PROPOSED BULDING ADDITTON FOR

Marty Nikodem Towne Court Town of Clayton Neenah,

ARCHITECTURAL / CIVIL

STRUCTURAL PLUMBING

- TITLE SHEET
- GENERAL INFORMATION GI.
- CI.O SITE PLAN
- $C \mid . \mid$ TOPOGRAPHIC SURVEY DRAINAGE AND GRADING PLAN C|.2
- CI.3
- LANDSCAPE PLAN CI.3
- C2. CONSTRUCTION DETAILS
- FLOOR PLAN A|.|
- DOOR & ROOM FINISH SCHEDULES A2.| \$ INTERIOR ELEVATIONS
- BUILDING ELEVATIONS A3.L
- A4.2 WALL SECTIONS

- SI.O GENERAL NOTES, DETAILS \$ SCHEDULES
- SI.I FOUNDATION PLAN

- EROSION AND SEDIMENT CONTROL PLAN OF 4 ANCHOR BOLT PLAN F2 OF 4 ANCHOR BOLT DETAILS

	1		2			3		4 EXCEPT MARINE		5 AND MARINE 4		5		7	1	8
CLIMATE ZONE	All other	Group R	All other	Group R	All other	Group R	All other	Group R	All other	Group R	All other	Group R	All other	Group R	All other	Group F
	0						Root	s								
Insulation entirely above deck	U-0.063	U-0.048	U-0.048	U-0.048	U-0.048	U-0.048	U-0.048	U-0.048	<mark>U-0.048</mark>	U-0.048	U-0.048	U-0.048	U-0.039	U-0.039	U-0.039	U-0.039
Metal buildings	U-0.065	U-0.065	U-0.055	U-0.055	U-0.055	U-0.055	U-0.055	U-0.055	U-0.055	U-0.055	U-0.049	U-0.049	U-0.049	U-0.049	U-0.035	U-0.035
Attic and other	U-0.034	U-0.027	U-0.027	U-0.027	U-0.027	U-0.027	U-0.027	U-0.027	U-0.027	U-0.027	U-0.027	U-0.027	U-0.027	U-0.027	U-0.027	U-0.027
							Walls, Aboy	ve Grade								
Mass	U-0.58	U-0.151	U-0.151	U-0.123	U-0.123	U-0.104	U-0.104	U-0.090	U-0.090	U-0.080	U-0.080	U-0.071	U-0.071	U-0.071	U-0.071	U-0.052
Metal building	U-0.093	U-0.093	U-0.093	U-0.093	U-0.084	U-0.084	U-0.084	U-0.084	U-0.069	U-0.069	U-0.069	U-0.069	U-0.057	U-0.057	U-0.057	U-0.057
Metal framed	U-0.124	U-0.124	U-0.124	U-0.064	U-0.084	U-0.064	U-0.064	U-0.064	U-0.064	U-0.064	U-0.064	U-0.057	U-0.064	U-0.052	U-0.064	U-0.037
Wood framed and other	U-0.089	U-0.089	U-0.089	U-0.089	U-0.089	U-0.089	U-0.089	U-0.064	U-0.064	U-0.051	U-0.051	U-0.051	U-0.051	U-0.051	U-0.036	U-0.036
		51 S					Walls, Belo	w Grade								20
Below-grade wall ^a	C-1.140	C-1.140	C-1.140	C-1.140	C-1.140	C-1.140	C-1.140	C-0.119	C-0.119	C-0.119	C-0.119	C-0.119	C-0.119	C-0.092	C-0.119	C-0.075
							Floo	rs								
Mass	U-0.322	U-0.322	U-0.107	U-0.087	U-0.107	U-0.087	U-0.087	U-0.074	U-0.074	U-0.064	U-0.064	U-0.057	U-0.064	U-0.051	U-0.057	U-0.05
Joist/Framing	U-0.282	U-0.282	U-0.052	U-0.052	_	U-0.033	U-0.033	U-0.033	U-0.033	U-0.033	U-0.033	U-0.033	U-0.033	U-0.033	U-0.033	U-0.03
						S	lab-on-Gra	de Floors								
Unheated slabs	F-0.730	F-0.730	F-0.730	F-0.730	F-0.730	F-0.730	F-0.730	F-0.540	F-0.730	F-0.540	F-0.540	F-0.520	F-0.520	F-0.520	F-0.520	F-0.51
Heated slabs	F-1.020	F-1.020	F-1.020	F-1.020	F-0.900	F-0.900	F-0.860	F-0.860	F-0.860	F-0.860	F-0.860	F-0.688	F-0.830	F-0.688	F-0.688	F-0.68

- WALL SECTIONS A4.

IS A DESIGN BUILD PROJECT FOR PLUMBIN THE PLUMBING CONTRACTOR SHALL PROVIDE PLANS TO THE GENERAL CONTRACTOR & THE SUPERVISING PROFESSIONAL FOR REVIEW PRIOR TO THE PLANS BEING SUBMITTED TO WISCONSIN PEPARTMENT OF COMMERCE. THE PLUMBING CONTRACTOR SHAL MAINTAIN A SET OF DRAWINGS AT THE SITE TO RECORD ANY CHANGES TO THE DESIGN. THIS DRAWING OF RECORD & STATE APPROVED PLANS THE DETURNED OVER TO THE PROJECT MANAGER AT THE END OF THE PROJECT.

HVAC

THIS IS A DESIGN BUILD PROJECT FOR HVAC. THE HVAC CONTRACTOR SHALL PROVIDE PLANS TO THE GENERAL CONTRACTOR & THE SUPERVISING PROFESSIONA FOR REVIEW PRIOR TO THE PLANS BEING SUBMITTED TO WISCONSIN DEPARTMENT OF COMMERCE. THE HVAC CONTRACTOR SHALL MAINTAIN A SET OF DRAWINGS AT THE SITE TO RECORD ANY CHANGES TO THE DESIGN. THIS DRAWING OF RECORD & STATE APPROVED PLANS SHALL BE TURNED OVER TO THE PROJECT MANAGER AT THE END OF THE PROJECT.

ELECTRICAL

THIS IS A DESIGN BUILD PROJECT FOR ELECTRICAL. THE ELECTRICAL CONTRACTOR SHALL PROVIDE PLANS TO THE GENERAL CONTRACTOR & THE SUPERVISING PROFESSIONAL FOR REVIEW PRIOR TO THE PLANS BEING SUBMITTED TO WISCONSIN DEPARTMENT OF COMMERCE. THE ELECTRICAL CONTRACTOR SHALL MAINTAIN A SET OF DRAWINGS AT THE SITE TO RECORD ANY CHANGES TO THE DESIGN. THIS DRAWING OF RECORD & STATE APPROVED PLANS SHALL BE TURNED OVER TO THE PROJECT MANAGER AT THE END OF THE PROJECT.

BUILDING ENVELOPE SYSTEMS

ENERGY CONSERVATION CODE REQUIREMENTS THIS PROJECT WAS DESIGNED UNDER THE WISCONSIN BUILDING CODE BASED ON IBC 2015. THE BUILDING ENVELOPE IS BASE ON THE PRESCRIPTIVE REQUIREMENTS OF TABLE 502.2.2 IECC 2009 AS ALLOWED BY THE STATE OF WISCONSIN

ROOF METAL BUILDING ROOF PURLIN DRAPED ROOF INSULATION 2 LAYER SYSTEM RII WITH THERMAL BLOCKS AND R 19 BELOW U=.043 REQUIRED .049 OR LESS

WALLS METAL BUILDING WALL SYSTEM GIRT FILL SYSTEM SINGLE LAYER INSUL R25 W/ THERMAL TAPE ON FACE OF GIRTS U=.059 REQUIRED .069 OR LESS

FLOOR UNHEATED 6" CONCRETE RIO 24" DEEP MIN PERIMETER F=.54 REQUIRED .54 OR LESS DOORS U=.70 FOR SWING DOORS AND U=.70 FOR SECTIONAL OVERHEAD DOORS WINDOWS SEE TABLE C402.4 BELOW

CLIMATE ZONE	1		2		3		4 EXCEPT MARINE		5 AND MARINE 4		6		7		8	
					Č - 1	Vertical	fenestra	tion			(i)	_			-	
U-factor	ik	ala Si	2		1	and a		in 1	4		S	han di	-		S	
Fixed fenestration	0.	50	0.50		0.	0.46		0.38 0.1		38	0.36		0_	29	0.29	
Operable fenestration	0.	65	0.	0.65		0.60		45	0.45		0.43		0_	37	0.37	
Entrance doors	1.	10	0.83		0.77		0.	0.77		0.77		77	0.1	77	0.1	77
SHGC		1														
Orientation ^a	SEW	N	SEW	N	SEW	N	SEW	N	SEW	N	SEW	N	SEW	N	SEW	N
PF < 0.2	0.25	0.33	0.25	0.33	0.25	0.33	0.40	0.53	0.40	0.53	0.40	0.53	0.45	NR	0.45	N
$0.2 \le PF < 0.5$	0.30	0.37	0.30	0.37	0.30	0.37	0.48	0.58	0.48	0.58	0.48	0.58	NR	NR	NR	NR
PF ≥ 0.5	0.40	0.40	0.40	0.40	0.40	0.40	0.64	0.64	0.64	0.64	0.64	0.64	NR	NR	NR	NR
						Sk	ylights									
U-factor	0.	75	0.	65	0.	55	0.	50	0.	50	0.	50	0.:	50	0.:	50
SHGC	0.	35	0.	35	0.35		0.40		0.40		0.40		NR		NR	

... inducates vertical ienestration onemed, within 4.) degrees of true north. "SEW" indicates orientations other than "N." For buildings in the southern iemisphere, reverse south and north. Buildings located at less than 23.5 degrees latitude shall use SEW for all orientations.

PROJECT INFORMATION

<u>ADDRESS:</u> 2770 TOWNE COURT CLAYTON, WI 54956 STORAGE S-<u>USE:</u> NON-SEPARATED OWNER MARTY NIKODEM 250 ALDER AVE. OMRO, WI 54963 CONTACT: MARTY NIKODEM

DESIGNERS OF RECORD: RCHITECT, FIGHER & ASSOCIATES ARCHITECTS / PLANNERS WI3654 BALSAM LAKE RD. CRIVITZ, WI 54114 P.(920) 376-0007

CONTACT: RICHARD J. FISHER AIA

OCCUPANCY: STORAGE S-I F-I & B NON-SEPARATED TYPE OF CONSTRUCTION: IIB (METAL FRAMED UNPROTECTED) NON-SPRINKLERED OCCUPANT LOAD 20

EGESS WIDTH REQUIRED .2"/ OCCUPANT NON-SPRINKLED 138 X .2 = 28" REQUIRED

EXIT ACCESS TRAVEL DISTANCE 250' PER TABLE 1004.2.4

TOILET FACILITIES PER TABLE 2902. UNISEX TOILET ROOM I W.C., I LAV. PROVIDED EACH SPACE

GRADE PLAN DETERMINATION

NUMBER OF STORIES (2) THIS BUILDING HAS TWO FLOOR LEVELS

BUILDING CODE ANALYSIS

LESS THAN 15 OCC. ECAH SPACE ALLOWS SINGLE UP TO 50% OF TOILETS MAY BE URINALS I SERVICE SINK PROVIDED EACH SPACE

THE GREATEST HGT. FROM GRADE TO TOP OF WALL IS 22'-O" ALLOWABLE HEGHT PER TABLE 503 IS 55'

AREA PER FLOOR

EXISTING BUILDING AREA PROPOSD BUILDING ADDITION 4,247 S.F.

6,050 S.F 10,297 S.F.

ALLOWABLE AREA PER FLOOR STORAGE S-I 17,500 PER TABLE 503 ONE STORY AND 40' IN HEIGHT

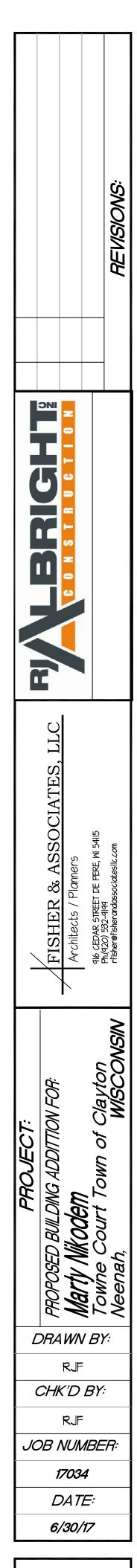
FIRE FIGHTING APPARATUS THE BUILDING IS LIMITED IN AREA THE FIRELANE IS UNOBSTRUCTED THE FIRELANE IS WITHIN 150 ' OF ALL PARTS OF THE EXTERIOR WALL WITH A MIN. UNOBSTRUCTED HEIGHT OF 13'-6" THE BUILDING IS 20'-0" TALL CONTROL AREAS

NO HAZARDOUS MATERIALS WILL BE STORED WITHIN THIS BUILDING PER TABLES 307.7(1) AND 307.7 (2)

