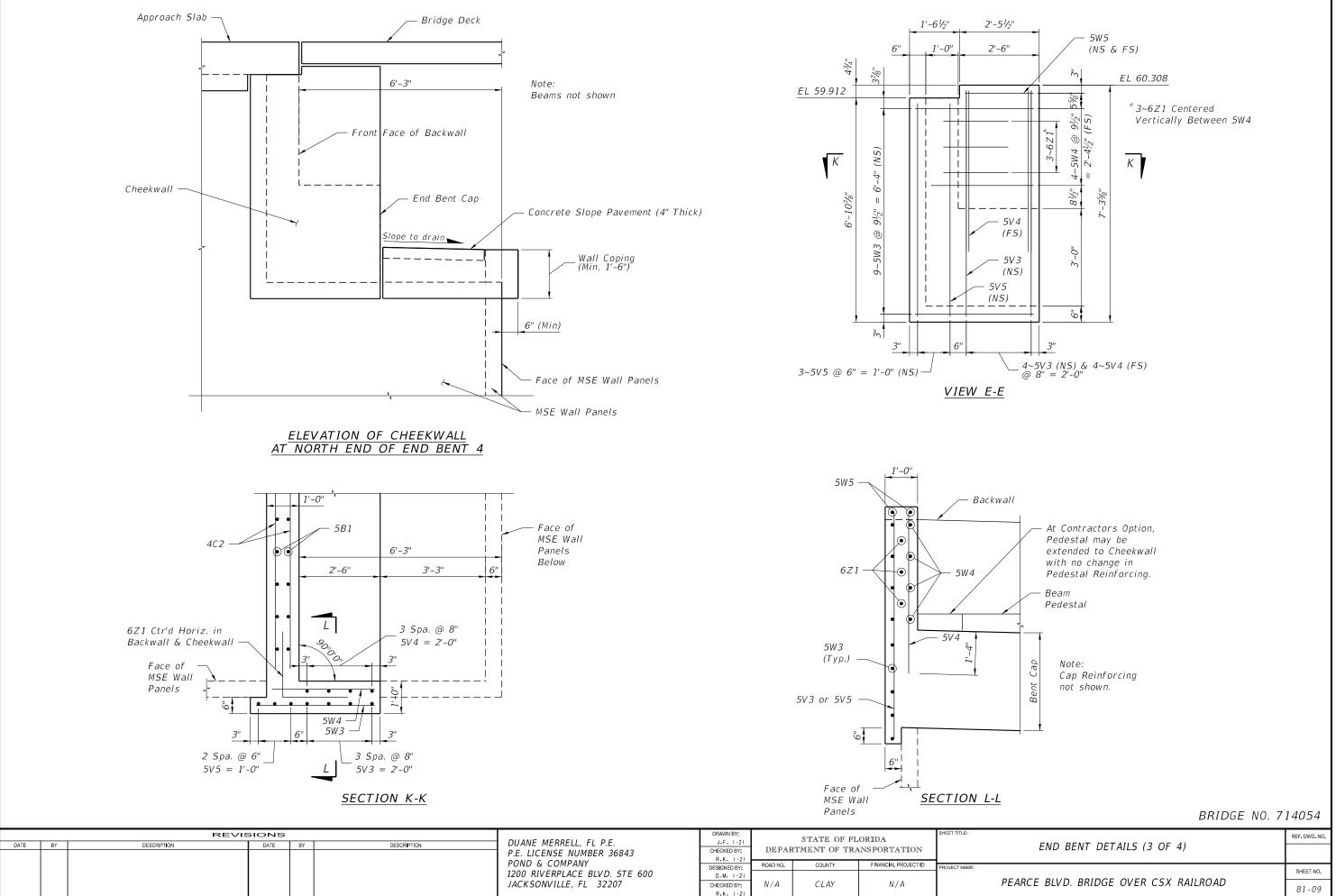


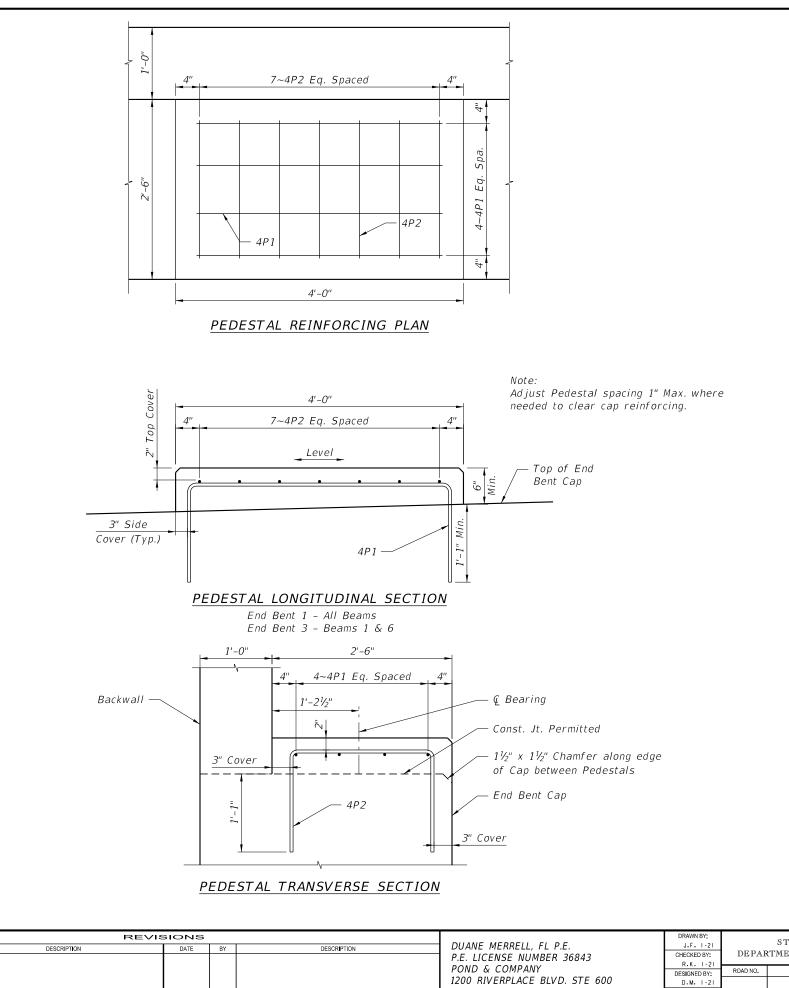




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						P.E. LICENSE NUMBER 36843	CHECKED BY: R.K. I-21	DEPAR	CIMENI OF IK	ANSPORTATION		-
						POND & COMPANY	DESIGNED BY:	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:	
						1200 RIVERPLACE BLVD. STE 600	D.M. 1-21				1	
						JACKSONVILLE, FL 32207	CHECKED BY:	N/A	CLAY	N/A		PEARCE
							R.K. I-21					
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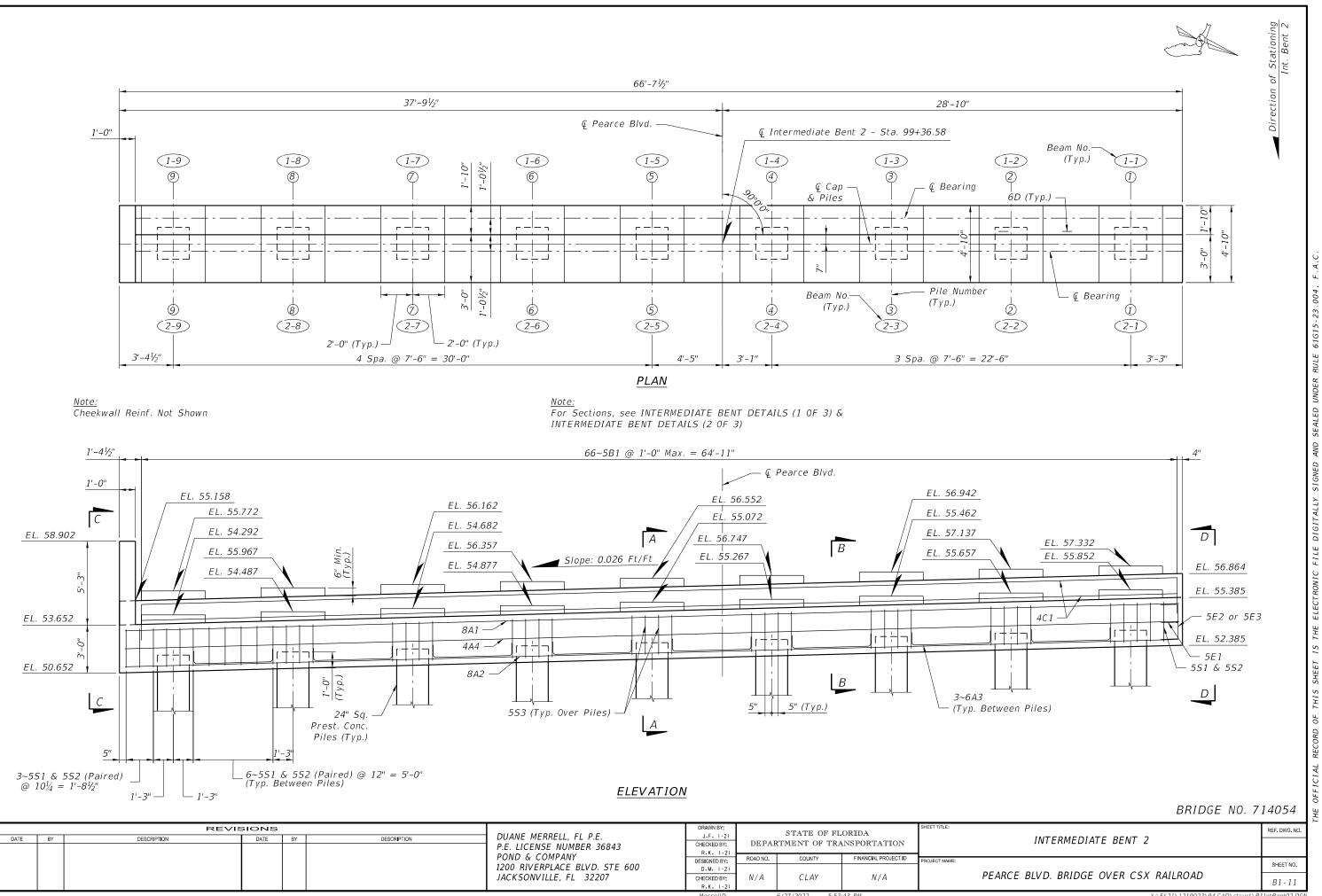
JACKSONVILLE, FL 32207

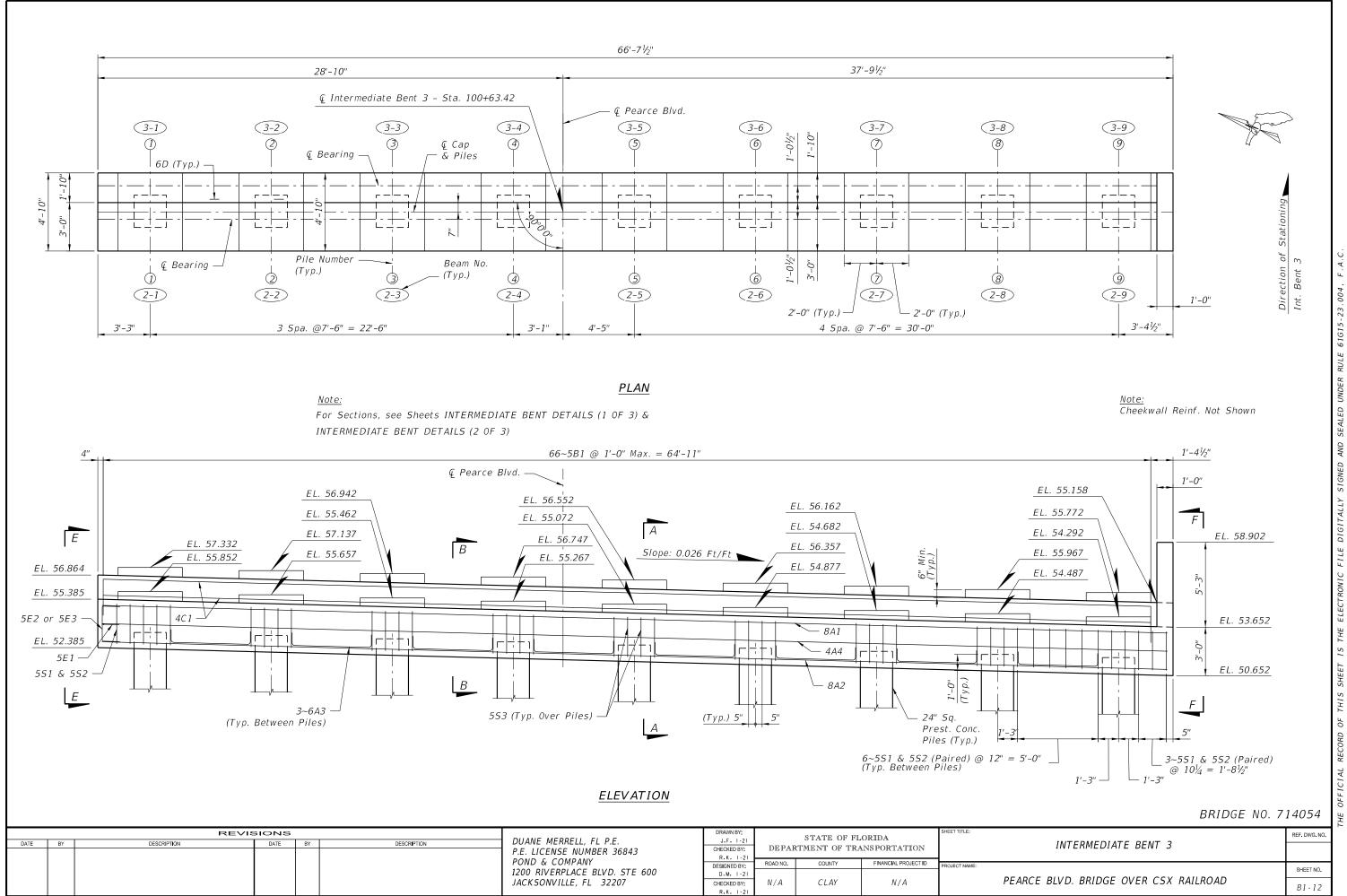
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D.M. 1-21				1	
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R.K. I-21					
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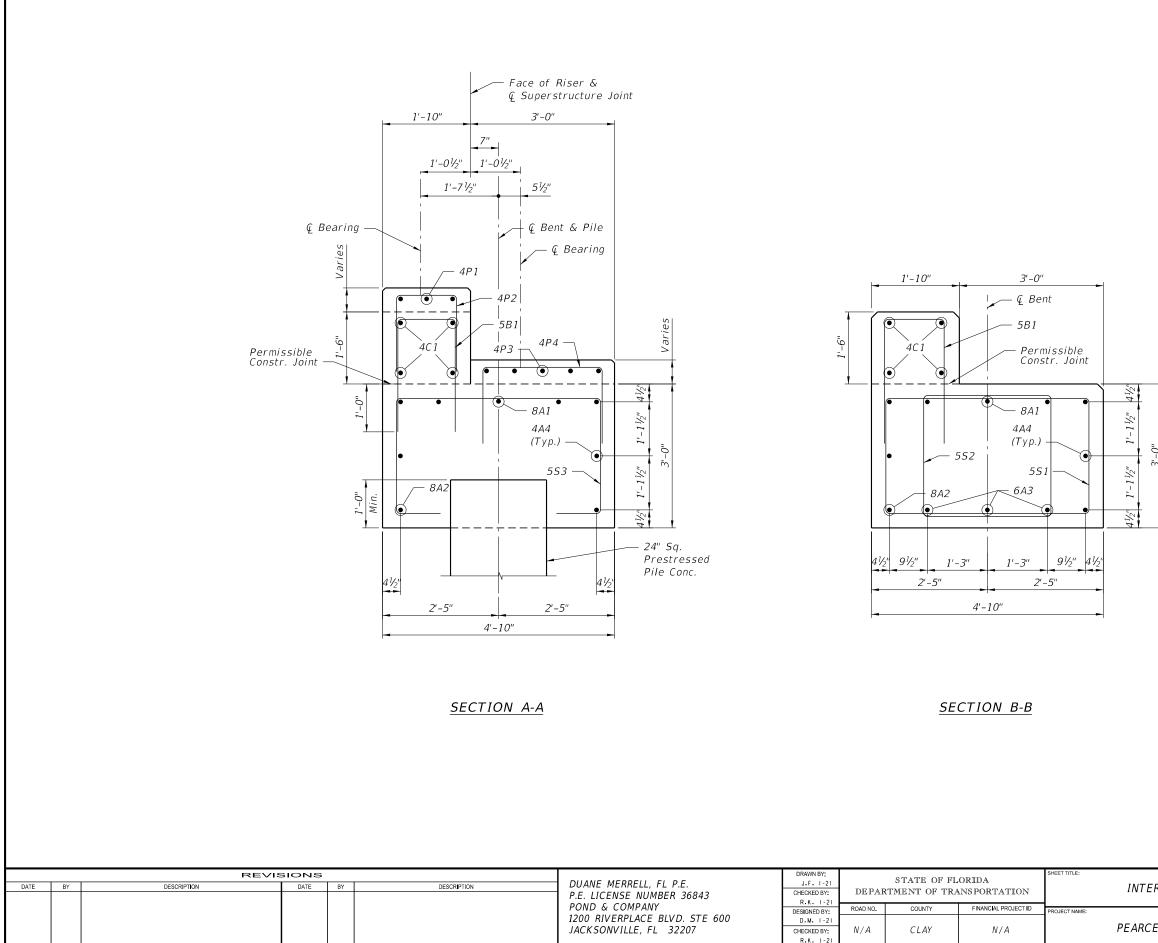
BRIDGE NO. 714054 REF. DWG. NO. END BENT DETAILS (4 OF 4) SHEET NO. BLVD. BRIDGE OVER CSX RAILROAD B1-10





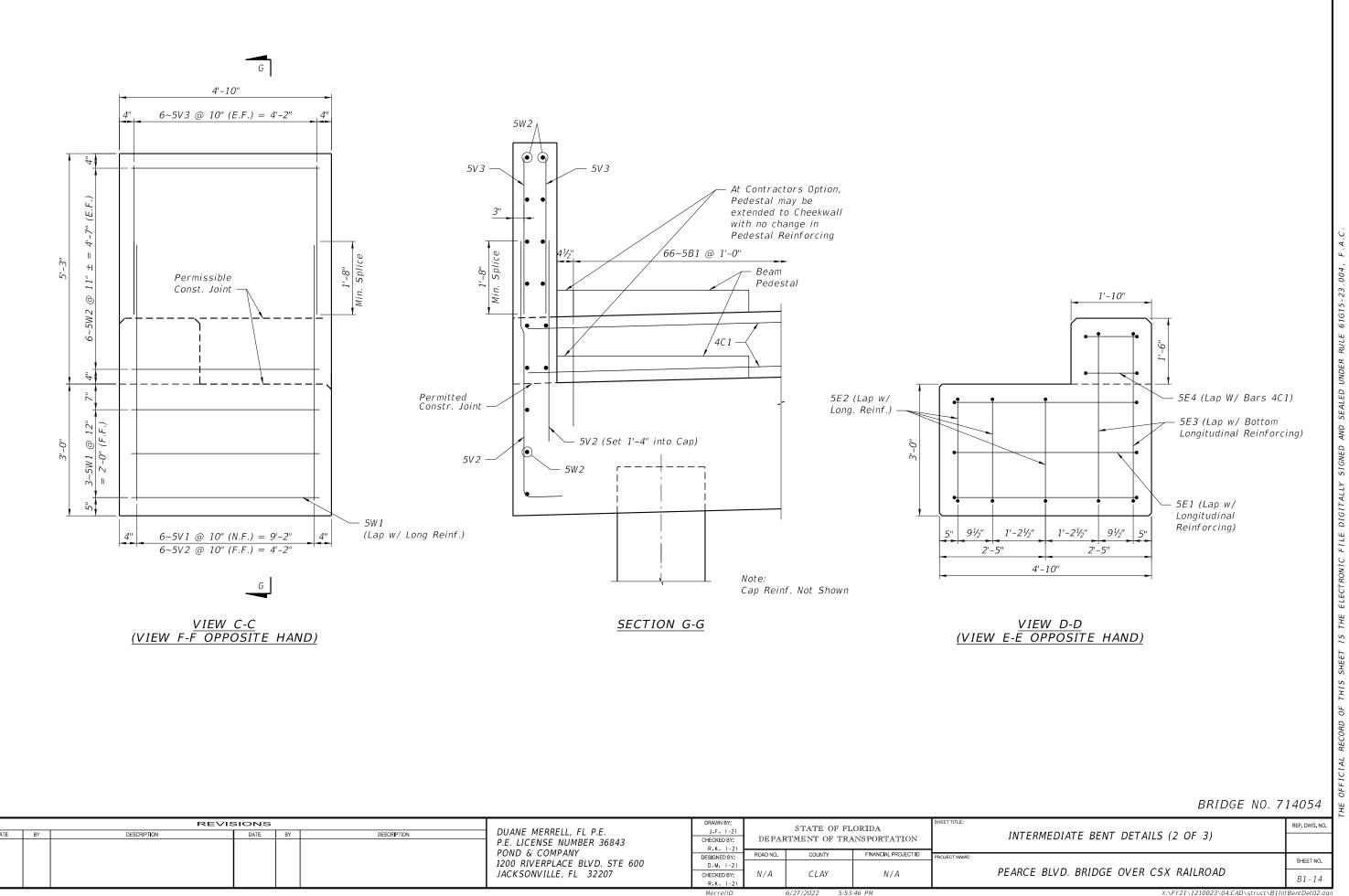
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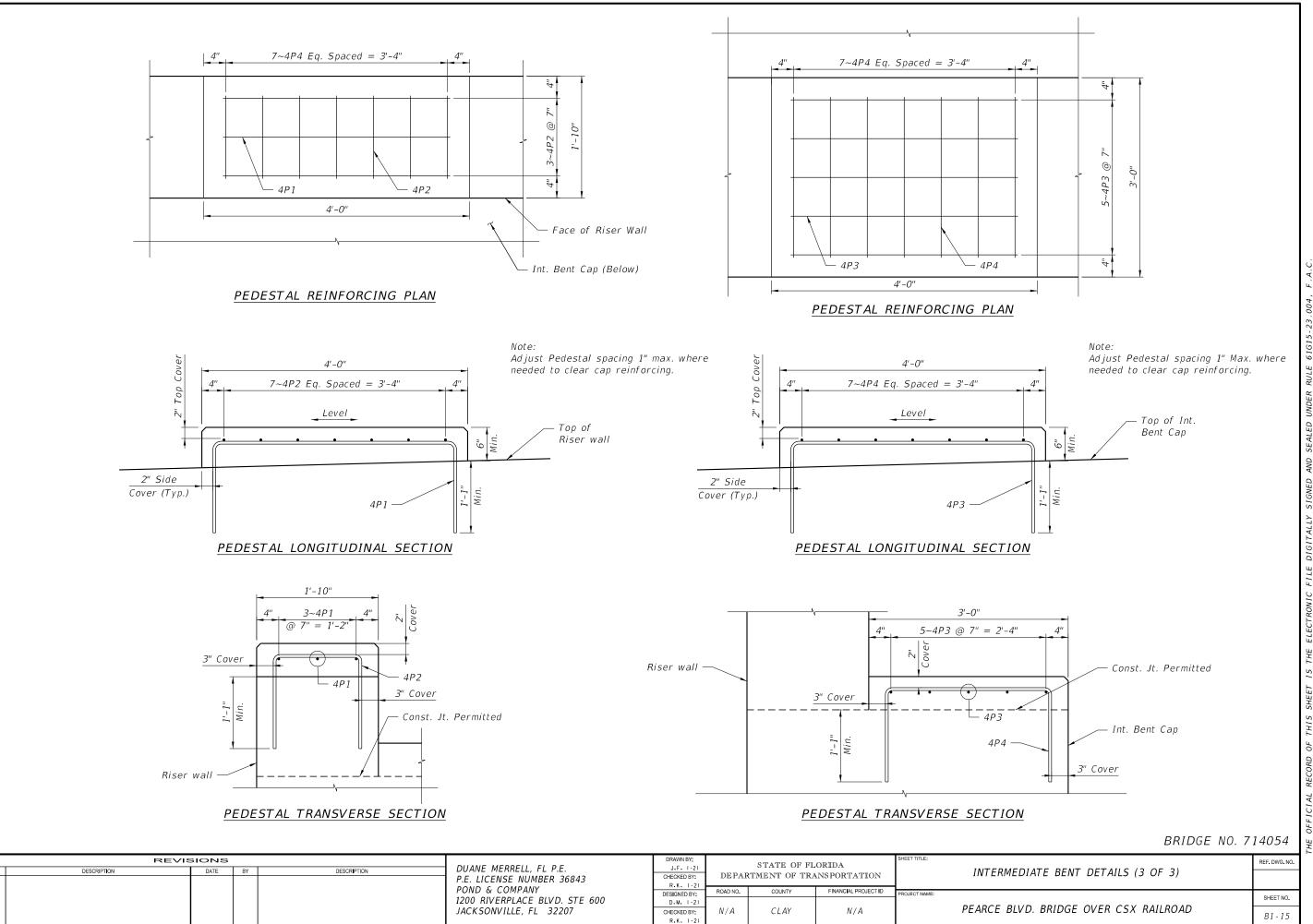


BRIDGE NO. 7	14054	
	REF. DWG. NO.	
RMEDIATE BENT DETAILS (1 OF 3)		
	SHEET NO.	
E BLVD. BRIDGE OVER CSX RAILROAD	B1-13	

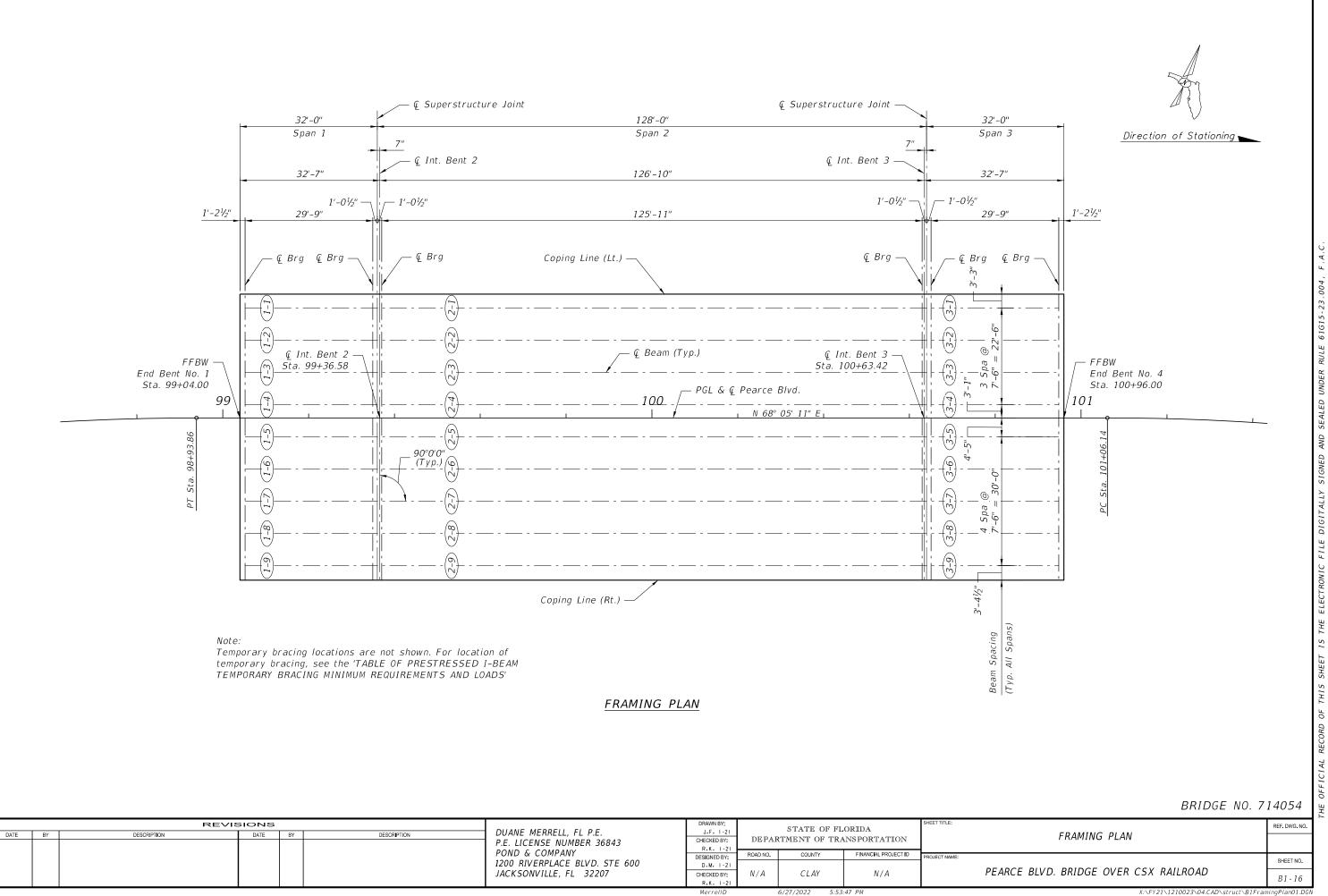
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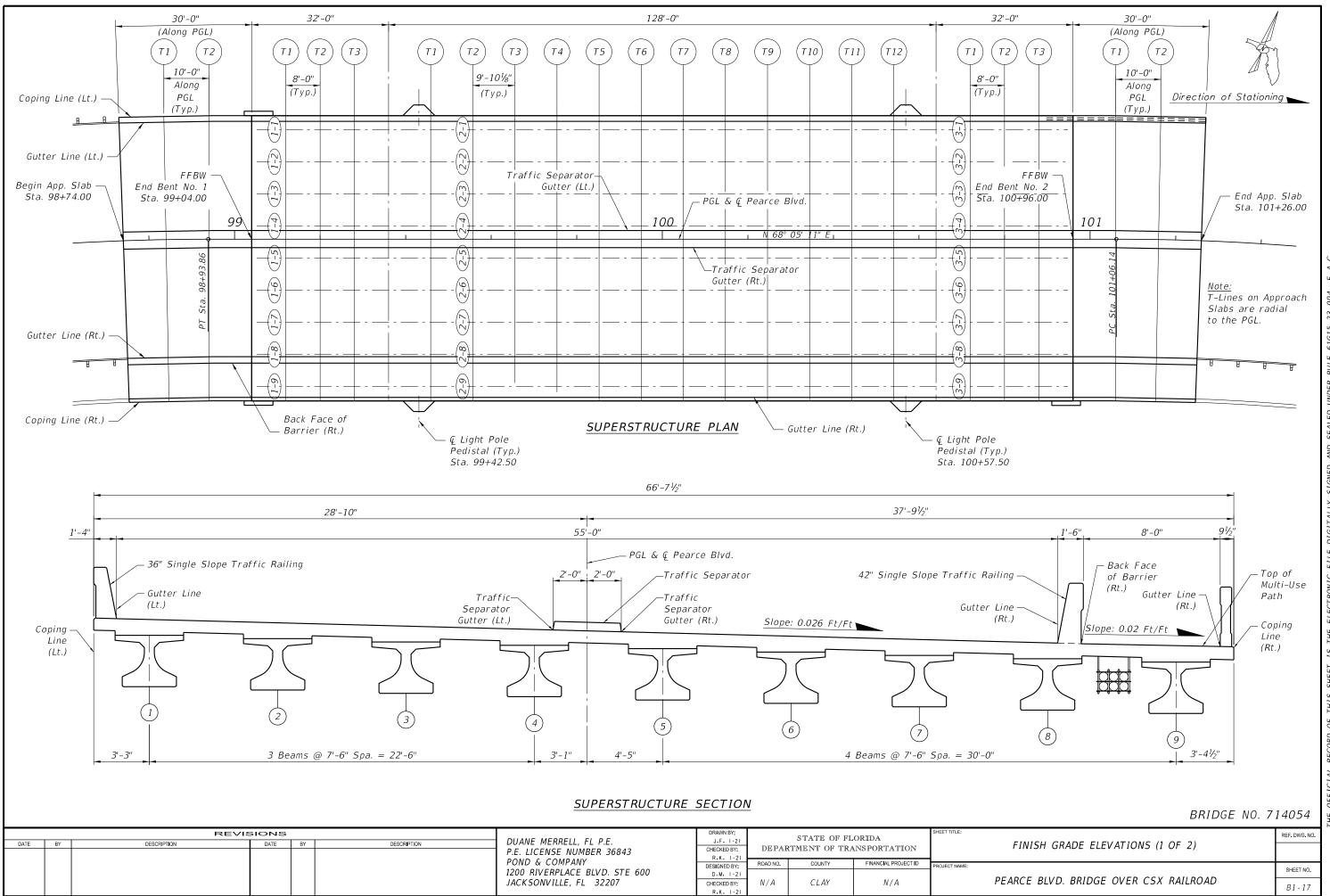
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POND & COMPANY R.K. 1-21 ROAD NO. COUNTY FINANCIAL PROJECT ID 1200 RIVERPLACE BLVD. STE 600 D.M. 1-21 DESIGNED BY: D.M. 1-21 PROJECT NAME: JACKSONVILLE, FL 32207 CHECKED BY: N/A CLAY N/A PEARCE	DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION			DEPAF				INTER
1200 RIVERPLACE BLVD. STE 600 JACKSONVILLE, FL 32207 Describer 120 Describer 120 Descr									ROAD NO	COUNTY	FINANCIAL PROJECT ID		
							1200 RIVERPLACE BLVD. STE 600		110/10/110	000111		PROJECT NAME:	
							JACKSONVILLE, FL 32207	CHECKED BY: R.K. I-21	N/A	CLAY	N/A		PEARCE



DATE BY



6161 SEALED AND



				S	PAN 1			
				End Bent 1	T-1	T-2	T-3	IB-2
		Line (Lt.)		61.058	61.169	61.270	61.361	61.442
	Gutter	Line (Lt.)		61.024	61.134	61.235	61.326	61.408
	Beam 1			60.974	61.084	61.185	61.276	61.358
	Beam 2)		60.779	60.889	60.990	61.081	61.163
	Beam 3	2		60.584	60.694	60.795	60.886	60.968
	Beam 4	!		60.389	60.499	60.600	60.691	60.773
	Traffic	Separator (Gutter (Lt.)	60.361	60.471	60.572	60.663	60.745
	PGL	1	. ,	60.309	60.419	60.520	60.611	60.693
	Traffic	Separator (Gutter (Rt.)	60.257	60.367	60.468	60.559	60.641
	Beam 5		()	60.194	60.304	60.405	60.496	60.578
	Beam 6			59.999	60.109	60.210	60.301	60.383
	Beam 7			59.804	59.914	60.015	60.106	60.188
	Beam 8			59.609	59.719	59.820	59.911	59.993
		Line (Rt.)		59.594	59.704	59.805	59.896	59.978
		ace of Traff	ic Railing	59.555	59.665	59.766	59.857	59.939
	Beam 9		re nannig	59,446	59.557	59.658	59.749	59.830
		Line (Rt.)		59.395	59.505	59.606	59.697	59.779
		Line (Rt.)		59.379	59.489	59.590	59.681	59.763
	coping			33.373	55.105	55.550	33.001	33.703
AN 2								
	Т-6	T-7	T-8	T-9	T-10	T - 11	T-12	IB-3
3	61.748	61.748	61.733	61.704	61.661	61.602	61.530	61.442
9	61.713	61.713	61.699	61.670	61.626	61.568	61.495	61.408
9	61.663	61.663	61.649	61.620	61.576	61.518	61.445	61.358
4 9	61.468	61.468	61.454	61.425	61.381	61.323	61.250	61.163
19 1	61.273 61.078	61.273 61.078	61.259 61.064	61.230 61.035	61.186 60.991	61.128 60.933	61.055 60.860	60.968 60.773
6 6	61.050	61.050	61.036	61.007	60.963	60.905	60.832	60.745
2A	60.998	60.998	60.984	60.955	60.911	60.853	60.780	60.693
24 22	60.946	60.946	60.932	60.903	60.859	60.801	60.728	60.641
9	60.883	60.883	60.869	60.840	60.796	60.738	60.665	60.578
4	60.688	60.688	60.674	60.645	60.601	60.543	60.470	60.383
9	60.493	60.479	60.450	60.450	60.406	60.348	60.275	60.188
34	60.298	60.298	60.284	60.255	60.211	60.153	60.080	59.993
9	60.283	60.283	60.269	60.240	60.196	60.138	60.065	59.978
0	60.244	60.244	60.230	60.201	60.157	60.099	60.026	59.939
21	60.136	60.136	60.121	60.092	60.049	59.990	59.918	59.830
0	60.084	60.084	60.070	60.041	59.997	59.939	59.866	59.779
4	60.068	60.068	60.054	60.025	59.981	59.923	59.850	59.763

						SPAN 2					
	IB-2	T-1	T-2	T-3	T-4	T-5	T-6	T-7	T-8	T-9	T-10
Coping Line (Lt.)	61.442	61.530	61.602	61.661	61.704	61.733	61.748	61.748	61.733	61.704	61.661
Gutter Line (Lt.)	61.408	61.495	61.568	61.626	61.670	61.699	61.713	61.713	61.699	61.670	61.626
Beam 1	61.358	61.445	61.518	61.576	61.620	61.649	61.663	61.663	61.649	61.620	61.576
Beam 2	61.163	61.250	61.323	61.381	61.425	61.454	61.468	61.468	61.454	61.425	61.381
Beam 3	60.968	61.055	61.128	61.186	61.230	61.259	61.273	61.273	61.259	61.230	61.186
Beam 4	60.773	60.860	60.933	60.991	61.035	61.064	61.078	61.078	61.064	61.035	60.991
Traffic Separator Gutter (Lt.)	60.745	60.832	60.905	60.963	61.007	61.036	61.050	61.050	61.036	61.007	60.963
PGL	60.693	60.780	60.853	60.911	60.955	60.984	60.998	60.998	60.984	60.955	60.911
Traffic Separator Gutter (Rt.)	60.641	60.728	60.801	60.859	60.903	60.932	60.946	60.946	60.932	60.903	60.859
Beam 5	60.578	60.665	60.738	60.796	60.840	60.869	60.883	60.883	60.869	60.840	60.796
Beam 6	60.383	60.470	60.543	60.601	60.645	60.674	60.688	60.688	60.674	60.645	60.601
Beam 7	60.188	60.275	60.348	60.406	60.450	60.479	60.493	60.479	60.450	60.450	60.406
Beam 8	59.993	60.080	60.153	60.211	60.255	60.284	60.298	60.298	60.284	60.255	60.211
Traffic Railing Gutter Line (Rt.) Back Face of Traffic Railing	59.978	60.065	60.138	60.196	60.240	60.269	60.283	60.283	60.269	60.240	60.196
Back Face of Traffic Railing	59.939	60.026	60.099	60.157	60.201	60.230	60.244	60.244	60.230	60.201	60.157
Beam 9	59.830	59.918	59.990	60.049	60.092	60.121	60.136	60.136	60.121	60.092	60.049
Gutter Line (Rt.)	59.779	59.866	59.939	59.997	60.041	60.070	60.084	60.084	60.070	60.041	59.997
Coping Line (Rt.)	<u>59.763</u>	59.850	59.923	59.981	60.025	60.054	60.068	60.068	60.054	60.025	59.981

	S	PAN 3			
	IB-3	T-1	T-2	T-3	End Bent 4
Coping Line (Lt.)	61.442	61.361	61.270	61.169	61.058
Gutter Line (Lt.)	61.408	61.326	61.235	61.134	61.024
Beam 1	61.358	61.276	61.185	61.084	60.974
Beam 2	61.163	61.081	60.990	60.889	60.779
Beam 3	60.968	60.886	60.795	60.694	60.584
Beam 4	60.773	60.691	60.600	60.499	60.389
Traffic Separator Gutter (Lt.)	60.745	60.663	60.572	60.471	60.309
PGL	60.693	60.611	60.520	60.419	60.309
Traffic Separator Gutter (Rt.)	60.641	60.559	60.468	60.367	60.257
Beam 5	60.578	60.496	60.405	60.304	60.194
Beam 6	60.383	60.301	60.210	60.109	59.999
Beam 7	60.188	60.106	60.015	59.914	59.804
Beam 8	59.993	59.911	59.820	59.719	59.609
Traffic Railing Gutter Line (Rt.)	59.978	59.896	59.805	59.704	59.594
Back Face of Traffic Railing	59.939	59.857	59.766	59.665	59.555
Beam 9	59.830	59.749	59.658	59.557	59.446
Gutter Line (Rt.)	59.779	59.697	59.606	59.505	59.395
Coping Line (Rt.)	59.763	59.681	59.590	59.489	59.379

APPROACH SLAB BEGIN Begin Slab

60.559

60.524

59.861

59.809

59.757

59.094

59.055

58.895

58.879

Coping Line (Lt.) Gutter Line (Lt.) Traffic Separator Gutter (Lt.)

