



TREE REMOVAL PERMIT APPLICATION

ARBORIST COSTS INVOICE WILL BE MAILED TO APPLICANT AFTER
PROCESSING IF CITY ARBORIST REVIEW IS REQUIRED

Applicant Information ☐ Property Owner ☒ Agent

Name: Whitney Blunt Company: Blu Fish Collaborative
Address: PO BOX 40792 City/Zip: Austin, TX 78704 Phone: 512-388-4115 x 1
Email: whitney@bfcollaborative.com State Contractor License # N/A

Property Owner Information (if different):

Name: Dripping Springs ISD Phone: 512-858-3000
Address: 300 Sportsplex Dr., Dripping Springs, TX 78620

Owner/Agent Statement

Property Owner Consent—I am the legal owner of record of the land specified in this application or am authorized and empowered to act as an agent on behalf of the owner of record on all matters relating to this application. I declare that the foregoing is true and correct and accept that false or inaccurate owner authorization may invalidate or delay action on this application.

- A tree permit is non-transferable and must be kept on site when any work described in the permit is taking place.
- It is understood and agreed by the permittee that when any work is completed it shall constitute an acceptance of the permit general provisions.
- Any person who violates any provision of Dripping Springs City Ordinance Article 28.06 is subject to civil actions, and administrative penalties punishable by a fine not exceeding \$2,000.

Signature:  Date: 12/08/2025

Tree Information

☐ City Tree ☐ Residential: ☐ Front Yard ☐ Back Yard ☐ Side Yard
☒ Private Protected Tree ☒ Commercial

Proposed Activity: ☐ Prune ☒ Remove ☐ Plant ☐ Encroach into CRZ ☐ Other _____

Address/Location of Tree: Darden Hill Rd. & Sawyer Ranch Rd.

Number of Trees: 166 Tree Species and Diameter: See spreadsheet.

Reason for Action**: For mass grading and to build proposed high school and associated facilities.

**Any of the following items may be required to accompany this application:

- | | |
|-----------------------------------|---|
| ❖ Arborist report (if needed) | ❖ Authorization of the property owner |
| ❖ Landscape or tree planting plan | ❖ Tree replacement plan |
| ❖ Tree protection plan | |
| ❖ Site map | ❖ Any other information as deemed necessary |

