Detention Pond and Storm Report Lee's Famous Recipe Franklin

Located at

Section 26, Town 2, Range 5N MRs. City of Franklin, Warren County, Ohio

For: The Ideal Company Inc.

Date: 06/18/2025



VICINITY MAP

Prepared By:

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Lee's Famous Recipe Franklin Storm Water Detention Calculations Summary

City of Franklin, Warren County

Overview:

This report covers the design of the proposed detention basin to handle the increase of runoff from existing to developed conditions for the Lee's Famous Recipe Franklin site.

Site Description:

The Lee's Famous Recipe Franklin project is comprised of a 1.63 acre site which will contain a proposed commercial building and parking lot. In its pre-developed state, the site is an open space with grass cover in fair condition. In its post-developed state, the site will contain a proposed commercial building, parking lot and grass. The detention pond will be designed for the 5-year critical storm event to detain the increase of runoff produced by the increase in impervious surface area.

Hydrologic Methodologies:

The hydrologic methodology used for the design was the Soil Conservation Service (SCS) TR-55 Method. All supporting calculations can be seen on the attached computer printouts generated on USDA Win TR-55 and Civil3D Hydrographs.

Calculations Summary:

Below is a summary of the storm water detention calculations. Please refer to the attached computer printouts for additional details.

Hydrologic Soil Group C was used for these calculations. A soil map from the USDA Web Soil Survey website has been included in this report.

The proposed detention basin will adequately handle the increase in stormwater runoff from the added impervious area.

Total Drainage Area = 1.06 acres. The area of the site not routed to the detention pond will drain as it did under its pre-developed conditions.

Time of Concentration (Tc)

- Existing conditions = 10 minutes
- Developed conditions = 10 minutes

Runoff Coefficient (CN)

- Existing = 79 (Grass, Hydrologic Soil Group C, Fair Condition)
- Developed = 85 (User Defined 0.58 Acres Grass, Hydrologic Soil Group C, Fair Condition; 0.48 Acres Paved Parking Lots, Roofs, Driveways)

Pre-developed Peak Flow Rate		Developed Peak Flow Rate	
Storm Event	Peak Flow (cfs)	Storm Event	Peak Flow (cfs)
1 Year	1.24	1 Year	1.80
2 Year	1.67	2 Year	2.30
5 Year	2.36	5 Year	3.08
10 Year	2.97	10 Year	3.75
25 Year	3.86	25 Year	4.68
50 Year	4.38	50 Year	5.22
100 Year	4.90	100 Year	5.76

Critical Storm: The critical storm event was calculated to be the 5-Year critical storm. Therefore, the runoff from the 5-year storm under developed conditions shall be held back to the runoff from the 1-year storm event in the pre-developed state. The runoff for the 10, 25, 50 and 100-year storm events on the developed site shall be equal or less than the runoff from their respective storms for the pre-developed site.

The detention basin has a storage volume of 7,900 ft^3 and a top of pond elevation of 808.50 and a 20' spillway elevation of 808.10.

Detention Outlet Structure:

The detention basin will have a 2-2B outlet structure. The catch basin will have a 4" diameter orifice at an invert elevation of 805.10. The rim of the 2-2B catch basin has an elevation of 808.10. The outlet structure will discharge through a proposed 12" pipe at an elevation of 805.10.

The table below shows the summary of the detention basin.

Storm Event	Peak Inflow	Peak Outflow	Storage Provided	Max. Elevation of Pond
	(cfs)	(cfs)	(ft ³)	(ft)
1 Year	1.24	0.41	1,526	806.39
2 Year	1.67	0.46	2,074	806.64
5 Year	2.36	0.52	2,962	807.01
10 Year	2.97	0.57	3,756	807.28
25 Year	3.86	0.62	4,943	807.67
50 Year	4.38	0.65	5,653	807.89
100 Year	4.90	0.67	6,382	808.09