PGL PGL Traffic Separator Gutter (Rt.) Traffic Railing Gutter Line (Rt.) Back Face of Traffic Railing Gutter Line (Rt.) Coping Line (Rt.)

T-1

60.740

60.706

60.043

59.991

59.939

59.276

59.237

59.077

59.061

T-2

60.907

60.872

60.209

60.157

60.105

59.442

59.403

59.243

59.227

End Bent 1

61.058 61.024

60.361

60.309

60.257

59.594

59.555

59.395

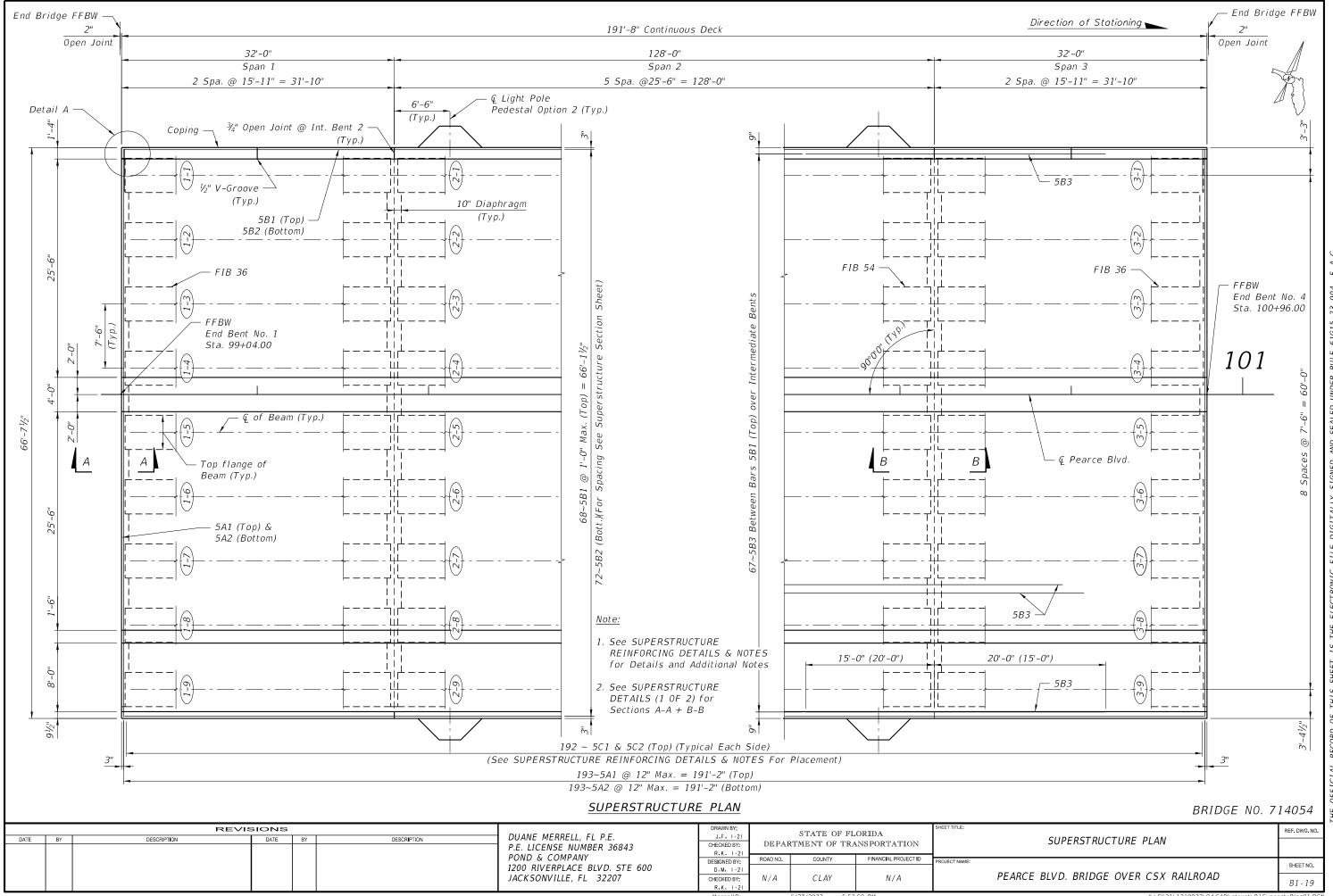
59.379

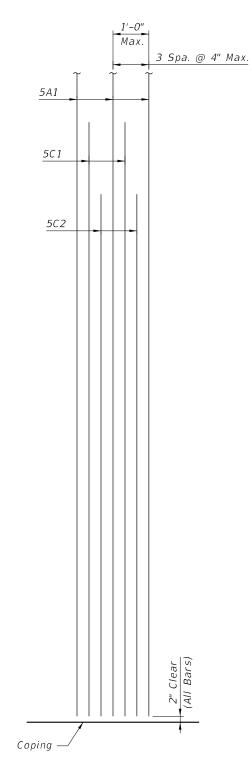
AF	PROACH SLA	B END		
	End Bent 4	T-1	T-2	End Slab
Coping Line (Lt.)	61.058	60.907	60.740	60.559
Gutter Line (Lt.)	61.024	60.872	60.706	60.524
Traffic Separator Gutter (Lt.)	60.361	60.209	60.043	59.861
PGL	60.309	60.157	59.991	59.809
Traffic Separator Gutter (Rt.)	60.257	60.105	59.939	59.757
Traffic Railing Gutter Line (Rt.)	59.607	59.455	59.289	59.107
Back Face of Traffic Railing	59.555	59.442	59.276	59.094
Gutter Line (Rt.)	59.395	59.282	59.116	58.934
Coping Line (Rt.)	59.379	59.266	59.100	58.918

Note: See FINISH GRADE ELEVATIONS (1 OF 2) for Elevation Locations.

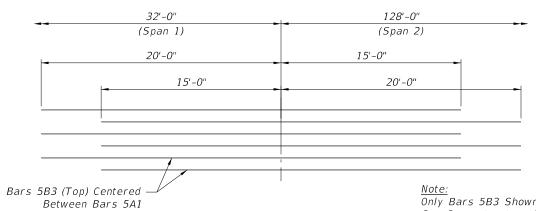
FINISH GRADE ELEVATIONS

BRIDGE N													14054	
	REVISIONS			DUANE MERRELL, FL P.E.	DRAWN BY: J.F. 1-21	STATE OF FLORIDA			SHEET TITLE:		REF. DWG. NO.			
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION	P.E. LICENSE NUMBER 36843	CHECKED BY: R.K. I-21	DEPARTMENT OF TRANSPORTATION				FINISH GRADE ELEVATIONS (2 OF 2)		
						POND & COMPANY 1200 RIVERPLACE BLVD. STE 600	DESIGNED BY: D.M. I-21	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:		SHEET NO.	
						JACKSONVILLE, FL 32207	CHECKED BY: R.K. I-21	N/A	CLAY	N/A		PEARCE BLVD. BRIDGE OVER CSX RAILROAD	B1-18	
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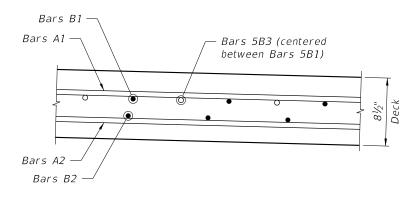


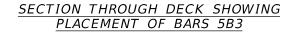
PLACEMENT OF BARS 5C1 & 5C2



PLAN VIEW SHOWING PLACEMENT OF BARS 5B3

(Shown at Int. Bent 2, Similar at Int Bent 3)





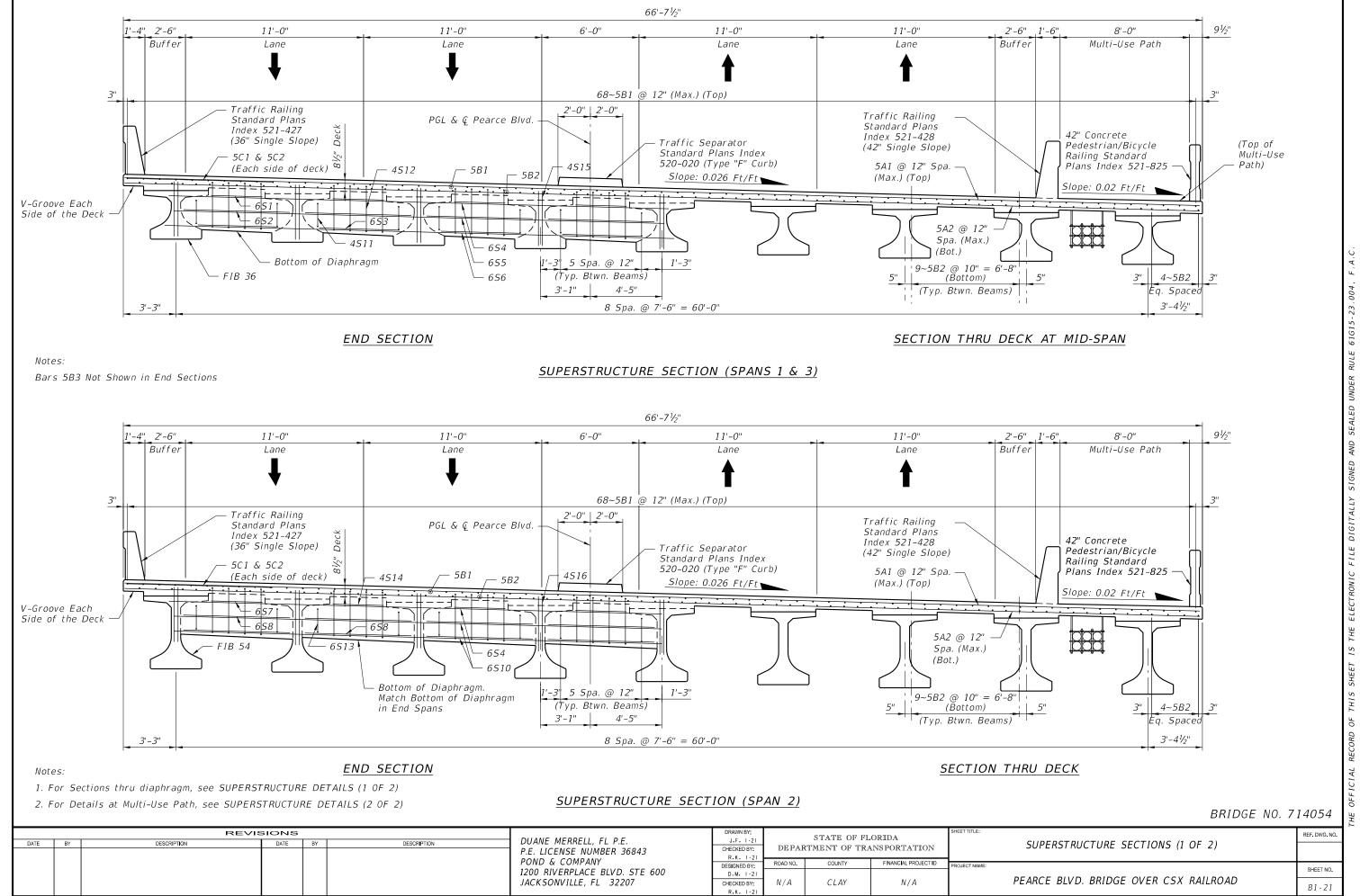
Reinforcing Notes:

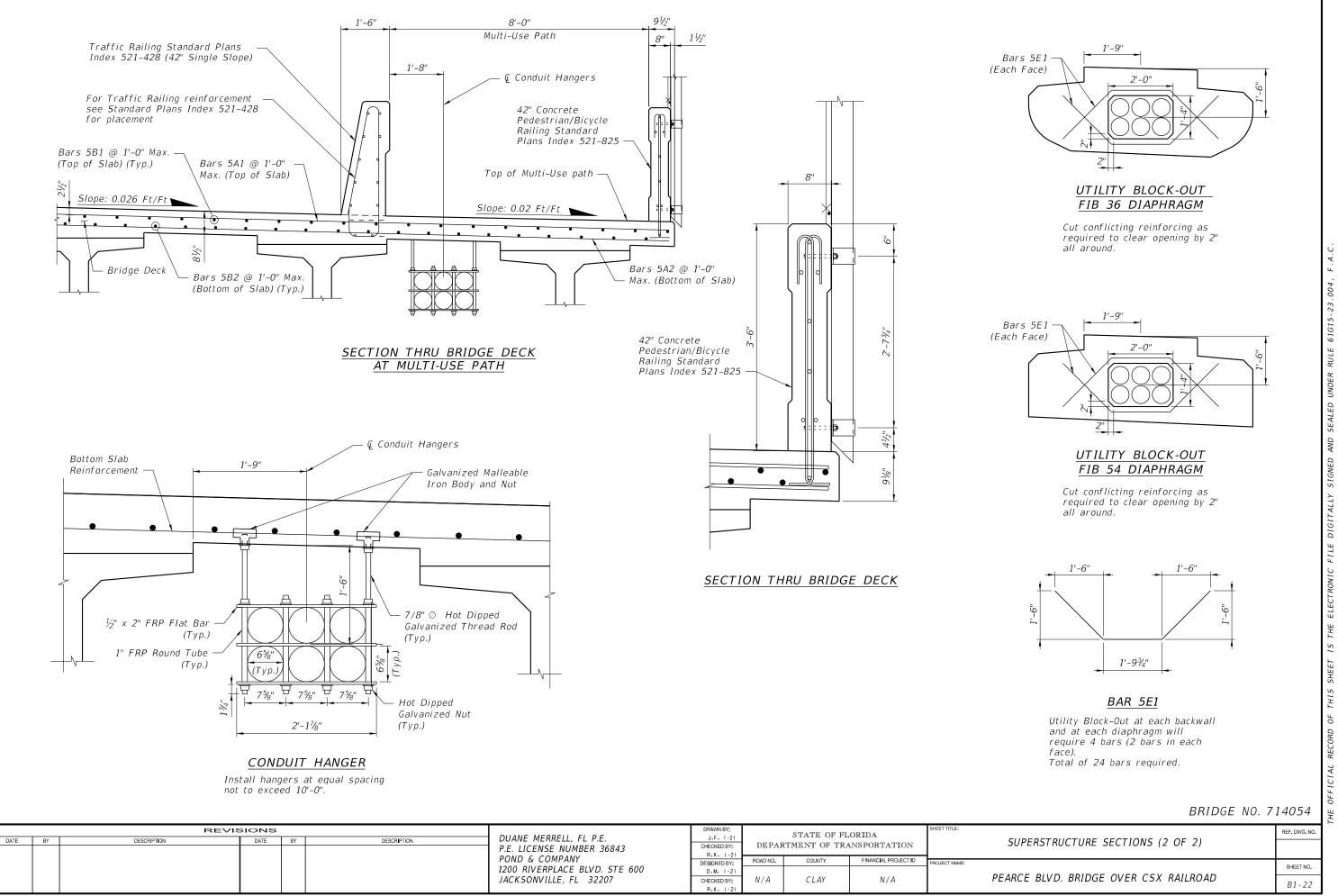
- 1. Provide 1'-6" Lap Splice in top and bottom longitudinal reinforcing.
- 2. Provide 2'-0" Lap in top and bottom longitudinal reinforcing.
- З. Stagger Splice locations in adjacent longitudinal bars.
- 4. For placement of 5B2 bars, see SUPERSTRUCTURE SECTIONS (1 OF 2).

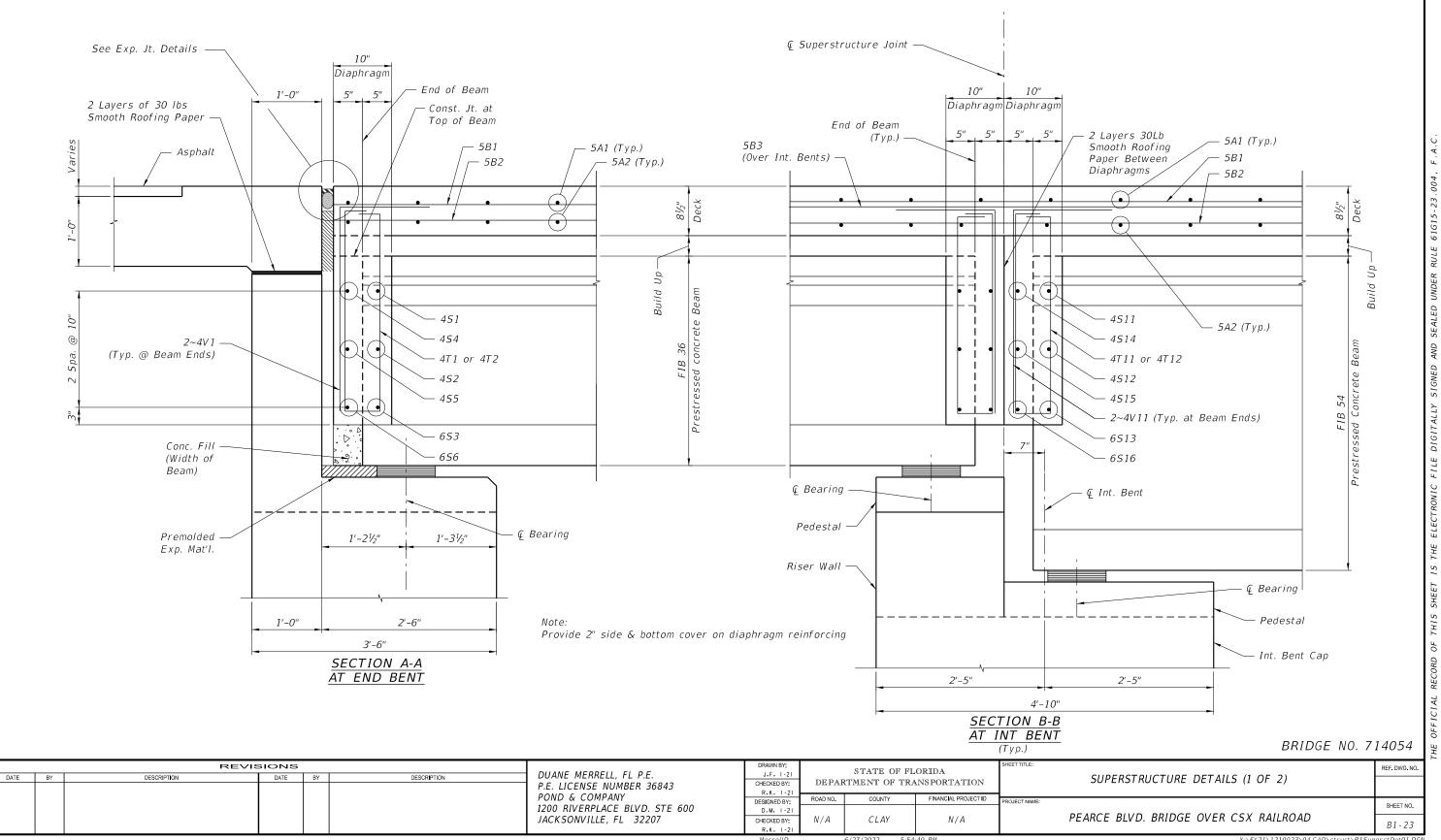
							BRIDGE NO. 7	14054	T HF
REVISIONS		DRAWN BY		STATE OF FI	ORIDA	SHEET TITLE:		REF. DWG. NO.	1
DATE BY DESCRIPTION DATE BY DESCRIPTION	DUANE MERRELL, FL P.E. P.E. LICENSE NUMBER 36843 POND & COMPANY	J.F. I-21 CHECKED BY: R.K. I-21	DEPARTMENT OF TRANSPORTATION			SUPERSTRUCTURE REINFORCING DETAILS & NOTES		l	
	1200 RIVERPLACE BLVD. STE 600	DESIGNED BY: D.M. -2	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:		SHEET NO.	ĺ
	JACKSONVILLE, FL 32207	CHECKED BY: R.K. I-21	N/A	CLAY	N/A		PEARCE BLVD. BRIDGE OVER CSX RAILROAD	B1-20	
		MerrellD		6/27/2022 5:54	1:03 PM		X:\FY21\1210023\04_CAD\struct\B1Su	inerstDet03 dan	í

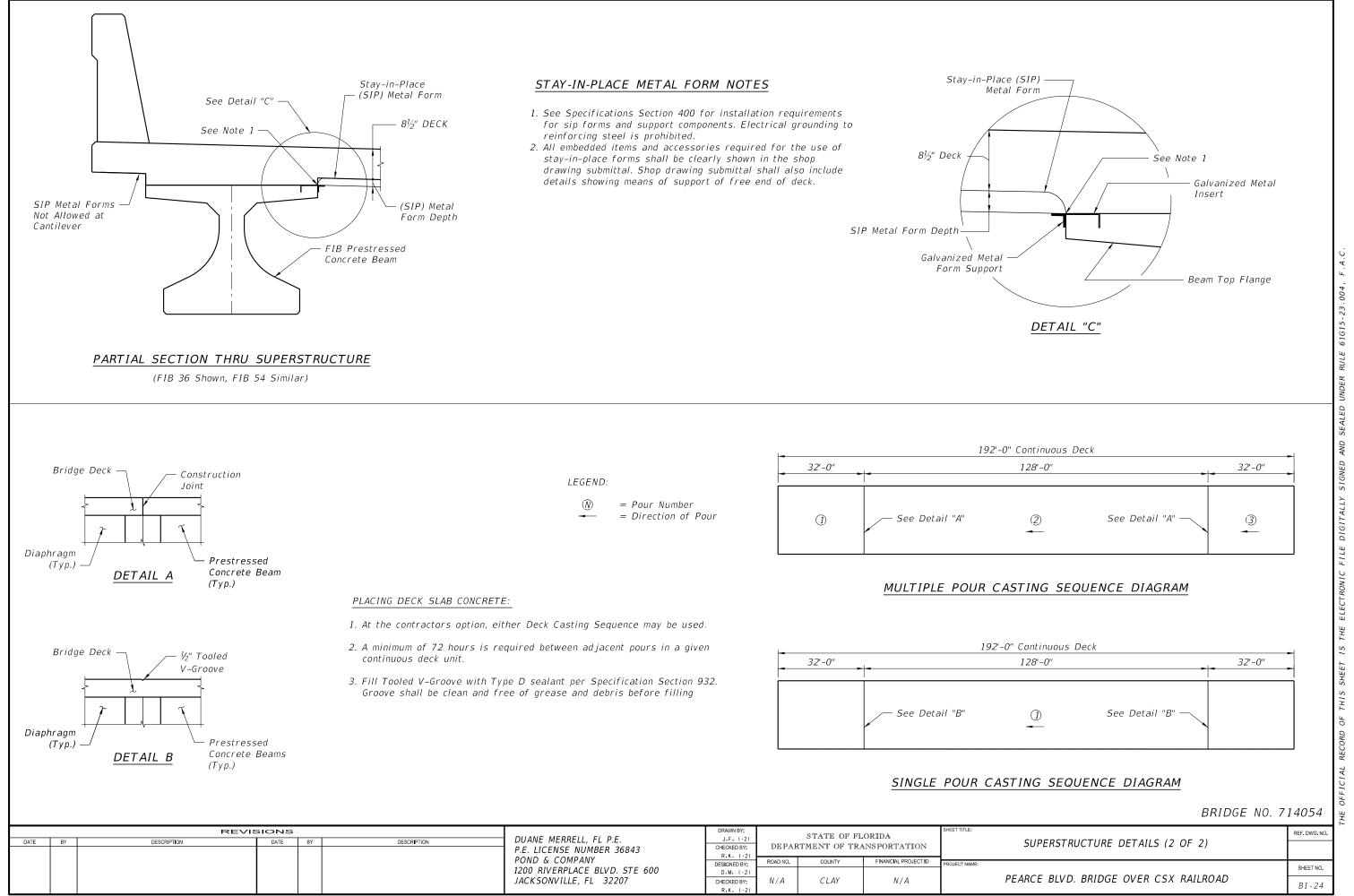
Only Bars 5B3 Shown. See Superstructure Plan for complete deck reinforcing.

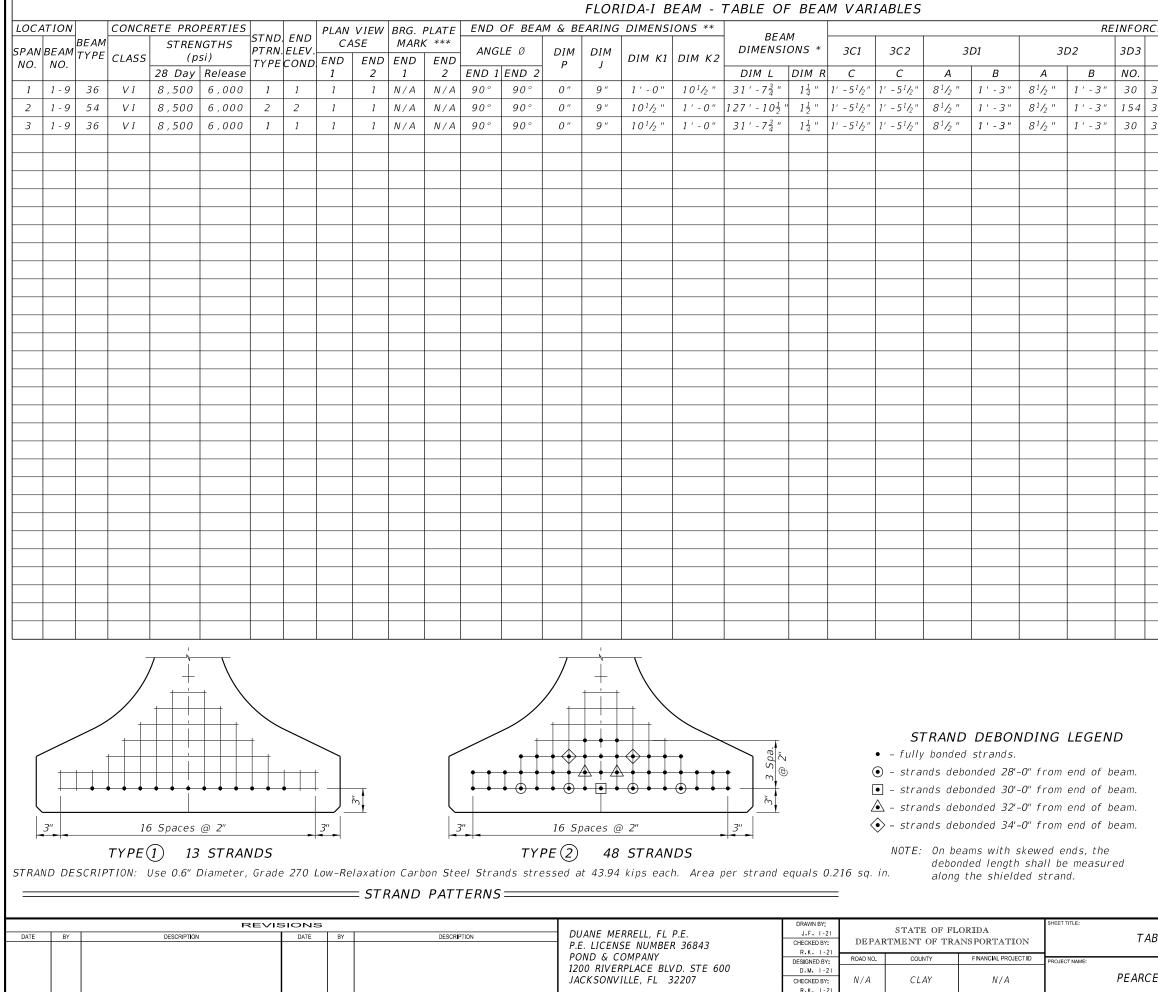












4M1	4M2	4M3	5K	NO.	OF BARS		ES	SP. BARS 5K *
D	D	NO.	NO.	<i>S1</i>	<i>S2</i>	53	54	V1
' - 8 "	3'-8"	22	64	7	4	3	1	6 "
' - 8 "	3'-8"	116	192	30	20	14	9	6 "
' - 8 "	3'-8"	22	64	7	4	3	1	6 "

DIMENSION NOTES

- * All longitudinal beam dimensions shown on this sheet with a single asterisk (*) are measured along the centerline of beam. Dimension "R" is calculated at mid-height of the beam.
- ** End beam bearing dimensions "J" and "K" are measured perpendicular to *Q* Bearing along the bottom of the beam.

BEARING PLATES

*** See the Bearing Plate Data Table for details.

BRIDGE NO. 7	14054	THE
	REF. DWG. NO.	
BLE OF BEAM VARIABLES (1 OF 2)		
	SHEET NO.	
E BLVD. BRIDGE OVER CSX RAILROAD	B1-25	

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E I C I

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NOTE: Work this sheet with Standard Plans Index 450–010 and the applicable "Florida-I Beam Standard Details" Index.

NOTES:	1. Work this sheet with Standard Plans Index 450-199. 2. All dimensions shown in this						ON DATA TA D I-BEAMS	BLE	Table Da	ate 07/01/17	POUREL	D EXPANSIO INDEX
	Table are in inches.	LOCA	TION		D THEORE P OVER Q		NET BEAM CAMBER (PRESTRESS	NET BEAM CAMBER (PRESTRESS	DEAD LOAD DEFLECTION DURING	BUILD-UP	LOCATION	DIM. "A" @ 70°F
		SPAN NO.	BEAM NO.	AT BEGIN SPAN DIM B	AT Q SPAN DIM C	AT END SPAN DIM D	- DEAD LOAD OF BEAM) @ RELEASE	- DEAD LOAD		CASE	End Bent 1 End Bent 4	2" 2"
		1	1	1.15	1.12	1.15	0.14	0.28	0.02	3		2
		1	9	1.15	1.12	1.15	0.14	0.28	0.02	3		
		1	2-8	1.15	1.12	1.15	0.14	0.28	0.03	3		
		2	1	1.12	3.67	1.12	1.67	3.91	2.79	4	NOTE:	1
		2	9	1.12	3.67	1.12	1.67	3.91	2.79	4		istment per 10°
		2	2-8	1.12	3.88	1.12	1.67	3.91	3.00	4	Expansion Jo	oint. Work this
		3	1	1.15	1.12	1.15	0.14	0.28	0.02	3		
		3	9	1.15	1.12	1.15	0.14	0.28	0.02	3		
		3	2-8	1.15	1.12	1.15	0.14	0.28	0.03	3		

BEARING PAD DATA TABLE Table Date 7-01-13													
SPAN NO(s).	BEAM NO(s).	PAD TYPE	BEAM TYPE	BEAM END *									
1	1 Thru 9	F	FIB 36	1 & 2									
2	1 Thru 9	F	FIB 54	1 & 2									
3	2 Thru 9	F	FIB 36	1 & 2									

NOTE [Notes Date 07-01-14]:

Work this table with Index 400-510 for Pad Types AA, AB, D, E, F, G, H, J & K, and/or any project specific bearing pads.

* END 1 = Begin Bridge end of beam (Back station). END 2 = End Bridge end of beam (Ahead station).

	BEARING PLATE DATA TABLE - TYPE 1															Table	Table Date 7-01-13		
	GENE	RAL BEAR	RING PL	ATE D	ATA			EMBEDDE	ED PLATE S (PLATE A)	BEVELED	В	EVELE	D PLAT	TE DIM	IENSIC	NS (P	LATE E	3)	
BRG. PLATE	SPAN	BEAM	PAD	BEAM	PLAN VIEW SLOPE ANG				hes)	PLATE REQUIRED				(inc	hes)			_	
MARK ***	NO(s).	NO(s).	TYPE	END	CASE	(%) **	Ø*	G	F	(Yes/No)	С	D	Е	F	W	Х	Ŷ	Ζ	
1-1	1	1-9	F	1&2	1	< 2.0	90	13 <u>-1</u>	36	NO	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	
2-1	2	1-9	F	1&2	1	< 2.0	90	13 <u>-1</u>	36	NO	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	
3-1	3	1-9	F	1&2	1	< 2.0	90	13 <u>-1</u>	36	NO	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	

NOTES:

See Standard Plans Index 450-511 for additional notes and details.

* \emptyset = Acute angle (\le 90°) measured from left or right side of Q Beam as required.

** Slope measured along (of Beam at (of Bearing. *** See "TABLE OF BEAM VARIABLES" and Standard Plans Index 450-010 for Florida-I Beams or Index 450-120 for AASHTO Type II Beams.