DSISD HIGH SCHOOL #2 HERITAGE TREE INVENTORY			
TREE #	TREE SPECIES	HERITAGE TREE REMAIN	HERITAGE TREE REMOVED
1065	POST OAK		20
1068	POST OAK	24.5	
1091	POST OAK		19
1092	LIVE OAK (10, 15)		20
1098	LIVE OAK (5, 5, 10, 8, 9.5)		24
1150	POST OAK		25
1153	LIVE OAK		18
1215	LIVE OAK (10, 30)		35
1216	POST OAK		18.5
1230	POST OAK		19
1251	CEDAR ELM		22.5
1262	POST OAK		18
1273	POST OAK (16.5, 18.5)		27
1342	LIVE OAK		28
2178	LIVE OAK		20
2193	LIVE OAK	27	
4235	LIVE OAK	22.5	
4239	LIVE OAK	24.5	
4247	LIVE OAK	20.5	
4269	LIVE OAK	35	
4274	LIVE OAK	27	
4279	LIVE OAK		20
4290	LIVE OAK		18
4336	26" LIVE OAK - DEAD		
4339	LIVE OAK	20	
4340	LIVE OAK	22	
4341	LIVE OAK	27	
4348	LIVE OAK		19
4349	LIVE OAK		20
4350	LIVE OAK		19
4351	LIVE OAK		19
4352	LIVE OAK		18
4353	LIVE OAK		18
4358	LIVE OAK		18
4367	LIVE OAK		27.5
4371	LIVE OAK		18
4374	POST OAK (20, 21)		31
4375	LIVE OAK (19, 14.5, 10.5)		31.5
4378	LIVE OAK		18
4379	LIVE OAK		21
4380	LIVE OAK (15, 13, 16, 16, 8, 17)		51
4381	LIVE OAK		32
4383	LIVE OAK		21
5012	LIVE OAK	24	
5018	POST OAK	22.5	
5019	POST OAK (11, 11, 11)	22	
5024	POST OAK	19	
5027	CEDAR ELM (20.5, 6, 3, 12)		31
5048	LIVE OAK	18	
5056	LIVE OAK	20	
5057	LIVE OAK	19	
5067	LIVE OAK	18.5	
5111	POST OAK		26
5154	LIVE OAK	20	
5172	LIVE OAK		18
5181	LIVE OAK		19
5199	LIVE OAK		23.5
5326	LIVE OAK (13, 15.5)	22	
5349	LIVE OAK		23
5361	LIVE OAK		21
5369	CEDAR ELM (7.5, 5, 10, 7.5, 5)	22.5	
5384	18" LIVE OAK - DEAD		
5385	18.5" LIVE OAK - DEAD		
5387	18" LIVE OAK - DEAD		
5404	LIVE OAK	21.5	
5413	LIVE OAK (14.5, 17.5)	25	
5432	LIVE OAK (13.5, 11.5)		19
5484	LIVE OAK (12, 10.5, 10)	22	
5509	ASHE JUNIPER (9, 9, 3.5, 6)	18	
5560	LIVE OAK (14.5, 15)	22	
5561	LIVE OAK	22	
5562	LIVE OAK	21	
5563	LIVE OAK	25	
5566	LIVE OAK	23	
5571	LIVE OAK	22	
5597	LIVE OAK	29	
5600	LIVE OAK	21	
5601	LIVE OAK	20	
5616	LIVE OAK	21	
5618	LIVE OAK (13, 11, 17)	29	
5622	POST OAK		20
5626	LIVE OAK		18
5639	LIVE OAK		21
5641	LIVE OAK		18
5643	LIVE OAK		23
5644	LIVE OAK		20

5653	POST OAK		24
5656	POST OAK		22.5
5657	POST OAK		20
5659	POST OAK		19
5663	LIVE OAK (16.5, 7, 8.5)		24
5664	LIVE OAK		19
5670	LIVE OAK		20
5671	LIVE OAK		29
5672	LIVE OAK		21
5673	LIVE OAK		21
5683	LIVE OAK		21
5685	LIVE OAK		21
5686	LIVE OAK		25
5688	LIVE OAK (17, 15)		24.5
5694	POST OAK (12, 11.5)		18
5697	LIVE OAK		22
5714	LIVE OAK		22
5715	LIVE OAK (10, 16, 7.5)	25	
5910	LIVE OAK	30	
1322R	LIVE OAK		32.5
1336R	LIVE OAK (12.5, 10.5, 18, 10, 2)		35.5
1337R	29" LIVE OAK - DEAD		
1338R	LIVE OAK		21
1339R	LIVE OAK		27
1350R	LIVE OAK		18
5881R	LIVE OAK		18
5894R	LIVE OAK (13.5, 10.5)		19
5913R	LIVE OAK (10.5, 14)		19
1317E	LIVE OAK	22	
1321E	LIVE OAK		23
1353E	LIVE OAK	20	
1354E	LIVE OAK	22	
1357E	LIVE OAK (6, 5, 10, 11)	21.5	
1360E	LIVE OAK	21	
1362E	LIVE OAK	21	
1365E	LIVE OAK	21	
1366E	LIVE OAK	22	
1387E	LIVE OAK	21.5	
1388E	LIVE OAK	28.5	
5361E	LIVE OAK (9, 12, 20.5)	31	
5366E	LIVE OAK	24	
5368E	LIVE OAK	19	
5371E	LIVE OAK		19
5407E	LIVE OAK (16, 25)	33	
5618E	CEDAR ELM	18	
5619E	LIVE OAK	26	
5620E	22" LIVE OAK - DEAD		
5623E	22" LIVE OAK - DEAD		
5627E	LIVE OAK	19	
5628E	LIVE OAK	24.5	
5630E	LIVE OAK	20	
5633E	LIVE OAK	28	
5646E	LIVE OAK	21	
5705E	LIVE OAK	22	
5721E	LIVE OAK	19	
5729E	LIVE OAK	25	
5733E	CEDAR ELM (12.5, 12.5, 13.5)	26	
5734E	LIVE OAK	20	
5736E	LIVE OAK	23	
5740E	LIVE OAK (9.5, 14)	19	
5771E	18" LIVE OAK - DEAD		
5774E	LIVE OAK (14.5, 15)	22	
5786E	LIVE OAK	24	
5794E	LIVE OAK	19	
5806E	LIVE OAK	24	
5845E	LIVE OAK	18	
5872E	LIVE OAK	24	
5931E	LIVE OAK	19	
5957E	LIVE OAK	19	
5962E	LIVE OAK	20	
5967E	LIVE OAK (17.5, 21)	30	
5969E	LIVE OAK	20	
6005E	LIVE OAK	19	
6201E	LIVE OAK	26	
6204E	CEDAR ELM (12, 11, 8.5)	22	
6205E	CEDAR ELM (6, 4, 11, 11)	21.5	
6208E	LIVE OAK	19	
6209E	LIVE OAK	18	
6220E	LIVE OAK (5.5, 11, 10)	19	
6226E	LIVE OAK	20	
	TOTAL INCHES	1,915.50	1,639.50