USDA Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey



USDA Natural Resources Conservation Service

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
UFhA	Urban land-Fincastle, Southern Ohio Till Plain, complex, 0 to 2 percent slopes	B/D	0.0	0.8%
UMrC2	Urban land-Miamian- Russell complex, 6 to 12 percent slopes, eroded		2.7	99.2%
URvB2	Urban land-Russell- Miamian complex, 2 to 6 percent slopes, moderately eroded	С	0.0	0.0%
Totals for Area of Intere	est	1	2.8	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

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WinTR-55 Current Data Description

--- Identification Data ---

User:GWGDate:4/24/2025Project:108.25Units:EnglishSubTitle:Lee's Original Recipe FranklinAreal Units:AcresState:OhioVarrenCounty:WarrenFilename:T:\BES_2025\108.25_Lee's Famous Recipe_Franklin\Eng\STORM\108 Ex.w55

--- Sub-Area Data ---

Name	Description	ı	Reach	Area(ac)	RCN	Тс	
Existing			Outlet	1.05	79	.105	
Total area:	1.05 (ac)						
		St	orm Data				
	Rainfal	ll Depth b	y Rainfall Ret	urn Period			
2-Yr (in)	5-Yr (in)	10-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	1-Yr (in)	
2.9	3.5	4.0	4.7	5.1	5.5	2.5	
Storm Data S Rainfall Dis Dimensionles	ource: tribution Type s Unit Hydrogı	War S: Typ raph: <st< td=""><td>ren County, OH e II andard></td><td>(NRCS)</td><td></td><td></td><td></td></st<>	ren County, OH e II andard>	(NRCS)			
						======	-
GWG	Le	ee's Origi Warren	108.25 nal Recipe Fran County, Ohio	nklin			
		S	torm Data				
	Rainfal	ll Depth b	y Rainfall Ret	urn Period			
2-Yr (in)	5-Yr (in)	10-Yr (in)	25-Yr (in)	50-Yr (in)	100-Yr (in)	l-Yr (in)	
2.9	3.5	4.0	4.7	5.1	5.5	2.5	
Storm Data S Rainfall Dis Dimensionles	ource: tribution Type s Unit Hydrogn	War 2: Typ raph: <st< td=""><td>ren County, OH e II andard></td><td>(NRCS)</td><td></td><td></td><td>_</td></st<>	ren County, OH e II andard>	(NRCS)			_
GWG	L6	ee's Origi Warren	108.25 nal Recipe Fra County, Ohio	nklin			:

WinTR-55, Version 1.00.10

Sub-Area Summary Table

Sub-Area Identifier	Drainage Area (ac)	Time c Concentra (hr)	of Curv tion Numb	e Receix er Read	ring Su zh Des	ub-Area scription	
Existing	1.05	0.105	79 ⁷	Outle	∙ ≥t		
Total Area:	1.05 (a	с)					
GWG		Lee's (Wa	108.2 Driginal Re Mrren Count	5 cipe Frank y, Ohio	clin		
	S	ub-Area Ti	me of Conc	entration	Details		
Sub-Area Identifier/	Flow Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
Existing SHEET SHALLOW	100 225	0.0981 0.0717	0.150 0.050				0.091 0.014
				Ti	ime of Conc	entration :	.105
GWG			108.2	5			

Lee's Original Recipe Franklin Warren County, Ohio

Sub-Area Land Use and Curve Number Details

10	otal Area / Weighted Curve Number			1.05	79
T/					
Existing Op	pen space; grass cover 50% to 75%	(fair) C	1.05	79
Sub-Area Identifier	Land Use		Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

Hydrograph type	= SCS Runoff	Peak discharge	= 1.236 cfs
Storm frequency	= 1 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 3,292 cuft
Drainage area	= 1.050 ac	Curve number	= 79
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 2.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

Hydrograph type	= SCS Runoff	Peak discharge	= 1.668 cfs
Storm frequency	= 2 yrs	Time to peak	= 12.03 hrs
Time interval	= 2 min	Hyd. volume	= 4,386 cuft
Drainage area	= 1.050 ac	Curve number	= 79
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 2.90 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

Hydrograph type	= SCS Runoff	Peak discharge	= 2.360 cfs
Storm frequency	= 5 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 6,155 cuft
Drainage area	= 1.050 ac	Curve number	= 79
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 3.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