_ L													
			REVIS	SIONS				DRAWN BY		STATE OF F	ORIDA	SHEET TITLE:	
	DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION	DUANE MERRELL, FL P.E.	J.F. I-21 CHECKED BY:	DEPAR		ANSPORTATION		TABLE
							P.E. LICENSE NUMBER 36843	R.K. 1-21	1/131 111	CIDIMA OF IN			
							POND & COMPANY	DESIGNED BY:	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:	
							1200 RIVERPLACE BLVD. STE 600	D.M. 1-21					PEARCE B
							JACKSONVILLE, FL 32207	CHECKED BY:	N/A	CLAY	N/A		PEARCE D
l								R.K. I-21 MerrellD		6/27/2022 5:5	1:42 PM		

	N JOINT DATA 7 458-110	TABLE	Table Date 1-01-09
DIM. "A" @ 70°F	TOTAL DESIGN MOVEMENT		' ADJUSTMENT PER 10°F
2"	⁵ ⁄8"		¹ /16"
2"	<i>5</i> /8"		ŀ∕16″

nent per 10°F shown is measured perpendicular to 🤅 Work this table with Standard Plan's Index 458-110.

> BRIDGE NO. 714054 REF. DWG. NO. LE OF BEAM VARIABLES (2 OF 2) SHEET NO. BLVD. BRIDGE OVER CSX RAILROAD B1-26 erst021

PRESTRESSED BEAM STABILITY AND TEMPORARY BRACING NOTES:

1. Ensure beam stability and design temporary beam bracing, including connections, in accordance with the Specifications and the FDOT Structures Manual. 2. Construction:

- a. Evaluate the beam stability and bracing requirements against the design assumptions including:
 - *i.* Loadings given in the plans.
 - ii. Beam Camber (less than 6 inches) and Beam Sweep (in compliance with Specification 450 requirements).
 - *iii. Bearings given in the plans.*
- b. Securely connect bracing to each beam. Do not allow the bracing to exert any vertical force on the outer edge of the top flange. Preform all bolt holes in beams and fill after use in accordance with the Specifications.

		1				
	STAGE I		STAGE 2		5	TAGE 3
BEAM NO.	BRACE ENDS PRIOR TO CRANE RELEASE? ¹ (YES/NO)	TOTAL LINES OF BRACING ^{2,3,7}	MINIMUM NUMBER OF ADJACENT BEAMS ERECTED	HORIZONTAL LOAD AT EACH BRACE ⁴ (KIP)	TOTAL LINES OF BRACING ^{3,5,7}	OVERTURNING MOMENT AT EACH BRACE ⁶ (KIP-FT)
1-9	Yes	2	5	2.64	2	22.82
1-9	Yes	2	5	18.51	2	34.89
1-9	Yes	2	5	2.64	2	22.82
	NO. 1-9 1-9	DLAM NO.PRIOR TO CRANE RELEASE?1 (YES/NO)1-9Yes1-9Yes	BEAM NO.BRACE ENDS PRIOR TO CRANE RELEASE?1 (YES/NO)TOTAL LINES OF BRACING2.3.71-9Yes21-9Yes2	BEAM NO.BRACE ENDS PRIOR TO CRANE RELEASE?1 (YES/NO)TOTAL LINES OF BRACING2,3,7MINIMUM NUMBER OF ADJACENT BEAMS ERECTED1-9Yes251-9Yes251-9Yes25	BEAM NO.BRACE ENDS PRIOR TO CRANE RELEASE?1 (YES/NO)TOTAL LINES OF BRACING2,3,7MINIMUM NUMBER OF ADJACENT BEAMS ERECTEDHORIZONTAL LOAD AT EACH BRACE4 (KIP)1-9Yes252.641-9Yes2518.51	BEAM NO.BRACE ENDS PRIOR TO CRANE RELEASE?1 (YES/NO)TOTAL LINES OF BRACING2.3.7MINIMUM NUMBER OF ADJACENT BEAMS ERECTEDHORIZONTAL LOAD AT EACH BRACE4 (KIP)TOTAL LINES OF BRACING3.5.71-9Yes252.6421-9Yes2518.512

1. Anchor Bracing loads to be determined by the Contractor.

2. Total lines of Stage 2 bracing, including end bracing, are required to be installed within 24 hours after initial beam placement.

3. Equally space bracing along the length of the beams allowing for variations due connection conflicts and skew.

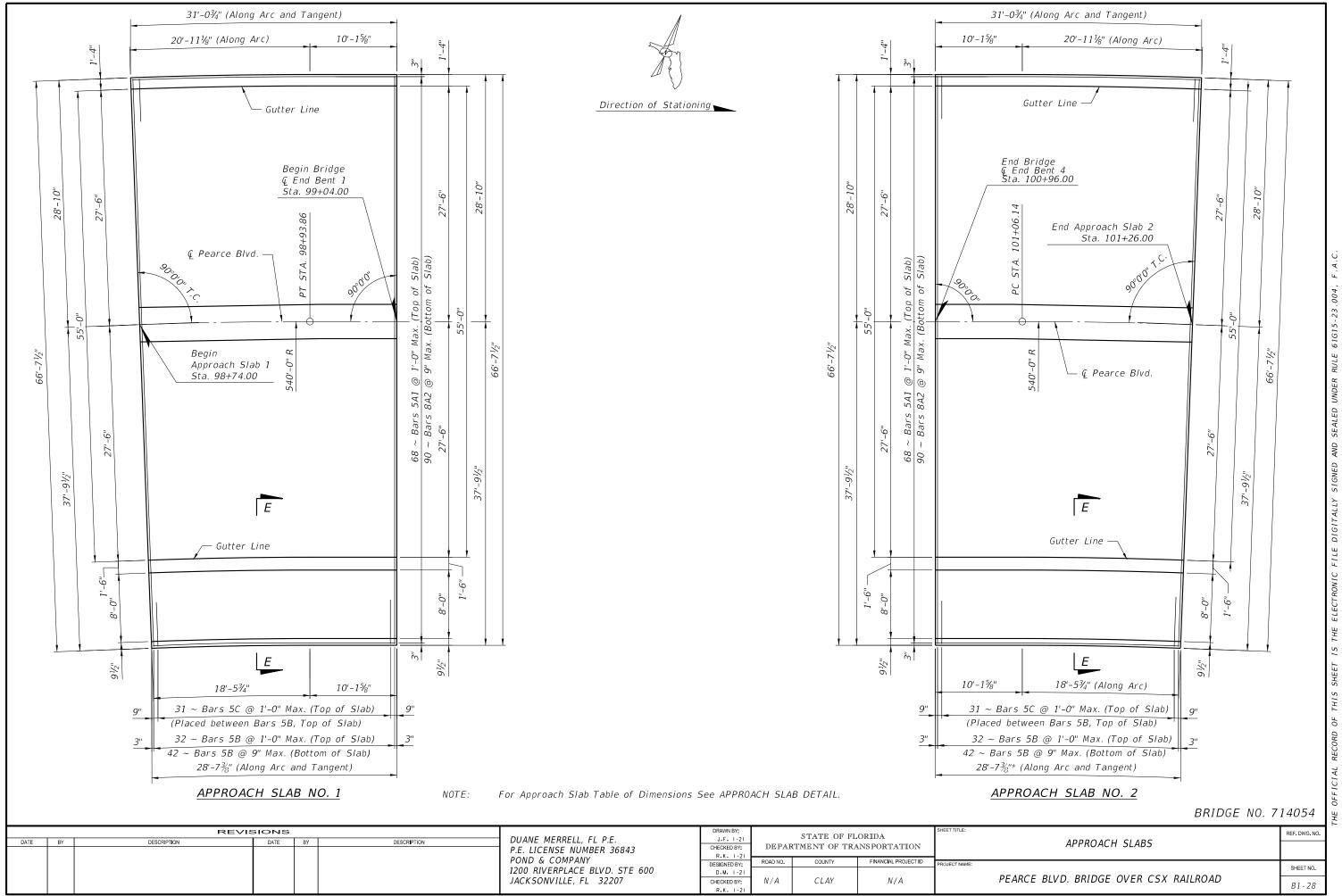
4. LRFD Strength III loads applied to beam at brace point (see SDG 11.6).

5. Total lines of Stage 3 bracing, including end bracing, are required to be installed prior to deck placement.

6. LRFD Strength I overturning moment applied to beam at brace point (see SDG 11.6).

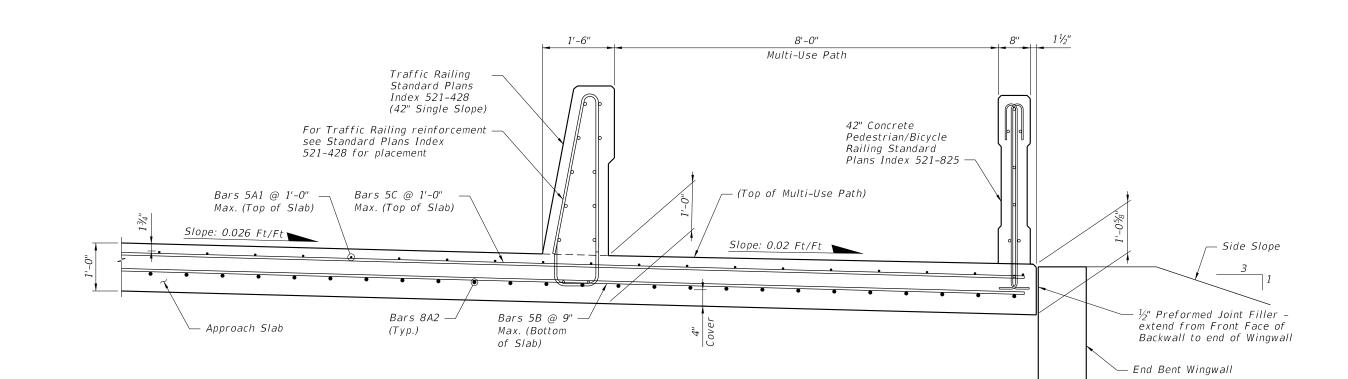
7. Submit shop drawings for temporary bracing plan including locations of preformed beam holes/inserts.

										BRIDGE NO. 7	14054
	REV	ISIONS				DRAWN BY		STATE O	F FLORIDA	SHEET TITLE:	REF. DWG. NO.
DATE BY	DESCRIPTION	DATE	BY	DESCRIPTION	DUANE MERRELL, FL P.E. P.E. LICENSE NUMBER 36843	J.F. 1-21 CHECKED BY: R.K. 1-21	CHECKED BY: DE PARTM		TRANSPORTATION	PRESTRESSED BEAM TEMPORARY BRACING DATA TABLES	
					POND & COMPANY 1200 RIVERPLACE BLVD. STE 600	DESIGNED BY: D.M. 1-21	ROAD NO.	COUNTY	FINANCIAL PROJECT ID		SHEET NO.
					JACKSONVILLE, FL 32207	CHECKED BY: R.K. I-21	N/A	CLAY	N/A	PEARCE BLVD. BRIDGE OVER CSX RAILROAD	B1 - 27
						MerrellD		6/27/2022	5:54:43 PM	X:\FY21\1210023\04.CAD\struct\B1DataTableBea	amBracing01.DGN



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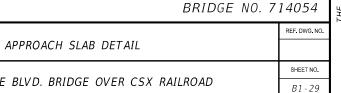
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SECTION E-E

APPROACH SLAB TABLE OF DIMENSIONS												
LOCATION												
LOCATION	I	ANGLE Ø										
Approach Sab No. 1	31'-0½"	28'-7%"	1'-4"	9½"	64'-	-6"	0°					
Approach Slab No. 2 $31'-0\frac{1}{8}"$ $28'-7\frac{5}{8}"$ $1'-4"$ $9\frac{1}{2}"$ $64'-6"$ 0°												
Dimension Notes: Dimensions L1 & L2 are measured along gutter line, inside face of parapet or inside face of railing on raised sidewalks. Dimensions L1 & L2 are arc dimensions within curved alignments. Work this Data Table with Standard Plans Index 400-090.												

		REVIS	SIONS				DRAWN BY:		STATE OF	FLOPIDA	SHEET TITLE:	
DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION	DUANE MERRELL, FL P.E. P.E. LICENSE NUMBER 36843	J.F. I-21 CHECKED BY: R.K. I-21	DEPAI		RANSPORTATION		A
						POND & COMPANY	DESIGNED BY:	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:	
						1200 RIVERPLACE BLVD. STE 600 JACKSONVILLE, FL 32207	D.M. 1-21 CHECKED BY: R.K. 1-21	N/A	CLAY	N/A		PEARCE I
							MerrellD		6/27/2022 5	:54:56 PM	-	



RECORD OF THIS SHEET IS THE ELECTRONIC FILE DIGITALLY SIGNED AND SEALED UNDER RULE 61G15-23.004, F.A.C. ICIAL Ŧ

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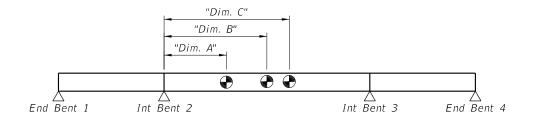
	MAF	₹ <i>K</i>	LENGTH				С	D	E	F	Н	J	K N	
	SIZE	DES	FT IN	BARS BAR	AC	G FT IN FR	FT IN FR	FT IN FR				P FT IN FR F		
		[]	LOCA	TION EN	D B	ENT 1			Na T	D. REQUI	IRED = 1			
	8	A 1	72- 2	60 41	\vdash	32-5	39-9					4-0		1
		A2	71-2	2 41		35-8	35- 6					3-0		1
		A3	6-10	32 11		4-10	1-0	1-0						
		B1 B2	4-5 68-3	134 1		4- 5 2- 0	66 2 1/							1
		B∠ D1	<u> </u>	82 1723		2-0	66-2 1/2 0-3	1- 6						
		P1	6-8	45 11		3-6	1-7	1 - 7						
		P2	5-2	63 11		2-0	1-7	1-7						
		S1	10-10	54 4		2-5	3-0							
		S2	6-8	54 4		2-5	0-11							
		S3	8-6	18 5		2-5	3-0	0- 4	0-4					
		V1 W1	<u>3-11</u> 5- 6	12 1 10 1		<u>3-11</u> 5-6								
			0 0	10 1										
			LDCA	TION EN	DВ	ENT 4		I	N	J. REQUI	IRED = 1	· ·	I	
	8		72-2	60 41		32-5	39-9					4-0		
		A2 A3	71- 2 6-10	2 41 32 11	\vdash	35-8	35- 6 1- 0	1-0				3-0		
		B1	4-5	134 1	\vdash	4-5								
	5	B2	66-3	82		2-0	66-21/2							
		D1	3-10	17 23		1-6	0-3	1-6						
		<i>P1</i>	6-8	45 11		3-6	1-7	1-7						
		P2 S1	5-2	63 11		<u>2-0</u> 2-5	1 - 7	1-7				<u> </u>		
		51 S2	10-10 6- 8	54 4 54 4		2-5	3- 0 0-11					+		+
		52 S3	8-6	18 5		2-5	3-0	0- 4	0-4					+
		V1	3-11	12 1		3-11								
	5	W1	5- 6	10 1		5-6								
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	8	A 1	69-8	7 41		31-0	38-8					3-6		1
		A2	69-8	2 41		27-3	42-5					3-6		1
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		<i>B1</i>	6-6	66 11		1-4	2-7	2-7				ļļ.		
	4	C1 E1	67- 9 7- 2	4 2 3 11		1-9 4-2	66- 0 1- 6	1- 6				<u> </u>		1
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		Ρ1	6-8	27 11		3-6	1-7	1-7						
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		S3	11-0	18 5		2-6	4-4	0-10	0-10					
		V1	7-5	6 10		5-11	1-6							
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		W1 W2	4-11	$\begin{array}{c c} 3 & 11 \\ 12 & 0 \end{array}$	\vdash	4-2	1-0	1-0				+		
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						1							QUEET TO F	
DATE BY DESCRIPTION DATE	BY		DESCRIPTIC	ON			RELL, FL P.E.		DRAWN BY: J.F. 1-21		STATE OF F		SHEET TITLE:	
						P.E. LICENS POND & CO	E NUMBER 36	843	CHECKED BY: R.K. I-2			RANSPORTATION		
						1200 RIVERI	PLACE BLVD.		DESIGNED BY: D.M. I-21	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:	
							LE, FL 3220		CHECKED BY: R.K. I-2	N/A	CLAY	N/A		PEARC
						L			R.K. I-2 MerrelID		6/27/2022 5:5		1	

	REF. DWG. NO.						
REINFORCING BAR LIST (1 OF 2)							
	SHEET NO.						
RCE BLVD. BRIDGE OVER CSX RAILROAD	B1-30						
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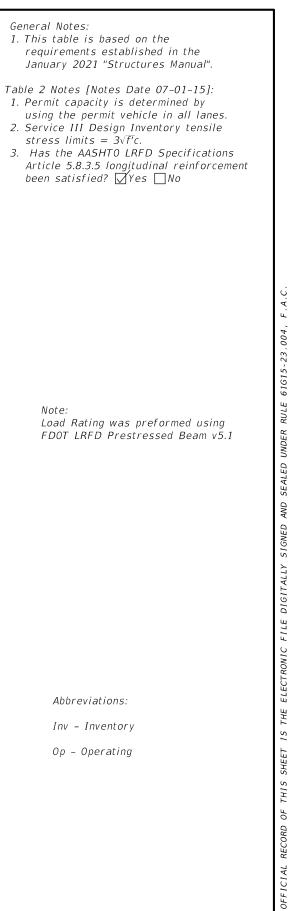
					MARK	LENGTH		TYP STY		С	D	E	F	Н	J	K	N	Φ
				-	SIZE DES	FT IN	BARS	BAR A G	FT IN FR	P FT IN FR	FT IN FR	FT IN FR	FT IN F	R FT IN FR	FT IN FR	FT IN FR	ND	ANG
				_		LDCA	TION	SUPERS	I STRUCTURE			/	VD. REQUI	IRED = 1				
				=	E A 1	69 4	107	41	70 4	78 0					2.0		1	
				-	5 A1 5 A2	68- 4 68- 4	193 193		30- 4 26- 7	38- 0 41- 9					2- 0 2- 0		1	
					5 B1	199- 4	68	41	8-0	60- 0	11-4	60- 0	60- 0		2- 0		4	
				-	5 B2	199- 4	68		40-0	60- 0	19- 4	60- 0	20- 0		2-0		4	
				-	5 B3 5 C1	35- 0 16- 8	134 384	1	35- 0 16- 8									
				-	5 C2	14-8	384	1	14-8									
					5 E1	6-4	24		1-93/	4 2-3	2-3						45	45
				-	4 S1 4 S2	<u>3-2</u> 6-6	32 32		<u>3-2</u> 6-6									
				-	4 S2 6 S3	4-11	32		4-11									
				-	4 S4	62-10	4		2-0	60-10							1	
					4 S5	62-4	4		2-0	60- 3 1/2							1	
				-	6 S6 4 S11	63- 6 3- 2	4 16		2- 0 3- 2	61-51/2							1	
				F	4 S11 4 S12	<u> </u>	16		6-7				1					
				F	6 S13	6-7	16		6-7									
					6 S14	62-10	2	2	2-0	60-10							1	
				F	6 S15	62-3	2		2-0	60-3							1	
				-	6 S16 4 T1	62- 3 4- 1	2 64		2-0 $1-6\frac{1}{2}$	60- 3 60- 4	0- 4	0- 4					1	
				F	4 T2	6-6	128		2-9	0-4	0-4	0-4						
					4 T11	4-4	64		1-8	0-4	0-4	0- 4						
					4 T12	6-6	128		2-9	0-4	0- 4	0- 4						
				-	4 V1 4 V11	3-3 3-3	72 72		2- 9 2- 9	0- 6 0- 6								
				_	, , , , , , , , , , , , , , , , , , , ,		12	10	2 3									
						LOCA	TION	APPROA	CH SLAB	& 2		1	VD. REQU	IRED = 2	1			
				_				70	10 0	500 5								
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				_	8 A2	VARY	90		10-0	502-5								2
						28-9	0		10- 0	568- 7								2
				-	5 B	68-3	74 31		2-0	66- 3							1	
					5 C	5-0	51	1	5-0		END OF L	ICT						
											END OF L	151						
			REVIS	BIONS	1				DILANE MEL	RRELL, FL P.E.		DRAWN BY: J.F. I -		STATE OF F	LORIDA	SHEET TITLE:		
DATE	BY	DESCRIPTION		DATE BY	+	DESCRIPTI	UN		P.E. LICENS	E NUMBER 36	843	CHECKED B' R.K. I	DEPAR	RTMENT OF TR	ANSPORTATIO			
									POND & CC 1200 RIVER	OMPANY PLACE BLVD. S	STE 600	DESIGNED B D+M+I-	Y: ROAD NO.	COUNTY	FINANCIAL PROJEC	PROJECT NAME		
										LLE, FL 3220		CHECKED B	(: N/A	CLAY	N/A			PEARC
												R.K. I-		6/27/2022 5:5	5.00 PM			

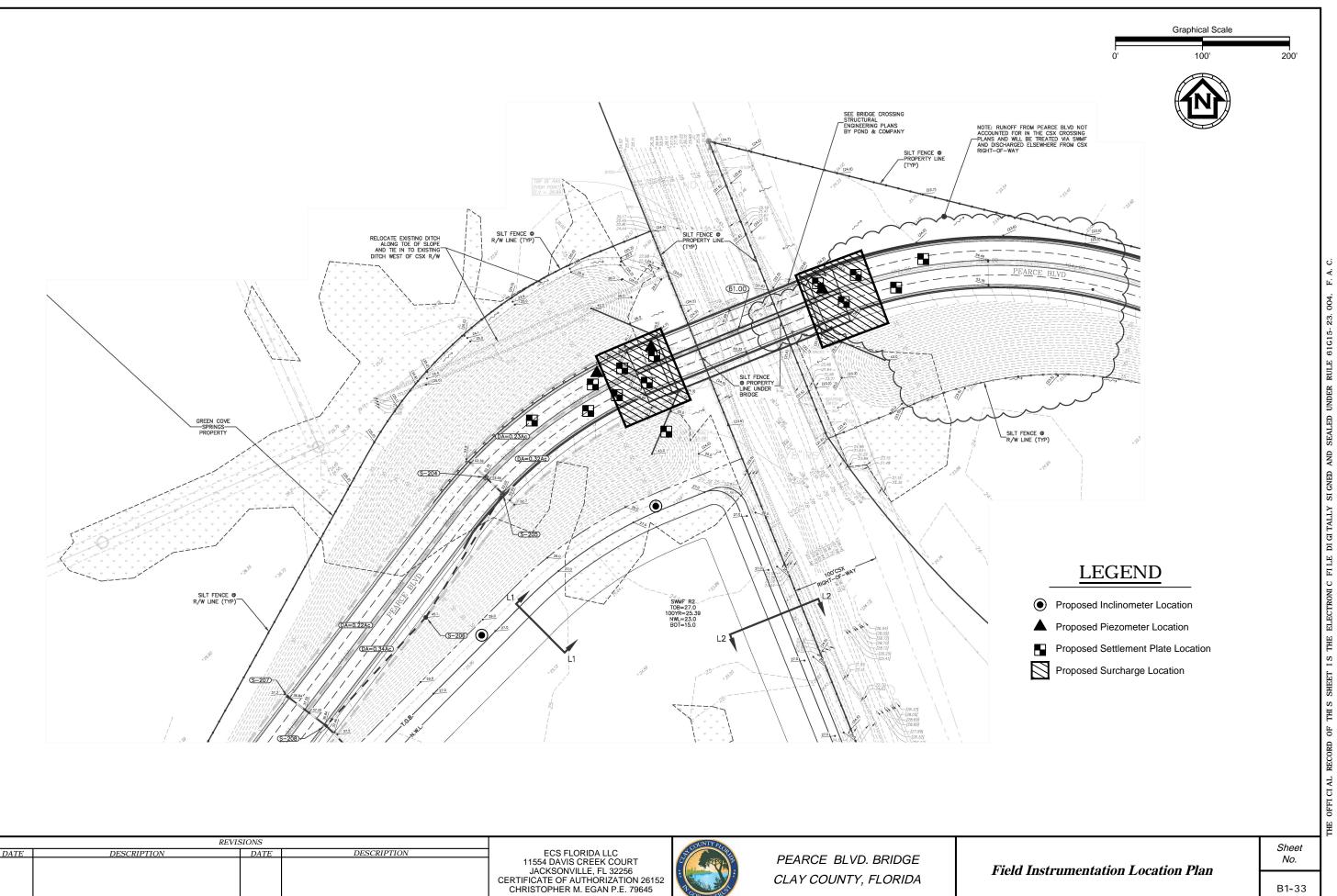
BRIDGE NO. 7	14054	THE OFFICIAL R
REINFORCING BAR LIST (2 OF 2)	REF. DWG. NO.	
	SHEET NO.	
RCE BLVD. BRIDGE OVER CSX RAILROAD	B1-31	
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									Table 2	- LRFR							
				Lc	pad Facto	ors	Мо	ment (Sti	rength) oi	r Stress (Service)		S	Shear (Stre	ength)		
Level	Limit State	Vehicle	Weight (tons)	LL	DC	DW	Distribution Factor (DF)	Rating Factor	Tons	Location	Dimension	Distribution Factor (DF)	Rating Factor	Tons	Location	Dimension	Comments: Interior/exterior beam DF method if other than LRFD. Other appropriate commen
	Strength I (Inv)	HL-93	N/A	1.75	1.25	1.50	0.72	1.31	N/A	"B"	61.70	0.78	1.77	N/A	"A"	21.41	Exterior Beam – Moment Interior Beam – Shear
Design Load Rating	Strength I (Op)	HL-93	N/A	1.35	1.25	1.50	0.72	1.69	N/A	"B"	61.70	0.78	2.29	N/A	"A"	21.41	Exterior Beam – Moment Interior Beam – Shear
_	Service III (Inv)	HL-93	N/A	0.80	1.00	1.00	0.72	1.16	N/A	"C"	62.96	N/A	N/A	N/A	N/A	N/A	Exterior Beam – Moment Interior Beam – Shear
Permit Load Rating	Strength II	FL120	60.0	1.35	1.25	1.50	0.72	1.50	90.28	"B	61.70	0.78	1.92	115.44	"A"	21.41	Exterior Beam – Moment Interior Beam – Shear

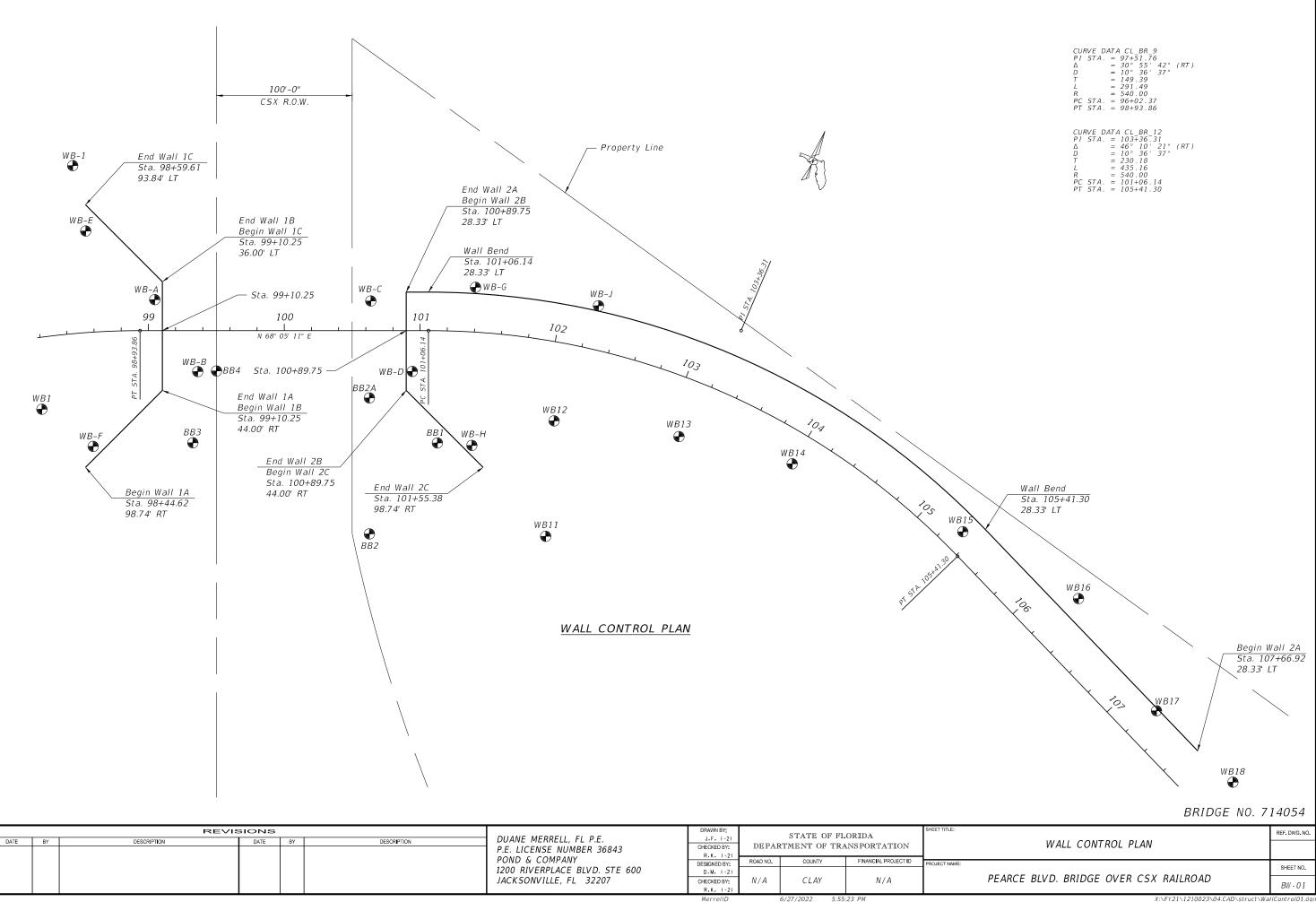


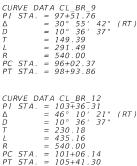
DATE BY DESCRIPTION DUANE MERELL, FL P.E. P.E. LICENSE NUMBER 36843 POND & COMPANY 1200 RIVERPLACE BLVD. STE 600 JACKSONVILLE, FL 32207 STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION SHEET TILE: LOAD RATING SUMMARY SHEET REF. DWG. NO. MA N.M21 N/M N/M CLAY N/A PONDER COMPT SHEET NO. B1-32					RATING LOCATIONS						BRIDGE NO.	714054	THE OFFI
1200 RIVERPLACE BLVD. STE 600 JACKSONVILLE, FL 32207 Designed by: Let blue blue blue blue blue blue blue blue	DATE	BY		DESCRIPTION	P.E. LICENSE NUMBER 36843	J.F. I-21 CHECKED BY:		RTMENT OF TRA	ANSPORTATION	SHEET TITLE:	LOAD RATING SUMMARY SHEET	REF. DWG. NO.	
MerrelID 6/27/2022 5:55:08 PM X:\FY21\1210023\04.CAD\struct\B1DataTableLoadRating01.DGN					1200 RIVERPLACE BLVD. STE 600	D.M. I-21 CHECKED BY:	N/A	CLAY		PROJECT NAME:		B1-32	

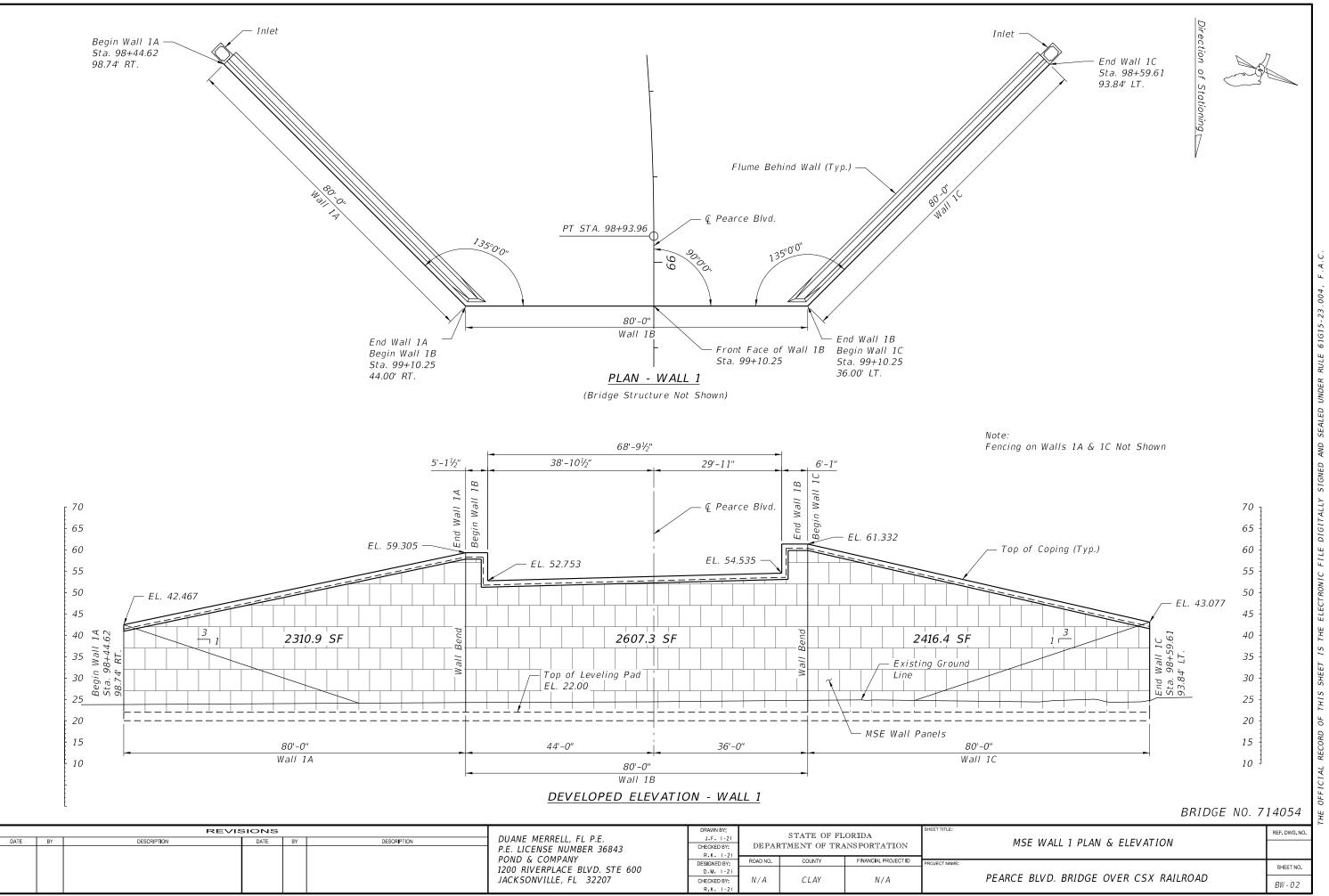




		REVIS	SIONS			COUNTYF
217	DATE	DESCRIPTION	DATE	DESCRIPTION	ECS FLORIDA LLC 11554 DAVIS CREEK COURT	S Cores
5-31					JACKSONVILLE, FL 32256	
					CERTIFICATE OF AUTHORIZATION 26152 CHRISTOPHER M. EGAN P.E. 79645	7
Ϋ́						OD WE T