CONSTRUCTION NOTES

1.0 MATERIALS

1.1 BACKFILL SOILS / DRAINAGE STONE

1.1.1 REINFORCED BACKFILL MATERIAL SPECIFIED BELOW SHALL BE FREE DRAINING. REINFORCED BACKFILL MATERIALS SHALL BE APPROVED BY THE OWNER OR OWNER'S REPRESENTATIVE AND SHALL MEET THE PHYSICAL PROPERTY REQUIREMENTS DEFINED IN SECTION 6.0. THE REINFORCED BACKFILL MATERIAL SHALL BE CRUSHED ANGULAR STONE MEETING THE FOLLOWING GRADATION:

SIEVE SIZE	PERCENT PASSING
4 inch	100
1 inch	0-100
3/4 inch	0-70
1/2 inch	0-40
No. 4	0-10

THE PORTION OF THE REINFORCED BACKFILL MATERIAL PASSING THE No. 40 SIEVE SHALL HAVE A LIQUID LIMIT OF LESS THAN 40 AND A PLASTICITY INDEX OF LESS THAN 20. REINFORCED BACKFILL MATERIAL SHALL BE CLASSIFIED PER THE UNIFIED SOIL CLASSIFICATION SYSTEM AS LOW PLASTICITY OR NON-PLASTIC SOILS.

1.1.2 SOIL FILL

SOIL FILL MATERIAL SHALL BE ON-SITE OR IMPORTED COMPRESSIBLE SOIL CLASSIFIED PER THE UNIFIED SOIL CLASSIFICATION SYSTEM AS LOW PLASTICITY (MAX PI=25), COMPACTED TO 95% STD. PROCTOR DENSITY.

1.2 GEGRID REINFORCING SHALL BE TENSAR UX1400 UNIAXIAL GEGRID AS MANUFACTURED BY THE TENSAR CORPORATION. DESIGNS PRESENTED HEREIN ARE VALID FOR TENSAR GEGRIDS OR ENGINEER APPROVED EQUAL.