Hydrograph type	= SCS Runoff	Peak discharge	= 2.973 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 7,718 cuft
Drainage area	= 1.050 ac	Curve number	= 79
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 4.00 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

Hydrograph type	= SCS Runoff	Peak discharge	= 3.859 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 10,004 cuft
Drainage area	= 1.050 ac	Curve number	= 79
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 4.70 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

Existing

Hydrograph type	= SCS Runoff	Peak discharge	= 4.375 cfs
Storm frequency	= 50 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 11,351 cuft
Drainage area	= 1.050 ac	Curve number	= 79
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 5.10 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Thursday, 04 / 24 / 2025

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 1

Existing

Hydrograph type	= SCS Runoff	Peak discharge	= 4.897 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 12,722 cuft
Drainage area	= 1.050 ac	Curve number	= 79
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 5.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



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WinTR-55 Current Data Description

--- Identification Data ---

User:GWGDate:6/17/2025Project:108.25Units:EnglishSubTitle:Lee's Original Recipe FranklinAreal Units:AcresState:OhioOhioCounty:WarrenFilename:T:\BES_2025\108.25_Lee's Famous Recipe_Franklin\Eng\STORM\108 Prop.w55

--- Sub-Area Data ---

Name	Description	Reach	Area(ac)	RCN	Tc	
Proposed	Pond Drainage	Outlet	1.06	85	0.1	
Total area:	1.06 (ac)					

--- Storm Data --

Rainfall Depth by Rainfall Return Period

2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	1-Yr
(in)	(in)	(in)	(in)	(in)	(in)	(in)
2.9	3.5	4.0	4.7	5.1	5.5	2.5

Storm Data Source:	Warren County, OH (NRCS)
Rainfall Distribution Type:	Type II
Dimensionless Unit Hydrograph:	<standard></standard>

GWG

108.25 Lee's Original Recipe Franklin Warren County, Ohio

Storm Data

Rainfall Depth by Rainfall Return Period

2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	1-Yr
(in)	(in)	(in)	(in)	(in)	(in)	(in)
2.9	3.5	4.0	4.7	5.1	5.5	2.5

Storm Data Source:Warren County, OH (NRCS)Rainfall Distribution Type:Type IIDimensionless Unit Hydrograph:<standard>

GWG

108.25 Lee's Original Recipe Franklin Warren County, Ohio

Watershed Peak Table

Sub-Area	Pea	ak Flow by R	ainfall R	leturn Peri	od		
or Reach	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr	1-Yr
Identifier	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)	(cfs)
SUBAREAS Proposed	2.46	3.30	4.00	5.01	5.58	6.16	1.93
REACHES							
OUTLET	2.46	3.30	4.00	5.01	5.58	6.16	1.93
GWG		Lee's Orig Warre	108.25 jinal Reci m County,	pe Frankli Ohio	n		
		Hydrograph	Peak/Peak	Time Tabl	e		
Sub-Area or Reach Identifier	Peak F] 2-Yr (cfs) (hr) (ow and Peak 5-Yr (cfs) hr) (h	Time (hr 10-Yr (cfs) r) (r) by Rainf 25-Yr (cfs) hr) (all Retur 50-Yr (cfs) hr)	n Period 100-Yr (cfs) (hr)	l-Yr (cfs) (hr)
SUBAREAS Proposed	2.46 11.93 11	3.30 94 11.	4.00 93 11	5.01 93 11	5.58 .93 1	6.16 1.93	1.93 11.94
REACHES							
OUTLET	2.46	3.30	4.00	5.01	5.58	6.16	1.93
GWG		Lee's Orig Warre	108.25 Jinal Reci n County,	pe Frankli Ohio	n		
		Sub-Ar	ea Summar	y Table			
Sub-Area Identifier	Drainage Area ((ac)	Time of Concentratic (hr)	Curve n Number	Receivin Reach	g Sub Desc	-Area ription	
Proposed	1.06	0.100	85	Outlet	Pond D	rainage	
Total Area	: 1.06 (ac)	I Contraction of the second					
GWG	Sub	Lee's Orig Warre D-Area Time	108.25 final Reci on County, of Concen	pe Frankli Ohio Ntration De	n tails		