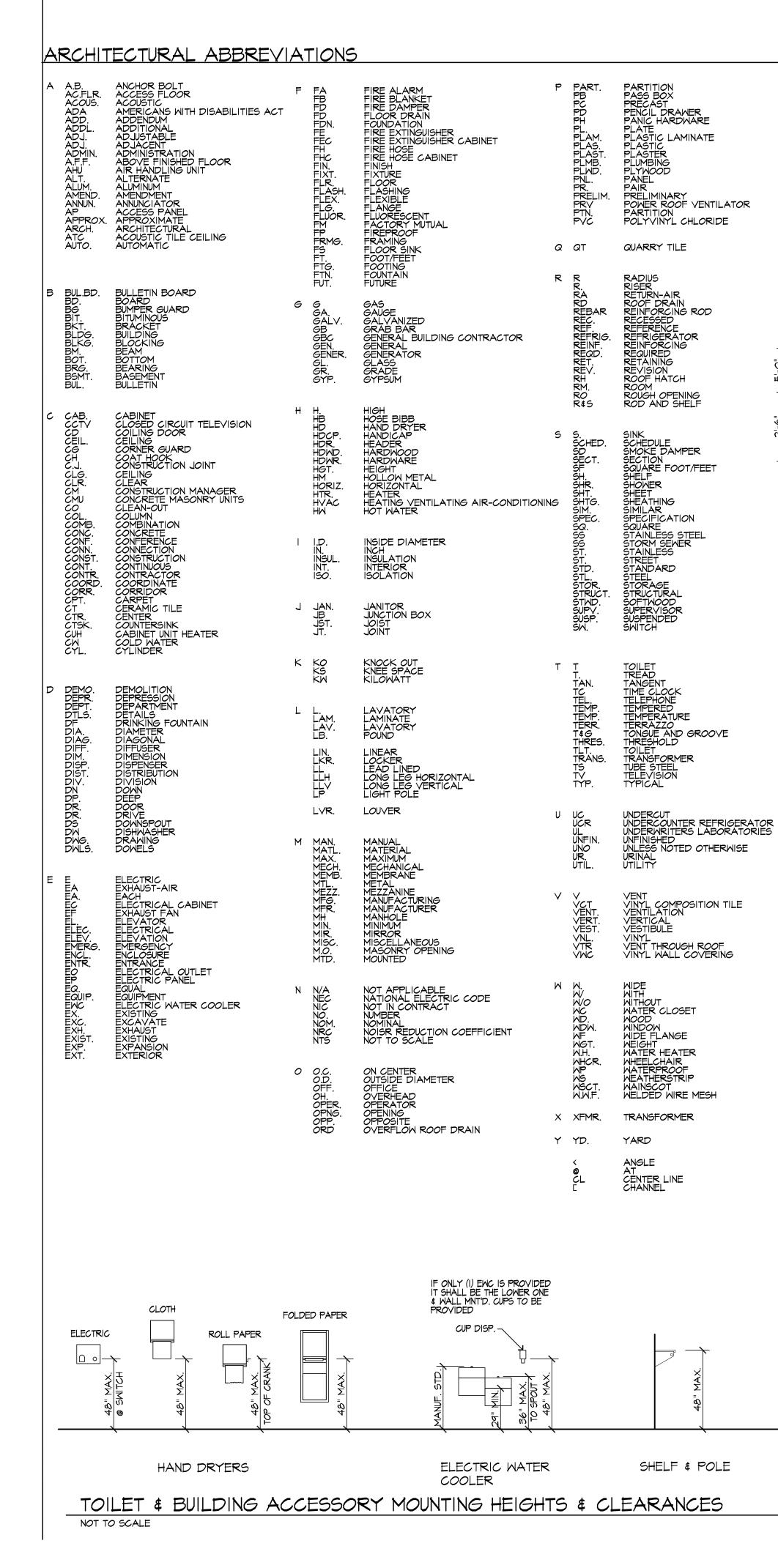


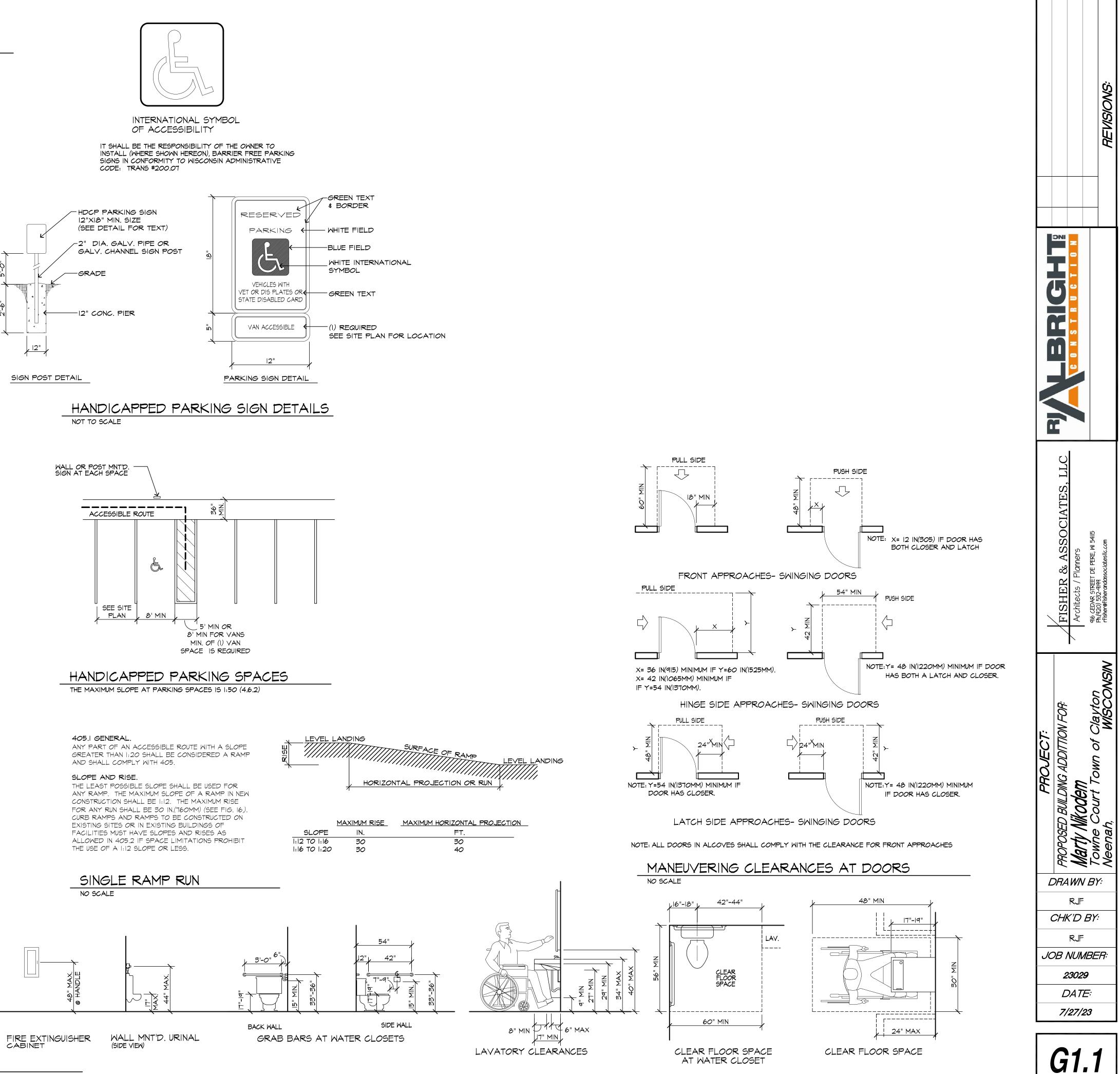
RELEASED FOR CONSTRUCTION RELEASED FOR SITE PLAN APPROVAL

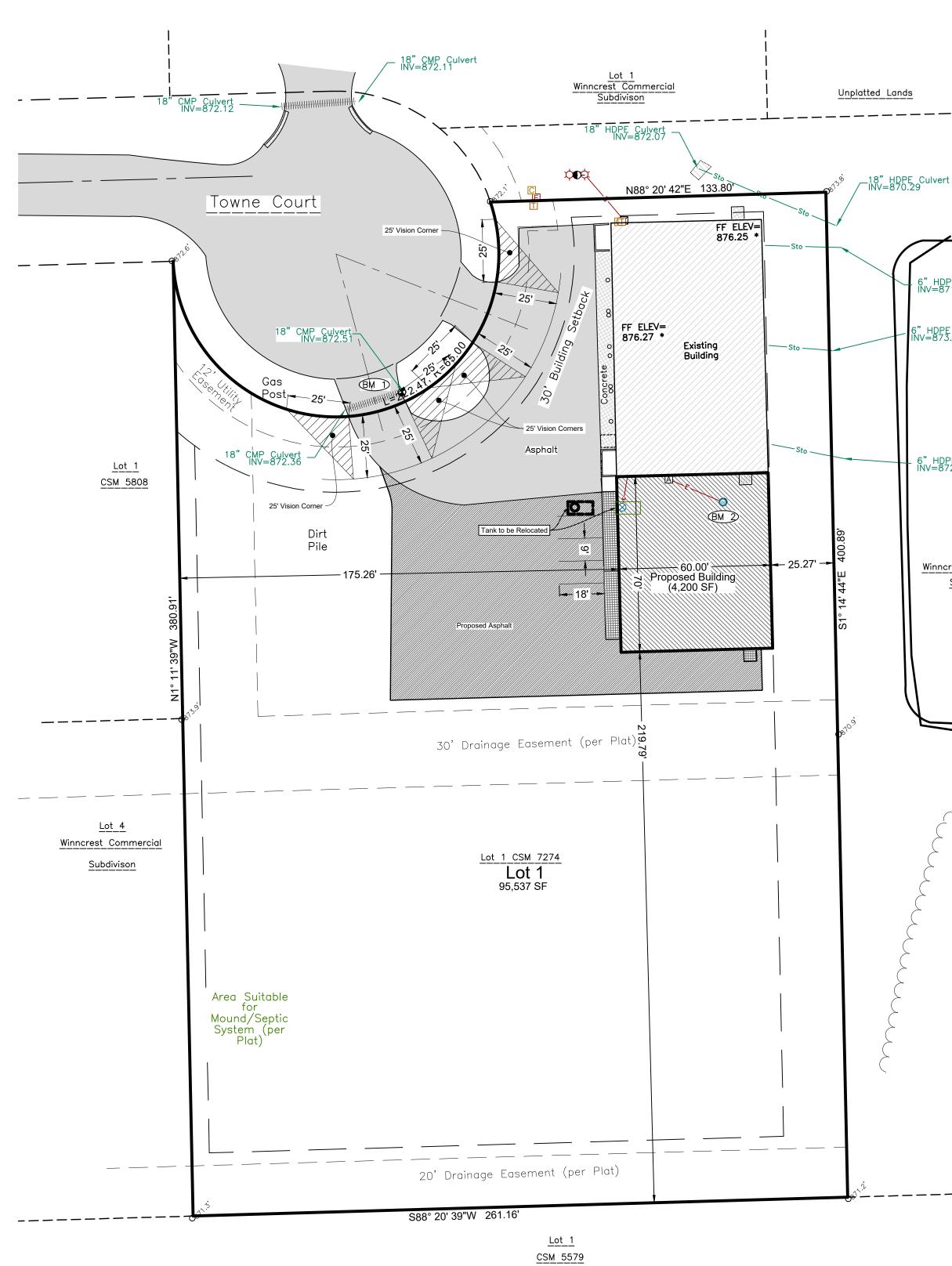
8/21/23 6/28/23











LEGEND

----Sto ------ Storm Sewer E E Underground Electric ------Culvert

Clean Out / Curb Stop / Pull Box 🛛 🖸 \odot Water MH / Well Utility Meter \boxtimes 🗘 Light Pole / Signal E Electric Pedestal Air Conditioner

 \otimes

Telephone Pedestal +799.9 Ex Spot Elevation

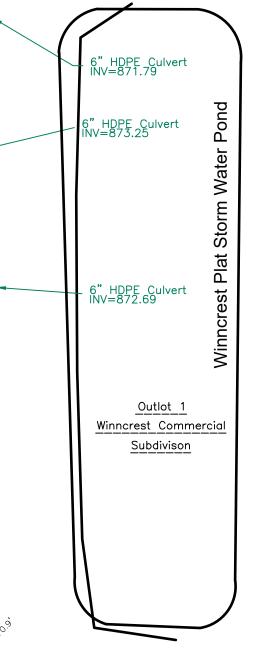
CATV Pedestal Gas Regulator Post / Guard Post Flag Pole Benchmark Asphalt Pavement Concrete Pavement Gravel

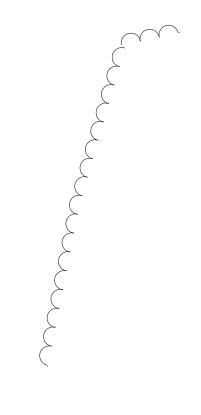
G

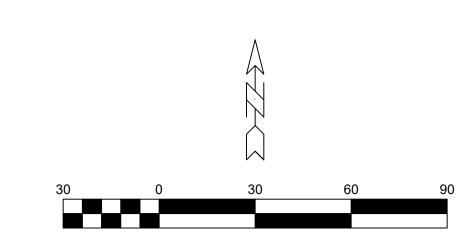
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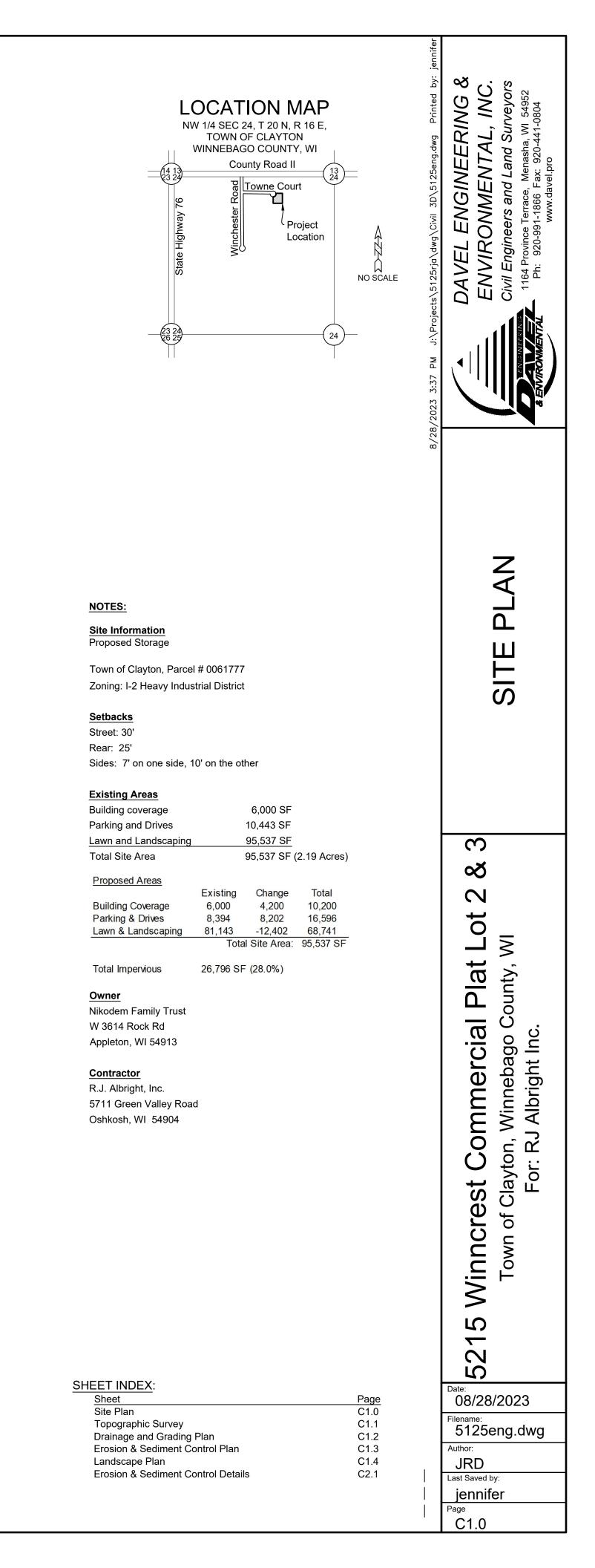
Unplatted Lands

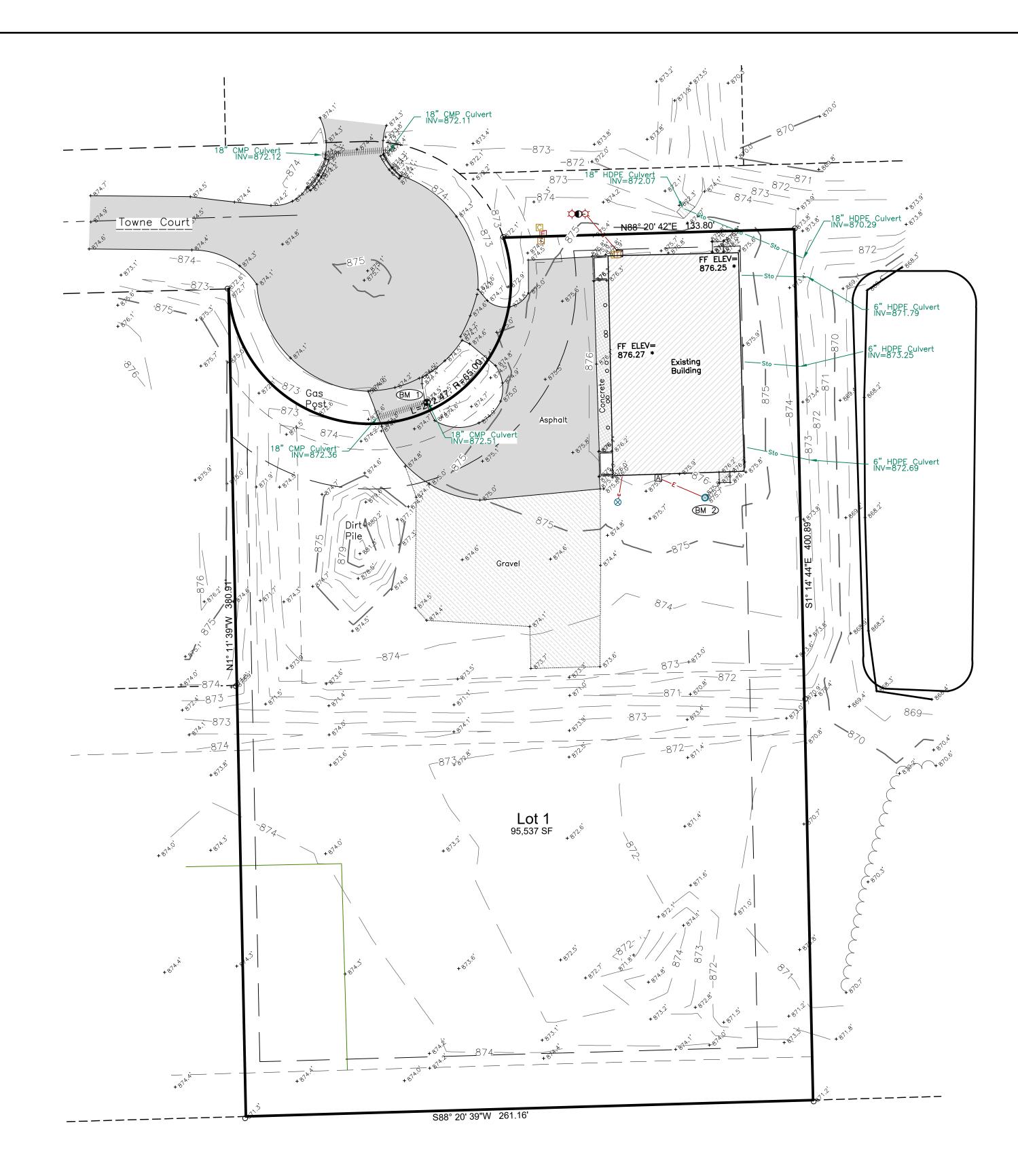
_18" HDPE_Culvert INV=870.29

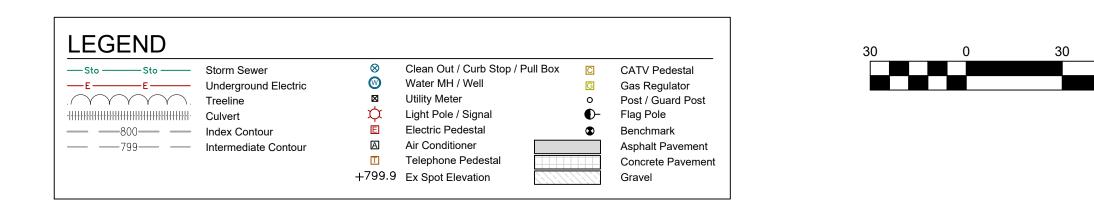












General Notes:

1. Zoning Information Town of Clayton: I-2 Heavy Industrial District Setbacks: Front Yard: 30 Feet Side Yard: 7 Feet & 10 Feet Rear Yard: 25 Feet Height: No Limitation

> Caveat: Building zones depicted are based on building setbacks in effect at the time of the survey and should not be relied upon without first obtaining written verification thereof from the Town of Clayton and any other local agencies.

2. Floodplain Information

- (Subject Site per FIRM Map No. 55139C0100E with and effective date of March 3, 2003) Mapped as "Zone X": Area determined to be outside the 0.2% annual chance floodplain.
- 3. Existing utilities shown are indicated in accordance with available records and field measurements. However, lacking excavation, the exact location of underground features cannot be accurately, completely, and reliably depicted. In addition, in some jurisdictions, 811 or other similar utility locate requests from surveyors may be ignored or result in an incomplete response. The contractor shall be responsible for obtaining exact locations & elevations of all utilities, including sewer & water from the the property owners of the respective utilities. All utility the property owners shall be notified by the contractor 72 hours prior to excavation. Contact Digger's Hotline (1-800-242-8511) for exact utility locations.

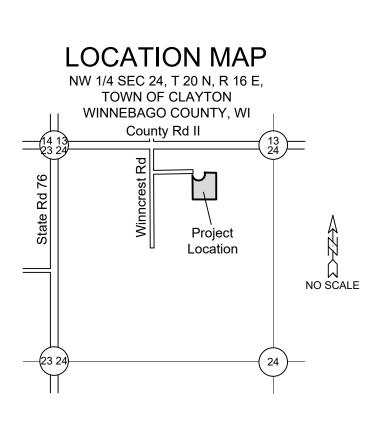
4. This is not a boundary survey.

SURVEYOR'S CERTIFICATE

I, Scott R. Andersen, hereby certify that I have surveyed this property and this topographical map is a true representation thereof and shows the size and location of the property and the location of all apparent roadways. I hereby certify that said topographical survey and map were made in accordance with acceptable professional standards and that the information contained thereon is, to the best of my knowledge, information and belief, a true and accurate representation thereof.