1.3 WALL FACING SHALL BE CHOPPED LIMESTONE BLOCK.

1.4 GEOTEXTILE FABRIC SHALL BE MIRAFI 140N OR APPROVED EQUAL.

1.5 REINFORCING BARS SHALL BE ASTM A615, GRADE 60.

1.6 MORTAR SHALL BE IN ACCORDANCE WITH ASTM C270-10, TYPE N.

2.0 TECHNICAL REQUIREMENTS

2.1 PRIOR TO CONSTRUCTION OF THE GEGRID REINFORCED WALL, THE CONTRACTOR SHALL CLEAR AND GRUB THE REINFORCED BACKFILL ZONE, REMOVING TOPSOILS, BRUSH, SOD OR OTHER ORGANIC OR DELTERIOUS MATERIALS. ANY UNSUITABLE SOILS SHALL BE OVER-EXCAVATED, REPLACED AND COMPACTED WITH REINFORCED BACKFILL MATERIAL TO PROJECT SPECIFICATIONS OR AS OTHERWISE DIRECTED BY THE OWNER'S GEOTECHNICAL ENGINEER.

2.2 BACKFILL MATERIALS SHALL BE PLACED FROM THE BACK OF THE BLOCK FACING UNITS TOWARDS THE TAIL OF THE GEGRID TO ENSURE FURTHER TENSIONING.

2.3 REINFORCED BACKFILL SHALL BE PLACED IN HORIZONTAL LAYERS NOT EXCEEDING 8 INCHES IN UNCOMPACTED THICKNESS.

2.4 ONLY HAND-OPERATED EQUIPMENT SHALL BE ALLOWED WITHIN THREE FEET OF THE BACK FACE OF WALL. COMPACTION SHALL BE ACHIEVED BY A LIGHTWEIGHT MECHANICAL TAMPER, ROLLER OR VIBRATORY SYSTEM. CARE SHALL BE EXERCISED DURING THE COMPACTION PROCESS TO AVOID MISALIGNMENT OF THE BLOCK UNITS.

2.5 REINFORCED BACKFILL MATERIAL DOES NOT REQUIRE DENSITY TESTING. COMPACTION FOR THIS TYPE OF MATERIAL SHALL CONTINUE UNTIL THERE IS NO EVIDENCE OF FURTHER COMPACTION, OR AS DIRECTED BY THE OWNER'S GEOTECHNICAL ENGINEER. SHOULD THE SUBGRADE, FOR ANY REASON OR CAUSE, LOSE THE REQUIRED STABILITY OR FINISH, IT SHALL BE RECOMPACTED AND REFINISHED AT THE CONTRACTOR'S EXPENSE.

2.6 THE CONTRACTOR SHALL HAVE AN APPROVED SET OF CONSTRUCTION DRAWINGS AND CONTRACT SPECIFICATIONS ON-SITE AT ALL TIMES DURING CONSTRUCTION OF THE RETAINING WALL.

3.0 GEOGRID PLACEMENT

3.1 GEGRID SHALL BE PLACED AT THE LOCATIONS AND ELEVATIONS SHOWN ON THE CONSTRUCTION DRAWINGS.

3.2 GEGRID EMBEDMENT LENGTH (GEL) SHALL BE AS SHOWN ON THE CONSTRUCTION DRAWINGS. GEGRID EMBEDMENT LENGTH (GEL) IS MEASURED FROM THE FRONT FACE OF THE WALL EXTENDING TO THE TAIL OF THE GEGRIDS.

3.3 GEGRID REINFORCEMENT SHALL BE CONTINUOUS THROUGHOUT THE DESIGNATED EMBEDMENT LENGTH(S).

3.4 TRACKED CONSTRUCTION EQUIPMENT SHALL NOT BE OPERATED DIRECTLY ON THE GEGRID. A MINIMUM FILL THICKNESS OF SIX INCHES IS REQUIRED FOR OPERATION OF TRACKED VEHICLES OVER THE GEGRID. TURNING OF TRACKED VEHICLES SHOULD BE KEPT TO A MINIMUM TO PREVENT TRACKS FROM DISPLACING THE FILL AND/OR THE GEGRID.