Sub-Area Identifie	Flow c/ Length (ft)	Slope (ft/ft)	Mannings's n	End Area (sq ft)	Wetted Perimeter (ft)	Velocity (ft/sec)	Travel Time (hr)
Proposed SHEET	100	0.1000	0.011				0.011
				Ti	me of Concer	ntration =	0.1
GWG		Lee's (Wa	108.25 Driginal Rec Marren County	ipe Frank , Ohio	lin		
	Sub-	-Area Land	l Use and Cu	rve Numbe	r Details		
Sub-Area Identifie	c I	and Use			Hydrologic Soil Group	Sub-Area Area (ac)	Curve Number
Proposed	Open space; Paved parkir	grass cov ng lots, r	ver > 75% coofs, drive	(good) ways	l) C C	.58 .48	74 98
	Total Area /	Weighted	l Curve Numb	er		1.06	85 ==

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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 2

Hydrograph type	= SCS Runoff	Peak discharge	= 1.798 cfs
Storm frequency	= 1 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 4,676 cuft
Drainage area	= 1.060 ac	Curve number	= 85
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 2.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 2

Hydrograph type	= SCS Runoff	Peak discharge	= 2.303 cfs
Storm frequency	= 2 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 5,970 cuft
Drainage area	= 1.060 ac	Curve number	= 85
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 2.90 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 2

Hydrograph type	= SCS Runoff	Peak discharge	= 3.082 cfs
Storm frequency	= 5 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 8,001 cuft
Drainage area	= 1.060 ac	Curve number	= 85
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 3.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 2

Hydrograph type	= SCS Runoff	Peak discharge	= 3.745 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 9,753 cuft
Drainage area	= 1.060 ac	Curve number	= 85
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 4.00 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 2

Hydrograph type	= SCS Runoff	Peak discharge	= 4.681 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 12,269 cuft
Drainage area	= 1.060 ac	Curve number	= 85
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 4.70 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 2

Proposed

Hydrograph type	= SCS Runoff	Peak discharge	= 5.219 cfs
Storm frequency	= 50 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 13,732 cuft
Drainage area	= 1.060 ac	Curve number	= 85
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 5.10 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 2

Proposed

Hydrograph type	= SCS Runoff	Peak discharge	= 5.758 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.00 hrs
Time interval	= 2 min	Hyd. volume	= 15,209 cuft
Drainage area	= 1.060 ac	Curve number	= 85
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 10.00 min
Total precip.	= 5.50 in	Distribution	= Type II
Storm duration	= 24 hrs	Shape factor	= 484



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Critical Storm Calculation

The Design was based on the Critical Storm Event for the proposed drainage area which was calculated as follows using the hydraulic volume calculated for the one-year storm event using Civil 3-D Hydrographs:

 $\frac{\text{Post 1 Year} - \text{Pre 1 Year}}{\text{Pre 1 Year}} \Rightarrow \frac{4,676 \text{ ft}^3 - 3,292 \text{ ft}^3}{3,292 \text{ ft}^3} \Rightarrow 42\% \Rightarrow 5 \text{ Year Critical Storm}$

Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Pond Data

Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	805.10	n/a	0	0
0.40	805.50	n/a	128	128
0.90	806.00	n/a	648	775
1.40	806.50	n/a	966	1,741
1.90	807.00	n/a	1,204	2,945
2.40	807.50	n/a	1,428	4,373
2.90	808.00	n/a	1,659	6,032
3.40	808.50	n/a	1,868	7,900

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 12.00	4.00	0.00	0.00	Crest Len (ft)	= 0.00	0.00	0.00	0.00
Span (in)	= 12.00	4.00	0.00	0.00	Crest El. (ft)	= 0.00	0.00	0.00	0.00
No. Barrels	= 1	1	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 805.10	805.10	0.00	0.00	Weir Type	=			
Length (ft)	= 22.00	0.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 2.00	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	y Wet area))	
Multi-Stage	= n/a	Yes	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Weir Structures

