- 1) INTERNATIONAL BUILDING CODE (IBC), 2018 EDITION 2) MANUAL FOR TIMBER CONSTRUCTION, AITC 4TH EDITION.
- 3) BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, ACI 318-19.
- 4) MANUAL OF STEEL CONSTRUCTION, AISC 2016 15TH EDITION. 5) COLD-FORMED STEEL DESIGN MANUAL, AISI CFS D100-13.

### B. DESIGN LOADS AND CRITERIA

1) UNIFORM GRAVITY LOADS (PSF):

LOCATION DEAD LOAD LIVE LOAD

RDDF47 (+ UNBALANCED DRIFTING)

### SNOW CRITERIA: GROUND SNOW LOAD = 56 PSF, Is= 1.1 EXPOSURE FACTOR Ce = 0.9, Ct = 1.1

2) WIND CRITERIA 3 SEC GUST WIND SPEED = 90 MPH

OCCUPANCY CATEGORY: II Iw = 1.0 / EXPOSURE 0

TOPOGRAPHIC ESCARPMENT Kzt= 1.28 INTERNAL PRESSURE COEFFICIENT: 0.18 ±

24 PSF MINIMUM FOR EXTERNAL WALL COMPONENTS & CLADDING 15 PSF MINIMUM NET UPLIFT FOR ROOF JOIST SPANSS LESS THAN 13FT,

16 PSF MINIMUM NET UPLIFT FOR ALL OTHER ROOF JOISTS

3) SEISMIC CRITERIA:

Ss = 0.15 / S1 = .04I = 1.0 / USE GROUP I

DESIGN CATEGORY A

AIR CONTENT 4.5% - 7.0%

ASTM C270, TYPE S

ASTM A615, GRADE 60, EXCEPT

ASTM C476 FINE, SLUMP XX"

2X4-2X10: HF #1 & BFTTFR.

POSTS: DF/L SELECT STRUCTURAL

ASTM A992, Fy = 50 KSI

ASTM A36, Fv = 36 KSI

AIR CONTENT 3.0% MAX INTERIOR SLABS

3/4" MAX NORMAL WEIGHT AGGREGATE

F7X-EXXX OR E70XX OR AS APPROVED

ANALYSIS PROCEDURE: SIMPLIFIED ANALYSIS LATERAL FORCE RESISTING SYSTEM:

STEEL MOMENT FRAMES NOT DETAILED FOR SEISMIC RESISTANCE

1,500 PSF ON APPROVED SUBGRADE, SEE SECTION D. 2 4) SOIL BEARING PRESSURE:

5) SOIL FRICTION COEFFICIENT:

35 PCF ACTIVE EQUIVALENT FLUID PRESSURE 200 PCF PASSIVE EQUIVALENT FLUID PRESSURE 65 PCF AT-REST EQUIVALENT FLUID PRESSURE

ASTM A706, GRADE 60, WHERE INDICATED TO BE WELDED.

ASTM F1554 GRADE 36 DR 55 W/ ASTM A563 HEAVY HEX NUTS

ASTM A325 TYPE 1 UNCOATED; STEEL TO STEEL CONNECTIONS

ASTM A307; WOOD OR WOOD TO STEEL CONNECTIONS OR ERECTION ONLY ASTM A108 GRADE 1010 - 1020, TYPE B, Fu = 60 KSI

ASTM A 36 ALL-THREAD ROD W/ CHISEL POINT & INJECTABLE ADHESIVE

OR HIT HY-20 W/ SCREEN TUBES FOR HOLLOW MASONRY OR AS APPROVED.

