

STORMWATER MANAGEMENT REPORT

LONGFIELD ESTATES

AP 103 LOT 2 & 14
1200 & 1202 HOPE STREET
BRISTOL, RI

JUNE, 2022

Revised: December 9, 2022

Prepared for:

MI 1200 HOPE ST., LLC.
133 OLD TOWER HILL ROAD, SUITE 1
WAKEFIELD, RI 02879

Prepared by:

Land Development Engineering & Consulting, LLC
207 High Point Ave., Unit 6
Portsmouth, RI 02871



207 High Point Avenue, Unit #6
Portsmouth, RI 02871
T: 401-354-2050 | F: 401-369-9775
EMAIL: mrussell@sde-ldec.com

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207 High Point Avenue, Unit #6
Portsmouth, RI 02871
T: 401-354-2050 | F: 401-369-9775
EMAIL: mrussell@sde-ldec.com

INTRODUCTION

This report was prepared to address the Stormwater Management System (SMS) for the Residential Development on AP 103 Lot 2 & 14, 1200 & 1202 Hope Street in Bristol. The project proposes construct four new residential buildings that will have a total of 8 residential units between them. The existing residential dwelling (Longfield) shall be renovated and will provide an additional 2 residential units. The Development will create 10 parking spaces with associated landscaping, utilities & stormwater management system. This report will outline and summarize the current SMS as well the improvements of the proposed development. Said improvements are intended to control peak runoff & volume rates for the added impervious area(s). Additionally, the proposed SMS will provide, at minimum, the water quality & stormwater recharge volumes as required. The SMS as proposed will comply with the Town of Bristol's & State of Rhode Island's Stormwater Management Policy (2018).

EXISTING CONDITIONS (SUMMARY)

The subject property is approximately 1.76 acres in area and fronts along Hope Street (Route 114). The existing ground cover consists mainly of a lawn with brush/woods along the edges of the property. There is also an existing dwelling and a gravel driveway for the dwelling. The site slopes generally towards the South/Southeast towards an existing wetland. The elevation change across the site is approximately 8 feet. Currently, most of the Runoff from the site sheet flows across the site to an isolated wetland in the southeastern corner of the lot. Some runoff in the Northwestern edge of the lot flows Southwest towards Hope Street (Route 114). The summary of the Pre-Development Analysis is located in Appendix 1 of this report.

PROPOSED CONDITIONS (SUMMARY)

The project proposes construct four new residential buildings that will have a total of 8 residential units between them. The existing residential dwelling (Longfield) shall be renovated and will provide an additional 2 residential units. The Development will create 10 parking spaces with associated landscaping, utilities & stormwater management system. Site grading will create the cuts & fills throughout the project limits to create the desired site layout and function and will maintain pre-development runoff patterns towards the Southeastern portion of the site. A stormwater collection system will be incorporated into the newly designed parking area to collect runoff from impervious surfaces and landscaped areas within the limit of disturbance. This system consists of dry swales, sediment forebays, lined bio-retention basins, and a dry extended detention basin. The forebays will provide pre-treatment for the bio-retention basins. The system will accommodate a 100-year storm event, handle the water quality volume (WQv) and provide for total suspended solids (TSS) removal. This overall system is designed to accommodate, at minimum, the water quality volume (WQv) required for new development projects. Low impact development practices (LID) will be employed to the maximum extent practicable. Summary of the Post-Development Conditions Analysis is located in Appendix 1 of this report.

SITE SOIL & GROUNDWATER CONDITIONS

The underlying watershed soils within the developed area consist of the Pittstown silt loam Hydrologic Group C). A soil evaluation and groundwater determination was performed in



207 High Point Avenue, Unit #6
Portsmouth, RI 02871
T: 401-354-2050 | F: 401-369-9775
EMAIL: mrussell@sde-ldc.com

December 2018. Soil textures below the fill consisted primarily of a silt loam mixture with groundwater elevation approximately 12" to 32" feet below original grade.

METHODOLOGY

HydroCad® Stormwater Modeling System was used to quantify stormwater runoff generated by WQv, 1-year, 2-year, 10-year and 25-year design storms in pre and post development conditions. The calculations were performed using "Dynamic Storage-Indication" to also analyze the impact of the pipe size, material and slope selection in upstream structures. The HydroCad® program utilizes Natural Resource Conservation Service (NRCS) techniques (TR-20) to predict stormwater runoff for given design storms. The calculations performed by HydroCad® are based on the NRCS model return frequency Type III distribution and a user specified design storms. The calculation is also performed using the simple dynamic method which utilizes *Rawls Rate* for infiltration based on soil texture.

The analysis is performed by modeling the drainage area as subcatchments and ponds. A subcatchment is an area that produces runoff and drains into a pond. A pond can be a natural depression, wetland, or manmade structure that detains or retains stormwater runoff. The drainage network pipe design adequacy is evaluated by integrating it in the HydroCad® pond model for drainage structures. The pipes are modeled as the pond outlet-culvert type. Manning's Equation and/or Hazen-Williams hydraulic equations were also utilized to determine the required pipe sizes as well as minimum and maximum pipe slopes.

DRAINAGE SYSTEM MODEL

The proposed development is analyzed by creating an existing condition or pre-development model and a full build-out or post-development model. The models were created to compare the existing and post-development runoff to the abutting properties and existing SMS. The post-development analysis results are also utilized to adequately size the proposed practices. Analysis within the site was performed using WQv, 1, 2, 10 and 25-year design storm projections. All excess stormwater runoff captured on site will be treated and retained/recharged on site. On site post-development runoff rates flowing overland toward abutting properties will not exceed pre-development runoff rates up to the 25-year event.

The pre-development HydroCad® model within the site consists of two subcatchments, One subcatchment corresponds to the Northwestern portion of the lot that flows southwest towards Hope Street (Route 114). The other subcatchment encompasses the rest of the lot that flows Southeasterly towards the isolated wetland in the corner of the lot. This was created to determine the existing stormwater runoff originating from the site flowing to Hope Street and the to the existing wetland.

Post-development subcatchment models were created for the site. Subcatchment models represent the drainage areas to each of the proposed reaches, drainage structures, or storage & treatment areas. Each of these areas provides elements of treatment, storage, and infiltration in order to effectively mitigate flows to the Point of Analysis in each analyzed storm event. See Post-Development output in Appendix B and summary at the end of this narrative.



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The majority of the paved & landscaped areas for the developed area discharge into a dry swale or sediment forebay before flowing into a bio-retention basin and a detention basin.

STORMWATER TREATMENT

Stormwater runoff will be treated through the use of Best Management Practices (BMP's). The BMP's used within the proposed development include dry-swales, forebays, bio-retention basins, underground storage systems, underground infiltration systems and a dry extended detention basin. These BMP's will aid in the removal of pollutants within the stormwater runoff as well as provide recharge to the groundwater aquifer.

This system will provide pollutant removal and treatment as required for this development.

ANALYSIS DATA

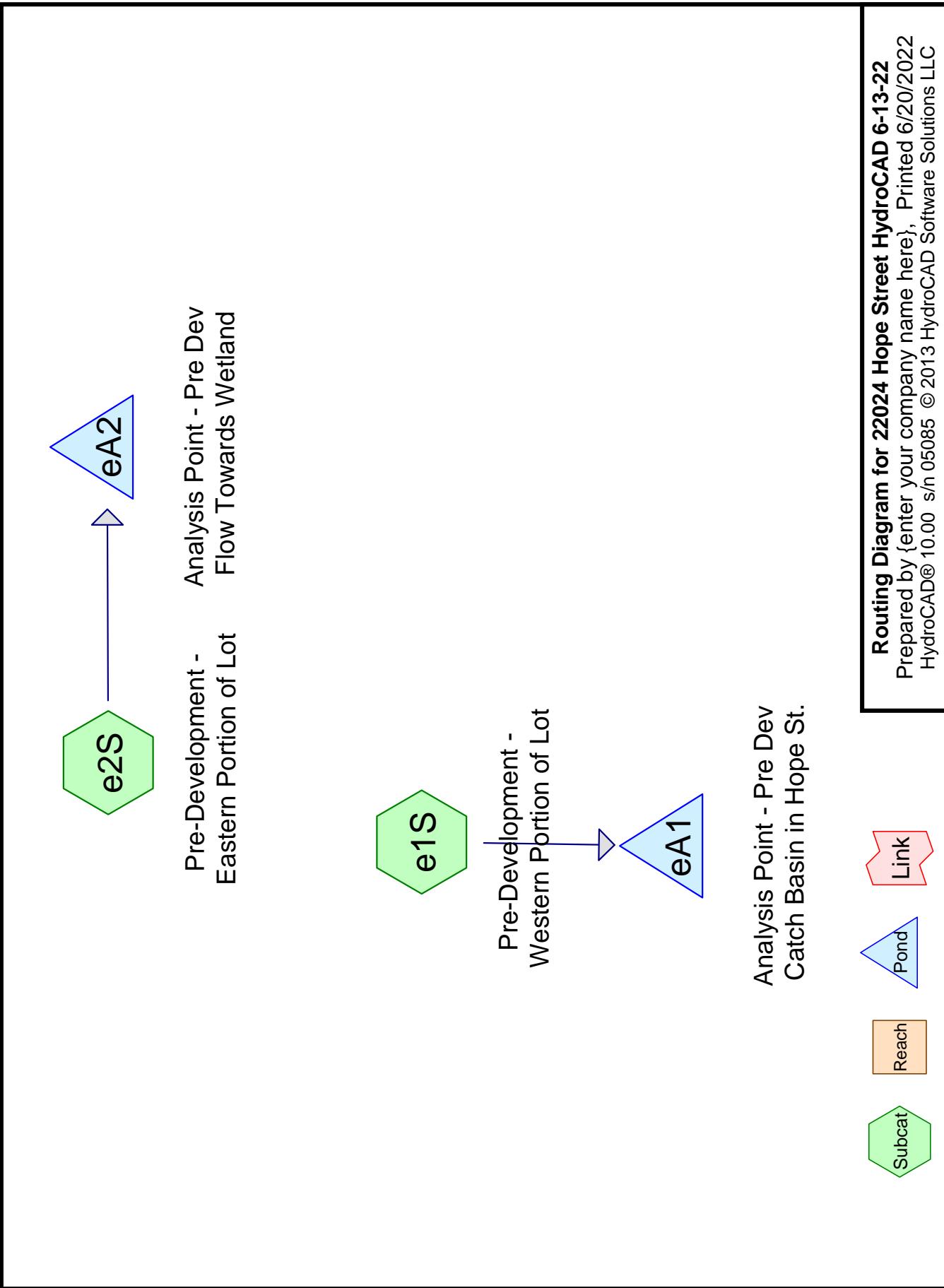
The following information was used in performing the calculations for the drainage system.

RUNOFF SUMMARY AT ANALYSIS POINTS		
Cover Description		
Cover Type	Hydrologic condition	Curve Number (Class: C)
Landscaping, Lawns	Good	74
Woods	Good	70
Gravel	Good	89
Brush	Good	65
Buildings	-	98
Pavement	-	98
Brick Walkways	-	98

Rainfall Data (Type III - 24 Hour Storm Duration*)

Storm Event	Rainfall
WQv	1.2 inches
1 - Year	2.8 inches
2 - Year	3.3 inches
10 - Year	4.9 inches
25 - Year	6.1 inches
100 - Year	8.6 inches





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Area Listing (selected nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
64,639	74	>75% Grass cover, Good, HSG C (e1S, e2S)
7,182	65	Brush, Good, HSG C (e1S, e2S)
2,708	96	Gravel surface, HSG C (e1S, e2S)
1,919	98	Paved parking, HSG C (e2S)
2,892	98	Roofs, HSG C (e2S)
79,340	75	TOTAL AREA

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Type III 24-hr 1-yr Rainfall=2.80"
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Time span=0.00-72.00 hrs, dt=0.04 hrs, 1801 points

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment e1S: Pre-Development - Western Portion of Lot

Runoff Area=9,269 sf 0.00% Impervious Runoff Depth=0.88"
Flow Length=163' Tc=6.8 min CN=76/0 Runoff=0.20 cfs 682 cf

Subcatchment e2S: Pre-Development - Eastern Portion of Lot

Runoff Area=70,071 sf 6.87% Impervious Runoff Depth=0.91"
Flow Length=366' Tc=18.3 min CN=74/98 Runoff=1.08 cfs 5,293 cf

Pond eA1: Analysis Point - Pre Dev Catch Basin in Hope St.

Inflow=0.20 cfs 682 cf
Primary=0.20 cfs 682 cf

Pond eA2: Analysis Point - Pre Dev Flow Towards Wetland

Inflow=1.08 cfs 5,293 cf
Primary=1.08 cfs 5,293 cf

Total Runoff Area = 79,340 sf Runoff Volume = 5,975 cf Average Runoff Depth = 0.90"
93.94% Pervious = 74,529 sf 6.06% Impervious = 4,811 sf

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Summary for Subcatchment e1S: Pre-Development - Western Portion of Lot

$$\text{Runoff} = 0.20 \text{ cfs} @ 12.11 \text{ hrs, Volume=} 682 \text{ cf, Depth=} 0.88"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description			
825	65	Brush, Good, HSG C			
1,021	96	Gravel surface, HSG C			
7,423	74	>75% Grass cover, Good, HSG C			
9,269	76	Weighted Average			
9,269	76	100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	20	0.0600	0.09		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
0.2	14	0.0050	1.14		Shallow Concentrated Flow, BC
					Unpaved Kv= 16.1 fps
0.3	33	0.0180	2.16		Shallow Concentrated Flow, CD
					Unpaved Kv= 16.1 fps
2.5	96	0.0160	0.63		Shallow Concentrated Flow, DE
					Woodland Kv= 5.0 fps
6.8	163	Total			

Summary for Subcatchment e2S: Pre-Development - Eastern Portion of Lot

$$\text{Runoff} = 1.08 \text{ cfs} @ 12.27 \text{ hrs, Volume=} 5,293 \text{ cf, Depth=} 0.91"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 1-yr Rainfall=2.80"

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Area (sf)	CN	Description
6,357	65	Brush, Good, HSG C
1,687	96	Gravel surface, HSG C
2,892	98	Roofs, HSG C
57,216	74	>75% Grass cover, Good, HSG C
1,919	98	Paved parking, HSG C
70,071	75	Weighted Average
65,260	74	93.13% Pervious Area
4,811	98	6.87% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
11.2	50	0.0260
Velocity (ft/sec)	Capacity (cfs)	Description
0.07	0.0260	Sheet Flow, AB
		Grass: Bermuda n= 0.410 P2= 3.30"
3.2	100	0.0110
		Shallow Concentrated Flow, BC
		Woodland Kv= 5.0 fps
3.9	216	0.0340
		Shallow Concentrated Flow, CD
		Woodland Kv= 5.0 fps
18.3	366	Total

Summary for Pond eA1: Analysis Point - Pre Dev Catch Basin in Hope St.

Inflow Area = 9,269 sf, 0.00% Impervious, Inflow Depth = 0.88" for 1-yr event
Inflow = 0.20 cfs @ 12.11 hrs, Volume= 682 cf
Primary = 0.20 cfs @ 12.11 hrs, Volume= 682 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Pond eA2: Analysis Point - Pre Dev Flow Towards Wetland

Inflow Area = 70,071 sf, 6.87% Impervious, Inflow Depth = 0.91" for 1-yr event
Inflow = 1.08 cfs @ 12.27 hrs, Volume= 5,293 cf
Primary = 1.08 cfs @ 12.27 hrs, Volume= 5,293 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

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Type III 24-hr 2-yr Rainfall=3.30"
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Time span=0.00-72.00 hrs, dt=0.04 hrs, 1801 points

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment e1S: Pre-Development - Western Portion of Lot

Runoff Area=9,269 sf 0.00% Impervious Runoff Depth=1.22"
Flow Length=163' Tc=6.8 min CN=76/0 Runoff=0.28 cfs 944 cf

Subcatchment e2S: Pre-Development - Eastern Portion of Lot

Runoff Area=70,071 sf 6.87% Impervious Runoff Depth=1.24"
Flow Length=366' Tc=18.3 min CN=74/98 Runoff=1.53 cfs 7,233 cf

Pond eA1: Analysis Point - Pre Dev Catch Basin in Hope St.

Inflow=0.28 cfs 944 cf
Primary=0.28 cfs 944 cf

Pond eA2: Analysis Point - Pre Dev Flow Towards Wetland

Inflow=1.53 cfs 7,233 cf
Primary=1.53 cfs 7,233 cf

Total Runoff Area = 79,340 sf Runoff Volume = 8,177 cf Average Runoff Depth = 1.24"
93.94% Pervious = 74,529 sf 6.06% Impervious = 4,811 sf

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Summary for Subcatchment e1S: Pre-Development - Western Portion of Lot

$$\text{Runoff} = 0.28 \text{ cfs} @ 12.11 \text{ hrs, Volume=} 944 \text{ cf, Depth=} 1.22"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description			
825	65	Brush, Good, HSG C			
1,021	96	Gravel surface, HSG C			
7,423	74	>75% Grass cover, Good, HSG C			
9,269	76	Weighted Average			
9,269	76	100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	20	0.0600	0.09		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
0.2	14	0.0050	1.14		Shallow Concentrated Flow, BC
					Unpaved Kv= 16.1 fps
0.3	33	0.0180	2.16		Shallow Concentrated Flow, CD
					Unpaved Kv= 16.1 fps
2.5	96	0.0160	0.63		Shallow Concentrated Flow, DE
					Woodland Kv= 5.0 fps
6.8	163	Total			

Summary for Subcatchment e2S: Pre-Development - Eastern Portion of Lot

$$\text{Runoff} = 1.53 \text{ cfs} @ 12.27 \text{ hrs, Volume=} 7,233 \text{ cf, Depth=} 1.24"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 2-yr Rainfall=3.30"

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Area (sf)	CN	Description	
6,357	65	Brush, Good, HSG C	
1,687	96	Gravel surface, HSG C	
2,892	98	Roofs, HSG C	
57,216	74	>75% Grass cover, Good, HSG C	
1,919	98	Paved parking, HSG C	
70,071	75	Weighted Average	
65,260	74	93.13% Pervious Area	
4,811	98	6.87% Impervious Area	
Tc (min)	Length (feet)	Slope (ft/ft)	
		Velocity (ft/sec)	
		Capacity (cfs)	
11.2	50	0.0260	0.07
			Sheet Flow, AB
			Grass: Bermuda n= 0.410 P2= 3.30"
3.2	100	0.0110	0.52
			Shallow Concentrated Flow, BC
			Woodland Kv= 5.0 fps
3.9	216	0.0340	0.92
			Shallow Concentrated Flow, CD
			Woodland Kv= 5.0 fps
18.3	366	Total	

Summary for Pond eA1: Analysis Point - Pre Dev Catch Basin in Hope St.

Inflow Area = 9,269 sf, 0.00% Impervious, Inflow Depth = 1.22" for 2-yr event
Inflow = 0.28 cfs @ 12.11 hrs, Volume= 944 cf
Primary = 0.28 cfs @ 12.11 hrs, Volume= 944 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Pond eA2: Analysis Point - Pre Dev Flow Towards Wetland

Inflow Area = 70,071 sf, 6.87% Impervious, Inflow Depth = 1.24" for 2-yr event
Inflow = 1.53 cfs @ 12.27 hrs, Volume= 7,233 cf
Primary = 1.53 cfs @ 12.27 hrs, Volume= 7,233 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

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Type III 24-hr 10-yr Rainfall=4.90"
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Time span=0.00-72.00 hrs, dt=0.04 hrs, 1801 points

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment e1S: Pre-Development - Western Portion of Lot

Runoff Area=9,269 sf 0.00% Impervious Runoff Depth=2.45"
Flow Length=163' Tc=6.8 min CN=760 Runoff=0.58 cfs 1,895 cf

Subcatchment e2S: Pre-Development - Eastern Portion of Lot

Runoff Area=70,071 sf 6.87% Impervious Runoff Depth=2.45"
Flow Length=366' Tc=18.3 min CN=74/98 Runoff=3.15 cfs 14,295 cf

Pond eA1: Analysis Point - Pre Dev Catch Basin in Hope St.

Inflow=0.58 cfs 1,895 cf
Primary=0.58 cfs 1,895 cf

Inflow=3.15 cfs 14,295 cf
Primary=3.15 cfs 14,295 cf

Total Runoff Area = 79,340 sf Runoff Volume = 16,190 cf Average Runoff Depth = 2.45"
93.94% Pervious = 74,529 sf 6.06% Impervious = 4,811 sf

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Summary for Subcatchment e1S: Pre-Development - Western Portion of Lot

Runoff = 0.58 cfs @ 12.10 hrs, Volume= 1,895 cf, Depth= 2.45"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description			
825	65	Brush, Good, HSG C			
1,021	96	Gravel surface, HSG C			
7,423	74	>75% Grass cover, Good, HSG C			
9,269	76	Weighted Average			
9,269	76	100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	20	0.0600	0.09		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
0.2	14	0.0050	1.14		Shallow Concentrated Flow, BC
					Unpaved Kv= 16.1 fps
0.3	33	0.0180	2.16		Shallow Concentrated Flow, CD
					Unpaved Kv= 16.1 fps
2.5	96	0.0160	0.63		Shallow Concentrated Flow, DE
					Woodland Kv= 5.0 fps
6.8	163	Total			

Summary for Subcatchment e2S: Pre-Development - Eastern Portion of Lot

Runoff = 3.15 cfs @ 12.26 hrs, Volume= 14,295 cf, Depth= 2.45"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 10-yr Rainfall=4.90"

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Area (sf)	CN	Description	
6,357	65	Brush, Good, HSG C	
1,687	96	Gravel surface, HSG C	
2,892	98	Roofs, HSG C	
57,216	74	>75% Grass cover, Good, HSG C	
1,919	98	Paved parking, HSG C	
70,071	75	Weighted Average	
65,260	74	93.13% Pervious Area	
4,811	98	6.87% Impervious Area	
Tc (min)	Length (feet)	Slope (ft/ft)	
		Velocity (ft/sec)	
		Capacity (cfs)	
11.2	50	0.0260	0.07
3.2	100	0.0110	0.52
3.9	216	0.0340	0.92
18.3	366	Total	

Sheet Flow, AB
Grass: Bermuda n= 0.410 P2= 3.30"
Shallow Concentrated Flow, BC
Woodland Kv= 5.0 fps
Shallow Concentrated Flow, CD
Woodland Kv= 5.0 fps

Summary for Pond eA1: Analysis Point - Pre Dev Catch Basin in Hope St.