Stage / Storage / Discharge Table

-	-												
Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	805.10	0.00	0.00									0.000
0.40	128	805.50	0.19 ic	0.18 ic									0.185
0.90	775	806.00	0.34 ic	0.33 ic									0.331
1.40	1,741	806.50	0.44 ic	0.44 ic									0.436
1.90	2,945	807.00	0.52 ic	0.52 ic									0.521
2.40	4,373	807.50	0.60 ic	0.60 ic									0.597
2.90	6.032	808.00	0.67 ic	0.66 ic									0.663
3.40	7,900	808.50	0.74 ic	0.72 ic									0.724

Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Pond No. 1 - POND

Pond Data

Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	805.10	n/a	0	0
0.40	805.50	n/a	128	128
0.90	806.00	n/a	648	775
1.40	806.50	n/a	966	1,741
1.90	807.00	n/a	1,204	2,945
2.40	807.50	n/a	1,428	4,373
2.90	808.00	n/a	1,659	6,032
3.40	808.50	n/a	1,868	7,900

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 12.00	4.00	0.00	0.00	Crest Len (ft)	= 0.00	0.00	0.00	0.00
Span (in)	= 12.00	4.00	0.00	0.00	Crest El. (ft)	= 0.00	0.00	0.00	0.00
No. Barrels	= 1	1	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 805.10	805.10	0.00	0.00	Weir Type	=			
Length (ft)	= 22.00	0.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 2.00	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	/Wet area)		
Multi-Stage	= n/a	Yes	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Weir Structures



Stage (ft)

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Pond Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Pond Data

Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	805.10	n/a	0	0
0.40	805.50	n/a	128	128
0.90	806.00	n/a	648	775
1.40	806.50	n/a	966	1,741
1.90	807.00	n/a	1,204	2,945
2.40	807.50	n/a	1,428	4,373
2.90	808.00	n/a	1,659	6,032
3.40	808.50	n/a	1,868	7,900

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 12.00	4.00	0.00	0.00	Crest Len (ft)	= 0.00	0.00	0.00	0.00
Span (in)	= 12.00	4.00	0.00	0.00	Crest El. (ft)	= 0.00	0.00	0.00	0.00
No. Barrels	= 1	1	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 805.10	805.10	0.00	0.00	Weir Type	=			
Length (ft)	= 22.00	0.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 2.00	0.00	0.00	n/a	-				
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	Wet area)		
Multi-Stage	= n/a	Yes	No	No	TW Elev. (ft)	= 0.00	,		

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Weir Structures



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 3

<no description>

Hydrograph type	= Reservoir	Peak discharge	= 0.414 cfs
Storm frequency	= 1 yrs	Time to peak	= 12.27 hrs
Time interval	= 2 min	Hyd. volume	= 4,673 cuft
Inflow hyd. No.	= 2 - Proposed	Max. Elevation	= 806.39 ft
Reservoir name	= POND	Max. Storage	= 1,526 cuft
Reservoir name	= POND	Max. Storage	= 1,526 cuft

Storage Indication method used.



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Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 3

<no description>

Hydrograph type	= Reservoir	Peak discharge	= 0.461 cfs
Storm frequency	= 2 yrs	Time to peak	= 12.27 hrs
Time interval	= 2 min	Hyd. volume	= 5,967 cuft
Inflow hyd. No.	= 2 - Proposed	Max. Elevation	= 806.64 ft
Reservoir name	= POND	Max. Storage	= 2,074 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 3

<no description>

= Reservoir	Peak discharge	= 0.522 cfs
= 5 yrs	Time to peak	= 12.30 hrs
= 2 min	Hyd. volume	= 7,997 cuft
= 2 - Proposed	Max. Elevation	= 807.01 ft
= POND	Max. Storage	= 2,962 cuft
	= Reservoir = 5 yrs = 2 min = 2 - Proposed = POND	= ReservoirPeak discharge= 5 yrsTime to peak= 2 minHyd. volume= 2 - ProposedMax. Elevation= PONDMax. Storage



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 3

<no description>

Hydrograph type	= Reservoir	Peak discharge	= 0.565 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.33 hrs
Time interval	= 2 min	Hyd. volume	= 9,749 cuft
Inflow hyd. No.	= 2 - Proposed	Max. Elevation	= 807.28 ft
Reservoir name	= POND	Max. Storage	= 3,756 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 3