3.5 RUBBER-TIRED VEHICLES MAY PASS OVER THE GEGRID REINFORCEMENT AT SLOW SPEEDS, LESS THAN 10 MPH. SUDDEN BRAKING AND SHARP TURNING SHALL BE AVOIDED.

3.6 UNIAXIAL GEGRID SHALL BE ROLLED OUT WITH THE LONG AXIS OF THE APERTURES (MACHINE DIRECTION) PERPENDICULAR TO THE WALL FACE.

3.7 UNIAXIAL GEGRIDS SHALL BE CUT NEXT TO THE CROSS-MACHINE DIRECTION BAR. THE CROSS-MACHINE DIRECTION BAR SHALL BE PLACED AND PULLED TAUT PRIOR TO FILL PLACEMENT.

3.8 A MINIMUM OF 3 INCHES OF FILL MATERIAL SHALL BE REQUIRED BETWEEN LAYERS OF UNIAXIAL GEGRID AND FILTER FABRIC UNLESS OTHERWISE SHOWN.

3.9 NO CHANGES TO THE GEGRID LAYOUT INCLUDING, BUT NOT LIMITED TO LENGTH, GEGRID TYPE OR ELEVATION SHALL BE MADE WITHOUT THE EXPRESSED PRIOR WRITTEN CONSENT OF HARRIS ENGINEERING GROUP.

4.0 BLOCK PLACEMENT

4.1 THE ALLOWABLE HORIZONTAL AND VERTICAL TOLERANCE FOR THE ERECTION OF THE WALLS SHALL BE LIMITED TO 1.5 inch IN 10.0 FEET OF LENGTH OR HEIGHT.

5.0 DRAINAGE

5.1 FOR WALLS NOT INCORPORATING FREE-DRAINING CRUSHED STONE BACKFILL, THE BACKFILL SURFACE SHALL BE GRADED AWAY FROM THE WALL FACE A MINIMUM OF 2 PERCENT SLOPE AND A TEMPORARY SOIL BERM SHALL BE CONSTRUCTED NEAR THE WALL CREST TO PREVENT SURFACE WATER RUNOFF FROM OVERTOPPING THE WALL. GRADING SHALL BE PERFORMED AT THE END OF EACH WORK DAY.

5.2 AT THE END OF EACH WORKDAY, BACKFILL SURFACE SHALL BE COMPACTED WITH A SMOOTH WHEEL ROLLER TO MINIMIZE PONDING OF WATER AND SATURATION OF THE BACKFILL.

5.3 PERMANENT SURFACE WATER DIVERSION AND/OR COLLECTION SHALL BE AS REQUIRED AND PROVIDED BY THE OWNER OR OWNER'S REPRESENTATIVE.

5.4 THE RETAINING WALL HAS BEEN DESIGNED ON THE ASSUMPTION THAT THE REINFORCED BACKFILL MATERIAL SHALL BE FREE OF SUBSURFACE DRAINAGE OF WATER (SEEPAGE). IF GROUND WATER IS ENCOUNTERED, HARRIS ENGINEERING GROUP SHALL BE CONTACTED IMMEDIATELY.

5.5 CARE SHALL BE TAKEN NOT TO CONTAMINATE THE GEOTEXTILE FABRIC AND/OR DRAINAGE STONE WITH FINE-GRAINED SOILS OR OTHER DELTERIOUS MATERIALS.

6.0 DESIGN PARAMETERS

6.1 DESIGN OF THE RETAINING WALLS IS BASED ON THE FOLLOWING PARAMETERS:

	EFFECTIVE FRICTION ANGLE	EFFECTIVE COHESION	MOIST UNIT WT
REINFORCED BACKFILL	34°	0 psf	120 pcf
RETAINED SOILS	22°	50 psf	120 pcf
FOUNDATION SOILS	22°	50 psf	120 pcf