Inflow Area = 9,269 sf, 0.00% Impervious, Inflow Depth = 2.45" for 10-yr event
Inflow = 0.58 cfs @ 12.10 hrs, Volume= 1,895 cf
Primary = 0.58 cfs @ 12.10 hrs, Volume= 1,895 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Pond eA2: Analysis Point - Pre Dev Flow Towards Wetland

Inflow Area = 70,071 sf, 6.87% Impervious, Inflow Depth = 2.45" for 10-yr event
Inflow = 3.15 cfs @ 12.26 hrs, Volume= 14,295 cf
Primary = 3.15 cfs @ 12.26 hrs, Volume= 14,295 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

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Time span=0.00-72.00 hrs, dt=0.04 hrs, 1801 points

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment e1S: Pre-Development - Western Portion of Lot

Runoff Area=9,269 sf 0.00% Impervious Runoff Depth=3.47"
Flow Length=163' Tc=6.8 min CN=760 Runoff=0.83 cfs 2,678 cf

Subcatchment e2S: Pre-Development - Eastern Portion of Lot

Runoff Area=70,071 sf 6.87% Impervious Runoff Depth=3.45"
Flow Length=366' Tc=18.3 min CN=74/98 Runoff=4.47 cfs 20,129 cf

Pond eA1: Analysis Point - Pre Dev Catch Basin in Hope St.

Inflow=0.83 cfs 2,678 cf
Primary=0.83 cfs 2,678 cf

Inflow=4.47 cfs 20,129 cf
Primary=4.47 cfs 20,129 cf

Total Runoff Area = 79,340 sf Runoff Volume = 22,806 cf Average Runoff Depth = 3.45"
93.94% Pervious = 74,529 sf 6.06% Impervious = 4,811 sf

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Type III 24-hr 25-yr Rainfall=6.10"
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Summary for Subcatchment e1S: Pre-Development - Western Portion of Lot

$$\text{Runoff} = 0.83 \text{ cfs} @ 12.10 \text{ hrs, Volume=} 2,678 \text{ cf, Depth=} 3.47"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description			
825	65	Brush, Good, HSG C			
1,021	96	Gravel surface, HSG C			
7,423	74	>75% Grass cover, Good, HSG C			
9,269	76	Weighted Average			
9,269	76	100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	20	0.0600	0.09		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
0.2	14	0.0050	1.14		Shallow Concentrated Flow, BC
					Unpaved Kv= 16.1 fps
0.3	33	0.0180	2.16		Shallow Concentrated Flow, CD
					Unpaved Kv= 16.1 fps
2.5	96	0.0160	0.63		Shallow Concentrated Flow, DE
					Woodland Kv= 5.0 fps
6.8	163	Total			

Summary for Subcatchment e2S: Pre-Development - Eastern Portion of Lot

$$\text{Runoff} = 4.47 \text{ cfs} @ 12.25 \text{ hrs, Volume=} 20,129 \text{ cf, Depth=} 3.45"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 25-yr Rainfall=6.10"

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Type III 24-hr 25-yr Rainfall=6.10"
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Area (sf)	CN	Description
6,357	65	Brush, Good, HSG C
1,687	96	Gravel surface, HSG C
2,892	98	Roofs, HSG C
57,216	74	>75% Grass cover, Good, HSG C
1,919	98	Paved parking, HSG C
70,071	75	Weighted Average
65,260	74	93.13% Pervious Area
4,811	98	6.87% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
11.2	50	0.0260
Velocity (ft/sec)	Capacity (cfs)	Description
0.07	0.07	Sheet Flow, AB
		Grass: Bermuda n= 0.410 P2= 3.30"
3.2	100	0.0110
		Shallow Concentrated Flow, BC
		Woodland Kv= 5.0 fps
3.9	216	0.0340
		Shallow Concentrated Flow, CD
		Woodland Kv= 5.0 fps
18.3	366	Total

Summary for Pond eA1: Analysis Point - Pre Dev Catch Basin in Hope St.

Inflow Area = 9,269 sf, 0.00% Impervious, Inflow Depth = 3.47" for 25-yr event
Inflow = 0.83 cfs @ 12.10 hrs, Volume= 2,678 cf
Primary = 0.83 cfs @ 12.10 hrs, Volume= 2,678 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Pond eA2: Analysis Point - Pre Dev Flow Towards Wetland

Inflow Area = 70,071 sf, 6.87% Impervious, Inflow Depth = 3.45" for 25-yr event
Inflow = 4.47 cfs @ 12.25 hrs, Volume= 20,129 cf
Primary = 4.47 cfs @ 12.25 hrs, Volume= 20,129 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

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Time span=0.00-72.00 hrs, dt=0.04 hrs, 1801 points

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment e1S: Pre-Development - Western Portion of Lot

Runoff Area=9,269 sf 0.00% Impervious Runoff Depth=5.71"
Flow Length=163' Tc=6.8 min CN=760 Runoff=1.36 cfs 4,408 cf

Subcatchment e2S: Pre-Development - Eastern Portion of Lot

Runoff Area=70,071 sf 6.87% Impervious Runoff Depth=5.66"
Flow Length=366' Tc=18.3 min CN=74/98 Runoff=7.34 cfs 33,076 cf

Pond eA1: Analysis Point - Pre Dev Catch Basin in Hope St.

Inflow=1.36 cfs 4,408 cf
Primary=1.36 cfs 4,408 cf

Pond eA2: Analysis Point - Pre Dev Flow Towards Wetland

Total Runoff Area = 79,340 sf Runoff Volume = 37,484 cf Average Runoff Depth = 5.67"
93.94% Pervious = 74,529 sf 6.06% Impervious = 4,811 sf

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Summary for Subcatchment e1S: Pre-Development - Western Portion of Lot

$$\text{Runoff} = 1.36 \text{ cfs} @ 12.10 \text{ hrs, Volume=} 4,408 \text{ cf, Depth=} 5.71"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description			
825	65	Brush, Good, HSG C			
1,021	96	Gravel surface, HSG C			
7,423	74	>75% Grass cover, Good, HSG C			
9,269	76	Weighted Average			
9,269	76	100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	20	0.0600	0.09		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
0.2	14	0.0050	1.14		Shallow Concentrated Flow, BC
					Unpaved Kv= 16.1 fps
0.3	33	0.0180	2.16		Shallow Concentrated Flow, CD
					Unpaved Kv= 16.1 fps
2.5	96	0.0160	0.63		Shallow Concentrated Flow, DE
					Woodland Kv= 5.0 fps
6.8	163	Total			

Summary for Subcatchment e2S: Pre-Development - Eastern Portion of Lot

$$\text{Runoff} = 7.34 \text{ cfs} @ 12.25 \text{ hrs, Volume=} 33,076 \text{ cf, Depth=} 5.66"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

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Type III 24-hr 100-yr Rainfall=8.60"
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Area (sf)	CN	Description	
6,357	65	Brush, Good, HSG C	
1,687	96	Gravel surface, HSG C	
2,892	98	Roofs, HSG C	
57,216	74	>75% Grass cover, Good, HSG C	
1,919	98	Paved parking, HSG C	
70,071	75	Weighted Average	
65,260	74	93.13% Pervious Area	
4,811	98	6.87% Impervious Area	
Tc (min)	Length (feet)	Slope (ft/ft)	
		Velocity (ft/sec)	
		Capacity (cfs)	
11.2	50	0.0260	0.07
			Sheet Flow, AB
			Grass: Bermuda n= 0.410 P2= 3.30"
3.2	100	0.0110	0.52
			Shallow Concentrated Flow, BC
			Woodland Kv= 5.0 fps
3.9	216	0.0340	0.92
			Shallow Concentrated Flow, CD
			Woodland Kv= 5.0 fps
18.3	366	Total	

Summary for Pond eA1: Analysis Point - Pre Dev Catch Basin in Hope St.

Inflow Area = 9,269 sf, 0.00% Impervious, Inflow Depth = 5.71" for 100-yr event
Inflow = 1.36 cfs @ 12.10 hrs, Volume= 4,408 cf
Primary = 1.36 cfs @ 12.10 hrs, Volume= 4,408 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Pond eA2: Analysis Point - Pre Dev Flow Towards Wetland

Inflow Area = 70,071 sf, 6.87% Impervious, Inflow Depth = 5.66" for 100-yr event
Inflow = 7.34 cfs @ 12.25 hrs, Volume= 33,076 cf
Primary = 7.34 cfs @ 12.25 hrs, Volume= 33,076 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

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Type III 24-hr WQv Rainfall=1.20"
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Time span=0.00-72.00 hrs, dt=0.04 hrs, 1801 points

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment e1S: Pre-Development - Western Portion of Lot

Runoff Area=9,269 sf 0.00% Impervious Runoff Depth=0.09"
Flow Length=163' Tc=6.8 min CN=76/0 Runoff=0.01 cfs 67 cf

Subcatchment e2S: Pre-Development - Eastern Portion of Lot

Runoff Area=70,071 sf 6.87% Impervious Runoff Depth=0.13"
Flow Length=366' Tc=18.3 min CN=74/98 Runoff=0.09 cfs 730 cf

Pond eA1: Analysis Point - Pre Dev Catch Basin in Hope St.

Inflow=0.01 cfs 67 cf
Primary=0.01 cfs 67 cf

Pond eA2: Analysis Point - Pre Dev Flow Towards Wetland

Inflow=0.09 cfs 730 cf
Primary=0.09 cfs 730 cf

Total Runoff Area = 79,340 sf Runoff Volume = 797 cf Average Runoff Depth = 0.12"
93.94% Pervious = 74,529 sf 6.06% Impervious = 4,811 sf

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Type III 24-hr WQv Rainfall=1.20"
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Summary for Subcatchment e1S: Pre-Development - Western Portion of Lot

$$\text{Runoff} = 0.01 \text{ cfs} @ 12.36 \text{ hrs, Volume=} 67 \text{ cf, Depth=} 0.09"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description			
825	65	Brush, Good, HSG C			
1,021	96	Gravel surface, HSG C			
7,423	74	>75% Grass cover, Good, HSG C			
9,269	76	Weighted Average			
9,269	76	100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.8	20	0.0600	0.09		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
0.2	14	0.0050	1.14		Shallow Concentrated Flow, BC
					Unpaved Kv= 16.1 fps
0.3	33	0.0180	2.16		Shallow Concentrated Flow, CD
					Unpaved Kv= 16.1 fps
2.5	96	0.0160	0.63		Shallow Concentrated Flow, DE
					Woodland Kv= 5.0 fps
6.8	163	Total			

Summary for Subcatchment e2S: Pre-Development - Eastern Portion of Lot

$$\text{Runoff} = 0.09 \text{ cfs} @ 12.25 \text{ hrs, Volume=} 730 \text{ cf, Depth=} 0.13"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr WQv Rainfall=1.20"

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Area (sf)	CN	Description
6,357	65	Brush, Good, HSG C
1,687	96	Gravel surface, HSG C
2,892	98	Roofs, HSG C
57,216	74	>75% Grass cover, Good, HSG C
1,919	98	Paved parking, HSG C
70,071	75	Weighted Average
65,260	74	93.13% Pervious Area
4,811	98	6.87% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
11.2	50	0.0260
Velocity (ft/sec)	Capacity (cfs)	Description
0.07	0.07	Sheet Flow, AB
		Grass: Bermuda n= 0.410 P2= 3.30"
3.2	100	0.0110
		Shallow Concentrated Flow, BC
		Woodland Kv= 5.0 fps
3.9	216	0.0340
		Shallow Concentrated Flow, CD
		Woodland Kv= 5.0 fps
18.3	366	Total

Summary for Pond eA1: Analysis Point - Pre Dev Catch Basin in Hope St.

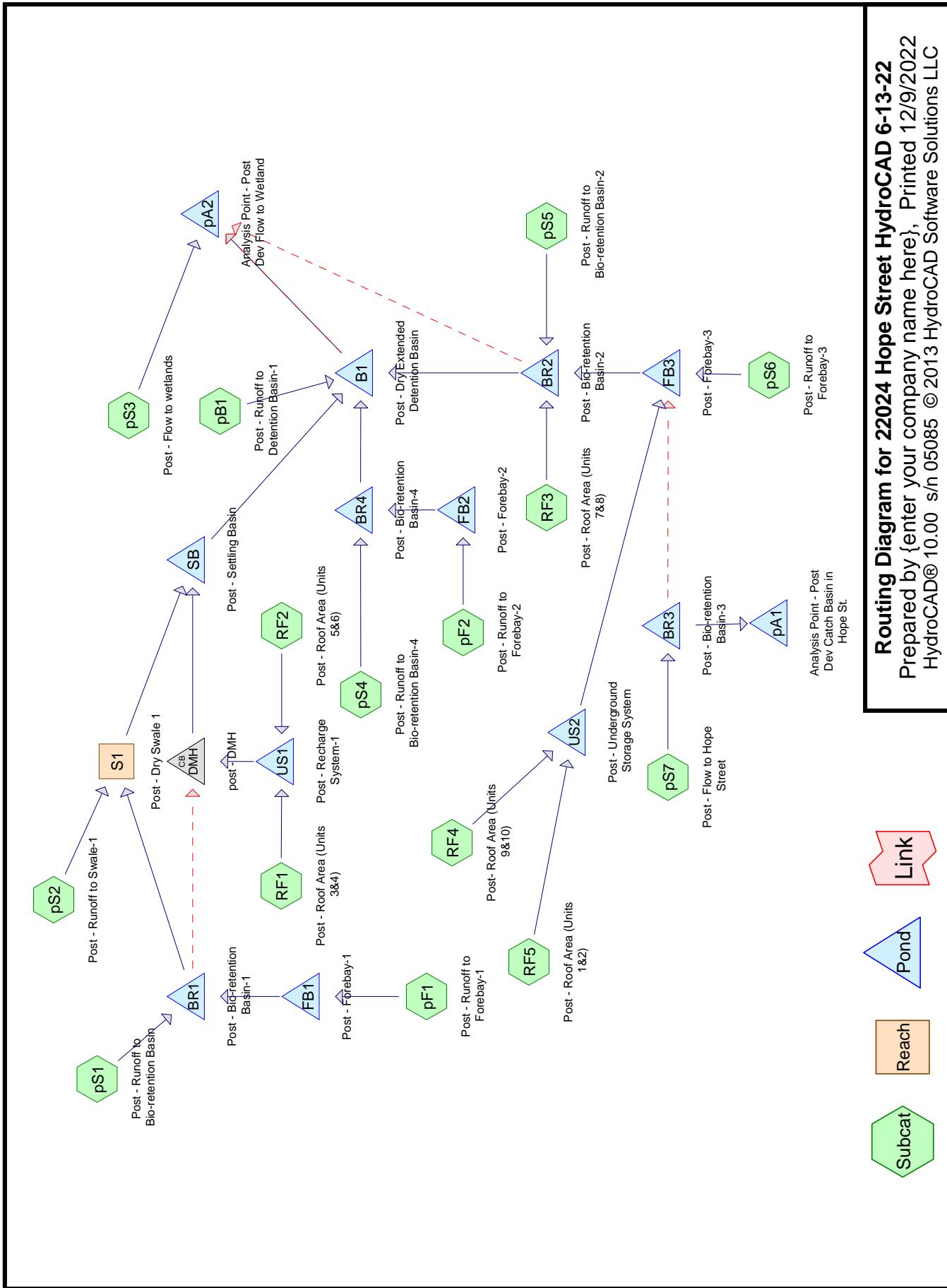
Inflow Area = 9,269 sf, 0.00% Impervious, Inflow Depth = 0.09" for WQv event
Inflow = 0.01 cfs @ 12.36 hrs, Volume= 67 cf
Primary = 0.01 cfs @ 12.36 hrs, Volume= 67 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Pond eA2: Analysis Point - Pre Dev Flow Towards Wetland

Inflow Area = 70,071 sf, 6.87% Impervious, Inflow Depth = 0.13" for WQv event
Inflow = 0.09 cfs @ 12.25 hrs, Volume= 730 cf
Primary = 0.09 cfs @ 12.25 hrs, Volume= 730 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs



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Area Listing (selected nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
53,735	74	>75% Grass cover, Good, HSG C (pB1, pF1, pF2, pS1, pS2, pS3, pS4, pS5, pS6, pS7)
956	98	Patio/Deck/Stairs (pF1, pS2, pS5, pS7)
11,048	98	Paved parking, HSG C (pF1, pF2, pS6)
12,326	98	Roofs, HSG C (RF1, RF2, RF3, RF4, RF5)
1,622	98	Walkway (pF1, pF2, pS6)
79,687	82	TOTAL AREA

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Runoff

Time span=0.00-72.00 hrs, dt=0.04 hrs, 1801 points
Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Pond B1: Post - Dry Extended Detention Basin

Peak Elev=80.64' Storage=900 cf Inflow=0.73 cfs 4,501 cf
Primary=0.33 cfs 4,171 cf Secondary=0.00 cfs 0 cf Outflow=0.33 cfs 4,171 cf

Pond BR1: Post - Bio-retention Basin-1

Peak Elev=85.42' Storage=1,018 cf Inflow=0.44 cfs 1,624 cf
Primary=0.00 cfs 0 cf Secondary=0.02 cfs 1,363 cf Outflow=0.02 cfs 1,363 cf

Pond BR2: Post - Bio-retention Basin-2

Peak Elev=82.25' Storage=927 cf Inflow=0.77 cfs 3,997 cf
Primary=0.53 cfs 1,926 cf Secondary=0.02 cfs 1,997 cf Outflow=0.55 cfs 3,923 cf

Pond BR3: Post - Bio-retention Basin-3

Peak Elev=84.96' Storage=510 cf Inflow=0.18 cfs 1,113 cf
Primary=0.02 cfs 126 cf Secondary=0.02 cfs 930 cf Outflow=0.04 cfs 1,056 cf

Pond BR4: Post - Bio-retention Basin-4

Peak Elev=83.83' Storage=277 cf Inflow=0.23 cfs 770 cf
Outflow=0.18 cfs 516 cf

Pond DMH: post - DMH

8.0" Round Culvert n=0.013 L=92.0' S=0.0087 '/' Peak Elev=83.00' Storage=86 cf Inflow=0.12 cfs 1,693 cf

Pond FB1: Post - Forebay-1

Peak Elev=86.14' Storage=210 cf Inflow=0.41 cfs 1,610 cf
Outflow=0.40 cfs 1,438 cf

Pond FB2: Post - Forebay-2

Peak Elev=83.87' Storage=86 cf Inflow=0.20 cfs 760 cf
Outflow=0.20 cfs 684 cf

Pond FB3: Post - Forebay-3

Peak Elev=82.18' Storage=243 cf Inflow=0.58 cfs 3,486 cf
Outflow=0.57 cfs 3,294 cf
Inflow=0.02 cfs 126 cf
Primary=0.02 cfs 126 cf

Pond pA1: Analysis Point - Post Dev Catch Basin in Hope St.