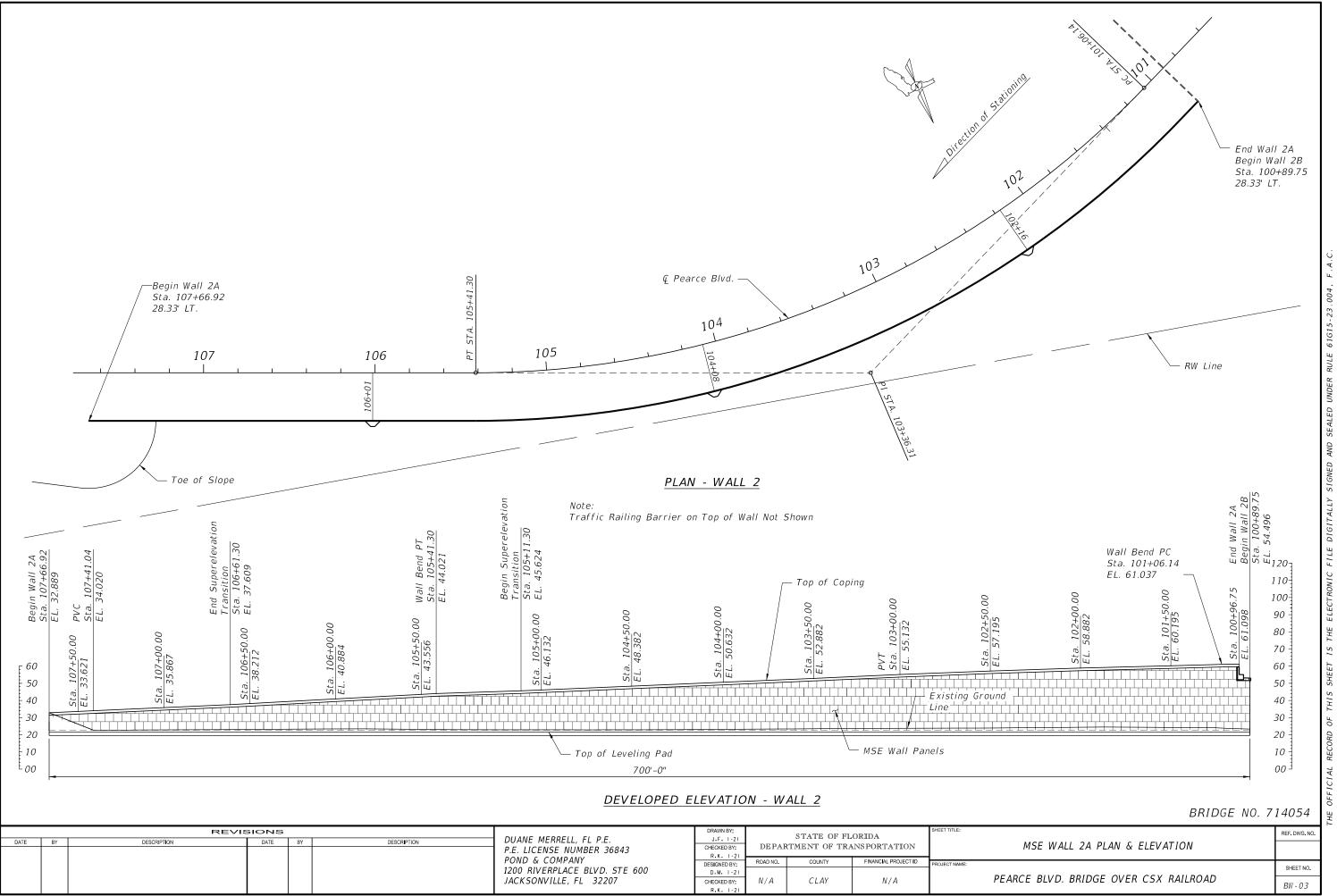




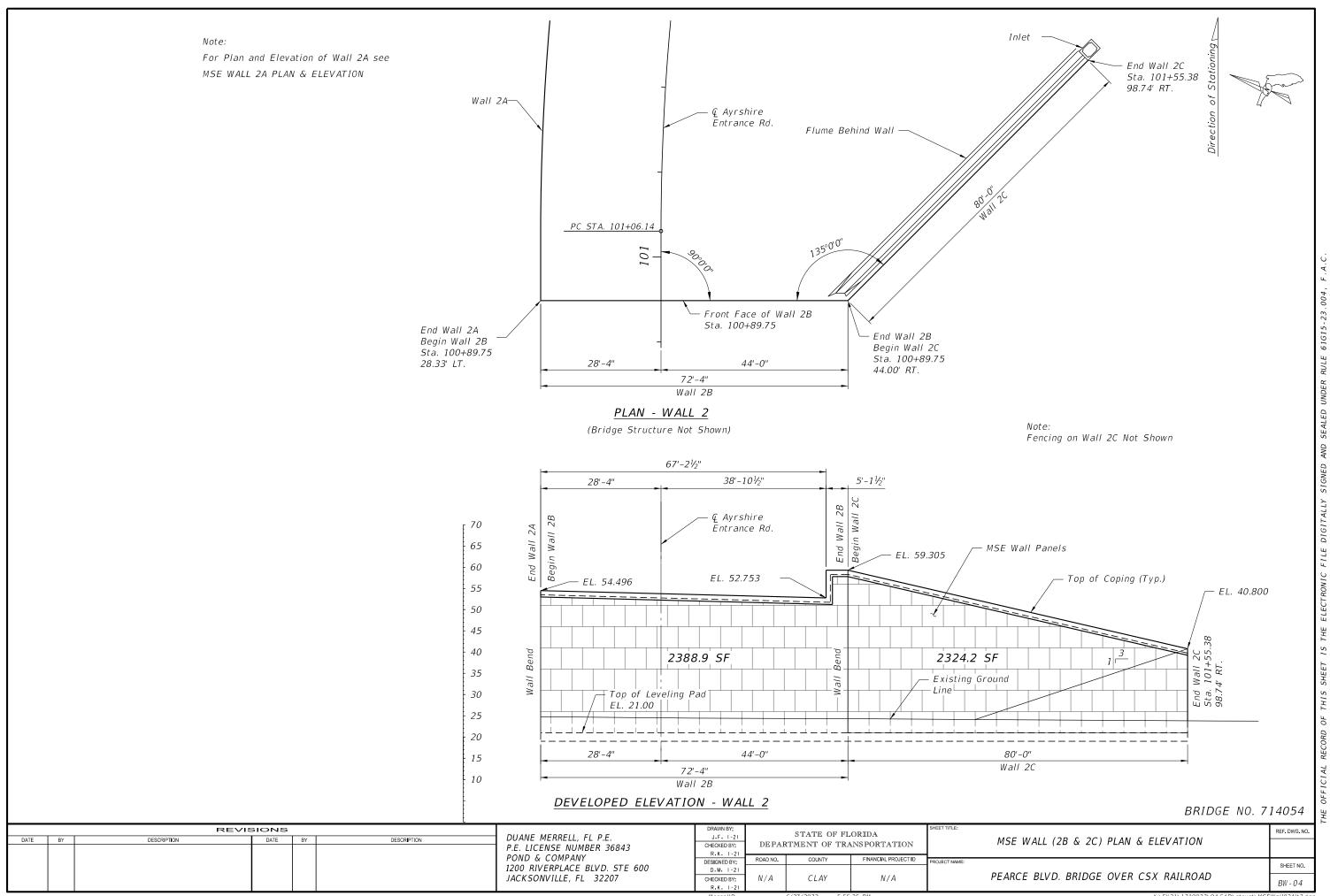


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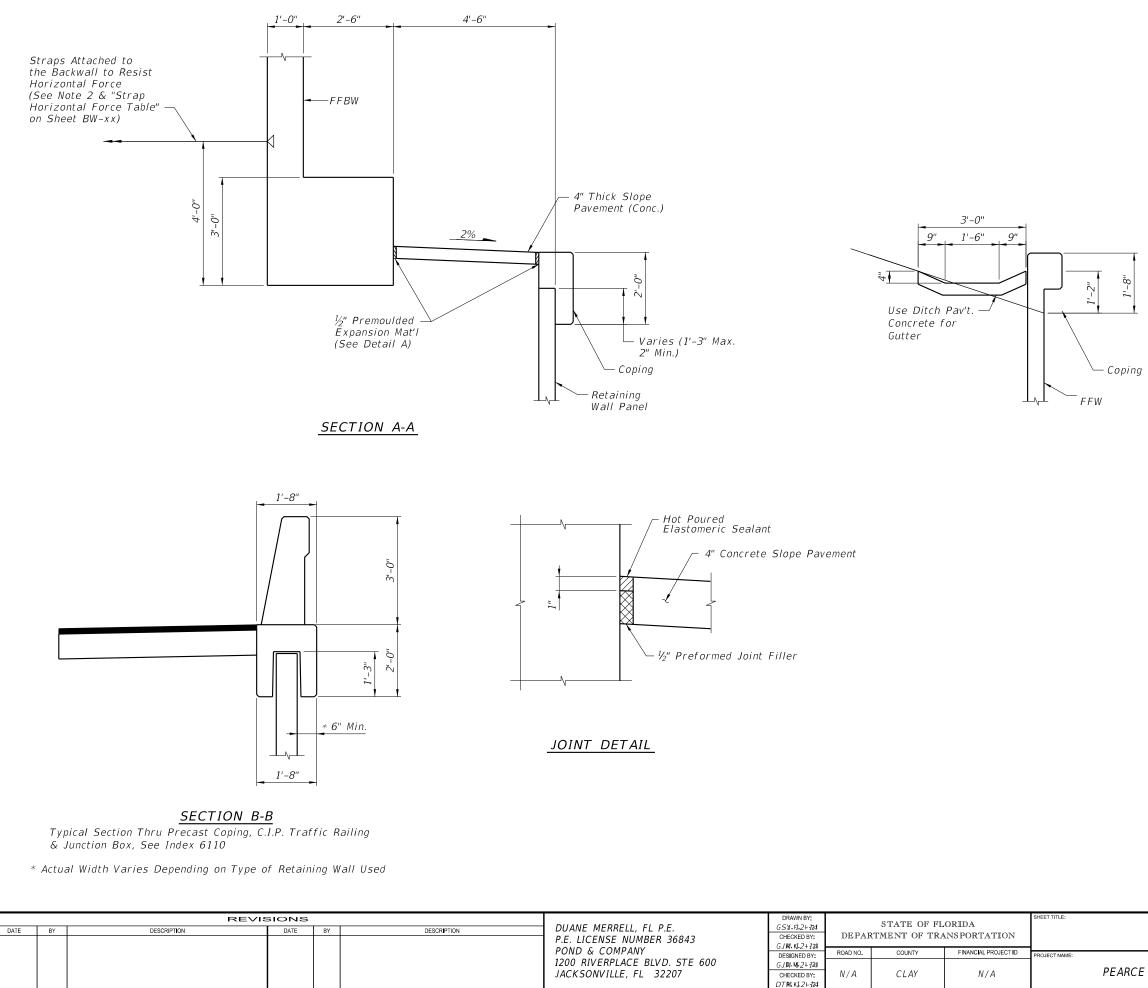


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BRIDGE NO. 7	14054	דחב י
	REF. DWG. NO.	
MSE WALL DETAILS		
	SHEET NO.	
BLVD. BRIDGE OVER CSX RAILROAD	BW - 05	
X:\FY21\1210023\04.CAD\struct\MSI	WallDet02.DGN	

A. IS THE ELECTRONIC FILE DIGITALLY SIGNED AND SEALED UNDER RULE 61G15-23.004, THIS SHEET RECORD OF ICI

		G	EOTECHNIC	CAL INFORM	IATION			Ta	ble Date 5-20-22
		Reinforced Soil & Random Backfill	Loose (N=4) Fine Sand	Firm (N=10) Fine Sand	Dense (N=15) Fine Sand	Hard (N=35) Fine Sand	Loose (N=5) Clayey Sand	Firm (N=10) Clayey Sand	Soft (WOH) Clay
Depth Below Existing	Wall No. 1		0	2, 67	22, 57	37, 77, 87	N/A	82	47
Ground Line (ft.)	Wall No. 2		12	0, 27	17	32, 57, 77, 87	72	47,82	N/A
Effective Unit	: Weight (pcf)	110	38	48	53	63	43	53	38
Cohesio	n (psf)	0	0	0	0	0	0	0	100
Internal Frid	ction Angle	30	28	30	31	37	26	28	0

NOTE:

If the unit weight and/or internal friction angle of the fill proposed by the Contractor differs from that shown above, the Project Engineer will contact both the District Geotechnical Engineer and the Wall Designer for a possible redesign.

		RETAINING	WALL VARIAE	BLES	Table Date 7-01-1.		
		Wall S	ettlement			Design High	
Wall No.	Long Term	Short Term	Differential	Settlement		Water Elevation	
Wan No.	Settlement (in.)	Settlement (in.)	Longitudinal (%) (ft./100ft.)	Transverse (in.)		(ft.)	
1	14.3	6.7	0.01	N/A		24	
2	11.5	8.9	0.0004	N/A		24	

NOTE:

Design walls for the settlements noted in the table.

Long term settlement is measured from the end of wall fill placement.

Transverse differential settlement is measured from the face of wall to

the end of the soil reinforcement.

	SOIL REII	VFORC	EMENT	LENG	THS FC	DR EXT	ERNAL	STABI	LITY	Table Date	1-01-11
. 1	Wall Height (ft.)	5	10	15	20	25	30	35	37		
all No.	Reinforcement Length (ft.)	7	14	20	27	34	40	46	47		
Ŵâ	Factored Bearing Resistance (psf)	2860	4342	5554	7029	8503	9720	10937	11063		
2	Wall Height (ft.)	5	10	15	20	25	30	35	37		
all No.	Reinforcement Length (ft.)	6	14	20	24	29	37	45	46		
Wa	Factored Bearing Resistance (psf)	2569	4342	5554	6240	7193	8941	10679	10871		

NOTES:

1. The reinforcement strap lengths shown above are the minimum lengths required for external stability. The reinforcement lengths used in the construction of the retaining walls will be the longer of that required for external or internal stability (determined by proprietary wall companies).

2. The Factored Bearing Resistances shown above are the critical (lowest) values from all the load cases analyzed using LRFD methodology.

		 			1	DRAWN BY:				SHEET TITLE:	
DATE	BY	DATE	BY	DESCRIPTION	DUANE MERRELL, FL P.E. P.E. LICENSE NUMBER 36843	J.F. I-21 CHECKED BY:	DEPAI	STATE OF I RTMENT OF T	LORIDA RANSPORTATION	one of the	
					POND & COMPANY	R.K. I-21 DESIGNED BY:	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	PROJECT NAME:	
					1200 RIVERPLACE BLVD. STE 600 JACKSONVILLE, FL 32207	D.M. 1-21 CHECKED BY: R.K. 1-21	N/A	CLAY	N/A		PEARCE
						MerrellD		6/27/2022 5:	55:27 PM		

Location

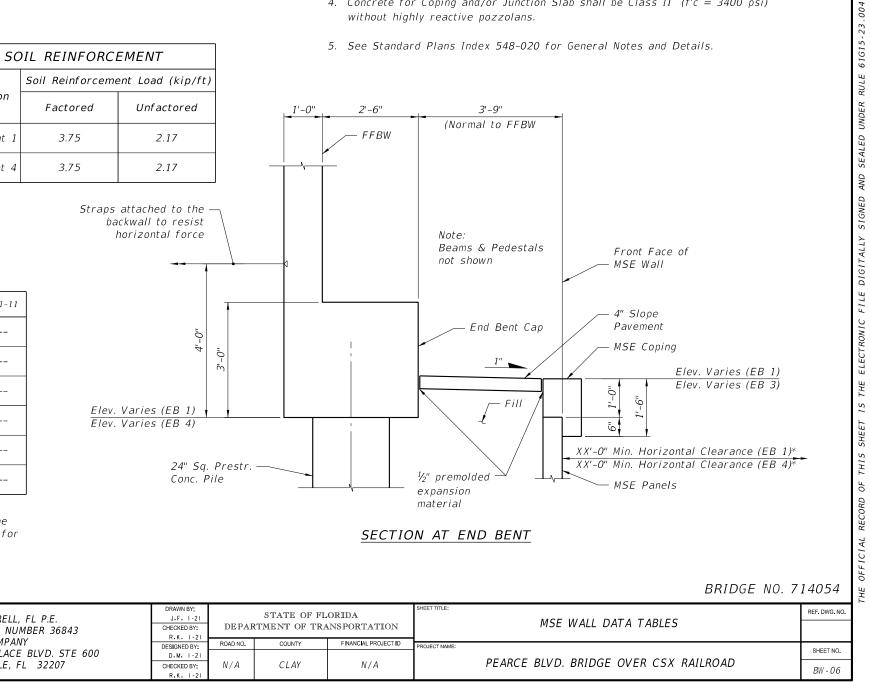
End Bent 1

End Bent 4

PERMANENT MSE RETAINING WALL SYSTEM DATA TABLES

NOTES [Notes Date 09-01-19]:

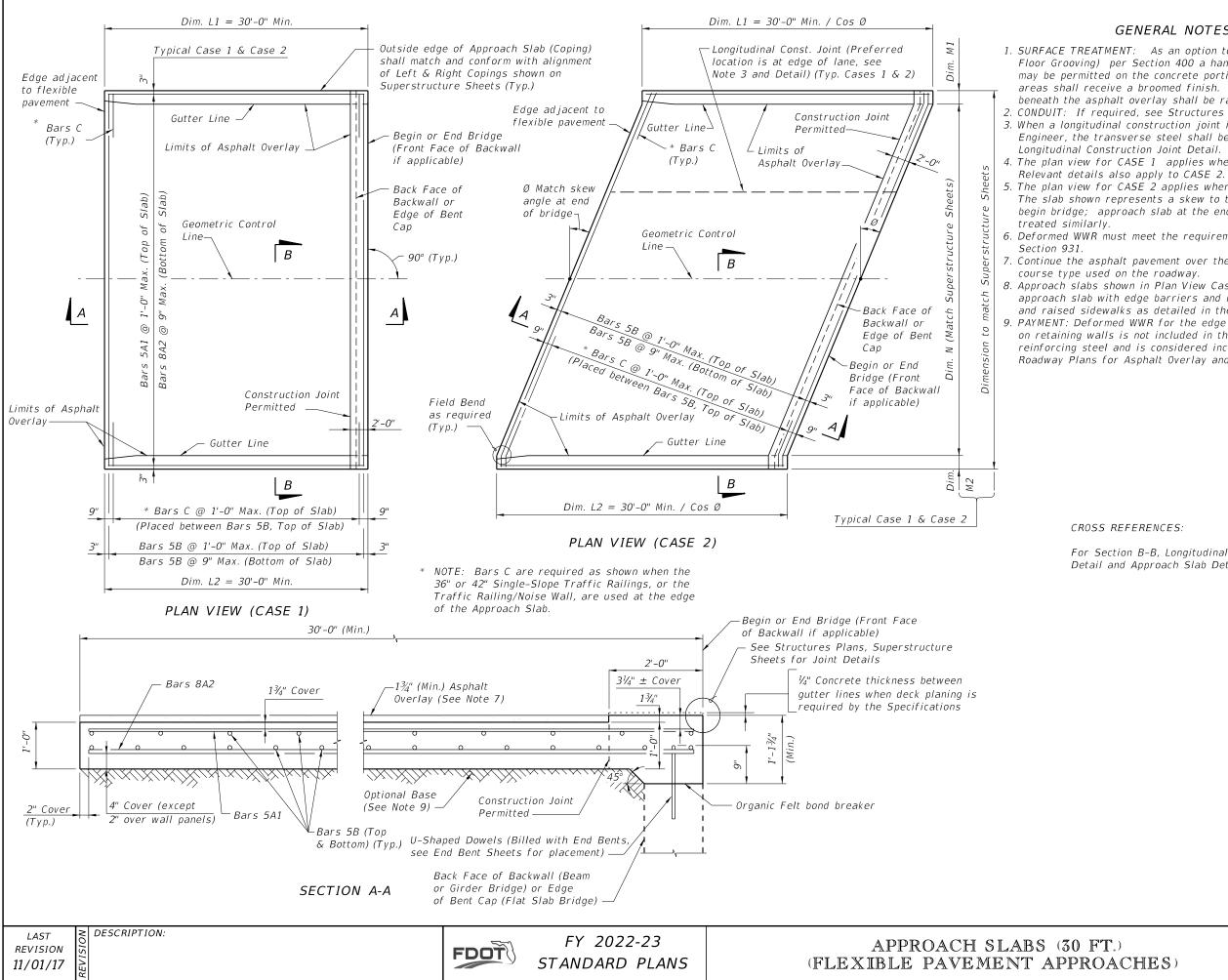
- 1. Concrete facing panel surfaces treatment will be Smooth Class II Finish.
- Plans Index 548-020 for allowable Wall Type substitutions. Wall No. 1 - FDOT Wall Type 2A Wall No. 2 - FDOT Wall Type 2A
- without highly reactive pozzolans.



2. If required, the soil reinforcement and fasteners for the abutment back wall will be designed and furnished by the proprietary wall company. The soil reinforcement will be designed to resist a factored horizontal load of 3.75 kips/ft. of back wall width. The cost of soil reinforcement and fasteners (if required) will be included in the cost of the Retaining Wall System.

3. Applicable FDOT Wall Types for each wall location are listed below. See the Approved Products List for approved Wall Systems and Standard

4. Concrete for Coping and/or Junction Slab shall be Class II (f'c = 3400 psi)



GENERAL NOTES

1. SURFACE TREATMENT: As an option to Class 4 Floor Finish (Bridge Floor Grooving) per Section 400 a hand tined or heavy broomed finish may be permitted on the concrete portion of the riding surface. Sidewalk areas shall receive a broomed finish. The top surface of the concrete beneath the asphalt overlay shall be raked.

2. CONDUIT: If required, see Structures Plans for Conduit Details. 3. When a longitudinal construction joint is necessary or allowed by the Engineer, the transverse steel shall be extended as shown in the

4. The plan view for CASE 1 applies when the skew angle $(\emptyset) = 0^{\circ}$.

5. The plan view for CASE 2 applies where the skew angle (\emptyset) is > 0°. The slab shown represents a skew to the right for an approach slab at begin bridge; approach slab at the end of bridge or a left skew shall be

6. Deformed WWR must meet the requirements of Specification

7. Continue the asphalt pavement over the approach slab and match the friction

8. Approach slabs shown in Plan View Cases 1 and 2 represent a typical approach slab with edge barriers and no sidewalks. Provide railings, parapets and raised sidewalks as detailed in the Contract Plans.

9. PAYMENT: Deformed WWR for the edge of Approach Slabs

on retaining walls is not included in the estimated quantity for

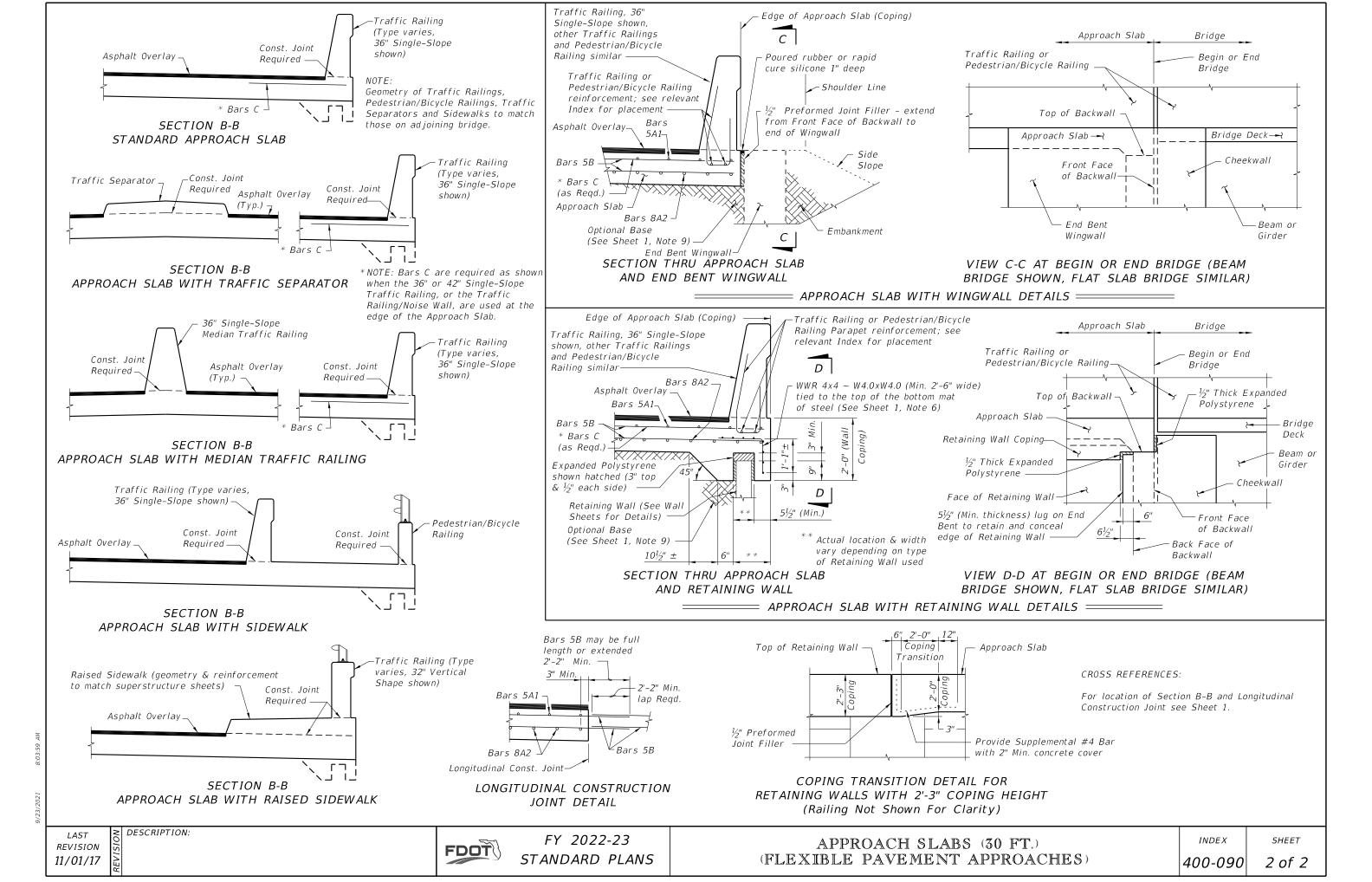
reinforcing steel and is considered incidental to the work. See

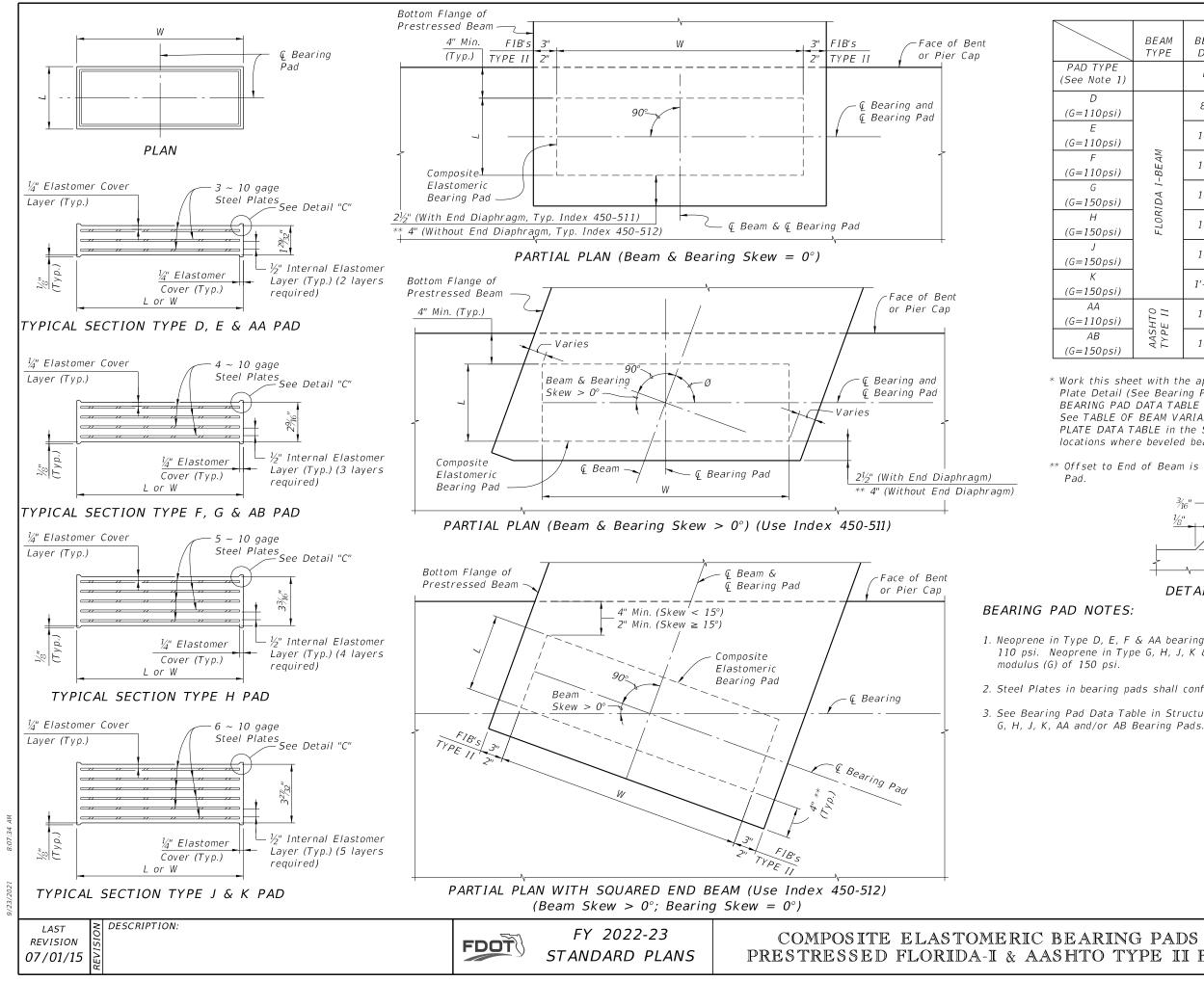
Roadway Plans for Asphalt Overlay and Optional Base details and quantities.

CROSS REFERENCES:

For Section B-B, Longitudinal Construction Joint Detail and Approach Slab Details see Sheet 2.

	INDEX	SHEET
CHES)	400-090	1 of 2

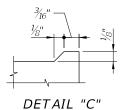




/	BEAM TYPE	BEARIN DIMEN	IG PAD ISIONS	*BEVELED BEARING PLATE DIMENSIONS		
)		L	W	С	D	
		8"	2'-8"	1'-0''	3'-0''	
	_	10"	2'-8"	1'-0"	3'-0''	
	FLORIDA I-BEAM	10"	2'-8"	1'-0"	3'-0''	
	IDA I-	10"	2'-8"	1'-0''	3'-0''	
	FLOR	10"	2'-8"	1'-0"	3'-0''	
		10"	2'-8"	1'-0"	3'-0''	
		1'-0''	2'-8"	1'-1½"	3'-0''	
	AASHTO TYPE II	10"	1'-2"	1'-0"	1'-4''	
	AAS TYP	10"	1'-2"	1'-0"	1'-4''	

* Work this sheet with the appropriate type Bearing Plate Detail (See Bearing Plate Data Table) and BEARING PAD DATA TABLE in the Structures Plans. See TABLE OF BEAM VARIABLES and BEARING PLATE DATA TABLE in the Structures Plans for locations where beveled bearing plates are required.

** Offset to End of Beam is reduced to 2" for Type K

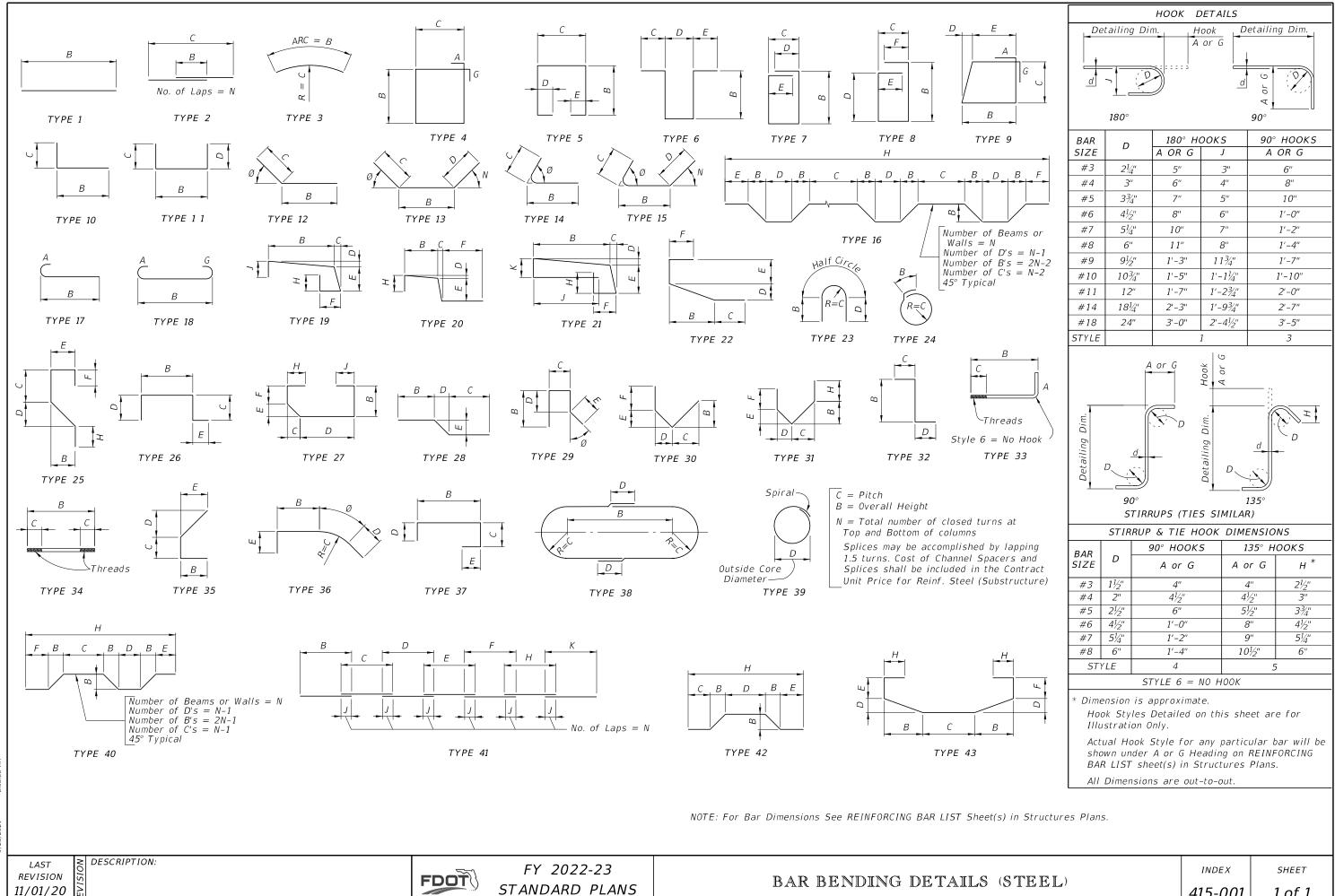


1. Neoprene in Type D, E, F & AA bearing pads shall have a shear modulus (G) of 110 psi. Neoprene in Type G, H, J, K & AB bearing pads shall have a shear

2. Steel Plates in bearing pads shall conform to ASTM A1011 Grade 36, Type 1.

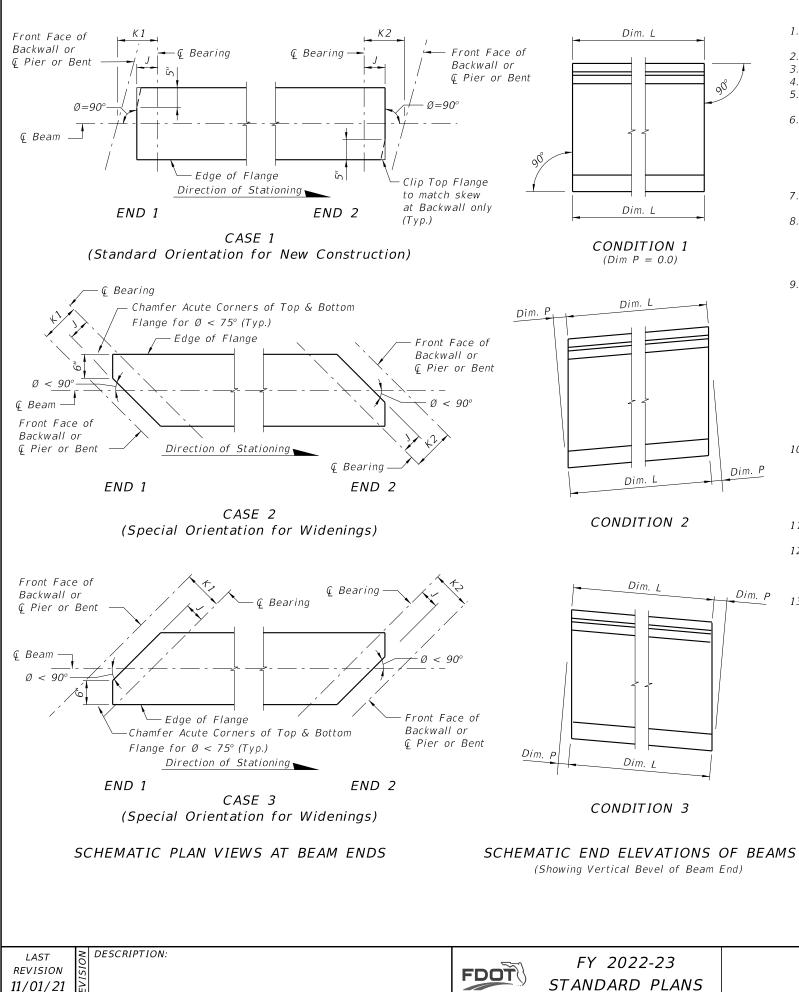
3. See Bearing Pad Data Table in Structures Plans for quantities of Type D, E, F,

IG PADS -	INDEX	SHEET
YPE II BEAM	400-510	1 of 1



415-001

1 of 1



BEAM NOTES

- 1. Work this Index with the Florida-I Beam Standard Details (Index 450-036 thru 450-096) and the Table of Beam Variables in Structures Plans.
- 2. All bar bend dimensions are out-to-out. 3. Concrete cover: 2 inches minimum
- 4. Stress Strands N to 10 kips each.
- 5. Place one (1) Bar 5K or 5Z at each location. Alternate the direction of the ends for each bar (see "ELEVATION AT END OF BEAM" in Standard Details.
- 6. Tie Bars 5K and 5Z to the fully bonded strands in the bottom or center row (see "STRAND PATTERN" on the Table of Beam Variables sheet in Structures Plans). A. At the Contractor's option, the length of the bottom legs of Bars 5K and 5Z may be
 - extended to facilitate tying to the exterior strands. Β. For deformed WWR, supplemental transverse #4 bars are permitted to support Pieces
 - K & S under the cross wires on the bottom row of strands.
- 7. Place Bars 3C1, 3D1 and 4M1 in beam END 1, and Bars 3C2, 3D2 and 4M2 in beam END 2. END 1 and END 2 are shown on the Standard Details "ELEVATION".
- 8. For Beams with vertically beveled end conditions: Place first row of Bars 3C1, 3C2, 3D1, 3D2, 5K, 5Y and 5Z parallel to the end of the beam. Progressively rotate remaining bars within the limits of Bars 5Z until vertical by adjusting the spacing at the top of beam up to a maximum of 1". For deformed WWR, cut top cross wire and rotate bars as required or reduce end cover at top of the beam to 1" minimum.
- 9. For beams with skewed end conditions:
 - A. Place end reinforcement parallel to the skewed end of the beam. End reinforcement is defined as Bars 3C1, 3C2, 3D1, 3D2, 5K, 4M1, 4M2, 5Y and 5Z placed within the limits of the spacing for Bars 3C in "ELEVATION AT END OF BEAM".
 - B. Beyond the limits of the spacing for Bars 3C, place Bars 3D3, 5K and 4M3 perpendicular to the longitudinal axis of the beam. Fan Bars as needed to avoid overlapping bars at the transition to Bars 3D3 and 4M3, and field cut to maintain minimum cover. Provide additional Bars 4M1, 4M2, 3D1 and 3D2 as required; additional bars are not included in the "BILL OF REINFORCING STEEL". For placement locations see Skewed Beam End Details for Widening Existing Bridges.
 - C. Adjust the dimensions of Bars 3C1, 3C2, 3D1, 3D2, 4M1 and 4M2 as shown on the Bending Diagram.

