



September 2, 2026

John Wiesemann, Code Enforcement Officer
Town of Casco
635 Meadow Road
Casco, Maine 04015

**Re: Response to Review Comments
 Country Village Assisted Living Expansion
 960 Meadow Road
 960 Meadow Road Holdings, LLC - Applicant**

Dear Mr. Wiesemann and Planning Board Members:

On behalf of 960 Meadow Road Holdings, LLC, applicant, we have prepared the enclosed revised design plans and documents addressing review comments for the proposed expansion of the existing Country Village Assisted Living Facility on Meadow Road. We were provided comments within a memorandum from the Town's Acting Planner, Doug Webster dated July 22, 2026, and an engineering peer review memorandum, provided by the Town's peer review engineer Sebago Technics, Inc., dated August 13, 2026. Below are our responses to the comments.

PLANNING COMMENTS

Procedural Matters

- *Right Title or Interest*
 - **To prove right, title or interest for Craig Alaimo to act as an authorized representative for the property owner during Town meetings, we provided a copy of the Annual Report for the corporation indicating that he is the manager and registered agent for 960 Meadow Road Holdings, LLC. In addition, the Planning Board was provided a letter utilized in the previous Zoning Board of Appeals submission, but was omitted from the Planning Board application submission. Both documents have been included in this submission.**
- *Public Notifications*
 - **The Planning Board public meeting notifications for the July 27, 2026 meeting were certified mailed to each abutter within 500 feet of the project site. The receipt cards were provided to the Town prior to the initial Planning Board meeting.**

Stormwater Comments

Comment a - There appears to be relatively extensive re-grading of the area to the west (downgradient) of the four proposed SSWD fields that come quite close to the Magiera property line running generally north-south.

Comment b - The termination of the rip-rap swale and associated level lip spreader is also quite close to the Magiera property line/corner on the northwest corner of the subject property

Comment c - The proposed stormwater settling area with the underdrain is in very close proximity (re-grading proposed up to the line) to the Magiera property line running generally west-east (perpendicular to Meadow Rd./Rt. 121). I believe that they have an existing parking area in this vicinity which is used in conjunction with their training business. Perhaps consideration should be given to the implications of existing pre-development conditions compared to proposed post-development conditions by extending both pre and post development contours over/onto the Magiera property line to better understand the likely outcome. The purpose of this is to enable the Board and stakeholders to make informed decisions

Additional Planning Board Discussion - It was also discussed at the Planning Board meeting to have the areas of proposed land disturbance be field surveyed as an alternative to the aerial contours to ensure construction remains on the project site and not expanding onto the abutter's property.

Response – We have worked with Survey, Inc., surveyor who prepared the boundary survey, to perform the on-the-ground topographic survey in the areas around the limits of the property with additional areas of the abutting property to the north. The results of this survey have been incorporated into the design plans.

Utilizing the new survey data, we further examined the drainage conditions from the abutting property to determine the limits of the watersheds and drainage directions. A third study point was created to analyze the peak rates of runoff generated by the project site crossing the northern property boundary. The results of the analysis indicate a reduction in peak rates of runoff at this location as the majority of the original land area that was draining to the north, is now directed away from the property and into the underdrained filter basin for stormwater treatment and detention. In addition, a portion of the existing development to the abutting property to the north was also added to the stormwater model to better understand the amount of flow draining from the abutters parking lot to the base of the steeper slope and into the forested wetlands.

As a result of the stormwater infrastructure, the peak rates of runoff at all study points have been reduced from the existing conditions. An updated stormwater management report has been included in this submission.

Comment d - Based on the proposed contour lines and the spot grades, it appears that the drainage from courtyard area (with pavers) drains to the proposed parking lot. Such relatively gentle grades with water sheet flow can be problematic with regards to winter storms (with rapidly falling temperatures) causing icing. This situation also increases the volume of sheet-flow water across the parking lot which is discharged upgradient of proposed disposal fields "A" and "B".

Response d – We did examine the ability to install a closed drainage system, which would address the concerns. The problem with a catch basin system is the need for the pipes to be underground

with adequate cover for frost protection, making the drainage system 4 to 5 feet lower than the surface grades. Due to the constraints of the property line, the proposed building and wastewater disposal bed “A”, there is limited space to provide the required underdrained filter basin’s treatment storage, and more importantly, its filter surface area, we could not make the basin any deeper than it is currently designed. The facility manager is aware of the project expansion and constraints and will take precautions for any potential icing.

Comment e - The two proposed rip-rap aprons apparently designed to address the stormwater sheet-flow from parking lot discharge just above proposed disposal fields “A” and “B”. Local Plumbing Inspectors are trained to attempt to avoid introducing water upgradient of disposal fields. Perhaps consideration be given to adding additional stormwater measures to minimize introducing water upgradient of the disposal fields.

Response e – A drainage swale between the pavement and the disposal fields was the intent of the design, but may not have been detailed appropriately. Additional spot grades have been added to the plan to indicate flow direction and depth of the swale. The riprap apron is intended to extend to the bottom of the swale and not extending toward the wastewater disposal beds.

Comment f - Given the parent soils together with the relatively large roof area, are the designed roof drip edge drainage sufficient to handle the anticipated volume? It is also noteworthy that some of the courtyard area has roof drip edge drainage that adds to the volume of water from the courtyard.

Response f – The design of the roof drip edges includes the use of the foundation drain below the stone section. With the use of the foundation drain, the amount of backfill sand and the crushed stone reservoir, the system has sufficient storage volume to detain the 25-year storm event and larger. To demonstrate the storage volume and its effectiveness during the rain storm event, the available stormwater storage has been incorporated into the stormwater model.

Buffering Comments

- The proposed buffering was discussed during the Planning Board meeting. Based on our understanding, the Planning Board and the abutting property owners agreed that the proposed buffering was adequate for the site.

PEER REVIEW ENGINEER COMMENTS

Comment 1 - Per Maine state plumbing regulations such a stormwater underdrained filtration measure shall be 25 feet from a subsurface wastewater system when the system is lined with a PVC membrane. Based on our measurements the treatment area is 22 feet measured from the proposed chamber corner to the spillway crest elevation. The pond should be shifted slightly to NW with the spillway relocated to the greatest practical extent possible.

Response 1 – The underdrained filter basin grading has been adjusted to provide 26 feet of separation between Wastewater Disposal Bed A and the pond side of the top of berm.

Comment 2 - The spillway orientation spills flooding overflow directly at the end of the proposed Wastewater bed “A” before turning westerly. The distance from the runoff in a flood condition is 11-

12 feet from the center of the floodway channel, and 8 ft from the riprap channel edge. This orientation places the septic system in direct line with approximately 4 inches of flood water 8 feet across at the wastewater system. We would suggest different alignments to avoid direct aim at the wastewater system.

Response 2 – To address the concerns, we have relocated the riprap spillway and swale closer to the north, further away from Wastewater Disposal Bed A.

Comment 3 - The parking lot sheet flows all runoff to the western back edge of the lot, where it appears there should be a swale to convey into the treatment filter swale. However, there is a riprap apron which tends to direct runoff over the wastewater area. There may be room for a thin swale or ditching but we have concerns that during winter months the area will become frozen and blocked by snow piles/ice and excessive melt or runoff will back up or flow into or over the wastewater system.

Response 3 – The drainage swale was the intent of the design, but may not have been detailed appropriately. Additional spot grades have been added to the plan to indicate flow direction and depth of the swale. The riprap apron is intended to extend to the bottom of the swale and not extending toward the wastewater disposal beds.

The grade between the wastewater disposal bed and the drainage swale to the underdrained filter basin has been raised an additional foot with the grade break located more to the west than originally designed in an attempt to avoid significant snow melt from draining over the wastewater disposal beds and directing more runoff to the new swale.

Comment 4 - Snow storage will be a concern for winter periods as it appears the snow piles from the parking areas will end up near the front edge of the wastewater systems and septic treatment unit.

Response 4 – See Response 3 above for revisions to the design. The concern with snow storage over the advanced wastewater treatment unit is warranted and will be conveyed to the owner and facilities manager to avoid piling snow above the unit. This will allow for access in the winter months if necessary.

Comment 5 - The underdrained soil filter shall identify the seasonal groundwater elevation within the pond. Even though the measure is lined with a PVC liner it would be important to know if the site needs to be dewatered to install or the.

Response 5 – Test pits and test borings were performed through the site by SW Cole and, as part of the design of the wastewater disposal system, by Main-Land Development Consultants. The results indicated groundwater around 24" to 30" from the surface. As the underdrained filter basin section will require approximately 42" to 66" of excavation, it is anticipated a portion of the excavation will be within groundwater. A Dewatering Geotextile Filter Bag detail has been included on Detail Sheet D-1 for any necessary site dewatering.

Comment 6 - Environmental Considerations- We have concerns with old septic system location and new expansion. We foresee a few issues in that the plan provided shows the old bed to be removed

and it transects the proposed septic system “a” and the proposed underdrained soil filter. It may be plausible to construct a clay or silt dam barrier between the two measures to avoid and possibility of one or the other cross contaminating. We would suggest a small dam a few feet wide in the trench for the foundation drain along the side of the filter system where the original septic bed was sitting. The applicant shall provide details as to how the old bed is to be removed and what protection will be implemented to prevent any transmission of groundwater underneath the filter bed to or from the new septic system.

Response 6 – Based on discussions with the geologist from Main-Land Development Consultants who performed the engineered wastewater system design, “it is anticipated that within three to four weeks of abandoning the existing leachfields in question, the harmful pathogens present during the online status of the leachfield will die-off due to lack of nutrients. Therefore, after that approximate period of time, that leachfield should basically be reverting back to inert mineral soil materials. This “abandon-in-place” approach is often used to de-commission existing leachfields as it has been found to be safe and saving of the extra work needed to remove it.”

Comment 7 - *The turning radius for vehicles around the rear island is tight and the lane narrows to 16 feet due to the angular parking. A turning template should be shown to see what the largest vehicle is that can manipulate the turn. Teh concern is larger trucks might not be able to maneuver if cars are parked there.*

Response 7 – Included in this submission are turning movement exhibits for the different vehicles that would typically be onsite.

As we have progressed with the design team for the project, there have been adjustments made to the design.

- **Building Foundation Revisions**
 - SW Cole was contracted with to provide geotechnical engineering services for the new building expansion. As part of the investigation, SW Cole performed test borings in the location of the proposed expansion. The results indicate there is relatively shallow bedrock in the majority of the footprint, especially where the original full basement section of the building was designed. To reduce rock excavation, the full basement area was moved from the northern side of the building to the southwestern corner, where the bedrock abruptly drops.

As a result, the exterior stairs to the basement were relocated to the western side of the building. To provide the space for the new staircase, the parking lot pavement had to shift to the west approximately 2 feet.

Adjustments to the utilities were also revised to enter the building at the full basement including the water lateral serving the new expansion’s domestic and sprinkler system and sewer service lateral leaving the building into a new septic tank.

- Fire Department Connection
 - The exterior fire department connection has been identified on the plan at the southwestern corner of the building, closest to the parking lot and in the area of the full basement for any internal maintenance.
- Sewer Layout Revisions
 - The configuration of the wastewater disposal fields and advanced treatment units remains the same as previously submitted, but the design removes the need for the combination septic tank and pump structures by draining the sewer flow by gravity to the pump station behind the dumpster pad. The invert elevations where the new sewer infrastructure connects to the existing sewer system were based on a field investigation into the size and inverts of the sewer laterals leaving the building.
- Lighting Plan
 - It is the intent of the applicant to install security and pedestrian lighting throughout the site, which was not included in the original design plan set. We have been working with a lighting company on the layout and is reflected on the updated design plans. In addition, the lighting representative performed a photometrics plan to ensure adequate lighting levels for the different uses of the site. The photometrics plan has been included as part of the design plan set and the specifications of the proposed light fixtures are included with this document for reference.
- Stormwater Management Design
 - With the revisions to the design indicated through this letter, the original stormwater management design was reviewed to ensure the system still provides the required stormwater treatment and flooding control. The system as currently proposed still meets the requirements and the updated Stormwater Management Report and calculations have been included in this submission for review.

We hope that we have addressed Town staff and the Town's peer review engineer comments and questions about the project and have included the updated design plans and supporting documentation as part of the submission. Upon review of the submittal, please let us know if you or the review engineers have any follow up comments. We look forward to discussing the project with the Planning Board.

Sincerely,

DM ROMA CONSULTING ENGINEERS



Jayson R. Haskell, P.E.
Southern Maine Regional Manager

enc.

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SECTION 1

ADDITIONAL RIGHT, TITLE OR INTEREST DOCUMENTS

Filing Fee:
\$85.00

STATE OF MAINE 2026 ANNUAL REPORT

Filing by April 1, 2026 will allow us to provide better service. The legal filing deadline is still **June 1, 2026**. A \$50 late filing fee will be assessed if the report is late.

Make check payable to:
Secretary of State

FOR LIMITED LIABILITY COMPANIES ON FILE AS OF
DECEMBER 31, 2025

Please file by
April 1, 2026

Pursuant to 31 MRSA §1665.2, the information on the report must be current as of the date signed.

Charter Number:
20182459DC

Jurisdiction:
ME

960 MEADOW ROAD HOLDING, LLC
CRAIG ALAIMO, REGISTERED AGENT
960 MEADOW ROAD
CASCO, ME 04015

Principal Home Office Address:

960 MEADOW ROAD
CASCO, ME 04015 US

1. A brief statement of the character of the business in which the limited liability company is actually engaged in the State of Maine; if none, so indicate: (31 MRSA §1665.1.D)

REAL ESTATE HOLDINGS

2. Name and physical address of at least one member, manager, or authorized person (31 MRSA §1665.1.E)

CRAIG ALAIMO	MANAGER	960 MEADOW ROAD CASCO ME 04015
Type or Print Name	Member, Manager, or Authorized Person	Physical Street Address, City, State, and Zip (PO Box Not Allowed)
Type or Print Name	Member, Manager, or Authorized Person	Physical Street Address, City, State, and Zip (PO Box Not Allowed)
Type or Print Name	Member, Manager, or Authorized Person	Physical Street Address, City, State, and Zip (PO Box Not Allowed)
Type or Print Name	Member, Manager, or Authorized Person	Physical Street Address, City, State, and Zip (PO Box Not Allowed)

Electronically Filed On: 05/29/2026

CRAIG ALAIMO
(AUTHORIZED SIGNATURE)

CRAIG ALAIMO
(TYPE OR PRINT NAME)
(Execution - 31 MRSA §1676.1.B. Penalty for untrue or omitted material facts - 31MRSA §1678)

OWNER
(CAPACITY/TITLE)

For questions related to this completed annual report, please contact 207-624-7752 or e-mail cec.corporations@maine.gov

May 26, 2026

**Re: Country Village Assisted Living Expansion
960 Meadow Road, Casco, Maine
Agent Authorization**

In my capacity as owner of the Country Village Assisted Living facility and sole member of 960 Meadow Road, LLC, record owner of property on Meadow Road in Casco, Maine identified as Lot 19 on the Town of Casco Tax Map 42, I have retained the services of DM Roma Consulting Engineers to act as authorized agent to apply for land use permits associated with the expansion of the existing Country Village Assisted Living facility.

Sincerely,

A handwritten signature in black ink, appearing to read 'Craig Alaimo', with a stylized flourish at the end.

Craig Alaimo
Owner

SECTION 2

STORMWATER MANAGEMENT REPORT



STORMWATER MANAGEMENT REPORT

COUNTRY VILLAGE ASSISTED LIVING FACILITY EXPANSION

CASCO, MAINE

(REVISED SEPTEMBER 2, 2026)

A. Narrative

960 Meadow Road Holdings, LLC, the applicant and landowner, is proposing to expand an existing assisted living facility on Meadow Road in Casco, Maine. The approximately 8.39-acre parcel is better identified as Block 9 Lot 1 on the Town of Casco Assessor's Map 211.

The applicant is proposing a 5,115 square foot expansion to accommodate additional resident beds and amenities. The development will also include improving the existing substandard parking lot for the building expansion. The building expansion will be served by the existing private well, a replacement subsurface wastewater disposal field.

B. Existing Conditions

The project site consists of the existing facility building, paved parking and driveway and landscaping. The property contains approximately 18,550 square feet (0.42 acres) of impervious surface and approximately 58,400 square feet (1.34 acres) of lawn and landscaping for a total existing developed area of approximately 76,950 square feet (1.77 acres). This is a small portion of the overall lot, which is primarily comprised of undeveloped woodland.

The area of the facility expansion is relatively flat (1% to 5%) with steeper areas to the west exceeding 3H:1V slope. The soils on the property were determined utilizing the Medium Intensity Soils Map for Cumberland County, Maine published by the Natural Resource Conservation Service. The soils boundaries with associated Hydrologic Soils Groups (HSG) designations are shown on the watershed maps and a Soils Map has been included as Attachment 1 of this report.

In general, the developed portion of the property drains from east to west. The flow that leaves the property is collected in an unnamed stream that continues northerly and crosses Fountain Hill Road. The stream continues beneath Edes Falls Road, eventually combining with Eastman Brook as Mill Brook. Mill Brook continues southerly and enters Crooked River. The river continues southerly through the Songo River to Sebago Lake. The streams are not identified by the Maine Department of Environmental Protection (MDEP) as Urban Impaired Streams, but Sebago Lake is considered a lake most at risk from new development as identified in Chapter 502.

C. Alterations to Land Cover

Based on the proposed redevelopment of the property, approximately 16,195 square feet (0.37 acre) of the existing impervious areas will remain or be maintained as part of the construction and approximately 2,355 square feet (0.05 acre) will be removed. In addition, there will be approximately 17,495 square feet (0.40 acre) of new impervious surface.

Since the fill slope on the downhill side of the new subsurface wastewater disposal field will be maintained as meadow, there will only be an increase in landscaped/lawn area of 2,040 square feet (0.05 acres) when compared to the existing land development. As a result of the project, the site will contain a total of 33,690 square feet (0.77 acres) of impervious surfaces, approximately 43,340 square feet (0.99 acres) of lawn and landscaping for a total site developed area of approximately 77,030 square feet (1.77 acres).

There project will generate approximately 74,940 square feet of land disturbance.

Since the existing impervious and developed area pre-dated 2005 and the revisions to Chapter 500, this area is considered “existing impervious area” and not included in determining thresholds for MDEP stormwater permitting. Since the project will create approximately 17,495 square feet of impervious surface and approximately 2,040 square feet of increased developed area, the project is not required to obtain an individual Stormwater Permit. A Stormwater Permit by Rule is required due to the proposed land disturbance being over one (1) acre. This permit has been acquired.

The project is subject to the Town of Casco Site Plan review process. Since the property contains a perennial stream, the project is subject to the Performance Standards in Section 215-5.31 Stormwater quality and phosphorous control related to stormwater treatment. In addition, as identified in Section 215-7.4 Submission requirements, the proposed development is required to submit a stormwater drainage plan which includes standards for quantity control of runoff leaving the property. These standards will be addressed in this report.

D. Methodology and Modeling Assumptions

The proposed stormwater management system has been designed utilizing Best Management Practices to maintain existing drainage patterns while providing stormwater quality improvement measures. The goal of the storm drainage system design is to remove potential stormwater pollutants from runoff generated by the development while providing attenuation of the peak rates of runoff leaving the site. The method utilized to predict the surface water runoff rates in this analysis is a computer program entitled HydroCAD, which is based on the same methods that were originally developed by the U.S. Department of Agriculture (USDA), Natural Resources Conservation Service, and utilized in the TR-20 modeling program. Peak rates of runoff are forecasted based upon land use, hydrologic soil conditions, vegetative cover, contributing watershed area, time of concentration, rainfall data, storage volumes of detention basins and the hydraulic capacity of structures. The computer model predicts the amount of runoff as a function of time, with the ability to include the attenuation effect due to dams, lakes, large wetlands, floodplains, and constructed stormwater management basins. The input data for rainfalls with statistical recurrence frequencies of 10 years and 25 years was obtained from Appendix H of the MDEP, Chapter 500 Stormwater Management, last revised in 2015. The National Weather Service

developed four synthetic storm types to simulate rainfall patterns around the country. For analysis in Cumberland County, Maine, the type III rainfall pattern with a 24-hour duration is appropriate.

E. Erosion and Sedimentation Control

The project is required by the Town and MDEP to provide permanent and temporary Erosion Control Best Management Practices. These methods are outlined in detail in the plan set.

F. Section 215-5.31 Stormwater quality and phosphorous control

This section of the Town of Casco zoning ordinance requires that any project site subject to Site Plan review and that contains a perennial stream is required to provide a stormwater plan addressing runoff treatment. To demonstrate adherence to this standard, we are proposing to utilize the standards included in MDEP Chapter 500 Stormwater Management. As the project is classified as a redevelopment project under the General Standards, the site's treatment requirement was determined utilizing the pollutant impact ranking system outlined in Chapter 500, Section 4.C.(2).(d). Table 2.

In the existing condition, the site consists of primarily vehicular driveways and parking utilized for parked vehicles for an extend period of time (over 2 hours). Based on Chapter 500, these land features are classified under Pollutant Rank 4. The pitched rooftop and remaining sidewalks and lawn areas are classified under Pollutant Rank 2. In the developed condition, the expanded parking will remain as Pollutant Rank 4 and expanded areas of Pollutant Rank 2 consisting of the majority of the building expansion and walkways. A portion of the expanded roof is essentially flat, which has a higher Pollutant Rank of 3. There will also be areas set aside for non-lawn landscaped areas in addition to a proposed stormwater treatment device, classified as Pollutant Rank 1.

When the pollutant ranks are applied to the associated areas, the existing pollutant impact ranking was determined to be 1.69 and the post developed condition pollutant impact ranking was determined to be 1.84, resulting in a "Ranked Impact Change Due to Redevelopment" of 0.15. Since the change is greater than 0 but less than 1, based on Chapter 500, Section 4.C.(2).(d). Table 3, 60% of the developed area is to be treated.

To meet the treatment standard, the stormwater infrastructure will incorporate a grassed underdrained filter basin and roof drip edges installed along the perimeter of the new building expansion. As a result of the proposed stormwater devices, treatment will be provided for 78% of the redeveloped portion of the site's developed area. The Stormwater Redevelopment Map is included in the plan set and the redevelopment calculations are included as Attachment 2 of this report.

Included as Attachment 3 of this report are the sizing calculations for the proposed underdrained filter basin. These calculations include:

- Storage Volume and Basin Floor surface area meeting *Chapter 7.1 Grassed Underdrained Soil Filter BMP* sizing criteria included in Volume III. BMP Technical Design Manual prepared by the MDEP.
- Spillway sizing calculations demonstrating one foot of freeboard to the top of berm during the 25-year storm event assuming failure of the other discharge devices.

- Hydrograph tables demonstrating the outlet controls to release the stormwater from the basin between 24 and 48 hours.
- Sizing calculations for the level spreaders located at the outfall of the discharge pipe from the basins meeting the sizing standards identified in *Section G(4) Level Spreaders* in Maine Erosion and Sediment Control Best Management Practices Manual for Designers and Engineers prepared by the MDEP.

The locations and construction detail of the proposed roof drip edges have been included within the construction details sheets and the sizing calculations to meet *Chapter 7.5 Roof Dripline Filters* sizing criteria included in Volume III. BMP Technical Design Manual prepared by the MDEP have been included as Attachment 4 of this report.

G. Flooding Standard

The project is required to meet the Town of Casco’s Site Plan review criteria and standard for Surface water drainage requiring the project to maintain or reduce the peak rate of runoff leaving the property during the 25-year storm frequency. To maintain the existing flow rates, the site design will include a grassed underdrained filter basins integrated into the project’s stormwater infrastructure. To demonstrate compliance with the Flooding Standard, three (3) study points were analyzed.

Study Point 1 (“SP1”) represents where flow generated by the eastern portion of the site, including portions of Meadow Road, that drain westerly, crossing the southern property boundary. Flow from this study point is then conveyed back onto the property.

Study Point 2 (“SP2”) represents the location where site generated flow enters the abutting forested wetlands located approximately 25 feet west of the western property boundary. These wetlands contain a stream that flows south to north.

Study Point 3 (“SP3”) represents the location where site generated flow draining in a northerly direction drains across the northerly property boundary adjacent to the abutter’s parking lot. This study point is tributary to the wetlands analyzed at SP2.

The results of the stormwater model incorporating the stormwater detention best management practices are summarized in Table 1:

Table 1 – Peak Rates of Stormwater Runoff		
Study Point	25-Year (cfs)	
	Pre	Post
SP-1	2.46	2.34
SP-2	16.55	15.94
SP-3	0.51	0.12

As illustrated in Table 1, the proposed project’s design, including the integration of the proposed BMPs, reduces the peak rates of runoff at all study points during the 25-year storm event.

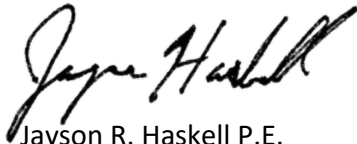
The watershed maps showing pre-development and post-development drainage patterns are included in the plan set and the computations performed with the HydroCAD software program are included as Attachment 5 of this report.

H. Maintenance of common facilities or property

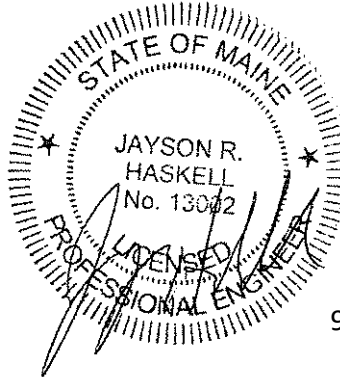
The applicant will be responsible for the maintenance of the stormwater facilities. An Inspection, Maintenance and Housekeeping Plan for the project has been created and has been included as Attachment 6 of this report.

Prepared by:

DM ROMA CONSULTING ENGINEERS



Jayson R. Haskell P.E.
Southern Maine Regional Manager

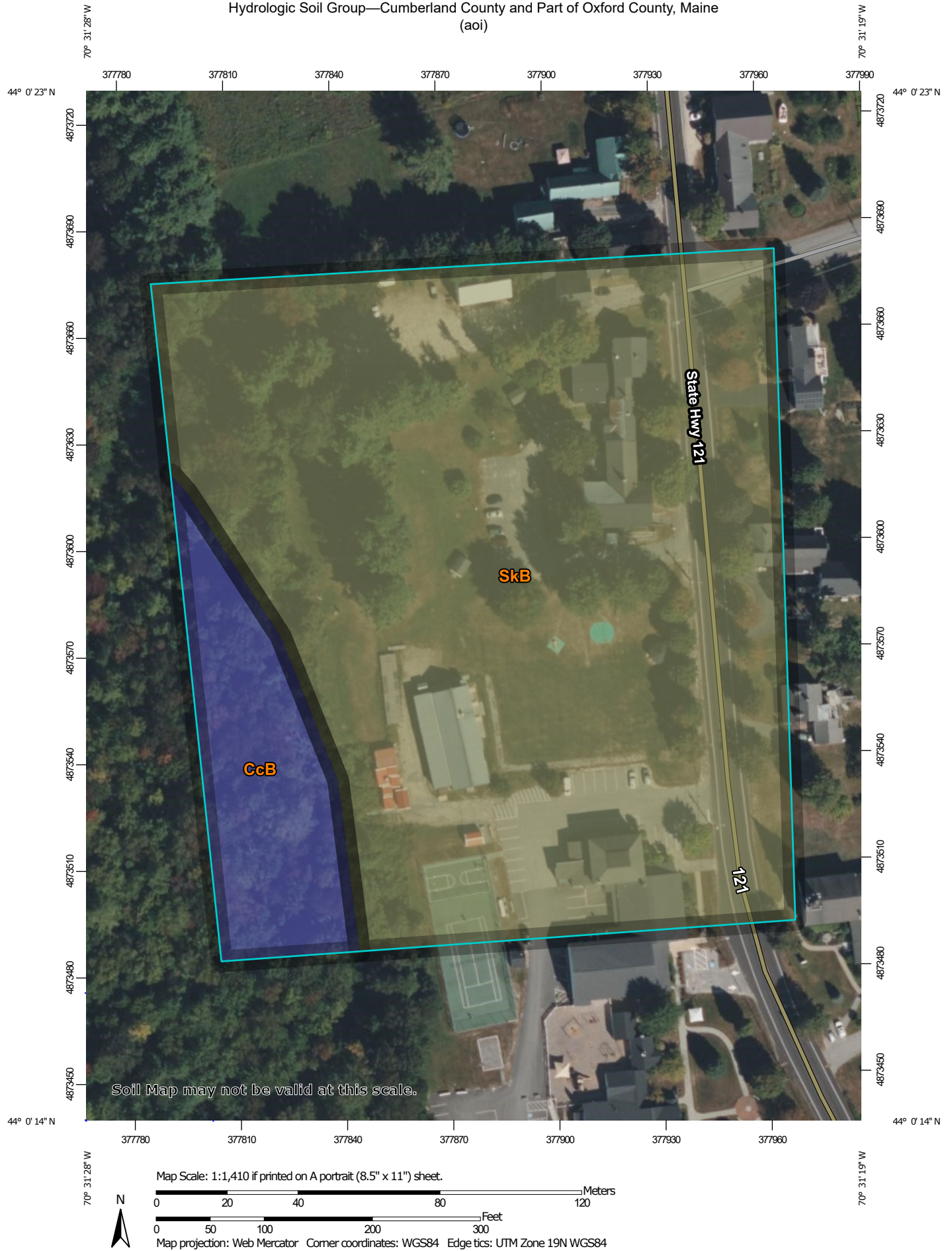


9-2-2026

ATTACHMENT 1

SOILS MAP

Hydrologic Soil Group—Cumberland County and Part of Oxford County, Maine
(aoi)



Hydrologic Soil Group—Cumberland County and Part of Oxford County, Maine
(aoi)

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

Soil Rating Polygons

 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Lines

 A
 A/D
 B
 B/D
 C
 C/D
 D
 Not rated or not available

Soil Rating Points

 A
 A/D
 B
 B/D

 C
 C/D
 D
 Not rated or not available

Water Features

 Streams and Canals

Transportation

 Rails
 Interstate Highways
 US Routes
 Major Roads
 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Cumberland County and Part of Oxford County, Maine
Survey Area Data: Version 22, Aug 29, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 19, 2020—Sep 20, 2020

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.



Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
CcB	Colonel fine sandy loam, 3 to 8 percent slopes	D	1.0	12.0%
SkB	Skerry fine sandy loam, 3 to 8 percent slopes	C/D	7.0	88.0%
Totals for Area of Interest			8.0	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition



ATTACHMENT 2

REDEVELOPMENT STANDARD CALCULATIONS

REDEVELOPMENT STORMWATER TREATMENT CALCULATIONS

Existing Conditions

Land Use	Area (SF)	Area (Ac.)	Pollutant Rank	Existing Impact
High Use Parking Lots and Roads	0	0.00	5	0.00
Other Road and Medium Use Parking Lots	7,360	0.17	4	0.68
Other parking lots & driveways/Flat Roof	0	0.00	3	0.00
Other roof, bikeways, grass, walkways	51,485	1.18	2	2.36
Non-grass landscape/stormwater system	0	0.00	1	0.00
Undeveloped Area	19,560	0.45	0	0.00
Totals	78,405	1.80		3.04

Existing Impact Rating / Redevelopment Area = 1.69

Proposed Conditions

Land Use	Area (SF)		Pollutant Rank	Proposed Impact
High Use Parking Lots and Roads	0	0.00	5	0.00
Other Road and Medium Use Parking Lots	14,310	0.33	4	1.31
Other parking lots & driveways/Flat Roof	850	0.02	3	0.06
Other roof, bikeways, grass, walkways	38,065	0.87	2	1.75
Non-grass landscape/stormwater system	8,085	0.19	1	0.19
Forest/Meadow	17,095	0.39	0	0.00
Totals	78,405	1.80		3.31

Proposed Impact Rating / Redevelopment Area = 1.84

Treatment Requirements

Ranked Impact Change Due to Redevelopment = 0.15

Percentage of Developed Area to be treated (Table 3) = 60%

Total Developed Area = 61,310 sf
(Deduct Forest/Meadow)

Developed Area Treatment

WS-20	845 sf	Roofline Dripedge (Deducted Existing Building from Total WS Area)
WS-21	2,615 sf	Roofline Dripedge
WS-22	2,240 sf	Roofline Dripedge
WS-23	42,260 sf	Underdrained Filter Basin (Deducted Existing Building from Total WS Area)
Total	47,960 sf	

% Redeveloped Area Treated = 78% > Required

ATTACHMENT 3

UNDERDRAINED FILTER BASIN SIZING CALCULATIONS

Underdrained Filter Basin Sizing Calculations

Filter Basin 1

Tributary Impervious Area=	20,170 sf	(WS-23 Impervious Area)
Tributary Landscaped Area=	22,090 sf	(WS-23 Landscaped Area)

Water Quality Volume (WQV) Calculation

WQV (Required) = 1"xImpervious Area + 0.4"xLandscaped Area

WQV (Required) = 2,417 cf

Stage Storage Volume

Elevation	Area (sf)	Storage (cf)
453	1,700	0
454	2,450	2,075
456	4,670	9,195

Outlet Elevation =	454.50
Storage Volume Provided=	3,439 cf > Required

Filter Bottom Calculation

Filter Area (Required) = 5%xImpervious Area + 2%xLandscaped Area

Filter Area Required = 1,450 sf

Filter Area Provided = 1,700 sf > Required

Level Spreader Sizing

Maine Erosion and Sedimentation Control BMPs - Section G4 Sizing Calculations

Level Spreader Length = 0.25 cfs per foot of level spreader during 10-year storm event

Discharge flow during 10-year storm	2.38 cfs	(See HydroCAD output)
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Required Level Spreader Length = 10 ft

Proposed Level Spreader Length = 15 ft

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Spillway Run

Type III 24-hr 25-Year Rainfall=5.80"

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Page 1

Summary for Pond FB:

Inflow Area = 42,260 sf, 47.73% Impervious, Inflow Depth = 4.54" for 25-Year event
 Inflow = 4.30 cfs @ 12.14 hrs, Volume= 15,995 cf
 Outflow = 3.80 cfs @ 12.20 hrs, Volume= 12,556 cf, Atten= 11%, Lag= 3.7 min
 Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
 Secondary = 3.80 cfs @ 12.20 hrs, Volume= 12,556 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Peak Elev= 454.89' @ 12.20 hrs Surf.Area= 3,438 sf Storage= 4,697 cf

Plug-Flow detention time= 136.1 min calculated for 12,543 cf (78% of inflow)
 Center-of-Mass det. time= 58.5 min (851.5 - 793.0)

Volume	Invert	Avail.Storage	Storage Description
#1	453.00'	9,195 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
453.00	1,700	0	0
454.00	2,450	2,075	2,075
456.00	4,670	7,120	9,195

PEAK WS ELEV. = 454.89
 TOP OF BERM = 456.00
 FREEBOARD = 456.00-454.89
 = 1.11' > 1.0' Required

Device	Routing	Invert	Outlet Devices
#1	Primary	450.00'	1.0" Vert. 1" Orifice in Cap X 0.00 C= 0.600
#2	Device 1	450.83'	4.0" Round Culvert L= 45.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 450.83' / 450.00' S= 0.0184 ' S= 0.0184 ' Cc= 0.900 n= 0.013, Flow Area= 0.09 sf
#3	Device 2	453.00'	2.410 in/hr Exfiltration over Surface area
#4	Secondary	454.50'	6.0' long x 11.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.55 2.60 2.70 2.67 2.67 2.67 2.66 2.64

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=453.00' TW=448.50' (Dynamic Tailwater)

1=1" Orifice in Cap (Controls 0.00 cfs)

2=Culvert (Passes 0.00 cfs of 0.40 cfs potential flow)

3=Exfiltration (Passes 0.00 cfs of 0.09 cfs potential flow)

Secondary OutFlow Max=3.80 cfs @ 12.20 hrs HW=454.89' TW=448.83' (Dynamic Tailwater)

4=Broad-Crested Rectangular Weir (Weir Controls 3.80 cfs @ 1.62 fps)

Drain Down Calculation

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Type III 24-hr WQ Storm Rainfall=2.67"

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Hydrograph for Pond FB:

Time (hours)	Inflow (cfs)	Storage (cubic-feet)	Elevation (feet)	Outflow (cfs)	Primary (cfs)	Secondary (cfs)
0.00	0.00	0	453.00	0.00	0.00	0.00
1.00	0.00	0	453.00	0.00	0.00	0.00
2.00	0.00	0	453.00	0.00	0.00	0.00
3.00	0.00	0	453.00	0.00	0.00	0.00
4.00	0.00	0	453.00	0.00	0.00	0.00
5.00	0.00	0	453.00	0.00	0.00	0.00
6.00	0.00	0	453.00	0.00	0.00	0.00
7.00	0.00	0	453.00	0.00	0.00	0.00
8.00	0.00	0	453.00	0.00	0.00	0.00
9.00	0.02	0	453.00	0.02	0.02	0.00
10.00	0.04	0	453.00	0.04	0.04	0.00
11.00	0.08	39	453.02	0.05	0.05	0.00
12.00	0.78	673	453.37	0.05	0.05	0.00
13.00	0.19	2,882	454.31	0.05	0.05	0.00
14.00	0.12	3,201	454.42	0.05	0.05	0.00
15.00	0.09	3,364	454.48	0.06	0.06	0.00
16.00	0.06	3,436	454.50	0.06	0.06	0.00
17.00	0.05	3,434	454.50	0.06	0.06	0.00
18.00	0.04	3,392	454.48	0.06	0.06	0.00
19.00	0.03	3,321	454.46	0.06	0.06	0.00
20.00	0.03	3,238	454.43	0.06	0.06	0.00
21.00	0.03	3,144	454.40	0.05	0.05	0.00
22.00	0.03	3,043	454.36	0.05	0.05	0.00
23.00	0.02	2,933	454.33	0.05	0.05	0.00
24.00	0.02	2,814	454.28	0.05	0.05	0.00
25.00	0.00	2,631	454.22	0.05	0.05	0.00
26.00	0.00	2,439	454.14	0.05	0.05	0.00
27.00	0.00	2,248	454.07	0.05	0.05	0.00
28.00	0.00	2,060	453.99	0.05	0.05	0.00
29.00	0.00	1,872	453.92	0.05	0.05	0.00
30.00	0.00	1,687	453.84	0.05	0.05	0.00
31.00	0.00	1,504	453.76	0.05	0.05	0.00
32.00	0.00	1,323	453.68	0.05	0.05	0.00
33.00	0.00	1,144	453.59	0.05	0.05	0.00
34.00	0.00	966	453.51	0.05	0.05	0.00
35.00	0.00	791	453.43	0.05	0.05	0.00
36.00	0.00	619	453.34	0.05	0.05	0.00
37.00	0.00	448	453.25	0.05	0.05	0.00
38.00	0.00	280	453.16	0.05	0.05	0.00
39.00	0.00	114	453.07	0.05	0.05	0.00
40.00	0.00	0	453.00	0.00	0.00	0.00
41.00	0.00	0	453.00	0.00	0.00	0.00
42.00	0.00	0	453.00	0.00	0.00	0.00
43.00	0.00	0	453.00	0.00	0.00	0.00
44.00	0.00	0	453.00	0.00	0.00	0.00
45.00	0.00	0	453.00	0.00	0.00	0.00
46.00	0.00	0	453.00	0.00	0.00	0.00
47.00	0.00	0	453.00	0.00	0.00	0.00
48.00	0.00	0	453.00	0.00	0.00	0.00

Rain event to fill to
18" (Channel
Protection Volume
elevation)

POND AT CPV AT 16 HRS
EMPTY AT 40 HRS
DRAWDOWN TIME = 40-16 = 24 HRS

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Level Spreader Sizing
Type III 24-hr 10-Year Rainfall=4.60"

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Page 1

Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment20: Runoff Area=1,645 sf 100.00% Impervious Runoff Depth=4.36"
Tc=6.0 min CN=98 Runoff=0.17 cfs 598 cf

Subcatchment21: Runoff Area=2,615 sf 100.00% Impervious Runoff Depth=4.36"
Tc=6.0 min CN=98 Runoff=0.26 cfs 951 cf

Subcatchment22: Runoff Area=2,240 sf 100.00% Impervious Runoff Depth=4.36"
Tc=6.0 min CN=98 Runoff=0.23 cfs 815 cf

Subcatchment23: Runoff Area=42,260 sf 47.73% Impervious Runoff Depth=3.39"
Flow Length=217' Tc=10.0 min CN=89 Runoff=3.25 cfs 11,939 cf

Reach 1R: Foundation Discharge Pipe Avg. Flow Depth=0.29' Max Vel=4.26 fps Inflow=0.51 cfs 2,364 cf
6.0" Round Pipe n=0.013 L=158.0' S=0.0196 '/' Capacity=0.79 cfs Outflow=0.51 cfs 2,364 cf

Reach R10: Riprap Swale Avg. Flow Depth=0.24' Max Vel=3.70 fps Inflow=2.39 cfs 14,305 cf
n=0.069 L=30.0' S=0.2833 '/' Capacity=40.80 cfs **Outflow=2.38 cfs** 14,305 cf

Pond DE: Drip Edge Peak Elev=455.57' Storage=35 cf Inflow=0.23 cfs 815 cf
Primary=0.20 cfs 815 cf Secondary=0.00 cfs 0 cf Outflow=0.20 cfs 815 cf

Pond DE2: Drip Edge Peak Elev=453.38' Storage=137 cf Inflow=0.43 cfs 1,549 cf
Primary=0.31 cfs 1,549 cf Secondary=0.00 cfs 0 cf Outflow=0.31 cfs 1,549 cf

Pond FB: Peak Elev=454.75' Storage=4,234 cf Inflow=3.25 cfs 11,939 cf
Primary=0.06 cfs 6,567 cf Secondary=1.95 cfs 5,374 cf Outflow=2.01 cfs 11,941 cf

Flow tributary to level spreader in 10-year storm event.
Based on sizing calculations provided within the MDEP
Erosion Control and Sedimentation Control Best
Management Practices Manual, for every 0.25 cubic
feet per second of peak runoff generated during the 10-
year storm event, requires 1 foot of level spreader.

Required Spreader Length=2.38 cfs / 0.25 cfs / per foot
Required Spreader Length=9.5 feet
Proposed Spreader Length=15 feet

ATTACHMENT 4

ROOF DRIP EDGE SIZING CALCULATIONS

Drip Edge Sizing Calculations

WQV (Required) = 1.0"xImpervious Area + 0.4"xLandscaped Area

Void Ratio of Reservoir Layer 40%

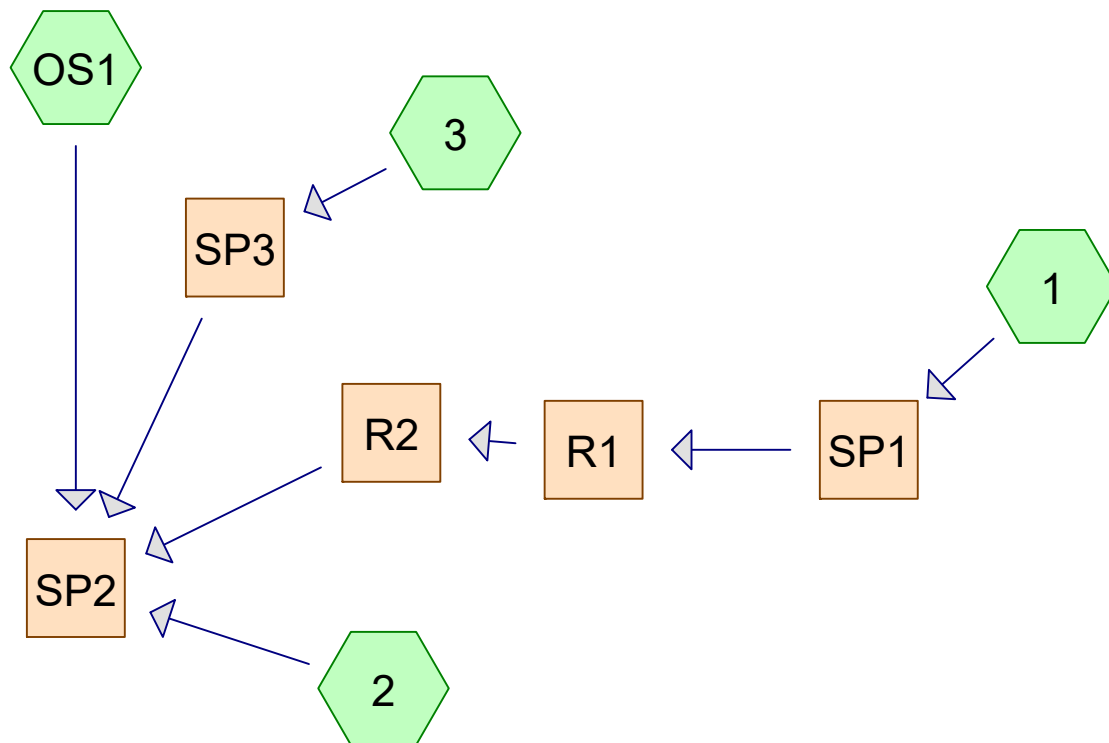
Void Ratio of Filter Layer 30%

SINGLE-FAMILY UNITS:

	Tributary Roof Area (sf)	WQV (Required)	Dripedge Surface Area (sf)	Reservoir Layer Depth (ft)	Filter Layer Depth (ft)	WQV (Provided)
Watershed						
WS-20 & 21	4,260	355.00	482.00	1.75	0.50	409.70
WS-22	2,240	186.67	220.00	1.75	0.50	187.00

ATTACHMENT 5

STORMWATER MODEL OUTPUT



Routing Diagram for 19063-PRE

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19063-PRE*Type III 24-hr 25-Year Rainfall=5.80"*

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment1: Runoff Area=21,330 sf 55.72% Impervious Runoff Depth=4.65"
Flow Length=319' Tc=6.6 min CN=90 Runoff=2.46 cfs 8,268 cf

Subcatchment2: Runoff Area=161,390 sf 18.81% Impervious Runoff Depth=3.91"
Flow Length=450' Tc=14.5 min CN=83 Runoff=12.84 cfs 52,534 cf

Subcatchment3: Runoff Area=6,360 sf 0.00% Impervious Runoff Depth=3.60"
Flow Length=150' Slope=0.0300 '/' Tc=11.7 min CN=80 Runoff=0.51 cfs 1,909 cf

SubcatchmentOS1: Runoff Area=17,140 sf 6.62% Impervious Runoff Depth=3.91"
Flow Length=113' Slope=0.1000 '/' Tc=12.6 min CN=83 Runoff=1.43 cfs 5,579 cf

Reach R1: Avg. Flow Depth=0.06' Max Vel=1.86 fps Inflow=2.46 cfs 8,268 cf
n=0.022 L=112.0' S=0.0402 '/' Capacity=142.93 cfs Outflow=2.44 cfs 8,268 cf

Reach R2: Avg. Flow Depth=0.09' Max Vel=4.75 fps Inflow=2.44 cfs 8,268 cf
n=0.022 L=150.0' S=0.1267 '/' Capacity=167.21 cfs Outflow=2.43 cfs 8,268 cf

Reach SP1: Inflow=2.46 cfs 8,268 cf
Outflow=2.46 cfs 8,268 cf

Reach SP2: Inflow=16.55 cfs 68,290 cf
Outflow=16.55 cfs 68,290 cf

Reach SP3: Inflow=0.51 cfs 1,909 cf
Outflow=0.51 cfs 1,909 cf

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Type III 24-hr 25-Year Rainfall=5.80"

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Page 3

Summary for Subcatchment 1:

Runoff = 2.46 cfs @ 12.10 hrs, Volume= 8,268 cf, Depth= 4.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

	Area (sf)	CN	Description
*	4,430	98	Building
*	7,455	98	Pavement
*	9,445	80	Lawn D
	21,330	90	Weighted Average
	9,445		44.28% Pervious Area
	11,885		55.72% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	49	0.0200	0.15		Sheet Flow, A to B Grass: Short n= 0.150 P2= 3.10"
1.0	270	0.0100	4.48	27.98	Trap/Vee/Rect Channel Flow, B to C Bot.W=0.00' D=0.50' Z= 50.0 & 0.0 ' Top.W=25.00' n= 0.013 Asphalt, smooth
6.6	319	Total			

Summary for Subcatchment 2:

Runoff = 12.84 cfs @ 12.20 hrs, Volume= 52,534 cf, Depth= 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

	Area (sf)	CN	Description
*	9,975	98	Building
*	20,375	98	Pavement
*	5,580	96	Gravel
*	97,305	80	Lawn D
*	18,490	77	Woods D
	9,665	78	Meadow, non-grazed, HSG D
	161,390	83	Weighted Average
	131,040		81.19% Pervious Area
	30,350		18.81% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.7	150	0.0300	0.21		Sheet Flow, A to B Grass: Short n= 0.150 P2= 3.10"
1.8	154	0.0400	1.40		Shallow Concentrated Flow, B to C Short Grass Pasture Kv= 7.0 fps
1.0	146	0.1300	2.52		Shallow Concentrated Flow, C to D Short Grass Pasture Kv= 7.0 fps
14.5	450	Total			

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Type III 24-hr 25-Year Rainfall=5.80"

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Summary for Subcatchment 3:

Runoff = 0.51 cfs @ 12.16 hrs, Volume= 1,909 cf, Depth= 3.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

	Area (sf)	CN	Description
*	5,985	80	Lawn D
*	135	77	Woods D
	240	78	Meadow, non-grazed, HSG D
	6,360	80	Weighted Average
	6,360		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.7	150	0.0300	0.21		Sheet Flow, A to B Grass: Short n= 0.150 P2= 3.10"

Summary for Subcatchment OS1:

Runoff = 1.43 cfs @ 12.17 hrs, Volume= 5,579 cf, Depth= 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

	Area (sf)	CN	Description
*	1,135	98	Pavement
*	3,290	96	Gravel
	6,005	80	>75% Grass cover, Good, HSG D
	1,315	78	Meadow, non-grazed, HSG D
	5,395	77	Woods, Good, HSG D
	17,140	83	Weighted Average
	16,005		93.38% Pervious Area
	1,135		6.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.6	113	0.1000	0.15		Sheet Flow, A to B Woods: Light underbrush n= 0.400 P2= 3.10"

Summary for Reach R1:

Inflow Area = 21,330 sf, 55.72% Impervious, Inflow Depth = 4.65" for 25-Year event
 Inflow = 2.46 cfs @ 12.10 hrs, Volume= 8,268 cf
 Outflow = 2.44 cfs @ 12.11 hrs, Volume= 8,268 cf, Atten= 1%, Lag= 0.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
 Max. Velocity= 1.86 fps, Min. Travel Time= 1.0 min
 Avg. Velocity = 0.52 fps, Avg. Travel Time= 3.6 min

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Type III 24-hr 25-Year Rainfall=5.80"

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Peak Storage= 147 cf @ 12.11 hrs

Average Depth at Peak Storage= 0.06'

Bank-Full Depth= 0.50' Flow Area= 22.5 sf, Capacity= 142.93 cfs

20.00' x 0.50' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 50.0 '/' Top Width= 70.00'

Length= 112.0' Slope= 0.0402 '/'

Inlet Invert= 458.50', Outlet Invert= 454.00'

**Summary for Reach R2:**

Inflow Area = 21,330 sf, 55.72% Impervious, Inflow Depth = 4.65" for 25-Year event

Inflow = 2.44 cfs @ 12.11 hrs, Volume= 8,268 cf

Outflow = 2.43 cfs @ 12.11 hrs, Volume= 8,268 cf, Atten= 0%, Lag= 0.4 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Max. Velocity= 4.75 fps, Min. Travel Time= 0.5 min

Avg. Velocity= 1.36 fps, Avg. Travel Time= 1.8 min

Peak Storage= 77 cf @ 12.11 hrs

Average Depth at Peak Storage= 0.09'

Bank-Full Depth= 1.00' Flow Area= 9.0 sf, Capacity= 167.21 cfs

5.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight

Side Slope Z-value= 4.0 '/' Top Width= 13.00'

Length= 150.0' Slope= 0.1267 '/'

Inlet Invert= 454.00', Outlet Invert= 435.00'

**Summary for Reach SP1:**

Inflow Area = 21,330 sf, 55.72% Impervious, Inflow Depth = 4.65" for 25-Year event

Inflow = 2.46 cfs @ 12.10 hrs, Volume= 8,268 cf

Outflow = 2.46 cfs @ 12.10 hrs, Volume= 8,268 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Summary for Reach SP2:

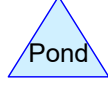
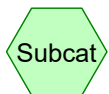
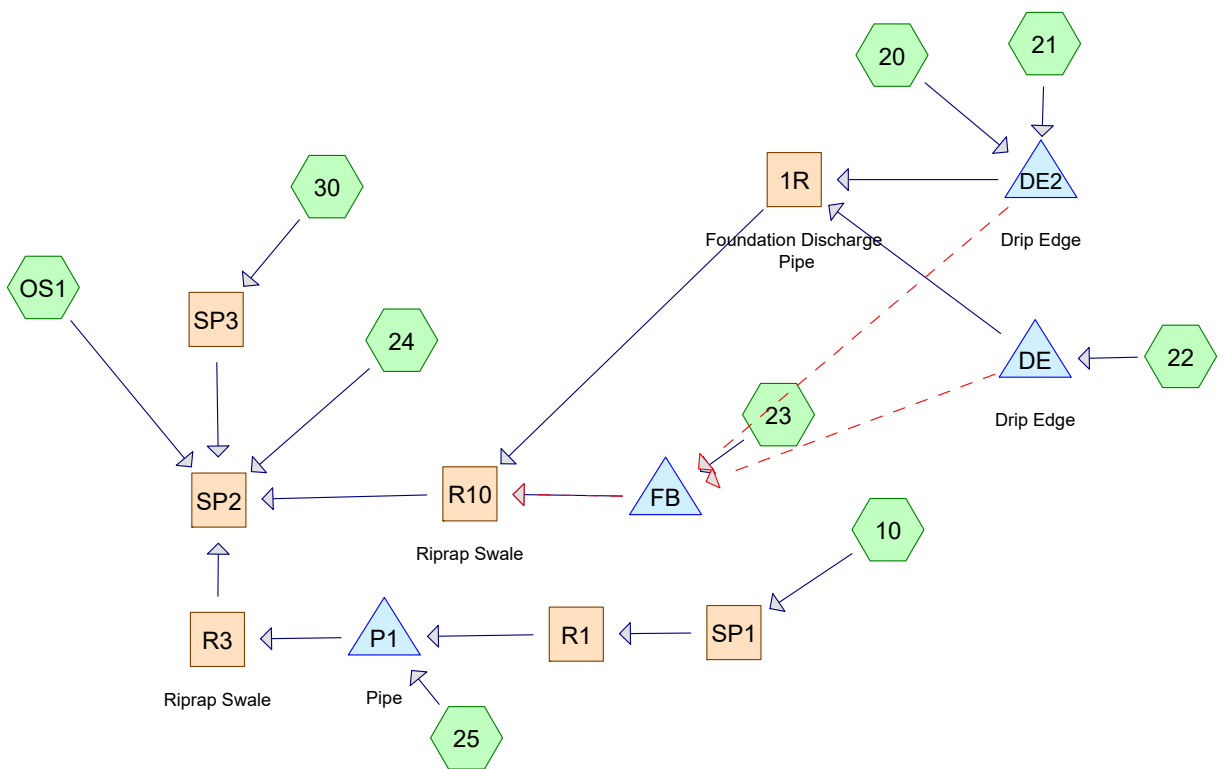
Inflow Area = 206,220 sf, 21.03% Impervious, Inflow Depth = 3.97" for 25-Year event
Inflow = 16.55 cfs @ 12.18 hrs, Volume= 68,290 cf
Outflow = 16.55 cfs @ 12.18 hrs, Volume= 68,290 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Summary for Reach SP3:

Inflow Area = 6,360 sf, 0.00% Impervious, Inflow Depth = 3.60" for 25-Year event
Inflow = 0.51 cfs @ 12.16 hrs, Volume= 1,909 cf
Outflow = 0.51 cfs @ 12.16 hrs, Volume= 1,909 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs



Routing Diagram for 19063-POST

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Type III 24-hr 25-Year Rainfall=5.80"

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Time span=0.00-48.00 hrs, dt=0.05 hrs, 961 points
 Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
 Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment10:	Runoff Area=20,260 sf 53.38% Impervious Runoff Depth=4.65" Flow Length=319' Tc=6.6 min CN=90 Runoff=2.34 cfs 7,853 cf
Subcatchment20:	Runoff Area=1,645 sf 100.00% Impervious Runoff Depth=5.56" Tc=6.0 min CN=98 Runoff=0.21 cfs 762 cf
Subcatchment21:	Runoff Area=2,615 sf 100.00% Impervious Runoff Depth=5.56" Tc=6.0 min CN=98 Runoff=0.33 cfs 1,212 cf
Subcatchment22:	Runoff Area=2,240 sf 100.00% Impervious Runoff Depth=5.56" Tc=6.0 min CN=98 Runoff=0.29 cfs 1,038 cf
Subcatchment23:	Runoff Area=42,260 sf 47.73% Impervious Runoff Depth=4.54" Flow Length=217' Tc=10.0 min CN=89 Runoff=4.30 cfs 15,995 cf
Subcatchment24:	Runoff Area=98,855 sf 19.22% Impervious Runoff Depth=3.91" Flow Length=439' Tc=16.8 min CN=83 Runoff=7.42 cfs 32,178 cf
Subcatchment25:	Runoff Area=19,890 sf 4.58% Impervious Runoff Depth=3.70" Flow Length=318' Slope=0.0300 '/ Tc=14.0 min CN=81 Runoff=1.52 cfs 6,136 cf
Subcatchment30:	Runoff Area=1,315 sf 0.00% Impervious Runoff Depth=3.60" Tc=6.0 min CN=80 Runoff=0.12 cfs 395 cf
SubcatchmentOS1:	Runoff Area=17,140 sf 6.62% Impervious Runoff Depth=3.91" Flow Length=113' Slope=0.1000 '/ Tc=12.6 min CN=83 Runoff=1.43 cfs 5,579 cf
Reach 1R: Foundation Discharge Pipe	Avg. Flow Depth=0.34' Max Vel=4.44 fps Inflow=0.63 cfs 3,013 cf 6.0" Round Pipe n=0.013 L=158.0' S=0.0196 '/ Capacity=0.79 cfs Outflow=0.62 cfs 3,013 cf
Reach R1:	Avg. Flow Depth=0.06' Max Vel=1.83 fps Inflow=2.34 cfs 7,853 cf n=0.022 L=112.0' S=0.0402 '/ Capacity=142.93 cfs Outflow=2.32 cfs 7,853 cf
Reach R10: Riprap Swale	Avg. Flow Depth=0.32' Max Vel=4.40 fps Inflow=4.24 cfs 19,009 cf n=0.069 L=30.0' S=0.2833 '/ Capacity=40.80 cfs Outflow=4.25 cfs 19,009 cf
Reach R3: Riprap Swale	Avg. Flow Depth=0.32' Max Vel=3.02 fps Inflow=2.84 cfs 13,989 cf n=0.069 L=69.0' S=0.1362 '/ Capacity=28.29 cfs Outflow=2.85 cfs 13,989 cf
Reach SP1:	Inflow=2.34 cfs 7,853 cf Outflow=2.34 cfs 7,853 cf
Reach SP2:	Inflow=15.94 cfs 71,150 cf Outflow=15.94 cfs 71,150 cf
Reach SP3:	Inflow=0.12 cfs 395 cf Outflow=0.12 cfs 395 cf

19063-POST*Type III 24-hr 25-Year Rainfall=5.80"*

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Pond DE: Drip Edge

Peak Elev=455.69' Storage=46 cf Inflow=0.29 cfs 1,038 cf
Primary=0.25 cfs 1,038 cf Secondary=0.00 cfs 0 cf Outflow=0.25 cfs 1,038 cf

Pond DE2: Drip Edge

Peak Elev=453.66' Storage=183 cf Inflow=0.54 cfs 1,975 cf
Primary=0.38 cfs 1,974 cf Secondary=0.00 cfs 0 cf Outflow=0.38 cfs 1,974 cf

Pond FB:

Peak Elev=454.88' Storage=4,647 cf Inflow=4.30 cfs 15,995 cf
Primary=0.06 cfs 6,865 cf Secondary=3.59 cfs 9,131 cf Outflow=3.65 cfs 15,996 cf

Pond P1: Pipe

Peak Elev=453.55' Storage=737 cf Inflow=3.57 cfs 13,989 cf
12.0" Round Culvert n=0.013 L=119.0' S=0.0050 '/' Outflow=2.84 cfs 13,989 cf

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Type III 24-hr 25-Year Rainfall=5.80"

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Summary for Subcatchment 10:

Runoff = 2.34 cfs @ 12.10 hrs, Volume= 7,853 cf, Depth= 4.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

	Area (sf)	CN	Description
*	4,430	98	Building
*	6,385	98	Pavement
*	9,445	80	Lawn D
	20,260	90	Weighted Average
	9,445		46.62% Pervious Area
	10,815		53.38% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	49	0.0200	0.15		Sheet Flow, A to B Grass: Short n= 0.150 P2= 3.10"
1.0	270	0.0100	4.48	27.98	Trap/Vee/Rect Channel Flow, B to C Bot.W=0.00' D=0.50' Z= 50.0 & 0.0 ' Top.W=25.00' n= 0.013 Asphalt, smooth
6.6	319	Total			

Summary for Subcatchment 20:

Runoff = 0.21 cfs @ 12.09 hrs, Volume= 762 cf, Depth= 5.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

	Area (sf)	CN	Description
*	845	98	New Building
*	800	98	Existing Building
	1,645	98	Weighted Average
	1,645		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment 21:

Runoff = 0.33 cfs @ 12.09 hrs, Volume= 1,212 cf, Depth= 5.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

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Type III 24-hr 25-Year Rainfall=5.80"

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	Area (sf)	CN	Description
*	2,615	98	Building
	2,615		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment 22:

Runoff = 0.29 cfs @ 12.09 hrs, Volume= 1,038 cf, Depth= 5.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

	Area (sf)	CN	Description
*	2,240	98	Building
	2,240		100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment 23:

Runoff = 4.30 cfs @ 12.14 hrs, Volume= 15,995 cf, Depth= 4.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

	Area (sf)	CN	Description
*	3,910	98	Building
*	16,260	98	Pavement
*	22,090	80	Lawn D
	42,260	89	Weighted Average
	22,090		52.27% Pervious Area
	20,170		47.73% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.8	120	0.0300	0.20		Sheet Flow, A to B Grass: Short n= 0.150 P2= 3.10"
0.2	97	0.0200	6.80	34.00	Trap/Vee/Rect Channel Flow, B to C Bot.W=2.00' D=1.00' Z= 3.0 ' /' Top.W=8.00' n= 0.022 Earth, clean & straight
10.0	217	Total			

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Type III 24-hr 25-Year Rainfall=5.80"

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Summary for Subcatchment 24:

Runoff = 7.42 cfs @ 12.23 hrs, Volume= 32,178 cf, Depth= 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

	Area (sf)	CN	Description
*	13,210	98	Pavement
*	5,785	98	Building
*	5,555	96	Gravel
*	47,575	80	Lawn D
	10,285	77	Woods, Good, HSG D
	16,445	71	Meadow, non-grazed, HSG C
	98,855	83	Weighted Average
	79,860		80.78% Pervious Area
	18,995		19.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.8	150	0.0200	0.18		Sheet Flow, A to B Grass: Short n= 0.150 P2= 3.10"
1.4	101	0.0280	1.17		Shallow Concentrated Flow, B to C Short Grass Pasture Kv= 7.0 fps
0.3	48	0.0250	2.55		Shallow Concentrated Flow, C to D Unpaved Kv= 16.1 fps
0.8	58	0.0300	1.21		Shallow Concentrated Flow, D to E Short Grass Pasture Kv= 7.0 fps
0.3	60	0.2500	3.50		Shallow Concentrated Flow, E to F Short Grass Pasture Kv= 7.0 fps
0.2	22	0.0900	1.50		Shallow Concentrated Flow, F to G Woodland Kv= 5.0 fps
16.8	439	Total			

Summary for Subcatchment 25:

Runoff = 1.52 cfs @ 12.19 hrs, Volume= 6,136 cf, Depth= 3.70"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

	Area (sf)	CN	Description
*	18,980	80	Lawn D
*	910	98	Pavement
	19,890	81	Weighted Average
	18,980		95.42% Pervious Area
	910		4.58% Impervious Area

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Type III 24-hr 25-Year Rainfall=5.80"

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.7	150	0.0300	0.21		Sheet Flow, A to B Grass: Short n= 0.150 P2= 3.10"
2.3	168	0.0300	1.21		Shallow Concentrated Flow, B to C Short Grass Pasture Kv= 7.0 fps
14.0	318	Total			

Summary for Subcatchment 30:

Runoff = 0.12 cfs @ 12.09 hrs, Volume= 395 cf, Depth= 3.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

Area (sf)	CN	Description
* 1,315	80	Grass A
1,315		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, 6 Minute Min. Tc

Summary for Subcatchment OS1:

Runoff = 1.43 cfs @ 12.17 hrs, Volume= 5,579 cf, Depth= 3.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-Year Rainfall=5.80"

Area (sf)	CN	Description
* 1,135	98	Pavement
* 3,290	96	Gravel
6,005	80	>75% Grass cover, Good, HSG D
1,315	78	Meadow, non-grazed, HSG D
5,395	77	Woods, Good, HSG D
17,140	83	Weighted Average
16,005		93.38% Pervious Area
1,135		6.62% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.6	113	0.1000	0.15		Sheet Flow, A to B Woods: Light underbrush n= 0.400 P2= 3.10"

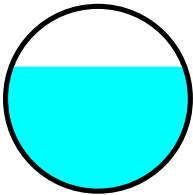
Summary for Reach 1R: Foundation Discharge Pipe