Pond pA2: Analysis Point - Post Dev Flow to Wetland

Inflow=0.39 cfs 6,541 cf
Primary=0.39 cfs 6,541 cf

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Subcatchment pB1: Post - Runoff to Detention Basin-1

Runoff Area=2,690 sf 0.00% Impervious Runoff Depth=0.78"
Tc=5.0 min CN=74/0 Runoff=0.05 cfs 176 cf

Subcatchment pF1: Post - Runoff to Forebay-1

Runoff Area=12,569 sf 42.19% Impervious Runoff Depth=1.54"
Flow Length=126' Tc=10.0 min CN=74/98 Runoff=0.41 cfs 1,610 cf

Subcatchment pF2: Post - Runoff to Forebay-2

Runoff Area=4,810 sf 62.31% Impervious Runoff Depth=1.90"
Flow Length=64' Tc=8.0 min CN=74/98 Runoff=0.20 cfs 760 cf

Subcatchment pS1: Post - Runoff to Bio-retention Basin

Runoff Area=2,851 sf 0.00% Impervious Runoff Depth=0.78"
Tc=5.0 min CN=74/0 Runoff=0.06 cfs 186 cf

Subcatchment pS2: Post - Runoff to Swale-1

Runoff Area=5,173 sf 9.22% Impervious Runoff Depth=0.95"
Tc=5.0 min CN=74/98 Runoff=0.12 cfs 409 cf

Subcatchment pS3: Post - Flow to wetlands

Runoff Area=5,708 sf 0.00% Impervious Runoff Depth=0.78"
Flow Length=453' Tc=22.1 min CN=74/0 Runoff=0.07 cfs 373 cf

Subcatchment pS4: Post - Runoff to Bio-retention Basin-4

Runoff Area=1,321 sf 0.00% Impervious Runoff Depth=0.78"
Flow Length=64' Tc=8.0 min CN=74/0 Runoff=0.02 cfs 86 cf

Subcatchment pS5: Post - Runoff to Bio-retention Basin-2

Runoff Area=1,840 sf 12.93% Impervious Runoff Depth=1.01"
Tc=5.0 min CN=74/98 Runoff=0.05 cfs 156 cf

Subcatchment pS6: Post - Runoff to Forebay-3

Runoff Area=13,640 sf 32.91% Impervious Runoff Depth=1.37"
Tc=5.0 min CN=74/98 Runoff=0.46 cfs 1,559 cf

Subcatchment pS7: Post - Flow to Hope Street

Runoff Area=16,759 sf 0.73% Impervious Runoff Depth=0.80"
Flow Length=104' Tc=30.8 min CN=74/98 Runoff=0.18 cfs 1,113 cf

Subcatchment RF1: Post - Roof Area (Units 3&4)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=2.57"
Tc=5.0 min CN=0/98 Runoff=0.16 cfs 547 cf

Subcatchment RF2: Post - Roof Area (Units 5&6)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=2.57"
Tc=5.0 min CN=0/98 Runoff=0.16 cfs 547 cf

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Subcatchment RF3: Post - Roof Area (Units 7&8)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=2.57"
Tc=5.0 min CN=0/98 Runoff=0.16 cfs 547 cf

Subcatchment RF4: Post- Roof Area (Units 9&10)

Runoff Area=2,102 sf 100.00% Impervious Runoff Depth=2.57"
Tc=5.0 min CN=0/98 Runoff=0.13 cfs 450 cf

Subcatchment RF5: Post - Roof Area (Units 1&2)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=2.57"
Tc=5.0 min CN=0/98 Runoff=0.16 cfs 547 cf

Reach S1: Post - Dry Swale 1

Avg. Flow Depth=0.06' Max Vel=0.89 fps Inflow=0.12 cfs 409 cf
n=0.035 L=150.0' S=0.0200 '/' Capacity=17.43 cfs Outflow=0.11 cfs 409 cf

Pond SB: Post - Settling Basin

Peak Elev=82.06' Storage=241 cf Inflow=0.20 cfs 2,102 cf
Outflow=0.17 cfs 1,883 cf

Pond US1: Post - Recharge System-1

Peak Elev=85.04' Storage=423 cf Inflow=0.32 cfs 1,094 cf
Discarded=0.01 cfs 764 cf Primary=0.10 cfs 330 cf Outflow=0.11 cfs 1,094 cf

Pond US2: Post - Underground Storage System

Peak Elev=83.44' Storage=203 cf Inflow=0.30 cfs 997 cf
4.0" Round Culvert n=0.013 L=100.0' S=0.0105 '/' Outflow=0.15 cfs 997 cf

Total Runoff Area = 79,687 sf Runoff Volume = 9,067 cf Average Runoff Depth = 1.37"
67.43% Pervious = 53,735 sf 32.57% Impervious = 25,952 sf

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Summary for Pond B1: Post - Dry Extended Detention Basin

Inflow Area =	57,220 sf	45.14% Impervious,	Inflow Depth = 0.94"	for 1-yr event
Inflow =	0.73 cfs @	12.21 hrs, Volume=	4,501 cf	
Outflow =	0.33 cfs @	12.71 hrs, Volume=	4,171 cf	Atten= 55%, Lag= 30.0 min
Primary =	0.33 cfs @	12.71 hrs, Volume=	4,171 cf	
Secondary =	0.00 cfs @	0.00 hrs, Volume=	0 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
 Peak Elev= 80.64' @ 12.71 hrs Surf.Area= 1,522 sf Storage= 900 cf
 Flood Elev= 82.50' Surf.Area= 2,300 sf Storage= 4,432 cf

Plug-Flow detention time= 152.3 min calculated for 4,169 cf (93% of inflow)
 Center-of-Mass det. time= 87.1 min (1,087.5 - 1,000.5)

Volume	Invert	Avail.Storage	Storage Description
#		4,432 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.00	1,273	0	0
81.00	1,660	1,467	1,467
82.00	2,080	1,870	3,337
82.50	2,300	1,095	4,432
Device	Routing	Invert	Outlet Devices
#1	Secondary	82.00'	4'0" long x 2'0" breadth Broad-Crested Weir
		Head (feet)	Head (feet)
		0.20	0.40
		0.60	0.80
		1.00	1.20
		1.40	1.60
		2.00	2.50
		3.00	3.50
		Coef. (English)	Coef. (English)
		2.54	2.61
		2.60	2.66
		2.70	2.77
		2.88	2.85
		3.07	3.20
		3.32	3.32
#2	Primary	80.45'	8.0" Round Culvert
		L= 12.0'	RCP, sq.cut end projecting,
		Inlet / Outlet Invert= 80.45' / 79.50'	Ke= 0.500
		S= 0.0792 "	Cc= 0.900
		n= 0.013	Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Primary	80.25'	4.0" Round Culvert
		L= 12.0'	RCP, sq.cut end projecting,
		Inlet / Outlet Invert= 80.25' / 79.50'	Ke= 0.500
		S= 0.0625 "	Cc= 0.900
		n= 0.013	Corrugated PE, smooth interior, Flow Area= 0.09 sf

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Primary OutFlow Max=0.33 cfs @ 12.71 hrs HW=80.64' (Free Discharge)
 2=Culvert (Inlet Controls 0.13 cfs @ 1.50 fps)
 3=Culvert (Inlet Controls 0.20 cfs @ 2.29 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=80.00' (Free Discharge)
 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond BR1: Post - Bio-retention Basin-1

Inflow Area =	15,420 sf	34.39% Impervious,	Inflow Depth =	1.26"	for 1-yr event
Inflow =	0.44 cfs @	12.15 hrs, Volume=		1,624 cf	
Outflow =	0.02 cfs @	15.75 hrs, Volume=		1,363 cf, Atten= 95%, Lag= 215.8 min	
Primary =	0.00 cfs @	0.00 hrs, Volume=		0 cf	
Secondary =	0.02 cfs @	15.75 hrs, Volume=		1,363 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 85.42' @ 15.75 hrs Surf.Area= 1,891 sf Storage= 1,018 cf
Flood Elev= 86.00' Surf.Area= 2,110 sf Storage= 1,567 cf

Plug-Flow detention time= 532.5 min calculated for 1,362 cf (84% of inflow)
Center-of-Mass det. time= 464.4 min (1,304.4 - 840.0)

Volume	Invert	Avail.Storage	Storage Description	
#1	85.00'	867 cf	Custom Stage Data (Prismatic) Listed below (Recalc)	
#2	83.00'	694 cf	Filter Material (Prismatic) Listed below (Recalc)	
#3	83.75'	7 cf	6.0" Round Pipe Storage Inside #2	L= 35.0'
1,567 cf Total Available Storage				
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
85.00	678	0	0	
86.00	1,055	867	867	

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.00	1,055	0	0
85.00	1,055	2,110	2,110

Device	Routing	Invert	Outlet Devices
#1 Primary	85.75'	4.0' long x 2.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 Coef. (English) 2.48 2.60 2.60 2.64 2.65 2.68 2.75 2.74 2.76 2.89 3.05 3.19 3.32	

#2 Device 3 83.00'
#3 Secondary 83.75'
0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'
6.0" Round Culvert L= 100.0' RCP, sq.cut end projecting, Ke= 0.500
Inlet / Outlet Invert= 83.75' / 82.85' S= 0.0090' Cc= 0.900
n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=83.00' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=0.02 cfs @ 15.75 hrs HW=85.42' (Free Discharge)
↳ 3=Culvert (Passes 0.02 cfs of 0.74 cfs potential flow)
↳ 2=Exfiltration (Exfiltration Controls 0.02 cfs)

Summary for Pond BR2: Post - Bio-retention Basin-2

Inflow Area = 22,694 sf, 52.62% Impervious, Inflow Depth = 2.11" for 1-yr event
Inflow = 0.77 cfs @ 12.09 hrs, Volume= 3,997 cf
Outflow = 0.55 cfs @ 12.21 hrs, Volume= 3,923 cf, Atten= 29%, Lag= 6.7 min
Primary = 0.53 cfs @ 12.21 hrs, Volume= 1,926 cf
Secondary = 0.02 cfs @ 12.21 hrs, Volume= 1,997 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 82.25' @ 12.21 hrs Surf.Area= 1,720 sf Storage= 927 cf
Flood Elev= 82.50' Surf.Area= 1,796 sf Storage= 1,140 cf

Plug-Flow detention time= 283.1 min calculated for 3,921 cf (98% of inflow)
Center-of-Mass det. time= 265.0 min (1,192.0 - 927.1)

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Volume	Invert	Avail.Storage	Storage Description
#1	81.35'	836 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	80.35'	292 cf	Filter Material (Prismatic) Listed below (Recalc)
#3	80.60'	12 cf	6.0" Round Pipe Storage Inside #2 L= 61.0'

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.35	563	0	0
82.00	745	425	425
82.50	898	411	836

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.35	898	0	0
81.35	898	898	898

Device	Routing	Invert	Outlet Devices
#1	Primary	82.10'	3.5' long x 2'0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Device 3	80.35'	0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'
#3	Secondary	80.60'	6.0" Round Culvert L= 62.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 80.60' / 79.00' S= 0.0258 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.52 cfs @ 12.21 hrs HW=82.25' (Free Discharge)
1=Broad-Crested Rectangular Weir (Weir Controls 0.52 cfs @ 0.99 fps)

Secondary OutFlow Max=0.02 cfs @ 12.21 hrs HW=82.25' (Free Discharge)
2=Culvert (Passes 0.02 cfs of 1.03 cfs potential flow)
3=Exfiltration (Exfiltration Controls 0.02 cfs)

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Summary for Pond BR3: Post - Bio-retention Basin-3

Inflow Area = 16,759 sf, 0.73% Impervious, Inflow Depth = 0.80" for 1-yr event
Inflow = 0.18 cfs @ 12.48 hrs, Volume= 1,113 cf
Outflow = 0.04 cfs @ 13.64 hrs, Volume= 1,056 cf, Atten= 78%, Lag= 69.8 min
Primary = 0.02 cfs @ 13.64 hrs, Volume= 126 cf
Secondary = 0.02 cfs @ 13.64 hrs, Volume= 930 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 84.96' @ 13.64 hrs Surf.Area= 1,310 sf Storage= 510 cf

Plug-Flow detention time= 371.6 min calculated for 1,056 cf (95% of inflow)
Center-of-Mass det. time= 345.5 min (1,236.9 - 891.4)

Volume Invert Avail.Storage Storage Description

#1	84.40'	371 cf	Custom Stage Data (Prismatic)	Listed below (Recalc)
#2	83.40'	224 cf	Filter Material (Prismatic)	Listed below (Recalc)
#3	83.65'	6 cf	6.0" Round Pipe Storage	686 cf Overall - 6 cf Embedded = 680 cf x 33.0% Voids Inside #2 L= 30.0'
		601 cf	Total Available Storage	

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
84.40	374	0	0
85.10	686	371	371

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.40	686	0	0
84.40	686	686	686

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Device Routing Invert Outlet Devices

Device	Routing	Invert	Outlet Devices
#1	Primary	84.95'	8.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Device 3	85.00'	6.0" Horiz. Orifice/Grate C=0.600 Limited to weir flow at low heads
#3	Secondary	83.65'	6.0" Round Culvert L= 68.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 83.65' / 83.00' S= 0.0096 ' Cc= 0.900
#4	Device 3	83.40'	n= 0.013 Concrete pipe, bends & connections, Flow Area= 0.20 sf 0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'

Primary OutFlow Max=0.02 cfs @ 13.64 hrs HW=84.96' (Free Discharge)
1=Broad-Crested Rectangular Weir (Weir Controls 0.02 cfs @ 0.26 fps)

Secondary OutFlow Max=0.02 cfs @ 13.64 hrs HW=84.96' (Free Discharge)
3=Culvert (Passes 0.02 cfs of 0.73 cfs potential flow)
2=Orifice/Grate (Controls 0.00 cfs)
4=Exfiltration (Exfiltration Controls 0.02 cfs)

Summary for Pond BR4: Post - Bio-retention Basin-4

Inflow Area = 6,131 sf, 48.88% Impervious, Inflow Depth = 1.51" for 1-yr event
Inflow = 0.23 cfs @ 12.12 hrs, Volume= 770 cf
Outflow = 0.18 cfs @ 12.22 hrs, Volume= 516 cf, Atten= 21%, Lag= 5.6 min
Primary = 0.18 cfs @ 12.22 hrs, Volume= 516 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 83.83' @ 12.22 hrs Surf.Area= 570 sf Storage= 277 cf
Flood Elev= 84.00' Surf.Area= 594 sf Storage= 325 cf

Plug-Flow detention time= 164.1 min calculated for 516 cf (67% of inflow)
Center-of-Mass det. time= 65.7 min (888.7 - 823.0)

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Volume	Invert	Avail.Storage	Storage Description
#1	83.00'	227 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	82.00'	98 cf	Filter Material (Prismatic) Listed below (Recalc)
			297 cf Overall x 33.0% Voids
		325 cf	Total Available Storage
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.00	157	0	0
84.00	297	227	227
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.00	297	0	0
83.00	297	297	297

Device	Routing	Invert	Outlet Devices
#1	Primary	83.75'	3.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.17 cfs @ 12.22 hrs HW=83.83' (Free Discharge)
t₁=Broad-Crested Rectangular Weir (Weir Controls 0.17 cfs @ 0.71 fps)

Summary for Pond DMH: post - DMH

Inflow Area = 5,112 sf, 100.00% Impervious, Inflow Depth = 3.98" for 1-yr event
Inflow = 0.12 cfs @ 12.40 hrs, Volume= 1,693 cf
Outflow = 0.12 cfs @ 12.40 hrs, Volume= 1,693 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.12 cfs @ 12.40 hrs, Volume= 1,693 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 83.00' @ 12.40 hrs
Flood Elev= 85.00'

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Device	Routing	Invert	Outlet Devices
#1 Primary	82.80'	8.0" Round Culvert	L= 92.0' RCP, sq cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 82.80' / 82.00' S= 0.0087' / Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=0.12 cfs @ 12.40 hrs HW=83.00 (Free Discharge)
↑=Culvert (Barrel Controls 0.12 cfs @ 2.05 fps)

Summary for Pond FB1: Post - Forebay-1

Inflow Area = 12,569 sf, 42.19% Impervious, Inflow Depth = 1.54" for 1-yr event
Inflow = 0.41 cfs @ 12.14 hrs, Volume= 1,610 cf
Outflow = 0.40 cfs @ 12.16 hrs, Volume= 1,438 cf, Atten= 1%, Lag= 1.3 min
Primary = 0.40 cfs @ 12.16 hrs, Volume= 1,438 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 86.14' @ 12.16 hrs Surf.Area= 278 sf Storage= 210 cf
Flood Elev= 86.50' Surf.Area= 350 sf Storage= 323 cf

Plug-Flow detention time= 93.2 min calculated for 1,437 cf (89% of inflow)
Center-of-Mass det. time= 40.2 min (836.1 - 795.9)

Volume	Invert	Avail Storage	Storage Description	Custom Stage Data (Prismatic) Listed below (Recalc)
#1	85.00'	323 cf		
Elevation	Surf Area (sq-ft)	Inc. Store (cubic-feet)	Cum. Store (cubic-feet)	
85.00	95	0	0	
86.00	250	173	173	
86.50	350	150	323	

Device	Routing	Invert	Outlet Devices	#1 Primary	86.00'	3.0 long x 2.0 breadth Broad-Crested Rectangular Weir	Head (feet)	0.20	0.40	0.60	0.80	1.00	1.20	1.40	1.60	1.80	2.00	2.50	3.00	3.50
							Coef. (English)	2.54	2.61	2.61	2.60	2.66	2.70	2.77	2.89	2.88	2.85	3.07	3.20	3.32

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Primary OutFlow Max=0.40 cfs @ 12.16 hrs HW=86.14' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.40 cfs @ 0.95 fps)

Summary for Pond FB2: Post - Forebay-2

Inflow Area = 4,810 sf, 62.31% Impervious, Inflow Depth = 1.90" for 1-yr event
Inflow = 0.20 cfs @ 12.11 hrs, Volume= 760 cf
Outflow = 0.20 cfs @ 12.12 hrs, Volume= 684 cf, Atten= 1%, Lag= 0.5 min
Primary = 0.20 cfs @ 12.12 hrs, Volume= 684 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 83.87' @ 12.12 hrs Surf.Area= 133 sf Storage= 86 cf
Flood Elev= 84.00' Surf.Area= 143 sf Storage= 103 cf

Plug-Flow detention time= 88.3 min calculated for 684 cf (90% of inflow)
Center-of-Mass det. time= 38.1 min (816.7 - 778.6)

Volume	Invert	Avail.Storage	Storage Description	Custom Stage Data (Prismatic) Listed below (Recalc)
#1	83.00'	103 cf		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
83.00 84.00	63 143	0 103	0 103	

Device	Routing	Invert	Outlet Devices	Broad-Crested	Rectangular	Weir
#1	Primary	83.80'	4.0' long x 2.0' breadth	Head (feet)	Coef. (English)	

Primary OutFlow Max=0.20 cfs @ 12.12 hrs HW=83.87' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.20 cfs @ 0.69 fps)

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Summary for Pond FB3: Post - Forebay-3

Inflow Area = 18,298 sf, 49.99% Impervious, Inflow Depth = 2.29" for 1-yr event
Inflow = 0.58 cfs @ 12.08 hrs, Volume= 3,486 cf
Outflow = 0.57 cfs @ 12.11 hrs, Volume= 3,294 cf, Atten= 3%, Lag= 1.2 min
Primary = 0.57 cfs @ 12.11 hrs, Volume= 3,294 cf

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 82.18' @ 12.11 hrs Surf.Area= 312 sf Storage= 243 cf
Flood Elev= 82.50' Surf.Area= 400 sf Storage= 358 cf

Plug-Flow detention time= 81.6 min calculated for 3,294 cf (94% of inflow)
Center-of-Mass det. time= 31.3 min (95.9 - 928.2)

Volume	Invert	Avail.Storage	Storage Description
#1	81.00'	358 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.00	121	0	0
82.00	263	192	192
82.50	400	166	358

Device	Routing	Invert	Outlet Devices
#1	Primary	82.00'	3'0" long x 2'0" breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.56 cfs @ 12.11 hrs HW=82.18' (Free Discharge)
↳₁=Broad-Crested Rectangular Weir (Weir Controls 0.56 cfs @ 1.06 fps)

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Summary for Pond pA1: Analysis Point - Post Dev Catch Basin in Hope St.

Inflow Area = 16,759 sf, 0.73% Impervious, Inflow Depth = 0.09" for 1-yr event
Inflow = 0.02 cfs @ 13.64 hrs, Volume= 126 cf
Primary = 0.02 cfs @ 13.64 hrs, Volume= 126 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Pond pA2: Analysis Point - Post Dev Flow to Wetland

Inflow Area = 62,928 sf, 41.05% Impervious, Inflow Depth = 1.25" for 1-yr event
Inflow = 0.39 cfs @ 12.67 hrs, Volume= 6,541 cf
Primary = 0.39 cfs @ 12.67 hrs, Volume= 6,541 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Subcatchment pB1: Post - Runoff to Detention Basin-1

Runoff = 0.05 cfs @ 12.09 hrs, Volume= 176 cf, Depth= 0.78"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description
2,690	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
*	0	Walkway
*	0	Roofs, HSG C
2,690	74	Weighted Average
2,690	74	100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pF1: Post - Runoff to Forebay-1

Runoff = 0.41 cfs @ 12.14 hrs, Volume= 1,610 cf, Depth= 1.54"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description
4,205	98	Paved parking, HSG C
7,266	74	>75% Grass cover, Good, HSG C
*	979	Walkway
*	119	Patio/Deck/Stairs
12,569	84	Weighted Average
7,266	74	57.81% Pervious Area
5,303	98	42.19% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
9.5	44	0.0300
0.5	82	0.0180
10.0	126	Total
Velocity (ft/sec)	Capacity (cfs)	Description
0.08	0.0272	Sheet Flow, AB
2.72	0.0203	Grass: Bermuda n= 0.410 P2= 3.30"
		Shallow Concentrated Flow, BC
		Paved Kv= 20.3 fps

Summary for Subcatchment pF2: Post - Runoff to Forebay-2

Runoff = 0.20 cfs @ 12.11 hrs, Volume= 760 cf, Depth= 1.90"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

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Area (sf)			CN	Description
1,813	74	>75%	Grass cover, Good, HSG C	
2,847	98	Paved parking, HSG C		
*	150	98	Walkway	
0	98	Roofs, HSG C		
4,810	89	Weighted Average		
1,813	74	37.69% Pervious Area		
2,997	98	62.31% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)
7.9	40	0.0400	0.08	Sheet Flow, AB
0.1	24	0.0200	2.87	Grass: Bermuda n= 0.410 P2= 3.30" Shallow Concentrated Flow, BC
8.0	64	Total		Paved Kv= 20.3 fps

Summary for Subcatchment pS1: Post - Runoff to Bio-retention Basin

$$\text{Runoff} = 0.06 \text{ cfs} @ 12.09 \text{ hrs, Volume=} 186 \text{ cf, Depth=} 0.78"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)			CN	Description
2,851	74	>75%	Grass cover, Good, HSG C	
*	0	98	Patio/Deck/Stairs	
2,851	74	Weighted Average		
2,851	74	100.00% Pervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)
5.0				Direct Entry, Runoff to swale

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Summary for Subcatchment pS2: Post - Runoff to Swale-1

$$\text{Runoff} = 0.12 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 409 \text{ cf, Depth=} 0.95"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description			
0	98	Paved parking, HSG C			
4,696	74	>75% Grass cover, Good, HSG C			
*	0	Walkway			
*	477	Patio/Deck/Stairs			
5,173	76	Weighted Average			
4,696	74	90.78% Pervious Area			
477	98	9.22% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Parking Runoff

Summary for Subcatchment pS3: Post - Flow to wetlands

$$\text{Runoff} = 0.07 \text{ cfs} @ 12.34 \text{ hrs, Volume=} 373 \text{ cf, Depth=} 0.78"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description
5,708	74	>75% Grass cover, Good, HSG C
0	98	Paved parking, HSG C
0	70	Woods, Good, HSG C
5,708	74	Weighted Average
5,708	74	100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.2	50	0.0260	0.07		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
8.6	183	0.0050	0.35		Shallow Concentrated Flow, BC
					Woodland Kv= 5.0 fps
1.9	117	0.0400	1.00		Shallow Concentrated Flow, CD
					Woodland Kv= 5.0 fps
0.3	103	0.0300	5.33	26.67	Channel Flow, DE
					Area= 5.0 sf Perim= 10.2' r= 0.49'
					n= 0.030 Earth, grassed & winding
22.1	453	Total			