ASTM B 633, CLASS SC1, TYPE III (SIMPSON TITEN HD'S OR EQUIV)

SUCH AS HILTI HIT HY-150 FOR CONCRETE & SOLID MASONRY OR

"STRUCTURAL I" OR "SHEATHING" SUITED FOR SPAN & USE

ASTM C1107, NON-METALLIC NON-SHRINK, 3 DAY f'c = 4000 PSI

8) FROST DEPTH: 48 INCHES

# C. MATERIALS

1) CLASS A CONCRETE: (USE UNLESS

6) LATERAL SOIL PRESSURE:

PORTLAND CEMENT ASTM C150 TYPE I/II FLY ASH ASTM C618, 10% - 25% BY WEIGHT NOTED OTHERWISE) WATER / CEMENT + FLY ASH = 0.45 MAXIMUM 28 DAY f'c = 4000 PSI

2) REINFORCING BARS:

3) ANCHOR RODS: 4) GROUT:

5) MORTAR: 6) MASONRY GROUT:

7) STRUCTURAL STEEL: W SHAPES

DTHER ROLLED SHAPES ASTM A36, Fy = 36 KSI

PLATES 8) HIGH STRENGTH BOLTS: 9) BOLTS:

10) HEADED ANCHOR STUDS:

11) WELD METAL: 12) ADHESIVE ANCHORS:

13) SCREW ANCHORS 14) WOOD FRAMING:

15) WOOD SHEATHING / PANELS: AMERICAN PLYWOOD ASSOCIATION (APA) RATED

D. FOUNDATIONS

1) FOUNDATIONS HAVE BEEN DESIGNED BASED ON INFORMATION PRESENTED IN THE IBC. FOLLOW RECOMMENDATIONS UNLESS SPECIFICALLY INDICATED OTHERWISE.

2) PLACE SLAB ON FIRM UNDISTURBED NATIVE MATERIAL. WITH THE TOP 6" SCARIFIED AND COMPACTED

OR ENGINEERED FILL PLACED OVER FIRM UNDISTURBED NATIVE MATERIAL. REMOVE EXISTING TOPSOIL AND ROOT MASS. ENGINEERED FILL MATERIAL SHALL BE MINUS 3" GRADED GRANULAR, APPROVED BY THE GEDTECHNICAL ENGINEER. PLACE ENGINEERED FILL IN UNIFORM LIFTS AND COMPACT TO 98%

STANDARD PROCTOR ACCORDING TO ASTM D698. PLAN LIMITS OF ENGINEERED FILL MUST EXTEND AT LEAST 4'-O" BEYOND ALL FOOTING EDGES. IF ENCOUNTERED, EXISTING FILL SHALL BE REMOVED TO AN APPROVED DEPTH AND REPLACED WITH ENGINEERED FILL AS DESCRIBED ABOVE, PLACED AND COMPACTED AS DESCRIBED ABOVE. 3) DO NOT BACKFILL WALLS WITH UNBALANCED SOIL LEVELS UNLESS ADEQUATELY SHORED OR

PERMANENT FLOOR PLATES ARE INSTALLED AND CONNECTIONS ARE COMPLETE - THIS DOES NOT INCLUDE RETAINING WALLS. THE CONTRACTOR IS RESPONSIBLE FOR TEMPORARY SHORING DESIGN AND 5) BACKFILL AND COMPACT BURIED WALLS OR GRADE BEAMS EVENLY ON EACH SIDE TO AVOID UNBALANCED LOADS. COMPACT LAYERS TO 95% STANDARD PROCTOR ACCORDING TO ASTM D698 EXCEPT

92% UNDER NON-PAVED AREAS. 6) ALWAYS PROVIDE POSITIVE SURFACE WATER DRAINAGE AWAY FROM THE STRUCTURE.

E. CONCRETE

1) PERFORM CONCRETE WORK IN ACCORDANCE WITH ACI 301-02 "STANDARD SPECIFICATION FOR STRUCTURAL CONCRETE" UNLESS MORE STRINGENT REQUIREMENTS ARE INDICATED. 2) MINIMUM REINFORCING BAR COVER: 3" AT UNFORMED SURFACES EXPOSED TO EARTH