Inflow Area = 6,500 sf, 100.00% Impervious, Inflow Depth = 5.56" for 25-Year event
 Inflow = 0.63 cfs @ 12.15 hrs, Volume= 3,013 cf
 Outflow = 0.62 cfs @ 12.16 hrs, Volume= 3,013 cf, Atten= 0%, Lag= 0.5 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
 Max. Velocity= 4.44 fps, Min. Travel Time= 0.6 min
 Avg. Velocity = 1.47 fps, Avg. Travel Time= 1.8 min

Peak Storage= 22 cf @ 12.16 hrs
 Average Depth at Peak Storage= 0.34'
 Bank-Full Depth= 0.50' Flow Area= 0.2 sf, Capacity= 0.79 cfs

6.0" Round Pipe
 n= 0.013
 Length= 158.0' Slope= 0.0196 '/
 Inlet Invert= 451.60', Outlet Invert= 448.50'

**Summary for Reach R1:**

Inflow Area = 20,260 sf, 53.38% Impervious, Inflow Depth = 4.65" for 25-Year event
 Inflow = 2.34 cfs @ 12.10 hrs, Volume= 7,853 cf
 Outflow = 2.32 cfs @ 12.11 hrs, Volume= 7,853 cf, Atten= 1%, Lag= 0.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
 Max. Velocity= 1.83 fps, Min. Travel Time= 1.0 min
 Avg. Velocity = 0.51 fps, Avg. Travel Time= 3.7 min

Peak Storage= 142 cf @ 12.11 hrs
 Average Depth at Peak Storage= 0.06'
 Bank-Full Depth= 0.50' Flow Area= 22.5 sf, Capacity= 142.93 cfs

20.00' x 0.50' deep channel, n= 0.022 Earth, clean & straight
 Side Slope Z-value= 50.0 '/ Top Width= 70.00'
 Length= 112.0' Slope= 0.0402 '/
 Inlet Invert= 458.50', Outlet Invert= 454.00'



Summary for Reach R10: Riprap Swale

Inflow Area = 48,760 sf, 54.70% Impervious, Inflow Depth = 4.68" for 25-Year event
 Inflow = 4.24 cfs @ 12.21 hrs, Volume= 19,009 cf
 Outflow = 4.25 cfs @ 12.21 hrs, Volume= 19,009 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
 Max. Velocity= 4.40 fps, Min. Travel Time= 0.1 min
 Avg. Velocity = 1.13 fps, Avg. Travel Time= 0.4 min

Peak Storage= 29 cf @ 12.21 hrs
 Average Depth at Peak Storage= 0.32'
 Bank-Full Depth= 1.00' Flow Area= 5.0 sf, Capacity= 40.80 cfs

2.00' x 1.00' deep channel, n= 0.069 Riprap, 6-inch
 Side Slope Z-value= 3.0 '/' Top Width= 8.00'
 Length= 30.0' Slope= 0.2833 '/'
 Inlet Invert= 448.50', Outlet Invert= 440.00'

**Summary for Reach R3: Riprap Swale**

Inflow Area = 40,150 sf, 29.20% Impervious, Inflow Depth = 4.18" for 25-Year event
 Inflow = 2.84 cfs @ 12.23 hrs, Volume= 13,989 cf
 Outflow = 2.85 cfs @ 12.24 hrs, Volume= 13,989 cf, Atten= 0%, Lag= 0.7 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
 Max. Velocity= 3.02 fps, Min. Travel Time= 0.4 min
 Avg. Velocity = 0.97 fps, Avg. Travel Time= 1.2 min

Peak Storage= 65 cf @ 12.24 hrs
 Average Depth at Peak Storage= 0.32'
 Bank-Full Depth= 1.00' Flow Area= 5.0 sf, Capacity= 28.29 cfs

2.00' x 1.00' deep channel, n= 0.069 Riprap, 6-inch
 Side Slope Z-value= 3.0 '/' Top Width= 8.00'
 Length= 69.0' Slope= 0.1362 '/'
 Inlet Invert= 451.40', Outlet Invert= 442.00'



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Summary for Reach SP1:

Inflow Area = 20,260 sf, 53.38% Impervious, Inflow Depth = 4.65" for 25-Year event
 Inflow = 2.34 cfs @ 12.10 hrs, Volume= 7,853 cf
 Outflow = 2.34 cfs @ 12.10 hrs, Volume= 7,853 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Summary for Reach SP2:

Inflow Area = 206,220 sf, 28.38% Impervious, Inflow Depth = 4.14" for 25-Year event
 Inflow = 15.94 cfs @ 12.22 hrs, Volume= 71,150 cf
 Outflow = 15.94 cfs @ 12.22 hrs, Volume= 71,150 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Summary for Reach SP3:

Inflow Area = 1,315 sf, 0.00% Impervious, Inflow Depth = 3.60" for 25-Year event
 Inflow = 0.12 cfs @ 12.09 hrs, Volume= 395 cf
 Outflow = 0.12 cfs @ 12.09 hrs, Volume= 395 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Summary for Pond DE: Drip Edge

Inflow Area = 2,240 sf, 100.00% Impervious, Inflow Depth = 5.56" for 25-Year event
 Inflow = 0.29 cfs @ 12.09 hrs, Volume= 1,038 cf
 Outflow = 0.25 cfs @ 12.13 hrs, Volume= 1,038 cf, Atten= 12%, Lag= 2.7 min
 Primary = 0.25 cfs @ 12.13 hrs, Volume= 1,038 cf
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Peak Elev= 455.69' @ 12.13 hrs Surf.Area= 220 sf Storage= 46 cf

Plug-Flow detention time= 7.0 min calculated for 1,037 cf (100% of inflow)

Center-of-Mass det. time= 7.2 min (752.8 - 745.7)

Volume	Invert	Avail.Storage	Storage Description
#1	455.17'	587 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
455.17	220	0.0	0	0
456.00	220	40.0	73	73
458.25	220	30.0	149	222
460.00	220	40.0	154	376
460.10	4,000	100.0	211	587

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Device	Routing	Invert	Outlet Devices
#1	Primary	455.17'	4.0" Vert. Orifice/Grate C= 0.600
#2	Secondary	456.00'	75.0' long x 2.0' breadth Broad-Crested Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00			
2.50 3.00 3.50			
Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88			
2.85 3.07 3.20 3.32			

Primary OutFlow Max=0.25 cfs @ 12.13 hrs HW=455.68' TW=451.93' (Dynamic Tailwater)↑**1=Orifice/Grate** (Orifice Controls 0.25 cfs @ 2.82 fps)**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=455.17' TW=453.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)**Summary for Pond DE2: Drip Edge**

Inflow Area = 4,260 sf, 100.00% Impervious, Inflow Depth = 5.56" for 25-Year event
 Inflow = 0.54 cfs @ 12.09 hrs, Volume= 1,975 cf
 Outflow = 0.38 cfs @ 12.17 hrs, Volume= 1,974 cf, Atten= 30%, Lag= 5.1 min
 Primary = 0.38 cfs @ 12.17 hrs, Volume= 1,974 cf
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

Peak Elev= 453.66' @ 12.17 hrs Surf.Area= 482 sf Storage= 183 cf

Plug-Flow detention time= 12.2 min calculated for 1,972 cf (100% of inflow)

Center-of-Mass det. time= 12.4 min (758.0 - 745.7)

Volume	Invert	Avail.Storage	Storage Description	
#1	452.67'	1,047 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
Elevation (feet)	Surf.Area (sq-ft)	Voids (%)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
452.67	482	0.0	0	0
453.50	482	40.0	160	160
455.75	482	30.0	325	485
457.50	482	40.0	337	823
457.60	4,000	100.0	224	1,047

Device	Routing	Invert	Outlet Devices
#1	Primary	452.67'	4.0" Vert. Orifice/Grate C= 0.600
#2	Secondary	457.50'	200.0' long x 2.0' breadth Broad-Crested Rectangular Weir
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00			
2.50 3.00 3.50			
Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88			
2.85 3.07 3.20 3.32			

Primary OutFlow Max=0.38 cfs @ 12.17 hrs HW=453.64' TW=451.93' (Dynamic Tailwater)↑**1=Orifice/Grate** (Orifice Controls 0.38 cfs @ 4.31 fps)**Secondary OutFlow** Max=0.00 cfs @ 0.00 hrs HW=452.67' TW=453.00' (Dynamic Tailwater)↑**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

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Type III 24-hr 25-Year Rainfall=5.80"

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Summary for Pond FB:

Inflow Area = 42,260 sf, 47.73% Impervious, Inflow Depth = 4.54" for 25-Year event
 Inflow = 4.30 cfs @ 12.14 hrs, Volume= 15,995 cf
 Outflow = 3.65 cfs @ 12.21 hrs, Volume= 15,996 cf, Atten= 15%, Lag= 4.4 min
 Primary = 0.06 cfs @ 12.21 hrs, Volume= 6,865 cf
 Secondary = 3.59 cfs @ 12.21 hrs, Volume= 9,131 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs
 Peak Elev= 454.88' @ 12.21 hrs Surf.Area= 3,422 sf Storage= 4,647 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 289.9 min (1,083.0 - 793.0)

Volume	Invert	Avail.Storage	Storage Description
#1	453.00'	9,195 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
453.00	1,700	0	0
454.00	2,450	2,075	2,075
456.00	4,670	7,120	9,195

Device	Routing	Invert	Outlet Devices
#1	Primary	450.00'	1.0" Vert. 1" Orifice in Cap C= 0.600
#2	Device 1	450.83'	4.0" Round Culvert L= 45.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 450.83' / 450.00' S= 0.0184 '/' Cc= 0.900 n= 0.013, Flow Area= 0.09 sf
#3	Device 2	453.00'	2.410 in/hr Exfiltration over Surface area
#4	Secondary	454.50'	6.0' long x 11.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.55 2.60 2.70 2.67 2.67 2.67 2.66 2.64

Primary OutFlow Max=0.06 cfs @ 12.21 hrs HW=454.87' TW=448.82' (Dynamic Tailwater)

↑ **1=1" Orifice in Cap** (Orifice Controls 0.06 cfs @ 10.58 fps)

↑ **2=Culvert** (Passes 0.06 cfs of 0.53 cfs potential flow)

↑ **3=Exfiltration** (Passes 0.06 cfs of 0.19 cfs potential flow)

Secondary OutFlow Max=3.53 cfs @ 12.21 hrs HW=454.87' TW=448.82' (Dynamic Tailwater)

↑ **4=Broad-Crested Rectangular Weir** (Weir Controls 3.53 cfs @ 1.58 fps)

Summary for Pond P1: Pipe

Inflow Area = 40,150 sf, 29.20% Impervious, Inflow Depth = 4.18" for 25-Year event
 Inflow = 3.57 cfs @ 12.13 hrs, Volume= 13,989 cf
 Outflow = 2.84 cfs @ 12.23 hrs, Volume= 13,989 cf, Atten= 20%, Lag= 6.2 min
 Primary = 2.84 cfs @ 12.23 hrs, Volume= 13,989 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.05 hrs

19063-POST

Type III 24-hr 25-Year Rainfall=5.80"

Prepared by DM Roma Consulting Engineers

Printed 9/2/2026

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Peak Elev= 453.55' @ 12.23 hrs Surf.Area= 933 sf Storage= 737 cf

Plug-Flow detention time= 3.3 min calculated for 13,989 cf (100% of inflow)

Center-of-Mass det. time= 2.6 min (805.2 - 802.7)

Volume	Invert	Avail.Storage	Storage Description
#1	452.00'	1,220 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
452.00	20	0	0
454.00	1,200	1,220	1,220

Device	Routing	Invert	Outlet Devices
#1	Primary	452.00'	12.0" Round Culvert L= 119.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 452.00' / 451.40' S= 0.0050 ' / ' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=2.83 cfs @ 12.23 hrs HW=453.54' TW=451.72' (Dynamic Tailwater)↑**1=Culvert** (Barrel Controls 2.83 cfs @ 3.61 fps)

ATTACHMENT 6

INSPECTION, MAINTENANCE AND HOUSEKEEPING PLAN



INSPECTION, MAINTENANCE, AND HOUSEKEEPING PLAN

COUNTRY VILLAGE ASSISTED LIVING FACILITY EXPANSION CASCO, MAINE

Responsible Party

Owner: 960 Meadow Road Holdings, LLC
960 Meadow Road
Casco, ME 04015

Prepared by

Engineer: **DM Roma Consulting Engineers**
Designer: Jayson Haskell, PE #13002
DEP Erosion Control Certification #4769

The owner/applicant is responsible for the maintenance of all stormwater management structures and related site components and the keeping of a maintenance log book with service records.

Records of all inspections and maintenance work performed must be kept on file with the owner and retained for a minimum of five years. The maintenance log will be made available to the Town upon request. At a minimum, the maintenance of stormwater management systems will be performed on the prescribed schedule.

The procedures outlined in this plan are provided as a general overview of the anticipated practices to be utilized on this site. In some instances, additional measures may be required due to unexpected conditions. *The Maine Erosion and Sedimentation Control BMP and Stormwater Management for Maine: Best Management Practices* Manuals published by the Maine Department of Environmental Protection (MDEP) should be referenced for additional information.

During Construction

- 1. Inspection and Corrective Action:** It is the contractor's responsibility to comply with the inspection and maintenance procedures outlined in this section. Inspection shall occur on all disturbed and impervious areas, erosion control measures, material storage areas that are exposed to precipitation, and locations where vehicles enter or exit the site. These areas shall be inspected, including winter work, at least once a week as well as 24 hours before and after a storm event generating more than 0.5 inch of rainfall over a 24-hour period and prior to completing permanent stabilization measures. A person with

knowledge of erosion and stormwater control, including the standards and conditions in the permit, shall conduct the inspections.

2. **Maintenance:** Erosion controls shall be maintained in effective operating condition until areas are permanently stabilized. If best management practices (BMPs) need to be repaired, the repair work should be initiated upon discovery of the problem but no later than the end of the next workday. If BMPs need to be maintained or modified, additional BMPs are necessary, or other corrective action is needed, implementation must be completed within seven calendar days and prior to any rainfall event.
3. **Construction vehicles and equipment:** Construction vehicles and equipment shall not be driven or stored within any proposed stormwater treatment ponds.
4. **Documentation:** A report summarizing the inspections and any corrective action taken must be maintained on site. The log must include the name(s) and qualifications of the person making the inspections; the date(s) of the inspections; and the major observations about the operation and maintenance of erosion and sedimentation controls, materials storage areas, and vehicle access points to the parcel. Major observations must include BMPs that need maintenance, BMPs that failed to operate as designed or proved inadequate for a particular location, and location(s) where additional BMPs are needed. For each BMP requiring maintenance, BMP needing replacement, and location needing additional BMPs, note in the log the corrective action taken and when it was taken. The log must be made accessible to Town staff, and a copy must be provided upon request. The owner shall retain a copy of the log for a period of at least three years from the completion of permanent stabilization.

Housekeeping

1. **Spill prevention:** Controls must be used to prevent pollutants from construction and waste materials on site to enter stormwater, which includes storage practices to minimize exposure of the materials to stormwater. The site contractor or operator must develop, and implement as necessary, appropriate spill prevention, containment, and response planning measures.
2. **Groundwater protection:** During construction, liquid petroleum products and other hazardous materials with the potential to contaminate groundwater may not be stored or handled in areas of the site draining to an infiltration area. An "infiltration area" is any area of the site that by design or as a result of soils, topography and other relevant factors accumulates runoff that infiltrates into the soil. Dikes, berms, sumps, and other forms of secondary containment that prevent discharge to groundwater may be used to isolate portions of the site for the purposes of storage and handling of these materials. Any project proposing infiltration of stormwater must provide adequate pre-treatment of stormwater prior to discharge of stormwater to the infiltration area, or provide for

treatment within the infiltration area, in order to prevent the accumulation of fines, reduction in infiltration rate, and consequent flooding and destabilization.

- 3. Fugitive sediment and dust:** Actions must be taken to ensure that activities do not result in noticeable erosion of soils or fugitive dust emissions during or after construction. Oil may not be used for dust control, but other water additives may be considered as needed. A stabilized construction entrance (SCE) should be included to minimize tracking of mud and sediment. If off-site tracking occurs, public roads should be swept immediately and no less than once a week and prior to significant storm events. Operations during dry months, that experience fugitive dust problems, should wet down unpaved access roads once a week or more frequently as needed with a water additive to suppress fugitive sediment and dust.
- 4. Debris and other materials:** Minimize the exposure of construction debris, building and landscaping materials, trash, fertilizers, pesticides, herbicides, detergents, sanitary waste and other materials to precipitation and stormwater runoff. These materials must be prevented from becoming a pollutant source.
- 5. Excavation de-watering:** Excavation de-watering is the removal of water from trenches, foundations, coffer dams, ponds, and other areas within the construction area that retain water after excavation. In most cases the collected water is heavily silted and hinders correct and safe construction practices. The collected water removed from the ponded area, either through gravity or pumping, must be spread through natural wooded buffers or removed to areas that are specifically designed to collect the maximum amount of sediment possible, like a cofferdam sedimentation basin. Avoid allowing the water to flow over disturbed areas of the site. Equivalent measures may be taken if approved by the Department.
- 6. Authorized Non-stormwater discharges:** It is the contractor's responsibility to identify and prevent contamination by non-stormwater discharges. Where allowed non-stormwater discharges exist, they must be identified and steps should be taken to ensure the implementation of appropriate pollution prevention measures for the non-stormwater component(s) of the discharge. Authorized non-stormwater discharges are:
 - (a) Discharges from firefighting activity;
 - (b) Fire hydrant flushings;
 - (c) Vehicle washwater if detergents are not used and washing is limited to the exterior of vehicles (engine, undercarriage and transmission washing is prohibited);
 - (d) Dust control runoff in accordance with permit conditions and Appendix (C)(3);
 - (e) Routine external building washdown, not including surface paint removal, that does not involve detergents;
 - (f) Pavement washwater (where spills/leaks of toxic or hazardous materials have not occurred, unless all spilled material had been removed) if detergents are not used;
 - (g) Uncontaminated air conditioning or compressor condensate;

- (h) Uncontaminated groundwater or spring water;
- (i) Foundation or footer drain-water where flows are not contaminated;
- (j) Uncontaminated excavation dewatering (see requirements in Appendix C(5));
- (k) Potable water sources including waterline flushings; and
- (l) Landscape irrigation.

- 7. Unauthorized non-stormwater discharges:** Approval from the Town does not authorize a discharge that is mixed with a source of non-stormwater, other than those discharges in compliance with Section 6 above. Specifically, the Town's approval does not authorize discharges of the following:
- (a) Wastewater from the washout or cleanout of concrete, stucco, paint, form release oils, curing compounds or other construction materials;
 - (b) Fuels, oils or other pollutants used in vehicle and equipment operation and maintenance;
 - (c) Soaps, solvents, or detergents used in vehicle and equipment washing; and
 - (d) Toxic or hazardous substances from a spill or other release.

Post construction

- 1. Inspection and Corrective Action:** All measures must be maintained by the owner in effective operating condition. An inspector hired by the owner shall at least annually inspect the stormwater management facilities. This person should have knowledge of erosion and stormwater control, including the standards and conditions of the site's approvals. The following areas, facilities, and measures must be inspected, and identified deficiencies must be corrected. Areas, facilities, and measures other than those listed below may also require inspection on a specific site.
- A. Vegetated Areas:** Inspect vegetated areas, particularly slopes and embankments, early in the growing season or after heavy rains to identify active or potential erosion problems. Replant bare areas or areas with sparse growth. Where rill is evident, armor the area with an appropriate lining or divert the erosive flows to on-site areas able to withstand the concentrated flows.
- B. Ditches, Swales, and Open Channels:** Inspect ditches, swales, and other open channels after heavy rains to remove any obstructions to flow, remove accumulated sediments and debris, control vegetative growth that could obstruct flow, and repair any erosion of the ditch lining. Vegetated ditches must be mowed at least annually or otherwise maintained to control the growth of woody vegetation and maintain flow capacity. Grass to be mowed to a minimum height of six inches. Any woody vegetation growing through riprap linings must also be removed. Repair any slumping side slopes as soon as practicable. The channel must receive adequate routine maintenance to maintain capacity and prevent or correct any erosion of the channel's bottom or side slopes.

- C. Culverts:** Inspect culverts in the spring, late fall, and after heavy rains to remove any obstructions to flow; remove accumulated sediments and debris at the riprap inlet, at the riprap outlet, and within the conduit; and to repair any erosion damage at the culvert's inlet and outlet.
- D. Underdrained Filter Basin:** The filter basins are not intended to function as snow storage areas. Inspector to verify that winter plowing operations are not dumping or pushing snow into the basins. The basins shall also not be used for vehicle or heavy equipment storage. Basins should be inspected after several major storm events (0.5 inches rainfall over 24 hours) to determine drawdown time during the first year. The basins to be inspected every six months thereafter with at least one inspection after a major storm event.

The basins should drain dry within 24 to 48 hours following a one-inch storm. If ponding exceeds 48 hours, the top of the filter bed must be rototilled to reestablish the soil's filtration capacity. If water ponds on the surface of the bed for more than 72 hours, the top several inches of the filter shall be replaced with fresh material. Inspect for debris and sediment build up in the forebays and basins and remove as needed. Mowing of the basins can only occur semi-annually to a height of no less than 6 inches utilizing a hand-held string trimmer or push-mower. Any bare areas or erosion rills shall be repaired with new filter media or sandy loam then seeded and mulched. The basins should also be inspected annually for destabilization of side slopes, embankment settling and other signs of structural failure.

- E. Level Spreader:** Level spreader should be inspected semi-annually and following major storm events for the first year and every six months thereafter to remove any obstructions to flow. Stormwater runoff should discharge from the level spreader as sheet flow, and any observed channelization of flows or erosion should be corrected immediately. Any woody vegetation growing through riprap must be removed. Replace riprap on areas where any underlying soil or sediment buildup is showing through the stone or where stones have been dislodged.
- F. Emergency Spillway:** Spillways should be inspected semi-annually and following major storm events for the first year and every six months thereafter to remove any obstructions to flow. Any woody vegetation growing through riprap lining must be removed. Replace riprap on areas where any underlying filter fabric is showing through the stone or where stones have been dislodged.
- G. Roofline Drip edges:** The drip edges should be inspected semi-annually and following major storm events for the first year and every six months thereafter. The reservoir crushed stone should drain within 24 to 48 hours following a major storm event. If ponding exceeds 48 hours, the stone reservoir course shall be removed and the filter bed be rototilled to reestablish the soil's filtration capacity. If water ponds in the reservoir course for more than 72 hours, the top several inches of the filter shall be

replaced with fresh material. Inspect for debris and sediment build up at surface and remove as needed. The drip edges are part of the stormwater management plan and cannot be paved over or altered in anyway.

- H. Regular Maintenance:** Clear accumulations of winter sand along roadway once a year, preferably in the spring. Accumulations on pavement may be removed by pavement sweeping. Accumulations of sand along pavement shoulders may be removed by grading excess sand to the pavement edge and removing it manually or by a front-end loader.
- I. Documentation:** Keep a log (report) summarizing inspections, maintenance, and any corrective actions taken. The log must include the date on which each inspection or maintenance task was performed, a description of the inspection findings or maintenance completed, and the name of the inspector or maintenance personnel performing the task. If a maintenance task requires the clean-out of any sediments or debris, indicate where the sediment and debris was disposed after removal. The log must be made accessible to Town staff upon request. The permittee shall retain a copy of the log for a period of at least five years from the completion of permanent stabilization. Attached is a sample log.

Duration of Maintenance

The “Responsible Party” shall perform maintenance as described.

MAINTENANCE LOG

COUNTRY VILLAGE ASSISTED LIVING FACILITY EXPANSION CASCO, MAINE

(GENERAL INSPECTION FORM PAGE 1 OF 2)

The following stormwater management and erosion control items shall be inspected and maintained as prescribed in the Maintenance Plan with recommended frequencies as identified below. The owner is responsible for keeping this maintenance log on file for a minimum of five years and shall provide a copy to the Town upon request. Inspections are to be performed by a qualified third-party inspector and all corrective actions shall be performed by personnel familiar with stormwater management systems and erosion controls.

Maintenance Item	Maintenance Event	Date Performed	Responsible Personnel	Comments
Vegetated Areas	Inspect slopes and embankments early in Spring.			
Drainage Swales	Inspect after major rainfall event			
	Inspect for erosion or slumping & repair			
	Mowed at least annually.			
	Inspect and remove obstructions as necessary.			
	Remove woody vegetation.			
	Replace riprap as necessary.			
Culverts	Inspect semiannually and after major rainfall.			
	Repair erosion at inlet or outlet of pipe.			
	Repair displaced riprap within inlet and outlet aprons.			
	Clean accumulated sediment in culverts when >20% full.			

MAINTENANCE LOG

COUNTRY VILLAGE ASSISTED LIVING FACILITY EXPANSION CASCO, MAINE

(GENERAL INSPECTION FORM PAGE 2 OF 2)

The following stormwater management and erosion control items shall be inspected and maintained as prescribed in the Maintenance Plan with recommended frequencies as identified below. The owner is responsible for keeping this maintenance log on file for a minimum of five years and shall provide a copy to the Town upon request. Inspections are to be performed by a qualified third-party inspector and all corrective actions shall be performed by personnel familiar with stormwater management systems and erosion controls.

Maintenance Item	Maintenance Event	Date Performed	Responsible Personnel	Comments
Roofline Dripedges	Check after each rainfall event to ensure that the stone reservoir drains within 24-48 hours.			
	Replace top several inches of filter if reservoir does not drain within 72 hours.			
	Inspect and remove sediment or debris build up on the surface of the stone			
	Inspect semi-annually for erosion or sediment accumulation and repair as necessary.			
Regular Maintenance	Clear accumulation of winter sand in paved areas annually.			

MAINTENANCE LOG

COUNTRY VILLAGE ASSISTED LIVING FACILITY EXPANSION CASCO, MAINE

(UNDERDRAINED FILTER BASIN)

Maintenance Item	Maintenance Event	Date Performed	Responsible Personnel	Comments
Underdrained Filter Basin	Check after each rainfall event to ensure that pond drains within 24-48 hours.			
	Replace top several inches of filter if pond does not drain within 72 hours.			
	Mow grass no more than twice a year to no less than 6 inches in height.			
	Inspect semi-annually for erosion or sediment accumulation and repair as necessary.			
	Inspector to verify basin not utilized for snow storage			
	Inspector to verify basin not utilized for vehicle or heavy equipment storage.			
Emergency Spillway	Inspect and remove obstructions as necessary.			
	Remove woody vegetation.			
	Replace riprap as necessary.			
Level Lip Spreaders	Inspect for sand accumulation and remove when pool volume is reduced by 25%.			
	Remove leaf litter, branches and tree growth from spreader.			
	Inspect to verify spreader is not utilized for snow storage.			

SECTION 3

TEST PITS AND BORING LOGS



BORING LOG

BORING NO.: **B-101**
SHEET: 1 of 1
PROJECT NO. 26-0162
DATE START: 3/13/2026
DATE FINISH: 3/13/2026

CLIENT: Woodhull
PROJECT: Proposed Building Addition
LOCATION: 960 Meadow Road, Casco, Maine

Drilling Information

LOCATION: See Exploration Location Plan ELEVATION (FT): 458' +/- TOTAL DEPTH (FT): 5.0 LOGGED BY: Kyle Kaserman
DRILLING CO.: Seaboard Drilling, LLC DRILLER: Jeremy Adams DRILLING METHOD: Hollow Stem Auger
RIG TYPE: Track Mounted Diedrich D-50 AUGER ID/OD: 2 1/4 in / 5 5/8 in SAMPLER: Standard Split-Spoon
HAMMER TYPE: Automatic HAMMER WEIGHT (lbs): 140 CASING ID/OD: N/A / N/A CORE BARREL: N/A
HAMMER CORRECTION FACTOR: HAMMER DROP (inch): 30
WATER LEVEL DEPTHS (ft): 2 ft Soils Saturated Below 2' +/-

GENERAL NOTES:

KEY TO NOTES AND SYMBOLS: Water Level
▽ At time of Drilling
▽ At Completion of Drilling
▽ After Drilling
D = Split Spoon Sample
U = Thin Walled Tube Sample
R = Rock Core Sample
V = Field Vane Shear
Pen. = Penetration Length
Rec. = Recovery Length
bpf = Blows per Foot
mpf = Minute per Foot
WOR = Weight of Rods
WOH = Weight of Hammer
RQD = Rock Quality Designation
PID = Photoionization Detector
S_v = Field Vane Shear Strength, kips/sq.ft.
q_u = Unconfined Compressive Strength, kips/sq.ft.
Ø = Friction Angle (Estimated)
N/A = Not Applicable

Elev. (ft)	Depth (ft)	Casing Pen. (bpf)	SAMPLE INFORMATION					Graphic Log	Sample Description & Classification	H ₂ O Depth	Remarks
			Sample No.	Type	Depth (ft)	Pen./ Rec. (in)	Blow Count or RQD				
455			1D		0-2	24/3	1-2-1-4	0.2	Vegetation / Topsoil		
									Medium dense, brown, silty SAND, trace gravel (Till)		
			2D		2-4	24/24	13-14-14-20	3.0	Medium dense, gray-brown, silty gravelly SAND (Till)	▽	
								4.7	Probable Weathered Bedrock		
5								///	Refusal at 5.0 feet Probable Bedrock		

Stratification lines represent approximate boundary between soil types, transitions may be gradual. Water level readings have been made at times and under conditions stated. Fluctuations of groundwater may occur due to other factors than those present at the time measurements were made.

BORING NO.: **B-101**



BORING LOG

BORING NO.: **B-102**
SHEET: 1 of 1
PROJECT NO. 26-0162
DATE START: 3/13/2026
DATE FINISH: 3/13/2026

CLIENT: Woodhull
PROJECT: Proposed Building Addition
LOCATION: 960 Meadow Road, Casco, Maine

Drilling Information

LOCATION: See Exploration Location Plan ELEVATION (FT): 459' +/- TOTAL DEPTH (FT): 2.7 LOGGED BY: Kyle Kaserman
DRILLING CO.: Seaboard Drilling, LLC DRILLER: Jeremy Adams DRILLING METHOD: Hollow Stem Auger
RIG TYPE: Track Mounted Diedrich D-50 AUGER ID/OD: 2 1/4 in / 5 5/8 in SAMPLER: Standard Split-Spoon
HAMMER TYPE: Automatic HAMMER WEIGHT (lbs): 140 CASING ID/OD: N/A / N/A CORE BARREL: N/A
HAMMER CORRECTION FACTOR: HAMMER DROP (inch): 30
WATER LEVEL DEPTHS (ft): 0 ft Soils Wet to Saturated From Surface

GENERAL NOTES:

KEY TO NOTES AND SYMBOLS: Water Level
▽ At time of Drilling D = Split Spoon Sample Pen. = Penetration Length WOR = Weight of Rods S_v = Field Vane Shear Strength, kips/sq.ft.
▽ At Completion of Drilling U = Thin Walled Tube Sample Rec. = Recovery Length WOH = Weight of Hammer q_u = Unconfined Compressive Strength, kips/sq.ft.
▽ After Drilling R = Rock Core Sample bpf = Blows per Foot RQD = Rock Quality Designation Ø = Friction Angle (Estimated)
V = Field Vane Shear mpf = Minute per Foot PID = Photoionization Detector N/A = Not Applicable

Elev. (ft)	Depth (ft)	Casing Pen. (bpf)	SAMPLE INFORMATION					Graphic Log	Sample Description & Classification	H ₂ O Depth	Remarks
			Sample No.	Type	Depth (ft)	Pen./ Rec. (in)	Blow Count or RQD				
			1D		0-2	24/12	1-1-1-5		0.3 Vegetation / Topsoil Very dense, gray, silty gravelly SAND (Till)		
			2D		2-2.4	5/4	50/5"		2.4 Probable Weathered Bedrock or Boulder		

Refusal at 2.7 feet
Probable Bedrock

Stratification lines represent approximate boundary between soil types, transitions may be gradual. Water level readings have been made at times and under conditions stated. Fluctuations of groundwater may occur due to other factors than those present at the time measurements were made.