D. WWR is not permitted for end reinforcement Bars 3D1, 3D2, 4M1 and 4M2; use bar reinforcement. 10. Contractor Options:

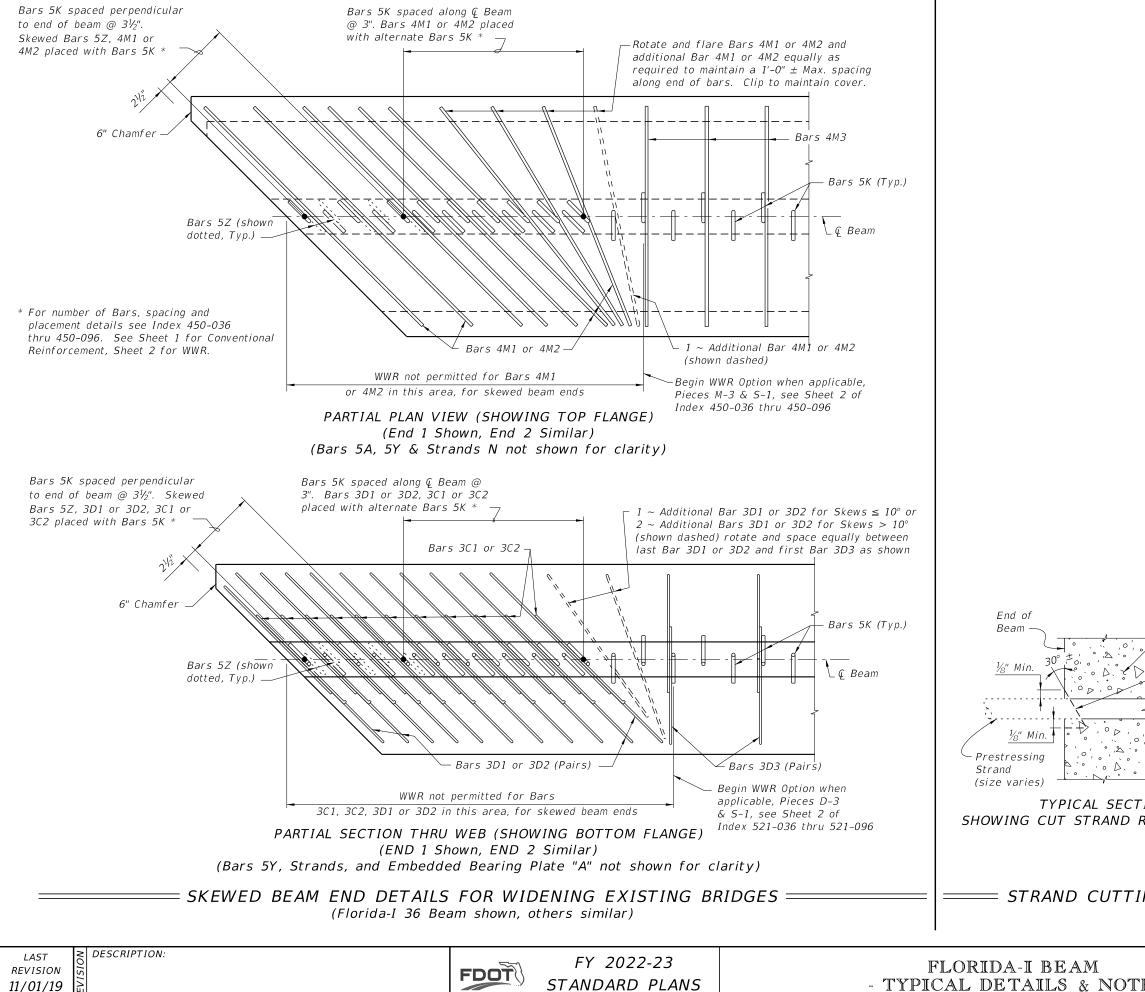
- A. Deformed WWR may be used in lieu of Bars 3D, 5K, 4M, and 5Z as shown on the Standard Details; except at skewed ends (see Note 9).
- B. Bars 3D1, 3D2 and 3D3 may be fabricated as a single bar with a 1'-0" minimum lap splice of the top legs, or the length of the bottom legs may be extended to facilitate tying to the exterior strands.
- 11. Embedment of Safety Line Anchorage Devices are permitted in the top flange to accommodate fall protection systems. See shop drawings for details and spacing of any required anchorage devices.
- 12. For beams with ends that will not be permanently encased in concrete diaphragms, cut wedges and recess Prestressing strands at the end of the beam without damaging the surrounding concrete. See "STRAND CUTTING AND PROTECTING DETAIL" on Sheet 2. Protect end of wedged recessed strands in accordance with Specification Section 450.
- 13. Holes in the beam web for temporary bracing or shipping devices must be formed prior to casting. Fill holes not meeting all the following criteria in accordance with Specification Section 450.
 - A. The superstructure environmental classification is slightly or moderately aggressive
- В. Clear cover to adjacent steel reinforcing is 1"or greater
- С. Hole inside diameter is 2" maximum
- Non-metallic, non-water absorbing forming materials such as PVC, D may be left in place permanently.



STANDARD PLANS

FLORIDA-I BEAM - TYPICAL DETAILS & NOT

	INDEX	SHEET
ES	450-010	1 of 2



	\ Beam	
FIB		FIB
Strand Recess (formed by cutting or grinding)		
-		Recessed
	TYPICAL SE FTER PROT	
NG AND PROTECT	ING DET.	AIL =====
	INDEX	SHEET
ES	450-010	2 of 2

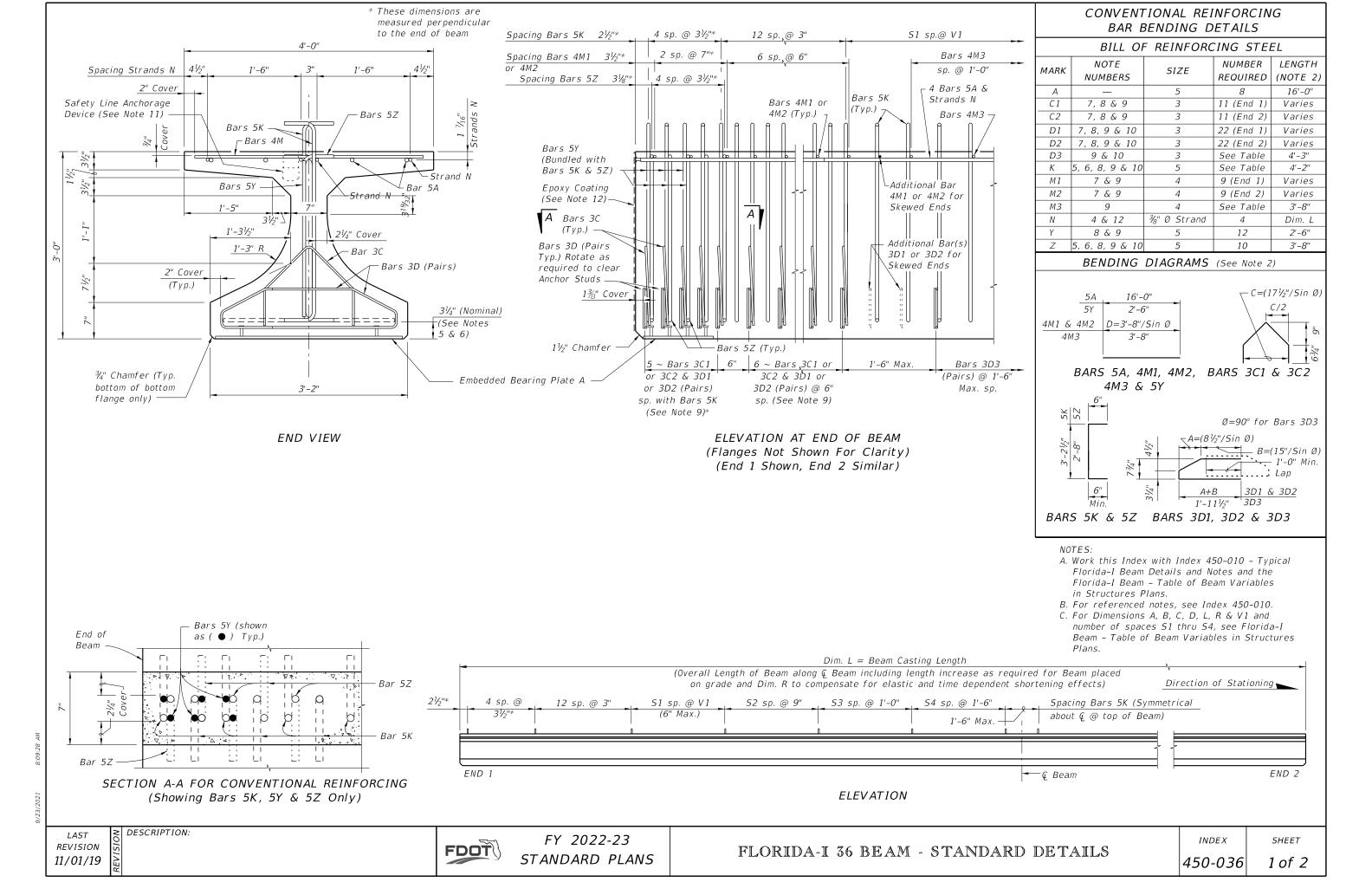
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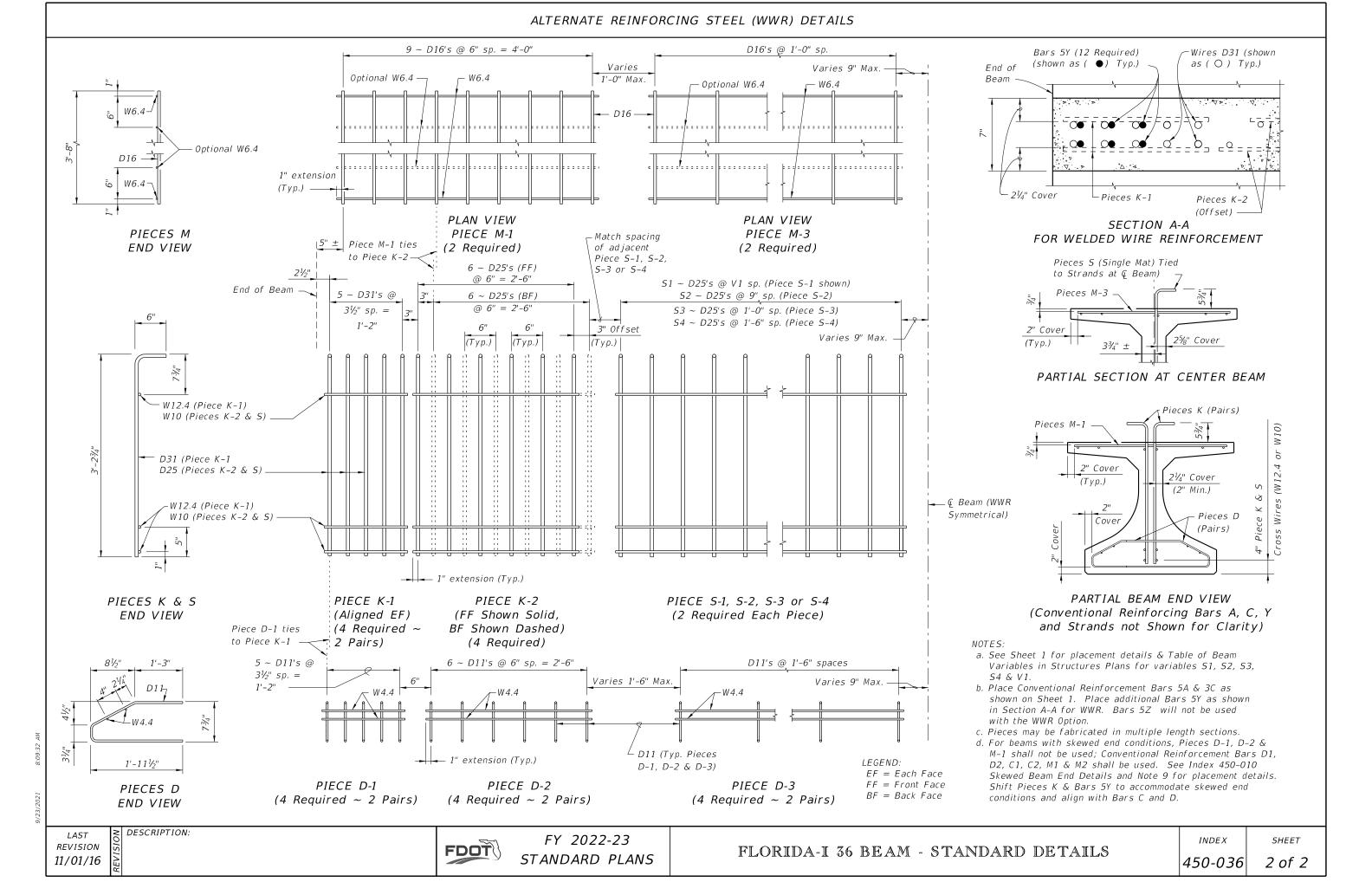
Epoxy Coating

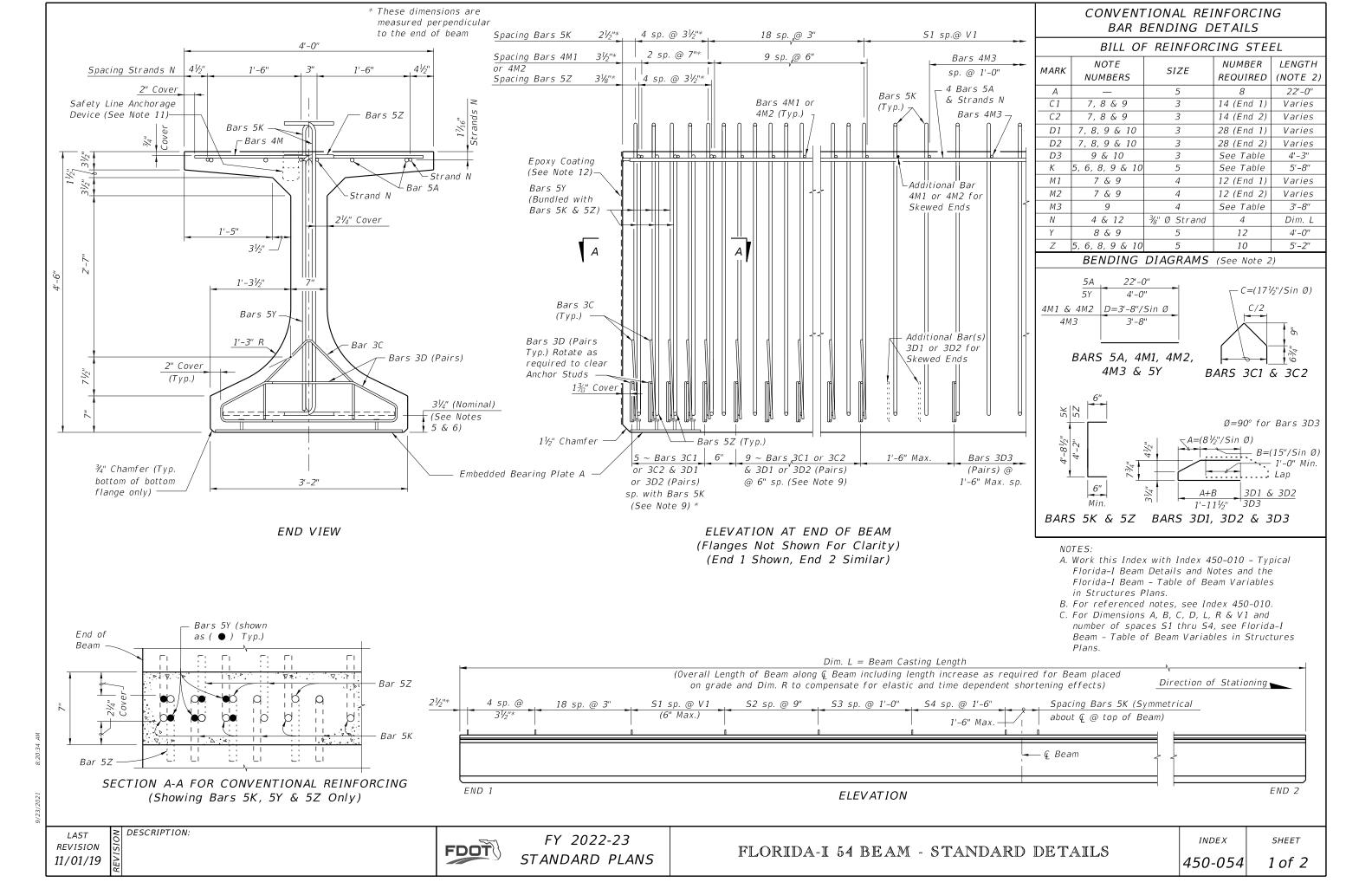
(See Note 12,

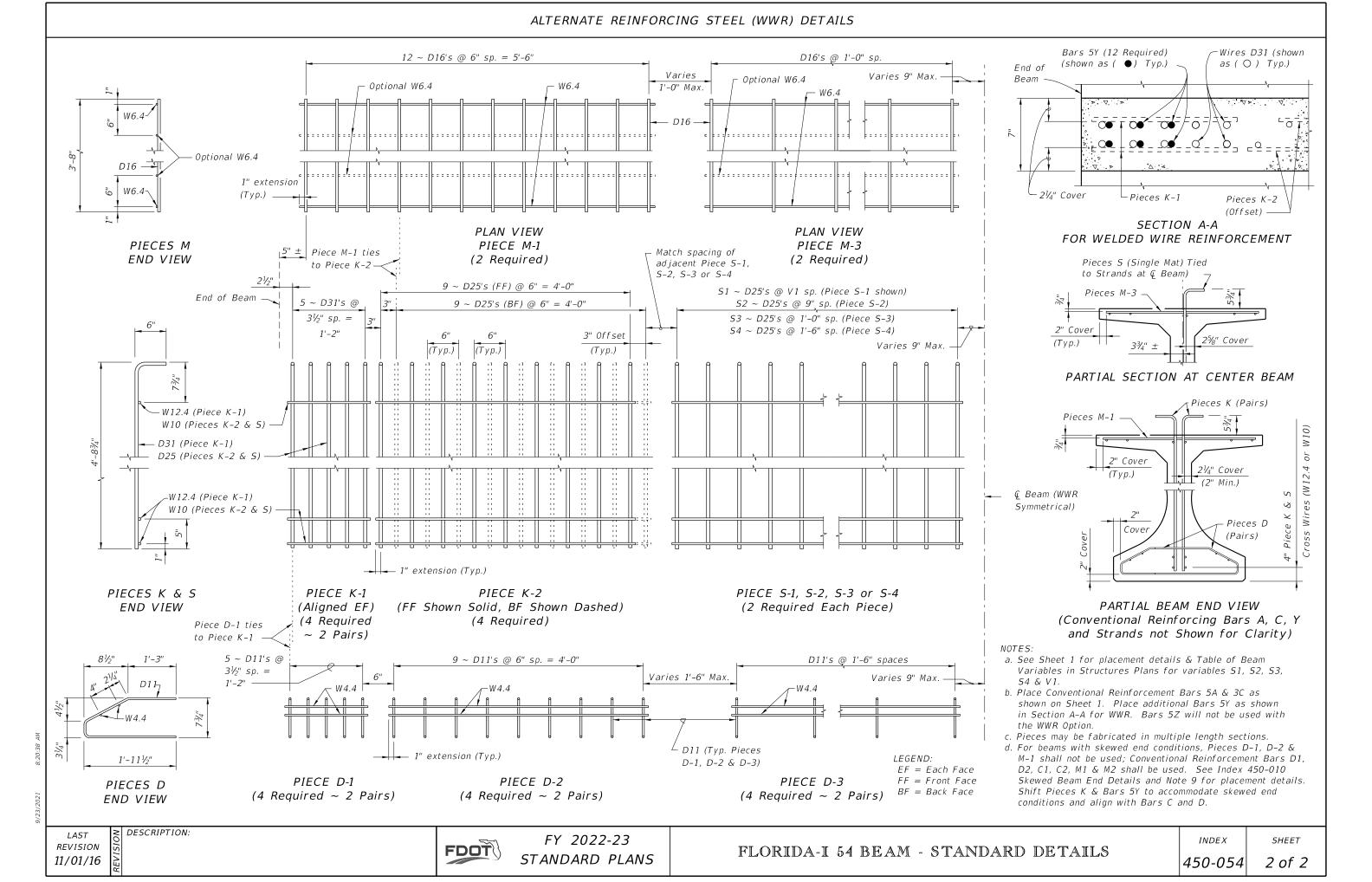
Sheet 1) —

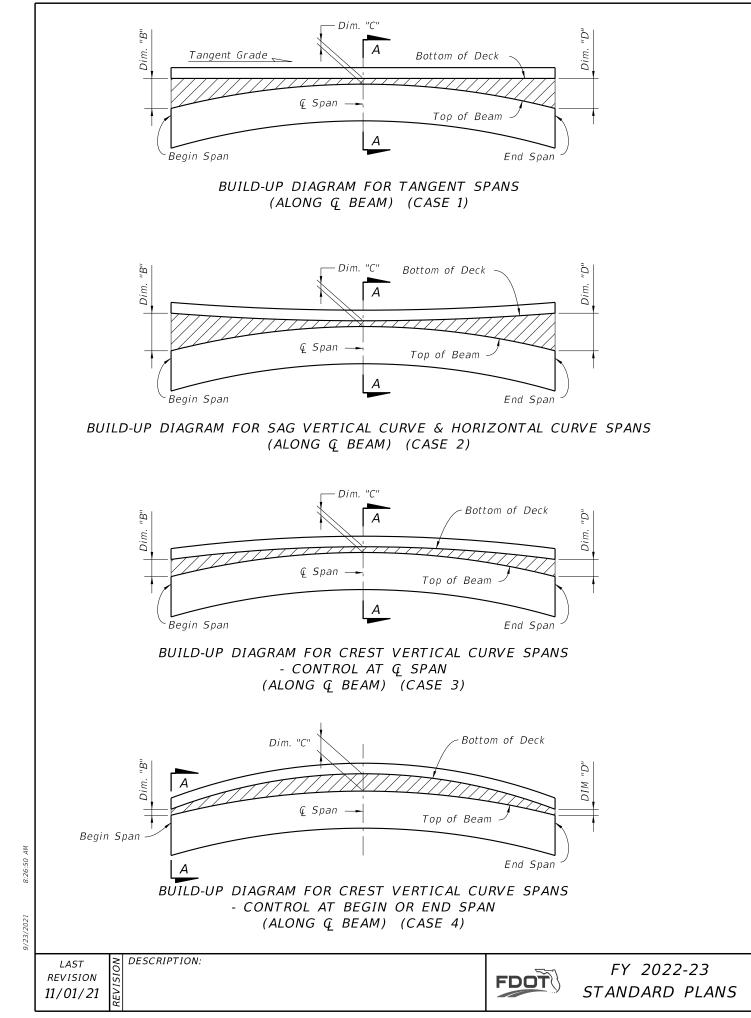
 $\binom{1}{16}$ " minimum thickness)







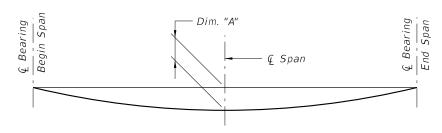




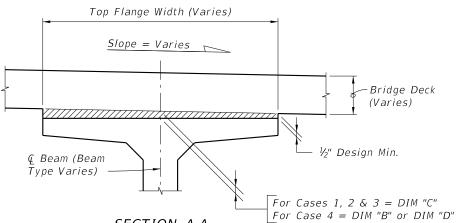
BEAM CAMBER AND BUILD-UP NOTES:

The build-up values given in the Data Table* are based on theoretical beam cambers. The Contractor shall monitor beam cambers for the purpose of predicting camber values at the time of the deck pour. If the predicted cambers based on field measurements differ more than +/- 1" from the theoretical "Net Beam Camber @ 120 Days" shown in the Data Table*, obtain approval from the Engineer to modify the build-up dimensions as required. When the measured beam cambers create a conflict with the bottom mat of deck steel, notify the Engineer a minimum of 21 days prior to casting.

Dim. "A" includes the weight of the Stay-In-Place Formwork.





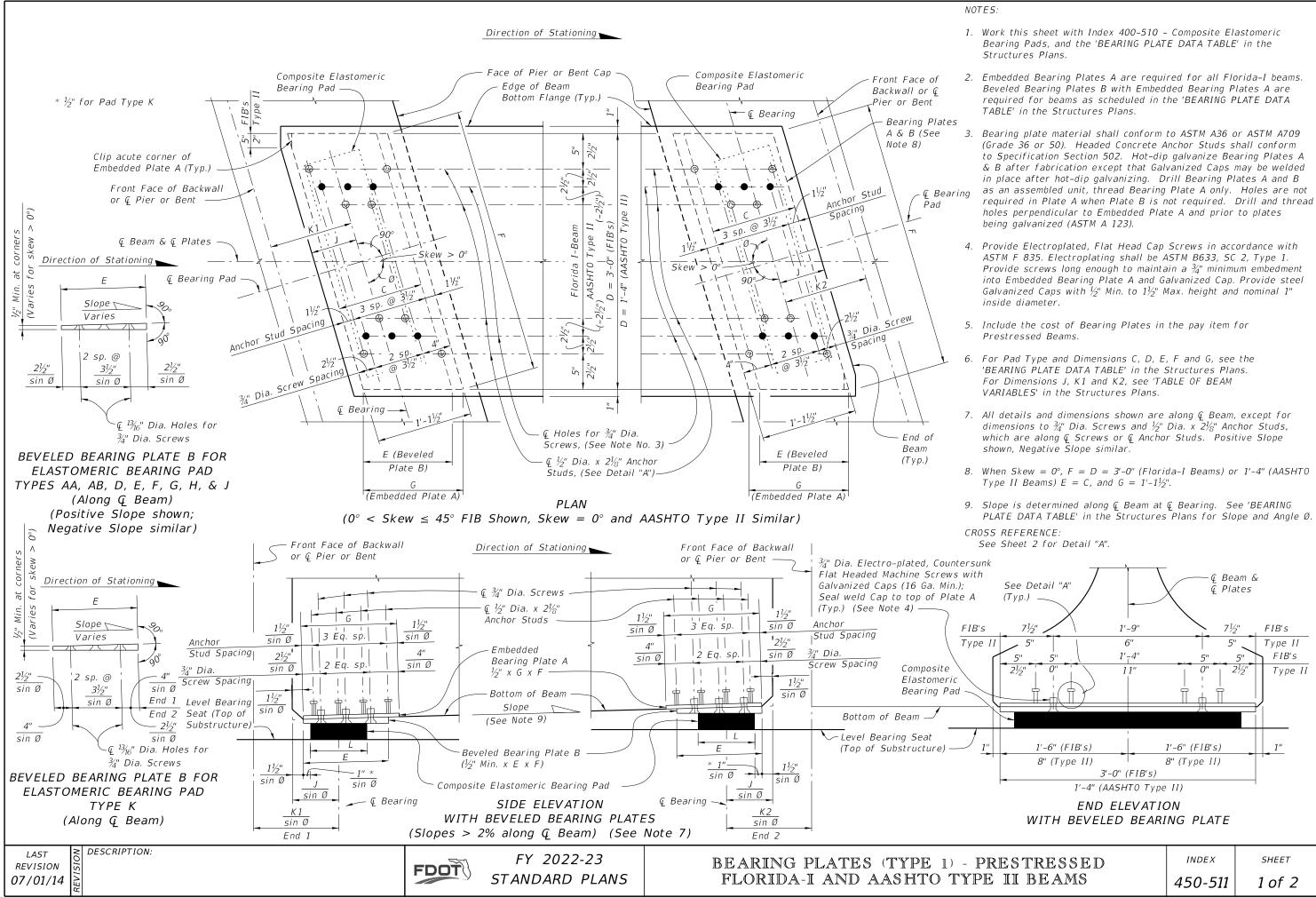


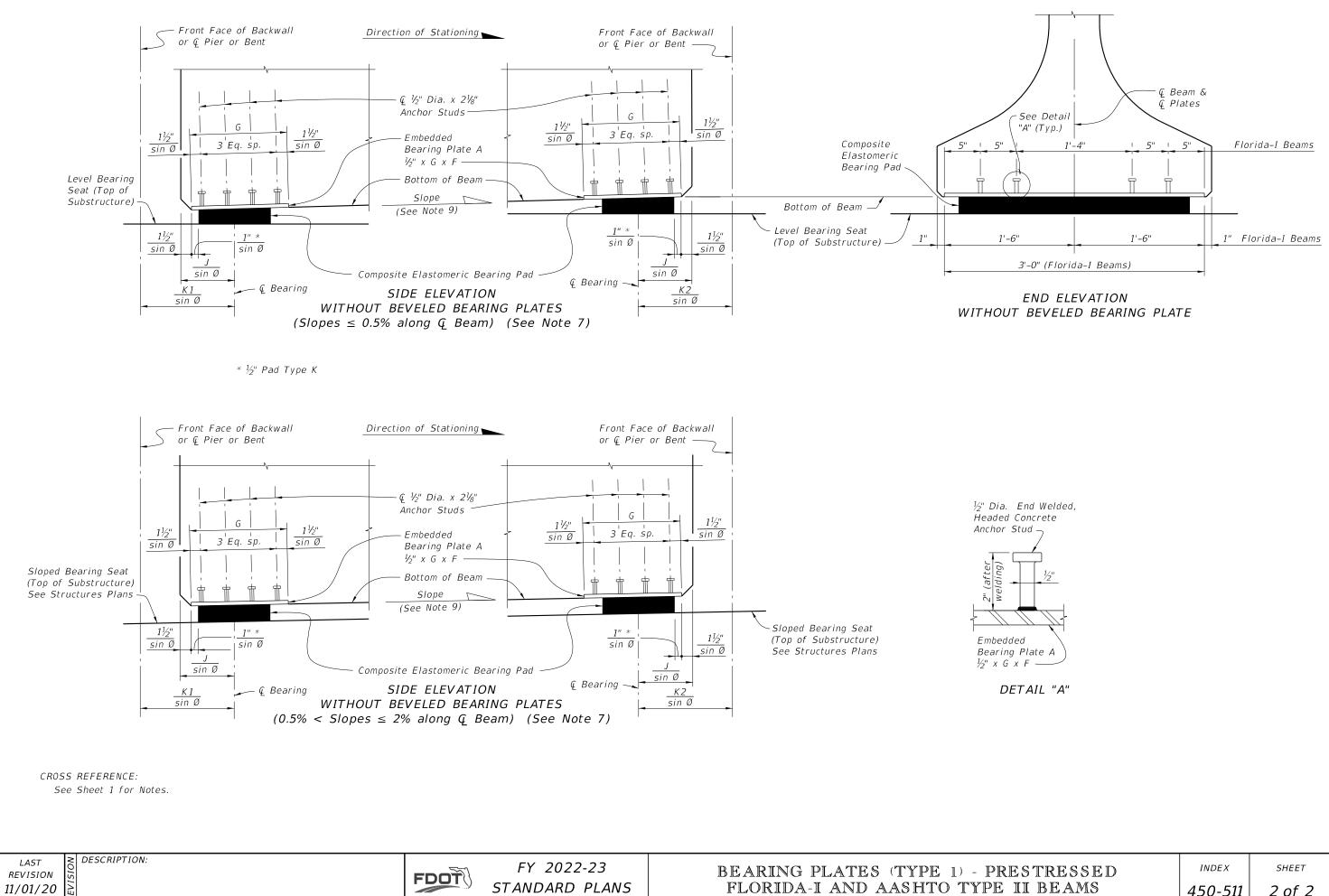
SECTION A-A BUILD-UP OVER BEAMS (Florida-I Beam Shown AASHTO Type II Similar)

* NOTE: Work this Index with the Build-up and Deflection Data Table for Florida-I and AASHTO Type II Beams in Structures Plans.

PRESTRESSED I-BEAMS BUILD-UP & DEFLECTION DA

	INDEX	SHEET
ТА	450-199	1 of 1





TRESSED	INDEX	SHEET
BEAMS	450-511	2 of 2

PRESTRESSED CONCRETE PILE NOTES:

- the Structures Plans.
- 2. Concrete:
- Α. (Index 455-031).
- Β. High Capacity Splice Collar: Class V (Special). See "GENERAL NOTES" in the Structures Plans for locations where C.
- the use of Highly Reactive Pozzolans is required. 3. Concrete strength at time of prestress transfer:
- Piles: 4,000 psi minimum. Α. B. High Moment Capacity Piles: 6,500 psi minimum.
- 4. Carbon-Steel Reinforcing:
 - Α. Bars: Meet the requirements of Specification Section 415.
 - Β.
- С. under final conditions in accordance with Specification Section 450.
- 5. Spiral Ties:
- A. Tie each wrap of the spiral strand to a minimum of two corner strands.
- B. One full turn required for spiral splices. 6. Pile Splices: Fill dowel holes and form the joint between pile sections with a Type AB Compound or an Epoxy Mortar as recommended by the Manufacturer.