2" AT FORMED SURFACES EXPOSED TO EARTH OR WEATHER FOR #6 AND LARGER 1 1/2" AT FORMED SURFACES EXPOSED TO EARTH OR WEATHER FOR #3-#5 AND, NOT EXPOSED TO EARTH OR WEATHER FOR REINFORCMENT OF BEAMS OR COLUMNS 1½″ AT SLABS AND WALLS NOT EXPOSED TO EARTH OR WEATHER

3) SPLICE REINFORCING BARS BY LAPPING ACCORDING TO THE SCHEDULE INDICATED. SPLICE WWF SHEETS BY LAPPING AT LEAST DNE PANEL WIDTH (TWD LONGITUDINAL BARS IN CONTACT) DR 10 INCHE MINIMUM. PLACE MECHANICAL CONNECTORS WHERE SHOWN. 4) ADD #5X6'-0" DIAGONAL EACH FACE AT ALL OPENING CORNERS AND #5X6'-0" DIAGONAL MID-DEPTH

AT ALL RE-ENTRANT SLAB CORNERS UNLESS SHOWN OTHERWISE. 5) SECURE ALL REINFORCING, INCLUDING WWF, IN POSITION WITH CHAIRS BEFORE CONCRETE PLACEMENT. CONCRETE DOBIES MAY BE USED TO POSITION SLAB ON GRADE REINFORCEMENT.

6) TIE DOWELS IN PLACE BEFORE PLACING CONCRETE. DO NOT STAB OR "WET-SET" DOWELS. 7) INSTALL AND SECURE EMBEDMENTS SUCH AS ANCHOR RODS AND EMBEDMENT PLATES WITHIN SPECIFIED TOLERANCES BEFORE CONCRETE PLACEMENT.

8) MECHANICALLY VIBRATE ALL CONCRETE PLACEMENTS EXCEPT SLABS LESS THAN 5" THICK. 9) PROTECT AND CURE ALL CONCRETE SURFACES. BEGIN CURING WALLS IMMEDIATELY AFTER STRIPPING FORMS AND FLATWORK IMMEDIATELY AFTER FINISHING. 10) CONCRETE SURFACES TO RECEIVE GROUT UNDER COLUMN BASEPLATES MUST BE PREPARED BY

LIGHT BUSH HAMMERING (1/4" AMPLITUDE) THE GROUTED AREA AND PRE-SDAKING.

1) TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT ALL SUPERIMPOSED LOADS INDICATED IN SECTION "B" AND LOADS TRANSFERRED BY FRAMING MEMBERS (IE. OVERFRAMING, STRUCTURAL FASCIA,...) INDICATED ON ROOF FRAMING PLAN(S) AND ANY ADDITIONAL LOADS REQUIRED. TRUSS

2) ENGINEERED WOOD PRODUCTS (WOOD I-JOISTS & LAMINATED VENEER LUMBER) SHOWN ON THE DRAWINGS ARE THE PRODUCTS OF ROSEBURG FOREST PRODUCTS AND ARE INDICATED BY THE MANUFACTURER'S STANDARD PRODUCT NUMBERS. THE INTENT OF THE DESIGN IS FOR THESE ITEMS TO BE ATTACHED TO EACH OTHER AND TO THE SURROUNDING STRUCXTURE TO BEHAVE AS A SYSTEM. WHETHER SHOWN OR NOT, PROVIDE ACCESSORY ITEMS (BLOCKING, CLIPS, STIFFENERS, STRAPS, ETC.) DESIGNED BY THE MANUFACTURER FOR A COMPLETE SYSTEM. FOLLOW ALL MANUFACTURERS RECOMMENDATIONS FOR INSTALLATION AND USE.

3) FRAMING CONNECTORS, ANCHORS, AND HANGERS SHOWN ON THE DRAWINGS ARE THE PRODUCTS OF SIMPSON STONG-TIE COMPANY, SAN LEANDRO, CALIFORNIA AND ARE DESIGNATED BY THE MANUFACTURERS STANDARD PRODUCT NUMBERS. FOLLOW ALL MANUFACTUER'S RECOMMENDATIONS FOR INSTALLATION AND USE. PRODUCTS WITH EQUIVALENT CAPACITY AND QUALITY MAY BE SUBSTITUTED AFTER A SUBSTITUTION ENGINEER.