BORING NO.: **B-102**



BORING LOG

BORING NO.: **B-103**
SHEET: 1 of 1
PROJECT NO. 26-0162
DATE START: 3/13/2026
DATE FINISH: 3/13/2026

CLIENT: Woodhull
PROJECT: Proposed Building Addition
LOCATION: 960 Meadow Road, Casco, Maine

Drilling Information

LOCATION: See Exploration Location Plan ELEVATION (FT): 457' +/- TOTAL DEPTH (FT): 2.6 LOGGED BY: Kyle Kaserman
DRILLING CO.: Seaboard Drilling, LLC DRILLER: Jeremy Adams DRILLING METHOD: Hollow Stem Auger
RIG TYPE: Track Mounted Diedrich D-50 AUGER ID/OD: 2 1/4 in / 5 5/8 in SAMPLER: Standard Split-Spoon
HAMMER TYPE: Automatic HAMMER WEIGHT (lbs): 140 CASING ID/OD: N/A / N/A CORE BARREL: N/A
HAMMER CORRECTION FACTOR: HAMMER DROP (inch): 30
WATER LEVEL DEPTHS (ft): Soils Wet From Surface

GENERAL NOTES:

KEY TO NOTES AND SYMBOLS: Water Level
▽ At time of Drilling D = Split Spoon Sample Pen. = Penetration Length WOR = Weight of Rods S_v = Field Vane Shear Strength, kips/sq.ft.
▽ At Completion of Drilling U = Thin Walled Tube Sample Rec. = Recovery Length WOH = Weight of Hammer q_u = Unconfined Compressive Strength, kips/sq.ft.
▽ After Drilling R = Rock Core Sample bpf = Blows per Foot RQD = Rock Quality Designation Ø = Friction Angle (Estimated)
V = Field Vane Shear mpf = Minute per Foot PID = Photoionization Detector N/A = Not Applicable

Elev. (ft)	Depth (ft)	Casing Pen. (bpf)	SAMPLE INFORMATION					Graphic Log	Sample Description & Classification	H ₂ O Depth	Remarks
			Sample No.	Type	Depth (ft)	Pen./ Rec. (in)	Blow Count or RQD				
455			1D		0-2	24/20	1-1-1-8		Topsoil / Dark brown silty SAND, with organics (FILL)		
			2D		2-2.1	1/4	50/1"		1.4 Medium dense, gray-brown, silty gravelly SAND (Till)		
									2.1 Probable Weathered Bedrock		
Refusal at 2.6 feet Probable Bedrock											

Stratification lines represent approximate boundary between soil types, transitions may be gradual. Water level readings have been made at times and under conditions stated. Fluctuations of groundwater may occur due to other factors than those present at the time measurements were made.

BORING NO.: **B-103**



BORING LOG

BORING NO.: **B-104**
SHEET: 1 of 1
PROJECT NO. 26-0162
DATE START: 3/13/2026
DATE FINISH: 3/13/2026

CLIENT: Woodhull
PROJECT: Proposed Building Addition
LOCATION: 960 Meadow Road, Casco, Maine

Drilling Information

LOCATION: See Exploration Location Plan ELEVATION (FT): 459' +/- TOTAL DEPTH (FT): 4.5 LOGGED BY: Kyle Kaserman
DRILLING CO.: Seaboard Drilling, LLC DRILLER: Jeremy Adams DRILLING METHOD: Hollow Stem Auger
RIG TYPE: Track Mounted Diedrich D-50 AUGER ID/OD: 2 1/4 in / 5 5/8 in SAMPLER: Standard Split-Spoon
HAMMER TYPE: Automatic HAMMER WEIGHT (lbs): 140 CASING ID/OD: N/A / N/A CORE BARREL: N/A
HAMMER CORRECTION FACTOR: HAMMER DROP (inch): 30
WATER LEVEL DEPTHS (ft): 0 ft Soils Wet From Surface

GENERAL NOTES:

KEY TO NOTES AND SYMBOLS: Water Level
▽ At time of Drilling D = Split Spoon Sample Pen. = Penetration Length WOR = Weight of Rods S_v = Field Vane Shear Strength, kips/sq.ft.
▽ At Completion of Drilling U = Thin Walled Tube Sample Rec. = Recovery Length WOH = Weight of Hammer q_u = Unconfined Compressive Strength, kips/sq.ft.
▽ After Drilling R = Rock Core Sample bpf = Blows per Foot RQD = Rock Quality Designation Ø = Friction Angle (Estimated)
V = Field Vane Shear mpf = Minute per Foot PID = Photoionization Detector N/A = Not Applicable

Elev. (ft)	Depth (ft)	Casing Pen. (bpf)	SAMPLE INFORMATION					Graphic Log	Sample Description & Classification	H ₂ O Depth	Remarks
			Sample No.	Type	Depth (ft)	Pen./ Rec. (in)	Blow Count or RQD				
455			1D		0-2	24/20	1-1-2-1		Vegetation / Topsoil		
									0.8 Loose, light brown, silty gravelly SAND (Till)		
			2D		2-3.7	20/24	6-13- 18- 50/2"		2.0 Medium dense to dense, gray-brown, silty gravelly SAND (Till)		

Refusal at 4.5 feet
Probable Bedrock

Stratification lines represent approximate boundary between soil types, transitions may be gradual. Water level readings have been made at times and under conditions stated. Fluctuations of groundwater may occur due to other factors than those present at the time measurements were made.

BORING NO.: **B-104**



BORING LOG

BORING NO.: **B-105**

SHEET: 1 of 1

PROJECT NO. 26-0162

DATE START: 3/13/2026

DATE FINISH: 3/13/2026

CLIENT: Woodhull

PROJECT: Proposed Building Addition

LOCATION: 960 Meadow Road, Casco, Maine

Drilling Information

LOCATION: See Exploration Location Plan ELEVATION (FT): 457' +/- TOTAL DEPTH (FT): 15.2 LOGGED BY: Kyle Kaserman
DRILLING CO.: Seaboard Drilling, LLC DRILLER: Jeremy Adams DRILLING METHOD: Hollow Stem Auger
RIG TYPE: Track Mounted Diedrich D-50 AUGER ID/OD: 2 1/4 in / 5 5/8 in SAMPLER: Standard Split-Spoon
HAMMER TYPE: Automatic HAMMER WEIGHT (lbs): 140 CASING ID/OD: N/A / N/A CORE BARREL: N/A
HAMMER CORRECTION FACTOR: HAMMER DROP (inch): 30
WATER LEVEL DEPTHS (ft): ∇ 5 ft Soils Saturated Below 5' +/-

GENERAL NOTES:

KEY TO NOTES AND SYMBOLS: Water Level
 ∇ At time of Drilling
 ∇ At Completion of Drilling
 ∇ After Drilling
D = Split Spoon Sample
U = Thin Walled Tube Sample
R = Rock Core Sample
V = Field Vane Shear
Pen. = Penetration Length
Rec. = Recovery Length
bpf = Blows per Foot
mpf = Minute per Foot
WOR = Weight of Rods
WOH = Weight of Hammer
RQD = Rock Quality Designation
PID = Photoionization Detector
S_v = Field Vane Shear Strength, kips/sq.ft.
q_u = Unconfined Compressive Strength, kips/sq.ft.
Ø = Friction Angle (Estimated)
N/A = Not Applicable

Elev. (ft)	Depth (ft)	Casing Pen. (bpf)	SAMPLE INFORMATION					Graphic Log	Sample Description & Classification	H ₂ O Depth	Remarks
			Sample No.	Type	Depth (ft)	Pen./ Rec. (in)	Blow Count or RQD				
			1D		0-2	24/3	2-2-1-2		Topsoil / Dark brown silty SAND, with organics (FILL)		
455			2D		2-4	24/18	8-17-20-20		1.5 Medium dense to dense, gray-brown, silty gravelly SAND (Till)		
	5		3D		5-7	24/22	11-31-43-44			∇	
450									6.5 Very dense, gray, silty gravelly SAND, with occasional cobbles (Till)		
	10		4D		10-12	24/16	18-26-38-47				
445											
	15		5D		15-15.2	2/2	50/2"		Refusal at 15.2 feet Probable Bedrock		

Stratification lines represent approximate boundary between soil types, transitions may be gradual. Water level readings have been made at times and under conditions stated. Fluctuations of groundwater may occur due to other factors than those present at the time measurements were made.

BORING NO.: **B-105**



BORING LOG

BORING NO.: **B-106**
SHEET: 1 of 1
PROJECT NO. 26-0162
DATE START: 3/13/2026
DATE FINISH: 3/13/2026

CLIENT: Woodhull
PROJECT: Proposed Building Addition
LOCATION: 960 Meadow Road, Casco, Maine

Drilling Information

LOCATION: See Exploration Location Plan ELEVATION (FT): 458' +/- TOTAL DEPTH (FT): 3.2 LOGGED BY: Kyle Kaserman
DRILLING CO.: Seaboard Drilling, LLC DRILLER: Jeremy Adams DRILLING METHOD: Hollow Stem Auger
RIG TYPE: Track Mounted Diedrich D-50 AUGER ID/OD: 2 1/4 in / 5 5/8 in SAMPLER: Standard Split-Spoon
HAMMER TYPE: Automatic HAMMER WEIGHT (lbs): 140 CASING ID/OD: N/A / N/A CORE BARREL: N/A
HAMMER CORRECTION FACTOR: HAMMER DROP (inch): 30
WATER LEVEL DEPTHS (ft): ∇ 2 ft Soils Saturated below 2' +/-

GENERAL NOTES:

KEY TO NOTES AND SYMBOLS: ∇ Water Level
 ∇ At time of Drilling
 ∇ At Completion of Drilling
 ∇ After Drilling
D = Split Spoon Sample
U = Thin Walled Tube Sample
R = Rock Core Sample
V = Field Vane Shear
Pen. = Penetration Length
Rec. = Recovery Length
bpf = Blows per Foot
mpf = Minute per Foot
WOR = Weight of Rods
WOH = Weight of Hammer
RQD = Rock Quality Designation
PID = Photoionization Detector
S_v = Field Vane Shear Strength, kips/sq.ft.
q_u = Unconfined Compressive Strength, kips/sq.ft.
Ø = Friction Angle (Estimated)
N/A = Not Applicable

Elev. (ft)	Depth (ft)	Casing Pen. (bpf)	SAMPLE INFORMATION					Graphic Log	Sample Description & Classification	H ₂ O Depth	Remarks
			Sample No.	Type	Depth (ft)	Pen./ Rec. (in)	Blow Count or RQD				
455			1D		0-2	24/10	1-1-2-3		Vegetation / Topsoil		
									0.7 Loose, light brown, silty SAND, some gravel (Till)		
			2D		2-2.9	11/11	18-50/5"		2.0 Dense, gray, silty gravelly SAND (Till)	∇	
									2.9 Probable Weathered Bedrock		
Refusal at 3.2 feet Probable Bedrock											

Stratification lines represent approximate boundary between soil types, transitions may be gradual. Water level readings have been made at times and under conditions stated. Fluctuations of groundwater may occur due to other factors than those present at the time measurements were made.

BORING NO.: **B-106**



BORING LOG

BORING NO.: **B-107**
SHEET: 1 of 1
PROJECT NO. 26-0162
DATE START: 3/13/2026
DATE FINISH: 3/13/2026

CLIENT: Woodhull
PROJECT: Proposed Building Addition
LOCATION: 960 Meadow Road, Casco, Maine

Drilling Information

LOCATION: See Exploration Location Plan ELEVATION (FT): 457.5' +/- TOTAL DEPTH (FT): 3.2 LOGGED BY: Kyle Kaserman
DRILLING CO.: Seaboard Drilling, LLC DRILLER: Jeremy Adams DRILLING METHOD: Hollow Stem Auger
RIG TYPE: Track Mounted Diedrich D-50 AUGER ID/OD: 2 1/4 in / 5 5/8 in SAMPLER: Standard Split-Spoon
HAMMER TYPE: Automatic HAMMER WEIGHT (lbs): 140 CASING ID/OD: N/A / N/A CORE BARREL: N/A
HAMMER CORRECTION FACTOR: HAMMER DROP (inch): 30
WATER LEVEL DEPTHS (ft): ∇ 2.5 ft Freewater Observed at 2.5' +/-

GENERAL NOTES:

KEY TO NOTES AND SYMBOLS: ∇ Water Level
 ∇ At time of Drilling
 ∇ At Completion of Drilling
 ∇ After Drilling
D = Split Spoon Sample
U = Thin Walled Tube Sample
R = Rock Core Sample
V = Field Vane Shear
Pen. = Penetration Length
Rec. = Recovery Length
bpf = Blows per Foot
mpf = Minute per Foot
WOR = Weight of Rods
WOH = Weight of Hammer
RQD = Rock Quality Designation
PID = Photoionization Detector
S_v = Field Vane Shear Strength, kips/sq.ft.
q_u = Unconfined Compressive Strength, kips/sq.ft.
Ø = Friction Angle (Estimated)
N/A = Not Applicable

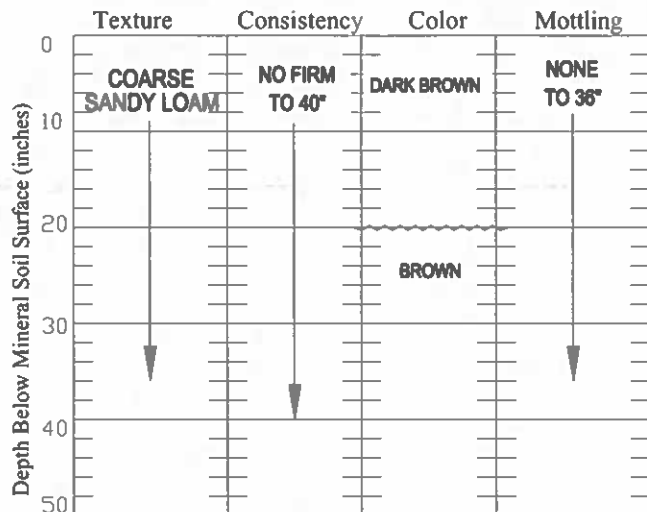
Elev. (ft)	Depth (ft)	Casing Pen. (bpf)	SAMPLE INFORMATION					Graphic Log	Sample Description & Classification	H ₂ O Depth	Remarks
			Sample No.	Type	Depth (ft)	Pen./ Rec. (in)	Blow Count or RQD				
455			1D		0-2	24/20	2-2-3-16		Topsoil / Dark brown silty SAND, with organics (FILL)		
									1.0 Medium dense, gray-brown, silty gravelly SAND (Till)		
			2D		2-2.2	2/2	50/2"		2.2 Probable Weathered Bedrock	∇	

Refusal at 3.2 feet
Probable Bedrock

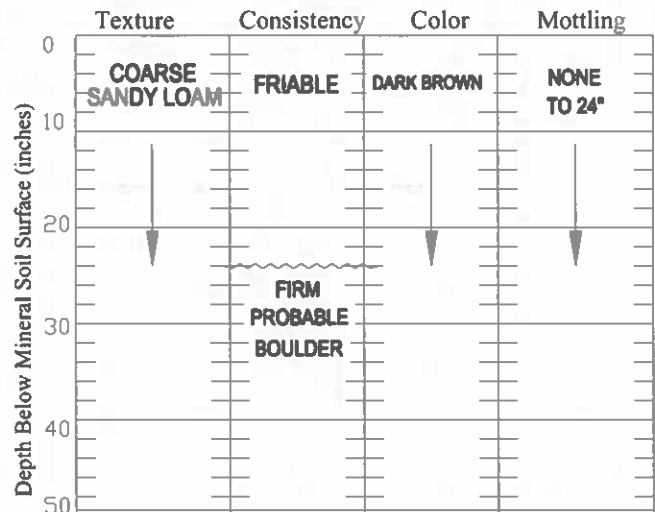
Stratification lines represent approximate boundary between soil types, transitions may be gradual. Water level readings have been made at times and under conditions stated. Fluctuations of groundwater may occur due to other factors than those present at the time measurements were made.

BORING NO.: **B-107**

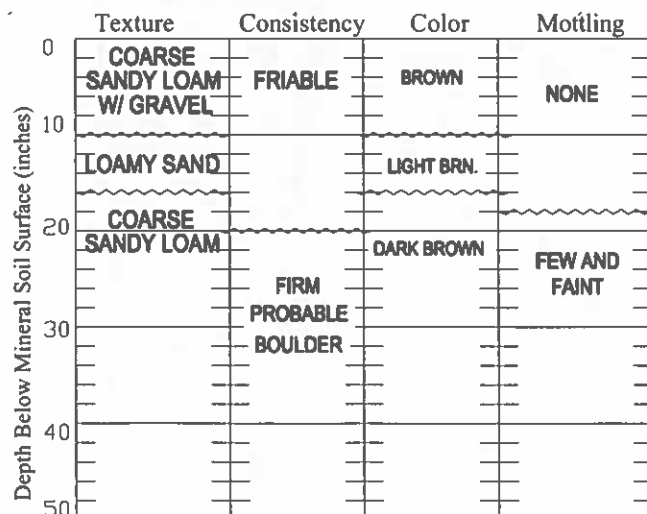
SOIL PROFILE/CLASSIFICATION INFORMATION

DETAILED DESCRIPTION OF
SUBSURFACE CONDITIONS AT PROJECT SITESProject Name:
COUNTRY VILLAGE ASSISTED LIVINGApplicant Name:
COUNTRY VILLAGE ASSISTED LIVINGProject Location (municipality):
960 MEADOW RD. CASCO, ME.Exploration Symbol # TB-1 Test Pit ☒ Boring Probe
4 " Organic horizon thickness Ground surface elev. _____
40 " Depth of exploration or to refusal

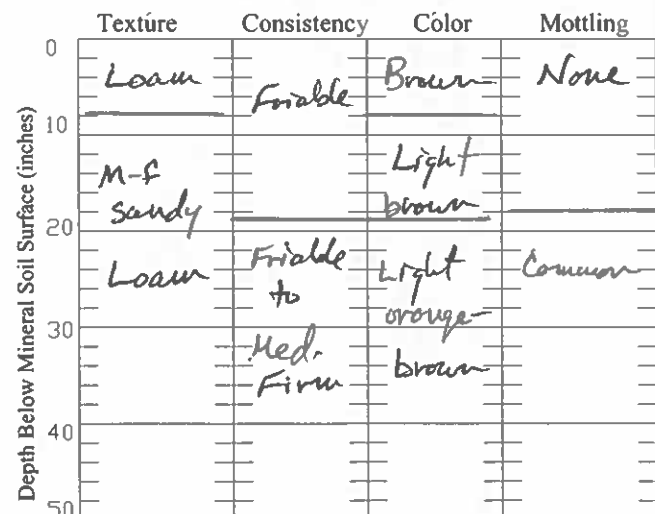
Soil Details by S.E. S.S.	Soil Classification	Slope	Limiting Factor	Groundwater
	3 C	Percent	>36 " Depth	Restrictive Layer
	Soil Series/Phase Name:		Hydric Non-hydric	Hydrologic Soil Group

Exploration Symbol # TB-2 Test Pit ☒ Boring Probe
4 " Organic horizon thickness Ground surface elev. _____
24 " Depth of exploration or to refusal

Soil Details by S.E. S.S.	Soil Classification	Slope	Limiting Factor	Groundwater
	3 C	Percent	24 " Depth	Restrictive Layer
	Soil Series/Phase Name:		Hydric Non-hydric	Hydrologic Soil Group

Exploration Symbol # TB-3 Test Pit ☒ Boring Probe
1 " Organic horizon thickness Ground surface elev. _____
20 " Depth of exploration or to refusal

Soil Details by S.E. S.S.	Soil Classification	Slope	Limiting Factor	Groundwater
	3 C	Percent	18 " Depth	Restrictive Layer
	Soil Series/Phase Name:		Hydric Non-hydric	Hydrologic Soil Group

Exploration Symbol # TB-4 Test Pit ☒ Boring Probe
3 " Organic horizon thickness Ground surface elev. _____
40 " Depth of exploration or to refusal

Soil Details by S.E. S.S.	Soil Classification	Slope	Limiting Factor	Groundwater
	3 C	Percent	14 " 18 " Depth	Restrictive Layer
	Soil Series/Phase Name:		Hydric Non-hydric	Hydrologic Soil Group

INVESTIGATOR INFORMATION AND SIGNATURE

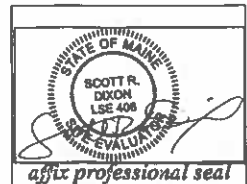
Signature Scott R. Dixon

Name Printed SCOTT R. DIXON

Title ☒ Licensed Site Evaluator ☒ Certified Soil Scientist ☒ Certified Geologist ☒ Professional Engineer

Date 3/18/28

Cert/Lic/Reg. # SE #406



SOIL PROFILE/CLASSIFICATION INFORMATION		DETAILED DESCRIPTION OF SUBSURFACE CONDITIONS AT PROJECT SITES
Project Name: COUNTRY VILLAGE ASSISTED LIVING	Applicant Name: COUNTRY VILLAGE ASSISTED LIVING	Project Location (municipality): 960 MEADOW RD. CASCO, ME.

Exploration Symbol # TB-C1 Test Pit ☒ Boring Probe
1 " Organic horizon thickness Ground surface elev. _____
40 " Depth of exploration or to refusal

Texture	Consistency	Color	Mottling
0 FINE SANDY LOAM	NO FIRM TO 40"	BROWN	NONE TO 40"
10 LOAMY FINE SAND		GRAY BROWN	
20			
30			
40			
50			

Depth Below Mineral Soil Surface (inches)

Soil Details by	S.E.	Soil Classification		Slope	Limiting Factor	Groundwater	☒ Restrictive Layer ☐ Bedrock
	▶▶	4	C		>40	"	
		Profile	Condition	Percent	Depth	"	
	Soil Series/Phase Name:						
S.S.							Hydrologic
▶▶							Soil Group

Exploration Symbol # TB-C2 Test Pit ☒ Boring Probe
2 " Organic horizon thickness Ground surface elev. _____
36 " Depth of exploration or to refusal

Texture	Consistency	Color	Mottling
0 FINE SANDY LOAM	FRIABLE	BROWN	NONE TO 36"
10 LOAMY FINE SAND		GRAY BROWN	
20			
30			
40	FIRM IN GRAVEL		
50			

Depth Below Mineral Soil Surface (inches)

Soil Details by	S.E.	Soil Classification		Slope	Limiting Factor	Groundwater	☒ Restrictive Layer ☐ Bedrock
	▶▶	3	C		36	"	
		Profile	Condition	Percent	Depth	"	
	Soil Series/Phase Name:						
S.S.							Hydrologic
▶▶							Soil Group

Exploration Symbol # TB-C3 Test Pit ☒ Boring Probe
1 " Organic horizon thickness Ground surface elev. _____
30 " Depth of exploration or to refusal

Texture	Consistency	Color	Mottling
0 FINE SANDY LOAM	FRIABLE	DARK BROWN	NONE TO 30"
10			
20 LOAMY FINE SAND		GRAY BROWN	
30			
40	FIRM IN GRAVEL		
50			

Depth Below Mineral Soil Surface (inches)

Soil Details by	S.E.	Soil Classification		Slope	Limiting Factor	Groundwater	☒ Restrictive Layer ☐ Bedrock
	▶▶	3	C		30	"	
		Profile	Condition	Percent	Depth	"	
	Soil Series/Phase Name:						
S.S.							Hydrologic
▶▶							Soil Group

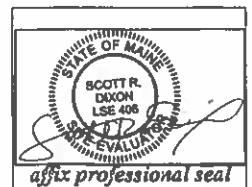
Exploration Symbol # TB-D1 Test Pit ☒ Boring Probe
1 " Organic horizon thickness Ground surface elev. _____
30 " Depth of exploration or to refusal

Texture	Consistency	Color	Mottling
0 COARSE SANDY LOAM	FRIABLE TO 30"	DARK BROWN	NONE TO 30"
10			
20 LOAMY FINE SAND		GRAY BROWN	
30			
40			
50			

Depth Below Mineral Soil Surface (inches)

Soil Details by	S.E.	Soil Classification		Slope	Limiting Factor	Groundwater	☒ Restrictive Layer ☐ Bedrock
	▶▶	3	C		>30	"	
		Profile	Condition	Percent	Depth	"	
	Soil Series/Phase Name:						
S.S.							Hydrologic
▶▶							Soil Group

INVESTIGATOR INFORMATION AND SIGNATURE			
Signature			
Name Printed			
SCOTT R. DIXON		Date	3/10/28
		Cert/Lic/Reg. #	SE #406
Title	☒ Licensed Site Evaluator ☒ Certified Soil Scientist ☒ Certified Geologist ☒ Professional Engineer		



SOIL PROFILE/CLASSIFICATION INFORMATION		DETAILED DESCRIPTION OF SUBSURFACE CONDITIONS AT PROJECT SITES
Project Name: COUNTRY VILLAGE ASSISTED LIVING	Applicant Name: COUNTRY VILLAGE ASSISTED LIVING	Project Location (municipality): 960 MEADOW RD. CASCO, ME.

Exploration Symbol # TB-D2 Test Pit ☒ Boring Probe
 1 " Organic horizon thickness Ground surface elev. _____
 24 " Depth of exploration or to refusal

Texture	Consistency	Color	Mottling
SANDY LOAM	FRIABLE	DARK BROWN	NONE
LOAMY FINE SAND		GRAY BROWN	
	FIRM		FEW AND FAINT

Depth Below Mineral Soil Surface (inches)

Soil Details by	S.E.	Soil Classification	Slope	Limiting Factor	Groundwater	Restrictive Layer	Bedrock
	3	C		20			
		Profile Condition	Percent	Depth			
S.S.		Soil Series/Phase Name:			Hydric	Hydrologic	
		-			Non-hydric	-	
						Soil Group	

Exploration Symbol # TB-D3 Test Pit ☒ Boring Probe
 1 " Organic horizon thickness Ground surface elev. _____
 30 " Depth of exploration or to refusal

Texture	Consistency	Color	Mottling
SANDY LOAM		DARK BROWN	NONE
	FRIABLE		
SANDY LOAM		GRAY BROWN	
		GRAY	FEW AND FAINT
	FIRM		

Depth Below Mineral Soil Surface (inches)

Soil Details by	S.E.	Soil Classification	Slope	Limiting Factor	Groundwater	Restrictive Layer	Bedrock
	3	C		18			
		Profile Condition	Percent	Depth			
S.S.		Soil Series/Phase Name:			Hydric	Hydrologic	
		-			Non-hydric	-	
						Soil Group	

Exploration Symbol # TB-PS1 Test Pit ☒ Boring Probe
 1 " Organic horizon thickness Ground surface elev. _____
 30 " Depth of exploration or to refusal

Texture	Consistency	Color	Mottling
COARSE SANDY LOAM	FRIABLE	DARK BROWN	NONE
		GRAY BROWN	
	FIRM		FEW AND FAINT

Depth Below Mineral Soil Surface (inches)

Soil Details by	S.E.	Soil Classification	Slope	Limiting Factor	Groundwater	Restrictive Layer	Bedrock
	3	C		24			
		Profile Condition	Percent	Depth			
S.S.		Soil Series/Phase Name:			Hydric	Hydrologic	
		-			Non-hydric	-	
						Soil Group	

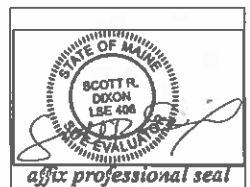
Exploration Symbol # TP-1 Test Pit ☒ Boring Probe
 3 " Organic horizon thickness Ground surface elev. _____
 32 " Depth of exploration or to refusal

Texture	Consistency	Color	Mottling
Loam (FILL)	Friable	Brown	None
Crushed stone (FILL)	Coarse	Brown gray	
Sandy	Friable	Brown	
Loam (native 3)			to bottom of pit

Depth Below Mineral Soil Surface (inches)

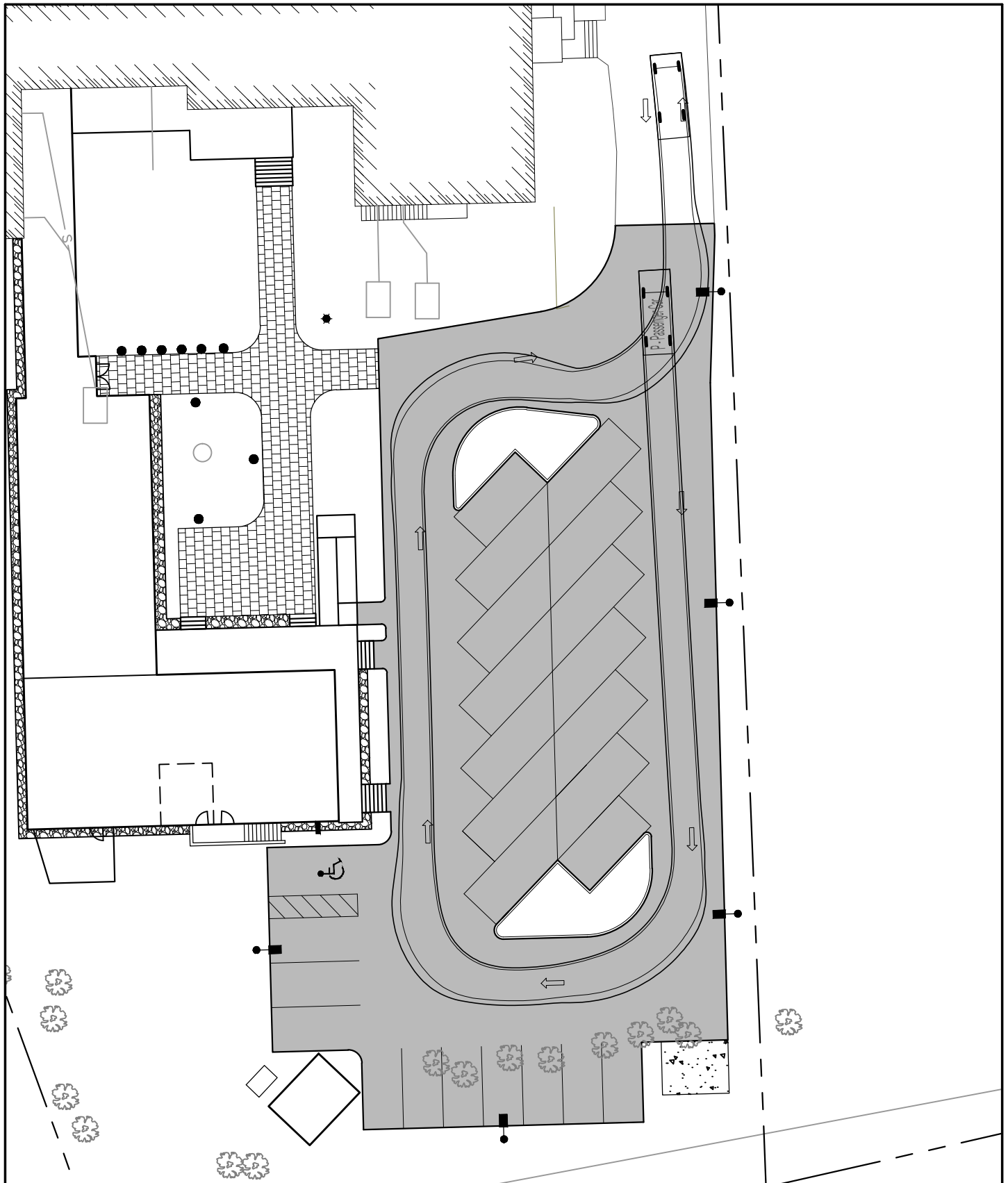
Soil Details by	S.E.	Soil Classification	Slope	Limiting Factor	Groundwater	Restrictive Layer	Bedrock
	3	C	5-7	-			
		Profile Condition	Percent	Depth			
S.S.		Soil Series/Phase Name:			Hydric	Hydrologic	
		-			Non-hydric	-	
						Soil Group	

INVESTIGATOR INFORMATION AND SIGNATURE			
Signature	Date <u>3/16/26</u>		
Name Printed <u>SCOTT R. DIXON</u>	Cert/Lic/Reg. # <u>SE #406</u>		
Title	☒ Licensed Site Evaluator	☒ Certified Soil Scientist	☒ Certified Geologist
	☒ Professional Engineer		



SECTION 4

TURNING MOVEMENT EXHIBITS



TURNING MOVEMENT EXHIBIT - PASSENGER CAR

COUNTRY VILLAGE ASSISTED LIVING FACILITY EXPANSION
CASCO, MAINE

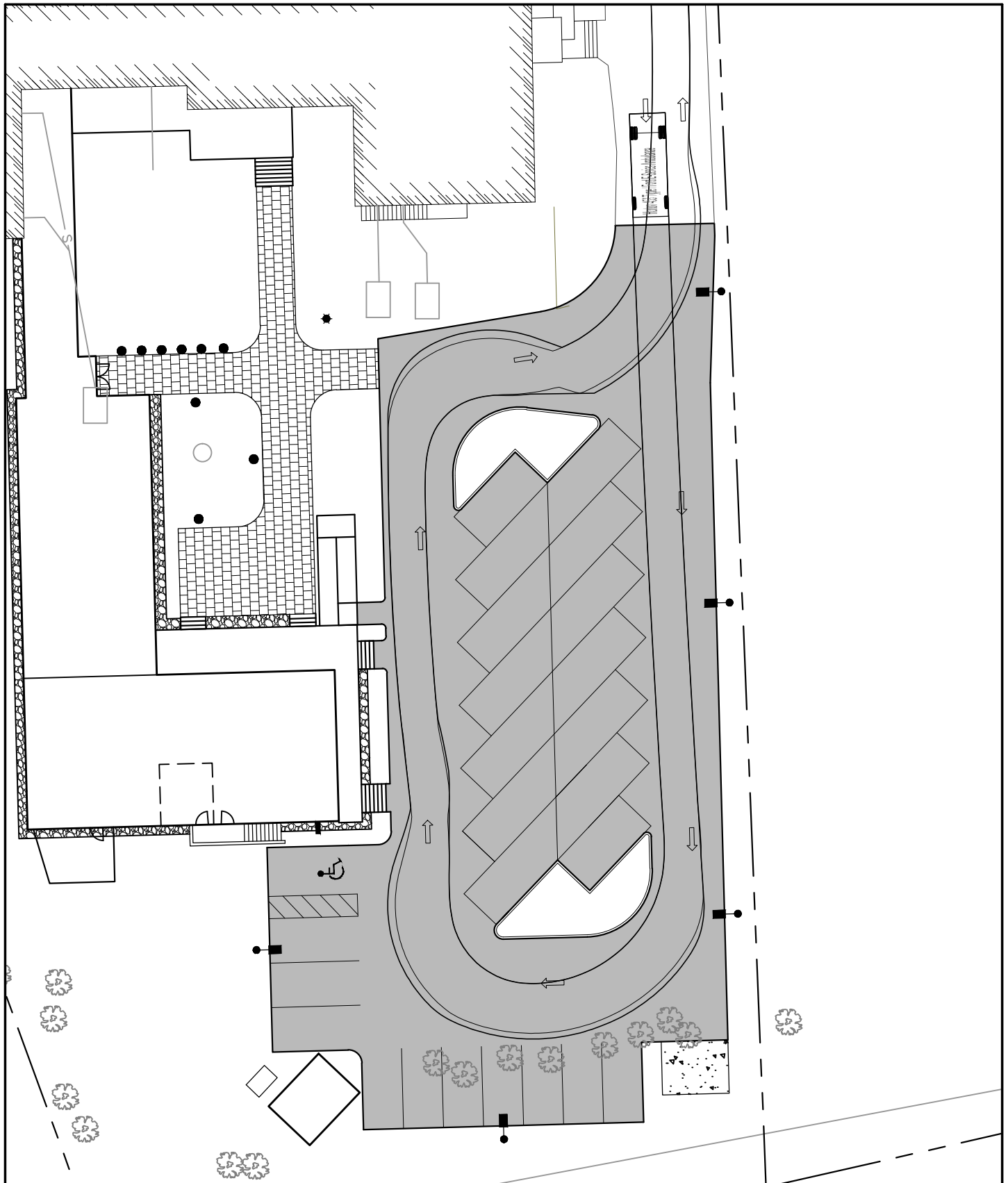
FOR RECORD OWNER:
960 MEADOW ROAD HOLDINGS, LLC
960 MEADOW ROAD
CASCO, MAINE 04015