Summary for Subcatchment pS4: Post - Runoff to Bio-retention Basin-4

Runoff = 0.02 cfs @ 12.13 hrs, Volume= 86 cf, Depth= 0.78"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description			
1,321	74	>75% Grass cover, Good, HSG C			
*	0	Paved parking, HSG C			
*	0	Walkway			
*	0	Roofs, HSG C			
1,321	74	Weighted Average			
1,321	74	100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	40	0.0400	0.08		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
0.1	24	0.0200	2.87		Shallow Concentrated Flow, BC
					Paved Kv= 20.3 fps
8.0	64	Total			

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Summary for Subcatchment pS5: Post - Runoff to Bio-retention Basin-2

$$\text{Runoff} = 0.05 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 156 \text{ cf, Depth=} 1.01"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description			
0	98	Paved parking, HSG C			
1,602	74	>75% Grass cover, Good, HSG C			
*	0	Walkway			
*	238	Patio/Deck/Stairs			
0	98	Roofs, HSG C			
1,840	77	Weighted Average			
1,602	74	87.07% Pervious Area			
238	98	12.93% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pS6: Post - Runoff to Forebay-3

$$\text{Runoff} = 0.46 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 1,559 \text{ cf, Depth=} 1.37"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description
3,996	98	Paved parking, HSG C
9,151	74	>75% Grass cover, Good, HSG C
*	493	Walkway
13,640	82	Weighted Average
9,151	74	67.09% Pervious Area
4,489	98	32.91% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pS7: Post - Flow to Hope Street

Runoff = 0.18 cfs @ 12.48 hrs, Volume= 1,113 cf, Depth= 0.80"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description
16,637	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
*	0	Walkways
*	122	Patio/Deck/Stairs
16,759	74	Weighted Average
16,637	74	99.27% Pervious Area
122	98	0.73% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
30.6	60	0.0030
0.2	44	0.0450
30.8	104	Total
		0.03
		3.42
		0.0410 P2= 3.30"
		Shallow Concentrated Flow, BC
		Unpaved Kv= 16.1 fps
		Sheet Flow, AB

Summary for Subcatchment RF1: Post - Roof Area (Units 3&4)

Runoff = 0.16 cfs @ 12.07 hrs, Volume= 547 cf, Depth= 2.57"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

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Area (sf)	CN	Description
2,556	98	Roofs, HSG C

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF2: Post - Roof Area (Units 5&6)

$$\text{Runoff} = 0.16 \text{ cfs} @ 12.07 \text{ hrs, Volume=} 547 \text{ cf, Depth=} 2.57"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF3: Post - Roof Area (Units 7&8)

$$\text{Runoff} = 0.16 \text{ cfs} @ 12.07 \text{ hrs, Volume=} 547 \text{ cf, Depth=} 2.57"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF4: Post- Roof Area (Units 9&10)

Runoff = 0.13 cfs @ 12.07 hrs, Volume= 450 cf, Depth= 2.57"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description
2,102	98	Roofs, HSG C
2,102	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF5: Post - Roof Area (Units 1&2)

Runoff = 0.16 cfs @ 12.07 hrs, Volume= 547 cf, Depth= 2.57"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 1-yr Rainfall=2.80"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

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Summary for Reach S1: Post - Dry Swale 1

Inflow Area = 20,593 sf, 28.07% Impervious, Inflow Depth = 0.24" for 1-yr event
Inflow = 0.12 cfs @ 12.08 hrs, Volume= 409 cf
Outflow = 0.11 cfs @ 12.16 hrs, Volume= 409 cf, Atten= 8%, Lag= 4.8 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Max. Velocity= 0.89 fps, Min. Travel Time= 2.8 min
Avg. Velocity = 0.30 fps, Avg. Travel Time= 8.3 min

Peak Storage= 19 cf @ 12.12 hrs

Average Depth at Peak Storage= 0.06'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 17.43 cfs

Custom cross-section, Length= 150.0' Slope= 0.0200 '/'
Constant n= 0.035 Earth, dense weeds
Inlet Invert= 85.00', Outlet Invert= 82.00'



Offset (feet)	Elevation (feet)	Chan.Depth (feet)	Storage (cubic-feet)	Discharge (cfs)
Depth (feet)	End Area (sq-ft)	Perim. (feet)		
-3.00	1.00	0.00	0	0.00
-1.00	0.00	1.00	0	0.00
1.00	0.00	1.00	600	600
3.00	1.00	0.00	0	17.43

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Summary for Pond SB: Post - Settling Basin

Inflow Area = 25,705 sf, 42.37% Impervious, Inflow Depth = 0.98" for 1-yr event
Inflow = 0.20 cfs @ 12.21 hrs, Volume= 2,102 cf
Outflow = 0.17 cfs @ 12.44 hrs, Volume= 1,883 cf, Atten= 15%, Lag= 13.8 min
Primary = 0.17 cfs @ 12.44 hrs, Volume= 1,883 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 82.06' @ 12.44 hrs Surf.Area= 347 sf Storage= 241 cf

Plug-Flow detention time= 143.1 min calculated for 1,882 cf (90% of inflow)
Center-of-Mass det. time= 53.2 min (1,188.1 - 1,134.9)

Volume	Invert	Avail Storage	Storage Description
#1	81.00'	416 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.00	108	0	0
82.00	331	220	220
82.50	456	197	416

Device	Routing	Invert	Outlet Devices
#1	Primary	82.00'	4.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61

Primary OutFlow Max=0.17 cfs @ 12.44 hrs HW=82.06' (Free Discharge)
↑**_1=Broad-Crested Rectangular Weir** (Weir Controls 0.17 cfs @ 0.65 fps)

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Summary for Pond US1: Post - Recharge System-1

Inflow Area =	5,112 sf, 100.00% Impervious, Inflow Depth = 2.57"	for 1-yr event
Inflow =	0.32 cfs @ 12.07 hrs, Volume= 1,094 cf	
Outflow =	0.11 cfs @ 12.33 hrs, Volume= 1,094 cf, Atten= 66%, Lag= 15.4 min	
Discarded =	0.01 cfs @ 8.60 hrs, Volume= 764 cf	
Primary =	0.10 cfs @ 12.33 hrs, Volume= 330 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 85.04' @ 12.33 hrs Surf.Area= 715 sf Storage= 423 cf
Flood Elev= 86.30' Surf.Area= 715 sf Storage= 837 cf

Plug-Flow detention time= 263.3 min calculated for 1,094 cf (100% of inflow)
Center-of-Mass det. time= 263.4 min (1,021.8 - 758.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	84.00'	405 cf	21.50'W x 33.25'L x 2.33'H Field A 1,668 cf Overall - 439 cf Embedded = 1,229 cf x 33.0% Voids
#2A	84.50'	439 cf	Infiltrator H-20 Chamber x 30 Inside #1 Effective Size= 27.5"W x 14.0"H => 2.34 sf x 6.25'L = 14.6 cf Overall Size= 34.0"W x 16.0"H x 6.25'L
		845 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	84.00'	0.500 in/hr Exfiltration over Surface area
#2	Primary	84.85'	6.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 84.85' / 84.60' S= 0.0208 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Discarded OutFlow Max=0.01 cfs @ 8.60 hrs HW=84.02' (Free Discharge)
↑1=Exfiltration (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.10 cts @ 12.33 hrs HW=85.04' (Free Discharge)
↑2=Culvert (Inlet Controls 0.10 cfs @ 1.49 fps)

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Summary for Pond US2: Post - Underground Storage System

Inflow Area = 4,658 sf, 100.00% Impervious, Inflow Depth = 2.57" for 1-yr event
Inflow = 0.30 cfs @ 12.07 hrs, Volume= 997 cf
Outflow = 0.15 cfs @ 12.21 hrs, Volume= 997 cf, Atten= 51%, Lag= 8.3 min
Primary = 0.15 cfs @ 12.21 hrs, Volume= 997 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 83.44' @ 12.21 hrs Surf.Area= 998 sf Storage= 203 cf

Plug-Flow detention time= 33.7 min calculated for 997 cf (100% of inflow)
Center-of-Mass det. time= 33.9 min (792.3 - 758.4)

Volume	Invert	Avail Storage	Storage Description
#1	83.15'	1,484 cf	18.0" Round Pipe Storage x 14 L= 60.0

Device	Routing	Invert	Outlet Devices
#1	Primary	83.15'	4.0" Round Culvert L= 100.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 83.15' / 82.10' S= 0.0105 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.09 sf

Primary OutFlow Max=0.14 cfs @ 12.21 hrs HW=83.44' (Free Discharge)
↳ 1=Culvert (Barrel Controls 0.14 cfs @ 2.39 fps)

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Time span=0.00-72.00 hrs, dt=0.04 hrs, 1801 points

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.

Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Pond B1: Post - Dry Extended Detention Basin

Peak Elev=80.81' Storage=1,161 cf Inflow=1.21 cfs 6,343 cf
Primary=0.66 cfs 6,012 cf Secondary=0.00 cfs 0 cf Outflow=0.66 cfs 6,012 cf

Peak Elev=85.76' Storage=1,328 cf Inflow=0.57 cfs 2,114 cf
Primary=0.02 cfs 81 cf Secondary=0.02 cfs 1,771 cf Outflow=0.04 cfs 1,852 cf

Peak Elev=82.31' Storage=973 cf Inflow=0.98 cfs 4,854 cf
Primary=0.84 cfs 2,685 cf Secondary=0.02 cfs 2,093 cf Outflow=0.86 cfs 4,778 cf

Peak Elev=84.99' Storage=528 cf Inflow=0.27 cfs 1,562 cf
Primary=0.16 cfs 501 cf Secondary=0.02 cfs 1,004 cf Outflow=0.17 cfs 1,505 cf

Peak Elev=83.86' Storage=284 cf Inflow=0.28 cfs 978 cf
Outflow=0.27 cfs 723 cf

Peak Elev=83.06' Inflow=0.20 cfs 2,272 cf
8.0" Round Culvert n=0.013 L=92.0' S=0.0087 '/' Outflow=0.20 cfs 2,272 cf

Peak Elev=86.16' Storage=216 cf Inflow=0.52 cfs 2,024 cf
Outflow=0.51 cfs 1,851 cf

Peak Elev=83.88' Storage=87 cf Inflow=0.25 cfs 933 cf
Outflow=0.25 cfs 857 cf

Peak Elev=82.21' Storage=253 cf Inflow=0.75 cfs 4,184 cf
Outflow=0.73 cfs 3,992 cf

Inflow=0.16 cfs 501 cf
Primary=0.16 cfs 501 cf

Inflow=0.76 cfs 8,631 cf
Primary=0.76 cfs 8,631 cf

Pond pA1: Analysis Point - Post Dev Catch Basin in Hope St.

Pond pA2: Analysis Point - Post Dev Flow to Wetland

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Subcatchment pB1: Post - Runoff to Detention Basin-1

Runoff Area=2,690 sf 0.00% Impervious Runoff Depth=1.10"
Tc=5.0 min CN=74/0 Runoff=0.08 cfs 247 cf
Runoff Area=12,569 sf 42.19% Impervious Runoff Depth=1.93"
Flow Length=126' Tc=10.0 min CN=74/98 Runoff=0.52 cfs 2,024 cf

Subcatchment pF1: Post - Runoff to Forebay-1

Runoff Area=4,810 sf 62.31% Impervious Runoff Depth=2.33"
Flow Length=64' Tc=8.0 min CN=74/98 Runoff=0.25 cfs 933 cf

Subcatchment pF2: Post - Runoff to Forebay-2

Runoff Area=2,851 sf 0.00% Impervious Runoff Depth=1.10"
Tc=5.0 min CN=74/0 Runoff=0.08 cfs 262 cf

Subcatchment pS1: Post - Runoff to Bio-retention Basin

Runoff Area=5,173 sf 9.22% Impervious Runoff Depth=1.28"
Tc=5.0 min CN=74/98 Runoff=0.17 cfs 554 cf

Subcatchment pS2: Post - Runoff to Swale-1

Runoff Area=5,708 sf 0.00% Impervious Runoff Depth=1.10"
Flow Length=453' Tc=22.1 min CN=74/0 Runoff=0.10 cfs 525 cf

Subcatchment pS3: Post - Flow to wetlands

Runoff Area=1,321 sf 0.00% Impervious Runoff Depth=1.10"
Flow Length=64' Tc=8.0 min CN=74/0 Runoff=0.03 cfs 122 cf

Subcatchment pS4: Post - Runoff to Bio-retention Basin-4

Runoff Area=1,840 sf 12.93% Impervious Runoff Depth=1.36"
Tc=5.0 min CN=74/98 Runoff=0.06 cfs 208 cf

Subcatchment pS5: Post - Runoff to Bio-retention Basin-2

Runoff Area=13,640 sf 32.91% Impervious Runoff Depth=1.75"
Tc=5.0 min CN=74/98 Runoff=0.60 cfs 1,989 cf

Subcatchment pS6: Post - Runoff to Forebay-3

Runoff Area=16,759 sf 0.73% Impervious Runoff Depth=1.12"
Flow Length=104' Tc=30.8 min CN=74/98 Runoff=0.27 cfs 1,562 cf

Subcatchment RF1: Post - Roof Area (Units 3&4)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=3.07"
Tc=5.0 min CN=0/98 Runoff=0.19 cfs 653 cf

Subcatchment RF2: Post - Roof Area (Units 5&6)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=3.07"
Tc=5.0 min CN=0/98 Runoff=0.19 cfs 653 cf

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Subcatchment RF3: Post - Roof Area (Units 7&8)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=3.07"
Tc=5.0 min CN=0/98 Runoff=0.19 cfs 653 cf

Subcatchment RF4: Post- Roof Area (Units 9&10)

Runoff Area=2,102 sf 100.00% Impervious Runoff Depth=3.07"
Tc=5.0 min CN=0/98 Runoff=0.16 cfs 537 cf

Subcatchment RF5: Post - Roof Area (Units 1&2)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=3.07"
Tc=5.0 min CN=0/98 Runoff=0.19 cfs 653 cf

Reach S1: Post - Dry Swale 1

Avg. Flow Depth=0.07' Max Vel=1.01 fps Inflow=0.17 cfs 635 cf
n=0.035 L=150.0' S=0.0200 '/' Capacity=17.43 cfs Outflow=0.16 cfs 635 cf

Pond SB: Post - Settling Basin

Peak Elev=82.10' Storage=253 cf Inflow=0.35 cfs 2,907 cf
Outflow=0.33 cfs 2,687 cf

Pond US1: Post - Recharge System-1

Peak Elev=85.11' Storage=461 cf Inflow=0.38 cfs 1,307 cf
Discarded=0.01 cfs 806 cf Primary=0.18 cfs 501 cf Outflow=0.19 cfs 1,307 cf

Pond US2: Post - Underground Storage System

Peak Elev=83.48' Storage=239 cf Inflow=0.35 cfs 1,191 cf
4.0" Round Culvert n=0.013 L=100.0' S=0.0105 '/' Outflow=0.17 cfs 1,191 cf

Total Runoff Area = 79,687 sf Runoff Volume = 11,577 cf Average Runoff Depth = 1.74"
67.43% Pervious = 53,735 sf 32.57% Impervious = 25,952 sf

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Summary for Pond B1: Post - Dry Extended Detention Basin

Inflow Area = 57,220 sf, 45.14% Impervious, Inflow Depth = 1.33" for 2-yr event
 Inflow = 1.21 cfs @ 12.24 hrs, Volume= 6,343 cf
 Outflow = 0.66 cfs @ 12.51 hrs, Volume= 6,012 cf, Atten= 45%, Lag= 16.7 min
 Primary = 0.66 cfs @ 12.51 hrs, Volume= 6,012 cf
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
 Peak Elev= 80.81' @ 12.51 hrs Surf.Area= 1,587 sf Storage= 1,161 cf
 Flood Elev= 82.50' Surf Area= 2,300 sf Storage= 4,432 cf

Plug-Flow detention time= 131.1 min calculated for 6,009 cf (95% of inflow)
 Center-of-Mass det. time= 72.4 min (1,082.4 - 1,010.0)

Volume	Invert	Avail.Storage	Storage Description
#1	80.00'	4,432 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.00	1,273	0	0
81.00	1,660	1,467	1,467
82.00	2,080	1,870	3,337
82.50	2,300	1,095	4,432
Device	Routing	Invert	Outlet Devices
#1	Secondary	82.00'	4'0" long x 2'0" breadth Broad-Crested Weir
		Head (feet)	0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50
		Coef. (English)	2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Primary	80.45'	8.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500
		Inlet / Outlet Invert= 80.45' / 79.50'	S= 0.0792' Cc= 0.900
		n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf	
#3	Primary	80.25'	4.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500
		Inlet / Outlet Invert= 80.25' / 79.50'	S= 0.0625' Cc= 0.900
		n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.09 sf	

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Primary OutFlow Max=0.66 cfs @ 12.51 hrs HW=80.81' (Free Discharge)
 └─2=Culvert (Inlet Controls 0.40 cfs @ 2.05 fps)
 └─3=Culvert (Inlet Controls 0.26 cfs @ 3.03 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=80.00' (Free Discharge)
 └─1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond BR1: Post - Bio-retention Basin-1

Inflow Area = 15,420 sf, 34.39% Impervious, Inflow Depth = 1.64" for 2-yr event
Inflow = 0.57 cfs @ 12.15 hrs, Volume= 2,114 cf
Outflow = 0.04 cfs @ 14.41 hrs, Volume= 1,852 cf, Atten= 93%, Lag= 135.7 min
Primary = 0.02 cfs @ 14.41 hrs, Volume= 81 cf
Secondary = 0.02 cfs @ 14.41 hrs, Volume= 1,771 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 85.76' @ 14.41 hrs Surf.Area= 2,021 sf Storage= 1,328 cf
Flood Elev= 86.00' Surf.Area= 2,110 sf Storage= 1,567 cf

Plug-Flow detention time= 609.7 min calculated for 1,851 cf (88% of inflow)
Center-of-Mass det. time= 553.1 min (1,385.9 - 832.8)

Volume	Invert	Avail.Storage	Storage Description
#1	85.00'	867 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	83.00'	694 cf	Filter Material (Prismatic) Listed below (Recalc)
#3	83.75'	7 cf	2,110 cf Overall - 7 cf Embedded = 2,103 cf x 33.0% Voids 6.0" Round Pipe Storage Inside #2 L= 35.0'
1,567 cf Total Available Storage			
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
85.00	678	0	0
86.00	1,055	867	867

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.00	1,055	0	0
85.00	1,055	2,110	2,110

Device	Routing	Invert	Outlet Devices
#1 Primary	85.75'	4.0' long x 2.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 Coef. (English) 2.48 2.60 2.60 2.64 2.65 2.68 2.75 2.74 2.76 2.89 3.05 3.19 3.32	

#2 Device 3 83.00'
#3 Secondary 83.75'
0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'
6.0" Round Culvert L= 100.0' RCP, sq.cut end projecting, Ke= 0.500
Inlet / Outlet Invert= 83.75' / 82.85' S= 0.0090' Cc= 0.900
n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.01 cfs @ 14.41 hrs HW=85.76' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.01 cfs @ 0.28 fps)

Secondary OutFlow Max=0.02 cfs @ 14.41 hrs HW=85.76' (Free Discharge)
↳ 3=Culvert (Passes 0.02 cfs of 0.80 cfs potential flow)
↳ 2=Exfiltration (Exfiltration Controls 0.02 cfs)

Summary for Pond BR2: Post - Bio-retention Basin-2

Inflow Area = 22,694 sf, 52.62% Impervious, Inflow Depth = 2.57" for 2-yr event
Inflow = 0.98 cfs @ 12.09 hrs, Volume= 4,854 cf
Outflow = 0.86 cfs @ 12.14 hrs, Volume= 4,778 cf, Atten= 12%, Lag= 2.9 min
Primary = 0.84 cfs @ 12.14 hrs, Volume= 2,685 cf
Secondary = 0.02 cfs @ 12.14 hrs, Volume= 2,093 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 82.31' @ 12.14 hrs Surf.Area= 1,737 sf Storage= 973 cf
Flood Elev= 82.50' Surf.Area= 1,796 sf Storage= 1,140 cf

Plug-Flow detention time= 244.9 min calculated for 4,775 cf (98% of inflow)
Center-of-Mass det. time= 228.6 min (1,144.5 - 916.0)

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Volume	Invert	Avail.Storage	Storage Description
#1	81.35'	836 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	80.35'	292 cf	Filter Material (Prismatic) Listed below (Recalc)
#3	80.60'	12 cf	6.0" Round Pipe Storage Inside #2 L= 61.0'

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.35	563	0	0
82.00	745	425	425
82.50	898	411	836

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.35	898	0	0
81.35	898	898	898

Device	Routing	Invert	Outlet Devices
#1	Primary	82.10'	3.5' long x 2'0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Device 3	80.35'	0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'
#3	Secondary	80.60'	6.0" Round Culvert L= 62.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 80.60' / 79.00' S= 0.0258 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.83 cfs @ 12.14 hrs HW=82.31' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.83 cfs @ 1.15 fps)

Secondary OutFlow Max=0.02 cfs @ 12.14 hrs HW=82.31' (Free Discharge)
↳ 2=Culvert (Passes 0.02 cfs of 1.04 cfs potential flow)
↳ 3=Exfiltration (Exfiltration Controls 0.02 cfs)

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Summary for Pond BR3: Post - Bio-retention Basin-3

Inflow Area = 16,759 sf, 0.73% Impervious, Inflow Depth = 1.12" for 2-yr event
Inflow = 0.27 cfs @ 12.46 hrs, Volume= 1,562 cf
Outflow = 0.17 cfs @ 12.79 hrs, Volume= 1,505 cf, Atten= 35%, Lag= 19.9 min
Primary = 0.16 cfs @ 12.79 hrs, Volume= 501 cf
Secondary = 0.02 cfs @ 12.79 hrs, Volume= 1,004 cf