<no description>

Hydrograph type	= Reservoir	Peak discharge	= 0.620 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.43 hrs
Time interval	= 2 min	Hyd. volume	= 12,265 cuft
Inflow hyd. No.	= 2 - Proposed	Max. Elevation	= 807.67 ft
Reservoir name	= POND	Max. Storage	= 4,943 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 3

<no description>

Hydrograph type	= Reservoir	Peak discharge	= 0.648 cfs
Storm frequency	= 50 yrs	Time to peak	= 12.47 hrs
Time interval	= 2 min	Hyd. volume	= 13,728 cuft
Inflow hyd. No.	= 2 - Proposed	Max. Elevation	= 807.89 ft
Reservoir name	= POND	Max. Storage	= 5,653 cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Hyd. No. 3

<no description>

Hydrograph type	= Reservoir	Peak discharge	= 0.675 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.50 hrs
Time interval	= 2 min	Hyd. volume	= 15,205 cuft
Inflow hyd. No.	= 2 - Proposed	Max. Elevation	= 808.09 ft
Reservoir name	= POND	Max. Storage	= 6,382 cuft





Brumbaugh Engineering & Surveying, LLC 2270 South Miami Street West Milton, OH 45383 Ph 937-698-3000, Fax 937-698-3928

Storm	Pipe Design	

DI 025 (00		11 43505											CILIC		
Ph 937-698-	-3000, Fax	937-698	-3928									Des:	GWG		
			Based	on Wa	rren Co.	5yr. Sto	rm					ckd:	JJB		
	Area			Equiv.	Total		Rain	Req.	Pipe		Vel.	Q	Vel.		Time
Location	Trib	Accum	C	Area	EqArea	Tc	Int	Q	Diam.	Slope	full	Cap.	Act	Len	Flow
Description	(Ac)	(Ac)		(Ac)	(Ac)	(min.)	(in.)	(cfs)	(in.)	(%)	(f/s)	(cfs)	(f/s)	(ft)	(min.)
Prop. CB 1 to															+
Prop. HW 1															
CB 1	0.46	0.46	0.60	0.28	0.28	10.0	4.82								
								1.3	12	0.25	2.5	1.9	2.7	113'	0.7
CB 3	0.11	0.85	0.90	0.10	0.57	10.7	4.69								
								2.7	12	0.60	3.8	3.0	4.2	11'	0.0
HW 1						10.7									
Prop. CB 2 to														<u> </u>	+
Prop. CB 3															
CB 2	0.28	0.28	0.70	0.20	0.20	10.0	4.82								
								0.9	12	0.25	2.5	1.9	2.7	93'	0.6
CB 3						10.6									
	_													ļ	
														ļ	
* As-Built Slope														<u> </u>	
Hydraulic Grade shown	in blue														
Inyuraulie Olade silowii	m onde.														

4/24/2025

Brumbaugh Eng	ة gineering	& Survey	ing, LI	.C						Job:	Ι	ee's Famo	ous Recip	e Frankli	n
2270 S	outh Miar	mi Street										Storm Pi	ipe Desig	'n	
West 1 Ph 937-698-	Milton, Ol 3000, Fax	H 45383 937-698-	3928 Based	on War	ren Co	100vr S	torm					Des:	GWG		
	Area		Dascu	Equiv.	Total	10091.5	Rain	Req.	Pipe		Vel.	Q Q	Vel.		Time
Location	Trib	Accum	С	Area	EqArea	Тс	Int	Q	Diam.	Slope	full	Cap.	Act	Len	Flow
Description	(Ac)	(Ac)		(Ac)	(Ac)	(min.)	(in.)	(cfs)	(in.)	(%)	(f/s)	(cfs)	(f/s)	(ft)	(min.)
Prop. OS to	-														
Prop. HW 2															
OS	1.05	1.05	0.60	0.63	0.63	10.7	6.87						Ī		
								4.3	12	2.00	7.0	5.5	7.7	22'	0.0
HW 2	\square					10.7									
* As-Built Slope Hydraulic Grade shown i	n blue														