*TURNING MOVEMENT
BASED ON AASHTO
PASSENGER CAR TEMPLATE

SCALE: 1"=30'
DATE: 9-1-2026
JOB NUMBER: 19063

DM ROMA
CONSULTING ENGINEERS

P.O. BOX 1116
WINDHAM, ME 04062
(207) 591-5055



TURNING MOVEMENT EXHIBIT - AMBULANCE

COUNTRY VILLAGE ASSISTED LIVING FACILITY EXPANSION
CASCO, MAINE

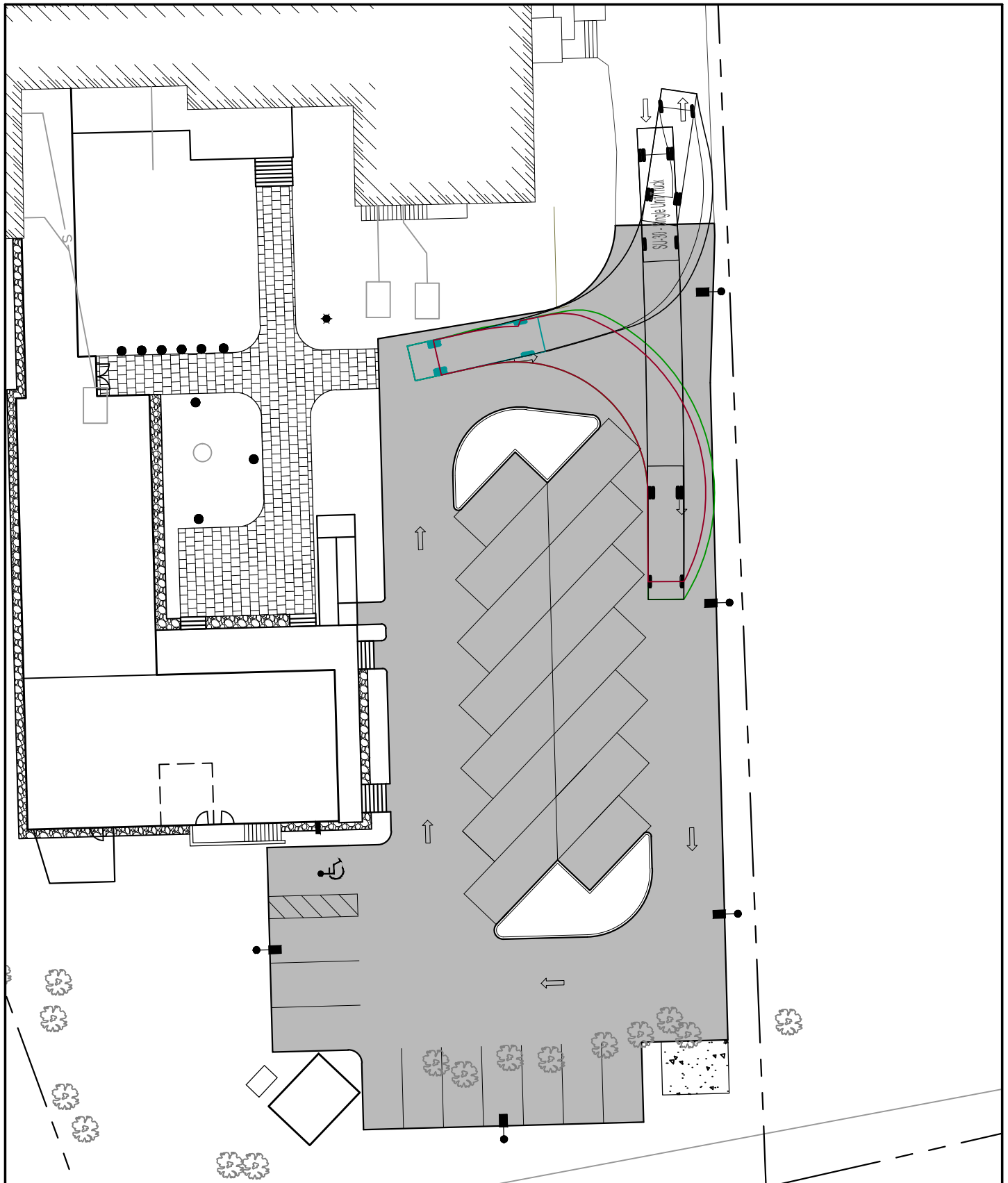
FOR RECORD OWNER:
960 MEADOW ROAD HOLDINGS, LLC
960 MEADOW ROAD
CASCO, MAINE 04015

*TURNING MOVEMENT
BASED ON A HORTON TYPE I
AMBULANCE

SCALE: 1"=30'
DATE: 9-1-2026
JOB NUMBER: 19063

DM ROMA
CONSULTING ENGINEERS

P.O. BOX 1116
WINDHAM, ME 04062
(207) 591-5055



TURNING MOVEMENT EXHIBIT - LARGE DELIVERY TRUCK

COUNTRY VILLAGE ASSISTED LIVING FACILITY EXPANSION
CASCO, MAINE

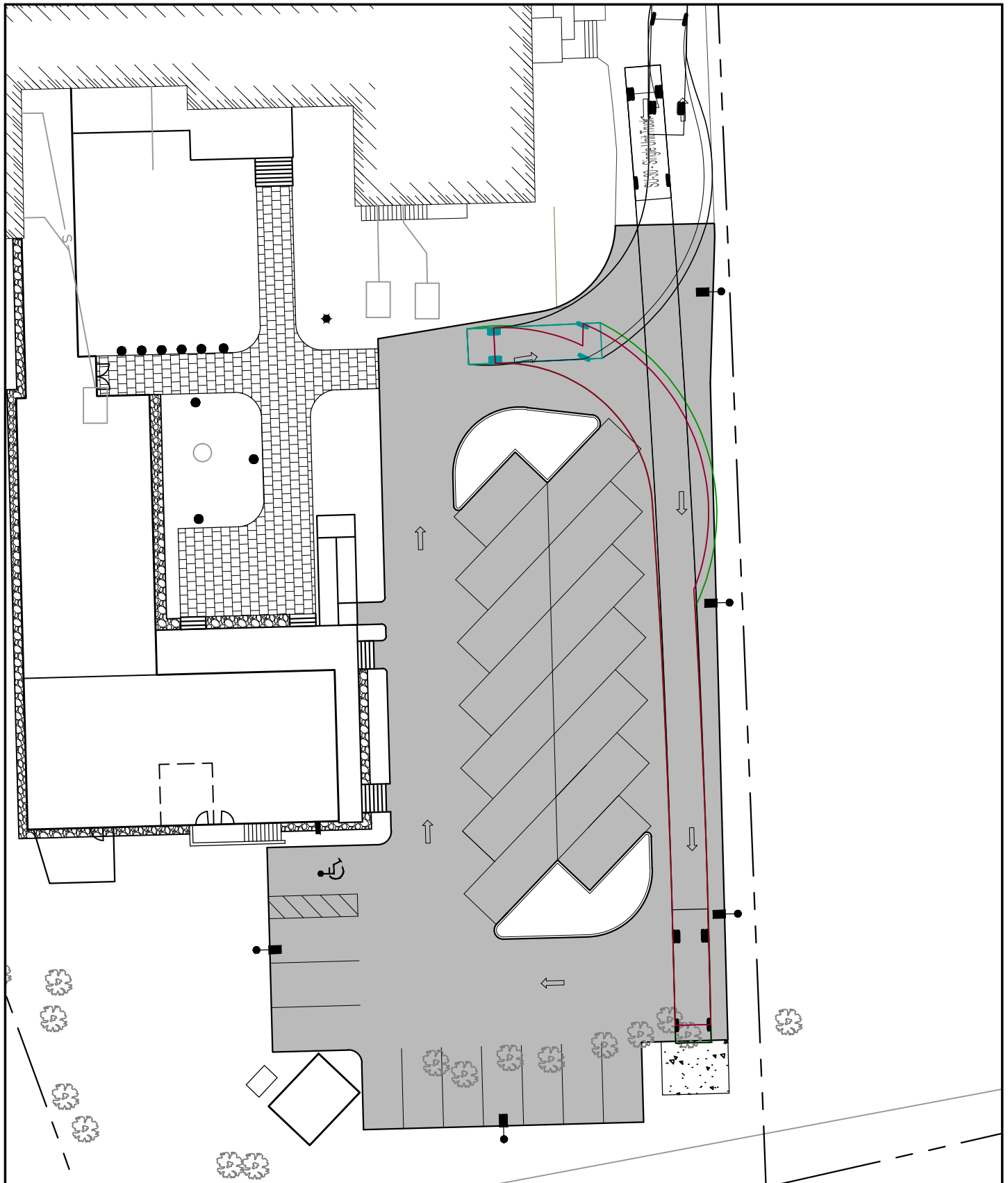
FOR RECORD OWNER:
960 MEADOW ROAD HOLDINGS, LLC
960 MEADOW ROAD
CASCO, MAINE 04015

*TURNING MOVEMENT
BASED ON AASHTO SU-30
TURNING TEMPLATE

SCALE: 1"=30'
DATE: 9-1-2026
JOB NUMBER: 19063

DM ROMA
CONSULTING ENGINEERS

P.O. BOX 1116
WINDHAM, ME 04062
(207) 591-5055



TURNING MOVEMENT EXHIBIT - TRASH HAULER

COUNTRY VILLAGE ASSISTED LIVING FACILITY EXPANSION
CASCO, MAINE

FOR RECORD OWNER:
960 MEADOW ROAD HOLDINGS, LLC
960 MEADOW ROAD
CASCO, MAINE 04015

*TURNING MOVEMENT
BASED ON AASHTO SU-30
TURNING TEMPLATE

SCALE: 1"=30'
DATE: 9-1-2026
JOB NUMBER: 19063

DM ROMA
CONSULTING ENGINEERS

P.O. BOX 1116
WINDHAM, ME 04062
(207) 591-5055

SECTION 5

LIGHTING FIXTURES

LED STREET & AREA LIGHTING



Shown with Pole Side Mount

Specifications and Features:

Radiant™ LED

- Conformal Coating LED light engine
- CCT: Standard-3000K, 4000K, & 5000K
- 2700K, Amber, LPS (1800K) Offered for Some Wattages - Consult Factory
- Static Colors Available - Contact Factory

Distribution

- Types I* II, III, IV, V (Type I Available in 70w & Higher Only)
- Full Cutoff

Electrical

- Electronic Driver, 120-277V, 50/60Hz or 347/480V, 50/60Hz
- 0-10v Dimmable Driver
- Standard Surge Protection Included; Surge Protection Device Meets IEEE C62.41 2002 C High 10kA

Controls (Optional)

- Button Eye Photo Control (PC)
- 3-Pin Twist-Lock Receptacle (PCR3)
- 7-Pin Twist-Lock Receptacle (PCR7)
- House Side Shield (H)

Housing

- Consists of a Molded Aluminum Rim and a Spun Aluminum Canopy that Combine for a Sleek and Compact Appearance
- Side Mount on a horizontal tenon

Finish

- Super Durable Polyester Powder Coat Finish. Custom Finishes Available, Including Patinas and All RAL Colors.

Listings & Ratings

- ETL Listed to UL 1598 Standard for Wet Location.
- LED is IP66 Rated

Warranty

- 5 Year Limited Warranty

EPA: 1.6
Weight: 32 lbs (Max.)



Order Information Example:

PMBOW1NL70W4KU5SMNBK

PMBOW1		SM						
Model	Globe/Lens	Wattage	CCT	Voltage	Optics	Mounting	Options	Finish
PMBOW1=Bowery 1 Radiant™ Side Mount	NL=No Secondary Lens L=SoftLED LumaLens Opal UV-Stabilized Polycarbonate Array Lens* *105W & Above Only	10W=10w 15W=15w 20W=20w 30W=30w 40W=40w 50W=50w 60W=60w 70W=70w 80W=80w 90W=90w 105W=105w 120W=120w 135W=135w 155W=155w	3K=3000K 4K=4000K 5K=5000K	U=120-277V H=347/480V	1=Type I* 2=Type II 3=Type III 4=Type IV 5=Type V *70W & Above Only	SM=Side Mount	N=None FS=Fuse, Single PC=Button Eye Photo Control PCR3=3-Pin Twist-Lock Receptacle PCR7=7-Pin Twist-Lock Receptacle H=House Side Shield	STANDARD FINISHES BK=Black BZ=Bronze DBZ=Dark Bronze WH=White GR=Green SM=Silver Metallic GY=Light Gray TBK=Textured Black CC=Custom Color (Consult Factory)

LED STREET & AREA LIGHTING

Lumen Output



13" Radiant™



12" Radiant™



8" Radiant™



6" Radiant™

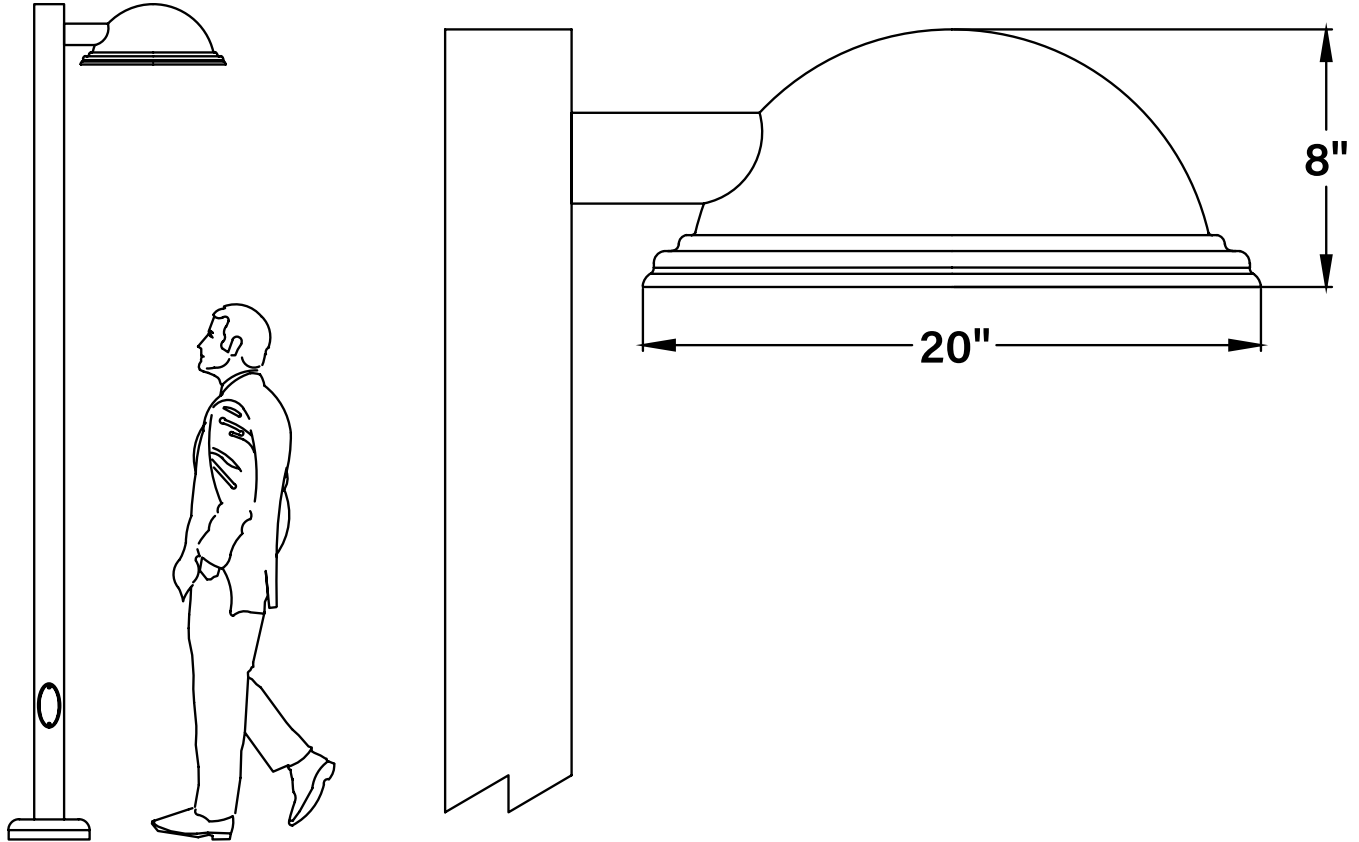
RADIANT™ LED LIGHT ENGINE SPECIFICATIONS:

Lumen Output for Bowery 1														
Wattage (Catalog Logic)	10w (10W)	15w (15W)	20w (20W)	30w (30W)	40w (40W)	50w (50W)	60w (60W)	70w (70W)	80w (80W)	90w (90W)	105w (105W)	120w (120W)	135w (135W)	155w (155W)
Radiant Size (DIA.)	6"	6"	6"	6"	6"	8"	8"	12"	12"	12"	13"	13"	13"	13"
Input Watts	10.2	14.6	20.4	30.6	40.8	51.0	56.6	67.9	77.6	87.3	101.9	116.5	131.0	152.9
Optic	CCT	Delivered Lumens												
1 = Type I Clear PC Lens	3K	NOT AVAILABLE						8,156	9,319	10,485	12,260	14,012	15,763	18,390
	4K							8,486	9,698	10,910	12,757	14,580	16,403	19,136
	5K							8,817	10,075	11,335	13,255	15,149	17,041	19,882
	BUG Rating							B3-U0-G2	B3-U0-G2	B3-U0-G2	B3-U0-G3	B3-U0-G3	B4-U0-G3	Bx-U0-Gx
2 = Type II Clear PC Lens	3K	1,314	1,877	2,628	3,941	5,110	6,595	7,328	8,819	10,078	11,338	13,259	15,152	17,047
	4K	1,367	1,953	2,735	4,102	5,086	6,863	7,625	9,176	10,487	11,798	13,796	15,767	17,739
	5K	1,420	2,029	2,841	4,261	5,524	7,130	7,922	9,533	10,896	12,257	14,333	16,380	18,428
	BUG Rating	B1-U0-G0	B1-U0-G0	B1-U0-G1	B1-U0-G1	B1-U0-G1	B2-U0-G1	B2-U0-G1	B2-U0-G2	B2-U0-G2	B2-U0-G2	B2-U0-G2	B3-U0-G2	B3-U0-G2
3 = Type III Clear PC Lens	3K	1,319	1,884	2,638	3,957	5,130	6,621	7,357	8,854	10,119	11,383	13,312	15,213	17,115
	4K	1,372	1,961	2,746	4,118	5,106	6,891	7,656	9,213	10,530	11,846	13,852	15,831	17,810
	5K	1,425	2,037	2,853	4,278	5,546	7,159	7,954	9,571	10,939	12,307	14,391	16,446	18,502
	BUG Rating	B1-U0-G1	B1-U0-G1	B1-U0-G1	B1-U0-G1	B2-U0-G2	B2-U0-G2	B2-U0-G2	B2-U0-G2	B3-U0-G3	B3-U0-G3	B3-U0-G3	B3-U0-G3	B3-U0-G3
4 = Type IV Clear PC Lens	3K	1,203	1,719	2,407	3,611	4,659	6,042	6,714	8,079	9,233	10,388	12,147	13,882	15,617
	4K	1,252	1,790	2,505	3,757	4,848	6,288	6,986	8,407	9,608	10,809	12,640	14,445	16,251
	5K	1,301	1,859	2,603	3,904	5,037	6,531	7,258	8,734	9,982	11,229	13,132	15,007	16,883
	BUG Rating	B0-U0-G1	B1-U0-G1	B1-U0-G1	B1-U0-G2	B1-U0-G2	B1-U0-G2	B2-U0-G2	B3-U0-G3	B2-U0-G3	B2-U0-G3	B2-U0-G3	B2-U0-G3	B3-U0-G3
5 = Type V Clear PC Lens	3K	1,320	1,886	2,640	3,960	5,134	6,626	7,362	8,860	10,125	11,391	13,321	15,223	17,126
	4K	1,373	1,962	2,748	4,121	5,109	6,895	7,661	9,219	10,536	11,853	12,861	15,841	17,822
	5K	1,426	2,038	2,854	4,281	5,550	7,163	7,959	9,578	10,946	12,315	14,400	16,457	18,514
	BUG Rating	B1-U0-G0	B1-U0-G0	B2-U0-G1	B2-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G1	B3-U0-G2	B3-U0-G2	B3-U0-G2	B4-U0-G2	B4-U0-G2	B4-U0-G3
5 = Type V with LumaLens	3K	NOT AVAILABLE									8,077	9,231	10,385	12,115
	4K										8,404	9,604	10,805	12,606
	5K										8,732	9,978	11,225	13,097
	BUG Rating										B3-U3-G2	B3-U3-G3	B3-U3-G3	B3-U3-G3

Please refer to the fixture's IES file for detailed information. Subject to change.

LED STREET & AREA LIGHTING

Dimensions



LED POST TOP LUMINAIRES



Portsmouth with Lens



Portsmouth with No Lens

← Fitter section slips over 3" or 2 1/2" pole or tenon

Specifications and Features:

Radiant™ LED

- Conformal Coating LED light engine
- CCT: Standard-3000K, 4000K, & 5000K
- 2700K, Amber, LPS (1800K) Offered for Some Wattages - Consult Factory
- Static Colors Available - Contact Factory

Distribution

- Types I* II, III, IV, V (Type I Available in 70w & Higher Only)
- Full Cutoff (No Lens Option Only)

Electrical

- Electronic Driver, 120-277V, 50/60Hz or 347/480V, 50/60Hz
- 0-10V Dimmable Driver
- Standard Surge Protection Included (6kV Line-Line; 10kV Line-Earth)

Controls (Optional)

- Button Eye Photo Control (PC)

Housing

- Cast Aluminum Cage, Roof and 3" Slip Fitter

Finish

- Super Durable Polyester Powder Coat Finish. Custom Finishes Available, Including Patinas and All RAL Colors.

Listings & Ratings

- ETL Listed to UL 1598 Standard for Wet Location and IP66.
- LED is IP66 Rated

Warranty

- 5 Year Limited Warranty



EPA: 1.7
Weight: 38 lbs (Max.)

Order Information Example: PORTNL60W4KU3JNNBK

PORT									
Model	Globe/Lens	Wattage	CCT	Voltage	Optics	Finial	Mounting	Options	Finish
PORT=Portsmouth	NL=No Secondary Lens CA=Clear Acrylic CP=Clear Polycarbonate FA=Frosted Acrylic FP=Frosted Polycarbonate WA=Opal White Acrylic PA=Pattern Acrylic DA=Seeded Acrylic	40W=40w 50W=50w 60W=60w 70W=70w 80W=80w 90W=90w	3K=3000K 4K=4000K 5K=5000K	U=120-277V H=347/480V	1=Type I* 2=Type II 3=Type III 4=Type IV 5=Type V *70W & Above Only	D=GF4 (Standard) B=GF2 C=GF3 F=GF6 H=GF8 I=GF9 J=GF10 K=GF12 O=GF17 P=GF18 V=GF24 N=None	N=None* PEN=Pendant Mount *Standard Post Mount	N=None PC=Button Eye Photo Control	STANDARD FINISHES BK=Black BZ=Bronze DBZ=Dark Bronze WH=White GR=Green SM=Silver Metallic GY=Light Gray TBK=Textured Black CC=Custom Color (Consult Factory)

LED POST TOP LUMINAIRES

Photometric Data

For Lensed Fixtures



12" Radiant™



8" Radiant™

RADIANT™ LED LIGHT ENGINE SPECIFICATIONS:

Lumen Output for Portsmouth							
Wattage (Catalog Logic)		40w (40W)	50w (50W)	60w (60W)	70w (70W)	80w (80W)	90w (90W)
Radiant Size (DIA.)		8"	8"	8"	12"	12"	12"
Input Watts		40.8	51.0	56.6	67.9	77.6	87.3
Optic	CCT	Delivered Lumens					
1 = Type I Clear, Frosted, Prismatic or Seedy	3K	NOT AVAILABLE			8,165	9,331	10,498
	4K				8,335	9,525	10,716
	5K				8,461	9,670	10,879
	BUG Rating				B3-U3-G2	B3-U3-G2	B3-U3-G2
2 = Type II Clear, Frosted, Prismatic or Seedy	3K	4,703	5,878	7,055	8,230	9,405	10,582
	4K	4,800	6,001	7,200	8,401	9,600	10,801
	5K	4,874	6,091	7,310	8,528	9,747	10,965
	BUG Rating	B1-U3-G1	B1-U3-G1	B2-U3-G2	B2-U3-G2	B3-U3-G2	B2-U3-G2
3 = Type III Clear, Frosted, Prismatic or Seedy	3K	4,695	5,869	7,043	8,217	9,390	10,564
	4K	4,792	5,991	7,188	8,387	9,584	10,783
	5K	4,866	6,081	7,298	8,514	9,731	10,947
	BUG Rating	B2-U3-G2	B2-U3-G2	B2-U3-G2	B2-U3-G2	B3-U3-G3	B3-U3-G3
4 = Type IV Clear, Frosted, Prismatic or Seedy	3K	4,591	5,739	6,887	8,034	9,182	10,330
	4K	4,686	5,858	7,029	8,201	9,372	10,544
	5K	4,758	5,947	7,136	8,326	9,515	10,705
	BUG Rating	B1-U3-G2	B2-U3-G2	B2-U3-G2	B2-U3-G2	B2-U3-G3	B3-U3-G3
5 = Type V Clear, Frosted, Prismatic or Seedy	3K	4,804	6,005	7,207	8,407	9,608	10,809
	4K	4,903	6,130	7,355	8,582	9,807	11,033
	5K	4,979	6,222	7,467	8,712	9,956	11,201
	BUG Rating	B3-U3-G1	B3-U3-G1	B3-U3-G2	B3-U3-G2	B3-U3-G2	B4-U3-G2
5 = Type V with White-Opal Lens	3K	3,857	4,821	5,785	6,750	7,714	8,679
	4K	3,937	4,921	5,906	6,890	7,874	8,858
	5K	3,997	4,997	5,996	6,995	7,994	8,993
	BUG Rating	B1-U4-G2	B2-U4-G3	B2-U5-G3	B2-U5-G3	B2-U5-G3	B3-U5-G4

Please refer to the fixture's IES file for detailed information. Subject to change.

LED POST TOP LUMINAIRES

Photometric Data

For Open Full Cutoff Fixtures



12" Radiant™



8" Radiant™

RADIANT™ LED LIGHT ENGINE SPECIFICATIONS:

Lumen Output for Portsmouth with No Secondary Lens							
Wattage (Catalog Logic)		40w (40W)	50w (50W)	60w (60W)	70w (70W)	80w (80W)	90w (90W)
Radiant Size (DIA.)		8"	8"	8"	12"	12"	12"
Input Watts		40.8	51.0	56.6	67.9	77.6	87.3
Optic	CCT	Delivered Lumens					
1 = Type I Clear PC Lens	3K	NOT AVAILABLE			7,291	8,332	9,374
	4K				7,442	8,505	9,568
	5K				7,555	8,634	9,714
	BUG Rating				B2-U0-G2	B2-U0-G2	B3-U0-G3
2 = Type II Clear PC Lens	3K	4,358	5,447	6,537	7,626	8,716	9,805
	4K	4,448	5,560	6,672	7,785	8,896	10,008
	5K	4,516	5,644	6,774	7,903	9,032	10,161
	BUG Rating	B1-U0-G1	B1-U0-G1	B2-U0-G1	B2-U0-G1	B2-U0-G1	B2-U0-G2
3 = Type III Clear PC Lens	3K	4,569	5,711	6,853	7,996	9,138	10,280
	4K	4,664	5,829	6,996	8,161	9,327	10,493
	5K	4,734	5,919	7,102	8,285	9,469	10,652
	BUG Rating	B2-U0-G2	B2-U0-G2	B2-U0-G2	B2-U0-G2	B3-U0-G3	B3-U0-G3
4 = Type IV Clear PC Lens	3K	4,562	5,703	6,843	7,985	9,125	10,265
	4K	4,657	5,821	6,986	8,150	9,314	10,478
	5K	4,728	5,910	7,092	8,274	9,456	10,638
	BUG Rating	B1-U0-G2	B1-U0-G3	B1-U0-G3	B2-U0-G3	B2-U0-G3	B2-U0-G3
5 = Type V Clear PC Lens	3K	4,842	6,052	7,262	8,473	9,683	10,983
	4K	4,942	6,177	7,413	8,648	9,884	11,119
	5K	5,017	6,272	7,526	8,780	10,034	11,289
	BUG Rating	B3-U0-G1	B3-U0-G1	B3-U0-G2	B3-U0-G2	B3-U0-G2	B4-U0-G2

Please refer to the fixture's IES file for detailed information. Subject to change.

LED POST TOP LUMINAIRES

Style Options



**Pendant Mount:
No Lens, No Bottom**



**Pendant Mount:
Lens, No Bottom**



**Pendant Mount:
Lens, Bottom**



**Pendant Mount:
No Lens, No Bottom with Stem**



**Pendant Mount:
Lens, No Bottom with Stem**



**Pendant Mount:
Lens, Bottom with Stem**

LED POST TOP LUMINAIRES

Fixture Finials



GF2 - 6"



GF3 - 6"



GF4 - 6"



GF5 (Plastic)
GF6 (Cast) - 4"



GF8 - 4"



GF9 - 4.25"



GF10 - 3.5"



GF12 - 2.75"



GF17 - 4"



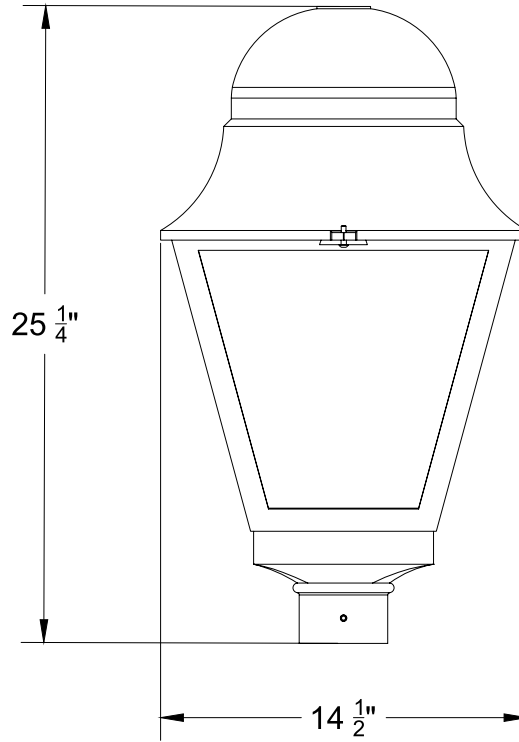
GF18 - 2.75"



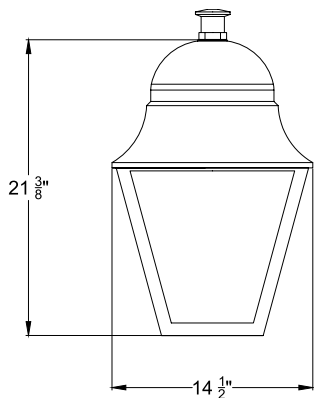
GF24 - 4.75"

LED POST TOP LUMINAIRES

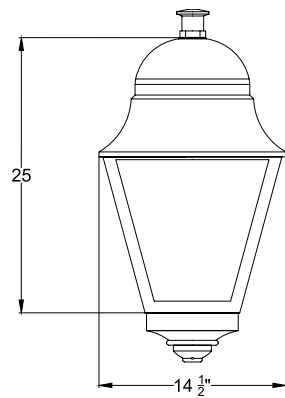
Dimensions



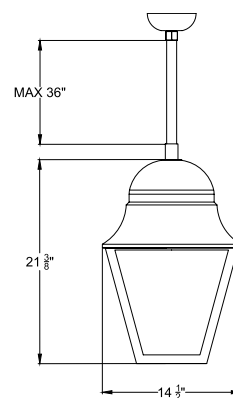
Post Top Mount



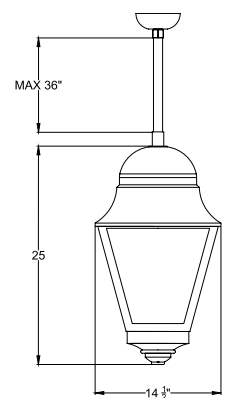
Pendant Mount



**Pendant Mount
with Bottom**



**Pendant Mount
with Stem**



**Pendant Mount
with Stem & Bottom**

ROUND BOLLARD LIGHTING



CAVRD
Round Head

CAVAG
Angled Head

CAVSQ
Square Head

Specifications and Features:

Radiant™ LED

- Conformal Coating LED light engine
- CCT: Standard-3000K, 4000K, & 5000K
- 2700K, Amber, LPS (1800K) Offered for Some Wattages - Consult Factory
- Static Colors Available - Contact Factory

Distribution

- Types II, III, IV, V

Electrical

- Electronic Driver, 120-277V, 50/60Hz or 347/480V, 50/60Hz
- 0-10v Dimmable Driver
- Standard Surge Protection Included (6kV Line-Line; 10k-V Line-Earth)

Controls (Optional)

- Button Eye Photo Control (PC)
- House Side Shield (H)

Housing

- Aluminum Head with Extruded Aluminum Body.