LAY PLYWOOD PANELS WITH FACE GRAIN PERPENDICULAR TO SUPPORTS. STAGGER ALL END JOINTS AND PLACE AS INDICATED IN "CASE 1" DF IBC TABLE 2306. 3. 1 MATERIAL

O. 131" DIAM AT 12" AT INTERMEDIATE SUPPORTS,

½″ 32/16 SPAN RATING 5) ALL LAG BOLTS SHALL HAVE LEAD HOLES DRILLED THE SMAE DIAMETER FOR THE SHANK AND 50% OF

THE SHANK DIAMETER OF THE THREADED PORTION. LUBRICATE THREADS BEFORE INSTALLATION. 6) STAGGER ALL END JOINTS 32" MINIMUM. FASTEN PANELS TO SUPPORTING FRAMING AND BLOCKING AS INDICATED. (SEE SHEAR WALL SCHEDULE AND FRAMING PLANS FOR CRITICAL NAILING.)

9) FASTENERS SHALL NOT BE LESS THAN 36" FROM PANEL EDGES. 8) NAIL HEADS SHALL NOT PENETRATE BEYOND A FLUSH CONDITION WITH FACE OF SHEATHING. 9) NAILING REQUIREMENTS NOT SPECIFIED ON THE CONSTRUCTION DOCUMENTS SHALL BE IN ACCORDANCE

10) FASTEN BUILT UP OR 2 PLY + MEMBERS TOGETHER PER IBC TABLE OR MANUFACTURERS RECOMMENDATIONS.

1) DETAIL, FABRICATE AND ERECT STRUCTURAL STEEL IN ACCORDANCE WITH THE LRFD, 3RD EDITION

OF AISC "MANUAL OF STEEL CONSTRUCTION AND AISC CODE OF STANDARD PRACTICE." 2) ALL STEEL TO STEEL BOLTED CONNECTIONS SHALL CONFORM TO THE CURRENT

TO A SNUG-TIGHT CONDITION UNLESS INDICATED TO BE PRE-TENSIONED. BRACE OR MOMETN FRAME AND DRAG / CHORD CONNECTIONS HAVE BEEN DESIGNED AS SLIP CRITICAL AND MUST PRE-TENSIONED. TENSION BOLTS INDICATED AS SUCH BY EMPLOYING ONE OF THE FOLLOWING METHODS: TENSION CONTROLLED BOLTS (TWIST-OFF BOLTS) PREFERRED,

4) PERFORM SHOP AND FIELD WELDING IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY'S STRUCTURAL WELDING CODE. SHOP OR FIELD WELDS AT NON-BOLTED CONNECTIONS THAT ARE NOT SPECIFICALLY DETAILED SHALL BE 3/16" CONTINUOUS FILLETS AT EACH CONTACT EDGE OR SUFACE.

6) HEADED ANCHOR STUD WELDING MUST CONFORM TO INSTALLATION SPECIFICATIONS PROVIDED BY

BY THE CONTRACTOR AND RE-TESTED AT THEIR EXPENSE.

SCHEDULED TO RECEIVE, SHOP OR FIELD INSTALLED, HEADED ANCHOR STUDS 9) THE ERECTOR SHALL NOT EMPLOY FIT-UP MEANS BEYOND THE USE OF DRIFT PINS OR MINOR HOLE REAMING. CORRECTION OF FIT-UP ERRORS OR MODIFICATIONS, INCLUDING ANCHOR RODS, OF ANY DEGREE SHALL BE DISCUSSED WITH THE FABRICATOR AND ENGINEER AND METHODS APPROVED BY

10) GROUT UNDER COLUMN BASEPLATES AFTER THE STRUCTURE IS ERECT AND PLUMB BUT BEFORE

1) SPECIAL INSPECTIONS DESCRIBED BELOW WILL BE PERFORMED UNDER SEPARATE CONTRACT BY THE ENGINEER APPRISED OF WORK PROGRESS AS IT PERTAINS TO SPECIAL INSPECTIONS TESTING REQUIREMENTS.