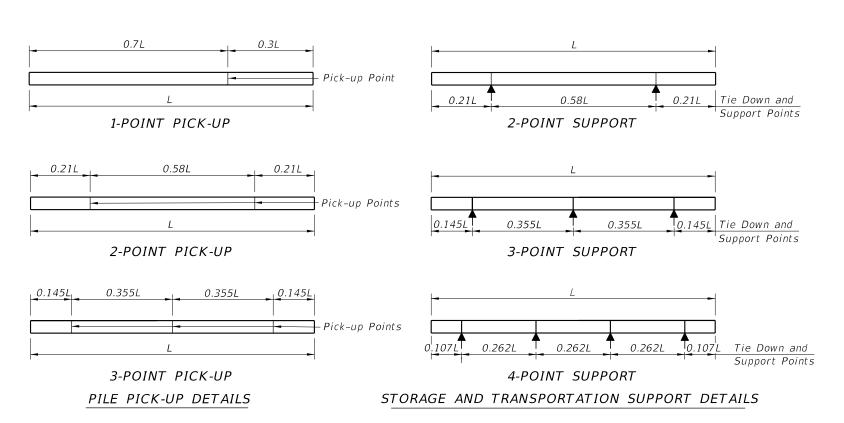
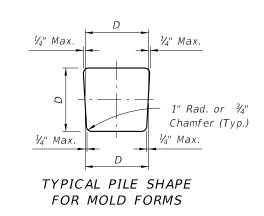
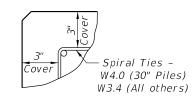


	TABLE OF MAXIMUM PILE PICK-UP AND SUPPORT LENGTHS								
D = Square Pile Size (inches) Required Storage and Disk Up Date							Dick Up Datail		
	\frown	12	14	18			Pick-Up Detail		
	Maximum	48	52	59	68	87	2, 3, or 4 point	1 Point	
	Pile Length	69	75	85	98	124	2, 3, or 4 point	2 Point	
	(Feet)	99	107	121	140	178	3 or 4 point	3 Point	





DETAIL SHOWING TYPICAL COVER

LAST REVISION 11/01/20

DESCRIPTION:



FY 2022-23 STANDARD PLANS

SQUARE PRESTRESSED CONCRET - TYPICAL DETAILS & NOTI

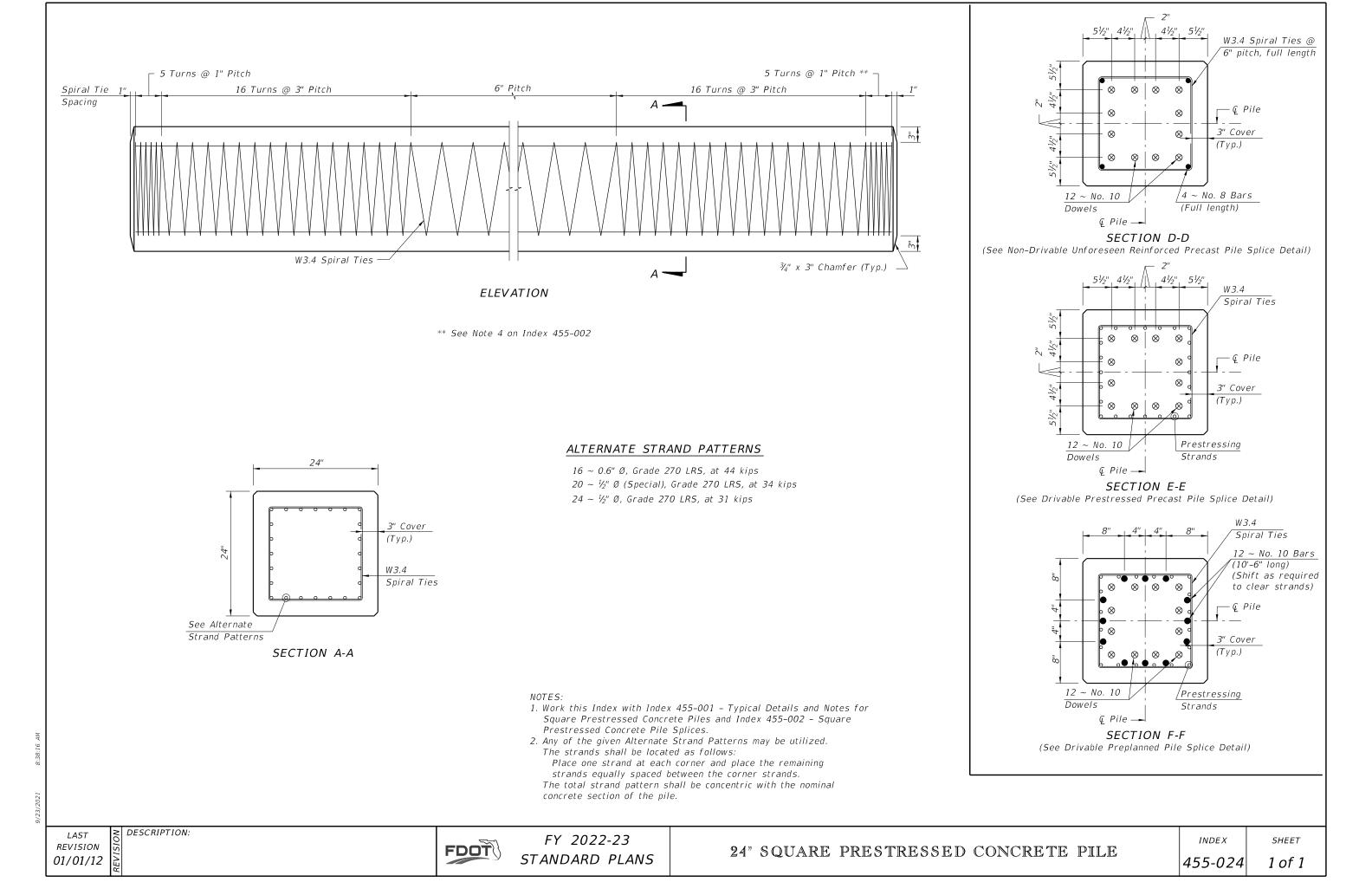
1. Work this Index with the Square Prestressed Concrete Pile Splices (Index 455-002), the Prestressed Concrete Pile Standards (Index 455-012 thru 455-030), the High Moment Capacity Square Prestressed Concrete Pile (Index 455-031) and the Pile Data Table in

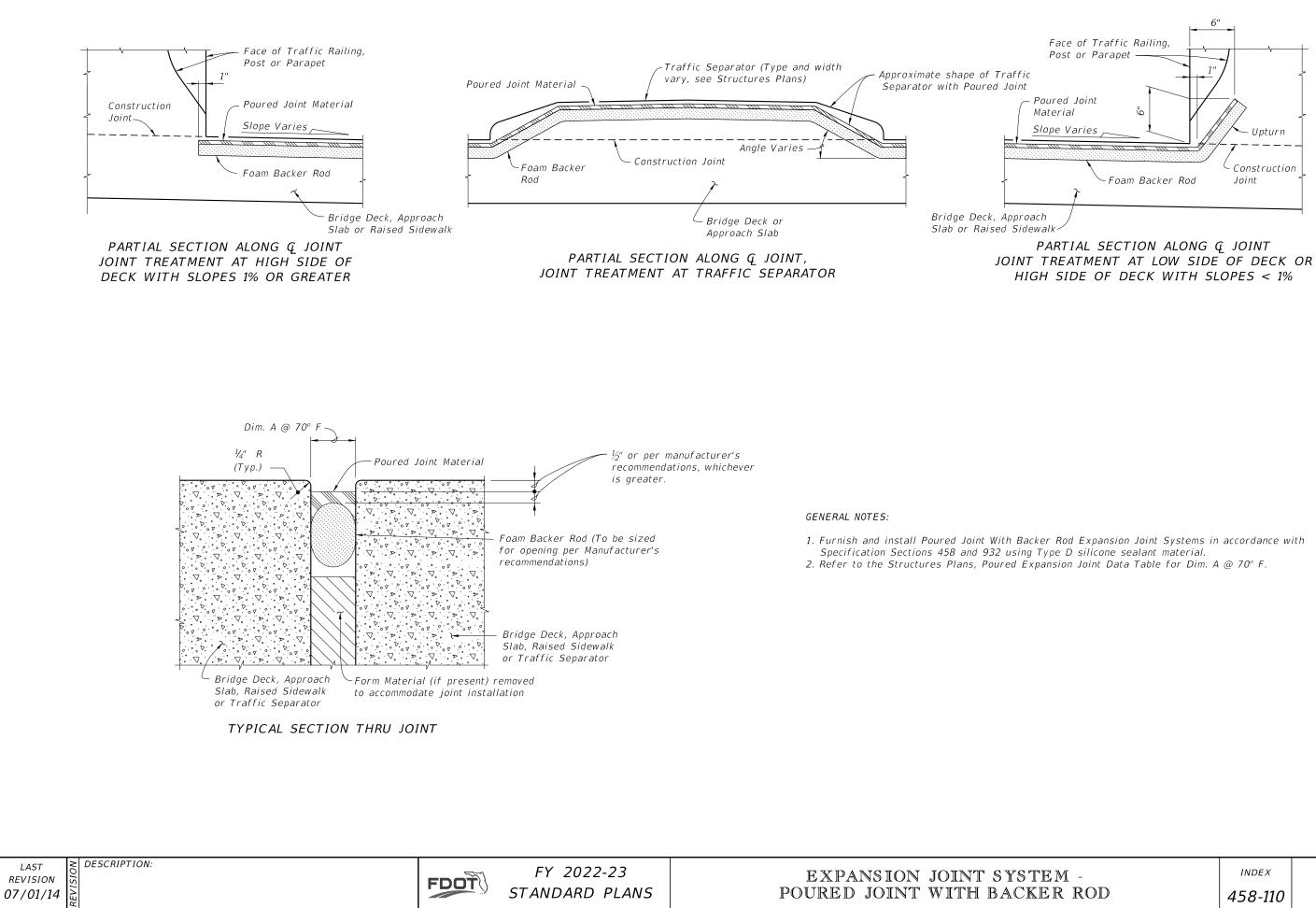
Piles: Class V (Special), except use Class VI for High Moment Capacity Pile

Prestressing Strands: Meet the requirements of Specification Section 933. Protect all strands permanently exposed to the environment and not embedded

Epoxy Compound in accordance with Specification Section 962. Use an Epoxy Bonding

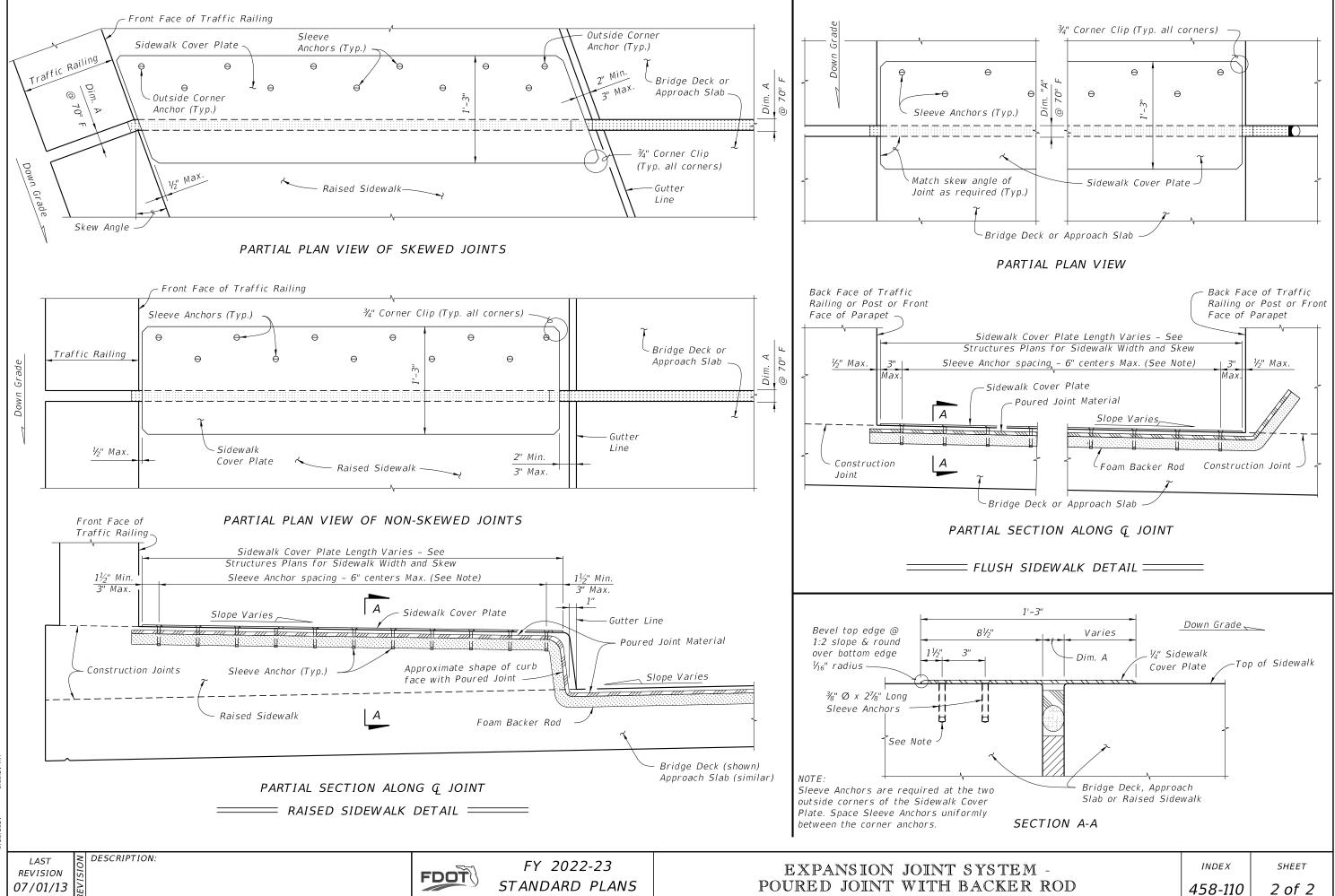
E PILES	INDEX	SHEET
ES	455-001	1 of 1

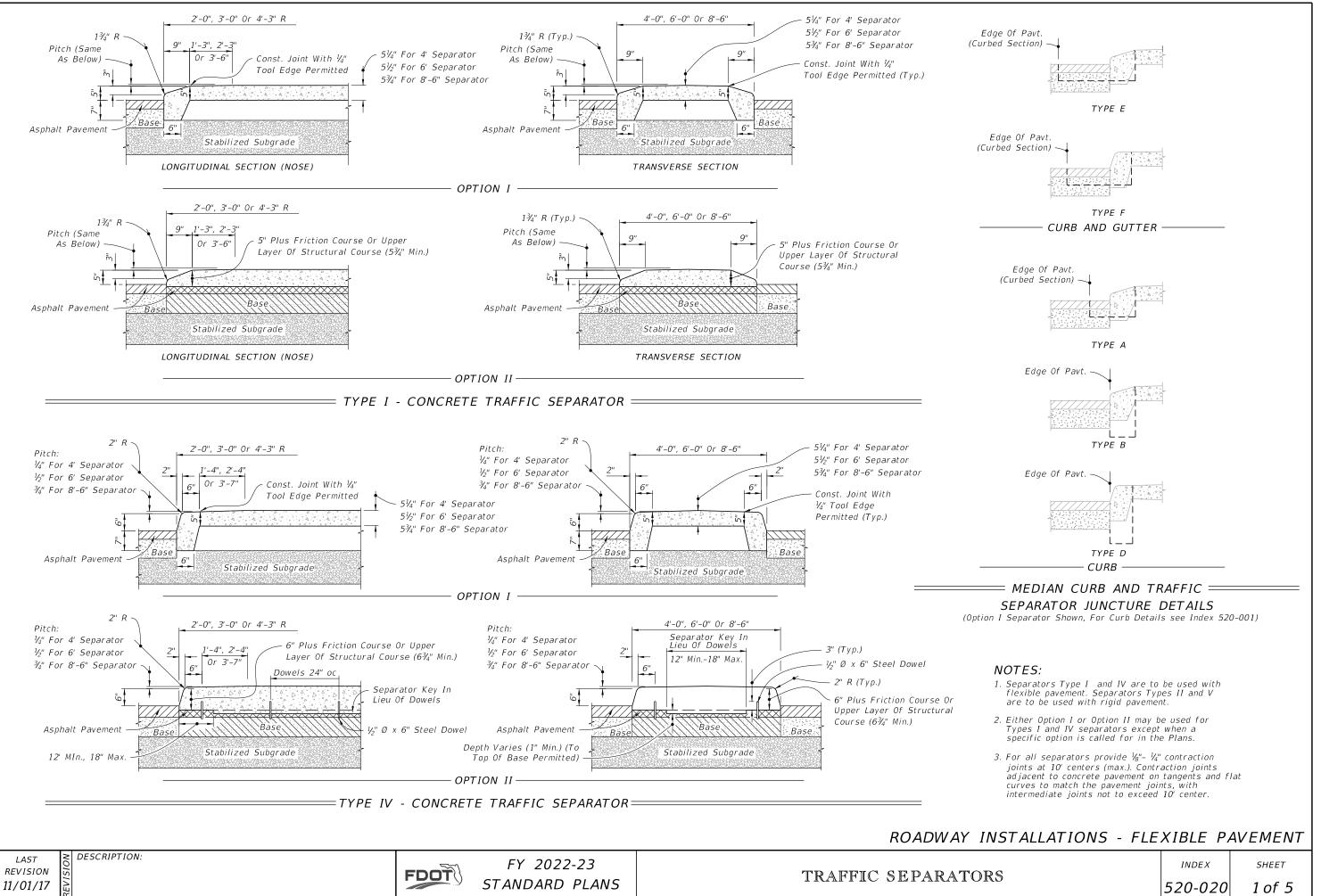


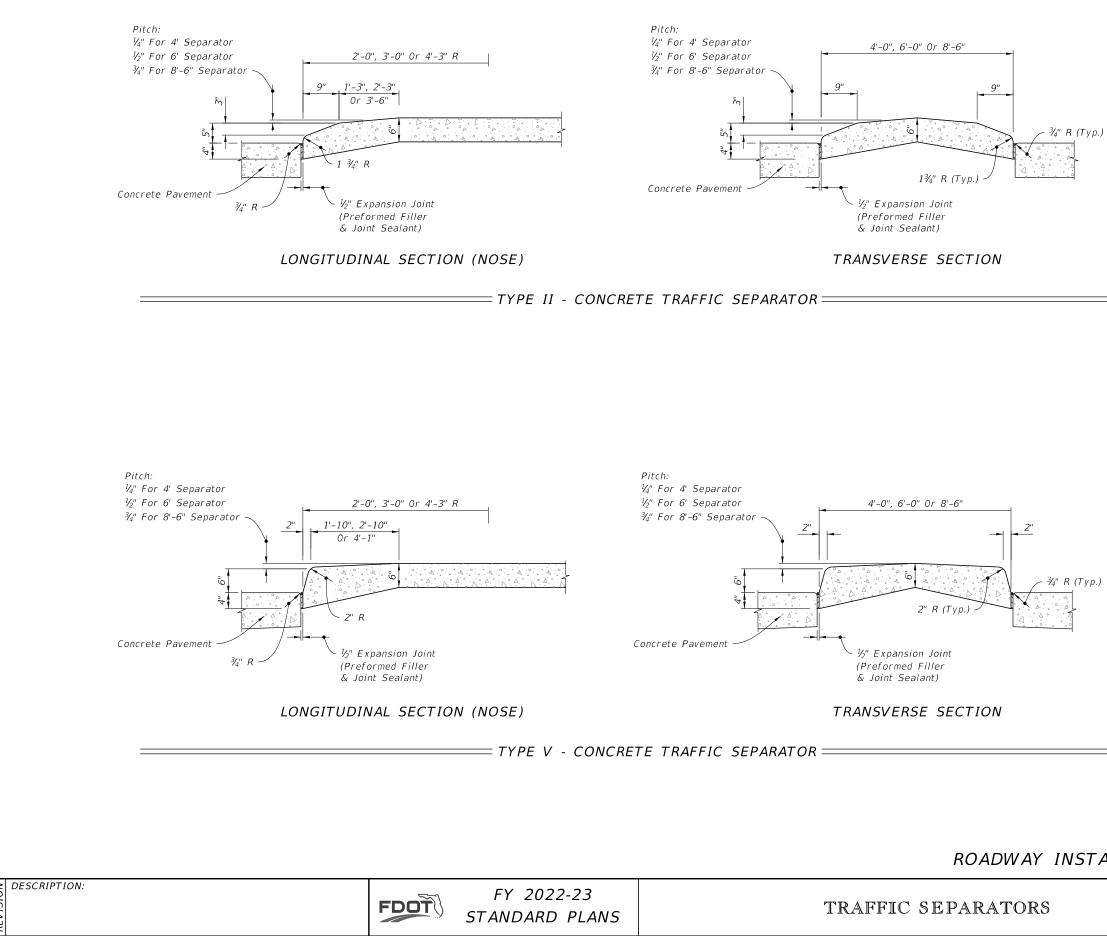


LAST REVISION

_	INDEX	SHEET
ROD	458-110	1 of 2







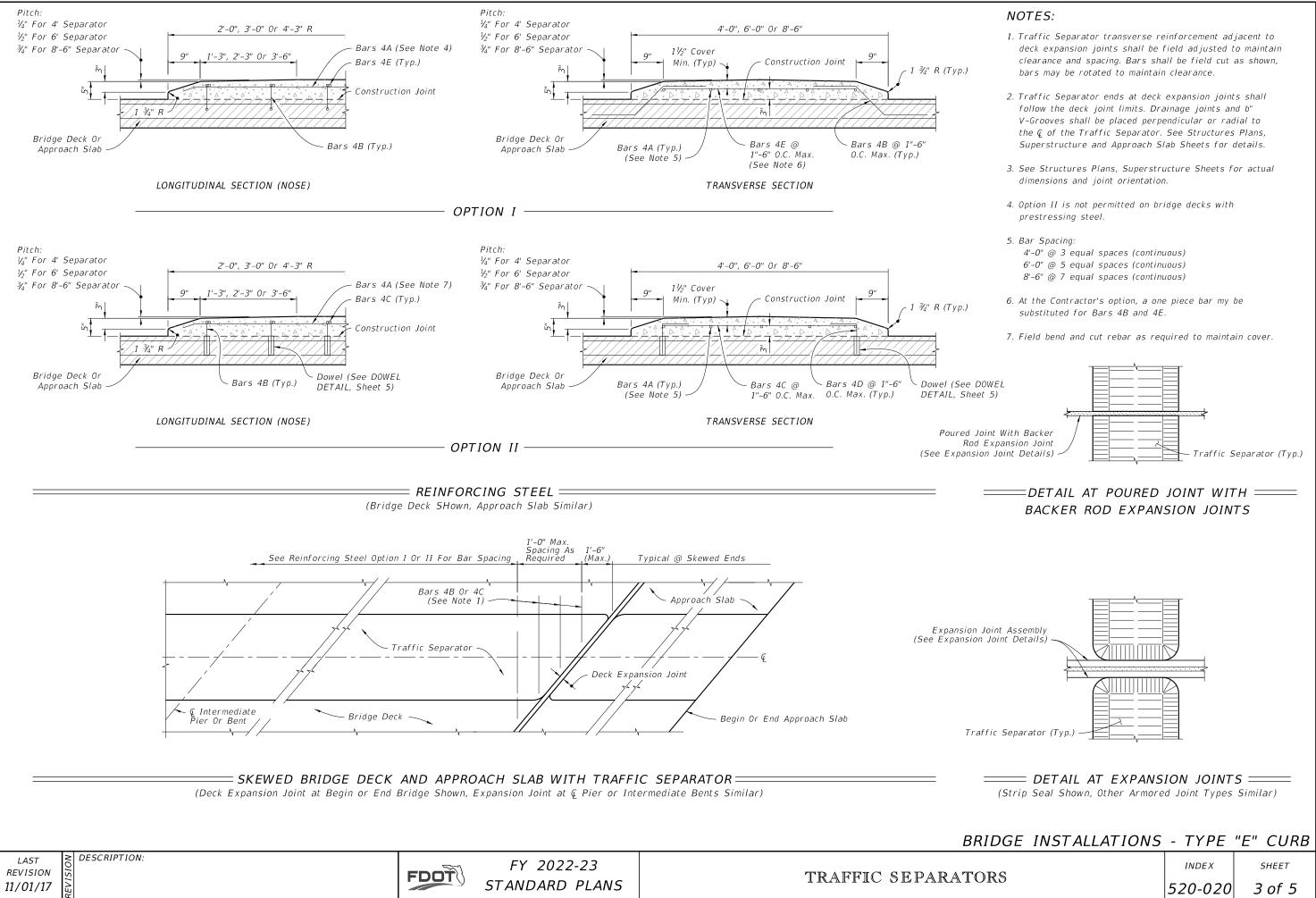
LAST

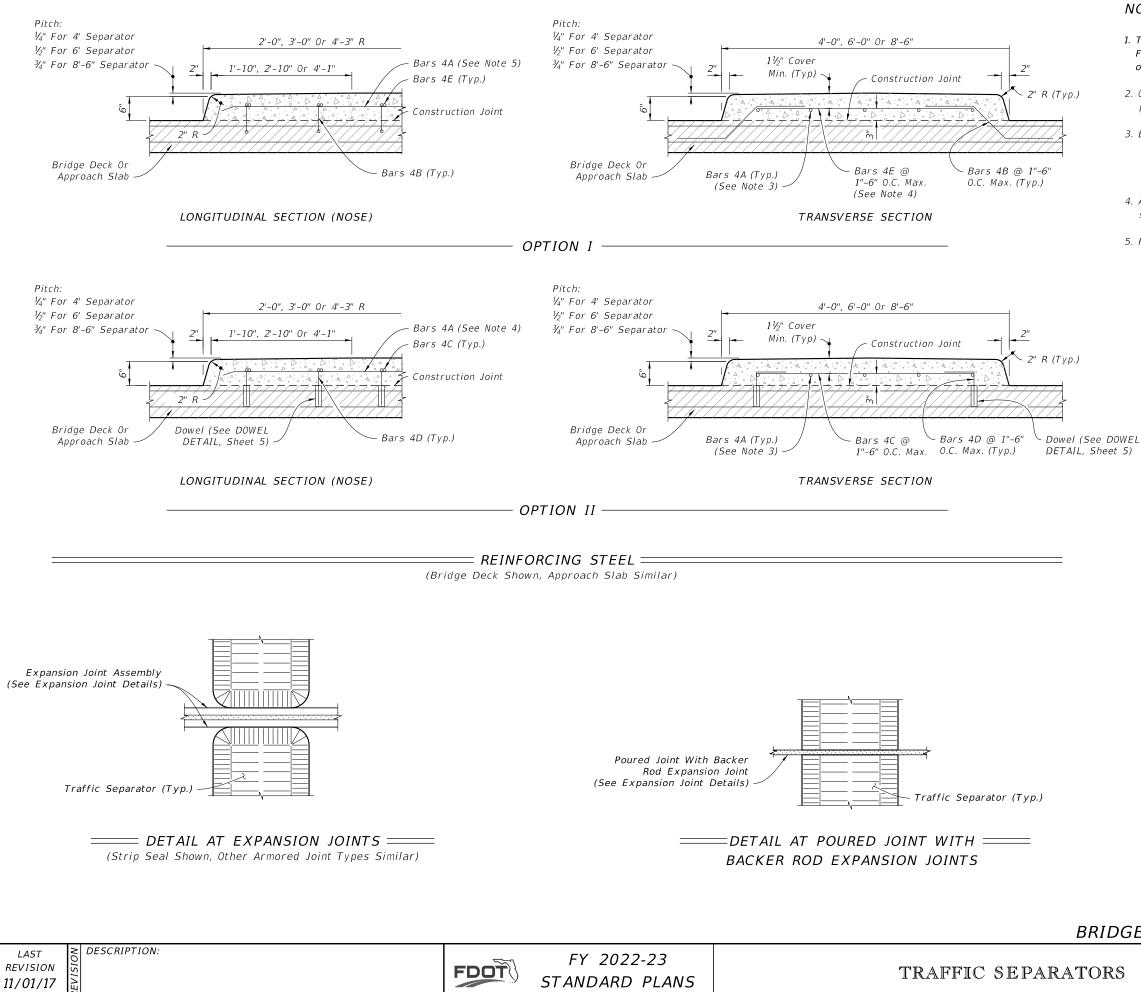
REVISION

11/01/17

ROADWAY INSTALLATIONS - RIGID PAVEMENT

INDEX	SHEET
520-020	2 of 5





NOTES:

1. Treatment of separators on straight bridges shown. For additional notes and treatment of separators on skewed bridges, see Sheet 2.

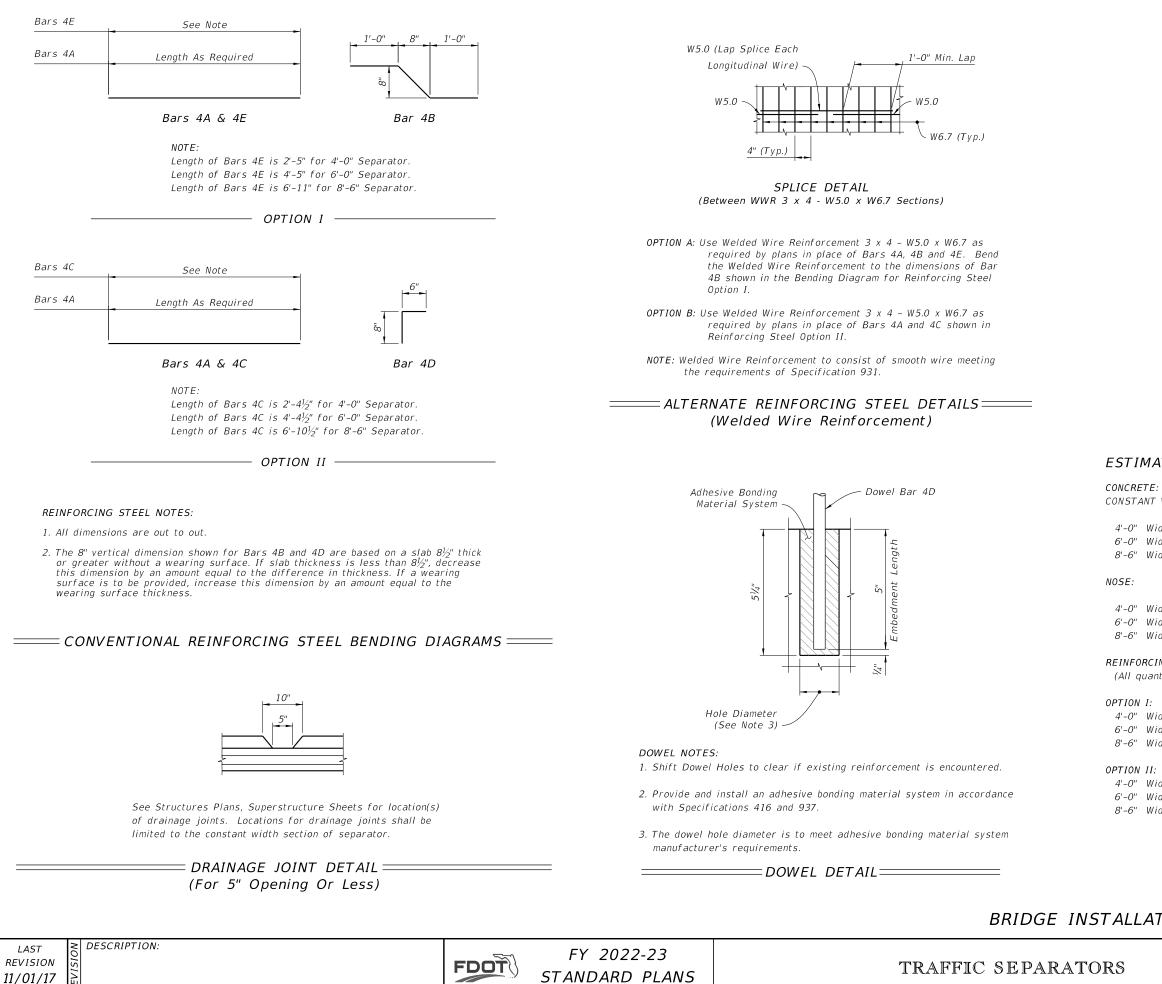
2. Option II is not permitted on bridge decks with prestressing steel.

Bar Spacing:
 4'-0" @ 3 equal spaces (continuous)
 6'-0" @ 5 equal spaces (continuous)
 8'-6" @ 7 equal spaces (continuous)

4. At the Contractor's option, a one piece bar my be substituted for Bars 4B and 4E.

5. Field bend and cut rebar as required to maintain cover.

E INSTALLATIONS	- TYPE	"F" CURB
	INDEX	SHEET
	520-020	4 of 5



ESTIMATED TRAFFIC SEPARATOR QUANTITIES:

CONSTANT WIDTH OF SEPARATOR:

TYPE "E"	TYPE "F"
Width = 0.056 CY per Ft.	– 0.072 CY per Ft.
Width = 0.089 CY per Ft.	– 0.112 CY per Ft.
$Width = 0.132 \ CY \ per \ Ft.$	– 0.164 CY per Ft.

<u>TYPE "E"</u>		<u></u>
Width = 0.080 CY	-	0.109 CY
Width = 0.193 CY	-	0.257 CY
Width = 0.403 CY	-	0.536 CY

REINFORCING STEEL:

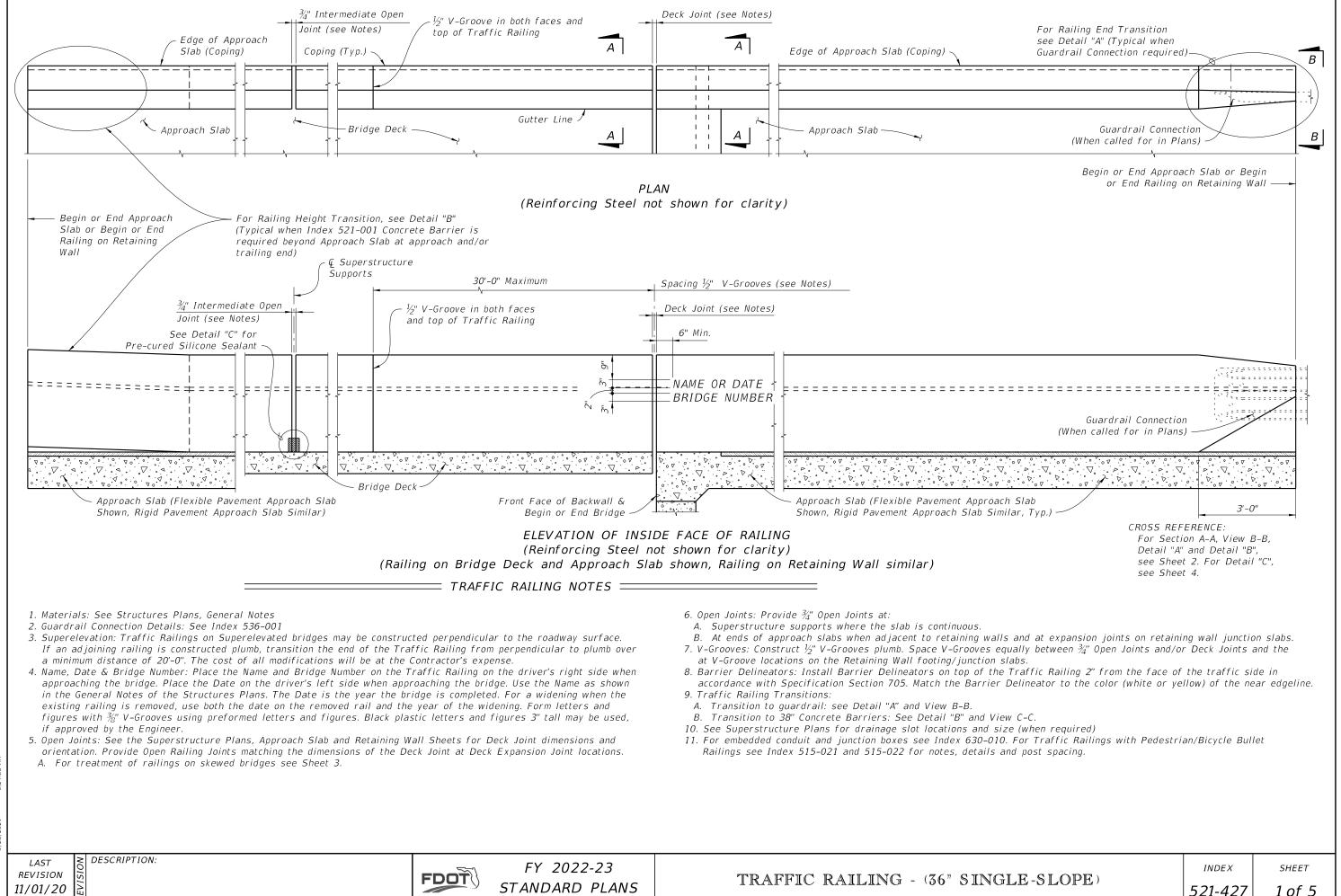
(All quantities are based on an $8\frac{1}{2}$ " slab.)

4'-0" Width - 6.37 Lbs. per Ft. 6'-0" Width - 8.60 Lbs. per Ft. 8'-6" Width - 11.05 Lbs. per Ft.

- 4'-0" Width 4.77 Lbs. per Ft. 6'-0" Width - 7.00 Lbs. per Ft.
- 8'-6" Width 9.45 Lbs. per Ft.