Finish

- Super Durable Polyester Powder Coat Finish. Custom Finishes Available, Including Patinas and All RAL Colors.

Listings & Ratings

- ETL Listed to UL 1598 Standard for Wet Location and IP66.
- LED is IP66 Rate

Warranty

- 5 Year Limited Warranty



Double Head
2 Heads @ 180°

Double Head
3 Heads @ 120°



ROUND BOLLARD LIGHTING

Order Information Example:		CAVRDSG20W3KU2NSM					
Model	Number of Heads	Wattage Per Head	CCT	Voltage	Optics	Options	Finish
CAVRD=Round CAV Bollard CAVSQ=Square CAV Bollard CAVAG=Angled CAV Bollard	SG=Single (1x) DB=Double (2x @ 180°) TR=Triple (3x @ 120°)	10W=10w 15W=15w 20W=20w 30W=30w	3K=3000K 4K=4000K 5K=5000K	U=120-277V H=347/480V	2=Type II 3=Type III 4=Type IV 5=Type V	N=None PC=Button Eye Photo Control H=House Side Shield USB=Dual USB Port* GFIC=GFI In-Use Cover, Cast Aluminum* GFIP=GFI In-Use Cover, Plastic* *Indicate location	STANDARD FINISHES BK=Black BZ=Bronze DBZ=Dark Bronze WH=White GR=Green SM=Silver Metallic GY=Light Gray TBK=Textured Black CC=Custom Color (Consult Factory)

Photometric Data



6" Radiant™

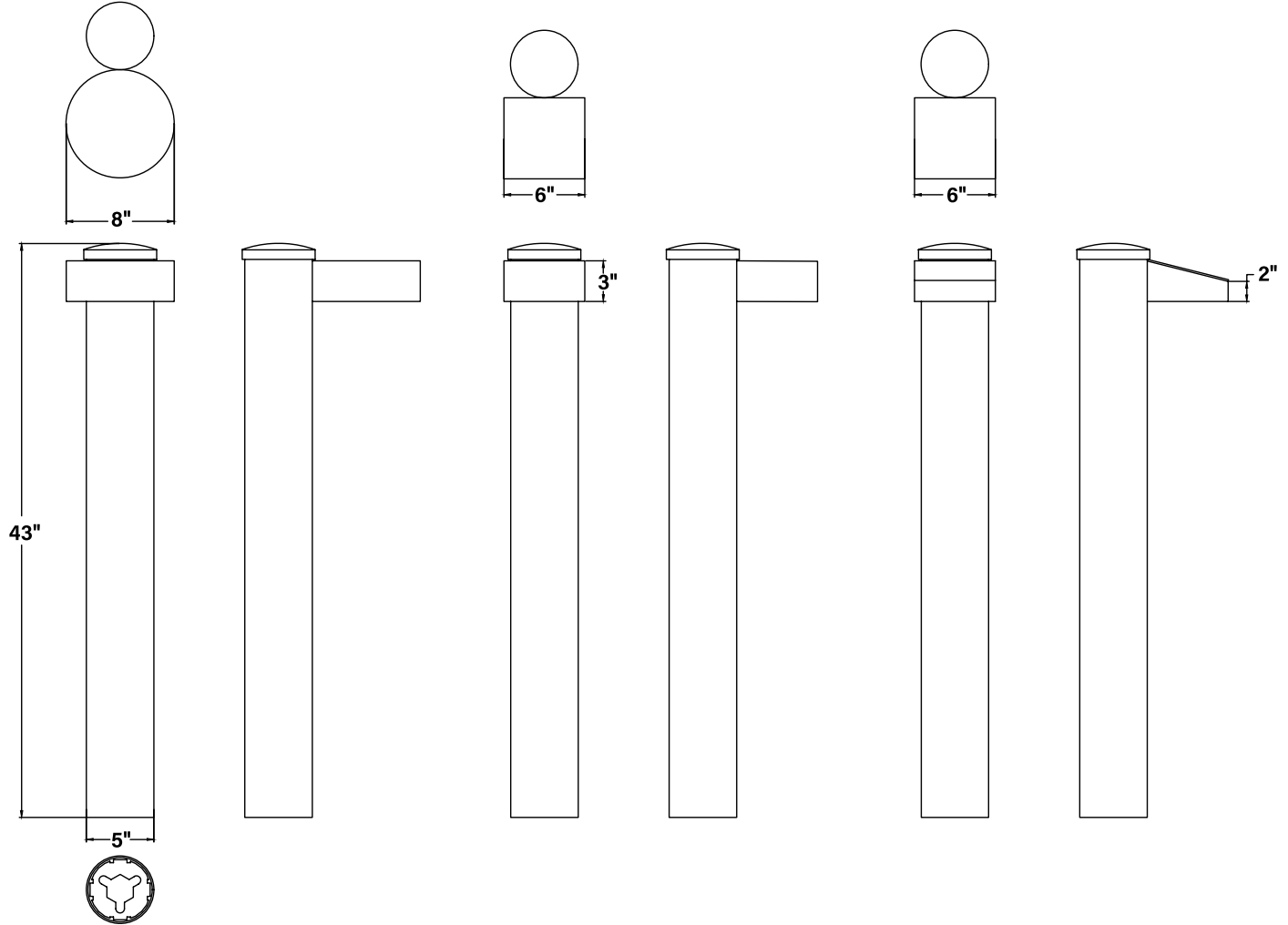
RADIANT™ LED LIGHT ENGINE SPECIFICATIONS:

Lumen Output for Cavalier RD-SQ-AG (Open Bollard)					
Wattage (Catalog Logic)		10 Watts (10W)	15 Watts (15W)	20 Watts (20W)	30 Watts (30W)
Radiant Size (DIA.)		6"	6"	6"	6"
Input Watts		10.2	14.6	20.4	30.6
Optic	CCT	Delivered Lumens			
2 = Type II Clear PC Lens	3K	1,099	1,634	2,179	3,268
	4K	1,112	1,668	2,224	3,366
	5K	1,129	1,634	2,258	3,387
	BUG Rating	B0-U0-G0	B1-U0-G0	B1-U0-G1	B1-U0-G1
3 = Type III Clear PC Lens	3K	1,142	1,713	2,284	3,427
	4K	1,165	1,749	2,332	3,497
	5K	1,183	1,776	2,367	3,550
	BUG Rating	B1-U0-G1	B1-U0-G1	B1-U0-G1	B1-U0-G1
4 = Type IV Clear PC Lens	3K	1,140	1,711	2,281	3,422
	4K	1,164	1,747	2,329	3,492
	5K	1,182	1,773	2,364	3,546
	BUG Rating	B0-U0-G0	B1-U0-G1	B1-U0-G1	B1-U0-G2
5 = Type V Clear PC Lens	3K	1,210	1,815	2,420	3,632
	4K	1,235	1,854	2,471	3,706
	5K	1,254	1,882	2,508	3,762
	BUG Rating	B1-U0-G0	B1-U0-G1	B2-U0-G1	B2-U0-G1

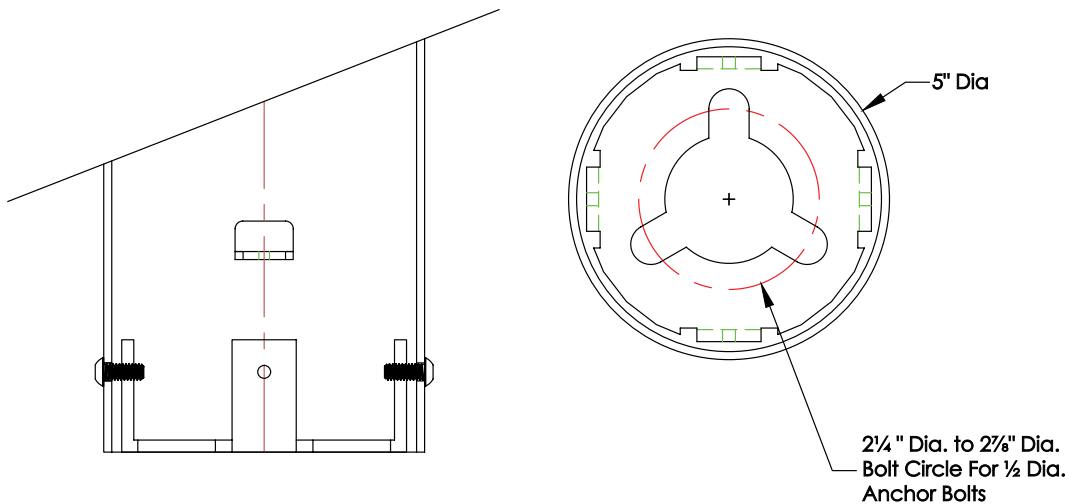
Please refer to the fixture's IES file for detailed information. Subject to change.

ROUND BOLLARD LIGHTING

Dimensions

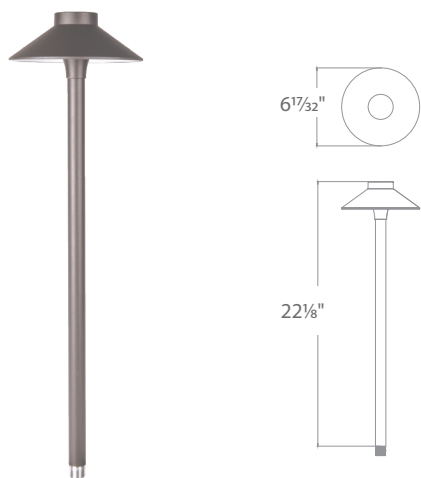


Anchor Base Details



TIKKI LED AREA LIGHT
6041

WAC
LANDSCAPE LIGHTING



Fixture Type:

Catalog Number:

Project:

Location:

PRODUCT DESCRIPTION

Sturdy side mount design works well in applications where directional illumination is desired

SPECIFICATIONS

Input: 9-15VAC (Transformer is required)
Power: 6.5W / 9.5VA (Use VA value for sizing the transformer and calculate voltage drop.)
Brightness: 225 lm
CRI: 90+
Rated Life: 60,000 hours

FEATURES

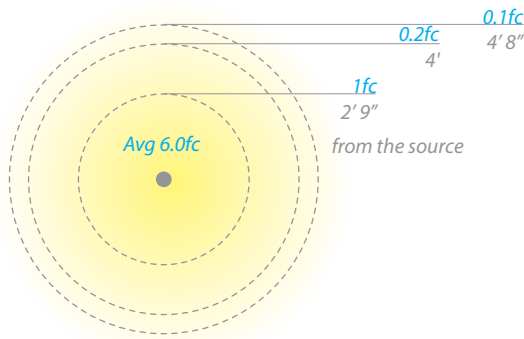
- IP66 rated, Protected against powerful water jets
- Factory sealed water tight fixtures
- Solid diecast corrosion resistant aluminum alloy
- Recommended spacing for installation: Residential: 7 to 9ft; Commercial: 5 to 7ft
- Mounting stake, 6 foot lead wire, and direct burial gel filled wire nuts are included
- Maintains constant lumen output against voltage drop
- UL & cUL 1838 Listed

ORDERING NUMBER

Color Temp		Finish
6041	Tiki	27 2700K Warm White
		30 3000K Pure White
		BBR Bronze on Brass BZ Bronze on Aluminum BK Black on Aluminum

6041-__BK

Example: 6041-30BK



TIKKI LED AREA LIGHT
6041

WAC
LANDSCAPE LIGHTING

Surface Mount Flange/Stake



Includes three 7 inch threaded stainless steel stabilizing pins for ground mounting or surface mounts with four screws or over a junction box

5000-SCP-BZ
Bronze on Aluminum

Additional
Mounting Stake



9000-ST9-BZ
Durable PVC stake



Guardian Mount

*Heavy duty stainless steel spike to position fixture.
Formed from a single piece of metal*

9000-SP9-BZ
Stainless Steel

Magnetic Transformers

*Stainless Steel, 12-15V output, IP65 rated, UL 1838 listed
See transformer spec sheet for details and its accessories*

9075-TRN-SS
75W Max

9150-TRN-SS
150W Max

9300-TRN-SS
300W Max

9600-TRN-SS
600W Max



NU essentials

4" Round & Square Downlight Standard White



PROJECT INFORMATION		
JOB NAME		TYPE
ORDERING CODE		

30° - 60° BEAM		
(Note: Specifications are subject to change without notice)		
9mm COB PERFORMANCE DATA		
LED LIGHT ENGINE	NOMINAL DELIVERED LUMENS	SYSTEM WATTAGE
10LM	1070LM @30K/90CRI	11W
13LM	1360LM @30K/90CRI	14W
15LM	1560LM @30K/90CRI	16W
20LM	2000LM @30K/90CRI	21W
Notes	Delivered lumens based on 40D optic with np lens, (see page 2)	

14° - 25° BEAM		
(Note: Specifications are subject to change without notice)		
6mm COB PERFORMANCE DATA		
LED LIGHT ENGINE	NOMINAL DELIVERED LUMENS	SYSTEM WATTAGE
10LM	970LM @30K/90CRI	11W
15LM	1450LM @30K/90CRI	18W
Notes	Delivered lumens based on 25D optic with no lens, (see page 2)	



FEATURES

- Economical choice for budget projects
- Thermally optimized for LED longevity
- Fully casted assembly
- 14° - 80° optical beam control
- Packaging material consists of 60-80% post-consumer recycled fiber

DIMMING AND CONTROLS

- eldoLED 0-10V, 0.1% and 1%
- Leading & trailing edge (Triac/ELV), dim to 1%

CONSTRUCTION

- Aluminum trim and bezel
- Electrocoated 16-gauge cold-rolled steel construction
- Accommodates ceiling thickness from 1/4" to 1"

LED

- 90 CRI: SDCM = 2-step MacAdam Ellipse, Lumen Maintenance: L₇₀ > 66,000 hrs

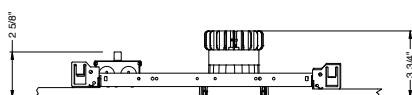
LISTING

- ULus Listed to UL1598; cUL Listed to CSA C22.2 #250.0
- NSF/ANSI-2 with lens (Non-Food and Splash Zones)
- IP44 with lens - suitable for wet location with lens - suitable damp location without lens
- 5 Year Limited warranty

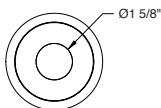
ELECTRICAL

- 120V-277V, 120 only Triac / ELV
- Power factor ≥ 0.9
- 2kV driver input surge protection
- 7W and 10W (T20 CEC) EM 90min battery
- Max. ambient installation temperature 104°F (40°C)

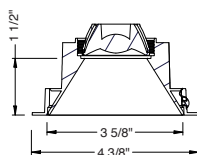
FIXTURE HEIGHT



ROUND

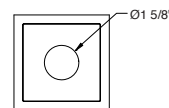


ceiling cutout
4" diameter
ceiling thickness
1/4" to 1"

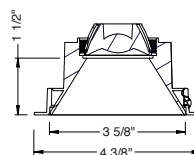


aperture dimensions

SQUARE



ceiling cutout
4" x 4" square
ceiling thickness
1/4" to 1"



aperture dimensions

PROJECT INFORMATION			
JOB NAME			TYPE
ORDERING CODE			

15°- 80° BEAM ORDERING CODE							
SERIES	NUE4	NUE					
TYPE	RD	round downlight					
	QD	square downlight					
	RDT	round downlight trimless					
	QDT	square downlight trimless					
LED	SW	standard white					
DELIVERED LUMENS	10LM	1070 lm					
	13LM	1360 lm					
	15LM	1560 lm					
	20LM	2000 lm					
	Standard Configuration: 3000K 90CRI 40D NL						
Calculated Delivered Lumens = [Delivered Lumen Value] x [CCT Multiplier] x [Reflector Multiplier]							
CCT	27K	2700K	CCT	2700K	3000K	3500K	4000K
	30K	3000K	CRI	90+	90+	90+	90+
	35K	3500K	CCT MULTIPLIER FOR LUMEN OUTPUT	0.96	1	1.03	1.06
	40K	4000K					
CRI	90	90 CRI					
OPTIC & LM MULTIPLIER SEE PAGE 3	14D ¹	14 degrees					(0.90)
	25D ¹	25 degrees					(1.00)
	30D	30 degrees					(0.95)
	40D	40 degrees					(1.00)
	60D	60 degrees					(1.00)
For 80° choose '60D' + 'DL' lens.							
LENS	NL ²	no lens					(1.00)
	CL ³	clear lens					(0.95)
	DL ³	diffused lens					(0.76)
	HET ³	high efficiency textured lens, similar to Solite™					(0.92)
	HCL ⁴	honeycomb louver					(0.64)
TRIM COLOR	BK	black					
	WH	white					
	MC	matte chrome					
Not applicable to trimless option. Do not include in trimless ordering code.							
BEZEL COLOR	BK	black					(0.95)
	WH	white					(1.00)
	MC	matte chrome					(0.99)

15°- 80° BEAM ORDERING CODE		
MOUNTING OPTIONS SEE PAGE 4, 7	NC	new construction plate
	IC	insulation contact housing
	ICAT	insulation contact/airtight housing
	CP	chicago plenum housing
	RET	retrofit, no ceiling plate
	NCIC ⁵	new construction with ceiling fitting plate, insulation contact rated (no housing required)
	RETIC ⁵	retrofit, no ceiling fitting plate, insulation contact rated (no housing required)
VOLTAGE	I20	120V
	UNV	120V-277V
DIMMING	DIM10	eldoLED flicker free 0-10V dim to 1%
	ELVI ⁶	leading edge & trailing edge (Triac/ELV) dim to 1%
ELECTRICAL OPTIONS	EM7	remote emergency battery, 90 min at 7W
	EM10CA20	emergency battery backup, 90 minutes at 10W to LED, CA title 20

ADDITIONAL MOUNTING INFO	
New Construction Bar Hangers (included)	Mounting Length: 14-3/4" to 26"
Extension Kit p/n: K20266 (ordered separately, 1 per fixture)	<i>Extends a pair of Bar Hangers</i> Total Mounting Length: 29" to 48"

YOUR COMPLETED ORDERING CODE	
Follow the steps to specify your fixture, example:	
NUE4 - RD - SW - 20LM - 30K - 90 - 30D - NL - WH - WH - NC - UNV - DIM10	

NOTES

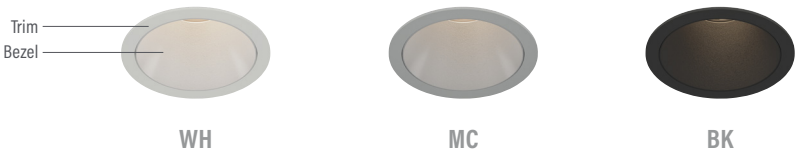
1. 14D/25D available in 10LM/15LM. Not available with DL lens.
2. NL lens option does not meet requirements for WET/NSF listings.
3. CL/DL/HET lens options with IC or ICAT housing are JA8 compliant.
4. HCL cannot be used with lens (max 1 media).
5. NCIC/RETIC are available in 10LM/13LM rated for insulation contact (IC), no "IC-Housing" required.
6. ELVI available in 120V.

alphabet

PROJECT INFORMATION		
JOB NAME		TYPE
ORDERING CODE		

TRIM OPTIONS

TRIM/BEZEL COLORS



OPTICAL OPTIONS

- UGR calculation based on CIE 117-1995; room size: 4H X 8H, reflectance: 70/50/20;
- UGR calculation based on 15LM fixtures, unless otherwise noted.

HCL

Honeycomb Louver



OPTIC	BEAM ANGLE	UGR
14D	13	≤5
25D	24	≤5
30D	30	8.5
40D	37	7.7
60D	47	13.9

NL

No Lens



OPTIC	BEAM ANGLE	UGR
14D	14	6.1
25D	25	≤5
30D	31	14
40D	41	11
60D	58	17.9

CL

Clear Lens



OPTIC	BEAM ANGLE	UGR
14D	14	6.5
25D	26	≤5
30D	32	13.4
40D	42	11.9
60D	58	17.9

DL

Diffused Lens



OPTIC	BEAM ANGLE	UGR
30D	65	21.2
40D	70	22.3
60D	80	23.2

HET

High Efficiency Textured Lens

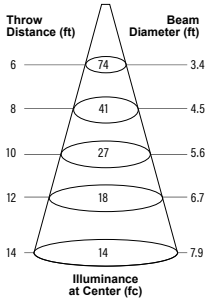


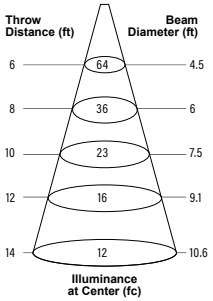
OPTIC	BEAM ANGLE	UGR
14D	18	7.8
25D	27	≤5
30D	34	13.5
40D	41	13.9
60D	56	18.8

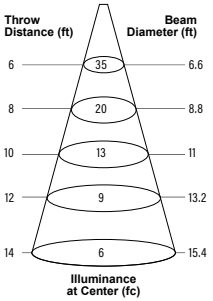
PROJECT INFORMATION			
JOB NAME		TYPE	
ORDERING CODE			

PHOTOMETRIC DATA

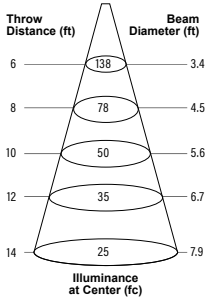
10LM	
	90CRI
2700K	0.96
3000K	1
3500K	1.03
4000K	1.06
FC Formula = CBCP / Distance ²	

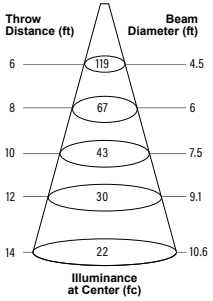
30D 31° WITH NO LENS	
	
WATTS: 10.4	LPW: 97.2
LUMENS: 1011	CCT: 3000K
INTENSITY	
DEGREE	CANDELA
0	2653
5	2580
15	1442
25	496
35	152
45	30

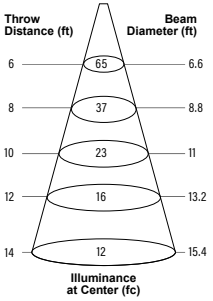
40D 41° WITH NO LENS	
	
WATTS: 10.4	LPW: 102.9
LUMENS: 1070	CCT: 3000K
INTENSITY	
DEGREE	CANDELA
0	2293
5	2242
15	1761
25	631
35	100
45	15

60D 58° WITH NO LENS	
	
WATTS: 10.4	LPW: 100.6
LUMENS: 1046	CCT: 3000K
INTENSITY	
DEGREE	CANDELA
0	1251
5	1242
15	1137
25	813
35	300
45	47

20LM	
	90CRI
2700K	0.96
3000K	1
3500K	1.03
4000K	1.06
FC Formula = CBCP / Distance ²	

30D 31° WITH NO LENS	
	
WATTS: 20.7	LPW: 91.5
LUMENS: 1893	CCT: 3000K
INTENSITY	
DEGREE	CANDELA
0	4969
5	4832
15	2700
25	930
35	285
45	56

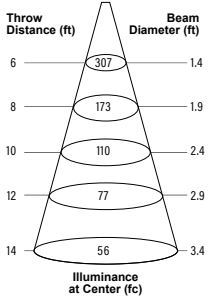
40D 41° WITH NO LENS	
	
WATTS: 20.7	LPW: 96.8
LUMENS: 2004	CCT: 3000K
INTENSITY	
DEGREE	CANDELA
0	4294
5	4198
15	3298
25	1182
35	187
45	27

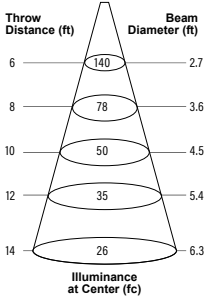
60D 58° WITH NO LENS	
	
WATTS: 20.7	LPW: 94.7
LUMENS: 1959	CCT: 3000K
INTENSITY	
DEGREE	CANDELA
0	2343
5	2326
15	2129
25	1523
35	561
45	88

PROJECT INFORMATION			
JOB NAME		TYPE	
ORDERING CODE			

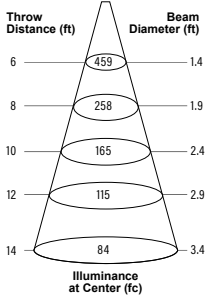
PHOTOMETRIC DATA

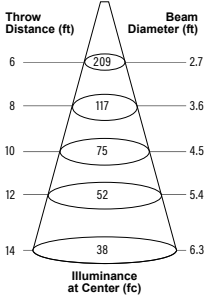
10LM	
	90CRI
2700K	0.96
3000K	1
3500K	1.03
4000K	1.06
FC Formula = CBCP / Distance ²	

14D 13.7° WITH NO LENS	
	
WATTS: 11.1	LPW: 78.4
LUMENS: 870	CCT: 3000K
INTENSITY	
DEGREE	CANDELA
0	11050
5	8165
15	579
25	164
35	55
45	12

25D 25° WITH NO LENS	
	
WATTS: 11.1	LPW: 87.5
LUMENS: 972	CCT: 3000K
INTENSITY	
DEGREE	CANDELA
0	5024
5	4701
15	1724
25	116
35	25
45	5

15LM	
	90CRI
2700K	0.96
3000K	1
3500K	1.03
4000K	1.06
FC Formula = CBCP / Distance ²	

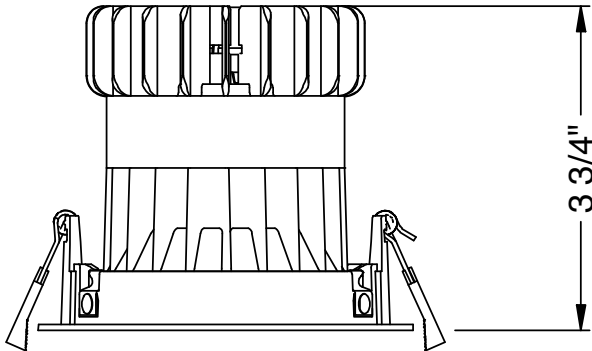
14D 13.7° WITH NO LENS	
	
WATTS: 18.1	LPW: 71.9
LUMENS: 1301	CCT: 3000K
INTENSITY	
DEGREE	CANDELA
0	16518
5	12205
15	865
25	245
35	82
45	18

25D 25° WITH NO LENS	
	
WATTS: 18.1	LPW: 80.3
LUMENS: 1453	CCT: 3000K
INTENSITY	
DEGREE	CANDELA
0	7509
5	7028
15	2578
25	173
35	37
45	8

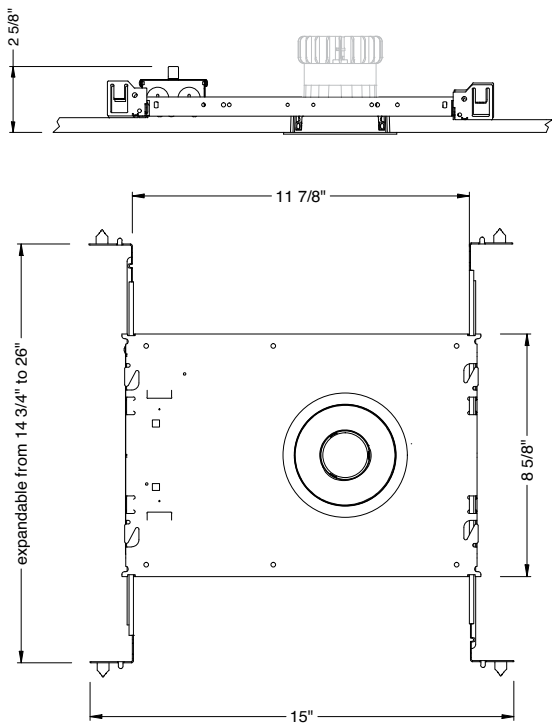
PROJECT INFORMATION			
JOB NAME		TYPE	
ORDERING CODE			

MOUNTING OPTIONS

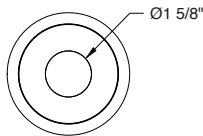
FIXTURE HEIGHT



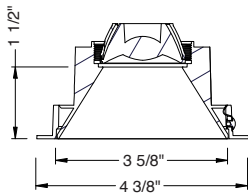
NC - NEW CONSTRUCTION
TRIM



ROUND

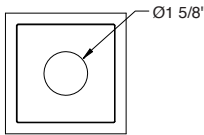


ceiling cutout
4" diameter
ceiling thickness
1/4" to 1"

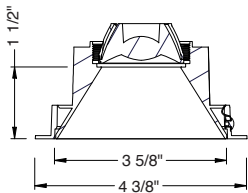


aperture dimensions

SQUARE



ceiling cutout
4" x 4" square
ceiling thickness
1/4" to 1"

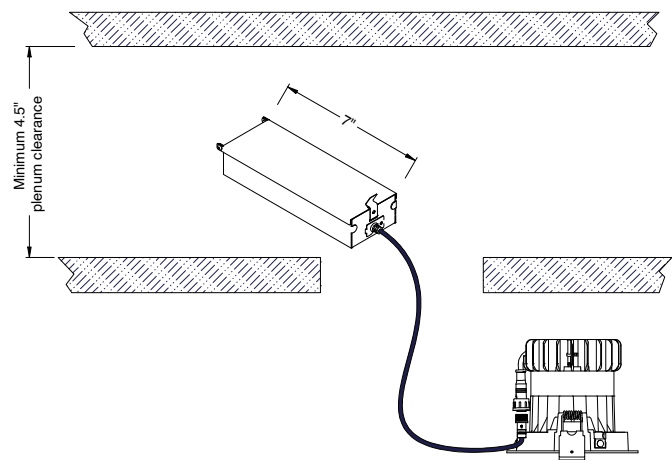


aperture dimensions

PROJECT INFORMATION			
JOB NAME		TYPE	
ORDERING CODE			

MOUNTING OPTIONS (CONTINUED)

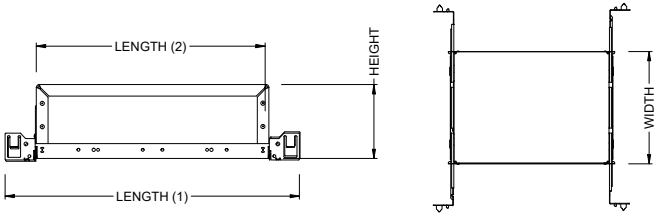
NC - RET - NCIC - RETIC



RATINGS / CERTIFICATIONS	NC	RET	IC	ICAT	CP	NCIC	RETIC
TYPE NON-IC	✓	✓					
TYPE IC			✓	✓		✓	✓
CHICAGO PLENUM (CCEA)					✓		
SUITABLE FOR AIR HANDLING PLENUMS	✓	✓	✓	✓	✓	✓	✓

CEILING THICKNESS	
FIXTURE TYPE	MOUNTING TYPE
	NC, IC, ICAT, CP, RET, NCIC, RETIC
	STANDARD CEILING THICKNESS
TRIM	1/4" to 1"
TRIMLESS	1/4" to 1"

IC - INSULATION CONTACT HOUSING
ICAT - INSULATION CONTACT / AIR TIGHT
CP - CHICAGO PLENUM



BOX SIZE	L 1	L 2	W	H
A	15 - 7/16"	11 - 3/4"	8 - 1/2"	6 - 3/4"

BOX TYPE / LUMEN OUTPUT - STANDARD BEAMS (30° - 80)			
LUMEN OUTPUT	BOX - A		
	IC	ICAT	CP
10LM	✓	✓	✓
15LM	✓	✓	✓
20LM	✓	✓	✓

PROJECT INFORMATION			
JOB NAME		TYPE	
ORDERING CODE			

BATTERY OPTIONS

EMERGENCY BATTERY

IOTA's ILB Battery Backups are UL Listed LED emergency drivers that allow the same LED fixture to be used for both normal and emergency operation. In the event of a power failure, the ILB switches to the emergency mode and operates the existing fixture for 90 minutes. The unit contains a battery, charger, and converter circuit in a single can. The Constant Power design of the ILB maintains the output wattage to the LED array even as the system voltage diminishes. UL 924 Listed for U.S. and Canada. UL 1310 Certified, Output Class 2 Compliant. Includes single-piece TBTS test switch and charge indicator accessory kit. For use with switched and unswitched fixtures, and includes Two-wire universal AC input. Meets or exceeds all National Electric Code and Life Safety Code Emergency Lighting Requirements. Rated for use in Plenum, Damp Location, Recessed Type IC, and Enclosed and Gasketed Luminaires.