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 84.99' @ 12.79 hrs Surf.Area= 1,323 sf Storage= 528 cf

Plug-Flow detention time= 291.9 min calculated for 1,505 cf (96% of inflow)
Center-of-Mass det. time= 271.8 min (1,152.9 - 881.1)

Volume	Invert	Avail.Storage	Storage Description
#1	84.40'	371 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	83.40'	224 cf	Filter Material (Prismatic) Listed below (Recalc)
			686 cf Overall - 6 cf Embedded = 680 cf x 33.0% Voids
#3	83.65'	6 cf	6.0" Round Pipe Storage Inside #2 L= 30.0'
		601 cf	Total Available Storage
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
84.40	374	0	0
85.10	686	371	371
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.40	686	0	0
84.40	686	686	686

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Device Routing Invert Outlet Devices

Device	Routing	Invert	Outlet Devices
#1	Primary	84.95'	8.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Device 3	85.00'	6.0" Horiz. Orifice/Grate C=0.600 Limited to weir flow at low heads
#3	Secondary	83.65'	6.0" Round Culvert L= 68.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 83.65' / 83.00' S= 0.0096 ' Cc= 0.900
#4	Device 3	83.40'	n= 0.013 Concrete pipe, bends & connections, Flow Area= 0.20 sf 0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'

Primary OutFlow Max=0.16 cfs @ 12.79 hrs HW=84.99' (Free Discharge)
↳ **1=Broad-Crested Rectangular Weir** (Weir Controls 0.16 cfs @ 0.50 fps)

Secondary OutFlow Max=0.02 cfs @ 12.79 hrs HW=84.99' (Free Discharge)
↳ **3=Culvert** (Passes 0.02 cfs of 0.73 cfs potential flow)
↳ **2=Orifice/Grate** (Controls 0.00 cfs)
↳ **4=Exfiltration** (Exfiltration Controls 0.02 cts)

Summary for Pond BR4: Post - Bio-retention Basin-4

Inflow Area = 6,131 sf, 48.88% Impervious, Inflow Depth = 1.91" for 2-yr event
Inflow = 0.28 cfs @ 12.12 hrs, Volume= 978 cf
Outflow = 0.27 cfs @ 12.15 hrs, Volume= 723 cf, Atten= 5%, Lag= 1.8 min
Primary = 0.27 cfs @ 12.15 hrs, Volume= 723 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 83.86' @ 12.15 hrs Surf.Area= 574 sf Storage= 284 cf
Flood Elev= 84.00' Surf.Area= 594 sf Storage= 325 cf

Plug-Flow detention time= 140.3 min calculated for 723 cf (74% of inflow)
Center-of-Mass det. time= 52.2 min (868.9 - 816.7)

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Volume	Invert	Avail.Storage	Storage Description
#1	83.00'	227 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	82.00'	98 cf	Filter Material (Prismatic) Listed below (Recalc)
			297 cf Overall x 33.0% Voids
		325 cf	Total Available Storage
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.00	157	0	0
84.00	297	227	227
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.00	297	0	0
83.00	297	297	297

Device	Routing	Invert	Outlet Devices
#1	Primary	83.75'	3.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.26 cfs @ 12.15 hrs HW=83.86' (Free Discharge)
†=Broad-Crested Rectangular Weir (Weir Controls 0.26 cfs @ 0.83 fps)

Summary for Pond DMH: post - DMH

Inflow Area = 5,112 sf, 100.00% Impervious, Inflow Depth = 5.33" for 2-yr event
Inflow = 0.20 cfs @ 12.23 hrs, Volume= 2,272 cf
Outflow = 0.20 cfs @ 12.23 hrs, Volume= 2,272 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.20 cfs @ 12.23 hrs, Volume= 2,272 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 83.06' @ 12.23 hrs
Flood Elev= 85.00'

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Device	Routing	Invert	Outlet Devices
#1 Primary	82.80'	8.0" Round Culvert	L= 92.0' RCP, sq cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 82.80' / 82.00' S= 0.0087' / Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=0.20 cfs @ 12.23 hrs HW=83.06' (Free Discharge)
↑=Culvert (Barrel Controls 0.20 cfs @ 2.36 fps)

Summary for Pond FB1: Post - Forebay-1

Inflow Area = 12,569 sf, 42.19% Impervious, Inflow Depth = 1.93" for 2-yr event
Inflow = 0.52 cfs @ 12.14 hrs, Volume= 2,024 cf
Outflow = 0.51 cfs @ 12.16 hrs, Volume= 1,851 cf, Atten= 1%, Lag= 1.2 min
Primary = 0.51 cfs @ 12.16 hrs, Volume= 1,851 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 86.16' @ 12.16 hrs Surf.Area= 283 sf Storage= 216 cf
Flood Elev= 86.50' Surf.Area= 350 sf Storage= 323 cf

Plug-Flow detention time= 80.4 min calculated for 1,851 cf (91% of inflow)
Center-of-Mass det. time= 35.1 min (829.1 - 793.9)

Volume	Invert	Avail Storage	Storage Description	Custom Stage Data (Prismatic) Listed below (Recalc)
#1	85.00'	323 cf		
Elevation	Surf Area (sq-ft)	Inc. Store (cubic-feet)	Cum. Store (cubic-feet)	
85.00	95	0	0	
86.00	250	173	173	
86.50	350	150	323	

Device	Routing	Invert	Outlet Devices	
#1 Primary	86.00'	3.0 long x 2.0 breadth Broad-Crested Rectangular Weir	Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32	

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Primary OutFlow Max=0.51 cfs @ 12.16 hrs HW=86.16' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.51 cfs @ 1.03 fps)

Summary for Pond FB2: Post - Forebay-2

Inflow Area = 4,810 sf, 62.31% Impervious, Inflow Depth = 2.33" for 2-yr event
Inflow = 0.25 cfs @ 12.11 hrs, Volume= 933 cf
Outflow = 0.25 cfs @ 12.12 hrs, Volume= 857 cf, Atten= 1%, Lag= 0.5 min
Primary = 0.25 cfs @ 12.12 hrs, Volume= 857 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 83.88' @ 12.12 hrs Surf.Area= 134 sf Storage= 87 cf
Flood Elev= 84.00' Surf.Area= 143 sf Storage= 103 cf

Plug-Flow detention time= 77.0 min calculated for 857 cf (92% of inflow)
Center-of-Mass det. time= 34.0 min (810.3 - 776.3)

Volume	Invert	Avail.Storage	Storage Description	Custom Stage Data (Prismatic) Listed below (Recalc)
#1	83.00'	103 cf		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
83.00	63	0	0	
84.00	143	103	103	

Device	Routing	Invert	Outlet Devices	4.0' long x 2.0' breadth Broad-Crested Rectangular Weir
#1	Primary	83.80'		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.25 cfs @ 12.12 hrs HW=83.88' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.25 cfs @ 0.74 fps)

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Summary for Pond FB3: Post - Forebay-3

Inflow Area = 18,298 sf, 49.99% Impervious, Inflow Depth = 2.74" for 2-yr event
Inflow = 0.75 cfs @ 12.08 hrs, Volume= 4,184 cf
Outflow = 0.73 cfs @ 12.10 hrs, Volume= 3,992 cf, Atten= 3%, Lag= 1.1 min
Primary = 0.73 cfs @ 12.10 hrs, Volume= 3,992 cf

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 82.21' @ 12.10 hrs Surf.Area= 320 sf Storage= 253 cf
Flood Elev= 82.50' Surf.Area= 400 sf Storage= 358 cf

Plug-Flow detention time= 72.1 min calculated for 3,990 cf (95% of inflow)
Center-of-Mass det. time= 27.7 min (946.9 - 919.2)

Volume	Invert	Avail.Storage	Storage Description
#1	81.00'	358 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.00	121	0	0
82.00	263	192	192
82.50	400	166	358

Device	Routing	Invert	Outlet Devices	Weir
#1	Primary	82.00'	3'0" long x 2'0" breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32	

Primary OutFlow Max=0.72 cfs @ 12.10 hrs HW=82.21' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.72 cfs @ 1.16 fps)

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Summary for Pond pA1: Analysis Point - Post Dev Catch Basin in Hope St.

Inflow Area = 16,759 sf, 0.73% Impervious, Inflow Depth = 0.36" for 2-yr event
Inflow = 0.16 cfs @ 12.79 hrs, Volume= 501 cf
Primary = 0.16 cfs @ 12.79 hrs, Volume= 501 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Pond pA2: Analysis Point - Post Dev Flow to Wetland

Inflow Area = 62,928 sf, 41.05% Impervious, Inflow Depth = 1.65" for 2-yr event
Inflow = 0.76 cfs @ 12.50 hrs, Volume= 8,631 cf
Primary = 0.76 cfs @ 12.50 hrs, Volume= 8,631 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Subcatchment pB1: Post - Runoff to Detention Basin-1

Runoff = 0.08 cfs @ 12.08 hrs, Volume= 247 cf, Depth= 1.10"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description
2,690	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
*	0	Walkway
*	0	Roofs, HSG C
2,690	74	Weighted Average
2,690	74	100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pF1: Post - Runoff to Forebay-1

Runoff = 0.52 cfs @ 12.14 hrs, Volume= 2,024 cf, Depth= 1.93"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description
4,205	98	Paved parking, HSG C
7,266	74	>75% Grass cover, Good, HSG C
*	979	Walkway
*	119	Patio/Deck/Stairs
12,569	84	Weighted Average
7,266	74	57.81% Pervious Area
5,303	98	42.19% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
9.5	44	0.0300
0.5	82	0.0180
10.0	126	Total
Velocity (ft/sec)	Capacity (cfs)	Description
0.08	0.0272	Sheet Flow, AB
2.72	0.0272	Grass: Bermuda n= 0.410 P2= 3.30"
		Shallow Concentrated Flow, BC
		Paved Kv= 20.3 fps

Summary for Subcatchment pF2: Post - Runoff to Forebay-2

Runoff = 0.25 cfs @ 12.11 hrs, Volume= 933 cf, Depth= 2.33"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 2-yr Rainfall=3.30"

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Type III 24-hr 2-yr Rainfall=3.30"
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Area (sf) CN Description				
1,813	74	>75% Grass cover, Good, HSG C		
2,847	98	Paved parking, HSG C		
*	150	Walkway		
0	98	Roofs, HSG C		
4,810	89	Weighted Average		
1,813	74	37.69% Pervious Area		
2,997	98	62.31% Impervious Area		
Tc Length Slope Velocity Capacity Description	(min) (feet) (ft/ft) (ft/sec)	(cfs)		
7.9	40	0.0400	0.08	Sheet Flow, AB
0.1	24	0.0200	2.87	Grass: Bermuda n= 0.410 P2= 3.30" Shallow Concentrated Flow, BC
8.0	64	Total		Paved Kv= 20.3 fps

Summary for Subcatchment pS1: Post - Runoff to Bio-retention Basin

$$\text{Runoff} = 0.08 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 262 \text{ cf, Depth=} 1.10"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 2-yr Rainfall=3.30"

Area (sf) CN Description				
2,851	74	>75% Grass cover, Good, HSG C		
*	0	Patio/Deck/Stairs		
2,851	74	Weighted Average		
2,851	74	100.00% Pervious Area		
Tc Length Slope Velocity Capacity Description	(min) (feet) (ft/ft) (ft/sec)	(cfs)		
5.0				Direct Entry, Runoff to swale

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Summary for Subcatchment pS2: Post - Runoff to Swale-1

Runoff = 0.17 cfs @ 12.08 hrs, Volume= 554 cf, Depth= 1.28"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description			
0	98	Paved parking, HSG C			
4,696	74	>75% Grass cover, Good, HSG C			
*	0	Walkway			
*	477	Patio/Deck/Stairs			
5,173	76	Weighted Average			
4,696	74	90.78% Pervious Area			
477	98	9.22% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Parking Runoff

Summary for Subcatchment pS3: Post - Flow to wetlands

Runoff = 0.10 cfs @ 12.33 hrs, Volume= 525 cf, Depth= 1.10"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description
5,708	74	>75% Grass cover, Good, HSG C
0	98	Paved parking, HSG C
0	70	Woods, Good, HSG C
5,708	74	Weighted Average
5,708	74	100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.2	50	0.0260	0.07		Sheet Flow, AB
8.6	183	0.0050	0.35		Grass: Bermuda n= 0.410 P2= 3.30" Shallow Concentrated Flow, BC
1.9	117	0.0400	1.00		Woodland Kv= 5.0 fps Shallow Concentrated Flow, CD
0.3	103	0.0300	5.33	26.67	Woodland Kv= 5.0 fps Channel Flow, DE Area= 5.0 sf Perim= 10.2' r= 0.49' n= 0.030 Earth, grassed & winding
22.1	453	Total			

Summary for Subcatchment pS4: Post - Runoff to Bio-retention Basin-4

Runoff = 0.03 cfs @ 12.12 hrs, Volume= 122 cf, Depth= 1.10"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description
1,321	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
*	0	Walkway
*	0	Roofs, HSG C
1,321	74	Weighted Average
1,321	74	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	40	0.0400	0.08		Sheet Flow, AB
0.1	24	0.0200	2.87		Grass: Bermuda n= 0.410 P2= 3.30" Shallow Concentrated Flow, BC
8.0	64	Total			Paved Kv= 20.3 fps

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Summary for Subcatchment pS5: Post - Runoff to Bio-retention Basin-2

Runoff = 0.06 cfs @ 12.08 hrs, Volume= 208 cf, Depth= 1.36"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description			
0	98	Paved parking, HSG C			
1,602	74	>75% Grass cover, Good, HSG C			
*	0	Walkway			
*	238	Patio/Deck/Stairs			
0	98	Roofs, HSG C			
1,840	77	Weighted Average			
1,602	74	87.07% Pervious Area			
238	98	12.93% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pS6: Post - Runoff to Forebay-3

Runoff = 0.60 cfs @ 12.08 hrs, Volume= 1,989 cf, Depth= 1.75"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description
3,996	98	Paved parking, HSG C
9,151	74	>75% Grass cover, Good, HSG C
*	493	Walkway
13,640	82	Weighted Average
9,151	74	67.09% Pervious Area
4,489	98	32.91% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pS7: Post - Flow to Hope Street

Runoff = 0.27 cfs @ 12.46 hrs, Volume= 1,562 cf, Depth= 1.12"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description
16,637	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
*	0	Walkways
*	122	Patio/Deck/Stairs
16,759	74	Weighted Average
16,637	74	99.27% Pervious Area
122	98	0.73% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
		(ft/sec)
30.6	60	0.0030
0.2	44	0.0450
30.8	104	Total
		0.03
		3.42
		0.19

Sheet Flow, AB
 Grass: Bermuda n= 0.410 P2= 3.30"
Shallow Concentrated Flow, BC
 Unpaved Kv= 16.1 fps

Summary for Subcatchment RF1: Post - Roof Area (Units 3&4)

Runoff = 0.19 cfs @ 12.07 hrs, Volume= 653 cf, Depth= 3.07"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 2-yr Rainfall=3.30"

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Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF2: Post - Roof Area (Units 5&6)

Runoff = 0.19 cfs @ 12.07 hrs, Volume= 653 cf, Depth= 3.07"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF3: Post - Roof Area (Units 7&8)

Runoff = 0.19 cfs @ 12.07 hrs, Volume= 653 cf, Depth= 3.07"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF4: Post- Roof Area (Units 9&10)

Runoff = 0.16 cfs @ 12.07 hrs, Volume= 537 cf, Depth= 3.07"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description
2,102	98	Roofs, HSG C
2,102	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF5: Post - Roof Area (Units 1&2)

Runoff = 0.19 cfs @ 12.07 hrs, Volume= 653 cf, Depth= 3.07"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 2-yr Rainfall=3.30"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

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Summary for Reach S1: Post - Dry Swale 1

Inflow Area = 20,593 sf, 28.07% Impervious, Inflow Depth = 0.37" for 2-yr event
Inflow = 0.17 cfs @ 12.08 hrs, Volume= 635 cf
Outflow = 0.16 cfs @ 12.15 hrs, Volume= 635 cf, Atten= 6%, Lag= 4.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Max. Velocity= 1.01 fps, Min. Travel Time= 2.5 min
Avg. Velocity = 0.32 fps, Avg. Travel Time= 7.7 min

Peak Storage= 24 cf @ 12.11 hrs

Average Depth at Peak Storage= 0.07'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 17.43 cfs

Custom cross-section, Length= 150.0' Slope= 0.0200 '/'
Constant n= 0.035 Earth, dense weeds
Inlet Invert= 85.00', Outlet Invert= 82.00'



Offset (feet)	Elevation (feet)	Chan.Depth (feet)	Storage (cubic-feet)	Discharge (cfs)
-3.00	1.00	0.00		
-1.00	0.00	1.00		
1.00	0.00	1.00		
3.00	1.00	0.00		
Depth (feet)	End Area (sq-ft)	Perim. (feet)	Storage (cubic-feet)	Discharge (cfs)
0.00	0.0	2.0	0	0.00
1.00	4.0	6.5	600	17.43

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Summary for Pond SB: Post - Settling Basin

Inflow Area = 25,705 sf, 42.37% Impervious, Inflow Depth = 1.36" for 2-yr event
Inflow = 0.35 cfs @ 12.17 hrs, Volume= 2,907 cf
Outflow = 0.33 cfs @ 12.26 hrs, Volume= 2,687 cf, Atten= 6%, Lag= 5.2 min
Primary = 0.33 cfs @ 12.26 hrs, Volume= 2,687 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 82.10' @ 12.24 hrs Surf.Area= 355 sf Storage= 253 cf

Plug-Flow detention time= 123.1 min calculated for 2,686 cf (92% of inflow)
Center-of-Mass det. time= 42.7 min (1,221.5 - 1,178.8)

Volume	Invert	Avail Storage	Storage Description
#1	81.00'	416 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.00	108	0	0
82.00	331	220	220
82.50	456	197	416

Device	Routing	Invert	Outlet Devices
#1	Primary	82.00'	4.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61

Primary OutFlow Max=0.30 cfs @ 12.26 hrs HW=82.10' (Free Discharge)
↑**_1=Broad-Crested Rectangular Weir** (Weir Controls 0.30 cfs @ 0.79 fps)

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Summary for Pond US1: Post - Recharge System-1

Inflow Area =	5,112 sf, 100.00% Impervious, Inflow Depth = 3.07"	for 2-yr event
Inflow =	0.38 cfs @ 12.07 hrs, Volume= 1,307 cf	
Outflow =	0.19 cfs @ 12.21 hrs, Volume= 1,307 cf, Atten= 50%, Lag= 8.2 min	
Discarded =	0.01 cfs @ 8.08 hrs, Volume= 806 cf	
Primary =	0.18 cfs @ 12.21 hrs, Volume= 501 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 85.11' @ 12.21 hrs Surf.Area= 715 sf Storage= 461 cf
Flood Elev= 86.30' Surf.Area= 715 sf Storage= 837 cf

Plug-Flow detention time= 237.9 min calculated for 1,306 cf (100% of inflow)
Center-of-Mass det. time= 238.1 min (992.9 - 754.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	84.00'	405 cf	21.50'W x 33.25'L x 2.33'H Field A 1,668 cf Overall - 439 cf Embedded = 1,229 cf x 33.0% Voids
#2A	84.50'	439 cf	Infiltrator H-20 Chamber x 30 Inside #1 Effective Size= 27.5"W x 14.0"H => 2.34 sf x 6.25'L = 14.6 cf Overall Size= 34.0"W x 16.0"H x 6.25'L
		845 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	84.00'	0.500 in/hr Exfiltration over Surface area
#2	Primary	84.85'	6.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 84.85' / 84.60' S= 0.0208 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Discarded OutFlow Max=0.01 cfs @ 8.08 hrs HW=84.02' (Free Discharge)
↑1=Exfiltration (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.18 cts @ 12.21 hrs HW=85.11' (Free Discharge)
↑2=Culvert (Inlet Controls 0.18 cfs @ 1.74 fps)

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Summary for Pond US2: Post - Underground Storage System

Inflow Area = 4,658 sf, 100.00% Impervious, Inflow Depth = 3.07" for 2-yr event
Inflow = 0.35 cfs @ 12.07 hrs, Volume= 1,191 cf
Outflow = 0.17 cfs @ 12.21 hrs, Volume= 1,191 cf, Atten= 52%, Lag= 8.5 min
Primary = 0.17 cfs @ 12.21 hrs, Volume= 1,191 cf

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 83.48' @ 12.21 hrs Surf.Area= 1,041 sf Storage= 239 cf

Plug-Flow detention time= 33.5 min calculated for 1,191 cf (100% of inflow)
Center-of-Mass det. time= 32.8 min (787.7 - 754.8)

Volume	Invert	Avail Storage	Storage Description
#1	83.15'	1,484 cf	18.0" Round Pipe Storage x 14 L= 60.0

Device	Routing	Invert	Outlet Devices
#1	Primary	83.15'	4.0" Round Culvert L= 100.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 83.15' / 82.10' S= 0.0105 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.09 sf

Primary OutFlow Max=0.17 cfs @ 12.21 hrs HW=83.48 (Free Discharge)
↳ 1=Culvert (Barrel Controls 0.17 cfs @ 2.45 fps)

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Time span=0.00-72.00 hrs, dt=0.04 hrs, 1801 points

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Pond B1: Post - Dry Extended Detention Basin

Peak Elev=81.45' Storage=2,259 cf Inflow=2.88 cfs 12,754 cf
Primary=1.80 cfs 12,424 cf Secondary=0.00 cfs 0 cf Outflow=1.80 cfs 12,424 cf

Peak Elev=85.91' Storage=1,474 cf Inflow=1.01 cfs 3,815 cf
Primary=0.63 cfs 1,542 cf Secondary=0.02 cfs 2,013 cf Outflow=0.66 cfs 3,555 cf

Peak Elev=82.41' Storage=1,058 cf Inflow=1.66 cfs 7,690 cf
Primary=1.53 cfs 5,316 cf Secondary=0.02 cfs 2,300 cf Outflow=1.55 cfs 7,616 cf

Peak Elev=85.04' Storage=559 cf Inflow=0.57 cfs 3,215 cf
Primary=0.52 cfs 1,964 cf Secondary=0.05 cfs 1,194 cf Outflow=0.57 cfs 3,158 cf

Peak Elev=83.90' Storage=297 cf Inflow=0.48 cfs 1,685 cf
Outflow=0.46 cfs 1,430 cf

Peak Elev=83.20' Inflow=0.45 cfs 3,094 cf
8.0" Round Culvert n=0.013 L=92.0' S=0.0087 '/' Outflow=0.45 cfs 3,094 cf

Peak Elev=86.24' Storage=237 cf Inflow=0.89 cfs 3,444 cf
Outflow=0.88 cfs 3,272 cf

Peak Elev=83.92' Storage=91 cf Inflow=0.41 cfs 1,510 cf
Outflow=0.40 cfs 1,434 cf

Peak Elev=82.30' Storage=283 cf Inflow=1.28 cfs 6,491 cf
Outflow=1.26 cfs 6,299 cf

Pond pA1: Analysis Point - Post Dev Catch Basin in Hope St.