BRIDGE INSTALLATIONS - TYPE "E" AND "F" CURB

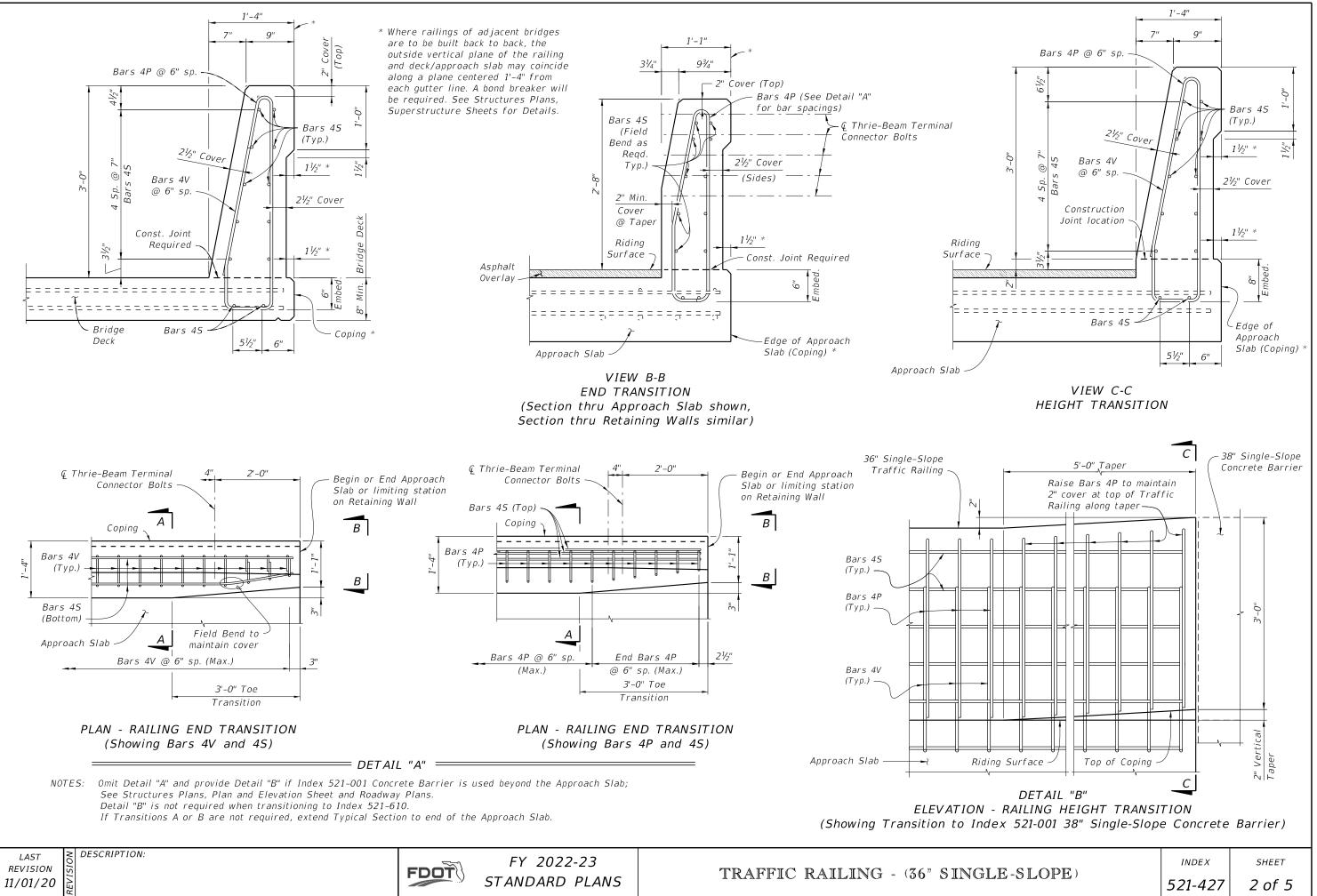
INDEX	SHEET
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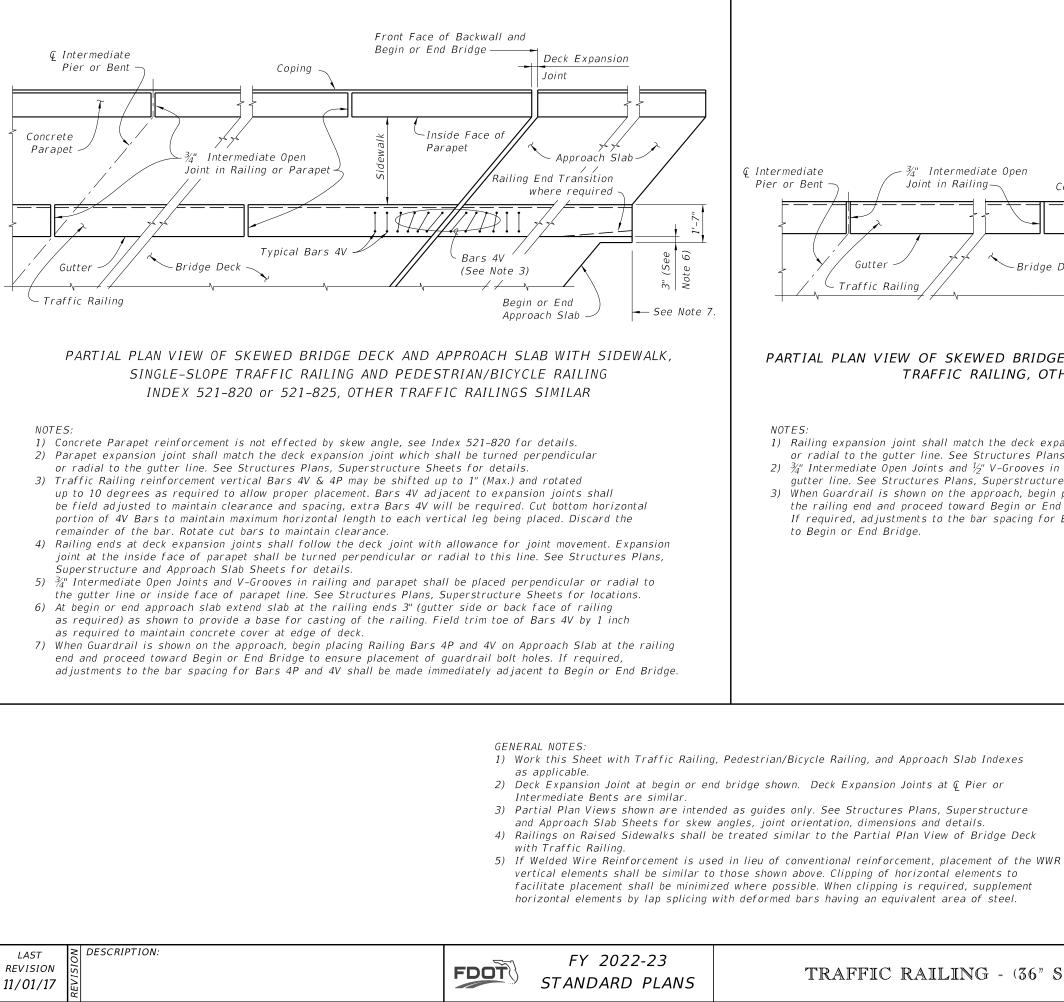
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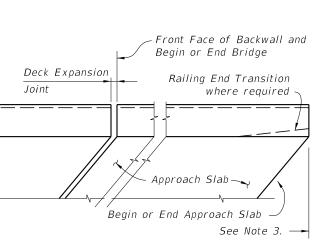




9/23/2021



TRAFFIC RAILING - (36" SINGLE



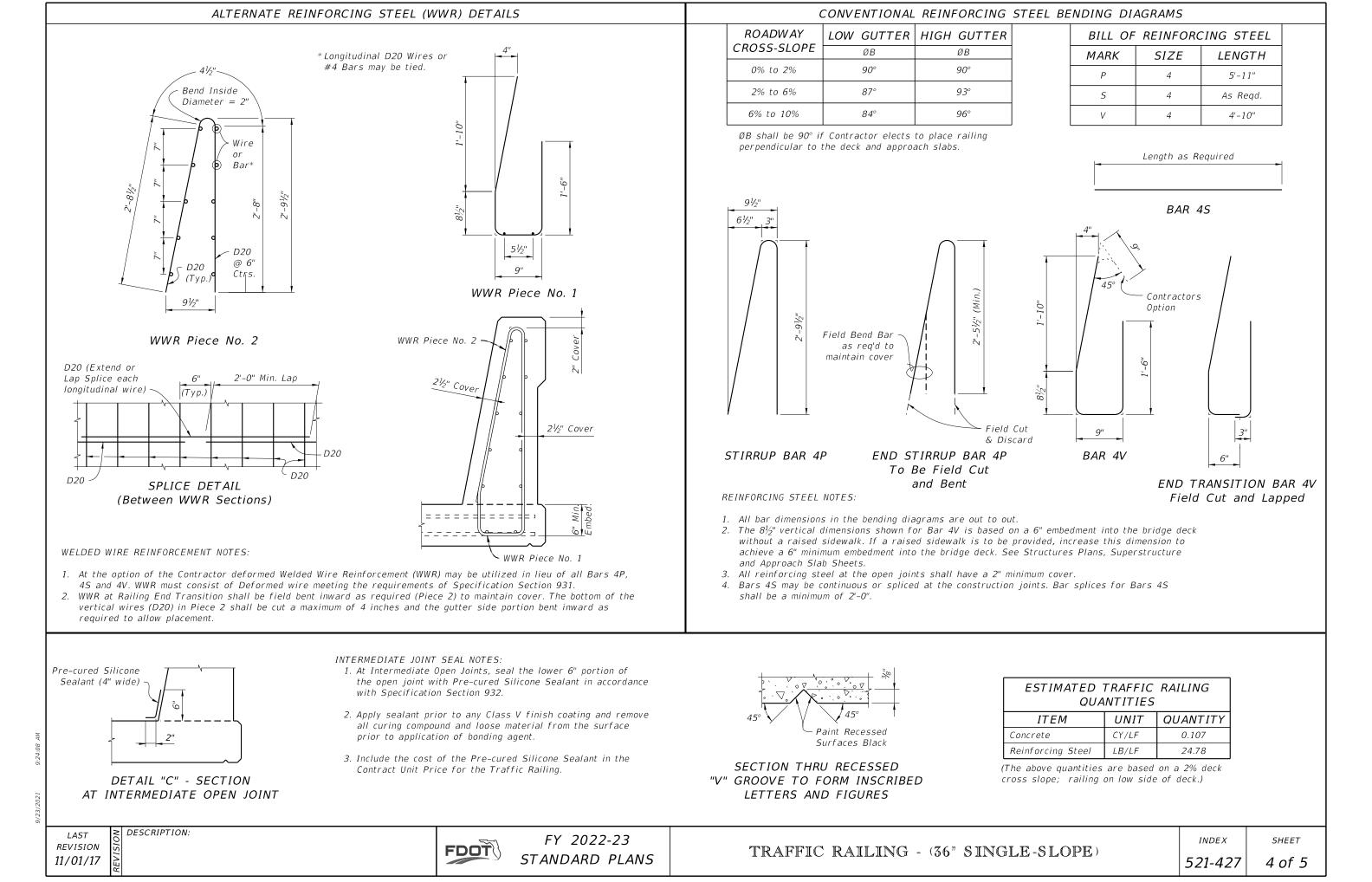
Coping __

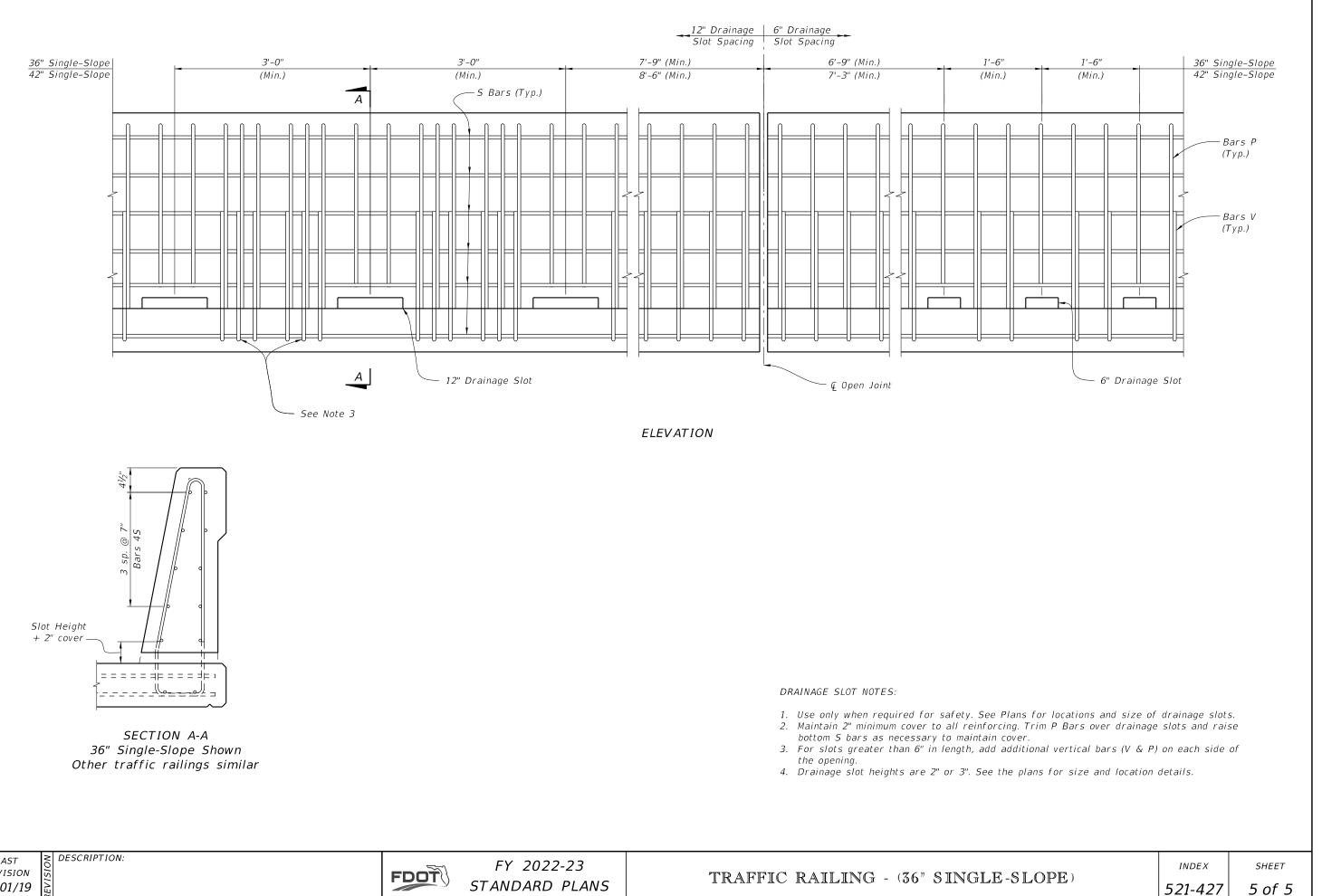
— Bridge Deck —

PARTIAL PLAN VIEW OF SKEWED BRIDGE DECK AND APPROACH SLAB WITH SINGLE-SLOPE TRAFFIC RAILING, OTHER TRAFFIC RAILINGS SIMILAR

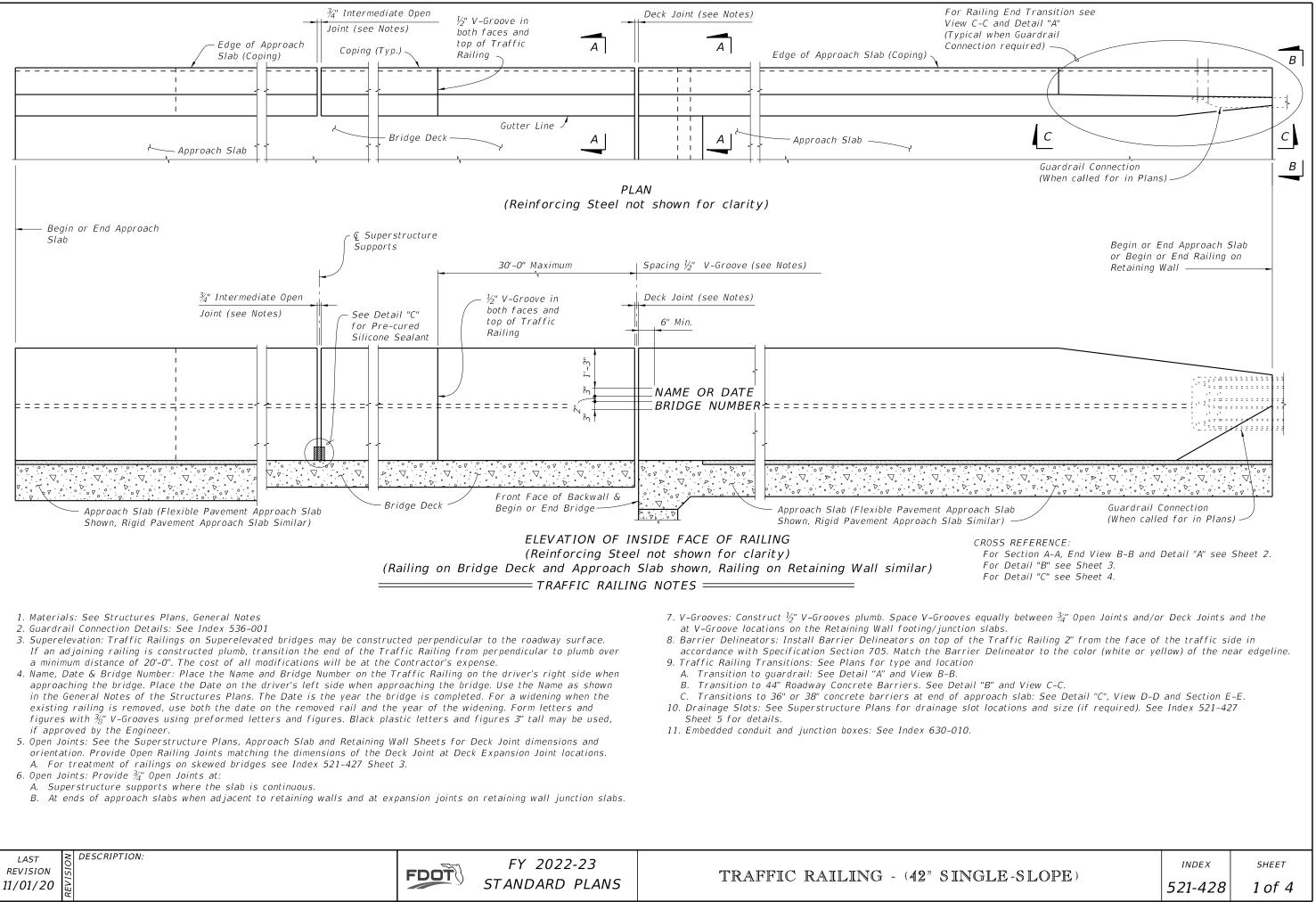
1) Railing expansion joint shall match the deck expansion joint which shall be turned perpendicular or radial to the gutter line. See Structures Plans, Superstructure Sheets for details. 2) $\frac{3}{4''}$ Intermediate Open Joints and $\frac{1}{2''}$ V-Grooves in railing shall be placed perpendicular or radial to the gutter line. See Structures Plans, Superstructure and Approach Slab Sheets for locations. 3) When Guardrail is shown on the approach, begin placing Railing Bars 4P and 4V on Approach Slab at the railing end and proceed toward Begin or End Bridge to ensure placement of guardrail bolt holes. If required, adjustments to the bar spacing for Bars 4P and 4V shall be made immediately adjacent

-SLOPE)	INDEX	SHEET
	521-427	3 of 5

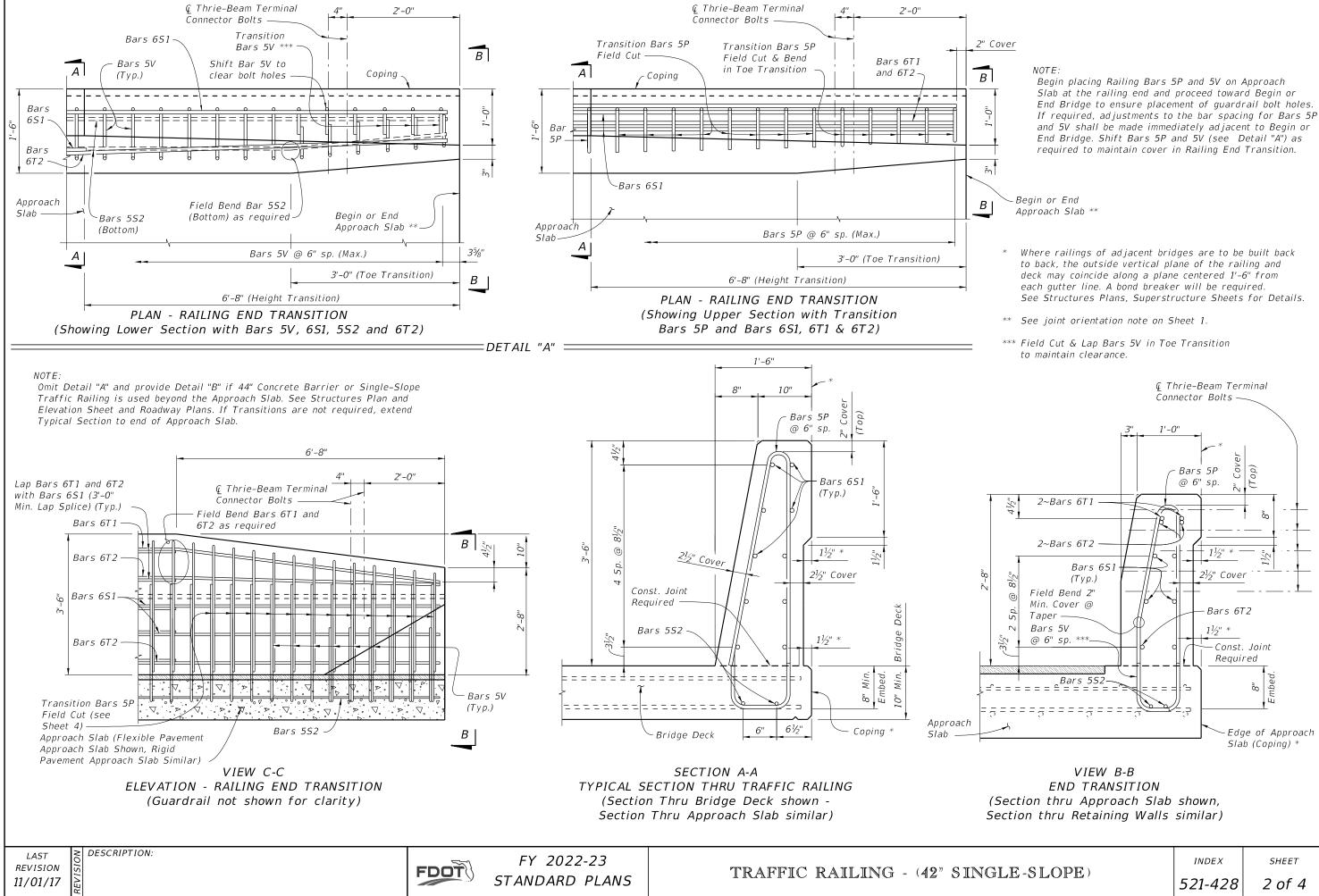








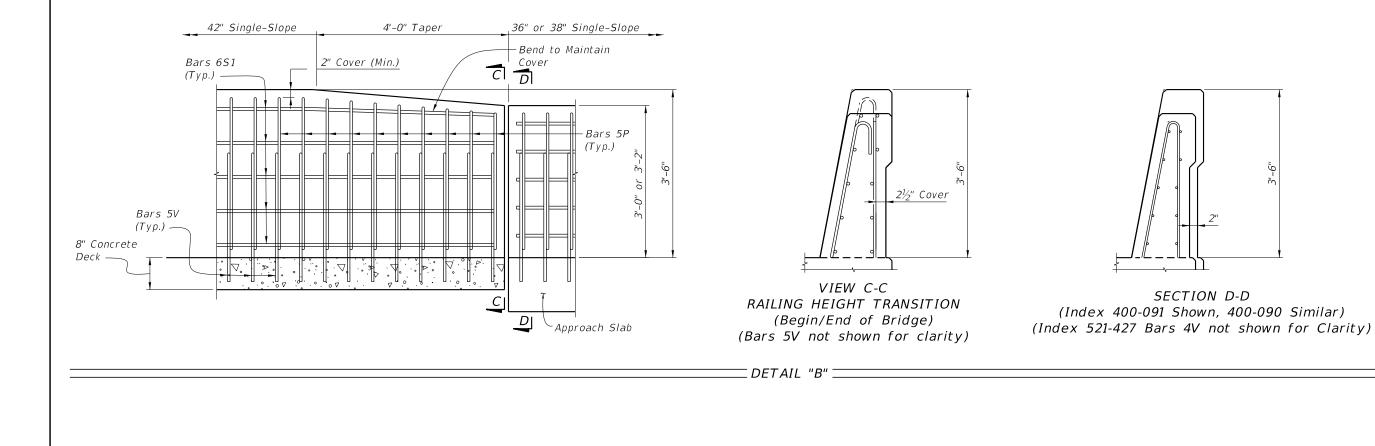




	INDEX	SHEET
SLOPE)	521-428	2 of 4

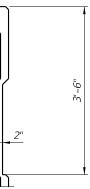
- NOTE:

- and 521–610 as necessary.
- cover at top of traffic railing.



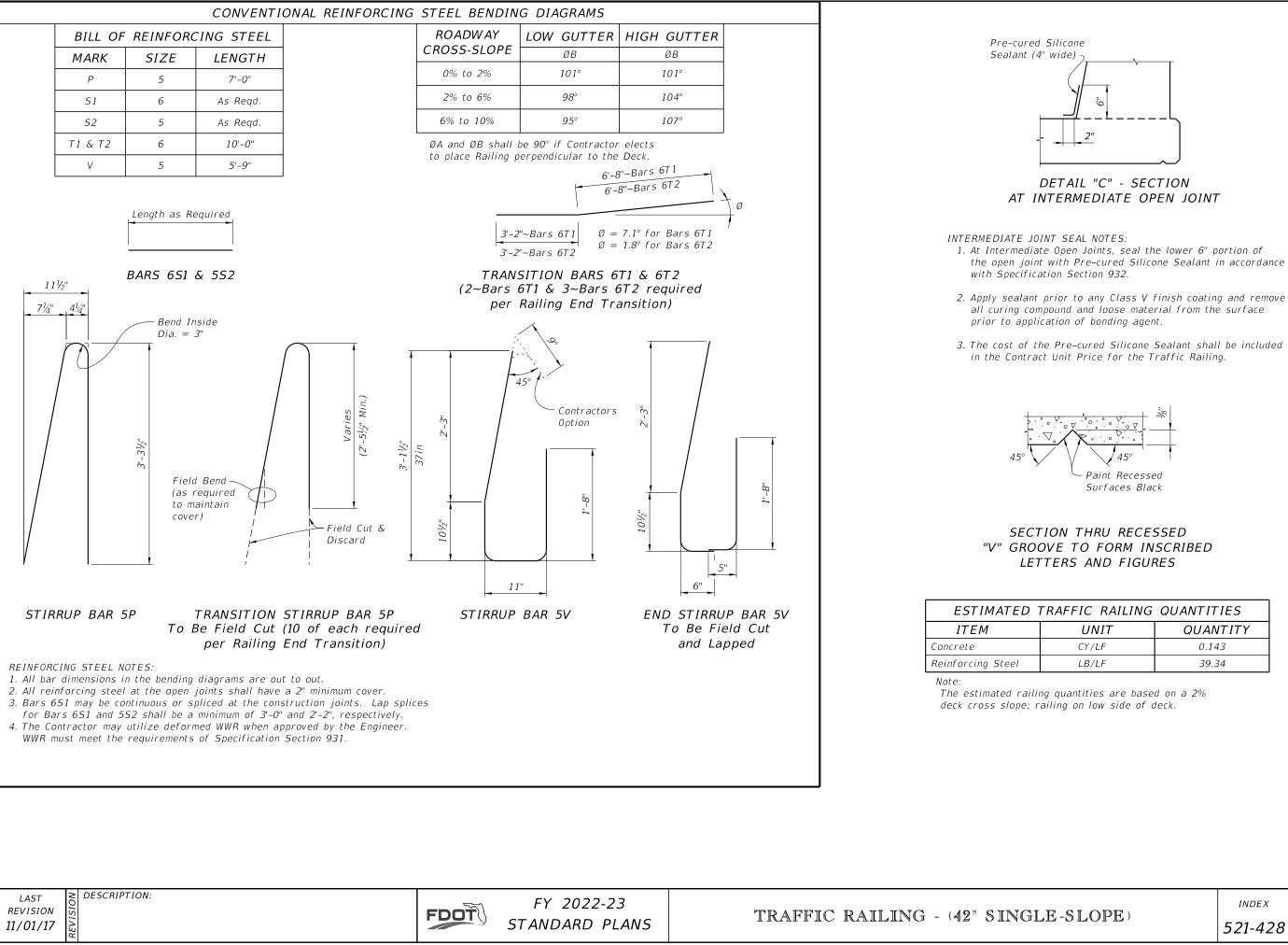


1. Provide Detail "B" height transition where 42" Traffic Railings are required on bridge, and 36" or 38" Barriers are shown on approaches. See Structures Plans for coping details. 2. Work Detail "B" with Indexes 400-090 or 400-091, 521-427, 3. Field cut 5P Bars as shown to maintain 2" min. (4" max.)



SECTION D-D (Index 400-091 Shown, 400-090 Similar)