BENCHMARKS (NAVD88 WI-Geoid 12A)

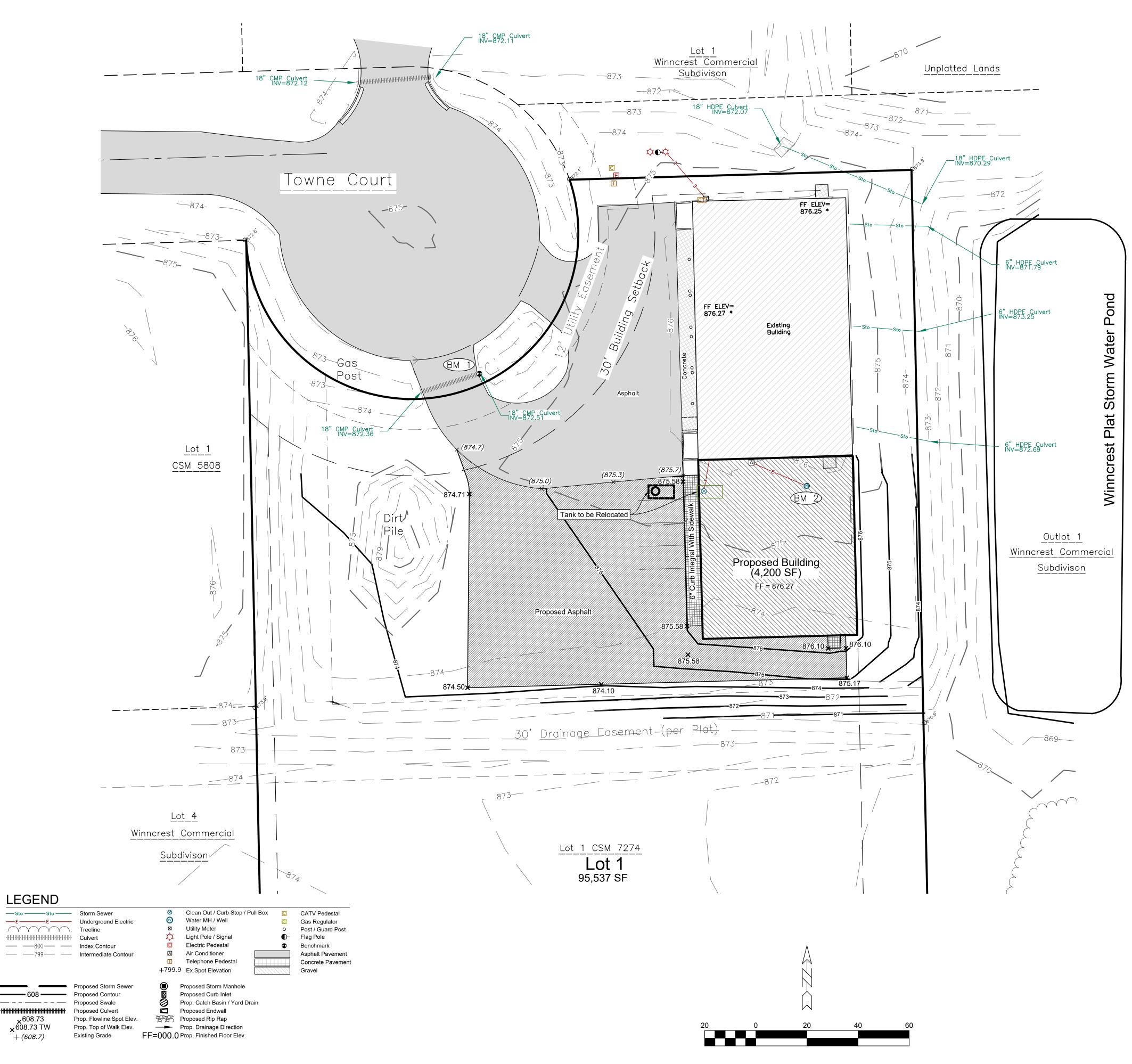
- BM 1 Top Center of Well Adjacent to southeast corner of main building Elev 877.54
- BM 2 Top of Culvert on east end of culvert at entrance to parking lot ±580' E of BM 1, N R/W Deerwood Ave Elev 874.15





	Civil Engineers and Land Surveyors A ENVROMMENTAL Ph: 920-991-1866 Fax: 920-441-0804 www.davel.pro
TOPOGRAPHIC	SURVEY
5215 Winncrest Commerical Plat Lot 2 & 3	Town of Clayton, Winnebago County, WI For: RJ Albright Inc.
Date: 08/1/2 Filename: 5125T Author: SRA Last Saved by jennife Page	OPO.dwg

C1.1



608.73 ×608.73 TW + (608.7)

NOTES:

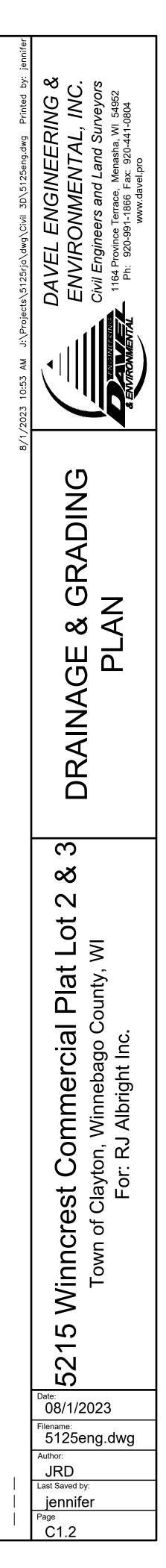
- 1. Existing utilities shown are indicated in accordance with available records and field measurements. The contractor shall be responsible for obtaining exact locations & elevations of all utilities, including sewer and water from the owners of the respective utilities. All utility owners shall be notified by the contractor 72 hours prior to excavation. Contact Digger's Hotline (1-800-242-8511) for exact utility locations.
- 2. The Contractor shall verify all staking and field layout against the plan and field conditions prior to constructing the work and immediately notify the Engineer of any discrepancies. 3. Vegetation beyond slopes shall remain.
- 4. The contractor shall minimize the area disturbed by construction as the project is constructed. Disturbed areas shall be seeded as soon as final grade is established. Contractor shall replace topsoil and then seed, fertilize and mulch all lawn areas within 1 week of topsoil placement.
- 5. Contractor shall remove all excess materials from the site. Earthwork contractors shall verify topsoil depth. 6. All sediment and erosion control devices and methods shall be in accordance with
- the Wisconsin DNR Technical Standards. 7. The contractor shall make weekly inspections and inspections within 1 day of any
- rainfall exceeding 0.5 inches of the sediment and erosion control devices throughout construction. The contractor shall repair or maintain erosion control devices as necessary. The inspection reports shall be made available to the owner at the end of the construction or upon demand during construction.
- 8. The outside services are shown to stop at a point 5 feet outside the foundation wall. The Contractor shall be responsible for coordination of continuation of the services into the building to properly coincide with the interior plumbing plans, and compliance with all plumbing permits.
- 9. Contractor is responsible for compliance with Department of Safety & Professional Services, Chapter SPS 382, for lateral construction and cleanout locations.
- 10. Updated survey and title search have not been authorized and the boundary and easements shown may be inaccurate or incomplete.

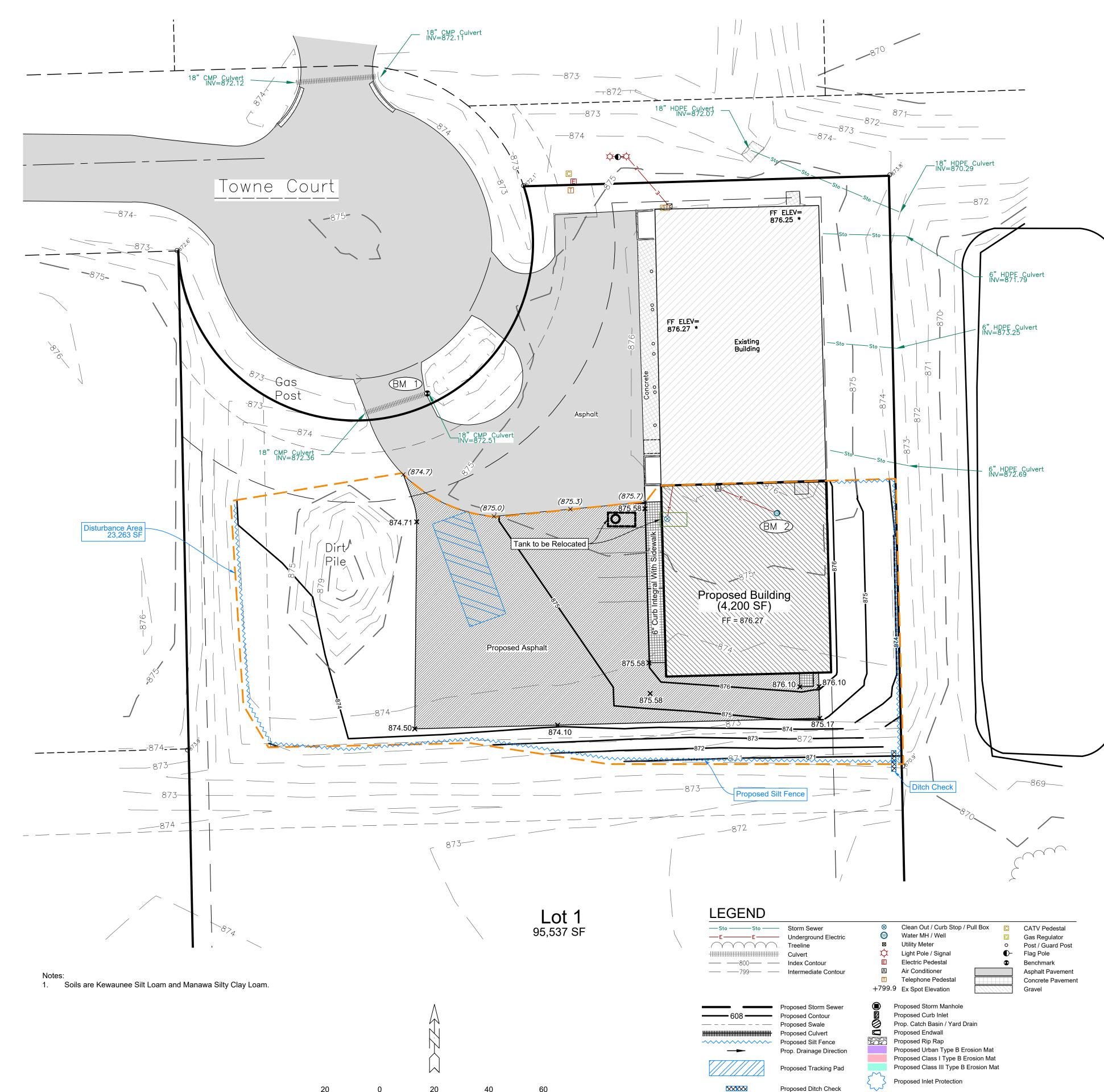
STORM WATER PLAN:

1. This property is tributary to the Winncrest Plat Storm Water Pond located directly east. This pond provides both peak flow control and water quality treatment. The pond was designed for properties having a proposed runoff curve number (RCN) of 94. This project has an RCN of 80.7.

BENCHMARKS:

- 1. Top of iron rod on north property corner on east end of cul de sac, Elev. = 872.10
- 2. Top of iron rod on northwest property corner on cul de sac, Elev. = 872.66







Total site disturbance: 0.79 acres

Planned Sediment and Erosion Control Practices cleaned daily or as necessary as no sediment flushing is allowed.

Onsite soils are generally silty clay loam being Manawa silty clay loam, Navan silt loam, and Kewaunee silt loam. The site discharges to the adjacent southwest property and is within the Lake Butte des Morts watershed.

- 1) Diverting Flow
- water pond.

2) Overland Flow

- following locations:
- i) along the site boundary where runoff will leave the site;
- In addition to mulching, Erosion Mat will be used in the following areas: i) on all permanent and temporary diversions;
- 3) Trapping Sediment in Channelized Flow Additional ditch checks may be required in areas where erosion is occurring.
- 4) Permanent Channel Stabilization will require riprap to prevent erosion of the slope.
- i) drainage swales as indicated on the plans;
- the plan.
- requirements of Technical Standard 1061. Dewatering is not anticipated for this project.
- Sequence of Construction Obtain plan approval and other applicable permits.
- 1) Install all erosion control measures. August 2023.
- Temporary seeding is required on all disturbed soils if conditions allow. August 2023. 3) Site grading. August 2023.
- necessary. August 2023
- stabilization has occurred.

Note: The dates provided are approximate for construction and subject to weather conditions and overall project schedule. Several work items as listed above may occur simultaneously with others.

Maintenance Plan

Type of Inlet Protection

- height of the structure. In addition, the following measures shall be taken:
- dense vegetated cover.
- 3) Avoid the use of fertilizers and pesticides in or adjacent to channels or ditches. 4) Construction and waste materials shall be properly disposed.

Weekly inspection reports shall be maintained by the contractor. These reports shall document inspections and maintenance performed. The date and time of the inspections, the inspector's name, and the status of construction and any maintenance performed. Refer to Appendix C of the Erosion and Sediment Control Plan (report) or visit <u>http://dnr.wi.gov/runoff/stormwater/constrforms.htm</u> for a template. Upon request, the inspection reports shall be made available to the owner, the engineer, Winnebago County, or the Wisconsin Department of Natural Resources.

All erosion control practices shall be in place prior to disturbing the site. All sediment and erosion control devices and methods shall be in accordance with DNR Technical Standards and the WisDOT Erosion Control product acceptability lists (PAL). It is the responsibility of the Contractor to minimize the area disturbed and the duration of the disturbance. Erosion control measures shall be maintained on a continuing basis until the site is permanently stabilized. Erosion controls must be in place at the end of each work day with all off-site sediments being

L ENGINEERING & RONMENTAL, INC. incers and Land Surveyors

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a) Permanent Diversion - Intended to divert runoff around disturbed areas to a location where the water can be discharged without adversely impacting the receiving area or channel. Permanent diversions will be used to route runoff to the drainage swales and storm

b) Temporary Diversion - Intended to divert runoff around disturbed areas to a location where the water can be discharged with out adversely impacting the receiving area or channel. Unlike a permanent diversion, the temporary diversion will be removed upon the completion of the project. Temporary diversions will be used upslope of any soil piles to reduce the amount of sediment transported. There is a temporary diversion along the east property line to divert potential turbid construction site runoff to the proposed onsite storm water pond. All diversions shall be installed and maintained in accordance with DNR Technical Standard 1066.

a) Silt Fence - Intended to provide a temporary barrier to the transportation of sediment offsite. Silt fence also reduces the velocity of sheet flow; thereby reducing the erosion potential of flowing water. Silt fencing is not to be used in areas of channelized flow and sediment deposits shall be removed when a 6 inch depth is reached. The silt fence shall be repaired or replaced as necessary to maintain a barrier. All Silt Fence shall be installed and maintained in accordance with DNR Technical Standard 1056. It will be placed at the

b) Mulching and Erosion Mat - Intended to reduce the amount of erosion caused by raindrop impact, high overland and concentrated flow velocities and assist the establishment of both temporary and permanent vegetation. All Erosion Mat shall be installed and maintained in accordance with DNR Technical Standards 1052 and 1053 and all Mulching with DNR Technical Standard 1058.

ii) and on any areas with slopes greater than 4:1 or as specified on the plan.

c) Seeding - Intended to provide a reduction of overland flow velocities and stabilize disturbed areas. Seeding will be used on all disturbed areas within seven days of the completion of the activity that will disturb the area. All seeding shall be in accordance with DNR Technical Standard 1059. Seed mixture 40 (per WisDOT Specifications, Section 630) shall be applied at 5 pounds per 1000 square feet for permanent seeding prior to September 15th. If required, temporary seeding shall consist of Oats, Rye, Winter Wheat, and/or Annual Ryegrass applied at rates and during the season specified by the Technical Standard but no later than November 1st. Sod placement may occur at anytime sod is available and the sod and soil are not frozen.

a) Ditch Checks - Intended to settle suspended sediment in channelized flow by reducing the flow velocity. All Ditch Checks shall be installed and maintained in accordance with DNR Technical Standard 1062. Ditch Checks will be used where indicated on the plan.

a) Armored Waterway - Intended to establish a non-erosive lining in the channel to prevent erosion. This can be accomplished using riprap. All areas immediately downstream of curb cuts will be stabilized using riprap. Additionally, the swale along the pavement edge

b) Vegetated Waterway - Intended to establish permanent vegetation to reduce the velocity of concentrated runoff thereby protecting the waterway from erosion. Vegetated waterways will be used in the following areas:

5) Stone Tracking Pad - Intended to reduce the amount of sediment transported onto public roads. The Tracking Pad shall be installed and maintained in accordance with DNR Technical Standard 1057. A tracking pad will be constructed at the site entrances as indicated on

6) Dust Control - Intended to reduce surface to air transport of dust during construction. Dust control shall be implemented with use of methods provided in DNR Technical Standard 1068. These methods include the use of polymers, seeding, and mulch.

7) Dewatering BMP- Intended to reduce the amount of sediment conveyed due to dewatering practices. Dewatering practices require compliance with DNR Technical Standard 1061. The use of geotextile bags is required to prevent sedimentation. The bags shall meet the

8) Sediment Basin - The existing ponds will serve as a sediment basin during construction. The sediment basin is designed in accordance with DNR Technical Standard 1064 utilizing the post construction primary orifice and outlet with a temporary reduced primary orifice. Upon final stabilization of the site, the remaining sediment storage capacity of the pond shall be verified with a 5-foot average depth. If inadequate sediment storage is available the accumulated sediment shall be removed and disposed of according to the Operation and Maintenance Plan.

2) Strip topsoil prior to filling activities, haul away excess. Stabilize topsoil in accordance with the appropriate WDNR Technical Standard.

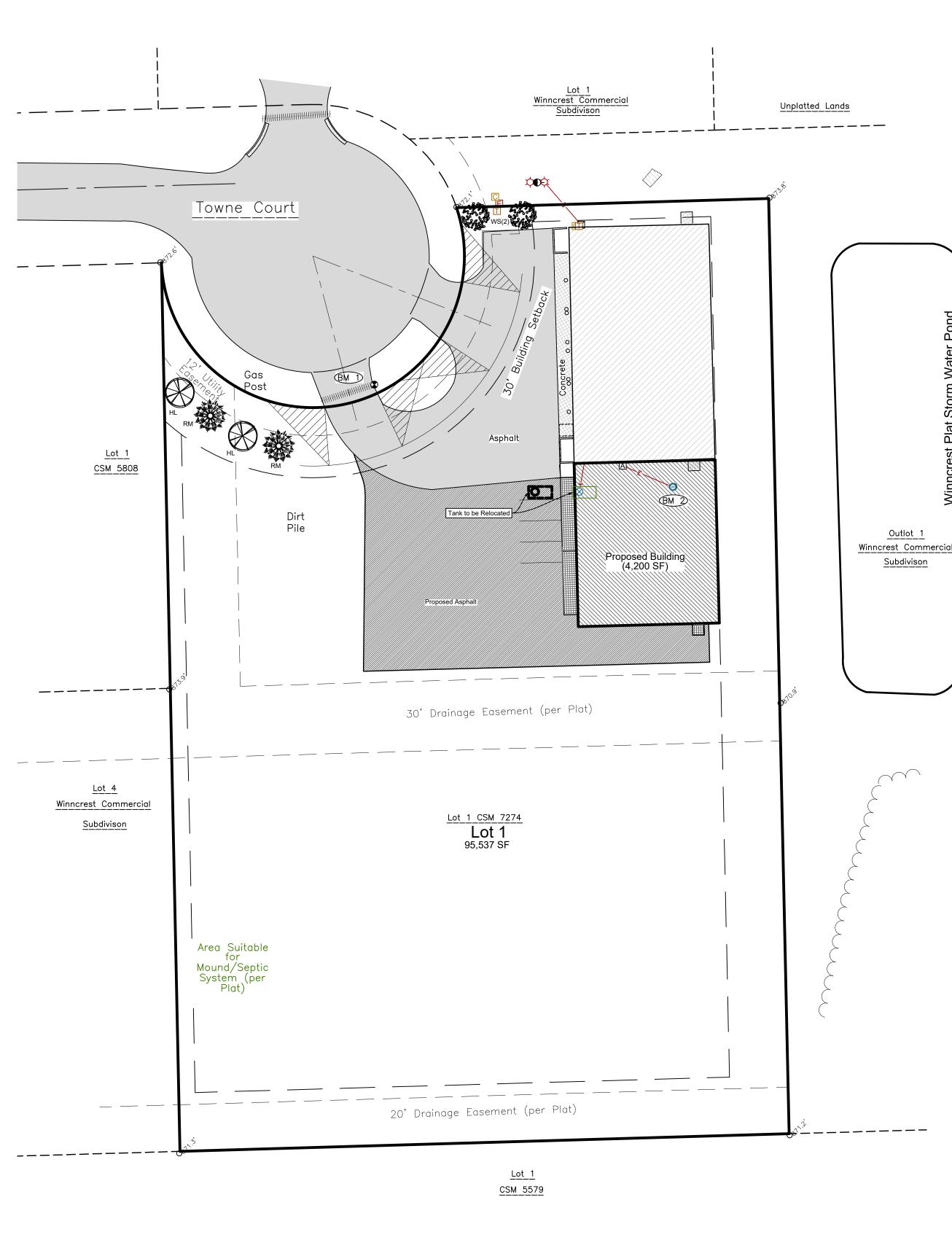
4) Construct buildings, driveways and parking areas upon completion of pond construction. Field inspect and add additional measures if

5) Stabilize lawn and ditch areas no later than one week after final grade is established. November 2023 6) Watering may be necessary to establish healthy and well rooted vegetation. Temporary measures may only be removed once final site

The contractor is responsible for inspection and maintenance of sediment and erosion control measures until the project is completed. The inspections shall be made every seven days or within 24-hours of a rainfall event of 0.50-inch or greater. Any practices that are damaged or not working properly shall be repaired by the end of they day. Accumulated sediment shall be removed when it has reached a height of one-half the

1) All seeded areas will be re-seeded and mulched as necessary according to the specifications in the planned practices to maintain a vigorous,

2) Remove silt fence and temporary structures only after final stabilization and vegetative cover is established.