REMOTE TEST SWITCH

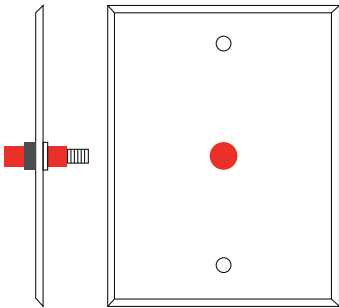
The Remote Test Switch may be mounted adjacent to the LED Fixture by others.

EMERGENCY BATTERY ACCESS

Above ceiling access is required.

REMOTE LOCATION

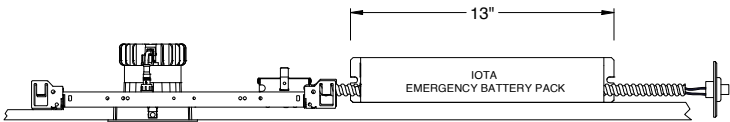
Maximum remote mounting distance of the emergency driver shall be 50 feet. Remote location wiring provided by others. Follow all Local and National Electric/ Building Codes.



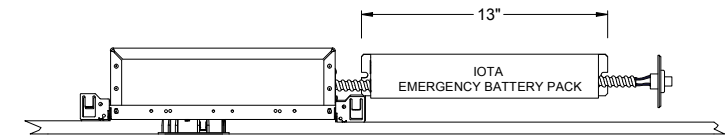
Remote Test Switch

EM MODE OUTPUT (DELIVERED LUMENS)		
LUMEN ORDERING CODE	EM7	EM10
ALL OPTIONS (10LM TO 20LM)	750LM	1070LM
Notes: Based on 30K, 90CRI		

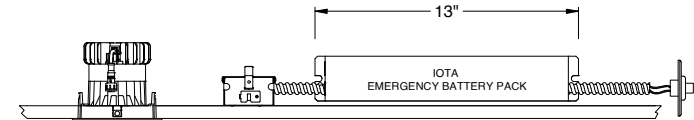
NC - NEW CONSTRUCTION
WITH EM BATTERY (REMOTE TEST SWITCH)



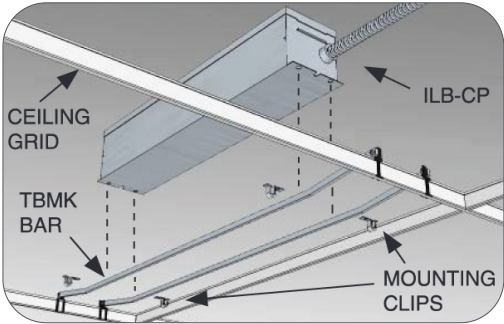
IC / ICAT / CP - INSULATION CONTACT HOUSING
WITH EM BATTERY (REMOTE TEST SWITCH)



RET - RETROFIT
WITH EM BATTERY (REMOTE TEST SWITCH)



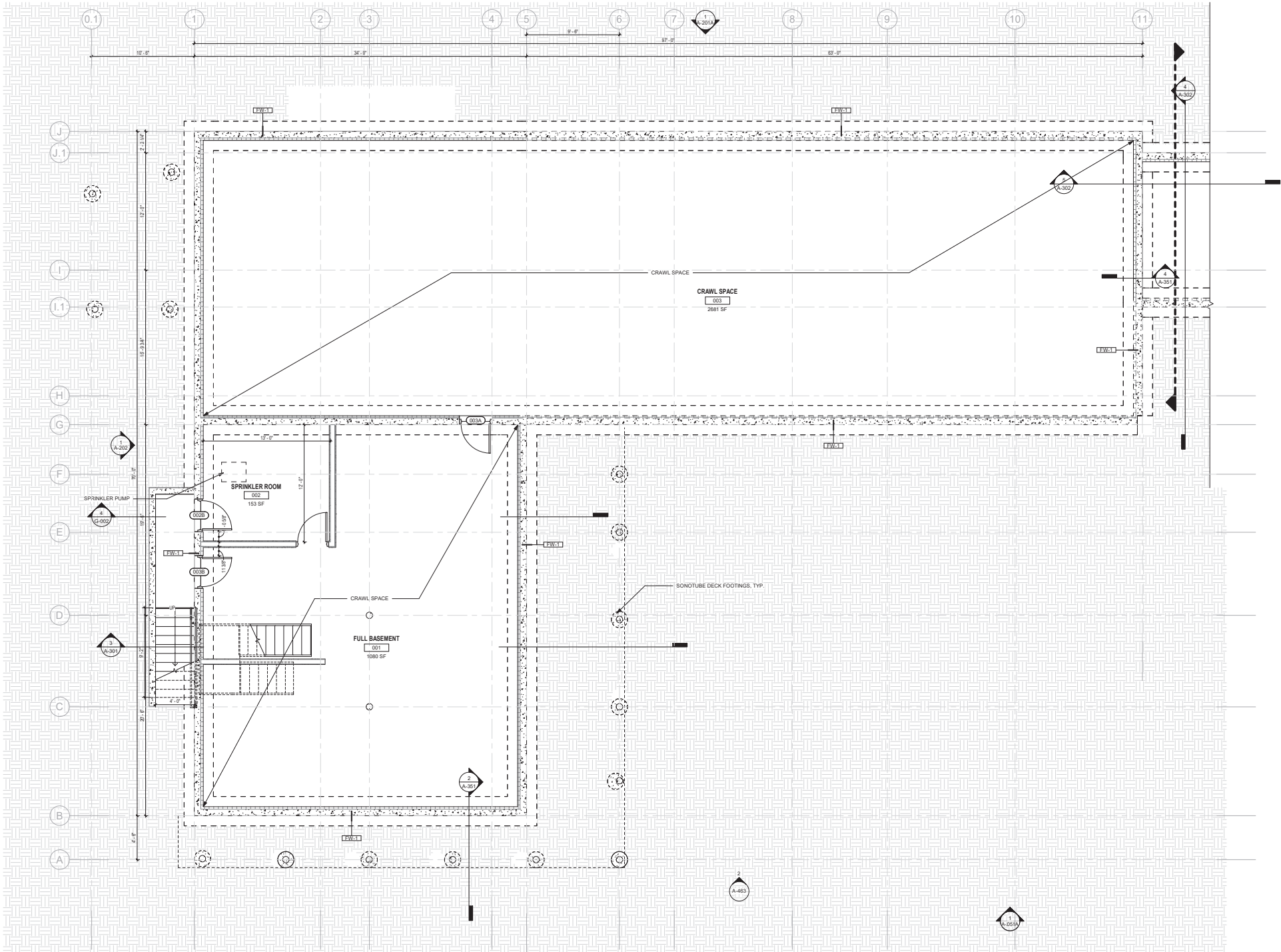
Accessory
76066 Optional T-Grid Mounting Kit¹



1. Can be used with any of the Emergency Battery Backups. Must be ordered as a separate line item.

SECTION 6

REVISED ARCHITECTURAL PLANS



1 NEW ADDITION BASEMENT AND FOUNDATION
1/4" = 1'-0"

- GENERAL NOTES - FLOOR PLAN**
- This is a SD Issuance released for review. Additional information, dimensions, clarifications, and specifications to be requested from the Architect and/or Owner as required to perform the Work.
 - Dimensions are from face of core structure material or to grid, UNO.
 - Specifications: Refer to the following: Assemblies: Refer to Sheet A-400 for Wall, Floor, Roof, Foundation and Ceiling assemblies. Doors and Windows: Refer to Sheet A-410 for Window and door types.
 - MEP: Mechanical, Electrical and Plumbing work is in progress. Refer to MEP sheets for preliminary design. The Architect has made basic assumptions for size and space requirements and locations for incoming service. Contractor is to review and bring to the Architect's attention if additional information is required.
 - Refer to G-001 for additional Document Use Guidelines.
 - Finishes: Ceiling: painted GWB, color TBD
 - Flooring: A. Units: Low-gloss 2x2 Carpet Tile B. Unit Bathrooms: non-slip porcelain tile C. Common areas: non-slip LVT/Marmoleum
 - Millwork: A. Units: Wall-mounted casework with veneer finish as shown on A-705(A-702, cont. solid wood counter to match casework. Kitchenette: Upper and lower cabinets in communal room per plan, painted finish and solid surface countertop.
- KEY NOTES**

WOODHULL
83 High Street
Portland, ME 04101
(207) 263-3777
woodhullmaine.com

**PRELIMINARY
NOT FOR
CONSTRUCTION**

900 Main Street
Canaan, ME 04915

**Country Village
Assisted Living**

**CONSTRUCTION
DOCUMENTS-
PROGRESS
SET**

COUNTRY VILLAGE ASSISTED LIVING ADDITION

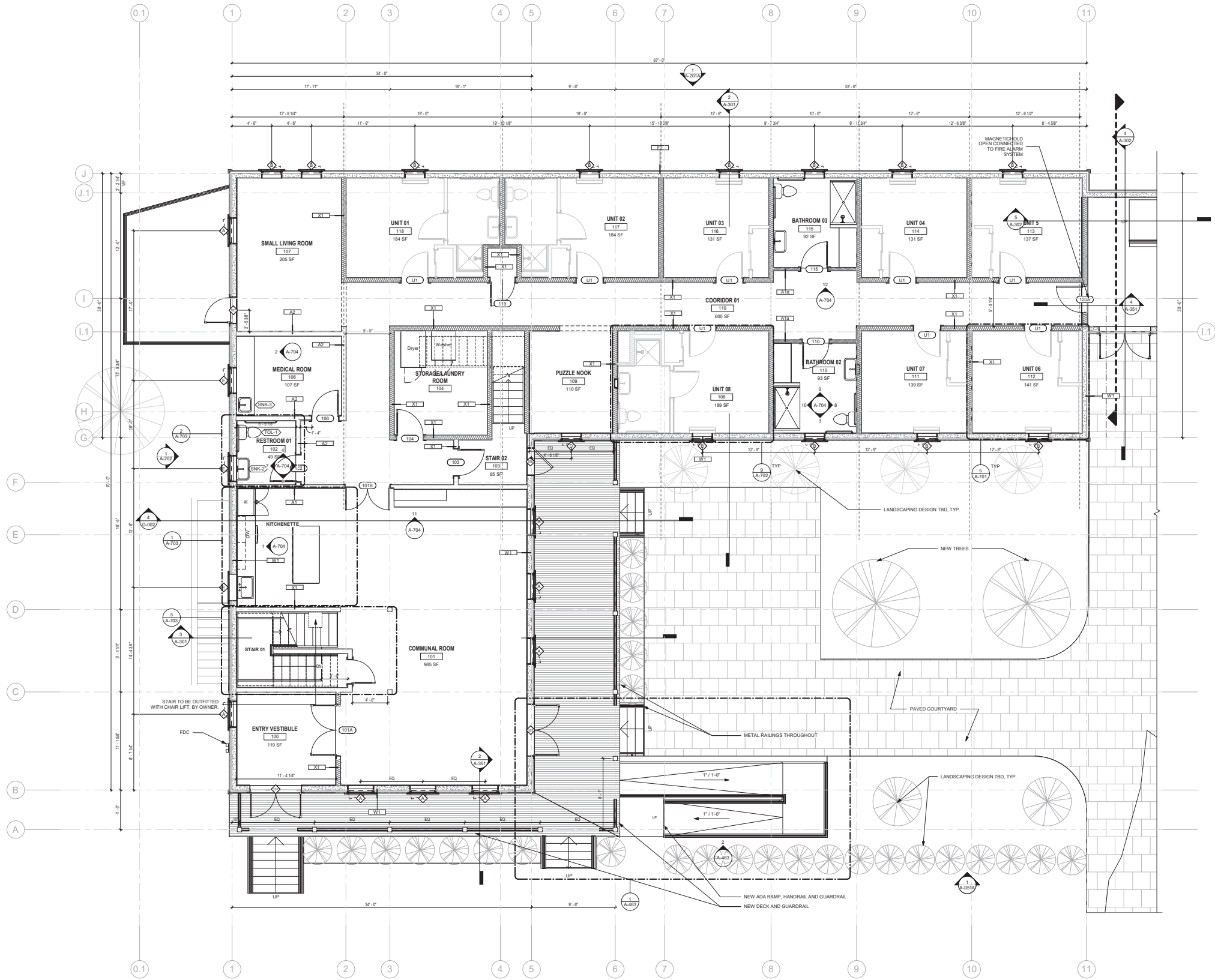
ISSUANCES:

REV	DATE	DESCRIPTION
1	2020-03-13	100% SCHEMATIC DESIGN
2	2020-06-28	50% CONSTRUCTION DOCUMENTS

PROJECT LEAD:
AP

REVIEWED BY:
PB

BASEMENT PLAN



1 NEW ADDITION 1ST FLOOR
1/4" = 1'-0"

- GENERAL NOTES - FLOOR PLAN**
- This is a 3D issuance released for review. Additional information, dimensions, clarifications, and specifications to be requested from the Architect and/or Owner as required to perform the Work.
 - Dimensions are from face of core structure material or to grid, UNO.
 - Specifications: Finishings: Assemblies: Refer to Sheet A-400 for Wall, Floor, Roof, Foundation and Ceiling assemblies. Doors and Windows: Refer to Sheet A-410 for Window and door types.
 - MEP: Mechanical, Electrical and Plumbing work is in progress. Refer to MEP sheets for preliminary design. The Architect has made basic assumptions for size and space requirements and locations for incoming service. Contractor is to review and bring to the Architect's attention if additional information is required.
 - Refer to G-001 for additional Document Use Guidelines.
 - Finishes:
 - A. Units: Low-gloss 2nd Carpet Tile
 - B. Unit Bathrooms: non-slip porcelain tile
 - C. Common areas: non-slip LVT/Marble
 - Milwork:
 - A. Units: Wall-mounted casework with veneer finish as shown on A-701/A-702, cont. solid wood counter to match casework.
 - B. Kitchenette: Upper and lower cabinets in communal room per plan, painted finish and solid surface countertop.
- KEY NOTES**

PRELIMINARY
NOT FOR
CONSTRUCTION

900 Meadow Road
Canaan, ME 04015

Country Village
Assisted Living

CONSTRUCTION
DOCUMENTS-
PROGRESS
SET

COUNTRY VILLAGE ASSISTED LIVING ADDITION

ISSUANCES:

REV	DATE	DESCRIPTION
1	2026 03 13	100% SCHEMATIC DESIGN
2	2026 06 28	50% CONSTRUCTION DOCUMENTS

PROJECT LEAD:

AP

REVIEWED BY:

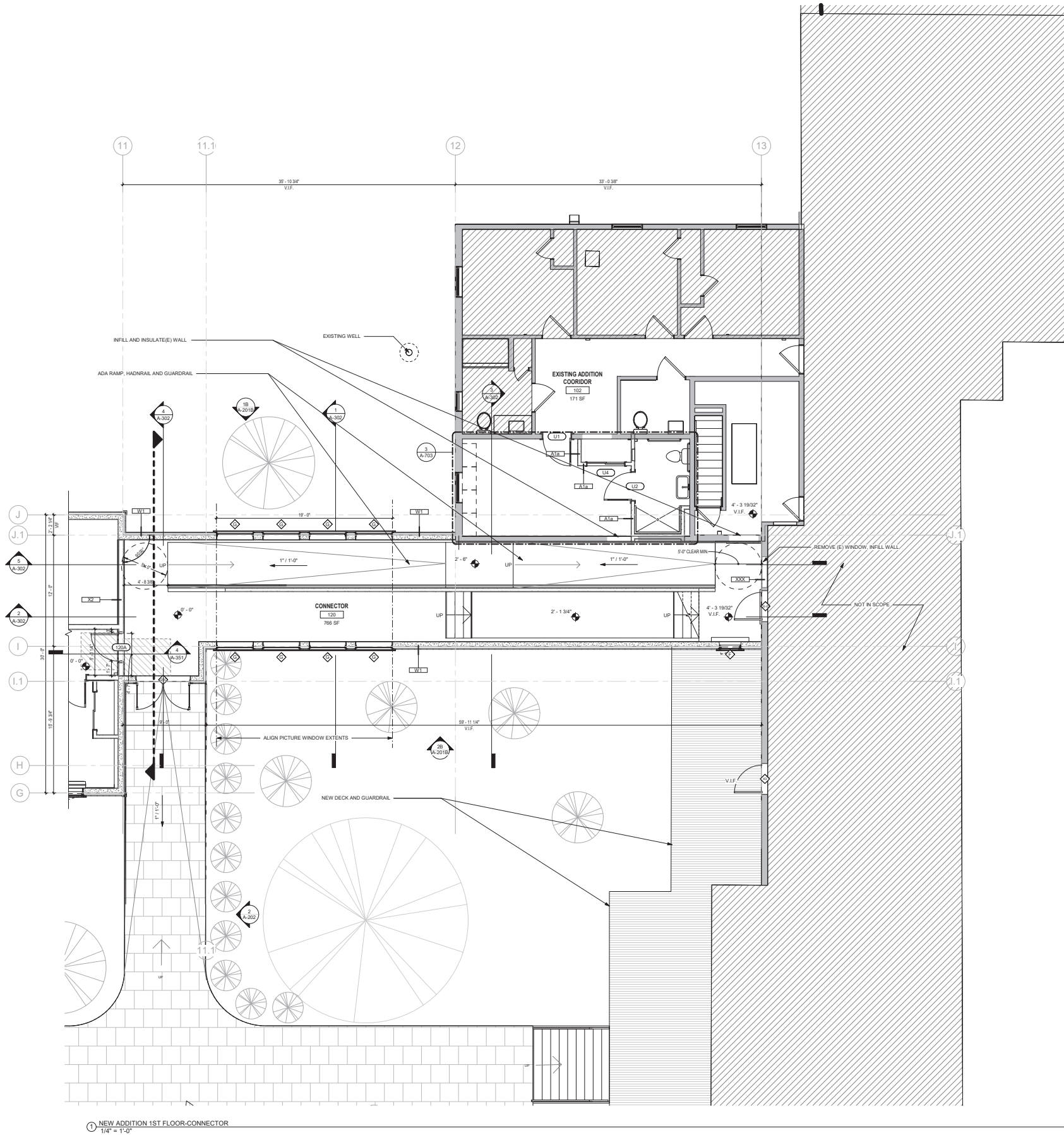
PB

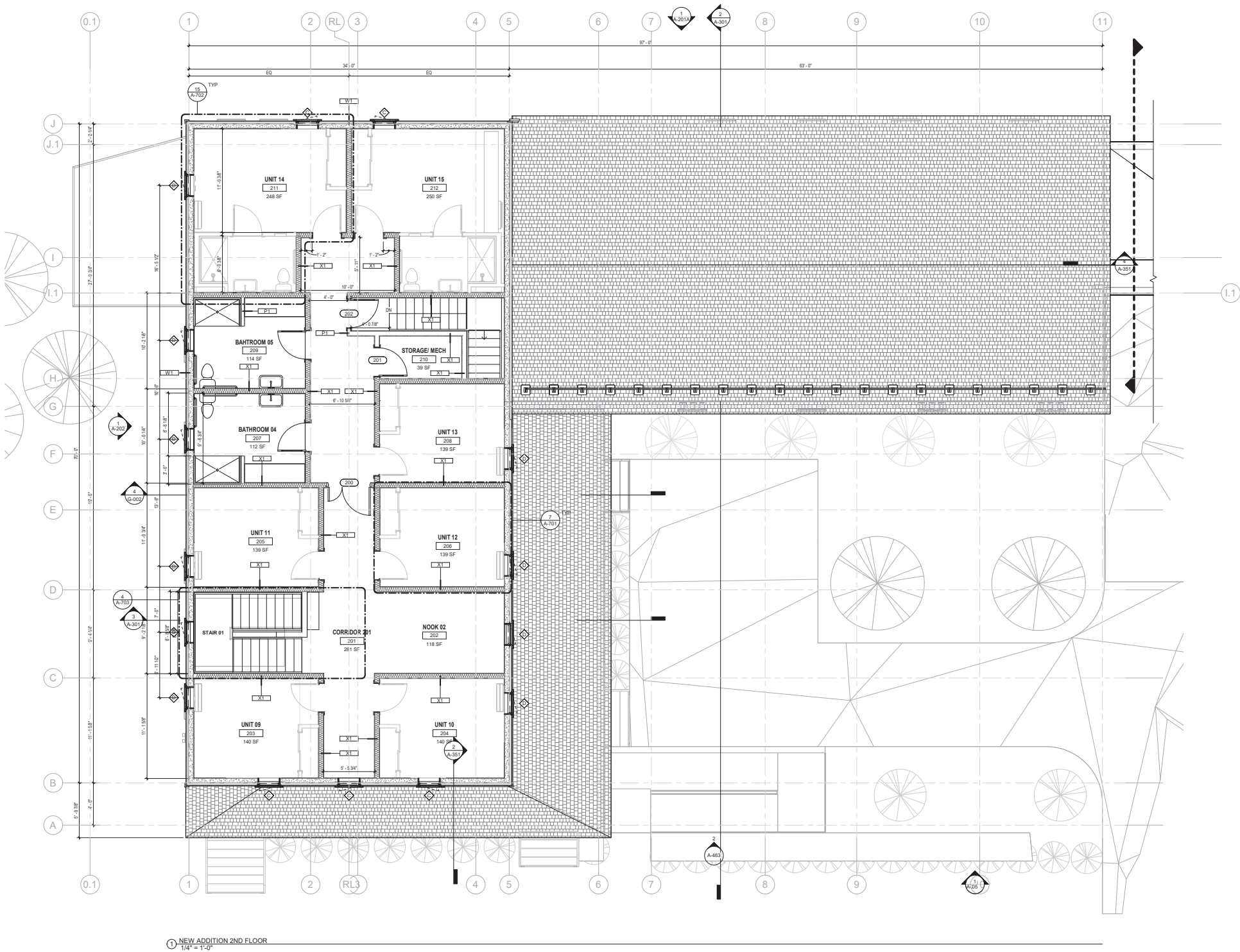
FIRST FLOOR PLAN

A-101A

WOODHULL
90 High Street
Portland, ME 04101
(207) 263-3777
woodhullmaine.com

GENERAL NOTES - FLOOR PLAN	
1.	This is a SD issuance release.
2.	Additional information, dimensions and specifications to be req for Architect and/or Owner as per drawings.
3.	Dimensions are from face or to or end, UNO.
4.	Specifications: Forthcoming.
5.	Assemblies: Refer to Schedule.
6.	Ref. Foundation and Ceiling.
7.	Doors and Windows: Refer.
8.	Window door types.
9.	MEP: Mechanical, Electrical, Plumbing in progress. Refer to MEP drawings.
10.	Architect's attention is required for size and space required for incoming service. Contractor to refer to Architect's attention if required.
11.	Refer to G-001 for additional drawings.
12.	Guidelines.
13.	Painting: Refer to G-001.
14.	Ceiling: painted GWB, colour as per Floorings.
15.	Units: Low-price 2x2 ft.
16.	Unit Bathrooms: none.
17.	Common Areas: none.
18.	Units: Wall-mounted.
19.	finish as shown on unit.
20.	solid wood counter top.
21.	Kitchenette: up to 10 sq. ft.
22.	communal room for 2 persons.
23.	and solid surface counter top.





- GENERAL NOTES - FLOOR PLAN:**
- This is a SD Issuance released for review. Additional information, dimensions, clarifications, and specifications to be requested from the Architect and/or Owner as required to perform the Work.
 - Dimensions are from face of core structure material or to grid, UNO.
 - Specifications: Forthcoming.
 - Assemblies: Refer to Sheet A-400 for Wall, Floor, Roof, Foundation and Ceiling assemblies.
 - Doors and Windows: Refer to Sheet A-410 for Window and door types.
 - MEP: Mechanical, Electrical and Plumbing work is in progress. Refer to MEP sheets for preliminary design. The Architect has made basic assumptions for size and space requirements and locations for incoming service. Contractor is to review and bring to the Architect's attention if additional information is required.
 - Refer to G-001 for additional Document Use Guidelines.
 - Finishes:
 - Ceiling: painted GWB, color TBD
 - Flooring:
 - A. Units: Low-vile 2x2 Carpet Tile
 - B. Unit Bathrooms: non-slip porcelain tile
 - C. Common areas: non-slip LVT/Marmoleum
 - Millwork:
 - A. Units: Wall-mounted casework with veneer finish as shown on A-701/A-702, cont. solid wood counter to match casework
 - B. Kitchenette: Upper and lower cabinets in communal room per plan, painted finish and solid surface countertop

KEY NOTES

PRELIMINARY
NOT FOR
CONSTRUCTION

900 Meadow Road
Canaan, ME 04015

Country Village
Assisted Living

CONSTRUCTION
DOCUMENTS
PROGRESS
SET

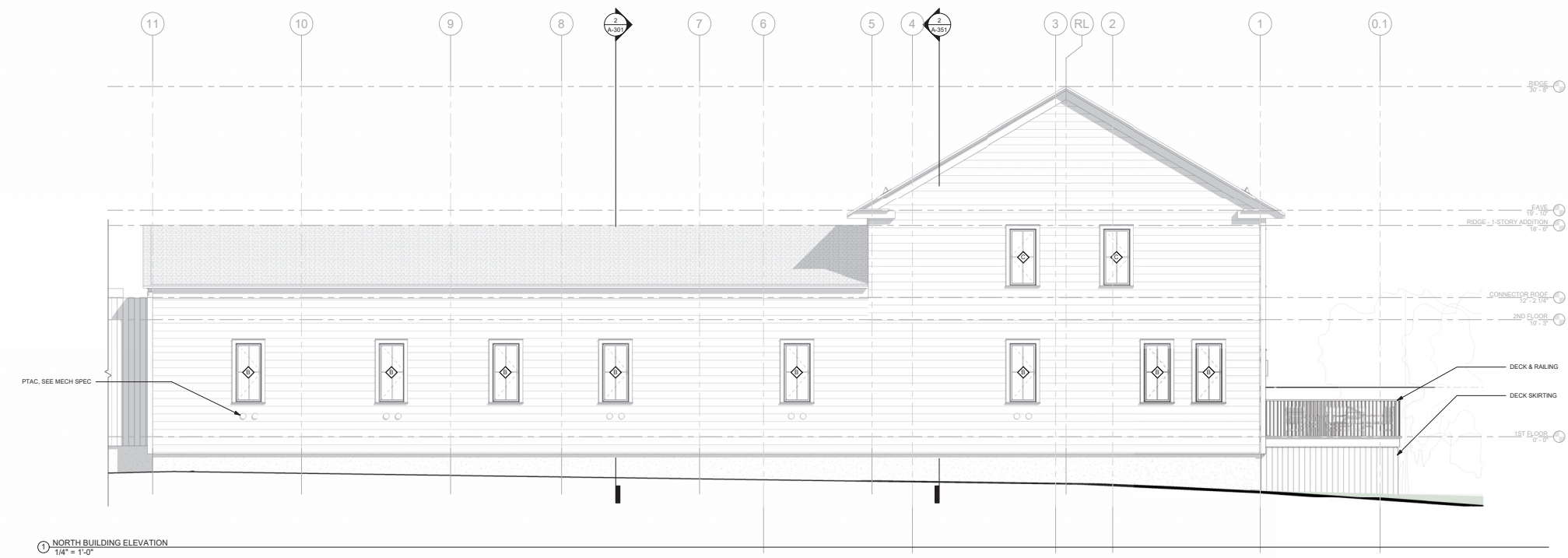
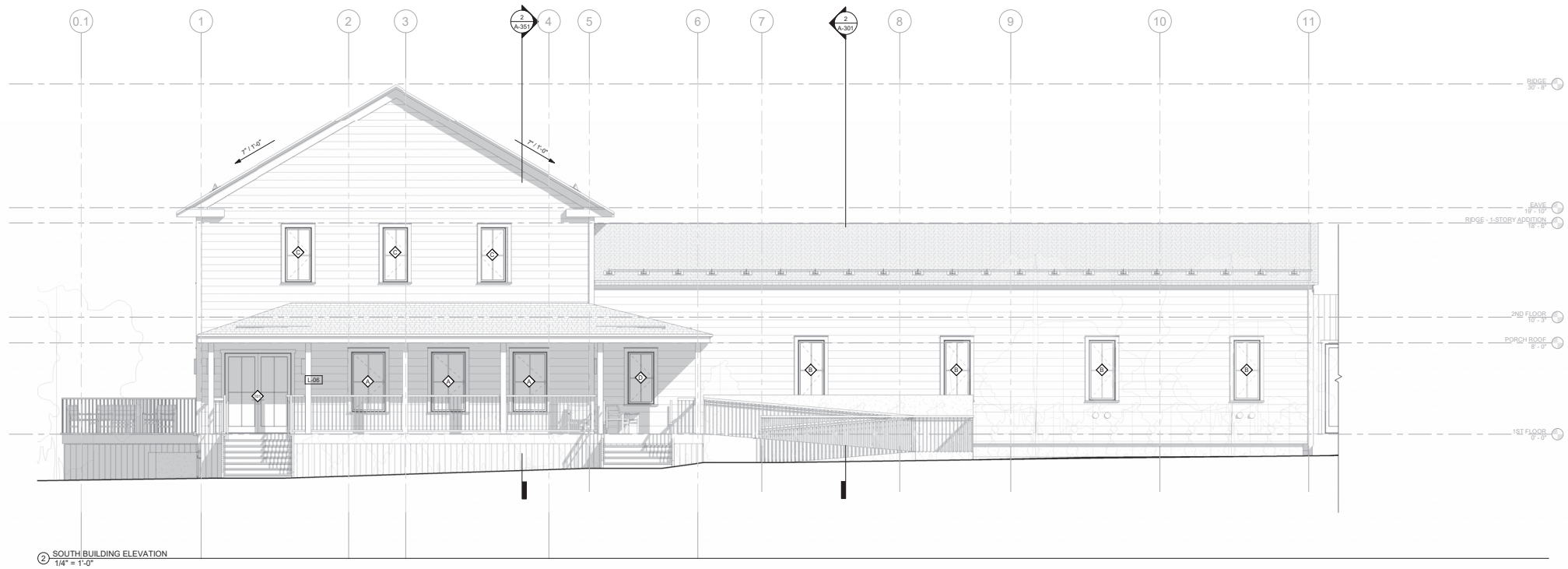
COUNTRY VILLAGE
ASSISTED LIVING
ADDITION

ISSUANCES:

REV	DATE	DESCRIPTION
1	2026 03 13	100% SCHEMATIC DESIGN
2	2026 06 26	50% CONSTRUCTION DOCUMENTS

PROJECT LEAD:
AP
REVIEWED BY:
PB

SECOND FLOOR PLAN



COUNTRY VILLAGE
ASSISTED LIVING
ADDITION

ISSUANCES:

REV	DATE	DESCRIPTION
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2	2020 06 28	50% CONSTRUCTION DOCUMENTS

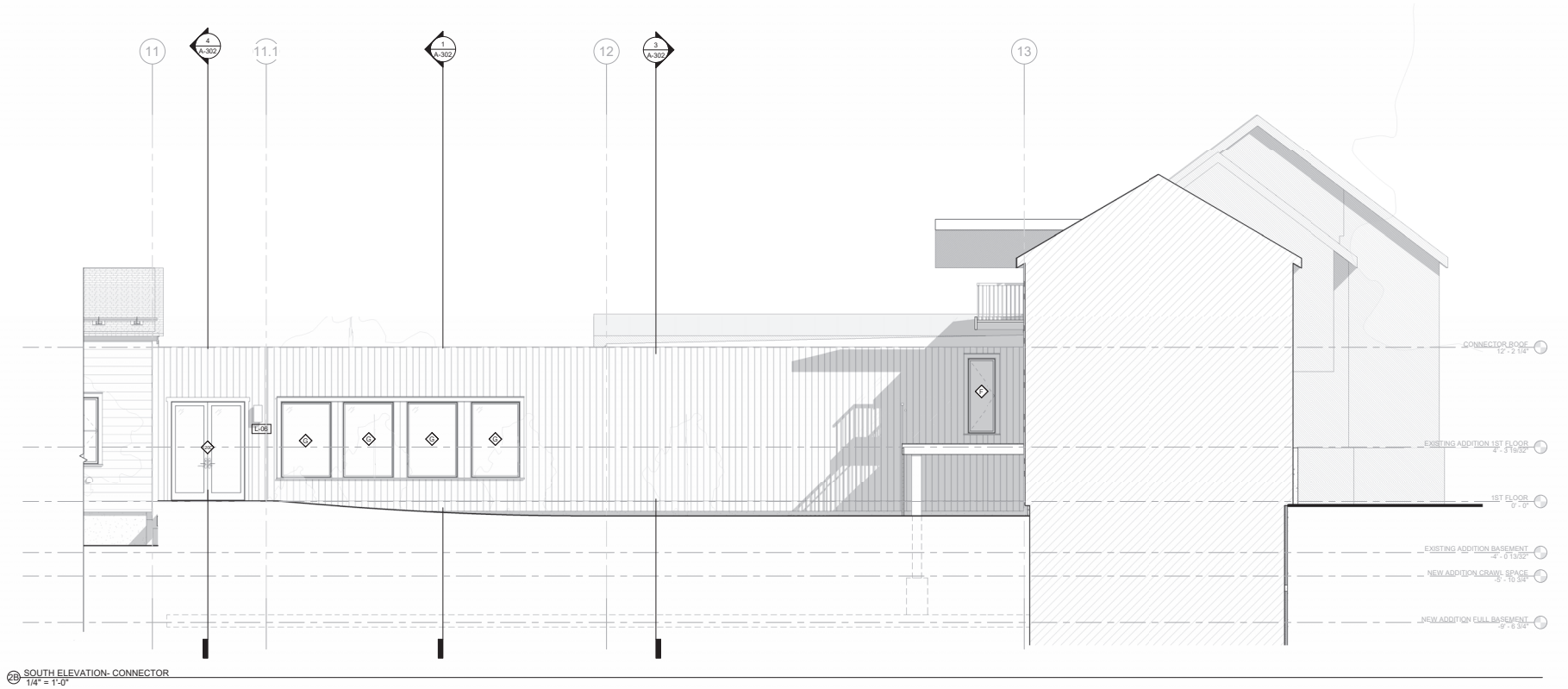
PROJECT LEAD:

AP

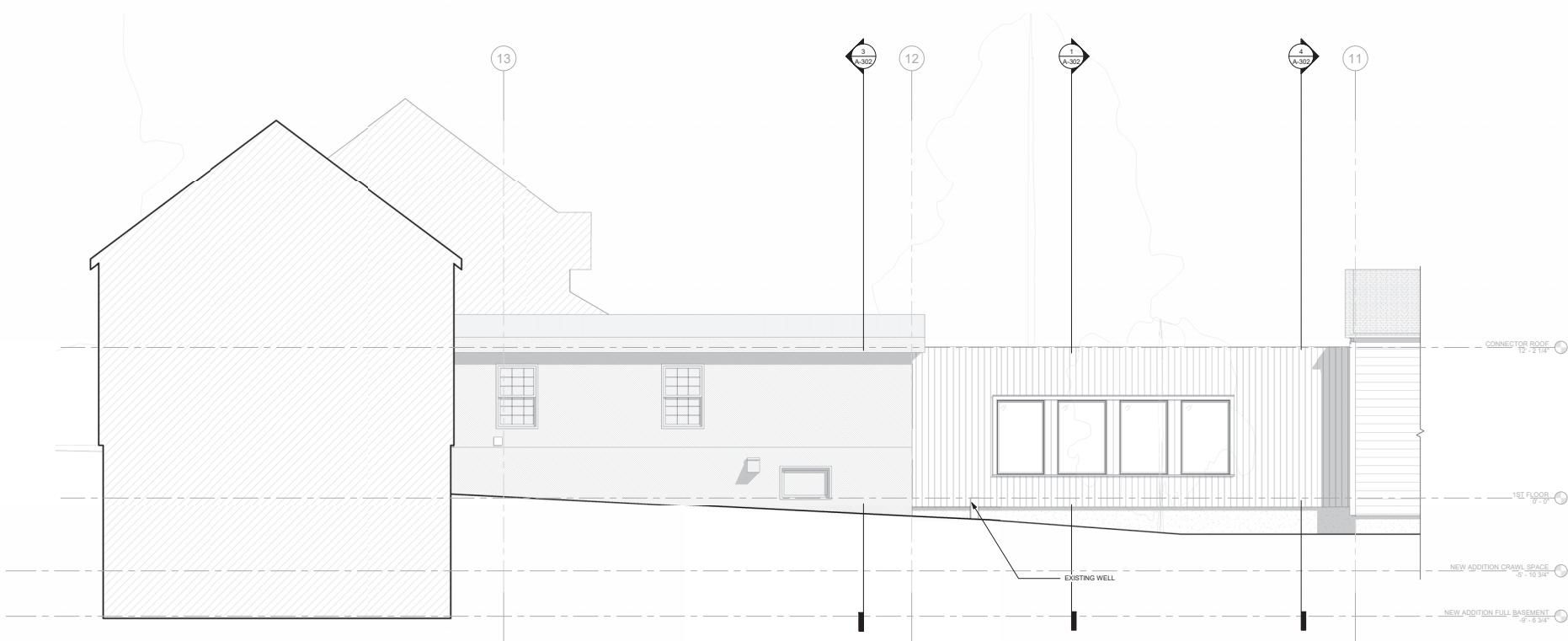
REVIEWED BY:

PB

EXTERIOR ELEVATIONS



16 SOUTH ELEVATION-CONNECTOR
1/4" = 1'-0"



16 NORTH ELEVATION-CONNECTOR
1/4" = 1'-0"

PRELIMINARY
NOT FOR
CONSTRUCTION

900 Meadow Road
Casco, ME 04015

Country Village
Assisted Living

CONSTRUCTION
DOCUMENTS-
PROGRESS
SET

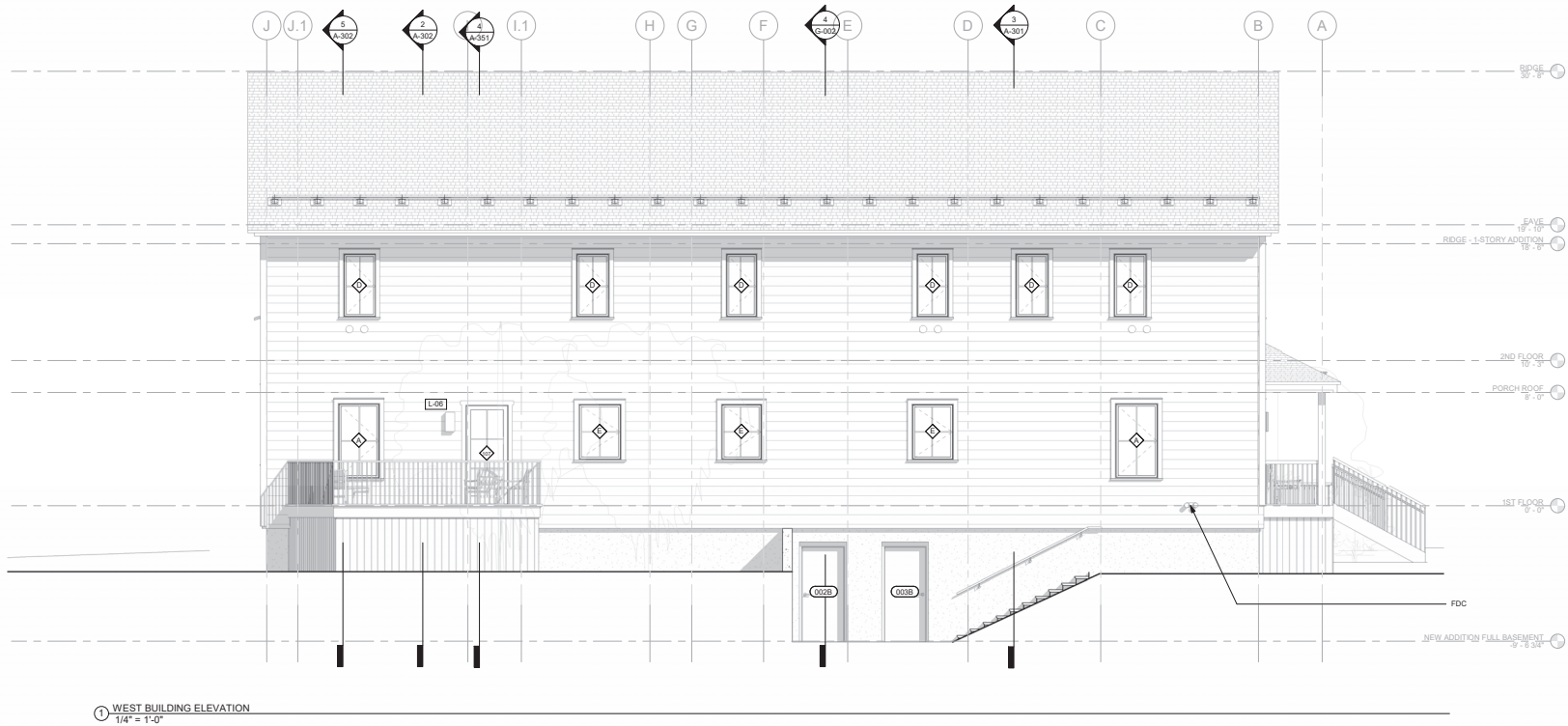
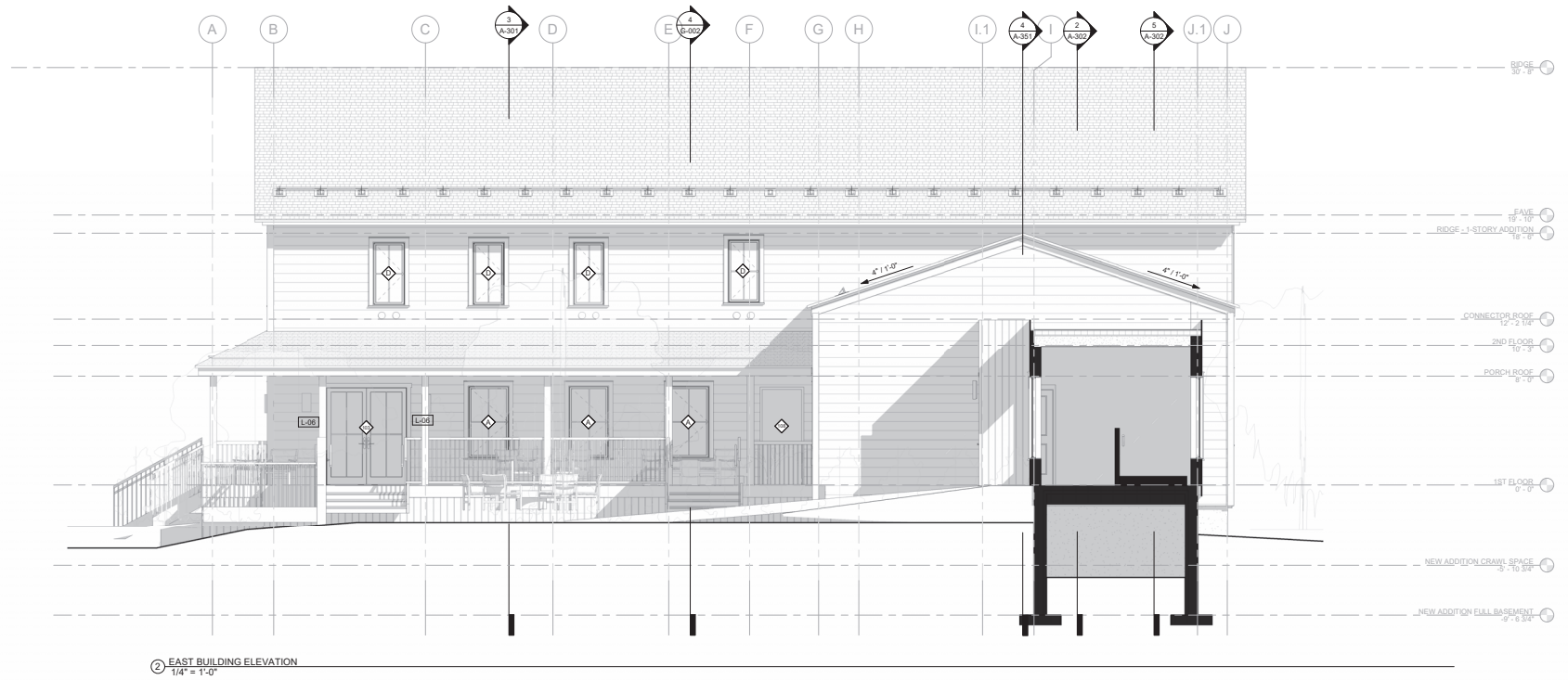
COUNTRY VILLAGE
ASSISTED LIVING
ADDITION

ISSUANCES:

REV	DATE	DESCRIPTION
1	2020 03 13	100% SCHEMATIC DESIGN
2	2020 06 28	50% CONSTRUCTION DOCUMENTS

PROJECT LEAD:
AP
REVIEWED BY:
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EXTERIOR
ELEVATIONS-CONNECTOR



PRELIMINARY
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900 Meadow Road
Casco, ME 04015

Country Village
Assisted Living

CONSTRUCTION
DOCUMENTS
PROGRESS
SET

COUNTRY VILLAGE ASSISTED LIVING ADDITION

ISSUANCES:

REV	DATE	DESCRIPTION
1	2025 03 13	100% SCHEMATIC DESIGN
2	2025 06 28	50% CONSTRUCTION DOCUMENTS

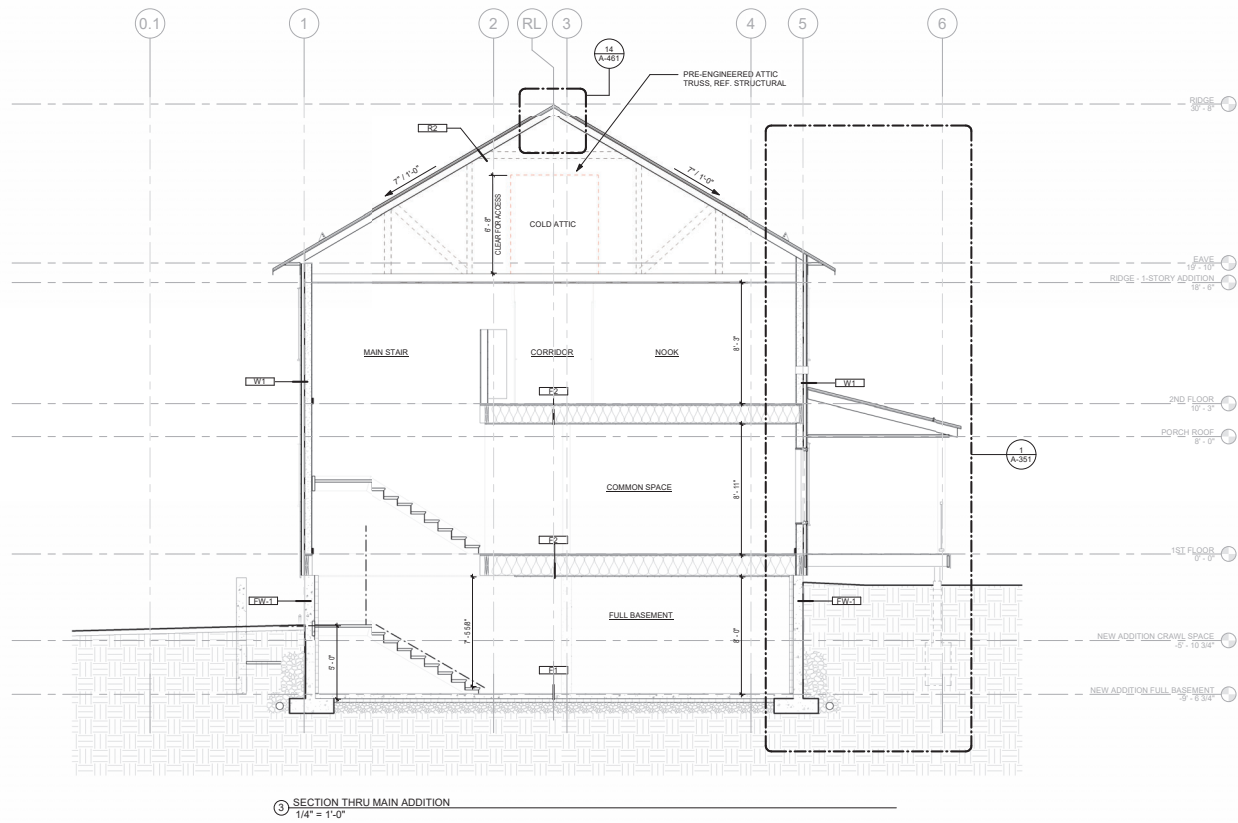
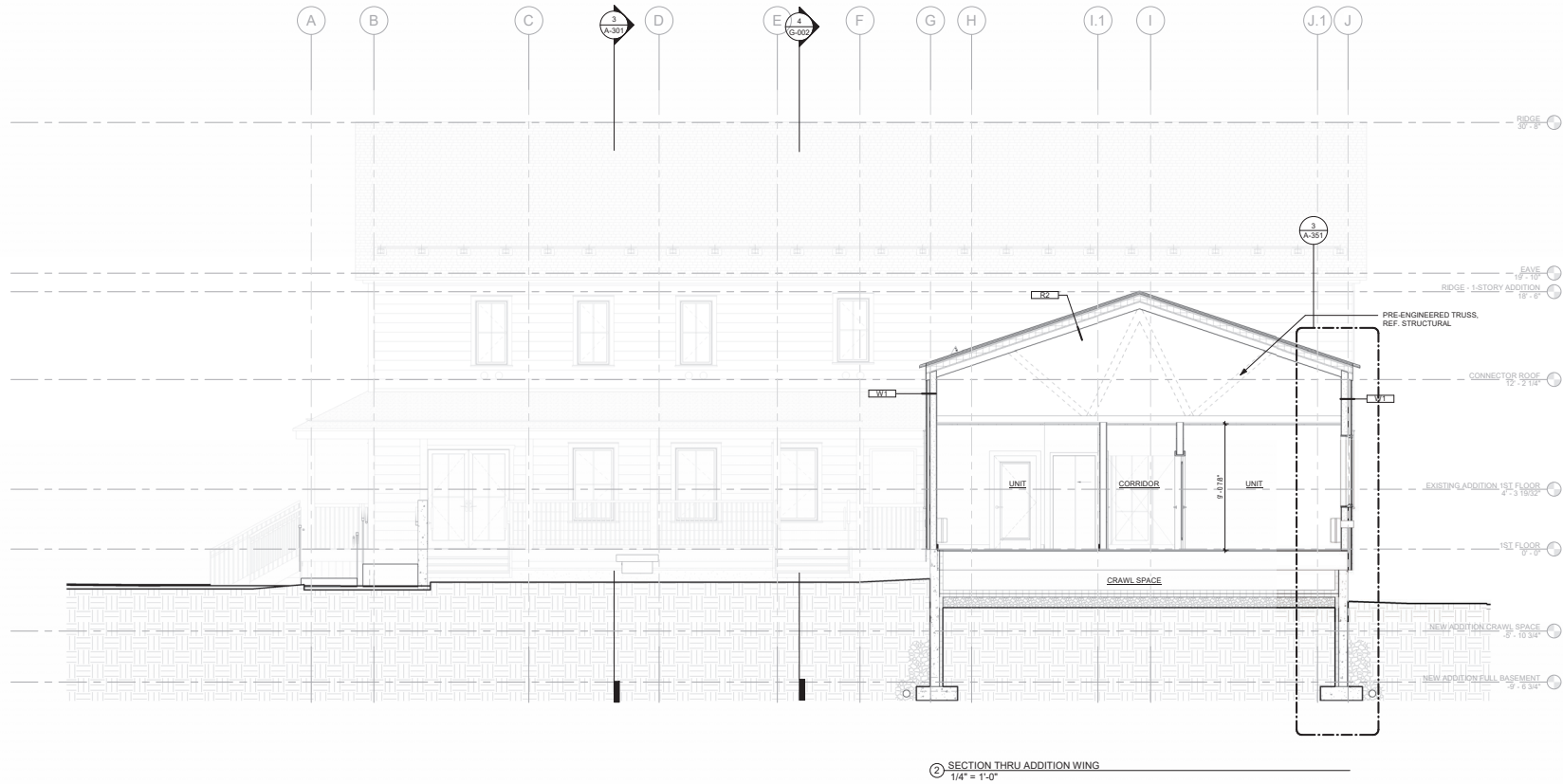
PROJECT LEAD:

AP

REVIEWED BY:

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EXTERIOR ELEVATIONS



PRELIMINARY
NOT FOR
CONSTRUCTION

900 Meadow Road
Casco, ME 04015

Country Village
Assisted Living

CONSTRUCTION
DOCUMENTS
PROGRESS
SET

COUNTRY VILLAGE ASSISTED LIVING ADDITION

ISSUANCES:

REV	DATE	DESCRIPTION
1	2025 03 13	100% SCHEMATIC DESIGN
2	2025 06 26	50% CONSTRUCTION DOCUMENTS

PROJECT LEAD:

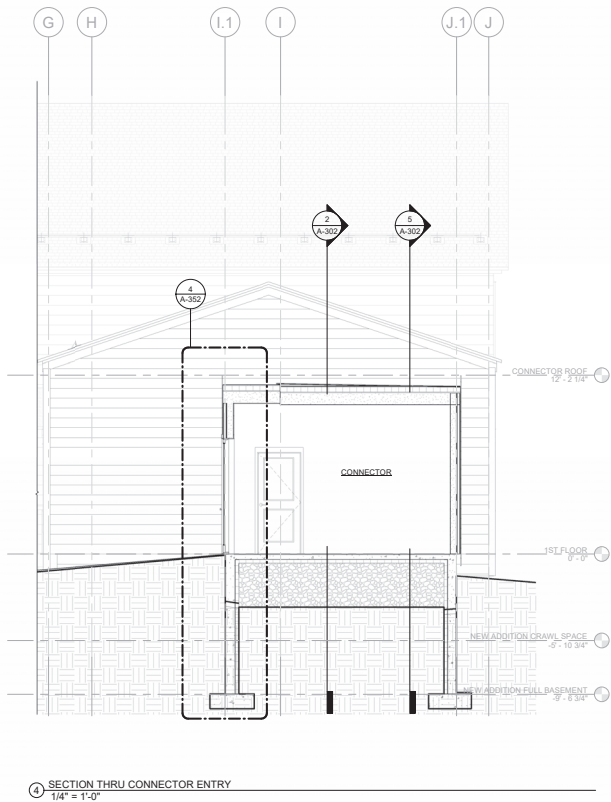
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REVIEWED BY:

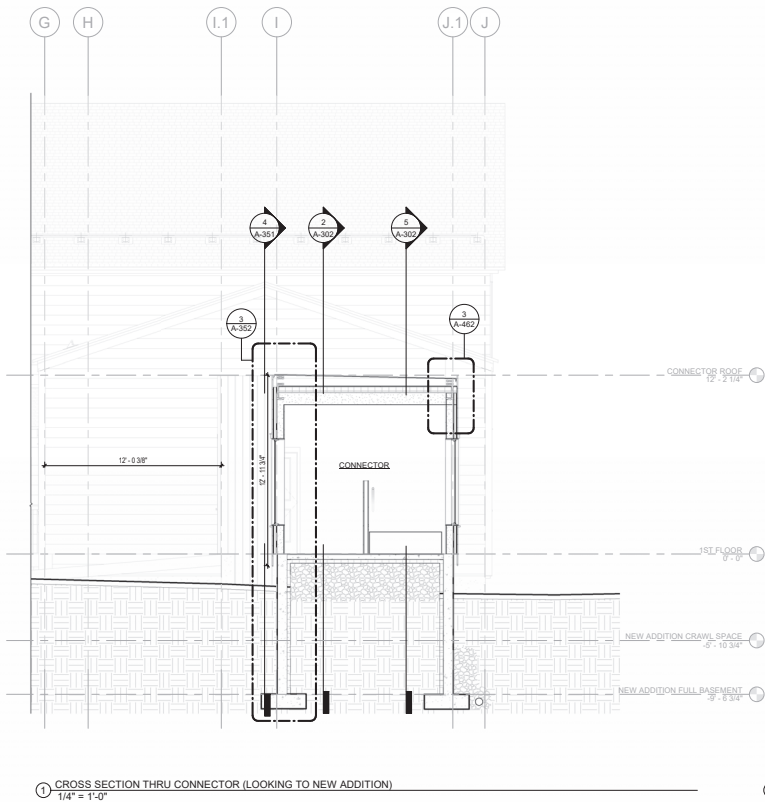
PB

BUILDING SECTIONS

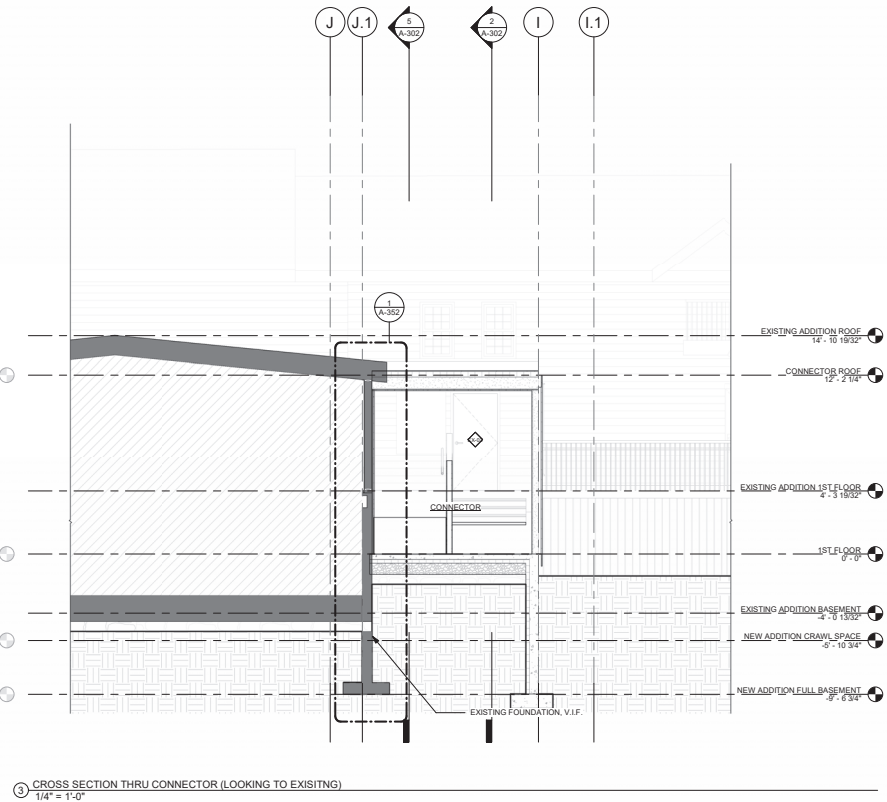
6/19/2024 4:56:45 PM



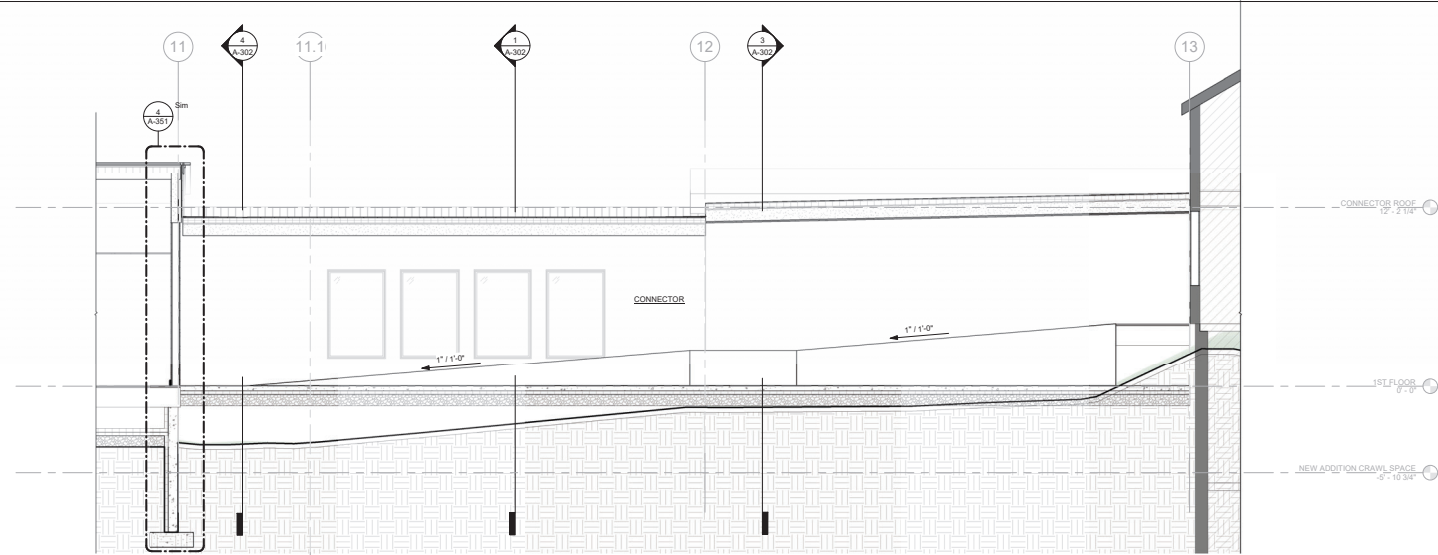
① SECTION THRU CONNECTOR ENTRY
1/4" = 1'-0"



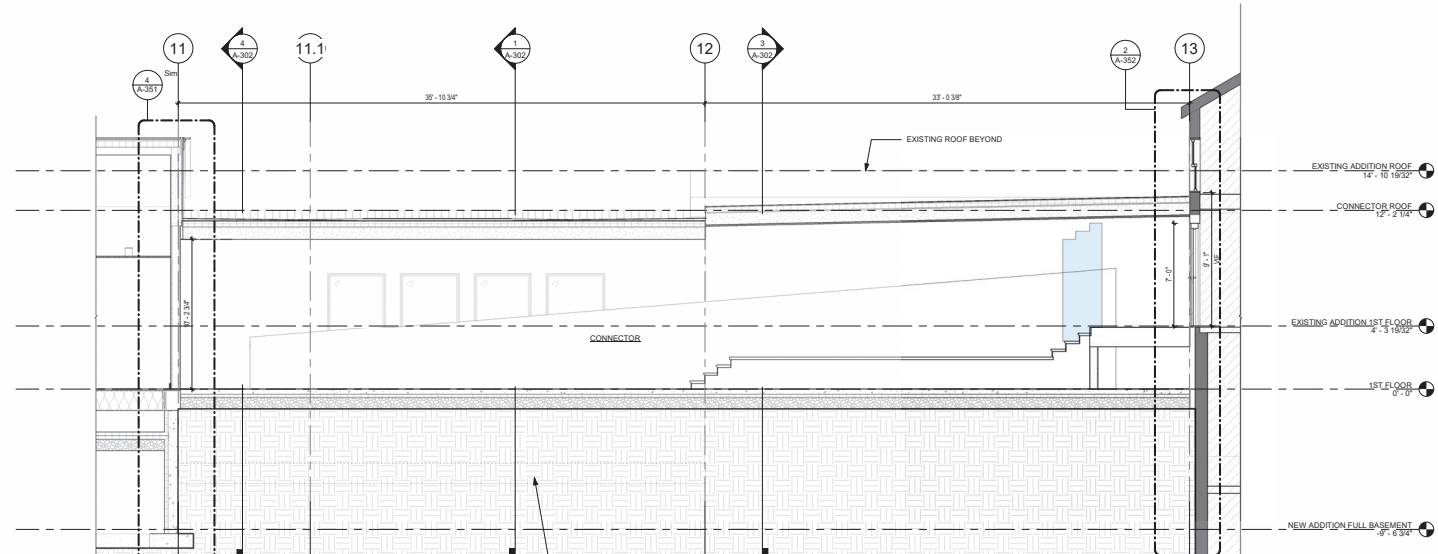
② CROSS SECTION THRU CONNECTOR (LOOKING TO NEW ADDITION)
1/4" = 1'-0"



③ CROSS SECTION THRU CONNECTOR (LOOKING TO EXISTING)
1/4" = 1'-0"



④ LONGITUDINAL SECTION THRU CONNECTOR RAMP
1/4" = 1'-0"



⑤ LONGITUDINAL SECTION THRU CONNECTOR STAIR
1/4" = 1'-0"

PRELIMINARY
NOT FOR
CONSTRUCTION

900 Main Street
Cassco, ME 04015

Country Village
Assisted Living

CONSTRUCTION
DOCUMENTS
PROGRESS
SET

COUNTRY VILLAGE ASSISTED LIVING ADDITION

ISSUANCES:

REV	DATE	DESCRIPTION
1	2024 06 28	ISS CONSTRUCTION DOCUMENTS

PROJECT LEAD:
AP
REVIEWED BY:
PB

BUILDING
SECTIONS-CONNECTOR