6.2 FACTORS OF SAFETY:

6.2.1 INTERNAL STABILITY:

MINIMUM FACTOR OF SAFETY FOR GEGRID OVERSTRESS	DESIGN = 1.5	FHWA = 1.5
MINIMUM FACTOR OF SAFETY FOR GEGRID PULLOUT	DESIGN = 1.5	FHWA = 1.5
MINIMUM FACTOR OF SAFETY FOR SLIDING AT LOWEST GEGRID	DESIGN = 1.5	FHWA = 1.5
SOIL-GEGRID INTERACTION COEFFICIENT	DESIGN = 0.7	
PERCENT COVERAGE OF GEGRID	DESIGN = 100	

6.2.2 EXTERNAL STABILITY:

MINIMUM FACTOR OF SAFETY FOR SLIDING AT BASE	DESIGN = 1.5	FHWA = 1.5
MINIMUM FACTOR OF SAFETY FOR OVERTURNING	DESIGN = 2.0	FHWA = 2.0
SURCHARGE LOADING	DESIGN = 250 psf	

7.0 SPECIAL PROVISIONS

7.1 THE DESIGN PRESENTED HEREIN IS BASED ON SOIL PARAMETERS, FOUNDATION CONDITIONS, GROUNDWATER CONDITIONS, AND LOADINGS STATED IN SECTION 6.0.

7.2 LOCATIONS AND GEOMETRY OF EXISTING STRUCTURES AND GRADE ABOVE AND BELOW THE WALLS MUST BE VERIFIED BY THE OWNER OR OWNER'S REPRESENTATIVE PRIOR TO CONSTRUCTION.

7.3 THE OWNER OR OWNER'S REPRESENTATIVE IS RESPONSIBLE FOR REVIEWING AND VERIFYING THAT THE ACTUAL SITE CONDITIONS ARE AS DESCRIBED IN SECTION 6.0 PRIOR TO AND DURING CONSTRUCTION. THE OWNER OR OWNER'S REPRESENTATIVE SHALL BE ON-SITE TO ASSURE THE PROVISIONS IN THE CONSTRUCTION NOTES ARE FOLLOWED.

7.4 THE OWNER OR OWNER'S REPRESENTATIVE SHALL CONTACT HARRIS ENGINEERING GROUP IF THE SOILS ENCOUNTERED APPEAR TO VARY FROM THOSE ENCOUNTERED AT THE BEGINNING OF CONSTRUCTION.

7.5 IF ANY ROCK FORMATIONS AND/OR GROUNDWATER ARE ENCOUNTERED DURING THE CONSTRUCTION OF THIS WALL, IMMEDIATELY CONTACT THE OWNER OR OWNER'S REPRESENTATIVE.

7.6 ANY REVISIONS TO DESIGN PARAMETERS STATED IN SECTION 6.0 OR STRUCTURE GEOMETRY SHALL REQUIRE DESIGN MODIFICATIONS PRIOR TO PROCEEDING WITH CONSTRUCTION.

7.7 THIS DESIGN IS VALID ONLY FOR THE DRIPPING SPRINGS HIGH SCHOOL NO. 2 PROJECT, DRIPPING SPRINGS, TEXAS.

8.0 OWNER'S RESPONSIBILITIES

8.1 OWNER SHALL BE RESPONSIBLE FOR CONFIRMING THAT ALL REQUIREMENTS SET FORTH ON THESE DRAWINGS ARE MET. ASSIGNMENT OR DELEGATION OF RESPONSIBILITIES BY OWNER TO OWNER'S REPRESENTATIVE SHALL NOT RELIEVE OWNER OF RESPONSIBILITY OF CONFIRMING THAT ALL REQUIREMENTS SET FORTH HEREIN ARE MET.