Inflow=0.52 cfs 1,964 cf
Primary=0.52 cfs 1,964 cf

Inflow=2.02 cfs 15,811 cf
Primary=2.02 cfs 15,811 cf

Pond pA2: Analysis Point - Post Dev Flow to Wetland

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Subcatchment pB1: Post - Runoff to Detention Basin-1

Runoff Area=2,690 sf 0.00% Impervious Runoff Depth=2.28"
Tc=5.0 min CN=74/0 Runoff=0.17 cfs 512 cf

Runoff Area=12,569 sf 42.19% Impervious Runoff Depth=3.29"
Flow Length=126' Tc=10.0 min CN=74/98 Runoff=0.89 cfs 3,444 cf

Subcatchment pF1: Post - Runoff to Forebay-1

Runoff Area=4,810 sf 62.31% Impervious Runoff Depth=3.77"
Flow Length=64' Tc=8.0 min CN=74/98 Runoff=0.41 cfs 1,510 cf

Runoff Area=2,851 sf 0.00% Impervious Runoff Depth=2.28"
Tc=5.0 min CN=74/0 Runoff=0.18 cfs 543 cf

Subcatchment pS1: Post - Runoff to Bio-retention Basin

Runoff Area=5,173 sf 9.22% Impervious Runoff Depth=2.50"
Tc=5.0 min CN=74/98 Runoff=0.35 cfs 1,079 cf

Runoff Area=5,708 sf 0.00% Impervious Runoff Depth=2.28"
Flow Length=453' Tc=22.1 min CN=74/0 Runoff=0.22 cfs 1,087 cf

Subcatchment pS2: Post - Runoff to Swale-1

Runoff Area=1,321 sf 0.00% Impervious Runoff Depth=2.28"
Flow Length=64' Tc=8.0 min CN=74/0 Runoff=0.07 cfs 252 cf

Runoff Area=1,840 sf 12.93% Impervious Runoff Depth=2.59"
Tc=5.0 min CN=74/98 Runoff=0.13 cfs 398 cf

Subcatchment pS3: Post - Flow to wetlands

Runoff Area=13,640 sf 32.91% Impervious Runoff Depth=3.07"
Tc=5.0 min CN=74/98 Runoff=1.08 cfs 3,487 cf

Runoff Area=16,759 sf 0.73% Impervious Runoff Depth=2.30"
Flow Length=104' Tc=30.8 min CN=74/98 Runoff=0.57 cfs 3,215 cf

Subcatchment pS4: Post - Runoff to Bio-retention Basin-4

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=4.66"
Tc=5.0 min CN=0/98 Runoff=0.29 cfs 993 cf

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=4.66"
Tc=5.0 min CN=0/98 Runoff=0.29 cfs 993 cf

Subcatchment pS5: Post - Runoff to Bio-retention Basin-2

Subcatchment pS6: Post - Runoff to Forebay-3

Subcatchment pS7: Post - Flow to Hope Street

Subcatchment RF1: Post - Roof Area (Units 3&4)

Subcatchment RF2: Post - Roof Area (Units 5&6)

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Subcatchment RF3: Post - Roof Area (Units 7&8)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=4.66"
Tc=5.0 min CN=0/98 Runoff=0.29 cfs 993 cf

Subcatchment RF4: Post- Roof Area (Units 9&10)

Runoff Area=2,102 sf 100.00% Impervious Runoff Depth=4.66"
Tc=5.0 min CN=0/98 Runoff=0.24 cfs 817 cf

Subcatchment RF5: Post - Roof Area (Units 1&2)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=4.66"
Tc=5.0 min CN=0/98 Runoff=0.29 cfs 993 cf

Reach S1: Post - Dry Swale 1

n=0.035 L=150.0' Avg. Flow Depth=0.19' Max Vel=1.74 fps Inflow=0.78 cfs 2,622 cf
S=0.02000 '/' Capacity=17.43 cfs Outflow=0.77 cfs 2,622 cf

Pond SB: Post - Settling Basin

Peak Elev=82.22' Storage=298 cf Inflow=1.05 cfs 5,715 cf
Outflow=1.04 cfs 5,496 cf

Pond US1: Post - Recharge System-1

Peak Elev=85.30' Storage=552 cf Inflow=0.57 cfs 1,987 cf
Discarded=0.01 cfs 906 cf Primary=0.42 cfs 1,080 cf Outflow=0.43 cfs 1,987 cf

Pond US2: Post - Underground Storage System

Peak Elev=83.60' Storage=378 cf Inflow=0.52 cfs 1,810 cf
4.0" Round Culvert n=0.013 L=100.0' S=0.0105 '/' Outflow=0.21 cfs 1,810 cf

Total Runoff Area = 79,687 sf Runoff Volume = 20,316 cf Average Runoff Depth = 3.06"
67.43% Pervious = 53,735 sf 32.57% Impervious = 25,952 sf

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Summary for Pond B1: Post - Dry Extended Detention Basin

Inflow Area = 57,220 sf, 45.14% Impervious, Inflow Depth = 2.67" for 10-yr event
 Inflow = 2.88 cfs @ 12.13 hrs, Volume= 12,754 cf
 Outflow = 1.80 cfs @ 12.44 hrs, Volume= 12,424 cf, Atten= 37%, Lag= 18.8 min
 Primary = 1.80 cfs @ 12.44 hrs, Volume= 12,424 cf
 Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
 Peak Elev= 81.45' @ 12.44 hrs Surf.Area= 1,850 sf Storage= 2,259 cf
 Flood Elev= 82.50' Surf.Area= 2,300 sf Storage= 4,432 cf

PIPlug-Flow detention time= 77.6 min calculated for 12,417 cf (97% of inflow)
 Center-of-Mass det. time= 43.9 min (969.7 - 925.8)

Volume	Invert	Avail.Storage	Storage Description
#1	80.00'	4,432 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.00	1,273	0	0
81.00	1,660	1,467	1,467
82.00	2,080	1,870	3,337
82.50	2,300	1,095	4,432
Device	Routing	Invert	Outlet Devices
#1	Secondary	82.00'	4'0" long x 2'0" breadth Broad-Crested Weir
		Head (feet)	0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50
		Coef. (English)	2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Primary	80.45'	8.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500
		Inlet / Outlet Invert= 80.45' / 79.50'	S= 0.0792' Cc= 0.900
		n= 0.013	Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Primary	80.25'	4.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500
		Inlet / Outlet Invert= 80.25' / 79.50'	S= 0.0625' Cc= 0.900
		n= 0.013	Corrugated PE, smooth interior, Flow Area= 0.09 sf

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Primary OutFlow Max=1.80 cfs @ 12.44 hrs HW=81.45' (Free Discharge)
 └─2=Culvert (Inlet Controls 1.37 cfs @ 3.94 fps)
 └─3=Culvert (Inlet Controls 0.43 cfs @ 4.90 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=80.00' (Free Discharge)
 └─1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond BR1: Post - Bio-retention Basin-1

Inflow Area = 15,420 sf, 34.39% Impervious, Inflow Depth = 2.97" for 10-yr event
Inflow = 1.01 cfs @ 12.14 hrs, Volume= 3,815 cf
Outflow = 0.66 cfs @ 12.30 hrs, Volume= 3,555 cf, Atten= 35%, Lag= 9.3 min
Primary = 0.63 cfs @ 12.30 hrs, Volume= 1,542 cf
Secondary = 0.02 cfs @ 12.30 hrs, Volume= 2,013 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 85.91' @ 12.30 hrs Surf.Area= 2,076 sf Storage= 1,474 cf
Flood Elev= 86.00' Surf.Area= 2,110 sf Storage= 1,567 cf

Plug-Flow detention time= 383.7 min calculated for 3,555 cf (93% of inflow)
Center-of-Mass det. time= 347.2 min (1,164.2 - 817.0)

Volume	Invert	Avail.Storage	Storage Description
#1	85.00'	867 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	83.00'	694 cf	Filter Material (Prismatic) Listed below (Recalc)
#3	83.75'	7 cf	2,110 cf Overall - 7 cf Embedded = 2,103 cf x 33.0% Voids 6.0" Round Pipe Storage Inside #2 L= 35.0'
1,567 cf Total Available Storage			
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
85.00	678	0	0
86.00	1,055	867	867

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.00	1,055	0	0
85.00	1,055	2,110	2,110

Device	Routing	Invert	Outlet Devices
#1 Primary	85.75'	4.0' long x 2.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 Coef. (English) 2.48 2.60 2.60 2.64 2.65 2.68 2.75 2.74 2.76 2.89 3.05 3.19 3.32	

#2 Device 3 83.00'
#3 Secondary 83.75'
0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'
6.0" Round Culvert L= 100.0' RCP, sq.cut end projecting, Ke= 0.500
Inlet / Outlet Invert= 83.75' / 82.85' S= 0.0090' Cc= 0.900
n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.62 cfs @ 12.30 hrs HW=85.91' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.62 cfs @ 0.98 fps)

Secondary OutFlow Max=0.02 cfs @ 12.30 hrs HW=85.91' (Free Discharge)
↳ 3=Culvert (Passes 0.02 cfs of 0.82 cfs potential flow)
↳ 2=Exfiltration (Exfiltration Controls 0.02 cfs)

Summary for Pond BR2: Post - Bio-retention Basin-2

Inflow Area = 22,694 sf, 52.62% Impervious, Inflow Depth = 4.07" for 10-yr event
Inflow = 1.66 cfs @ 12.09 hrs, Volume= 7,690 cf
Outflow = 1.55 cfs @ 12.12 hrs, Volume= 7,616 cf, Atten= 7%, Lag= 1.9 min
Primary = 1.53 cfs @ 12.12 hrs, Volume= 5,316 cf
Secondary = 0.02 cfs @ 12.12 hrs, Volume= 2,300 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 82.41' @ 12.12 hrs Surf.Area= 1,767 sf Storage= 1,058 cf
Flood Elev= 82.50' Surf.Area= 1,796 sf Storage= 1,140 cf

Plug-Flow detention time= 171.3 min calculated for 7,616 cf (99% of inflow)
Center-of-Mass det. time= 159.2 min (1,039.9 - 880.7)

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Volume	Invert	Avail.Storage	Storage Description
#1	81.35'	836 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	80.35'	292 cf	Filter Material (Prismatic) Listed below (Recalc)
#3	80.60'	12 cf	6.0" Round Pipe Storage Inside #2 L= 61.0'

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.35	563	0	0
82.00	745	425	425
82.50	898	411	836

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.35	898	0	0
81.35	898	898	898

Device	Routing	Invert	Outlet Devices
#1	Primary	82.10'	3.5' long x 2'0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Device 3	80.35'	0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'
#3	Secondary	80.60'	6.0" Round Culvert L= 62.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 80.60' / 79.00' S= 0.0258 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=1.53 cfs @ 12.12 hrs HW=82.41' (Free Discharge)
↑1=Broad-Crested Rectangular Weir (Weir Controls 1.53 cfs @ 1.43 fps)

Secondary OutFlow Max=0.02 cfs @ 12.12 hrs HW=82.41' (Free Discharge)
↑2=Culvert (Passes 0.02 cfs of 1.06 cfs potential flow)
↑2=Exfiltration (Exfiltration Controls 0.02 cfs)

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Summary for Pond BR3: Post - Bio-retention Basin-3

Inflow Area = 16,759 sf, 0.73% Impervious, Inflow Depth = 2.30" for 10-yr event
Inflow = 0.57 cfs @ 12.44 hrs, Volume= 3,215 cf
Outflow = 0.57 cfs @ 12.46 hrs, Volume= 3,158 cf, Atten= 0%, Lag= 1.0 min
Primary = 0.52 cfs @ 12.46 hrs, Volume= 1,964 cf
Secondary = 0.05 cfs @ 12.46 hrs, Volume= 1,194 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 85.04' @ 12.46 hrs Surf.Area= 1,344 sf Storage= 559 cf

Plug-Flow detention time= 164.1 min calculated for 3,158 cf (98% of inflow)
Center-of-Mass det. time= 153.5 min (1,013.6 - 860.1)

Volume Invert Avail.Storage Storage Description

#1	84.40'	371 cf	Custom Stage Data (Prismatic)	Listed below (Recalc)
#2	83.40'	224 cf	Filter Material (Prismatic)	Listed below (Recalc)
			686 cf Overall - 6 cf Embedded = 680 cf	x 33.0% Voids
#3	83.65'	6 cf	6.0" Round Pipe Storage	Inside #2 L= 30.0'

601 cf Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
84.40	374	0	0
85.10	686	371	371

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.40	686	0	0
84.40	686	686	686

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Device Routing

Invert

Outlet Devices

Device	Routing	Invert	Outlet Devices
#1	Primary	84.95'	8.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Device 3	85.00'	6.0" Horiz. Orifice/Grate C=0.600 Limited to weir flow at low heads
#3	Secondary	83.65'	6.0" Round Culvert L= 68.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 83.65' / 83.00' S= 0.0096' Cc= 0.900
#4	Device 3	83.40'	n= 0.013 Concrete pipe, bends & connections, Flow Area= 0.20 sf 0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'

Primary OutFlow Max=0.52 cfs @ 12.46 hrs HW=85.04' (Free Discharge)
↑**1=Broad-Crested Rectangular Weir** (Weir Controls 0.52 cfs @ 0.75 fps)

Secondary OutFlow Max=0.05 cfs @ 12.46 hrs HW=85.04' (Free Discharge)
↑**3=Culvert** (Passes 0.05 cfs of 0.74 cfs potential flow)
↓**2=Orifice/Grate** (Weir Controls 0.04 cfs @ 0.63 fps)
↓**4=Exfiltration** (Exfiltration Controls 0.02 cfs)

Summary for Pond BR4: Post - Bio-retention Basin-4

Inflow Area = 6,131 sf, 48.88% Impervious, Inflow Depth = 3.30" for 10-yr event
Inflow = 0.48 cfs @ 12.12 hrs, Volume= 1,685 cf
Outflow = 0.46 cfs @ 12.14 hrs, Volume= 1,430 cf, Atten= 3%, Lag= 1.0 min
Primary = 0.46 cfs @ 12.14 hrs, Volume= 1,430 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 83.90' @ 12.14 hrs Surf.Area= 581 sf Storage= 297 cf
Flood Elev= 84.00' Surf.Area= 594 sf Storage= 325 cf

Plug-Flow detention time= 102.0 min calculated for 1,430 cf (85% of inflow)
Center-of-Mass det. time= 37.2 min (840.0 - 802.8)

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Volume	Invert	Avail.Storage	Storage Description
#1	83.00'	227 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	82.00'	98 cf	Filter Material (Prismatic) Listed below (Recalc)
			297 cf Overall x 33.0% Voids
		325 cf	Total Available Storage
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.00	157	0	0
84.00	297	227	227
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.00	297	0	0
83.00	297	297	297

Device	Routing	Invert	Outlet Devices
#1	Primary	83.75'	3.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.46 cfs @ 12.14 hrs HW=83.90' (Free Discharge)
†=Broad-Crested Rectangular Weir (Weir Controls 0.46 cfs @ 0.99 fps)

Summary for Pond DMH: post - DMH

Inflow Area = 5,112 sf, 100.00% Impervious, Inflow Depth = 7.26" for 10-yr event
Inflow = 0.45 cfs @ 12.14 hrs, Volume= 3,094 cf
Outflow = 0.45 cfs @ 12.14 hrs, Volume= 3,094 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.45 cfs @ 12.14 hrs, Volume= 3,094 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 83.20' @ 12.14 hrs
Flood Elev= 85.00'

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Device	Routing	Invert	Outlet Devices
#1 Primary	82.80'	8.0" Round Culvert	L= 92.0' RCP, sq cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 82.80' / 82.00' S= 0.0087' / Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=0.44 cfs @ 12.14 hrs HW=83.20 (Free Discharge)
↑=Culvert (Barrel Controls 0.44 cfs @ 2.89 fps)

Summary for Pond FB1: Post - Forebay-1

Inflow Area = 12,569 sf, 42.19% Impervious, Inflow Depth = 3.29" for 10-yr event
Inflow = 0.89 cfs @ 12.14 hrs, Volume= 3,444 cf
Outflow = 0.88 cfs @ 12.16 hrs, Volume= 3,272 cf, Atten= 1%, Lag= 1.1 min
Primary = 0.88 cfs @ 12.16 hrs, Volume= 3,272 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 86.24' @ 12.16 hrs Surf.Area= 297 sf Storage= 237 cf
Flood Elev= 86.50' Surf.Area= 350 sf Storage= 323 cf

Plug-Flow detention time= 53.9 min calculated for 3,270 cf (95% of inflow)
Center-of-Mass det. time= 25.3 min (813.6 - 788.2)

Volume	Invert	Avail Storage	Storage Description	Custom Stage Data (Prismatic) Listed below (Recalc)
#1	85.00'	323 cf		
Elevation	Surf Area (sq-ft)	Inc. Store (cubic-feet)	Cum. Store (cubic-feet)	
85.00	95	0	0	
86.00	250	173	173	
86.50	350	150	323	

Device	Routing	Invert	Outlet Devices	
#1 Primary	86.00'	3.0 long x 2.0 breadth Broad-Crested Rectangular Weir	Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32	

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Primary OutFlow Max=0.88 cfs @ 12.16 hrs HW=86.24' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.88 cfs @ 1.24 fps)

Summary for Pond FB2: Post - Forebay-2

Inflow Area = 4,810 sf, 62.31% Impervious, Inflow Depth = 3.77" for 10-yr event
Inflow = 0.41 cfs @ 12.11 hrs, Volume= 1,510 cf
Outflow = 0.40 cfs @ 12.12 hrs, Volume= 1,434 cf, Atten= 1%, Lag= 0.4 min
Primary = 0.40 cfs @ 12.12 hrs, Volume= 1,434 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 83.92' @ 12.12 hrs Surf.Area= 136 sf Storage= 91 cf
Flood Elev= 84.00' Surf.Area= 143 sf Storage= 103 cf

Plug-Flow detention time= 54.3 min calculated for 1,433 cf (95% of inflow)
Center-of-Mass det. time= 25.4 min (796.2 - 770.8)

Volume	Invert	Avail.Storage	Storage Description	Custom Stage Data (Prismatic) Listed below (Recalc)
#1	83.00'	103 cf		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
83.00 84.00	63 143	0 103	0 103	

Device	Routing	Invert	Outlet Devices	4.0' long x 2.0' breadth Broad-Crested Rectangular Weir
#1	Primary	83.80'		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.40 cfs @ 12.12 hrs HW=83.92' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.40 cfs @ 0.87 fps)

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Summary for Pond FB3: Post - Forebay-3

Inflow Area = 18,298 sf, 49.99% Impervious, Inflow Depth = 4.26" for 10-yr event
Inflow = 1.28 cfs @ 12.08 hrs, Volume= 6,491 cf
Outflow = 1.26 cfs @ 12.09 hrs, Volume= 6,299 cf, Atten= 2%, Lag= 0.9 min
Primary = 1.26 cfs @ 12.09 hrs, Volume= 6,299 cf

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 82.30' @ 12.09 hrs Surf.Area= 345 sf Storage= 283 cf
Flood Elev= 82.50' Surf.Area= 400 sf Storage= 358 cf

Plug-Flow detention time= 52.4 min calculated for 6,295 cf (97% of inflow)
Center-of-Mass det. time= 20.0 min (905.8 - 885.8)

Volume	Invert	Avail.Storage	Storage Description
#1	81.00'	358 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.00	121	0	0
82.00	263	192	192
82.50	400	166	358

Device	Routing	Invert	Outlet Devices
#1	Primary	82.00'	3'0" long x 2'0" breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=1.23 cfs @ 12.09 hrs HW=82.29 (Free Discharge)
↳₁=Broad-Crested Rectangular Weir (Weir Controls 1.23 cfs @ 1.40 fps)

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Summary for Pond pA1: Analysis Point - Post Dev Catch Basin in Hope St.