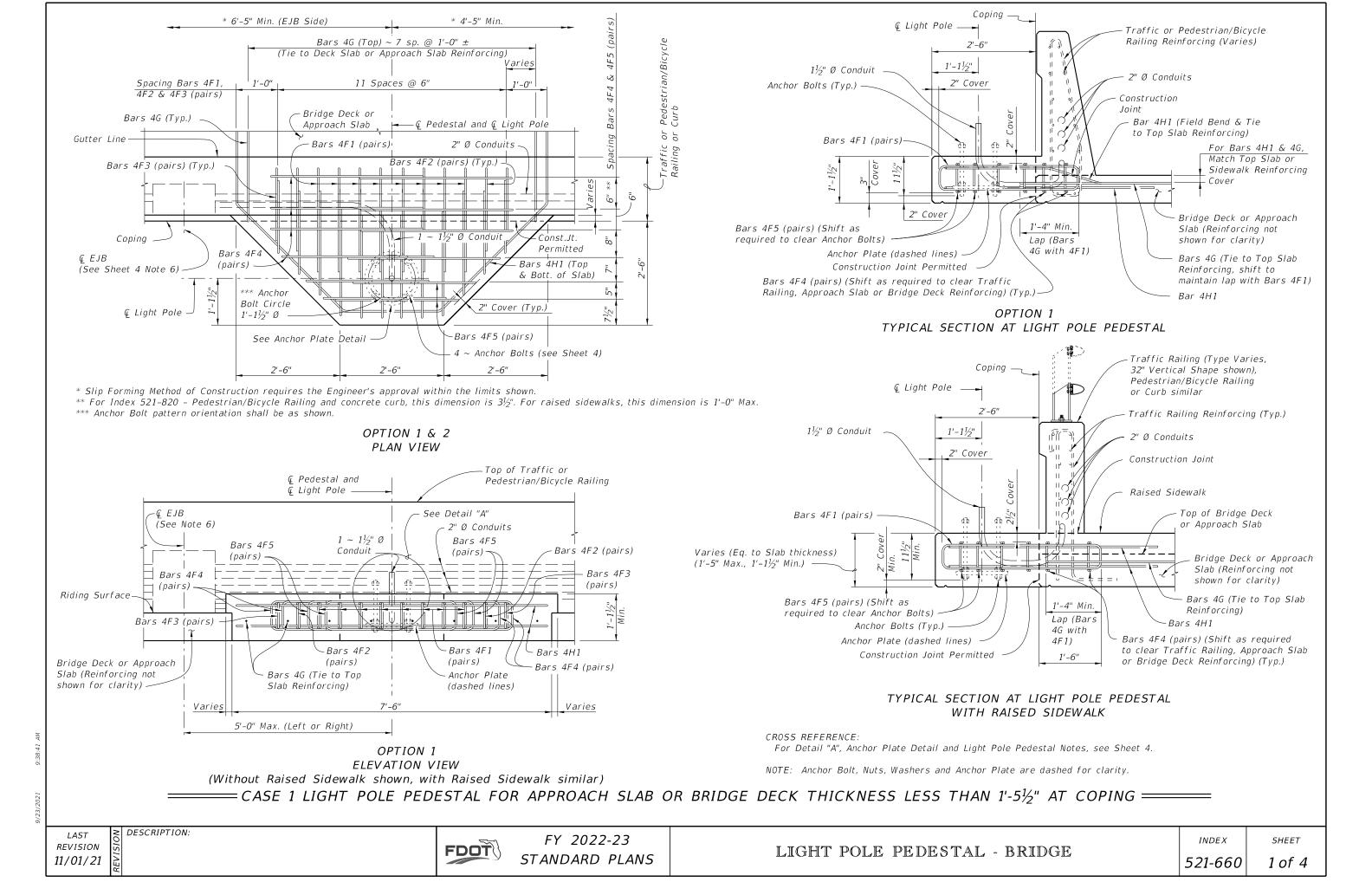
SLOPE)	INDEX	SHEET
	521-428	3 of 4

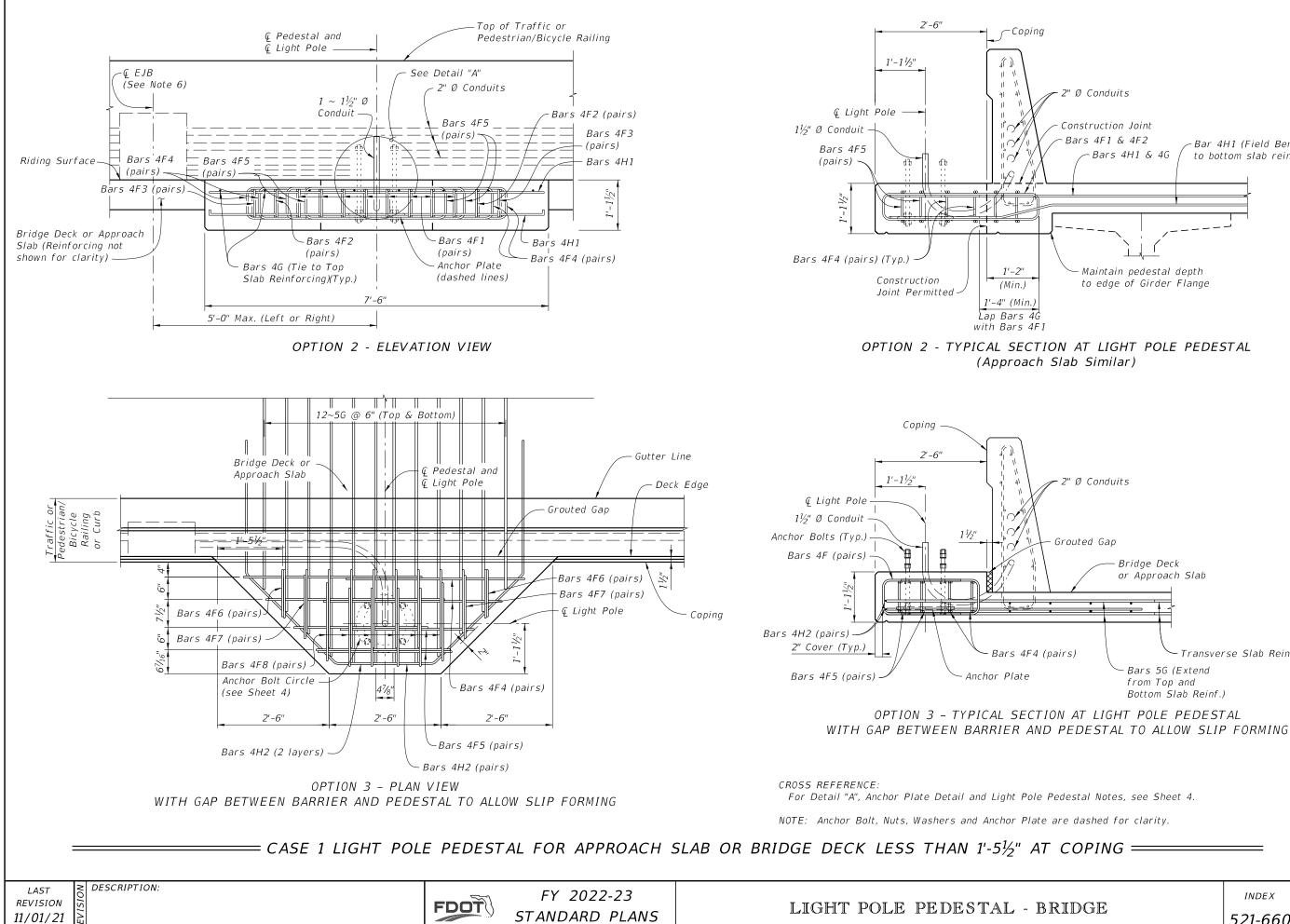


the open joint with Pre-cured Silicone Sealant in accordance

FIC RAILING QUANTITIES		
UNIT	QUANTITY	
CY/LF	0.143	
LB/LF	39.34	

SLODE	INDEX	SHEET
SLOPE)	521-428	4 of 4

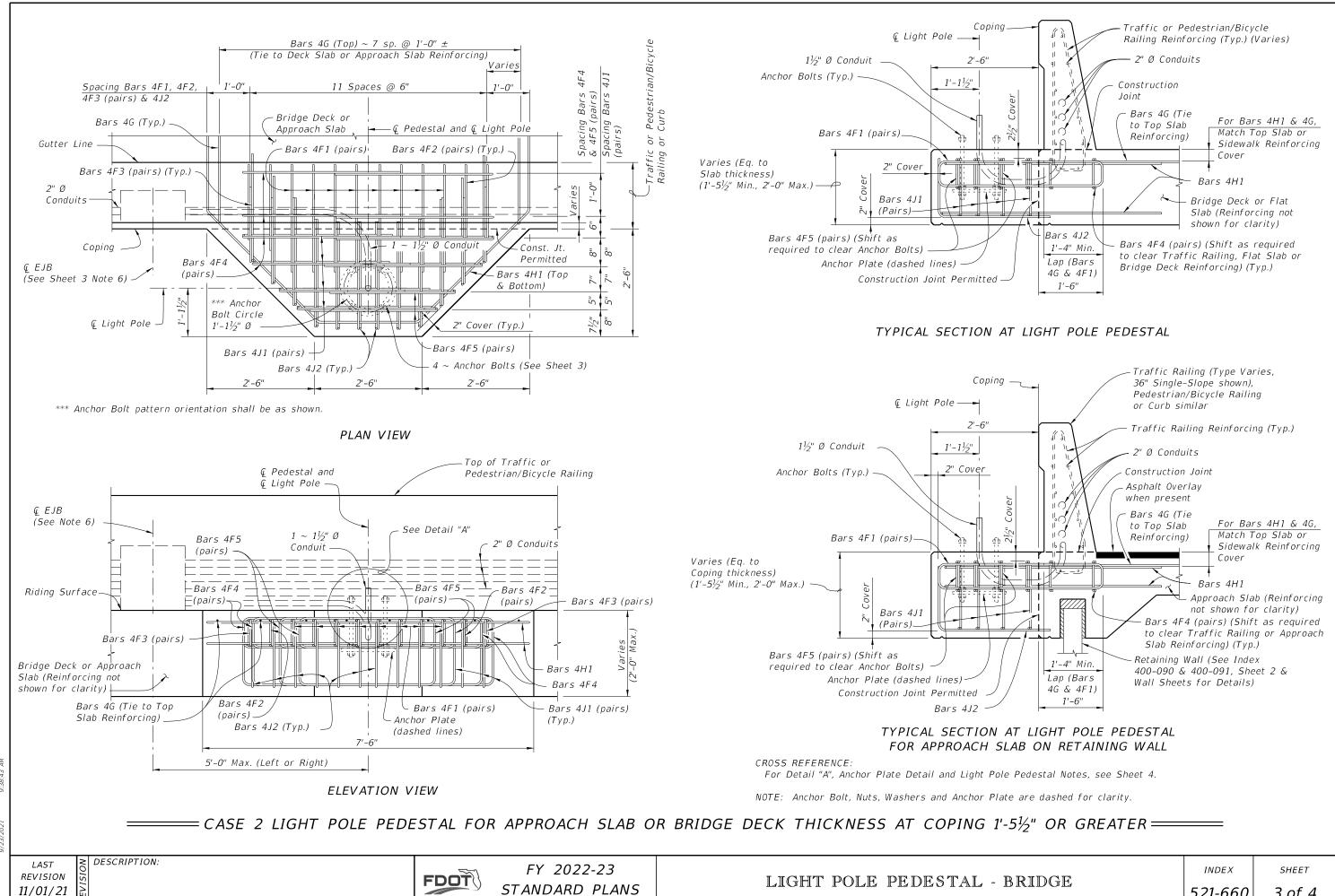




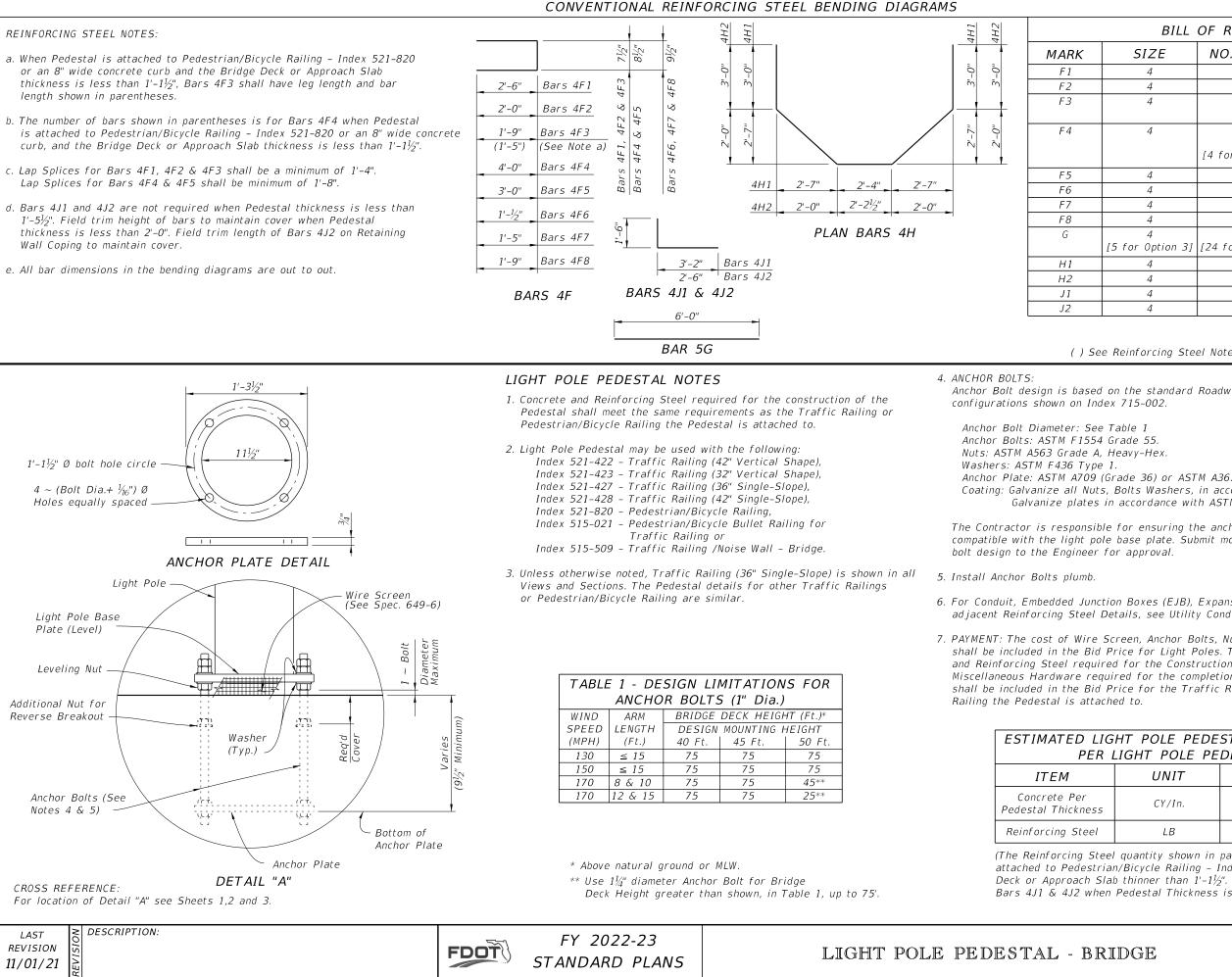


STANDARD PLANS

2" Ø Conduits Construction Joint Bars 4F1 & 4F2 Bar 4H1 (Field Bend & Tie -Bars 4H1 & 4G to bottom slab reinforcing) Maintain pedestal depth to edge of Girder Flange 2" Ø Conduits Grouted Gap Bridge Deck or Approach Slab Transverse Slab Reinf. Bars 5G (Extend from Top and Bottom Slab Reinf.) INDEX SHEET 521-660 2 of 4



DOE	INDEX	SHEET		
DGE	521-660	3 of 4		



BILL OF REINFORCING STEEL						
SIZE	NO. REQD.	LENGTH	NOTES			
4	16	5'-8''	С			
4	4	4'-8''	С			
4	4	4'-2" (3'-6")	а, с			
4	8 (6) [4 for Option 3]	8'-9"	b, c			
4	4	6'-9''	С			
4	4	2'-11"	-			
4	4	3'-8''	-			
4	12	4'-4''	-			
4 for Option 3]	8 [24 for Option 3]	6'-0''	-			
4	2	15'-8"	_			
4	2	13'-10''	-			
4	8	4'-8''	d			
4	12	4'-0''	d			

() See Reinforcing Steel Note a & b.

Anchor Bolt design is based on the standard Roadway Aluminum Light Pole

Coating: Galvanize all Nuts, Bolts Washers, in accordance with ASTM F2329. Galvanize plates in accordance with ASTM A123.

The Contractor is responsible for ensuring the anchor bolt configuration is compatible with the light pole base plate. Submit modifications of the anchor

6. For Conduit, Embedded Junction Boxes (EJB), Expansion/Deflection Fitting and adjacent Reinforcing Steel Details, see Utility Conduit Detail Sheets and Index 630-010.

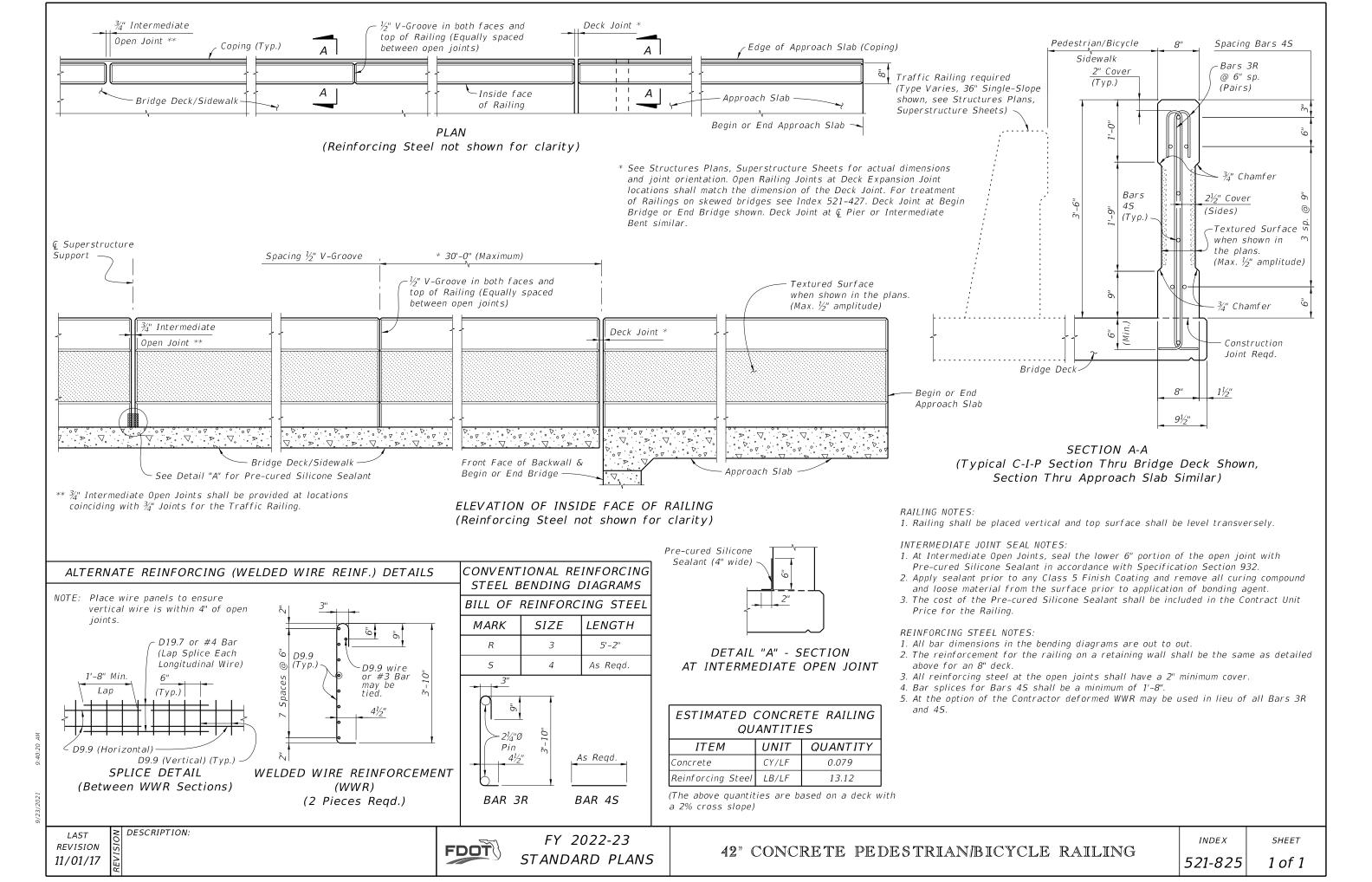
7. PAYMENT: The cost of Wire Screen, Anchor Bolts, Nuts, Washers and Anchor Plates shall be included in the Bid Price for Light Poles. The cost of all Labor, Concrete and Reinforcing Steel required for the Construction of the Pedestals, and Miscellaneous Hardware required for the completion of the Electrical System, shall be included in the Bid Price for the Traffic Railing or Pedestrian/Bicycle

ESTIMATED LIGHT POLE PEDESTAL QUANTITIES PER LIGHT POLE PEDESTAL

UNIT	QUANTITY
CY/In.	0.040
LB	195 (182)

(The Reinforcing Steel quantity shown in parenthesis is for a Pedestal attached to Pedestrian/Bicycle Railing - Index 521-820 with Bridge Deck or Approach Slab thinner than $1'-1\frac{1}{2}''$. Add 59 Lbs. for Bars 4J1 & 4J2 when Pedestal Thickness is $1'-5^{1}/_{2}"$ or greater)

	INDEX	SHEET
DGE	521-660	4 of 4



NOTES

DESIGN CRITERIA:

1. Design is based on the assumption that the material contained within the reinforced soil volume, methods of construction and quality of prefabricated materials are in accordance with Specification Section 548 and Chapter 3 of the FDOT Structures Design Guidelines.

SOIL PARAMETERS:

- 1. See Wall Control Drawings for soil characteristics of foundation material to be used in the design of the wall system.
- 2. The Contractor will provide soil design parameters for backfill material based on the actual soil characteristics utilized at the site.

MATERIALS:

1. See Specification Section 548 for material requirements.

CONSTRUCTION

- 1. Walls will be constructed in accordance with Specification Section 548 and the Wall Company's instructions.
- 2. For location and alignment of retaining walls, see Wall Control Drawings.
- 3. If required, locate manholes and drop inlets as shown on wall elevations.
- 4. Refer to Wall Control Drawings of individual walls for minimum reinforcement strip/mesh length, factored bearing resistance's, minimum wall embedment and anticipated long term and differential settlements.
- 5. The Contractor is responsible for controlling water during storm events as needed during construction.
- 6. It is the Contractor's responsibility to determine the location of any guardrail posts behind retaining wall panels. Prior to placement of the top layer of soil reinforcement, individual reinforcing strips/mesh may be skewed (15° maximum) to avoid the post locations if authorized by the Engineer. No cutting of soil reinforcement is allowed unless shown on Shop Drawings and approved by the Engineer. Any damage done to the soil reinforcement due to installation of the guardrail will be repaired by the Contractor at the Contractor's expense. Repair method will be approved by the Engineer.
- 7. If existing or future structures, pipes, foundations or guardrail posts within the reinforced soil volume interfere with the normal placement of soil reinforcement and specific directions have not been provided on the plans, the Contractor will notify the Engineer to determine what course of action shall be taken.
- 8. The Contractor is responsible for gradually displacing upper layer(s) of soil reinforcement downward (15° maximum from horizontal) to avoid cutting soil reinforcement and conflicts with paving and subgrade preparation. The Contractor's attention is directed especially to situations where roadway superelevation and/or soil mixing are anticipated.
- 9. For concrete facing panel surface treatment, see Wall Control Drawings. Extend surface treatment a minimum of 6" below final ground line.
- 10. Drive piles located within the soil volume prior to construction of the retaining wall, unless a method to protect the structure, acceptable to both the Engineer and Wall Company, is proposed and approved in writing. The portion of piles or drilled shafts extensions within the soil volume will be wrapped with polyethylene sheeting in accordance with Specification Section 459.
- 11. A structural extension of the connection of the retaining wall panel to soil reinforcement will be used whenever necessary to avoid cutting or excessive skewing (greater than 15°) of the soil reinforcement around obstructions (i.e., piles, pipes, manholes, drop inlets, etc.).
- 12. Steps in leveling pads will occur at MSE Wall panel interfaces. Panels will not cantilever more than 2" past the end of the upper tier leveling pad.
- 13. The top of the leveling pad or footing will be 2'-0" minimum below final ground line.
- 14. Top of leveling pad elevations shown in the Wall Control Drawings are maximum elevations. The constructed leveling pad elevations may be deeper based on the panel layout shown in the shop drawings.
- 15. The height of panels in the bottom course of MSE Walls must not be less than half the height of a standard panel.
- 16. Work this Index with Index 521-600 thru 521-650.

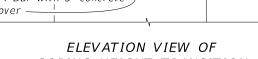
SHOP DRAWINGS:

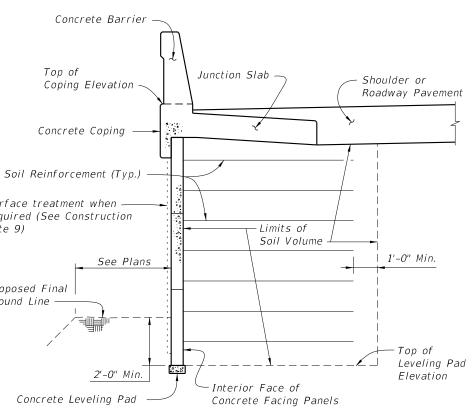
See Specification Section 548 for shop drawing requirements.

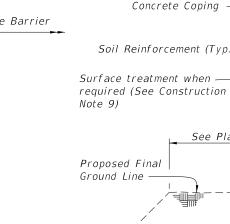
10'-0" Min. C-I-P Coping 12'-0" Min. Precast Coping/Concrete Barrier (Index 521-610, 521-620) 2'-0" 12" Coping Top of Coping -Transition - - -3"_ 1/3" Preformed Joint Filler Provide Supplemental #4 Bar with 3" concrete cover

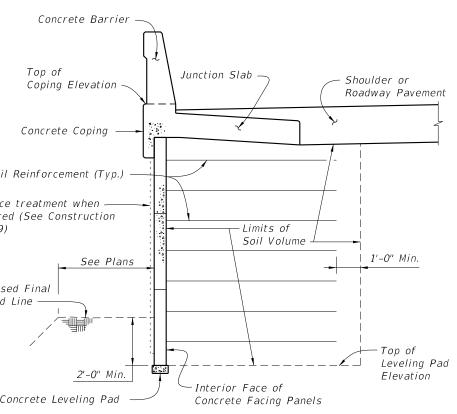
> ELEVATION VIEW OF COPING HEIGHT TRANSITION

Applicable		ility Requir n-Steel Rein		Durability Requirements (FRP Reinforcing)		Soil 0		Other Allowable FDOT Wall Types					
FDOT Wall	Concrete	Concrete	Pozzolan	Concrete	Concrete	Pozzolan	Reinforcement						
Type *	Cover (in.)	Class for Panels	Additions? **	Cover (in.)	Class for Panels	Additions?	Туре	2A	2B	2C	2D	2E	2F
Type 2A	2	II	No	1.5	II	No	Metal		1	~	1	~	1
Type 2B	2	IV	No	1.5	IV	No	Metal			1	1	~	-
Type 2C	3	IV	No	1.5	IV	No	Metal				1	1	1
Type 2D	3	IV	Yes	2	IV	No	Metal						1
Type 2E	3	IV	No	2	IV	No	Plastic						1
Type 2F	3	IV	Yes	2	IV	No	Plastic						
	Table in Co	ontract Plan	5.		1	11						1	
See Data	Table in Cc		5.			<u> </u>							
See Data			S.					GEN	ERAL	. NO	TES	AND	DET
See Data * Highly Re	eactive Pozz	colans.					RMANEN		ERAL		TES DEX	AND	DE1







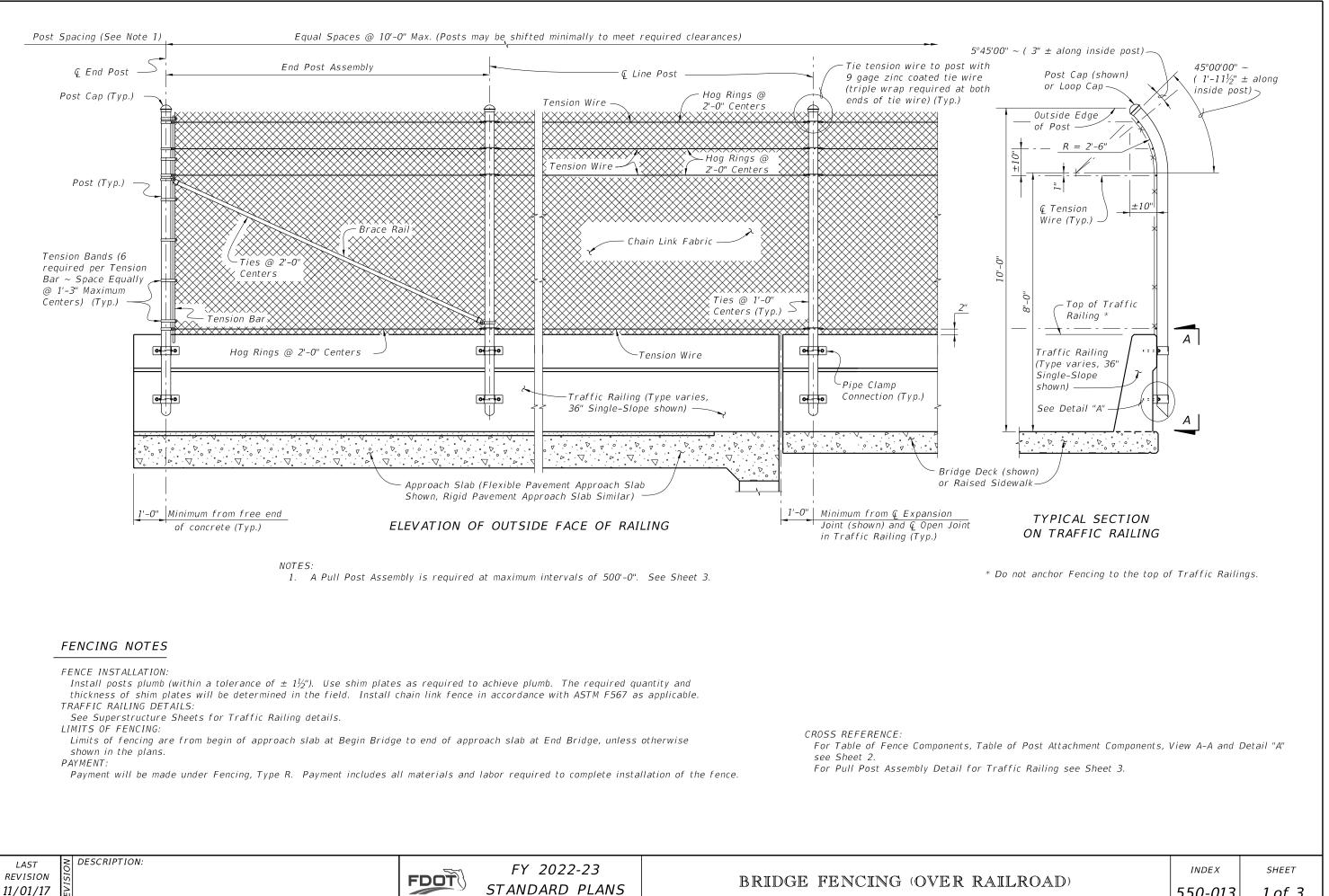




LAST	NC	DESCRIPTION:
REVISION	SI	
11/01/21	REVI	

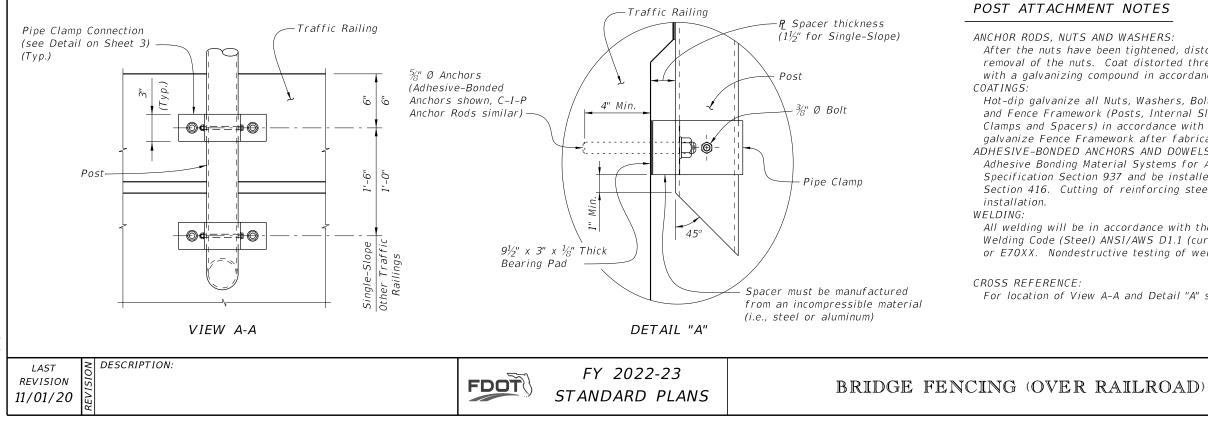


TYPICAL MSE RETAINING WALL SECTION WITH A CONCRETE BARRIER (Showing Limits of the Reinforced Soil Volume)



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	TABLE OI	F CHAIN LINK FENCE COMPONENTS	TABLE OF POST ATTACHMENT COMPONENTS				
COMPONENT	ASTM DESIGNATION	COMPONENT INFORMATION	COMPONENT		ASTM DESIGNATION	COMPONENT INFORMATION	
Posts	F1083	Galvanized Steel Pipe – 3½" NPS, Schedule 40 Regular Grade	Pipe Clamps		A36 or A709 Grade 36	¼" Steel P	
Chain Link Fabric (2" mesh with twisted	A392	Zinc Coated Steel - 9 gage (coated wire diameter), Class 2 Coating	Base Pl	lates	A36 or A709 Grade 36	¾" Steel Ŗ	
top and knuckled bottom selvage)	A491	Aluminum Coated Steel – 9 gage (coated wire diameter)	Shim Platas		A36 or A709 Grade 36 or	Plate thicknesses as required; Holes in shim	
		Polyvinyl Chloride (PVC) Coated Steel – 9 gage Class 2b			B209 Alloy 6061-T6 or B221 Alloy 6063-T5	plates will be $\frac{3}{4}$ " Ø	
Tie Wires	F626	Zinc Coated Steel Wire – 9 gage	Spacers		_	Plate thickness varies based on traffic railing type (See Detail "A")	
Brace Bands	F626	12 Gage (Min. thickness) x $\frac{3}{4}$ " (Min. width) Steel Bands (Beveled or Heavy)	Adhesive Anchor Rods		F1554 Grade 36	Fully threaded Headless Anchor Rods ~ $\frac{5}{8}$ " Ø x 6" (no spacer) or $\frac{5}{8}$ " Ø x (6" + spacer thickness)	
Tension Bars	F626	$3_{16}^{\prime\prime}$ (Min. thickness) x $3_4^{\prime\prime}$ (Min. width) x 6'-10" (Min. height) Steel Bars	eduo C-I-P Anchor Rods		F1554 Grade 36	Hex Head Anchor Rods ~ 5%" Ø x 6" (no spacer) or 5%" Ø x (6" + spacer thickness)	
Tension Bands	F626	14 Gage (Min. thickness) x ¾" (Min. width) Steel Bands	Bolts		A307	$\frac{3}{8}$ " Ø x $4\frac{3}{4}$ " Hex Head Bolts for Pipe Clamp Connections to Posts	
<i>Miscellaneous Fence Components</i>	F626	Zinc Coated Steel ~ (includes post or loop caps, horizontal and brace rail ends, combination rail ends, boulevard clamps and all other miscellaneous fittings & hardware)	Nuts		A563	Hex Nuts for Pipe Clamp Connections	
Tension Wire	A824 & A817 Type II (Zinc Coated Steel Wire) - 7 gage, Class 4 Coating Type I (Aluminum Coated Steel Wire) - 7 gage		Washers		F 42C	Flat Washers for Pipe Clamp	
					F 436	Connections	
Hog Rings	F626	Zinc Coated Steel Wire - 12 gage	Bearing (Plain N	ı Pads leoprene)	-	In accordance with Specification Section 932 for Ancillary Structures	
Brace Rails	F 1083	Galvanized Steel Pipe – $1_4^{\prime\prime\prime}$ NPS, Schedule 40 Regular Grade					



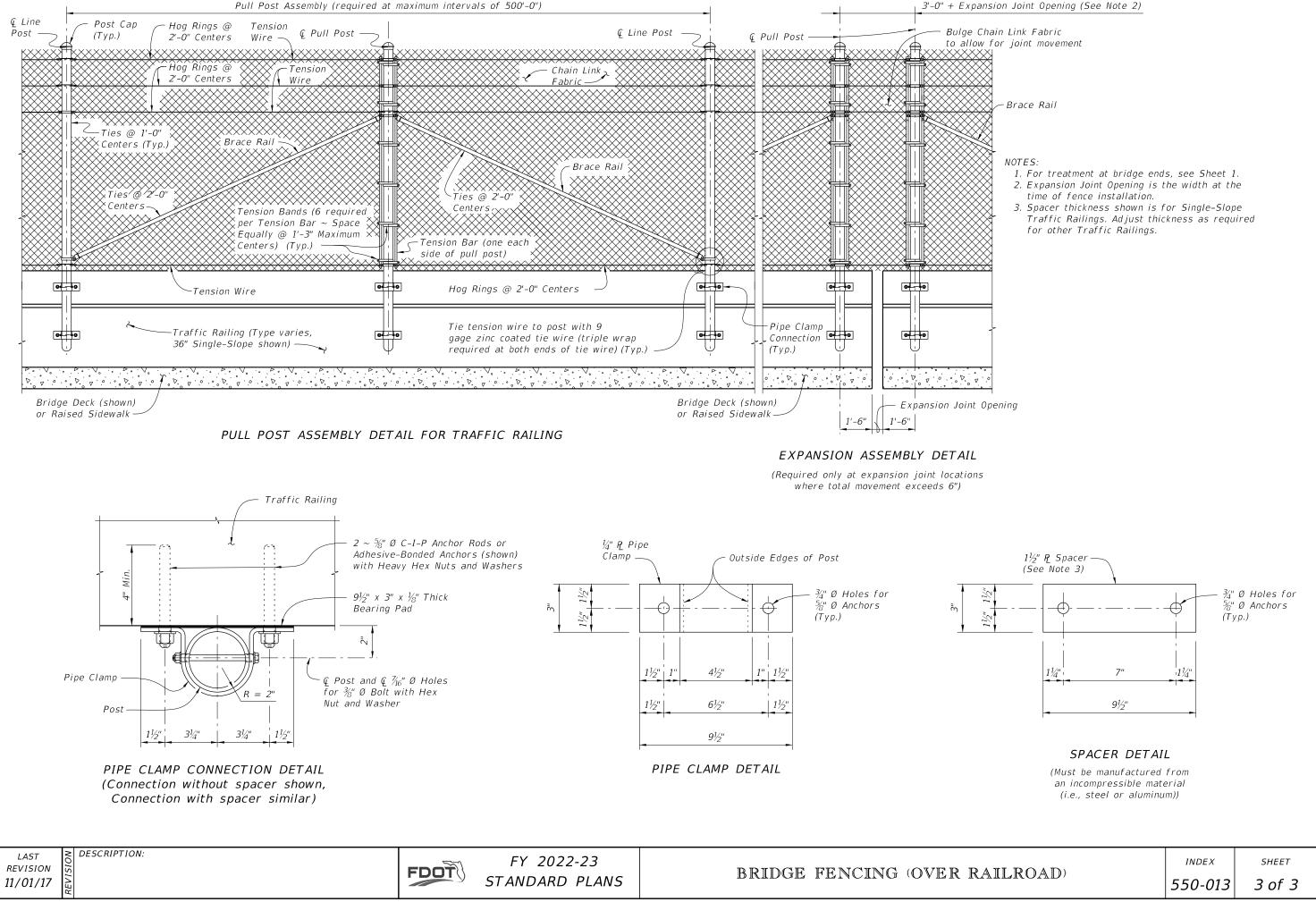
POST ATTACHMENT NOTES

- COATINGS:
- installation.

ANCHOR RODS, NUTS AND WASHERS: After the nuts have been tightened, distort the Anchor Rod threads to prevent removal of the nuts. Coat distorted threads and exposed trimmed ends of anchors with a galvanizing compound in accordance with Specification Section 562. Hot-dip galvanize all Nuts, Washers, Bolts, C-I-P Anchor Rods, Adhesive Anchors and Fence Framework (Posts, Internal Sleeves, Shim Plates, Base Plates, Pipe Clamps and Spacers) in accordance with Specification Section 962. Hot-dip galvanize Fence Framework after fabrication. ADHESIVE-BONDED ANCHORS AND DOWELS: Adhesive Bonding Material Systems for Anchors and Dowels will comply with Specification Section 937 and be installed in accordance with Specification Section 416. Cutting of reinforcing steel is permitted for drilled hole WELDING: All welding will be in accordance with the American Welding Society Structural Welding Code (Steel) ANSI/AWS D1.1 (current edition). Weld metal will be E60XX or E70XX. Nondestructive testing of welds is not required. CROSS REFERENCE: For location of View A-A and Detail "A" see Sheet 1. INDEX SHEET

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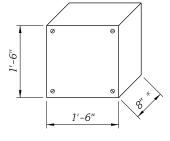
2 of 3



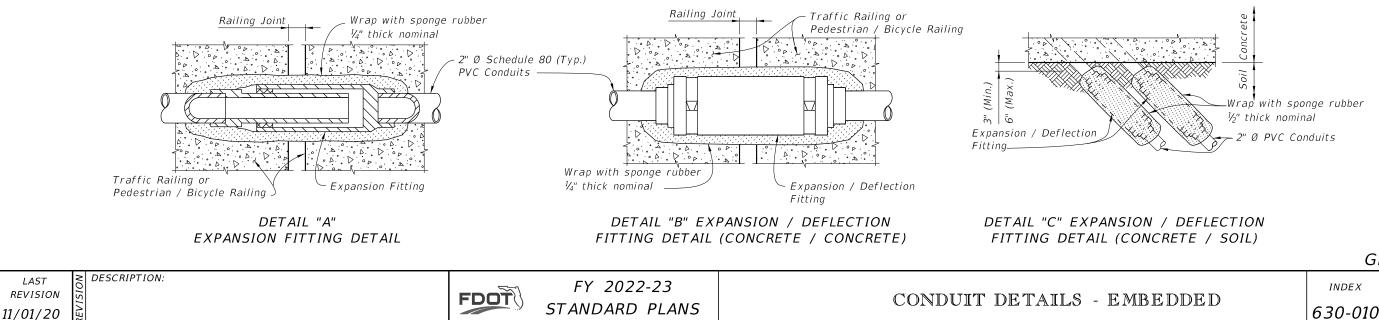
CONDUIT GENERAL NOTES:

- 1. Furnish and install approved Conduits, Fittings and Embedded Junction Boxes (EBJ's) in accordance with Specification Sections 630 and 635, this Standard, the National Electric Code (NEC) and as directed by the Engineer.
- 2. Furnish and install Embedded Junction Boxes (EJB) with weatherproof covers sized in accordance with NEC requirements and the maximum size limits shown. Install EJB adjacent to the Begin and End of Bridges, Begin and End of Retaining Walls, (except omit EJB adjacent to the Bridge unless a precast Traffic Railing with junction slab is used), and at other locations as necessary to maintain 300 foot maximum spacing. See Plans for additional locations and details.
- 3. For Conduit not designated for future use, see Plans for details. For Conduit designated for future use, stub out and cap the Conduit. Drive a 3'-0" \pm long $\frac{3}{4}$ " (min.) diameter Steel Pipe flush with the ground line adjacent to the end of the Conduit as shown on Sheets 2, 3 or 4. Provide the location of the stub out with Steel Pipe to the Engineer for inclusion on the As-Built Plans.
- 4. Shift vertical Railing reinforcement symmetrically to provide 2" clearance to EJB. Space shifted vertical reinforcement at minimum 3" centers. Cut horizontal Railing reinforcement to provide 2" clearance to EJB and provide supplemental reinforcement as shown. To facilitate placement of Conduit, Expansion Fittings, and Expansion/Deflection Fittings, shift reinforcing a maximum of 1" but do not cut railing reinforcing to facilitate Conduit or Fittings. Do not bundle Conduits, or Conduit and horizontal reinforcement.
- 5. Place conduits as indicated in this Standard unless Structures Plans indicate fewer.

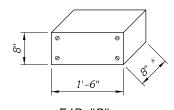




EJB "A" Double or Triple Conduit (Maximum Dimensions)



* Reduce to 6" maximum when installed in Pedestrian/ Bicycle Railings.



EJB "B" Single Conduit (Maximum Dimensions)

GENERAL

	01	
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