8.2 OWNER (OR OWNER-DESIGNATED REPRESENTATIVES) RESPONSIBILITIES, AS DESCRIBED IN PREVIOUS SECTIONS OF THESE NOTES, SHALL INCLUDE:

8.2.1 PERMANENT SURFACE WATER DIVERSION (SECTION 5.0).

8.2.2 CONFIRMATION OF GEOMETRY AND LOADING CONDITIONS FOR AREAS ADJACENT TO WALL (SECTION 7.0).

8.2.3 ASSURING CONFORMITY WITH CONSTRUCTION DRAWINGS AND NOTES DURING CONSTRUCTION BY ON-SITE INSPECTION (SECTION 7.0).

REVISIONS



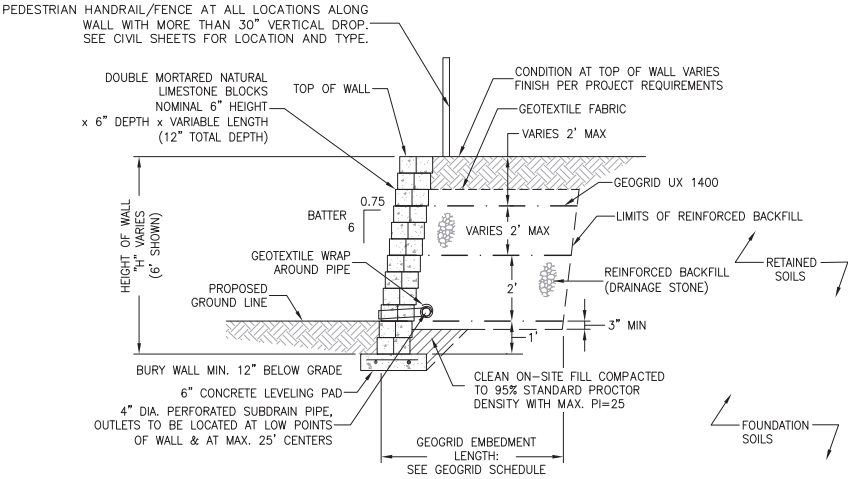
Phone: 512-797-7860
Reg. Eng. Firm #7-26488



DRIPPING SPRINGS HIGH SCHOOL NO. 2
RETAINING WALLS
DRIPPING SPRINGS, TEXAS

Scale: NOT TO SCALE
Date: 12/12/25
Drawn by: JF
Project No.: H225177

CONSTRUCTION NOTES



**TYPICAL CROSS-SECTION
LIMESTONE BLOCK WALL FILL CONDITION**
N.T.S.

GEOGRID SCHEDULE

HEIGHT OF WALL "H"	NO. OF LAYERS	GEOGRID EMBEDMENT LENGTH	GEOGRID TYPE
4.0'	2	5.0'	UX1400
5.0'	2	5.0'	UX1400
6.0'	3	5.0'	UX1400

- NOTES:
- 1) STEP TOP OF WALL TO CORRESPOND WITH SLOPE BEHIND WALL
 - 2) MINIMUM 5' GEOGRID LENGTH
 - 3) WALLS WITH "H" < 3.0' DO NOT REQUIRE GEOGRID

REVISIONS

STARS

STEVEN HARRIS
4436
10/2/20

Phone: 512-797-7860
Reg. Eng. Firm #7-26488

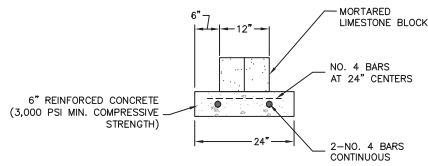
HARRIS
ENGINEERING GROUP

DRIPPING SPRINGS HIGH SCHOOL NO. 2
RETAINING WALL
DRIPPING SPRINGS, TEXAS

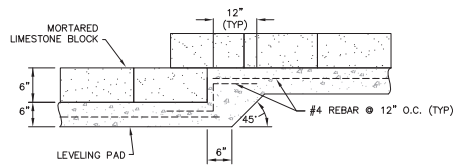
TYPICAL CROSS-SECTION

Scale: NOT TO SCALE
Date: 12/02/25
Drawn by: JF
Project No.: H6225171

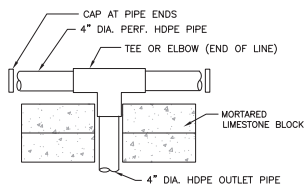
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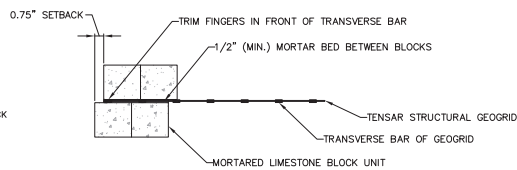
**TYPICAL SMALL LIMESTONE BLOCK MSE WALL
LEVELING PAD DETAIL**
N.T.S.