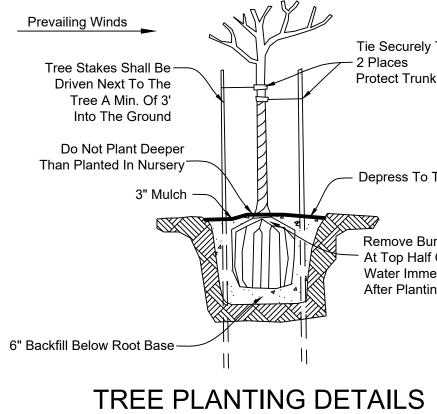
LEGEND

----Sto ------ Storm Sewer E E Underground Electric ------ Culvert

⊗ Clean Out / Curb Stop / Pull Box CATV Pedestal 🮯 🛛 Water MH / Well Utility Meter 🗘 🛛 Light Pole / Signal E Electric Pedestal Air Conditioner Telephone Pedestal +799.9 Ex Spot Elevation

Gas Regulator • Post / Guard Post • Flag Pole Benchmark Asphalt Pavement Concrete Pavement Gravel

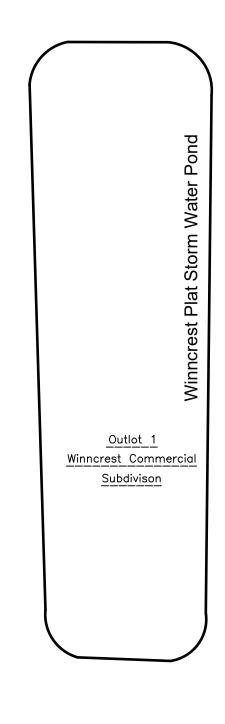
G

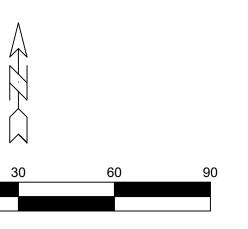


Landscape Requirements Three trees per 100 linear feet of frontage = 6 trees

 <u>Note:</u>
 All trees are to be mulched with hardwood mulch. Trees shall be minimum 4-foot height at time of planting. Species may be substitue based on availability and local nursery stock. Plant Schudu

	Plant Schudule										
I.D.	Common Name	Quantity									
WS	White Swamp Oak	2									
RM	Red Maple	2									
HL	Honeylocust	2									





30 0

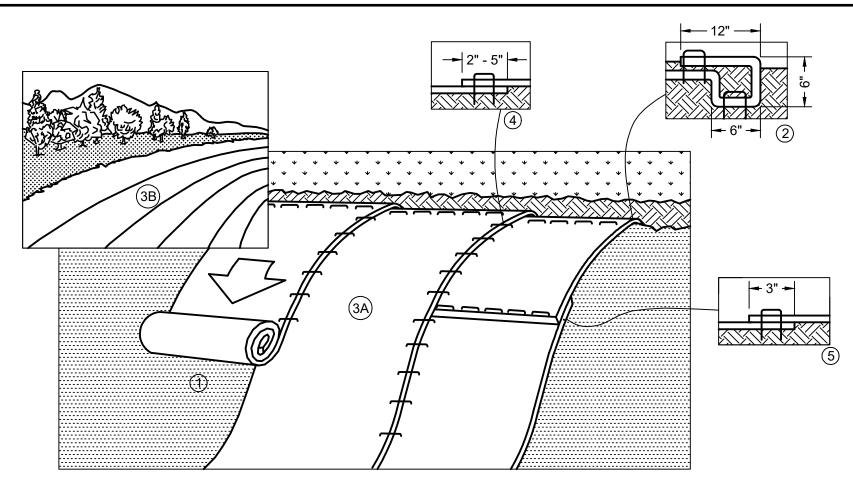
a Iu Lies Apploated by: jantifet by: jantife	DAVEL ENGINEERING & ENVIRONMENTAL, INC. Civil Engineers and Land Surveyors T164 Province Terrace, Menasha, WI 54952 Ph: 920-991-1866 Fax: 920-441-0804 www.davel.pro
d Wire Root. 875/202 8	
	LANDSCAPE PLAN
y	Date: Box Box County, Winnebago County, Winneba

Tie Securely To Tree Ir – 2 Places Protect Trunk From Tie

— Depress To Trap Wate

Remove Burlap And V At Top Half Of Ball Ro Water Immediately

After Planting.



1. Prepare soil before installing Rolled Erosion Control Products (RECP's), including any necessary application of lime, fertilizer, and seed.

Note: When using cell-o-seed do not seed prepared area. Cell-o-seed must be installed with paper side down. 2. Begin at the top of the slope by anchoring the RECP's in a 6" (15 cm) deep x 6" (15 cm) wide trench with approximately 12" (30 cm) of RECP's extended beyond the up-slope portion of the trench. Anchor the RECP's with a row of staples/stakes approximately 12" (30 cm) apart in the bottom of the trench. Backfill and compact the trench after stapling. Apply seed to compacted soil and fold remaining 12" (30 cm) portion of RECP's back over seed and compacted soil. Secure RECP's over compacted soil with a row of staples/stakes spaced approximately 12" (30 cm) apart across the width of the RECP's. 3. Roll the RECP's (A.) down or (B.) horizontally across the slope. RECP's will unroll with appropriate side against the soil surface. All RECP's must be securely fastened to soil surface by placing staples/stakes in appropriate locations as shown in the

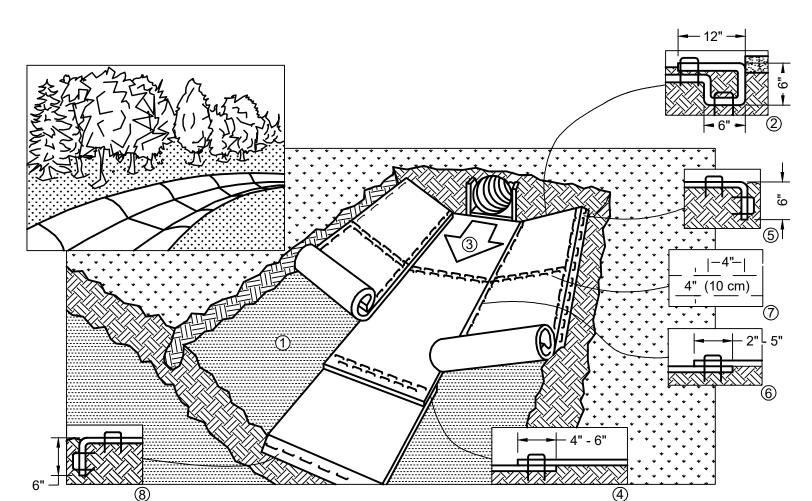
staple pattern guide. When using the Dot system, staples/stakes should be placed through each of the colored Dots corresponding to the appropriate staple pattern. 4. The edges of parallel RECP's must be stapled with approximately 2" - 5" (5 cm - 12.5 cm) overlap depending on RECP's type.

5. Consecutive RECP's spliced down the slope must be placed end over end (shingle style) with an approximate 3" (7.5 cm) overlap. Staple through overlapped area, approximately 12" (30 cm) apart across entire RECP's width. Note: * In loose soil conditions, the use of staple or stake lengths greater than 6" (30 cm) may be necessary to properly secure the RECP's.

6. Detail provided by North American Green (www.nagreen.com)

7. Turf Reinforcement Mats (TRM's) shall be installed in accordance with the above specifications for all RECP's. Anchoring size and pattern is to be installed per manufacturer specifications for clay soils having 4:1 slope. All TRM's shall be topsoil filled, seeded, and covered with a Class 2, Type B erosion mat in accordance with all manufacturer specifications.

EROSION/TURF REINFORCEMENT MAT SLOPE INSTALLATION



1. Prepare soil before installing Rolled Erosion Control Products (RECP's), including any necessary application of lime, fertilizer,

and seed.

Note: When using cell-o-seed do not seed prepared area. Cell-o-seed must be installed with paper side down.

2. Begin at the top of the channel by anchoring the RECP's in a 6" (15 cm) deep x 6" (15 cm) wide trench with approximately 12"

(30 cm) of RECP's extended beyond the up-slope portion of the trench. Anchor the RECP's with a row of staples/stakes approximately 12" (30 cm) apart in the bottom of the trench. Backfill and compact the trench after stapling. Apply seed to

compacted soil and fold remaining 12" (30 cm) portion of RECP's back over seed and compacted soil. Secure RECP's over compacted soil with a row of staples/stakes spaced approximately 12" (30 cm) across the width of the RECP's

3. Roll center RECP's in direction of water flow in bottom of channel. RECP's will unroll with appropriate side against the soil surface. All RECP's must be securely fastened to soil surface by placing staples/stakes in appropriate locations as shown in the

staple pattern guide. When using the DOT system, staples/stakes should be placed through each of the colored dots

corresponding to the appropriate staple pattern. 4. Place consecutive RECP's end over end (shingle style) with a 4" - 6" (10 cm - 15 cm) overlap. Use a double row of staples staggered 4" (10 cm) apart and 4" (10 cm) on center to secure RECP's.

5. Full length edge of RECP's at top of side slopes must be anchored with a row of staples/stakes approximately 12" (30 cm) apart in a 6" (15 cm) deep x 6" (15 cm) wide trench. Backfill and compact the trench after stapling.

6. Adjacent RECP's must be overlapped approximately 2" - 5" (5cm - 12.5 cm) (depending on RECP's type) and stapled. 7. In high flow channel applications a staple check slot is recommended at 30 to 40 foot (9 M - 12 M) intervals. Use a double row

of staples staggered 4" (10 cm) apart and 4" (10 cm) on center over entire width of the channel. 8. The terminal end of the RECP's must be anchored with a row of staples.stakes approximately 12" (30 cm) apart in a 6" (15

cm) deep x 6" (15 cm) wide trench. Backfill and compact the trench after stapling.

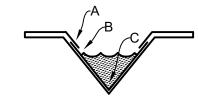
Note:

* In loose soil conditions, the use of staple or stake lengths greater than 6" (15 cm) may be necessary to properly anchor the RECP's.

Note:

9. Detail provided by North American Green (www.nagreen.com)

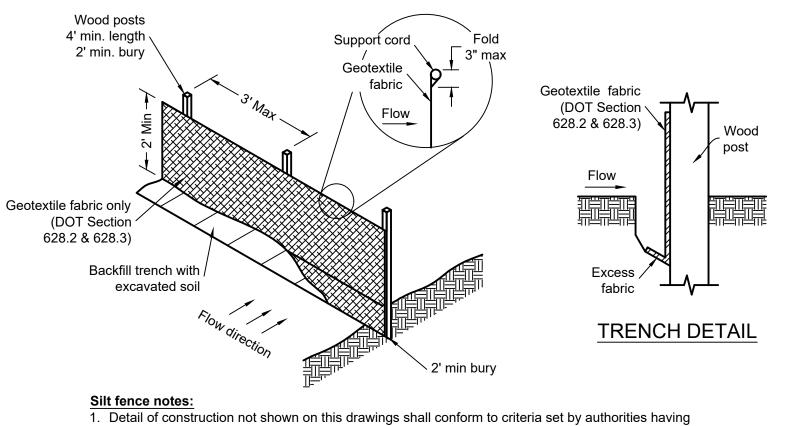
Critical Points



A. Overlaps and seams B. Projected Water line C. Channel Bottom/side slope vertices * Horizontal staple spacing should be altered if necessary to allow staples to secure the critical points along the channel surface.

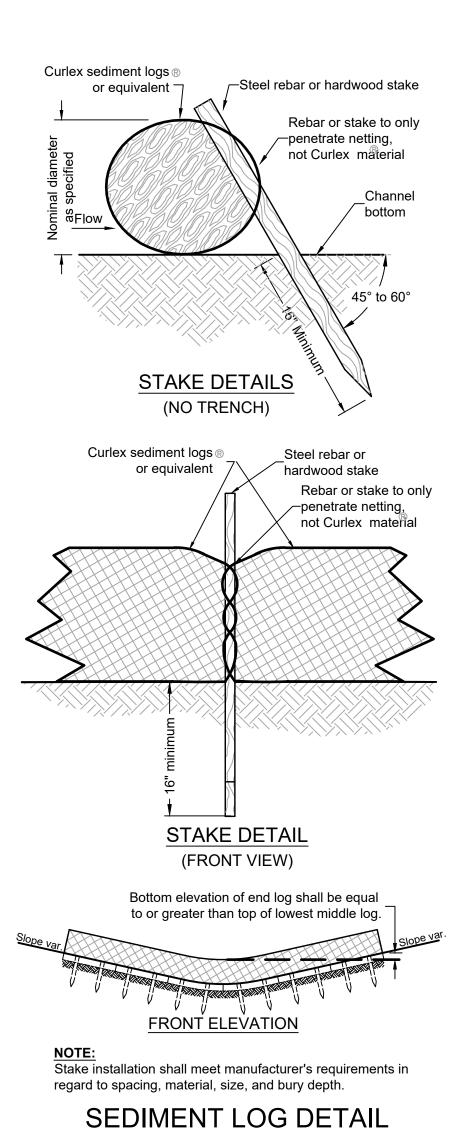
** In loose soil conditions, the use of staple or stake lengths greater than 6" (15 cm) may be necessary to properly anchor the RECP's.