Inflow Area = 16,759 sf, 0.73% Impervious, Inflow Depth = 1.41" for 10-yr event
Inflow = 0.52 cfs @ 12.46 hrs, Volume= 1,964 cf
Primary = 0.52 cfs @ 12.46 hrs, Volume= 1,964 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Pond pA2: Analysis Point - Post Dev Flow to Wetland

Inflow Area = 62,928 sf, 41.05% Impervious, Inflow Depth = 3.01" for 10-yr event
Inflow = 2.02 cfs @ 12.41 hrs, Volume= 15,811 cf
Primary = 2.02 cfs @ 12.41 hrs, Volume= 15,811 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Subcatchment pB1: Post - Runoff to Detention Basin-1

Runoff = 0.17 cfs @ 12.08 hrs, Volume= 512 cf, Depth= 2.28"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description
2,690	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
0	98	Walkway
0	98	Roofs, HSG C
2,690	74	Weighted Average
2,690	74	100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pF1: Post - Runoff to Forebay-1

Runoff = 0.89 cfs @ 12.14 hrs, Volume= 3,444 cf, Depth= 3.29"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description
4,205	98	Paved parking, HSG C
7,266	74	>75% Grass cover, Good, HSG C
*	979	Walkway
*	119	Patio/Deck/Stairs
12,569	84	Weighted Average
7,266	74	57.81% Pervious Area
5,303	98	42.19% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
9.5	44	0.0300
0.5	82	0.0180
10.0	126	Total
		0.08
		Sheet Flow, AB
		Grass: Bermuda n= 0.410 P2= 3.30"
		Shallow Concentrated Flow, BC
		Paved Kv= 20.3 fps

Summary for Subcatchment pF2: Post - Runoff to Forebay-2

Runoff = 0.41 cfs @ 12.11 hrs, Volume= 1,510 cf, Depth= 3.77"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 10-yr Rainfall=4.90"

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Area (sf)			CN	Description
1,813	74	>75%	Grass cover, Good, HSG C	
2,847	98	Paved parking, HSG C		
*	150	98	Walkway	
0	98	Roofs, HSG C		
4,810	89	Weighted Average		
1,813	74	37.69% Pervious Area		
2,997	98	62.31% Impervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)
7.9	40	0.0400	0.08	Sheet Flow, AB
0.1	24	0.0200	2.87	Grass: Bermuda n= 0.410 P2= 3.30" Shallow Concentrated Flow, BC
				Paved Kv= 20.3 fps
8.0	64	Total		

Summary for Subcatchment pS1: Post - Runoff to Bio-retention Basin

$$\text{Runoff} = 0.18 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 543 \text{ cf, Depth=} 2.28"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)			CN	Description
2,851	74	>75%	Grass cover, Good, HSG C	
*	0	98	Patio/Deck/Stairs	
2,851	74	Weighted Average		
2,851	74	100.00% Pervious Area		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)
				Direct Entry, Runoff to swale
				5.0

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Summary for Subcatchment pS2: Post - Runoff to Swale-1

Runoff = 0.35 cfs @ 12.08 hrs, Volume= 1,079 cf, Depth= 2.50"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description			
0	98	Paved parking, HSG C			
4,696	74	>75% Grass cover, Good, HSG C			
*	0	Walkway			
*	477	Patio/Deck/Stairs			
5,173	76	Weighted Average			
4,696	74	90.78% Pervious Area			
477	98	9.22% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Parking Runoff

Summary for Subcatchment pS3: Post - Flow to wetlands

Runoff = 0.22 cfs @ 12.32 hrs, Volume= 1,087 cf, Depth= 2.28"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description
5,708	74	>75% Grass cover, Good, HSG C
0	98	Paved parking, HSG C
0	70	Woods, Good, HSG C
5,708	74	Weighted Average
5,708	74	100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.2	50	0.0260	0.07		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
8.6	183	0.0050	0.35		Shallow Concentrated Flow, BC
					Woodland Kv= 5.0 fps
1.9	117	0.0400	1.00		Shallow Concentrated Flow, CD
					Woodland Kv= 5.0 fps
0.3	103	0.0300	5.33	26.67	Channel Flow, DE
					Area= 5.0 sf Perim= 10.2' r= 0.49'
					n= 0.030 Earth, grassed & winding
22.1	453	Total			

Summary for Subcatchment pS4: Post - Runoff to Bio-retention Basin-4

Runoff = 0.07 cfs @ 12.12 hrs, Volume= 252 cf, Depth= 2.28"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description			
1,321	74	>75% Grass cover, Good, HSG C			
*	0	Paved parking, HSG C			
*	0	Walkway			
*	0	Roofs, HSG C			
1,321	74	Weighted Average			
1,321	74	100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	40	0.0400	0.08		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
0.1	24	0.0200	2.87		Shallow Concentrated Flow, BC
					Paved Kv= 20.3 fps
8.0	64	Total			

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Summary for Subcatchment pS5: Post - Runoff to Bio-retention Basin-2

$$\text{Runoff} = 0.13 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 398 \text{ cf, Depth=} 2.59"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description			
0	98	Paved parking, HSG C			
1,602	74	>75% Grass cover, Good, HSG C			
*	0	Walkway			
*	238	Patio/Deck/Stairs			
0	98	Roofs, HSG C			
1,840	77	Weighted Average			
1,602	74	87.07% Pervious Area			
238	98	12.93% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pS6: Post - Runoff to Forebay-3

$$\text{Runoff} = 1.08 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 3,487 \text{ cf, Depth=} 3.07"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description
3,996	98	Paved parking, HSG C
9,151	74	>75% Grass cover, Good, HSG C
*	493	Walkway
13,640	82	Weighted Average
9,151	74	67.09% Pervious Area
4,489	98	32.91% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pS7: Post - Flow to Hope Street

Runoff = 0.57 cfs @ 12.44 hrs, Volume= 3,215 cf, Depth= 2.30"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description
16,637	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
*	0	Walkways
*	122	Patio/Deck/Stairs
16,759	74	Weighted Average
16,637	74	99.27% Pervious Area
122	98	0.73% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
		(ft/sec)
30.6	60	0.0030
0.2	44	0.0450
30.8	104	Total
		0.03
		3.42
		0.29

Sheet Flow, AB
 Grass: Bermuda n= 0.410 P2= 3.30"
Shallow Concentrated Flow, BC
 Unpaved Kv= 16.1 fps

Summary for Subcatchment RF1: Post - Roof Area (Units 3&4)

Runoff = 0.29 cfs @ 12.07 hrs, Volume= 993 cf, Depth= 4.66"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 10-yr Rainfall=4.90"

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Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF2: Post - Roof Area (Units 5&6)

$$\text{Runoff} = 0.29 \text{ cfs} @ 12.07 \text{ hrs, Volume=} 993 \text{ cf, Depth=} 4.66"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF3: Post - Roof Area (Units 7&8)

$$\text{Runoff} = 0.29 \text{ cfs} @ 12.07 \text{ hrs, Volume=} 993 \text{ cf, Depth=} 4.66"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF4: Post- Roof Area (Units 9&10)

$$\text{Runoff} = 0.24 \text{ cfs} @ 12.07 \text{ hrs, Volume=} 817 \text{ cf, Depth=} 4.66"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description
2,102	98	Roofs, HSG C
2,102	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF5: Post - Roof Area (Units 1&2)

$$\text{Runoff} = 0.29 \text{ cfs} @ 12.07 \text{ hrs, Volume=} 993 \text{ cf, Depth=} 4.66"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 10-yr Rainfall=4.90"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

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Summary for Reach S1: Post - Dry Swale 1

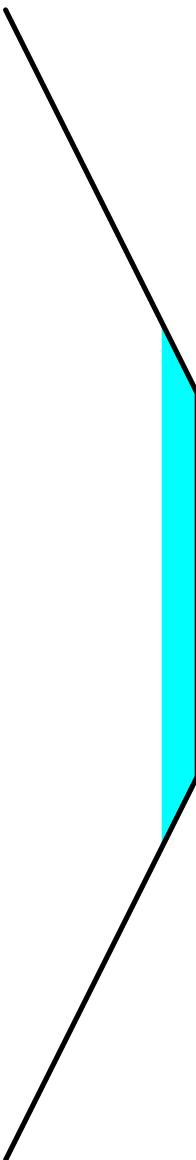
Inflow Area = 20,593 sf, 28.07% Impervious, Inflow Depth = 1.53" for 10-yr event
Inflow = 0.78 cfs @ 12.29 hrs, Volume= 2,622 cf
Outflow = 0.77 cfs @ 12.34 hrs, Volume= 2,622 cf, Atten= 1%, Lag= 2.7 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Max. Velocity= 1.74 fps, Min. Travel Time= 1.4 min
Avg. Velocity = 0.41 fps, Avg. Travel Time= 6.1 min

Peak Storage= 66 cf @ 12.32 hrs

Average Depth at Peak Storage= 0.19'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 17.43 cfs

Custom cross-section, Length= 150.0' Slope= 0.0200 '/'
Constant n= 0.035 Earth, dense weeds
Inlet Invert= 85.00', Outlet Invert= 82.00'



Offset (feet)	End Elevation (feet)	Area (sq-ft)	Perim. (feet)	Chan.Depth (feet)	Storage (cubic-feet)	Discharge (cfs)
-3.00	1.00	0.00	0.00	0.00	0	0.00
-1.00	0.00	1.00	1.00	0.00	0	0.00
1.00	0.00	1.00	1.00	0.00	0	0.00
3.00	1.00	0.00	0.00	0.00	0	0.00
0.00	0.0	4.0	2.0	0.00	0	0.00
1.00	0.0	4.0	6.5	0.00	600	17.43

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Summary for Pond SB: Post - Settling Basin

Inflow Area = 25,705 sf, 42.37% Impervious, Inflow Depth = 2.67" for 10-yr event
Inflow = 1.05 cfs @ 12.33 hrs, Volume= 5,715 cf
Outflow = 1.04 cfs @ 12.35 hrs, Volume= 5,496 cf, Atten= 1%, Lag= 1.2 min
Primary = 1.04 cfs @ 12.35 hrs, Volume= 5,496 cf

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 82.22' @ 12.35 hrs Surf.Area= 385 sf Storage= 298 cf

Plug-Flow detention time= 68.4 min calculated for 5,496 cf (96% of inflow)
Center-of-Mass det. time= 17.5 min (1,046.1 - 1,028.6)

Volume	Invert	Avail Storage	Storage Description
#1	81.00'	416 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.00	108	0	0
82.00	331	220	220
82.50	456	197	416

Device	Routing	Invert	Outlet Devices
#1	Primary	82.00'	4.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61

Primary OutFlow Max=1.03 cfs @ 12.35 hrs HW=82.22 (Free Discharge)
↑**1=Broad-Crested Rectangular Weir** (Weir Controls 1.03 cfs @ 1.19 fps)

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Summary for Pond US1: Post - Recharge System-1

Inflow Area =	5,112 sf, 100.00% Impervious, Inflow Depth = 4.66"	for 10-yr event
Inflow =	0.57 cfs @ 12.07 hrs, Volume= 1,987 cf	
Outflow =	0.43 cfs @ 12.14 hrs, Volume= 1,987 cf, Atten= 25%, Lag= 4.0 min	
Discarded =	0.01 cfs @ 6.40 hrs, Volume= 906 cf	
Primary =	0.42 cfs @ 12.14 hrs, Volume= 1,080 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 85.30' @ 12.14 hrs Surf.Area= 715 sf Storage= 552 cf
Flood Elev= 86.30' Surf.Area= 715 sf Storage= 837 cf

Plug-Flow detention time= 188.1 min calculated for 1,985 cf (100% of inflow)
Center-of-Mass det. time= 188.3 min (935.7 - 747.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	84.00'	405 cf	21.50'W x 33.25'L x 2.33'H Field A 1,668 cf Overall - 439 cf Embedded = 1,229 cf x 33.0% Voids
#2A	84.50'	439 cf	Infiltrator H-20 Chamber x 30 Inside #1 Effective Size= 27.5"W x 14.0"H => 2.34 sf x 6.25'L = 14.6 cf Overall Size= 34.0"W x 16.0"H x 6.25'L
		845 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	84.00'	0.500 in/hr Exfiltration over Surface area
#2	Primary	84.85'	6.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 84.85' / 84.60' S= 0.0208 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Discarded OutFlow Max=0.01 cfs @ 6.40 hrs HW=84.02' (Free Discharge)
↑1=Exfiltration (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.42 cts @ 12.14 hrs HW=85.29' (Free Discharge)
↑2=Culvert (Inlet Controls 0.42 cfs @ 2.27 fps)

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Summary for Pond US2: Post - Underground Storage System

Inflow Area = 4,658 sf, 100.00% Impervious, Inflow Depth = 4.66" for 10-yr event
Inflow = 0.52 cfs @ 12.07 hrs, Volume= 1,810 cf
Outflow = 0.21 cfs @ 12.50 hrs, Volume= 1,810 cf, Atten= 61%, Lag= 25.5 min
Primary = 0.21 cfs @ 12.50 hrs, Volume= 1,810 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 83.60' @ 12.29 hrs Surf.Area= 1,157 sf Storage= 378 cf

Plug-Flow detention time= 31.6 min calculated for 1,809 cf (100% of inflow)
Center-of-Mass det. time= 31.7 min (779.2 - 747.4)

Volume	Invert	Avail Storage	Storage Description
#1	83.15'	1,484 cf	18.0" Round Pipe Storage x 14 L= 60.0

Device	Routing	Invert	Outlet Devices
#1	Primary	83.15'	4.0" Round Culvert L= 100.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 83.15' / 82.10' S= 0.0105 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.09 sf

Primary OutFlow Max=0.21 cfs @ 12.50 hrs HW=83.57" (Free Discharge)
↳ 1=Culvert (Barrel Controls 0.21 cfs @ 2.42 fps)

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Time span=0.00-72.00 hrs, dt=0.04 hrs, 1801 points

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Pond B1: Post - Dry Extended Detention Basin

Primary=2.45 cfs 17,488 cf Secondary=0.19 cfs 97 cf Outflow=2.64 cfs 17,915 cf

Peak Elev=82.07' Storage=3,485 cf Inflow=4.24 cfs 17,915 cf
Primary=1.21 cfs 2,777 cf Secondary=0.02 cfs 2,138 cf Outflow=1.24 cfs 4,915 cf

Peak Elev=85.99' Storage=1,562 cf Inflow=1.37 cfs 5,174 cf
Primary=2.02 cfs 7,444 cf Secondary=0.02 cfs 2,374 cf Outflow=2.04 cfs 9,818 cf

Peak Elev=82.47' Storage=1,110 cf Inflow=2.17 cfs 9,891 cf
Primary=0.73 cfs 3,214 cf Secondary=0.09 cfs 1,321 cf Outflow=0.82 cfs 4,535 cf

Peak Elev=83.94' Storage=306 cf Inflow=0.63 cfs 2,242 cf
Outflow=0.61 cfs 1,987 cf

Peak Elev=85.06' Storage=574 cf Inflow=0.83 cfs 4,592 cf
Primary=0.73 cfs 3,214 cf Secondary=0.09 cfs 1,321 cf Outflow=0.82 cfs 4,535 cf

Peak Elev=83.26' Storage=252 cf Inflow=0.55 cfs 3,681 cf
8.0" Round Culvert n=0.013 L=92.0' S=0.0087 '/' Outflow=0.55 cfs 3,681 cf

Peak Elev=86.29' Storage=252 cf Inflow=1.19 cfs 4,570 cf
Outflow=1.18 cfs 4,397 cf

Peak Elev=83.94' Storage=94 cf Inflow=0.53 cfs 1,958 cf
Outflow=0.52 cfs 1,882 cf

Peak Elev=82.35' Storage=303 cf Inflow=1.67 cfs 8,282 cf
Outflow=1.64 cfs 8,090 cf

Pond pA1: Analysis Point - Post Dev Catch Basin in Hope St.

Inflow=0.73 cfs 3,214 cf
Primary=0.73 cfs 3,214 cf

Inflow=2.96 cfs 21,513 cf
Primary=2.96 cfs 21,513 cf

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Subcatchment pB1: Post - Runoff to Detention Basin-1

Subcatchment pF1: Post - Runoff to Forebay-1

Subcatchment pF2: Post - Runoff to Forebay-2

Subcatchment DS1: Post - Runoff to Bio-retention Basin

Supplementary Attachment D

Suhatchmen nS3: Post - Flow to wetlands

Schubestrommant nS1: Rest Runoff to Rio retention Basin 1

Substantiation of Data from the Rio Grande Basin

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Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=5.86"

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Subcatchment RF3: Post - Roof Area (Units 7&8)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=5.86"
Tc=5.0 min CN=0/98 Runoff=0.36 cfs 1,249 cf

Subcatchment RF4: Post- Roof Area (Units 9&10)

Runoff Area=2,102 sf 100.00% Impervious Runoff Depth=5.86"
Tc=5.0 min CN=0/98 Runoff=0.29 cfs 1,027 cf

Subcatchment RF5: Post - Roof Area (Units 1&2)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=5.86"
Tc=5.0 min CN=0/98 Runoff=0.36 cfs 1,249 cf

Avg. Flow Depth=0.27' Max Vel=2.15 fps Inflow=1.49 cfs 4,289 cf
n=0.035 L=150.0' S=0.0200 '/' Capacity=17.43 cfs Outflow=1.48 cfs 4,289 cf

Pond SB: Post - Settling Basin

Peak Elev=82.33 Storage=342 cf Inflow=1.97 cfs 7,971 cf
Outflow=1.95 cfs 7,751 cf

Pond US1: Post - Recharge System-1

Peak Elev=85.41' Storage=600 cf Inflow=0.72 cfs 2,497 cf
Discarded=0.01 cfs 953 cf Primary=0.52 cfs 1,544 cf Outflow=0.53 cfs 2,497 cf

Pond US2: Post - Underground Storage System

Peak Elev=83.72' Storage=515 cf Inflow=0.65 cfs 2,275 cf
4.0" Round Culvert n=0.013 L=100.0' S=0.0105 '/' Outflow=0.21 cfs 2,275 cf

Total Runoff Area = 79,687 sf Runoff Volume = 27,316 cf Average Runoff Depth = 4.11"
67.43% Pervious = 53,735 sf 32.57% Impervious = 25,952 sf

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Summary for Pond B1: Post - Dry Extended Detention Basin

Inflow Area =	57,220 sf	45.14% Impervious,	Inflow Depth = 3.76"	for 25-yr event
Inflow =	4.24 cfs @	12.16 hrs, Volume=	17,915 cf	
Outflow =	2.64 cfs @	12.39 hrs, Volume=	17,584 cf, Atten= 38%, Lag= 13.9 min	
Primary =	2.45 cfs @	12.39 hrs, Volume=	17,488 cf	
Secondary =	0.19 cfs @	12.39 hrs, Volume=	97 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2 Peak Elev= 82.07 @ 12.39 hrs Surf.Area= 2,111 sf Storage= 3,485 cf Flood Elev= 82.50 Surf.Area= 2,300 sf Storage= 4,432 cf

Plug-Flow detention time= 63.8 min calculated for 17,584 cf (98% of inflow)
 Center-of-Mass det. time= 37.7 min (932.8 - 895.0)

Volume	Invert	Avail.Storage	Storage Description
#1	80.00'	4,432 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.00	1,273	0	0
81.00	1,660	1,467	1,467
82.00	2,080	1,870	3,337
82.50	2,300	1,095	4,432
Device	Routing	Invert	Outlet Devices
#1	Secondary	82.00'	4.0' long x 2.0' breadth Broad-Crested Rectangular Weir
		Head (feet)	0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50
		Coef. (English)	2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Primary	80.45'	8.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500
		Inlet / Outlet Invert= 80.45' / 79.50'	S= 0.0792" Cc= 0.900
		n= 0.013	Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Primary	80.25'	4.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500
		Inlet / Outlet Invert= 80.25' / 79.50'	S= 0.0625" Cc= 0.900
		n= 0.013	Corrugated PE, smooth interior, Flow Area= 0.09 sf

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Primary OutFlow Max=2.45 cfs @ 12.39 hrs HW=82.07' (Free Discharge)
↳ 2=Culvert (Inlet Controls 1.91 cfs @ 5.46 fps)
↳ 3=Culvert (Inlet Controls 0.54 cfs @ 6.19 fps)

Secondary OutFlow Max=0.19 cfs @ 12.39 hrs HW=82.07' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.19 cfs @ 0.67 fps)

Summary for Pond BR1: Post - Bio-retention Basin-1

Inflow Area =	15,420 sf	34.39% Impervious,	Inflow Depth = 4.03"	for 25-yr event
Inflow =	1.37 cfs @ 12.14 hrs,	Volume= 5,174 cf		
Outflow =	1.24 cfs @ 12.20 hrs,	Volume= 4,915 cf, Atten= 9%, Lag= 3.6 min		
Primary =	1.21 cfs @ 12.20 hrs,	Volume= 2,777 cf		
Secondary =	0.02 cfs @ 12.20 hrs,	Volume= 2,138 cf		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 85.99' @ 12.20 hrs Surf.Area= 2,108 sf Storage= 1,562 cf
Flood Elev= 86.00' Surf.Area= 2,110 sf Storage= 1,567 cf

Plug-Flow detention time= 299.3 min calculated for 4,912 cf (95% of inflow)
Center-of-Mass det. time= 272.0 min (1,081.0 - 808.9)

Volume	Invert	Avail.Storage	Storage Description	
#1	85.00'	867 cf	Custom Stage Data (Prismatic)	Listed below (Recalc)
#2	83.00'	694 cf	Filter Material (Prismatic)	Listed below (Recalc)
#3	83.75'	7 cf	6.0" Round Pipe Storage	Inside #2 L= 35.0'
1,567 cf Total Available Storage				
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
85.00	678	0	0	
86.00	1,055	867	867	

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.00	1,055	0	0
85.00	1,055	2,110	2,110

Device	Routing	Invert	Outlet Devices
#1 Primary	85.75'	4.0' long x 2.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 Coef. (English) 2.48 2.60 2.60 2.64 2.65 2.68 2.75 2.74 2.76 2.89 3.05 3.19 3.32	

#2 Device 3 83.00'
#3 Secondary 83.75'
0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'
6.0" Round Culvert L= 100.0' RCP, sq.cut end projecting, Ke= 0.500
Inlet / Outlet Invert= 83.75' / 82.85' S= 0.0090' Cc= 0.900
n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=1.21 cfs @ 12.20 hrs HW=85.99' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 1.21 cfs @ 1.24 fps)

Secondary OutFlow Max=0.02 cfs @ 12.20 hrs HW=85.99' (Free Discharge)
↳ 3=Culvert (Passes 0.02 cfs of 0.84 cfs potential flow)
↳ 2=Exfiltration (Exfiltration Controls 0.02 cfs)