> REINFORCING STEEL: INSPECT BEFORE CONCRETE OR GROUT PLACEMENT. (INCLUDES SLABS-ON-GRADE AND ELEVATED SLABS)

REINFORCED CONCRETE: CONTINUOUS INSPECTION DURING CONCRETE PLACEMENT

AND ELEVATED SLABS.

(INCLUDES VERIFICATION OF PROPER MIX DESIGN AND CURING METHODS)

ANCHOR RODS: INSPECT ALL BEFORE CONCRETE PLACEMENT.

ADHESIVE ANCHORS: EMBEDMENT PLATES: INSPECT ALL BEFORE CONCRETE PLACEMENT.

PERIODIC INSPECTION DURING OR AFTER INSTALLATION. PERIODIC INSPECTION OF ALL WELDS. (INCLUDES STRUCTURAL STEEL, STEEL JOIST AND DECK,

HEADED ANCHOR STUDS, STAIRS & RAILING)

DOCUMENTATION, SUCH AS DETAIL DRAWINGS AND CALCULATIONS, FOR DEFERRED ITEMS WILL

1) OPEN WEB TRUSS

## K. MISCELLANEOUS

TYP

VERT

1) REFERENCE CIVIL DRAWINGS FOR BUILDING LOCATION AND ORIENTATION ON THE SITE.

2) DETAIL MARKS ANNOTATED ON PLANS ARE INTENDED TO INDICATE SPECIFIC CONFIGURATION(S) AND INFORMATION - FOR PLAN CLARITY, EVERY LOCATION WHERE A SPECIFIC DETAIL MAY

3) COORDINATE OPENINGS AND EMBEDDED ITEMS IN CONCRETE AND MASONRY WORK WITH ALL TRADES. 4) NOTIFY ENGINEER OF ANY DISCREPANCIES DISCOVERED WITH OTHER TRADES.

DIMENSIONS AND DETAILS WITH EQUIPMENT MANUFACTURERS AND TRADES. 7) TEMPORARILY BRACE THE STRUCTURE TO RESIST ALL LOADS OR COMBINATIONS OF LOADS UNTIL ALL PERMANENT ELEMENTS ARE IN PLACE AND ALL CONNECTIONS ARE COMPLETE AS SHOWN

8) COST ASSOCIATED WITH STRUCTURAL DRAWING CHANGES RESULTING FROM USE OF ALTERNATES OR SUBSTITUTIONS, INCLUDING MECHANICAL EQUIPMENT, ARE THE CONTRACTOR'S RESPONSIBILITY

DESIGNS MUST BE STAMPED WITH A SOUTH DAKOTA ENGINEERS SEAL ON THE DRAWINGS.

SUBMITTAL HAS BEEN PROVIDEDED BY THE GENERAL CONTRACTOR AND FINALAPPROVAL BY THE STRUCTURAL

4) FLOOR AND ROOF SHEATHING:

3/4" T&G, 40/20 MIN. SPAN RATING O.131"DIAMETER AT 6" AT ALL SUPPORTED PANEL EDGES,

GLUE AND NAIL TO SUPPORTING FRAMING O. 131" DIAMETER AT 6" AT ALL SUPPORTED PANEL EDGES, O. 131" DIAM AT 12" AT INTERMEDIATE SUPPORTS,

7) NO PANELS LESS THAN 12 INCHES WIDE SHALL BE USED.

WITH THE FASTENING SCHEDULE IN TABLE 2304. 9, 1 IN THE IBC.

G. STRUCTURAL STEEL

RCSC "SPECIFICATIONS FOR STRUCTURAL JOINTS" USING ASTM A325 BOLTS AS ENDORSED BY AISC 3) GENERALLY, BEAM CONNECTIONS HAVE BEEN DESIGNED AS BEARING TYPE AND BOLTS MAY BE INSTALLED

DIRECT TENSION INDICTOR (TENSION INDICATING WASHERS),  $\Box R$ TURN-DF-THE-NUT WITH COLOR MATCH-MARKING. COORDINATE BOLT TENSIONING WITH ENGINEER / INSPECTOR.