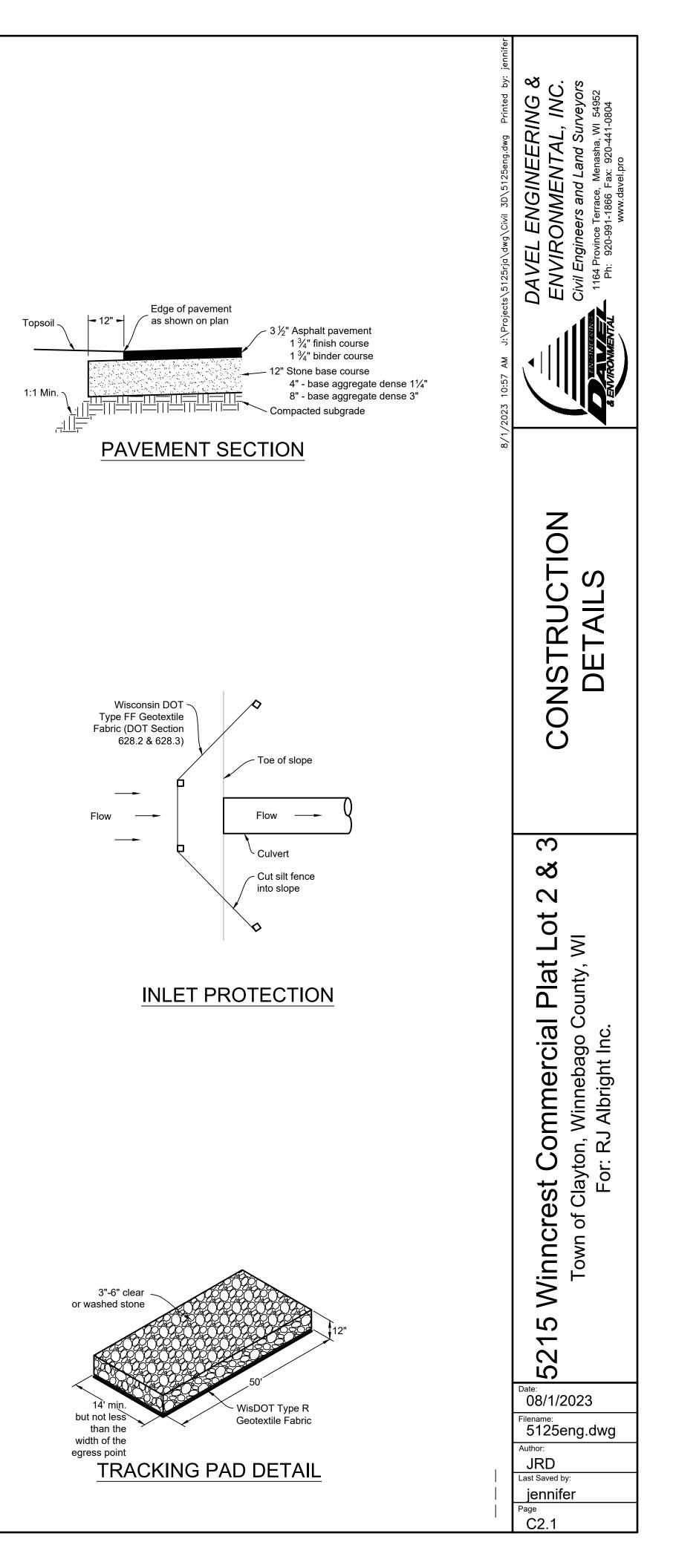
EROSION MAT CHANNEL INSTALLATION

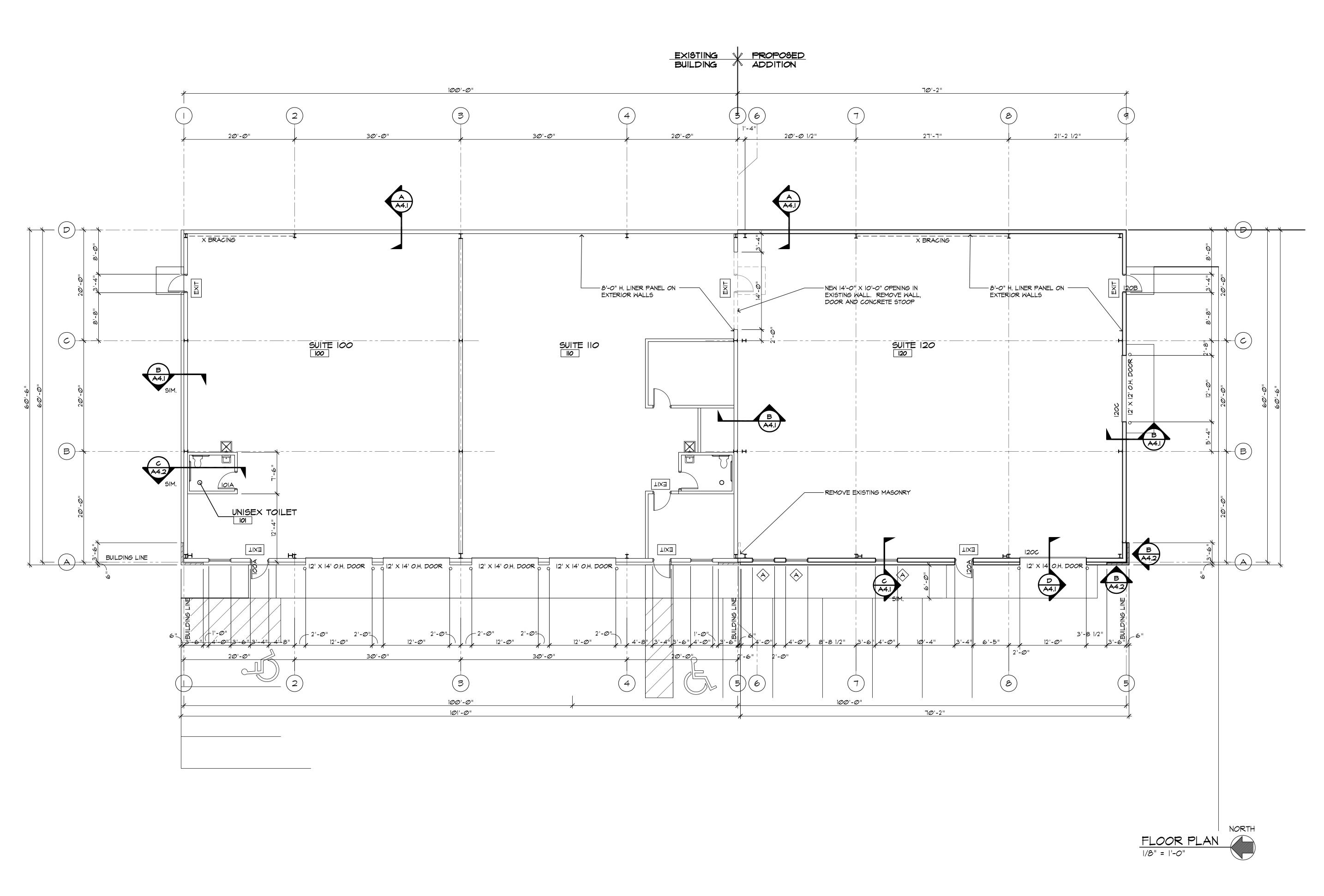


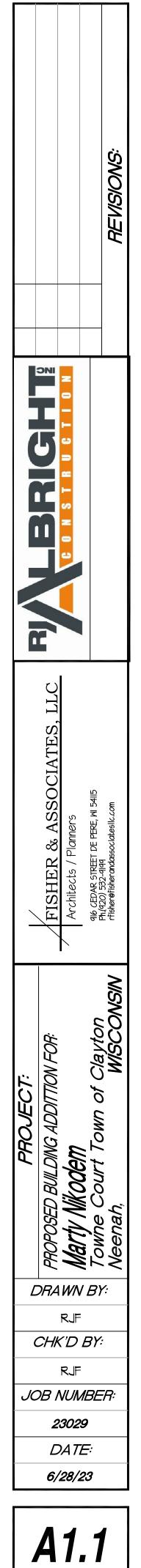
- jurisdiction and by DNR Technical Standard 1056 2. When possible, the silt fence should be constructed in an arc or horseshoe shape with the ends
- pointing upslope to maximize both strength and effectiveness.
- 3. Attach the fabric to the posts with wire staples or wooden lath and nails.
- 4. 8'-0" post spacing allowed if a woven geotextile fabric is used.
- 5. Trench shall be a minimum of 4" wide and 6" deep to bury and anchor the geotextile fabric. Fold material to fit trench and backfill and compact trench with excavated soil.
- 6. Geotextile fabric shall be reinforced with an industrial polypropylene netting with a maximum mesh
- spacing of 3/4" or equal. A heavy-duty nylon top support chord or equivalent is required. 7. Steel posts shall be studded "tee" or "u" type with a minimum weight of 128 lbs/lineal foot (without anchor). Fin anchors shall be a minimum size of 4" diameter or 1 1/2" x 3 1/2", except wood posts for geotextile fabric reinforced with netting shall be a minimum size of 1 1/8" x 1 1/8" oak or hickory.

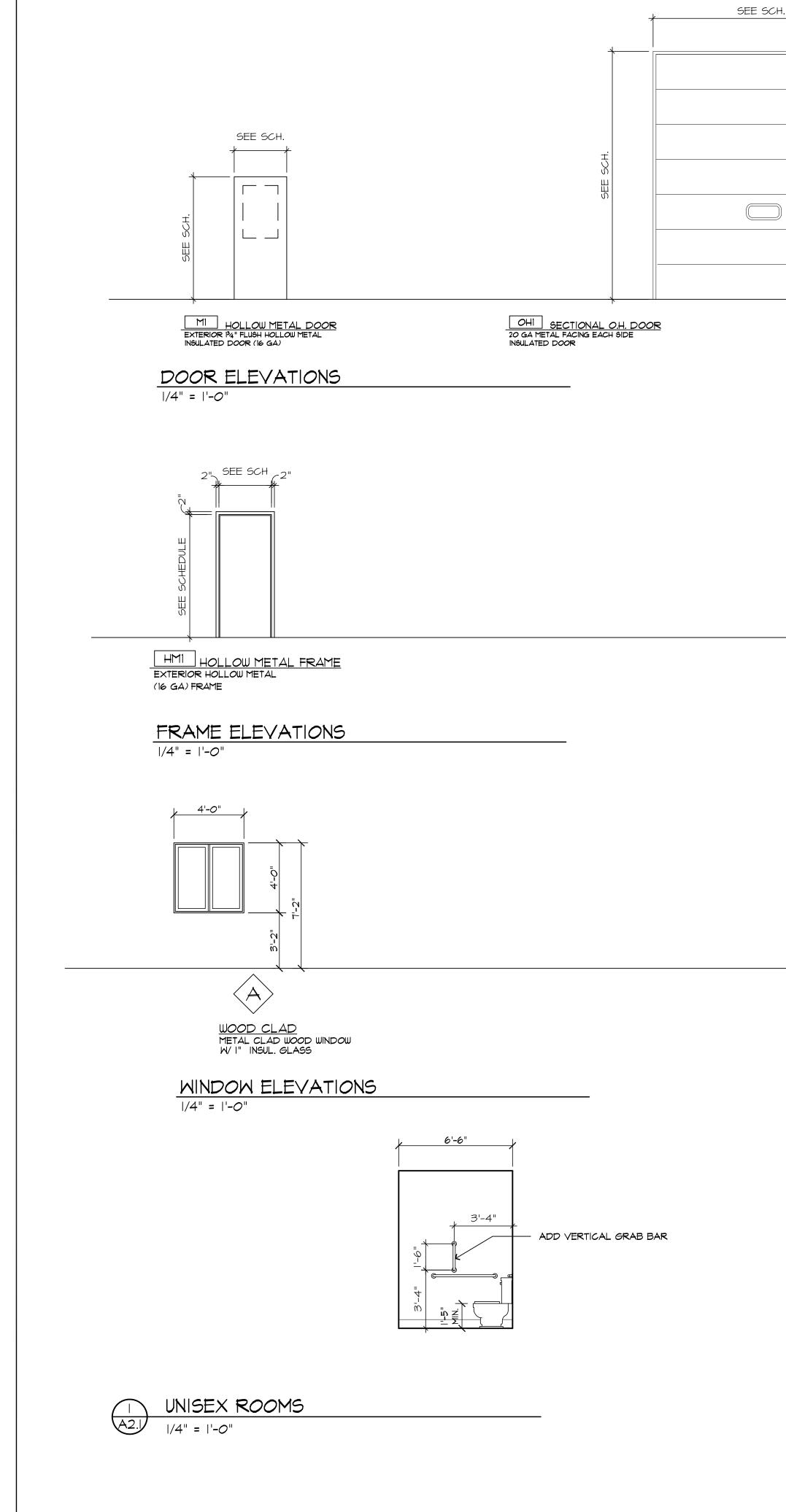
SILT FENCE INSTALLATION











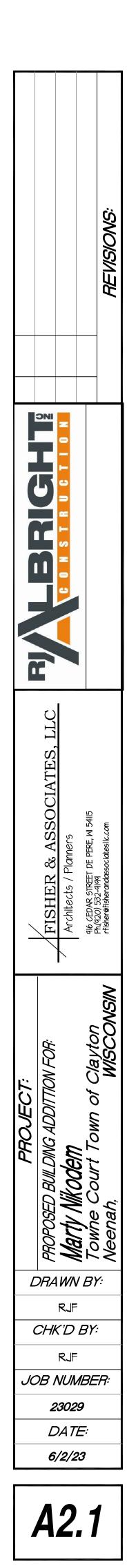
	DOOR AND FRAME SCHEDULE													
MARK	SIZE		ΤY	ΈΕ		FRA	ME		DETAIL					
		DOOR	FRAME	FIRE RATING (MINUTES)	HARDWARE	FRAME	ANCHOR	HEAD	JAMB	SILL	REQUIREMENTS			
12ØA	3'-Ø" × 7'-Ø" × 1-3/4"	MI	HMI		L1, C1, H3, T1, W91, R9	5 3/4								
12ØB	3'-Ø" × 7'-Ø" × 1-3/4"	MI	HMI		L1, C1, H3, T1, WS1, RS	5 3/4								
1200	12'-Ø" × 14'-Ø"	ОНІ			BY SUPPLIER						POWER OPERATOR			
1200	12'-Ø" × 12'-Ø"	ОНІ			BY SUPPLIER									

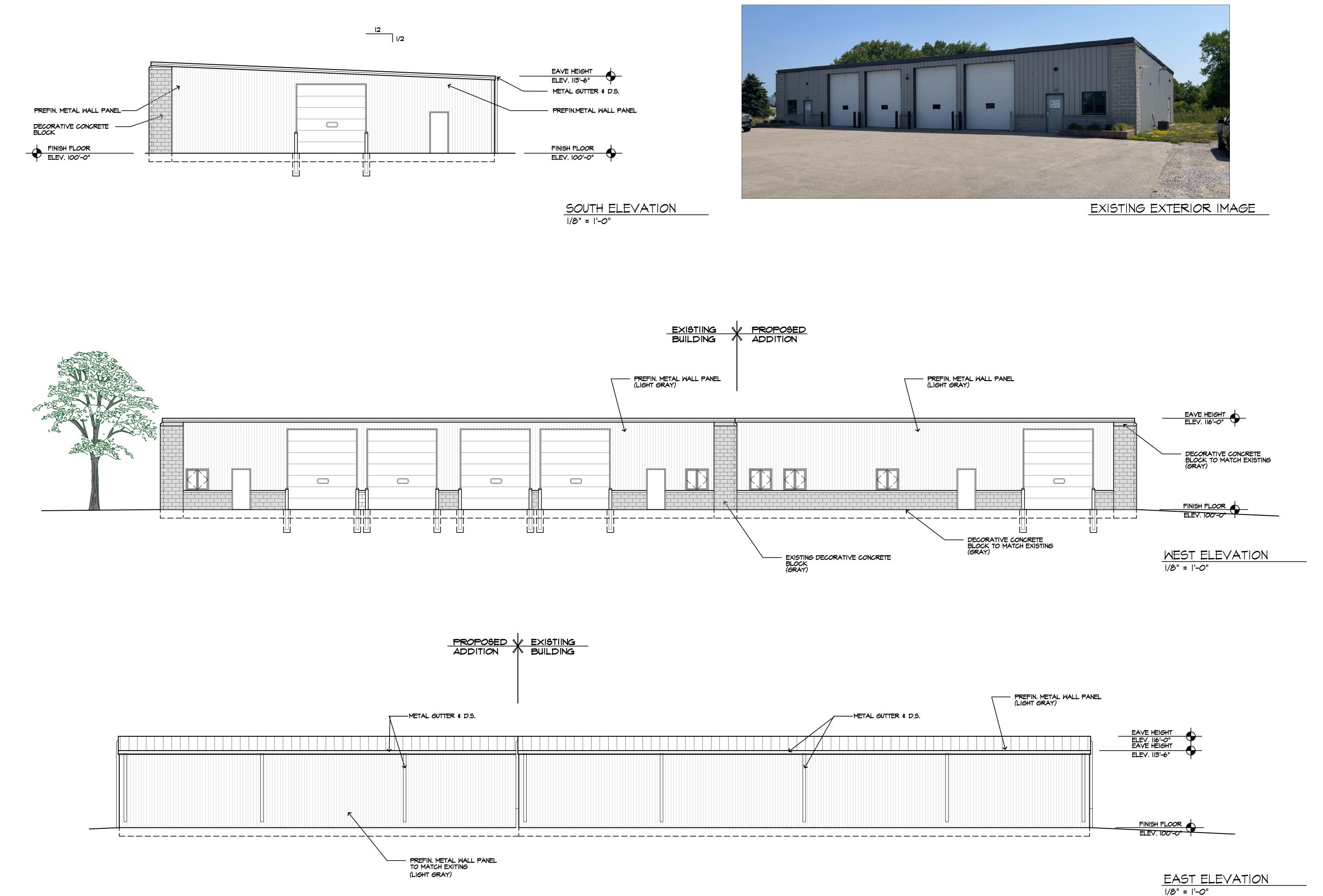
VERIFY ALL SCHEDULES W/ OWNER

	ROOM FINISH SCHEDULE										
	ROOM				WA	LLS		CEIL	_ING		
NO.	NAME	FLOOR	BASE	Ν	E	S	W	CLG	HGT	OTHER REQUIREMENTS	
120	SUITE 100	SC	-	MLP	MLP	ES	MLP	ES	VARIES	SEE PLAN FOR MLP HEIGHT	
101	UNISEX	SC	VB4	PD	PD	PD	PD	PD	8'-Ø"		

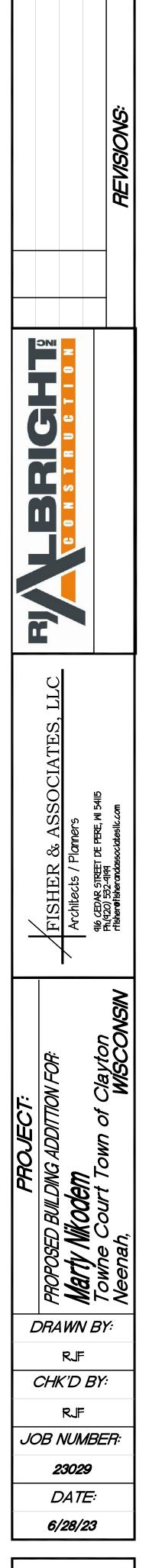
	FIN	ish ki	EY
	FLOOR		WALL
		PD	PAINTED DRYWALL
CTI	CERAMIC FLOOR TILE	FRP	FIBERGLASS PANEL OVER DRYWALL
CPT	CARPET-	CWT	CERAMIC WALL TILE
ШD	LAMINATED WOOD	MLP	METAL LINER PANEL
SC	SEALED CONC.		
	BASE		CEILING
VB4	4" VINYL BASE	ACTI	$2' \times 2'$ ACOUSTIC CEILING TILE - REVEALED EDGE
CT4	4" CERAMIC TILE	ACT2	$2' \times 2'$ ACOUSTIC CEILING TILE - VINYL COVERED D.W.
CT6	6" CERAMIC TILE	PD	PAINTED DRYWALL

-	SCH.