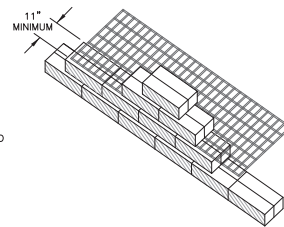
**TYPICAL SMALL LIMESTONE BLOCK MSE WALL
LEVELING PAD STEP DETAIL**
N.T.S.



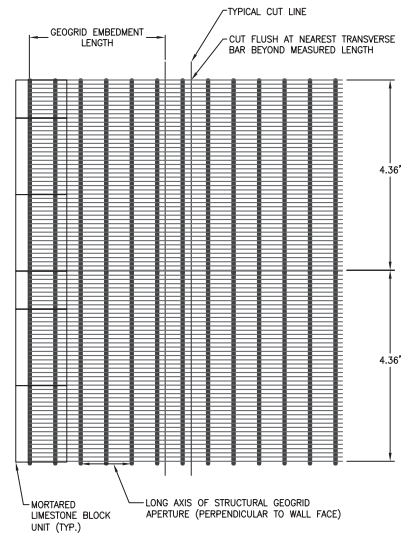
SUBDRAIN PIPE OUTLET DETAIL
N.T.S.



**NATURAL LIMESTONE BLOCK TO
GEOGRID CONNECTION DETAIL**
N.T.S.



**LIMESTONE BLOCK UNIT AND
GEOGRID CONNECTION DETAIL**
N.T.S.



GEOGRID ORIENTATION DETAIL
N.T.S.

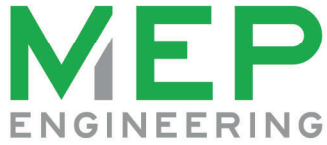
REVISIONS



Phone: 512-797-7860
Reg. Eng. Firm #7-26488
HARRIS
ENGINEERING GROUP

DRIPPING SPRINGS HIGH SCHOOL NO. 2
RETAINING WALL
DRIPPING SPRINGS, TEXAS
TYPICAL DETAILS

Scale: NOT TO SCALE
Date: 12/02/25
Drawn by: JF
Project No.: H025171
SHEET 3 OF 4



AUSTIN
1120 S Capital of Texas Hwy
Building 1, Suite 150
Austin, Texas 78746
512 306 9650

SAN ANTONIO
9830 Colonnade Blvd
Suite 230
San Antonio, Texas 78230
210 349 1400

Date: December 17, 2025

Ms. Julie Zitter
VLK Architects
2700 Via Fortuna, Suite 230
Austin, TX 78746

Dear Ms. Zitter:

This letter is in response to the "Conditional Approval" letter, dated November 19, 2025, from the City of Dripping Springs, referencing case "VAR2025-005 DRIPPING SPRINGS HIGH SCHOOL #2 CUT FILL VARIANCE." Comment #4 of that conditional approval stated the following: "Per [Site Development Ordinance 28.04.015 (i)] compliance with the City's lighting ordinance is a condition of variance approval. This site shall comply with the City's current lighting ordinance."

We respond as follows, to comment 4:

The exterior site lighting has been designed with the intent to fully comply with the City of Dripping Springs exterior lighting ordinance 24.06.

Sincerely,

A handwritten signature in red ink that reads 'Daniel Smith'.

Daniel Smith, PE
Electrical Engineer