5) ALL WELDERS SHALL HAVE EVIDENCE OF PASSING THE AWS STANDARD QUALIFICATION TEST FOR THE TYPE OF WORK BEING PERFORMED.

THE STUD MANUFACTURER. 7) NON-DESTRUCTIVE WELD TESTS MAY BE PERFORMED. DEFICIENT WELDS WILL BE CORRECTED

8) DO NOT SHOP PAINT FAYING SURFACES OR PRE-TENSIONED BOLTED CONNECTIONS OR SURFACES

THEM BEFORE ACTIONS ARE TAKEN.

PLACMENT OF ELEVATED SLABS OR ROOFING.

## H. SPECIAL INSPECTIONS

AGENCIES RETAINED BY THE PROJECT OWNER. THE CONTRACTOR IS RESPONSIBLE FOR KEEPING AND ENSURING THAT NO WORK REQUIRING SPECIAL INSPECTIONS IS CONCEALED BEFORE SPECIAL INSPECTIONS OCCUR. SEE PROJECT SPECIFICATIONS FOR OTHER INSPECTIONS AND MATERIALS

EXCEPT PERIODIC INSPECTION FOR SLABS-ON-GRADE

PERIODIC INSPECTION DURING OR AFTER INSTALLATION.

STRUCTURAL STEEL: WELDING:

I. ABBREVIATIONS LIST - (SOME OF THE LISTED ABBREVIATIONS MAY NOT APPEAR ON THE DRAWINGS)

ALTERNATE BEARING BTWN BETWEEN CENTERLINE CLR CLEAR CONCRETE MASONRY UNIT CMU COLUMN CDNN CONNECTION / CONNECTOR CONT CONTINUE / CONTINUOUS ZAH HEADED ANCHOR STUD PROJ PROJECTION REINF REINFORCEMENT / REINFORCING REQ REQUIRED SIM SIMILAR STIFF STIFFENER THICK/THICKNESS

UNLESS NOTED OTHERWISE

## J. DEFERRED SUBMITTALS

BE REVIEWED BY THE ENGINEER WHEN AVAILABLE AND FORWARDED TO THE BUILDING OFFICIAL.

TYPICAL

VERTICAL

DRAWING ELEVATIONS REFERENCE CIVIL DATUM. APPLY IS NOT ANNOTATED. CONTACT THE ENGINEER IF CLARIFICATION IS NEEDED.

5) CONSTRUCTION LOADS SHALL NOT BE GREATER THAN THE DESIGN LOADS INDICATED IN SECTION B. 1 UNLESS REVIEWED AND APPROVED BY THE ENGINEER. 6) EQUIPMENT OPENINGS INDICATED ARE FOR REFERENCE ONLY. COORDINATE EXACT LOCATIONS,

TYP. FOOTING, 4'-0"x4'-0"x1'-0", w/ (5) #5 EACH WAY ON BOTTOM-(15'-10 1/2" MIN.) (15'-10 1/2" MIN.) TYP. 16"Ø PIER (-4'-0") \_\_\_ PRE-ENGINEERED ROOF TRUSSES @ 2'-0" -5/8" ROOF SHEATHING T.O.P.



T.O.P.

SCALE = 1/4"=1'-0"

THESE DOCUMENTS ARE THE PROPERTY OF LANCE ENGINEERIN

PRECISION DESIGN

LANCE ENGINEERING

605-642-3680

19751 ST. ONGE RD

ST. ONGE, SD 57779

AND SHALL NOT BE USED FOR ANY

REVISIONS

LANCEL

~REDINGER

OTHER PROJECT. RJT Drawn By

4/16/21

LE # 20253

Cad File S101.DWG SHEET TITLE:

FOUNDATION PLAN

Checked By LLR

SHEET#