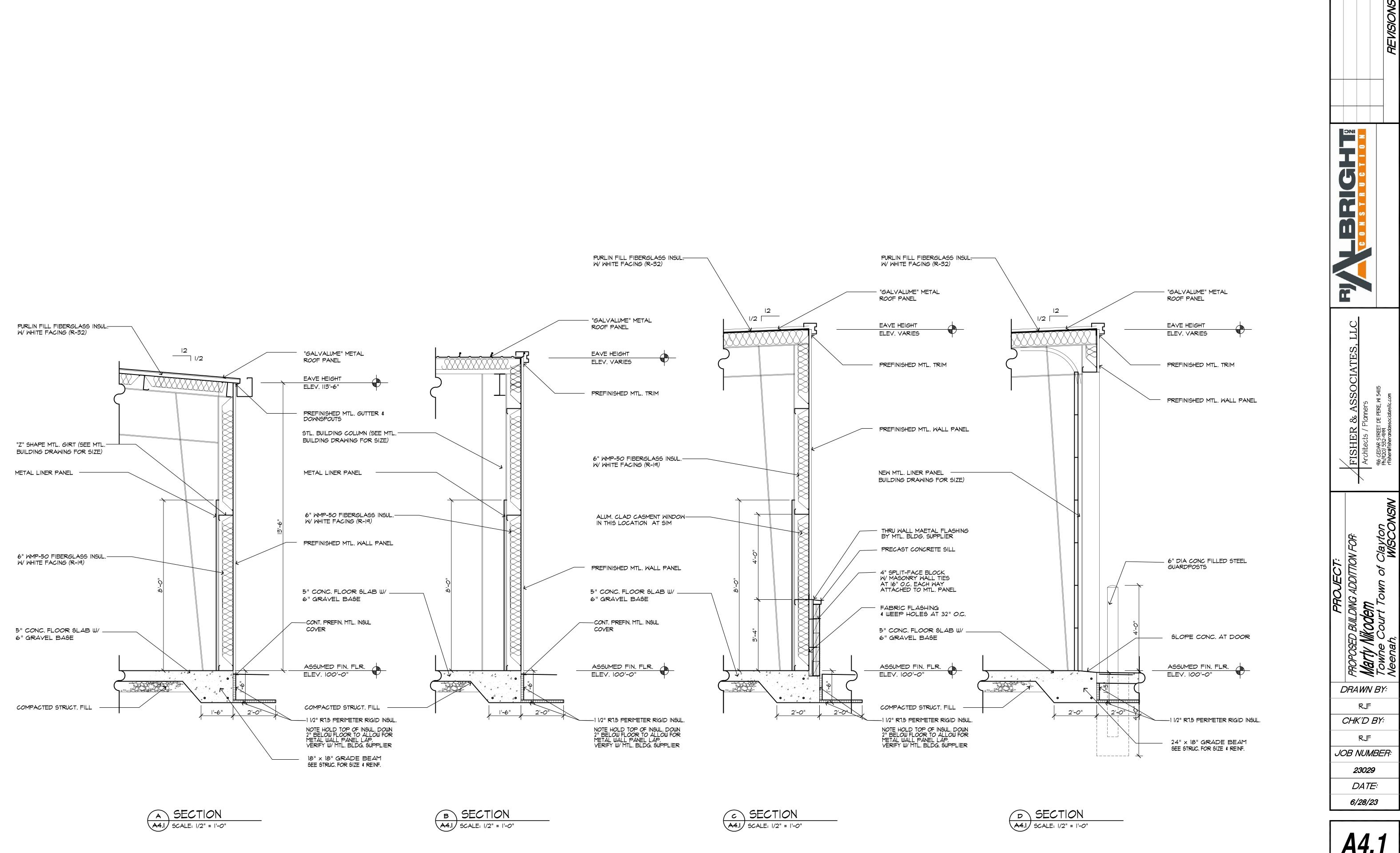






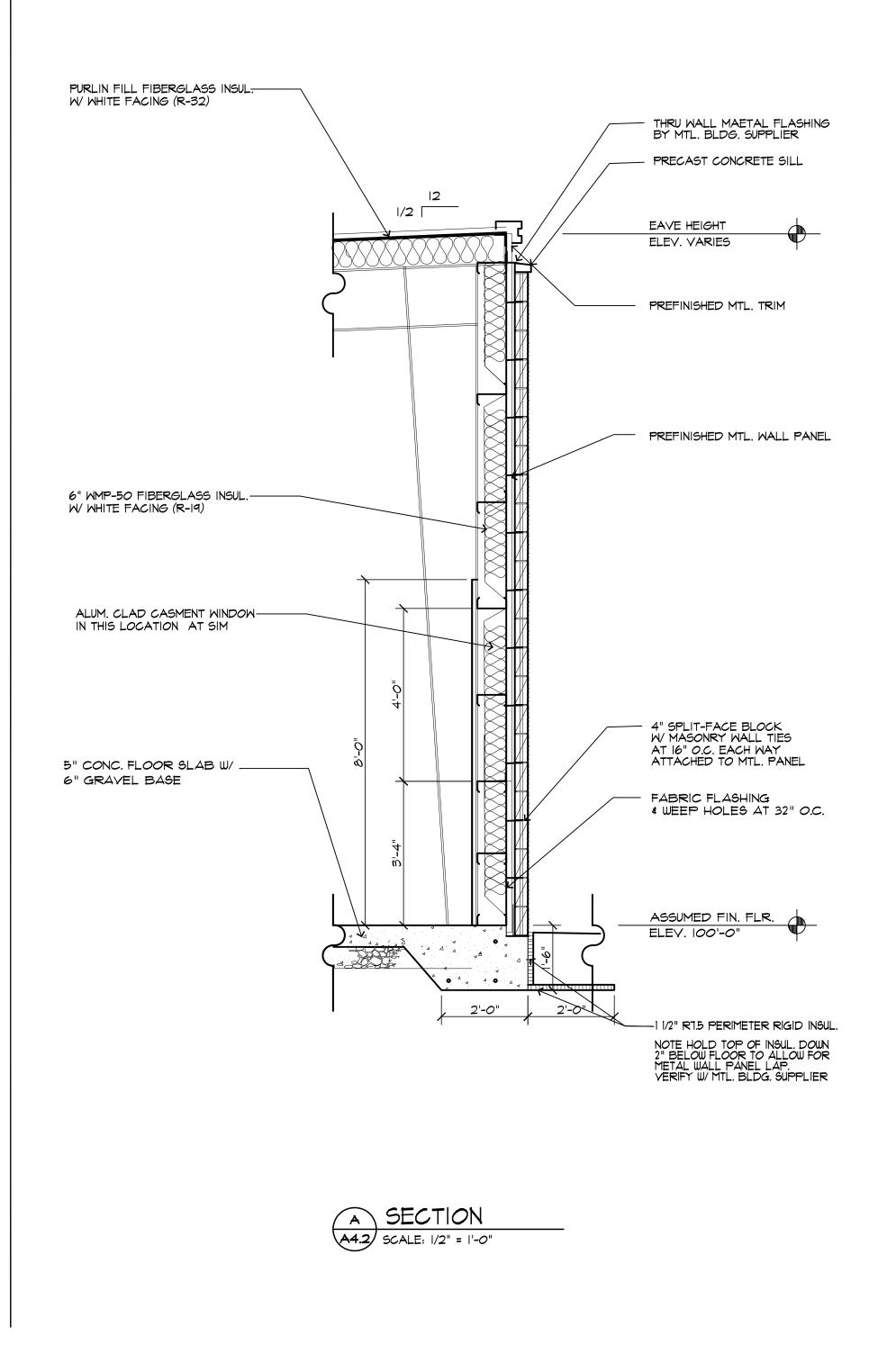


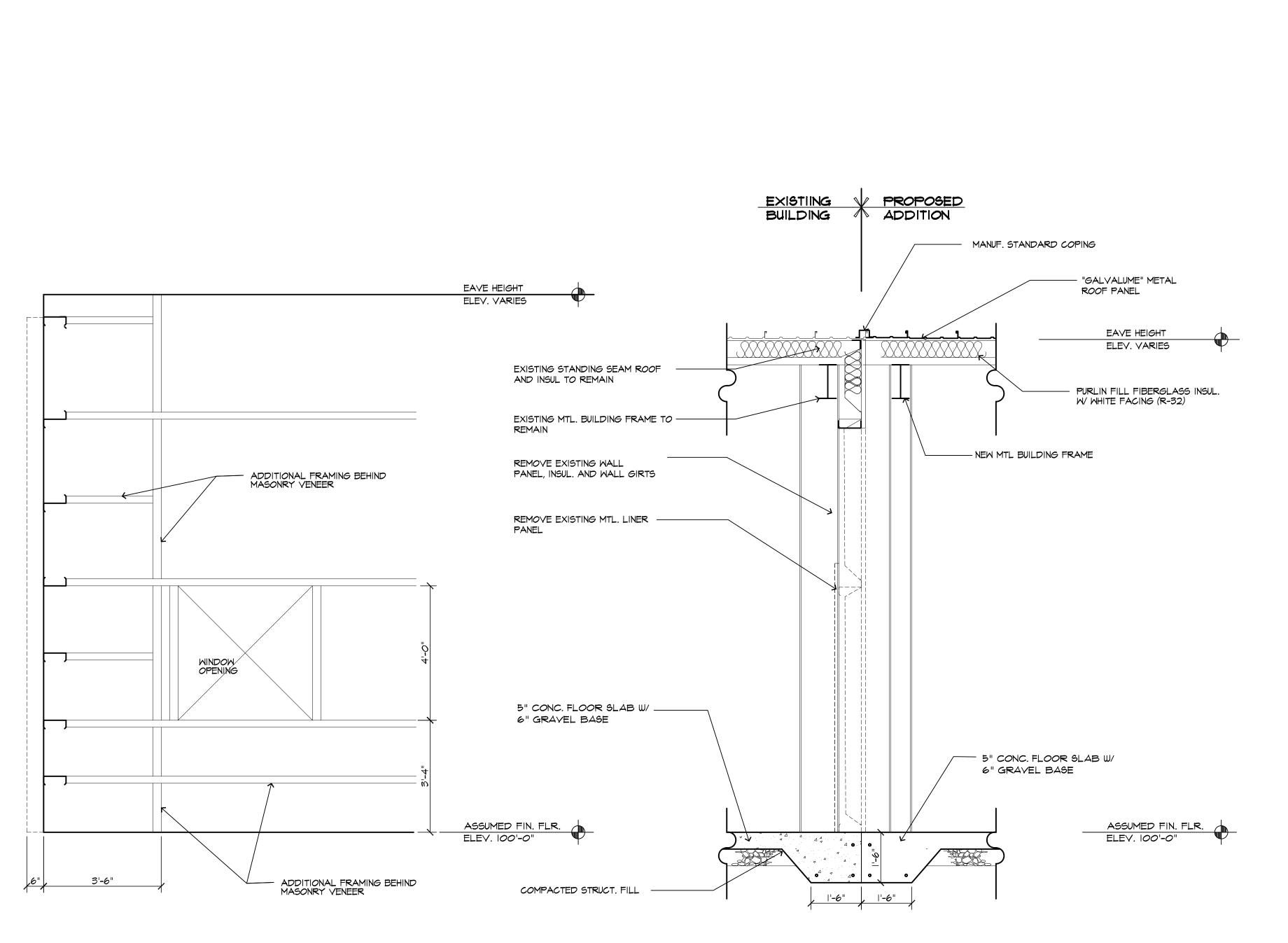
A3.1



A4.1

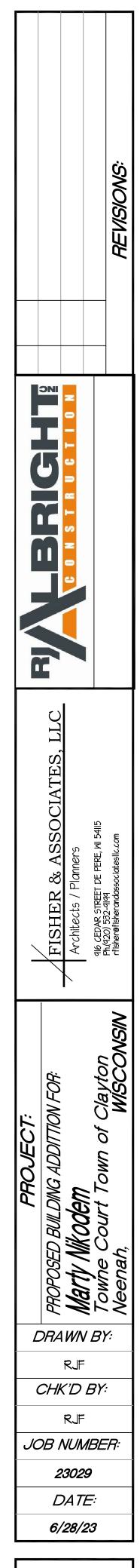
REVISIONS













I. ALL MATERIALS, WORKMANSHIP AND DETAILS SHALL CONFORM TO THE REQUIREMENTS OF	<u>REINFORCEMENT</u>				
THE 2009 EDITION OF THE "INTERNATIONAL BUILDING CODE".	I. DETAILING, FABRIC FOLLOWING:	CATION AND ERECT	TION OF 1	REINFOR	RCING STEE
 THE CONTRACTOR SHALL FAMILIARIZE HIMSELF WITH ALL ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS TO VERIFY THE LOCATION AND DIMENSIONS OF CHASES, INSERTS, OPENINGS, SLEEVES, REGLETS, DEPRESSIONS AND OTHER PROJECT REQUIREMENTS NOT 	ACI 318 - "B	PETAILS AND DETAIL BUILDING CODE REQU SI MANUAL OF STANI	UIREMENTS	5 FOR RE	EINFORCED
SHOWN ON THE STRUCTURAL DRAWINGS.	AMS DI.4 - " WRI - "WELD	"STRUCTURAL WELDIN DED WIRE FABRIC MA	NG CODE ANUAL OF	- REINFO STANDA	ORCING STE ARD PRACTI
APPROVAL FROM THE ARCHITECT & ENGINEER.	2. STEEL REINFORCIN WELDED WIRE FAB				
NOTIFY ARCHITECT OF ANY DISCREPANCIES. THE TYPICAL DETAILS SHOWN ON THE DRAWINGS SHALL BE APPLICABLE TO ALL PARTS	3. REINFORCEMENT F. REQUIRED REINFOR REINFORCEMENT S	RCING STEEL AND	THE NECE	ESSARY	r accesso
OF THE CONTRACT DRAWINGS UNLESS SPECIFICALLY NOTED OTHERWISE. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR JOB SAFETY DURING	4. CLEARANCES FOR SLABS, ETC.) 3" FR				
CONSTRUCTION.	REINFORCING. 5. CONTRACTOR SHA	LL REFER TO TYP	ICAL DET	TAILS SI	HOWN ON C
ESIGN LOADS	ADDITIONAL REINF 6. WHERE REINFORCE	ORCEMENT REQUIR	REMENTS.	•	
OW LOADS: SEISMIC LOADS PER IBC 1613: OUND SNOW LOAD PG = 35 PSF SS = 4.5% OF SNOW LOAD PF = 24.5 PSF S1 = 3.5%	1. WELDED WIRE FAB	ON APPLIES.			
IOW EXPOSURE CE = 1.0 OCCUPANCI CATEGORY = 11 IOW LOAD IMPORTANCE I = 1.0 SITE CLASS = D IFEMAL FACTOR C = 1.0 SEISMIC CATEGORY = A	8. CONTRACTOR SHA	LL NOTIFY ARCHIT	TECT OF O	COMPLE	
SEISMIC RESISTANCE SYSTEM STEEL SYSTEM (R DLLATERAL = 3 PSF	R=3.0) AND ALLOW AT LE ARCHITECT TO INS			HEDULE	D CONCRE
ND LOADS:					
ASIC WIND SPEED V= 90 MPH KPOSURE C KLOSED BUILDING					
PORTANCE FACTOR I	REINFORCEMEI				$\frac{5 \text{ SPLI}}{3000}$
	BAR SIZE CLASS A	TOP BARS	+ +		#5 #6 36 43
DUNDATIONS	SPLICE	OTHERS			36 43 28 33
FOUNDATION WORK FOR THIS PROJECT SHALL CONSIST OF SPREAD FOOTINGS, CONTINUOUS WALL FOOTINGS AND SLABS-ON-GRADE.	CLASS B SPLICE	TOP BARS OTHERS	28 22		47 56 36 43
ALL FOUNDATIONS SHALL BE SUPPORTED ON APPROVED EXISTING SUBGRADE OR	NOTES				+
APPROVED COMPACTED STRUCTURAL FILL HAVING A MINIMUM ALLOWABLE BEARING CAPACITY OF 2,000 PSF PRESUMED.		ICE LENGTHS SI	HALL RF	E CLAS	SS B UN
THE ENGINEER ASSUMES NO RESPONSIBILITY FOR THE VALIDITY OF THE SUBSURFACE CONDITIONS DESCRIBED IN THE DRAWINGS, SPECIFICATIONS, TEST BORINGS OR	NOTED OT				
GEOTECHNICAL REPORTS. THESE DATA ARE INCLUDED TO ASSIST THE CONTRACTOR DURING BIDDING AND SUBSEQUENT CONSTRUCTION AND TO REPRESENT CONDITIONS ONLY AT SPECIFIC LOCATIONS AT THE PARTICULAR TIME OBSERVATIONS WERE MADE.	AND NO	RMAL WEIGHT C	CONCRET	TE	
ALL EXTERIOR FOUNDATIONS SHALL BEAR ON APPROVED SUBGRADE AT A MINIMUM DEPTH	ARE CAL RESPECT	_CULATED PER A	ACI 318 FED VAL	8–95. UES F	SECTION
OF 4'-0" BELOW ADJACENT FINISH EXTERIOR GRADE, OR SHALL BE FROST PROTECTED SHALLOW FOUNDATIONS MEETING THE REQUIREMENTS OF SEI/ASCE 32-01.	CODE RE	NSVERSE REINFO EQUIREMENTS. I	LENGTHS	S ARE	IN INCH
FOOTING ELEVATIONS SHOWN ON THE DRAWINGS REPRESENT ESTIMATED DEPTHS AND ARE NOT TO BE CONSTRUED AS LIMITING THE AMOUNT OF EXCAVATION REQUIRED TO REACH SUITABLE BEARING MATERIAL.	OF CON	S ARE HORIZON CRETE CAST IN RCEMENT. (EXCE	THE M	IEMBER	R BELOW
SUITABLE BEARING MATERIAL. CONTRACTOR SHALL PROVIDE TEMPORARY SUPPORTS AS REQUIRED TO PREVENT HORIZONTAL MOVEMENT OR VERTICAL SETTLEMENT WHICH WILL ENDANGER ADJACENT	5. SPLICE A	AND DEVELOPME SED ON THE SF	ENT LEN	IGTHS	IN THIS
STRUCTURES, STREETS OR UTILITIES.	SIX(6) E THAN OF	BAR DIAMETERS R EQUAL TO TW	AND C	ONCRE	ETE COVE
CONTRACTOR SHALL PROVIDE CONTROL OF SURFACE AND SUBSURFACE WATER PROMPTLY TO INSURE THAT ALL FOUNDATION WORK IS DONE IN THE DRY.	WEIGHT BEAMS OR (CONCRETE. COLUMNS:			LEAST 1.
NO FOUNDATION(S) SHALL BE PLACED ON FROZEN SUBGRADE. PROTECT IN-PLACE FOUNDATIONS AND SLABS-ON-GRADE FROM FROST PENETRATION UNTIL	ALL OTHERS	S:	COVER	R AT L	LEAST
THE PROJECT IS COMPLETE. FOUNDATION WALLS SHALL BE BRACED DURING BACKFILLING AND COMPACTION			SPACI	ING AT	LEAST
OPERATIONS. BRACING SHALL BE LEFT IN POSITION UNTIL PERMANENT STRUCTURAL SUPPORT SYSTEM IS INSTALLED AND APPROVED BY ARCHITECT.		,			
BACKFILLING SHALL BE DONE SIMULTANEOUSLY ON BOTH SIDES OF WALL.	REINFORCED MASONRY	-	ERIALS AN	ND CONS'	TRUCTION S
ONCRETE	CONFORM TO THE FOLLO	DWING:			
CONCRETE WORK SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING STANDARDS:	ACI 530.1-05/ASCE 6-05 ACI 530-05/ASCE 5-05/*				
ACI 301 - "SPECIFICATIONS FOR STRUCTURAL CONCRETE" ACI MCP - "MANUAL OF CONCRETE PRACTICE"	STRUCTURES.				
ACI 318 - "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" ACI 318.1 - "BUILDING CODE REQUIREMENTS FOR STRUCTURAL PLAIN CONCRETE"	2. CONCRETE BLOCK SHALL NET CROSS SECTIONAL OF				
CONCRETE SHALL HAVE A MINIMUM 28-DAY ULTIMATE COMPRESSIVE STRENGTH AS FOLLOWS:	3. MORTAR SHALL BE TYPE				
PILE CAPS AND GRADE BEAMS 4,000 PSI SLABS-ON-GRADE 4,000 PSI FOOTINGS AND WALLS 3,000 PSI PDECKST CONCEPTION FOOD PSI	 GROUT SHALL CONFORM METHOD, CONFORMING TO DRAWINGS. 				
PRECAST CONCRETE 5,000 PSI EXTERIOR EXPOSED CONCRETE 4,000 PSI	5. THE REQUIRED MINIMUM 2 CONCRETE BLOCK, GROL				
CONCRETE MIX DESIGN (INCLUDING AGGREGATE SIZE, WATER/CEMENT RATIO, AIR ENTRAINMENT, ADMIXTURES AND SLUMP) SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF ANY WORK. MAXIMUM WATER/CEMENT RATIOS	BE 1,830 PSI. 6. ALL CONCRETE BLOCK M				PUNNING BI
PERMITTED AS FOLLOWS: 0.50 FOR SLABS-ON-GRADE	7. MASONRY BLOCK CELLS				
0.54 FOR BELOW GRADE CONCRETE 0.48 FOR EXPOSED CONCRETE	SOLID. FILLING CELLS W				
CONCRETE TO BE EXPOSED TO THE WEATHER SHALL HAVE AIR-ENTRAINING ADMIXTURE AS REQUIRED TO PROVIDE 4-6% AIR ENTRAINMENT.	 THE BASE OF EACH CELL REINFORCING STEEL SHAL 		,	,	
CONCRETE STRENGTH SHALL BE EVALUATED ACCORDING TO METHOD I OR METHOD 2 AS DESCRIBED IN ACI 301. THE RESULTS OF THESE ANALYSES SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO ANY WORK.	IO. PROVIDE CONTINUOUS RE	INFORCED BOND-BE	EAMS IN AI	LL REINF	FORCED MA
CONTRACTOR SHALL MAKE PROVISIONS TO ALLOW AN INDEPENDENT TESTING AGENCY, HIRED BY THE OWNER, TO CAST 4 TEST CYLINDERS FOR EACH 50 CUBIC YARDS OF	WALLS AT TOPS OF WALL CALLED FOR IN CONTRAC AND BE CONTINUOUS WITH	CT DRAWINGS. BONI	ID BEAM R	REINFORC	
CONCRETE PLACED OR FOR ANY ONE DAY'S OPERATION. TESTING AGENCY SHALL BE RESPONSIBLE FOR CASTING AND CURING SPECIMENS IN CONFORMANCE TO ASTM C3I AND TESTING SPECIMENS IN CONFORMANCE TO ASTM C39.	II. REINFORCED MASONRY W.			•	
CONSTRUCTION JOINTS SHOWN ON THE CONTRACT DRAWINGS SHALL NOT BE ALTERED	REINFORCING AT SPACIN OF 16" O.C.	U AD NUIED ON THE	L CONTRA	UI URAK	minos, BUT
WITHOUT WRITTEN APPROVAL OF THE ARCHITECT. DRAWINGS SHOWING THE LOCATION OF CONSTRUCTION JOINTS, CONTROL JOINTS AND PLACING SEQUENCE SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO THE	12. FILL CORES OF MASONR TIMES THE BEARING PLA				
PLACING SEQUENCE SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO THE PREPARATION OF REINFORCING SHOP DRAWINGS.	13. PROVIDE AND INSTALL O		7H 4" OF M	VALL THI	ICKNESS Ar
GROUT USED TO SET PLATES SHALL BE NON-SHRINK AND NON-METALLIC.	THE FOLLOWING SCHEDUL		I-BEARING	> WALLS	
FORMS MAY BE USED FOR UNEXPOSED CONCRETE SURFACES. EARTH FORMS ARE FORBIDDEN.		OPENING LINTEL 3'-0" 3 1/2X3	3 1/2X 5/16	6	
PROVIDE A MINIMUM OF 6" COMPACTED GRANULAR FILL UNDER ALL SLABS-ON-GRADE. . FLATWORK CONTRACTOR SHALL SUBMIT FLOOR SLAB PLACEMENT SEQUENCE TO ENGINEER		5'-0" L4X3 I,	/2X5/ 6 /2X5/ 6 /2X5/ 6		
FOR APPROVAL PRIOR TO BEGINNING WORK. FLOOR FLATNESS AND LEVELNESS: CONCRETE SLABS-ON-GRADE SHALL HAVE MINIMUM F			1/2×5/16 1/2×5/16		
NUMBERS OF FF 35/FL25 AS RECOGNIZED BY THE MOST CURRENT VERSION OF ASTM E 1155 AND ACI 302.1. SEE SPECIFICATIONS FOR FURTHER RESTRICTIONS IF APPLICABLE.	LINTELS SHALL BEAR ANGLE SHALL BE VER		N EACH SI	IDE OF C	OPENING. 1
5/8"	DIA. X 18" SMOOTH R	OD AT	_		
SAWCUT C.J. OR $_{\sim}$ 12" C	D.C. COAT ONE END REVENT BONDING				
	<u> </u>		 		· · · · · · · ·
		an an Natalian		· · · · ·	• • • • • • •
		\backslash			
					ED FILL
	<u></u>	DTE: TO BE U END OF	JSED A	٩T	
PLASTIC INSERT			JSED A EACH	AT POUR	

TNIOL NOITS

IN OF REINFORCING STEEL SHALL CONFORM TO THE

OF CONCRETE REINFORCEMENT" EMENTS FOR REINFORCED CONCRETE"

CODE - REINFORCING STEEL" AL OF STANDARD PRACTICE" ORM TO ASTM A615 (GRADE 60), DEFORMED.

ROVIDE AND SCHEDULE ON SHOP DRAWINGS ALL NECESSARY ACCESSORIES TO HOLD

THE CORRECT LOCATIONS. ONCRETE PLACED DIRECTLY ON EARTH (FOOTINGS, THER CONCRETE PROVIDE 2" CLEAR TO

L DETAILS SHOWN ON CONTRACT DRAWINGS FOR

SECTIONS, REINFORCEMENT IS CONSIDERED TYPICAL

NIMUM OF 6" AND BE TIED TOGETHER. T OF COMPLETION OF REINFORCEMENT INSTALLATION RE SCHEDULED CONCRETE PLACEMENT FOR

ENT AND SPLICE LENGTH SCHEDULE KSI f'c = 3000 PSI <u>#3</u> <u>#4</u> <u>#5</u> <u>#6</u> <u>#7</u> <u>#</u>8 <u>#9</u> <u>#10</u> <u>#11</u>

#3	#4	#5	#6	#/	#8	#9	#10	#11
22	29	36	43	63	72	91	91	101
17	22	28	33	48	55	62	70	78
28	37	47	56	81	93	105	118	131
22	29	36	43	63	72	81	91	101

ALL BE CLASS B UNLESS

ASED ON GRADE 60 REINFORCING BARS

NGTHS AND TENSION LAP SPLICE LENGTHS 318–95. SECTIONS 12.2 AND 12.15 VALUES FOR BEAMS AND COLUMNS ARE BASED CEMENT AND CONCRETE COVER MEETING MIN. NGTHS ARE IN INCHES.

BARS WITH MORE THAN 12" HE MEMBER BELOW THE

LENGTHS IN THIS SCHEDULE CING GREATER THAN OR EQUAL TO ID CONCRETE COVER GREATER 2) INCHES, IN NORMAL

COVER AT LEAST 1.0 BAR DIA. AND C.-C. SPACING AT LEAST 2.0 BAR DIA. COVER AT LEAST 1.0 BAR DIA. AND C.-C. SPACING AT LEAST 3.0 BAR DIA.

ALS AND CONSTRUCTION SHALL

ATIONS FOR MASONRY STRUCTURES"

CODE REQUIREMENTS FOR MASONRY -90. THE REQUIRED STRENGTH ON THE

T MAY BE PLACED BY THE "HIGH LIFT" IRS REQUIRED BY THE CONTRACT

STRENGTH OF THE COMBINATION OF

NET AREA OF THE WALL (F'M) SHALL

BE LAID IN A RUNNING BOND. . REINFORCING SHALL BE GROUTED

ACED, MUST HAVE A CLEANOUT HOLE.

5 IN ALL REINFORCED MASONRY N STEEL BEARINGS AND WHEREVER

EAM REINFORCING SHALL EXTEND INTO

SAUGE (TRUSS TYPE) HORIZONTAL ONTRACT DRAWINGS, BUT AT A MINIMUM

PLATES FOR A WIDTH EQUAL TO THREE

" OF WALL THICKNESS ACCORDING TO

ACH SIDE OF OPENING. LONG LEG OF



STRUCTURAL STEEL I. STRUCTURAL STEEL WORK SHALL CONFORM TO THE FOLLOWING: "SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION AISC OF STEEL FOR BUILDINGS".

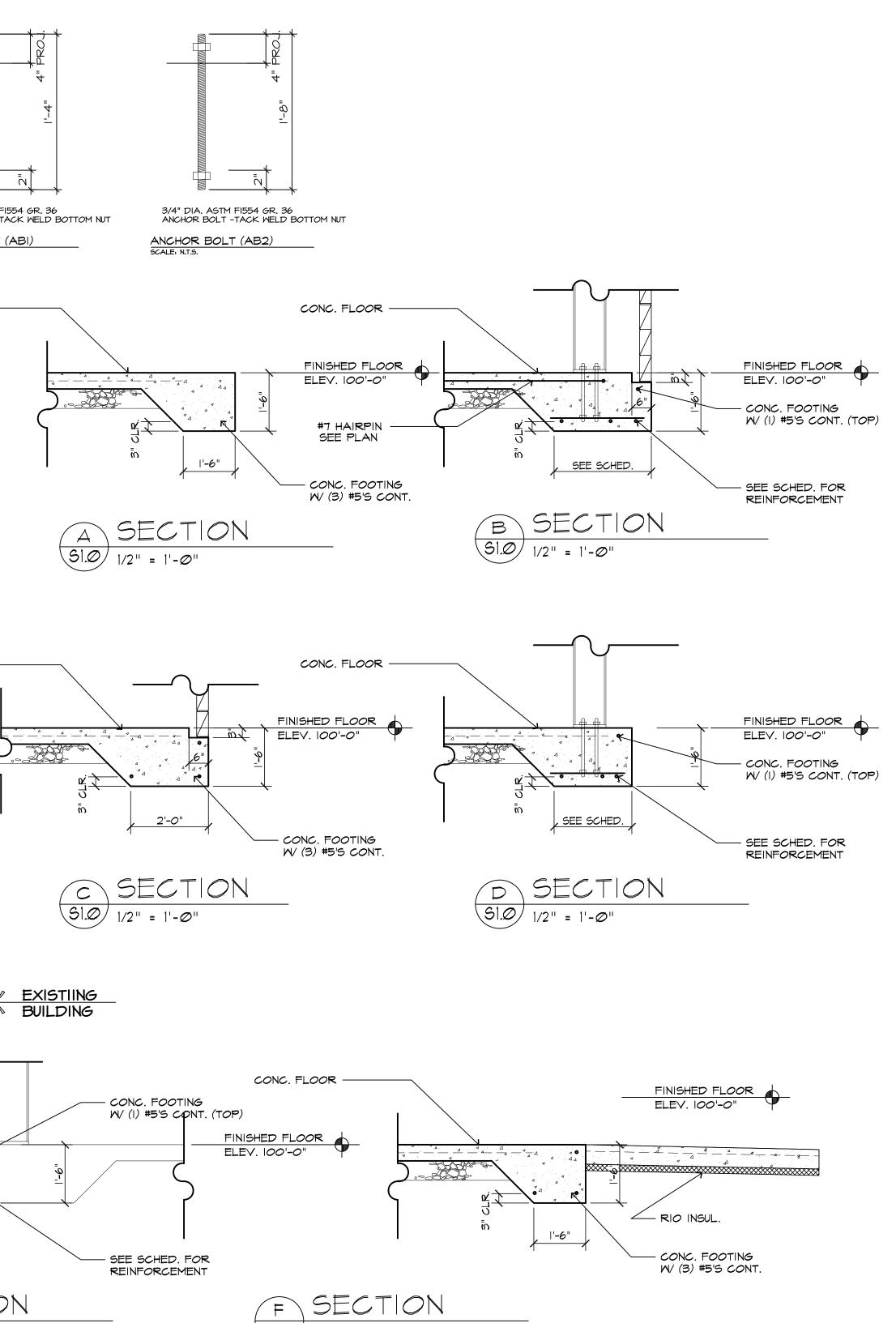
AISC - "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES". AWS DI.I - "STRUCTURAL WELDING CODE - STEEL".

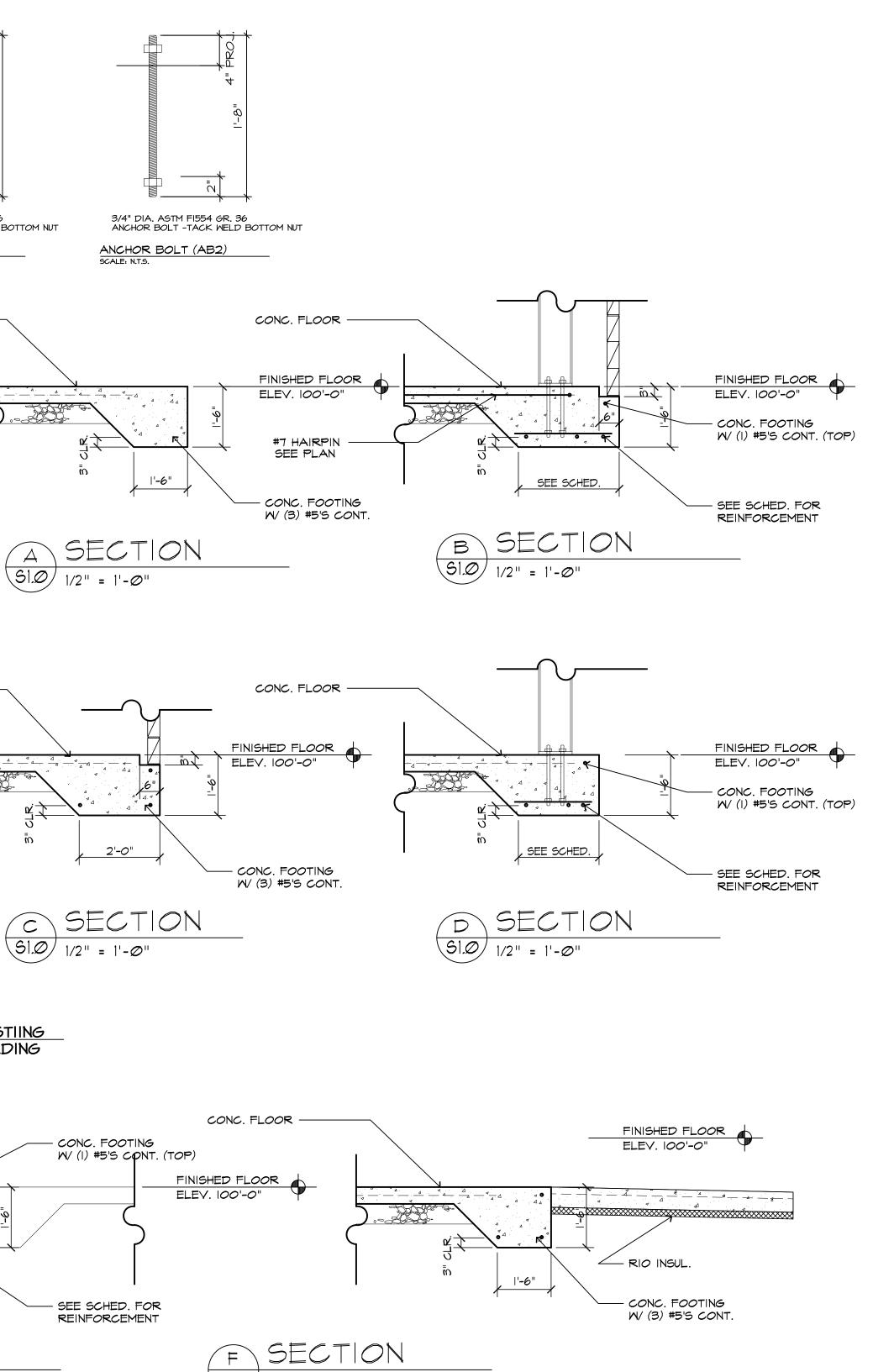
- AISC "STRUCTURAL STEEL DETAILING MANUAL".
- 2. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:
 - HOT ROLLED WIDE-FLANGE ASTM A992 (FY=50 KSI) ALL OTHER STRUCTURAL SHAPES AND PLATES - ASTM A36 (FY=36KSI) STRUCTURAL STEEL PIPE - ASTM A53 GRADE B (FY=35 KSI).
- TUBULAR STEEL ASTM A500 GRADE B (FY=46 KSI) HIGH STRENGTH BOLTS - ASTM A325N (BEARING TYPE) ASTM A325F (FRICTION TYPE)
- ASTM FI554 GRADE 36 THREADED ROD MAY BE USED IN PLACE OF ASTM FI554 ROD. 3. PROVIDE 2 MIL, THICK RED OR GREY OXIDE PRIMER ON ALL STEEL SURFACES UNLESS NOTED OTHERWISE.
- 4. ANCHOR BOLTS SHALL BE PRESET WITH TEMPLATES AT REQUIRED LOCATIONS. 5. LEVELING PLATES AND BEARING PLATES SHALL BE SET IN FULL BED OF NON-SHRINK
- 6. CONNECTIONS MAY BE EITHER BOLTED OR WELDED AT THE FABRICATOR'S OPTION. BOLTED CONNECTIONS SHALL BE AS FOLLOWS:
- MINIMUM BOLT DIAMETER: 3/4" SHEAR CONNECTIONS FOR MOMENT CONNECTED MEMBERS: FRICTION TYPE HIGH STRENGTH BOLTS IN SINGLE OR DOUBLE SHEAR.
- SHEAR CONNECTIONS FOR OTHER MEMBERS: BEARING TYPE HIGH STRENGTH BOLTS IN SINGLE OR DOUBLE SHEAR. SIMPLE SHEAR CONNECTIONS SHALL BE CAPABLE OF END ROTATION PER AISC REQUIREMENTS FOR "UNRESTRAINED MEMBERS".
- ALL BEAM CONNECTIONS NOT DETAILED, SHALL SUPPORT 1/2 THE TOTAL UNIFORM LOAD CAPACITY FOR THE GIVEN BEAM AND SPAN OR THE INDICATED REACTION, WHICHEVER IS GREATER. CONNECTIONS SHALL GENERALLY FOLLOW THE TYPES SHOWN IN THE "AISC MANUAL OF STEEL CONSTRUCTION," TABLE II, III, OR X.
- WELDS SHALL FULLY DEVELOP STRENGTH OF THE MATERIALS BEING WELDED, UNLESS NOTED OTHERWISE, EXCEPT THAT FILLET WELDS SHALL BE A MINIMUM 3/16".
- 9. WELDED CONNECTIONS SHALL BE MADE BY APPROVED CERTIFIED WELDERS USING FILLER METAL CONFORMING TO ETOXX.
- IO. CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SUPPORTS TO HOLD STRUCTURAL STEEL FRAMING SECURELY IN POSITION. TEMPORARY BRACING SHALL REMAIN UNTIL THE PERMANENT LATERAL BRACING HAS BEEN INSTALLED AND AND THE CONCRETE FOR FLOOR SLABS HAS ATTAINED 75% OF ITS REQUIRED STRENGTH. STRUCTURAL STEEL FRAMING SHALL BE TRUE AND PLUMB BEFORE FINAL BOLTING OR WELDING OF CONNECTIONS.
- 12. CONTRACTOR SHALL NOT MODIFY OR CUT ANY STRUCTURAL STEEL WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.
- 13. CONTRACTOR SHALL FIELD TOUCH UP ALL ABRASIONS, BURNS AND SIMILAR DEFECTS IN PAINT OF THE STRUCTURAL STEEL, JOISTS AND STEEL DECK.
- COLD FORMED STRUCTURAL STEEL MEMBERS INCLUDING LIGHT GAUGE STEEL ALL COLD FORMED STRUCTURAL MATERIAL AND WORK SHALL CONFORM TO THE LATEST EDITIONS OF THE FOLLOWING:
- AISI "SPECIFICATIONS FOR THE DESIGN OF LIGHT-GAUGE COLD-FORMED STEEL STRUCTURAL MEMBERS"
- AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" 2. COLD FORMED STEEL SHALL CONFORM TO THE FOLLOWING:
- LIGHT GAUGE STUDS, JOIST, TRACKS AND ACCESSORIES: ASTM A 653 /653 M, 660 GALVANIZED.

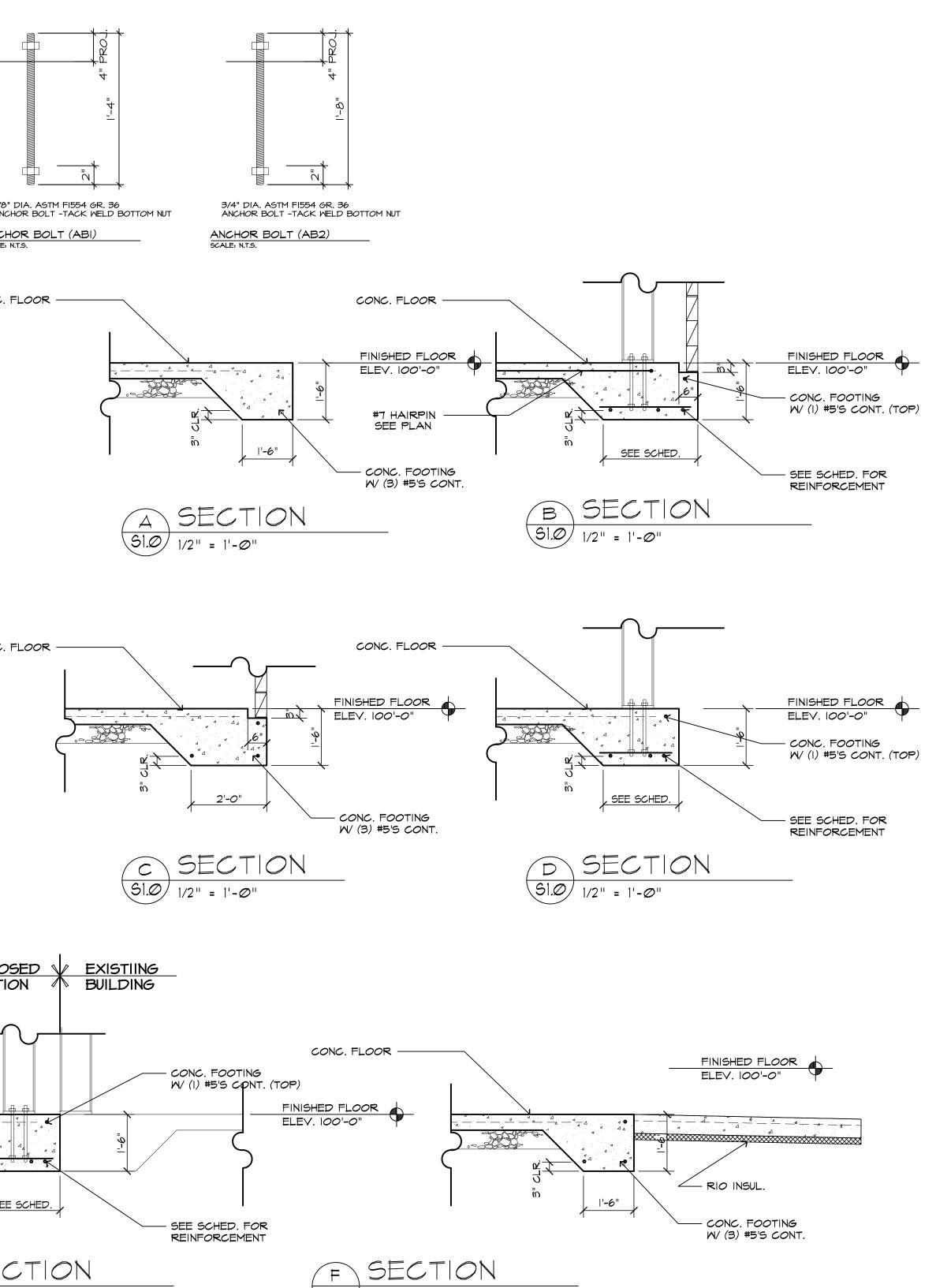
ASTM 5653 COLD FORMED GALVANIZED LIGHT GAUGE FRAMING MEMBERS

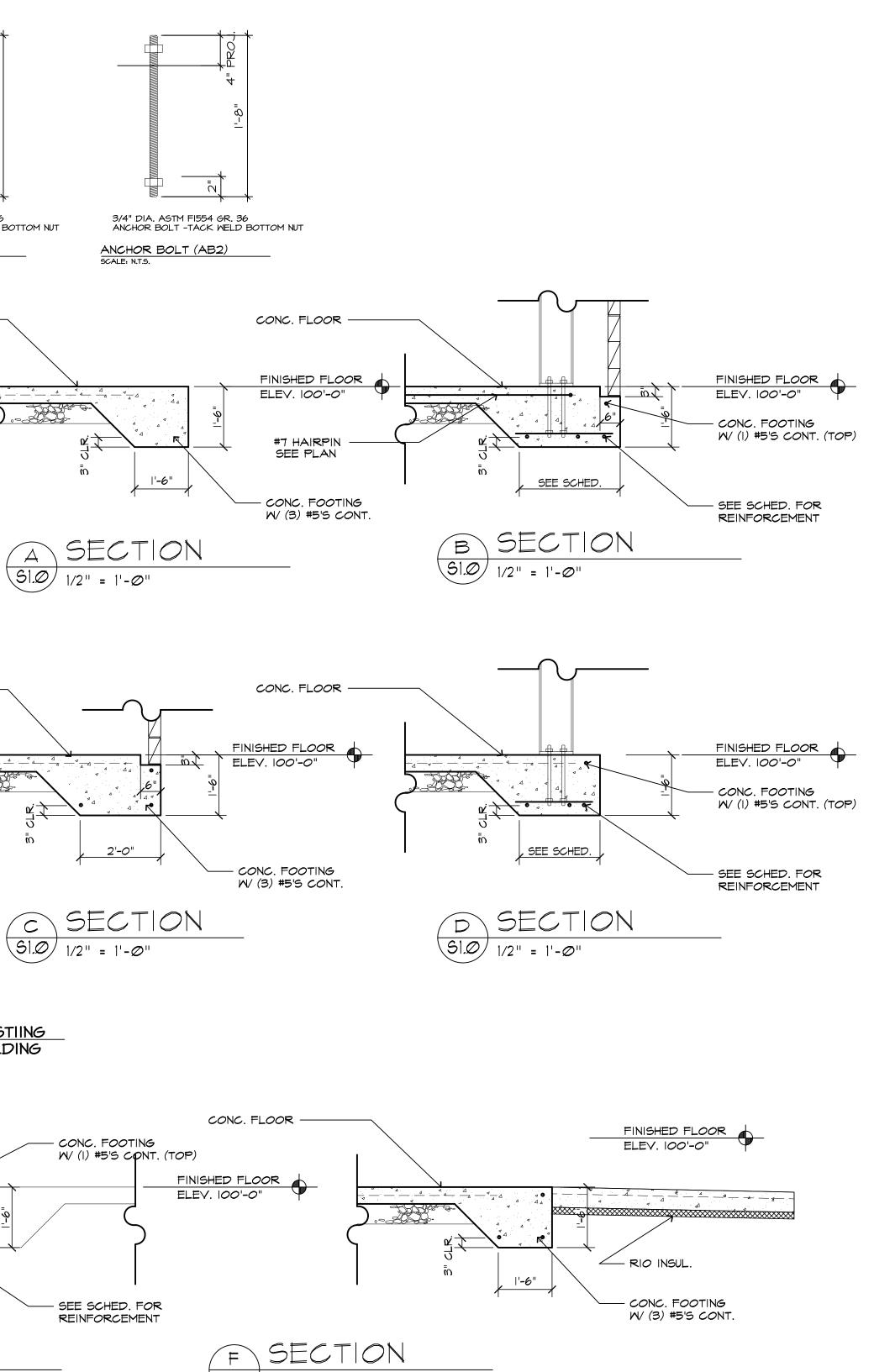
- A. STUDS AND JOISTS SHALL BE CHANNEL SHAPED WITH LIPPED FLANGES, PUNCHED WEB, SIZE, GAUGE AND GRADE AS SHOWN ON THE DRAWINGS (AS REQUIRED BY STRUCTURAL CALCULATIONS).
- B. TRACKS SHALL BE CHANNEL SHAPED SOLID WEB, DEPTH COMPATIBLE WITH STUDS, GAUGE AND GRADE AS SHOWN ON THE DRAWINGS (AS REQUIRED BY STRUCTURAL CALCULATIONS).
- C. FRAMING ACCESSORIES SHALL HAVE A MINIMUM YIELD STRENGTH OF 65 KSI.
- D. SCREWS SHALL BE CORROSION RESISTANT, SELF DRILLING PAN OR HEX WASHER HEAD AS SHOWN ON THE DRAWINGS (AS REQUIRED BY STRUCTURAL CALCULATIONS). E. POWDER ACTUATED FASTENERS: AISI 1062 OR 1065 STEEL, MINIMUM CORE HARDNESS 50 TO 54 HRC AND ZINC PLATED IN ACCORDANCE WITH ASTM B 633
- DIAMETER AND LENGTH AS SHOWN ON THE DRAWINGS (AS REQUIRED BY STRUCTURAL CALCULATIONS). F. ALL LIGHT GAUGE FRAMING MEMBERS SHALL BE DIETRICH CSJ OR CSW SERIES UNLESS NOTED OTHERWISE.
- G. WIND DESIGN LOADING FOR SECONDARY FRAMING PER (THE LATEST ADDITION OF ASCE 7 FOR COMPONENTS AND CLADDING, WIND DESIGN SPEED AND EXPOSURE ARE NOTED UNDER DESIGN LOADS.
- H. MATERIAL SUPPLIER IS RESPONSIBLE FOR MEMBER DESIGN, THEREFORE CALCULATIONS MUST BE INCLUDED WITH SHOP DRAWINGS FOR APPROVAL.

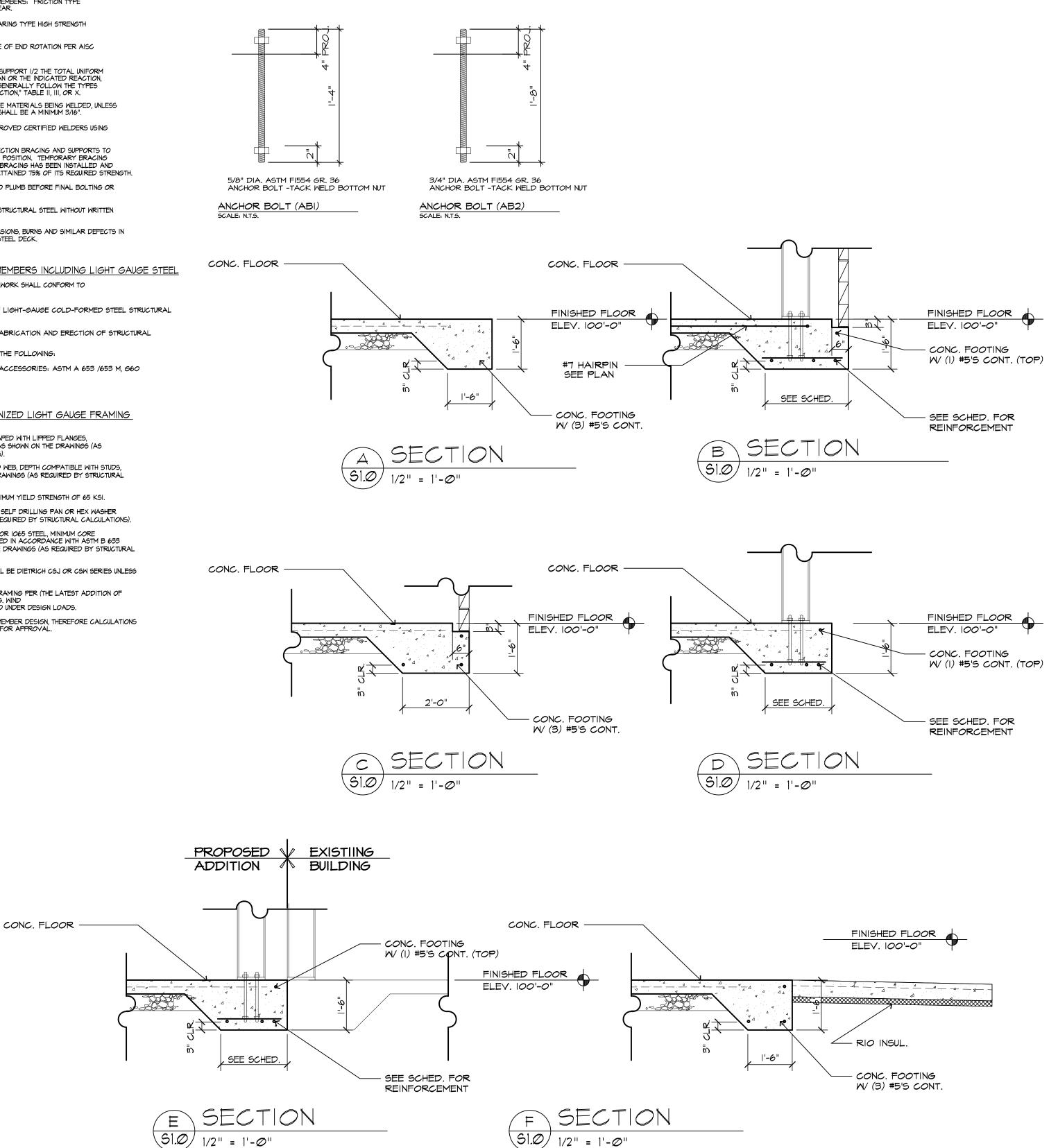
____ 5/8" DIA, ASTM FI554 GR, 36











FOUNDA DEPTH REINFORCING

18"

18"

18"

18"

18"

MARK | PLAN

F-I 3'-0" × 3'-0"

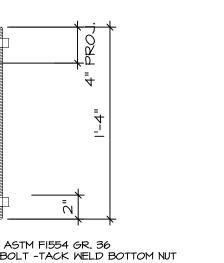
F-2 3'-0" × 4'-0"

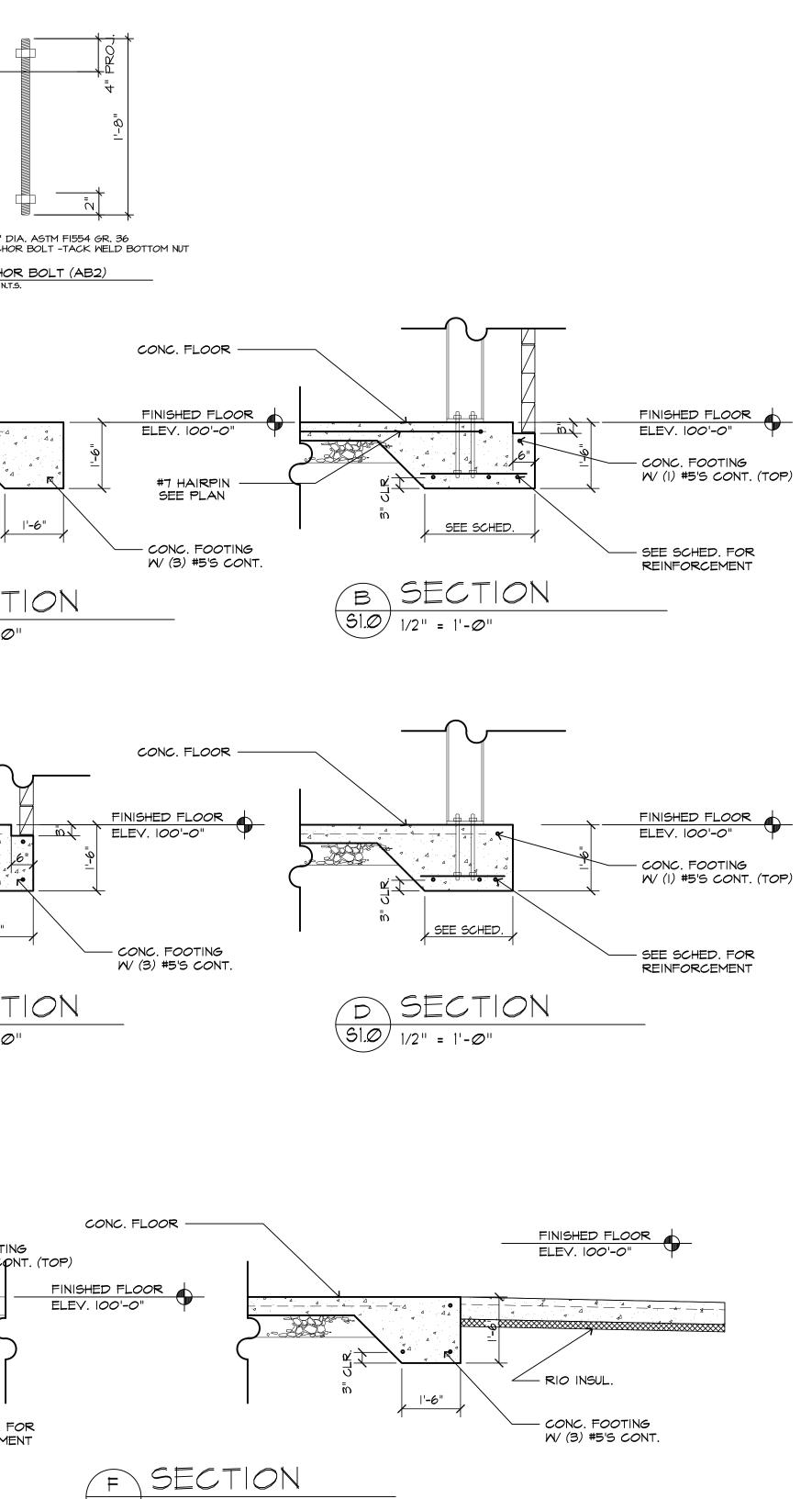
F-3 4'-0" × 7'-0"

F-4 | 4'-0" × 4'-0"

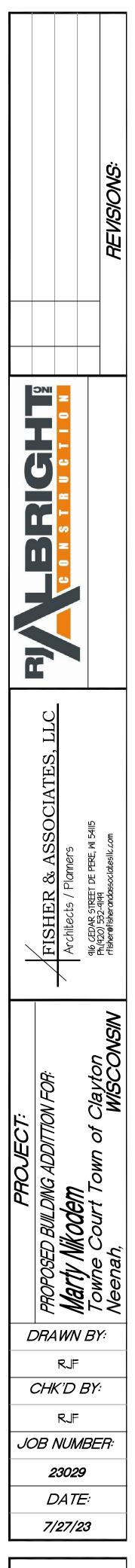
F-5 | 3'-0" × 4'-0"

DIMENSION





DUNDATI	UNDATION SCHEDULE										
REINFORCING	T.O. FTG.	ANCHOR BOLTS			PIER						
			T.O.P.	PLAN DIM.	REINFORCING						
(3) #5 EACH WAY	100'-0"	ABI									
(3) #5'S LONG WAY (4) #5'S SHORT WAY	100'-0"	ABI									
(4) #6'S LONG WAY (8) #6'S SHORT WAY	100'-0"	AB2									
(4) #5 EACH WAY	100'-0"	ABI									
(4) #6'S LONG WAY (8) #6'S SHORT WAY	100'-0"	ABI									
•		•									



S1.0