Summary for Pond BR2: Post - Bio-retention Basin-2

Inflow Area = 22,694 sf, 52.62% Impervious, Inflow Depth = 5.23" for 25-yr event
Inflow = 2.17 cfs @ 12.09 hrs, Volume= 9,891 cf
Outflow = 2.04 cfs @ 12.12 hrs, Volume= 9,818 cf, Atten= 6%, Lag= 1.8 min
Primary = 2.02 cfs @ 12.12 hrs, Volume= 7,444 cf
Secondary = 0.02 cfs @ 12.12 hrs, Volume= 2,374 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 82.47' @ 12.12 hrs Surf.Area= 1,786 sf Storage= 1,110 cf
Flood Elev= 82.50' Surf.Area= 1,796 sf Storage= 1,140 cf

Plug-Flow detention time= 138.4 min calculated for 9,818 cf (99% of inflow)
Center-of-Mass det. time= 128.7 min (986.4 - 857.7)

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Volume	Invert	Avail.Storage	Storage Description
#1	81.35'	836 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	80.35'	292 cf	Filter Material (Prismatic) Listed below (Recalc)
#3	80.60'	12 cf	6.0" Round Pipe Storage Inside #2 L= 61.0'

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.35	563	0	0
82.00	745	425	425
82.50	898	411	836

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.35	898	0	0
81.35	898	898	898

Device	Routing	Invert	Outlet Devices
#1	Primary	82.10'	3.5' long x 2'0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Device 3	80.35'	0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'
#3	Secondary	80.60'	6.0" Round Culvert L= 62.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 80.60' / 79.00' S= 0.0258 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=2.00 cfs @ 12.12 hrs HW=82.46' (Free Discharge)
↑1=Broad-Crested Rectangular Weir (Weir Controls 2.00 cfs @ 1.57 fps)

Secondary OutFlow Max=0.02 cfs @ 12.12 hrs HW=82.46' (Free Discharge)
↑2=Culvert (Passes 0.02 cfs of 1.07 cfs potential flow)
↑2=Exfiltration (Exfiltration Controls 0.02 cfs)

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Summary for Pond BR3: Post - Bio-retention Basin-3

Inflow Area = 16,759 sf, 0.73% Impervious, Inflow Depth = 3.29" for 25-yr event
 Inflow = 0.83 cfs @ 12.43 hrs, Volume= 4,592 cf
 Outflow = 0.82 cfs @ 12.45 hrs, Volume= 4,535 cf, Atten= 0%, Lag= 0.9 min
 Primary = 0.73 cfs @ 12.45 hrs, Volume= 3,214 cf
 Secondary = 0.09 cfs @ 12.45 hrs, Volume= 1,321 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Peak Elev= 85.06' @ 12.45 hrs Surf.Area= 1,354 sf Storage= 574 cf

Plug-Flow detention time= 119.0 min calculated for 4,535 cf (99% of inflow)
 Center-of-Mass det. time= 111.3 min (961.3 - 850.0)

Volume Invert Avail.Storage Storage Description

#1	84.40'	371 cf	Custom Stage Data (Prismatic)
#2	83.40'	224 cf	Filter Material (Prismatic)
			686 cf Overall - 6 cf Embedded = 680 cf x 33.0% Voids
#3	83.65'	6 cf	6.0" Round Pipe Storage Inside #2 L= 30.0'

601 cf Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
84.40	374	0	0
85.10	686	371	371

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.40	686	0	0
84.40	686	686	686

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Device Routing Invert Outlet Devices

Device	Routing	Invert	Outlet Devices
#1	Primary	84.95'	8.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Device 3	85.00'	6.0" Horiz. Orifice/Grate C=0.600 Limited to weir flow at low heads
#3	Secondary	83.65'	6.0" Round Culvert L= 68.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 83.65' / 83.00' S= 0.0096' / Cc= 0.900 n= 0.013 Concrete pipe, bends & connections, Flow Area= 0.20 sf
#4	Device 3	83.40'	0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'

Primary OutFlow Max=0.73 cfs @ 12.45 hrs HW=85.06' (Free Discharge)
1=Broad-Crested Rectangular Weir (Weir Controls 0.73 cfs @ 0.84 fps)

Secondary OutFlow Max=0.09 cfs @ 12.45 hrs HW=85.06' (Free Discharge)
3=Culvert (Passes 0.09 cfs of 0.75 cfs potential flow)
2=Orifice/Grate (Weir Controls 0.07 cfs @ 0.79 fps)
4=Exfiltration (Exfiltration Controls 0.02 cfs)

Summary for Pond BR4: Post - Bio-retention Basin-4

Inflow Area = 6,131 sf, 48.88% Impervious, Inflow Depth = 4.39" for 25-yr event
Inflow = 0.63 cfs @ 12.12 hrs, Volume= 2,242 cf
Outflow = 0.61 cfs @ 12.13 hrs, Volume= 1,987 cf, Atten= 2%, Lag= 0.9 min
Primary = 0.61 cfs @ 12.13 hrs, Volume= 1,987 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 83.94' @ 12.13 hrs Surf.Area= 585 sf Storage= 306 cf
Flood Elev= 84.00' Surf.Area= 594 sf Storage= 325 cf

Plug-Flow detention time= 86.5 min calculated for 1,987 cf (89% of inflow)
Center-of-Mass det. time= 32.9 min (828.5 - 795.7)

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Volume	Invert	Avail.Storage	Storage Description
#1	83.00'	227 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	82.00'	98 cf	Filter Material (Prismatic) Listed below (Recalc)
			297 cf Overall x 33.0% Voids
		325 cf	Total Available Storage
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.00	157	0	0
84.00	297	227	227
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.00	297	0	0
83.00	297	297	297

Device	Routing	Invert	Outlet Devices
#1	Primary	83.75'	3.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.60 cfs @ 12.13 hrs HW=83.93' (Free Discharge)
 $\downarrow t_1$ =Broad-Crested Rectangular Weir (Weir Controls 0.60 cfs @ 1.09 fps)

Summary for Pond DMH: post - DMH

Inflow Area = 5,112 sf, 100.00% Impervious, Inflow Depth = 8.64" for 25-yr event
 Inflow = 0.55 cfs @ 12.14 hrs, Volume= 3,681 cf
 Outflow = 0.55 cfs @ 12.14 hrs, Volume= 3,681 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.55 cfs @ 12.14 hrs, Volume= 3,681 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Peak Elev= 83.26' @ 12.14 hrs
 Flood Elev= 85.00'

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Device	Routing	Invert	Outlet Devices
#1 Primary	82.80'	8.0" Round Culvert	L= 92.0' RCP, sq cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 82.80' / 82.00' S= 0.0087' / Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=0.54 cfs @ 12.14 hrs HW=83.25 (Free Discharge)
↑=Culvert (Barrel Controls 0.54 cfs @ 3.04 fps)

Summary for Pond FB1: Post - Forebay-1

Inflow Area = 12,569 sf, 42.19% Impervious, Inflow Depth = 4.36" for 25-yr event
Inflow = 1.19 cfs @ 12.14 hrs, Volume= 4,570 cf
Outflow = 1.18 cfs @ 12.15 hrs, Volume= 4,397 cf, Atten= 1%, Lag= 1.0 min
Primary = 1.18 cfs @ 12.15 hrs, Volume= 4,397 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 86.29' @ 12.15 hrs Surf.Area= 307 sf Storage= 252 cf
Flood Elev= 86.50' Surf.Area= 350 sf Storage= 323 cf

Plug-Flow detention time= 43.4 min calculated for 4,395 cf (96% of inflow)
Center-of-Mass det. time= 21.1 min (805.7 - 784.6)

Volume	Invert	Avail Storage	Storage Description	Custom Stage Data (Prismatic) Listed below (Recalc)
#1	85.00'	323 cf		
Elevation	Surf Area (sq-ft)	Inc. Store (cubic-feet)	Cum. Store (cubic-feet)	
85.00	95	0	0	
86.00	250	173	173	
86.50	350	150	323	

Device	Routing	Invert	Outlet Devices	#1 Primary	86.00' 3.0 long x 2.0 breadth Broad-Crested Rectangular Weir	Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50	Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
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Primary OutFlow Max=1.17 cfs @ 12.15 hrs HW=86.28' (Free Discharge)
1=Broad-Crested Rectangular Weir (Weir Controls 1.17 cfs @ 1.37 fps)

Inflow Area = 4,810 sf, 62.31% Impervious, Inflow Depth = 4.88" for 25-yr event
 Inflow = 0.53 cfs @ 12.11 hrs, Volume= 1,958 cf
 Outflow = 0.52 cfs @ 12.12 hrs, Volume= 1,882 cf, Attenu= 1%, Lag= 0.4 min
 Primary = 0.52 cfs @ 12.12 hrs, Volume= 1,882 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
 Peak Elev= 83.94' @ 12.12 hrs Surf.Area= 138 sf Storage= 94 cf
 Flood Elev= 84.00' Surf.Area= 143 sf Storage= 103 cf

PIPlug-Flow detention time= 44.6 min calculated for 1,882 cf (96% of inflow)
 Center-of-Mass det time= 21.4 min (789.1 - 767.7)

Volume	Invert	Avail Storage	Storage Description
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
#1	83.00	103 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
83.00	63	0	0
84.00	143	103	103

Device	Routing	Invert	Outlet Devices	4.0' Long x 2.0' breadth Broad-Crested Rectangular Weir
#1	Primary	83.80'	Head (feet)	0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50
			Coef. (English)	2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Summary for Pond FB2: Post - Forebay-2

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Summary for Pond FB3: Post - Forebay-3

Inflow Area = 18,298 sf, 49.99% Impervious, Inflow Depth = 5.43" for 25-yr event
Inflow = 1.67 cfs @ 12.08 hrs, Volume= 8,282 cf
Outflow = 1.64 cfs @ 12.09 hrs, Volume= 8,090 cf, Atten= 1%, Lag= 0.8 min
Primary = 1.64 cfs @ 12.09 hrs, Volume= 8,090 cf

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 82.35' @ 12.09 hrs Surf.Area= 360 sf Storage= 303 cf
Flood Elev= 82.50' Surf.Area= 400 sf Storage= 358 cf

Plug-Flow detention time= 42.4 min calculated for 8,086 cf (98% of inflow)
Center-of-Mass det. time= 16.4 min (878.5 - 862.1)

Volume	Invert	Avail.Storage	Storage Description
#1	81.00'	358 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.00	121	0	0
82.00	263	192	192
82.50	400	166	358

Device	Routing	Invert	Outlet Devices	Weir
#1	Primary	82.00'	3'0" long x 2'0" breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32	

Primary OutFlow Max=1.61 cfs @ 12.09 hrs HW=82.35' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 1.61 cfs @ 1.53 fps)

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Summary for Pond pA1: Analysis Point - Post Dev Catch Basin in Hope St.

Inflow Area = 16,759 sf, 0.73% Impervious, Inflow Depth = 2.30" for 25-yr event
Inflow = 0.73 cfs @ 12.45 hrs, Volume= 3,214 cf
Primary = 0.73 cfs @ 12.45 hrs, Volume= 3,214 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Pond pA2: Analysis Point - Post Dev Flow to Wetland

Inflow Area = 62,928 sf, 41.05% Impervious, Inflow Depth = 4.10" for 25-yr event
Inflow = 2.96 cfs @ 12.38 hrs, Volume= 21,513 cf
Primary = 2.96 cfs @ 12.38 hrs, Volume= 21,513 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Subcatchment pB1: Post - Runoff to Detention Basin-1

Runoff = 0.24 cfs @ 12.08 hrs, Volume= 733 cf, Depth= 3.27"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description
2,690	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
*	0	Walkway
*	0	Roofs, HSG C
2,690	74	Weighted Average
2,690	74	100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pF1: Post - Runoff to Forebay-1

Runoff = 1.19 cfs @ 12.14 hrs, Volume= 4,570 cf, Depth= 4.36"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description
4,205	98	Paved parking, HSG C
7,266	74	>75% Grass cover, Good, HSG C
*	979	Walkway
*	119	Patio/Deck/Stairs
12,569	84	Weighted Average
7,266	74	57.81% Pervious Area
5,303	98	42.19% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
9.5	44	0.0300
0.5	82	0.0180
10.0	126	Total
		0.08
		Sheet Flow, AB
		Grass: Bermuda n= 0.410 P2= 3.30"
		Shallow Concentrated Flow, BC
		Paved Kv= 20.3 fps

Summary for Subcatchment pF2: Post - Runoff to Forebay-2

Runoff = 0.53 cfs @ 12.11 hrs, Volume= 1,958 cf, Depth= 4.88"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 25-yr Rainfall=6.10"

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Area (sf)			CN	Description
*	1,813	74	>75%	Grass cover, Good, HSG C
	2,847	98	Paved parking	HSG C
*	150	98	Walkway	
*	0	98	Roofs	HSG C
	4,810	89	Weighted Average	
	1,813	74	37.69% Pervious Area	
	2,997	98	62.31% Impervious Area	
Tc	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)
7.9	40	0.0400	0.08	Sheet Flow, AB
				Grass: Bermuda n= 0.410 P2= 3.30"
0.1	24	0.0200	2.87	Shallow Concentrated Flow, BC
				Paved Kv= 20.3 fps
	8.0	64	Total	

Summary for Subcatchment pS1: Post - Runoff to Bio-retention Basin

$$\text{Runoff} = 0.26 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 777 \text{ cf, Depth=} 3.27"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)			CN	Description
*	2,851	74	>75%	Grass cover, Good, HSG C
*	0	98	Patio/Deck/Stairs	
	2,851	74	Weighted Average	
	2,851	74	100.00% Pervious Area	
Tc	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)
				Direct Entry, Runoff to swale
	5.0			

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Summary for Subcatchment pS2: Post - Runoff to Swale-1

$$\text{Runoff} = 0.49 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 1,512 \text{ cf, Depth=} 3.51"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description			
0	98	Paved parking, HSG C			
4,696	74	>75% Grass cover, Good, HSG C			
*	0	Walkway			
*	477	Patio/Deck/Stairs			
5,173	76	Weighted Average			
4,696	74	90.78% Pervious Area			
477	98	9.22% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Parking Runoff

Summary for Subcatchment pS3: Post - Flow to wetlands

$$\text{Runoff} = 0.32 \text{ cfs} @ 12.31 \text{ hrs, Volume=} 1,555 \text{ cf, Depth=} 3.27"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description
5,708	74	>75% Grass cover, Good, HSG C
0	98	Paved parking, HSG C
0	70	Woods, Good, HSG C
5,708	74	Weighted Average
5,708	74	100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.2	50	0.0260	0.07		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
8.6	183	0.0050	0.35		Shallow Concentrated Flow, BC
					Woodland Kv= 5.0 fps
1.9	117	0.0400	1.00		Shallow Concentrated Flow, CD
					Woodland Kv= 5.0 fps
0.3	103	0.0300	5.33	26.67	Channel Flow, DE
					Area= 5.0 sf Perim= 10.2' r= 0.49'
					n= 0.030 Earth, grassed & winding
22.1	453	Total			

Summary for Subcatchment pS4: Post - Runoff to Bio-retention Basin-4

Runoff = 0.11 cfs @ 12.12 hrs, Volume= 360 cf, Depth= 3.27"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description			
1,321	74	>75% Grass cover, Good, HSG C			
*	0	Paved parking, HSG C			
*	0	Walkway			
*	0	Roofs, HSG C			
1,321	74	Weighted Average			
1,321	74	100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	40	0.0400	0.08		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
0.1	24	0.0200	2.87		Shallow Concentrated Flow, BC
					Paved Kv= 20.3 fps
8.0	64	Total			

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Summary for Subcatchment pS5: Post - Runoff to Bio-retention Basin-2

$$\text{Runoff} = 0.18 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 553 \text{ cf, Depth=} 3.60"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description			
0	98	Paved parking, HSG C			
1,602	74	>75% Grass cover, Good, HSG C			
*	0	Walkway			
*	238	Patio/Deck/Stairs			
0	98	Roofs, HSG C			
1,840	77	Weighted Average			
1,602	74	87.07% Pervious Area			
238	98	12.93% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pS6: Post - Runoff to Forebay-3

$$\text{Runoff} = 1.45 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 4,686 \text{ cf, Depth=} 4.12"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description
3,996	98	Paved parking, HSG C
9,151	74	>75% Grass cover, Good, HSG C
*	493	Walkway
13,640	82	Weighted Average
9,151	74	67.09% Pervious Area
4,489	98	32.91% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pS7: Post - Flow to Hope Street

Runoff = 0.83 cfs @ 12.43 hrs, Volume= 4,592 cf, Depth= 3.29"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description
16,637	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
*	0	Walkways
*	122	Patio/Deck/Stairs
16,759	74	Weighted Average
16,637	74	99.27% Pervious Area
122	98	0.73% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
		(ft/sec)
30.6	60	0.0030
0.2	44	0.0450
30.8	104	Total
		0.03
		3.42
		0.00

Sheet Flow, AB
Grass: Bermuda n= 0.410 P2= 3.30"
Shallow Concentrated Flow, BC
Unpaved Kv= 16.1 fps

Summary for Subcatchment RF1: Post - Roof Area (Units 3&4)

Runoff = 0.36 cfs @ 12.07 hrs, Volume= 1,249 cf, Depth= 5.86"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 25-yr Rainfall=6.10"

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Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF2: Post - Roof Area (Units 5&6)

$$\text{Runoff} = 0.36 \text{ cfs} @ 12.07 \text{ hrs, Volume=} 1,249 \text{ cf, Depth=} 5.86"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF3: Post - Roof Area (Units 7&8)

$$\text{Runoff} = 0.36 \text{ cfs} @ 12.07 \text{ hrs, Volume=} 1,249 \text{ cf, Depth=} 5.86"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF4: Post- Roof Area (Units 9&10)

Runoff = 0.29 cfs @ 12.07 hrs, Volume= 1,027 cf, Depth= 5.86"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description
2,102	98	Roofs, HSG C
2,102	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF5: Post - Roof Area (Units 1&2)

Runoff = 0.36 cfs @ 12.07 hrs, Volume= 1,249 cf, Depth= 5.86"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 25-yr Rainfall=6.10"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

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Summary for Reach S1: Post - Dry Swale 1

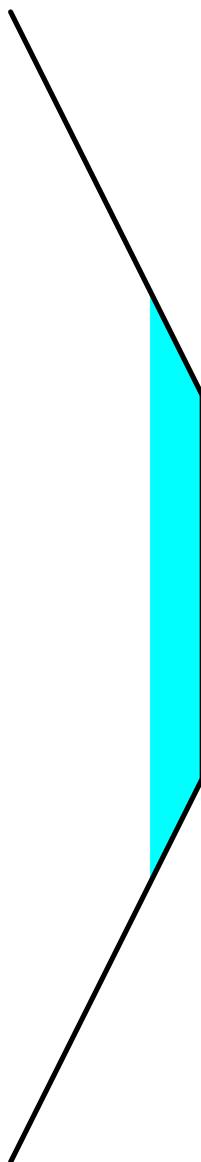
Inflow Area = 20,593 sf, 28.07% Impervious, Inflow Depth = 2.50" for 25-yr event
Inflow = 1.49 cfs @ 12.18 hrs, Volume= 4,289 cf
Outflow = 1.48 cfs @ 12.22 hrs, Volume= 4,289 cf, Atten= 1%, Lag= 2.2 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Max. Velocity= 2.15 fps, Min. Travel Time= 1.2 min
Avg. Velocity = 0.47 fps, Avg. Travel Time= 5.4 min

Peak Storage= 103 cf @ 12.20 hrs

Average Depth at Peak Storage= 0.27'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 17.43 cfs

Custom cross-section, Length= 150.0' Slope= 0.0200 '/'
Constant n= 0.035 Earth, dense weeds
Inlet Invert= 85.00', Outlet Invert= 82.00'



Offset (feet)	End Elevation (feet)	Area (sq-ft)	Perim. (feet)	Chan.Depth (feet)	Storage (cubic-feet)	Discharge (cfs)
-3.00	1.00	0.00	0.00	0.00	0	0.00
-1.00	0.00	1.00	1.00	0.00	0	0.00
1.00	0.00	1.00	1.00	0.00	0	0.00
3.00	1.00	0.00	0.00	0.00	0	0.00
0.00	0.0	2.0	2.0	0	0	0.00
1.00	4.0	6.5	6.5	0.00	600	17.43

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Summary for Pond SB: Post - Settling Basin

Inflow Area = 25,705 sf, 42.37% Impervious, Inflow Depth = 3.72" for 25-yr event
Inflow = 1.97 cfs @ 12.21 hrs, Volume= 7,971 cf
Outflow = 1.95 cfs @ 12.23 hrs, Volume= 7,751 cf, Atten= 1%, Lag= 0.8 min
Primary = 1.95 cfs @ 12.23 hrs, Volume= 7,751 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 82.33' @ 12.23 hrs Surf.Area= 413 sf Storage= 342 cf

Plug-Flow detention time= 51.7 min calculated for 7,751 cf (97% of inflow)
Center-of-Mass det. time= 12.4 min (986.3 - 973.9)

Volume	Invert	Avail Storage	Storage Description
#1	81.00'	416 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.00	108	0	0
82.00	331	220	220
82.50	456	197	416

Device	Routing	Invert	Outlet Devices
#1	Primary	82.00'	4.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61

Primary OutFlow Max=1.94 cfs @ 12.23 hrs HW=82.33 (Free Discharge)
↑**1=Broad-Crested Rectangular Weir** (Weir Controls 1.94 cfs @ 1.48 fps)

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Summary for Pond US1: Post - Recharge System-1

Inflow Area =	5,112 sf, 100.00% Impervious, Inflow Depth = 5.86"	for 25-yr event
Inflow =	0.72 cfs @ 12.07 hrs, Volume= 2,497 cf	
Outflow =	0.53 cfs @ 12.14 hrs, Volume= 2,497 cf, Atten= 26%, Lag= 4.0 min	
Discarded =	0.01 cfs @ 4.96 hrs, Volume= 953 cf	
Primary =	0.52 cfs @ 12.14 hrs, Volume= 1,544 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 85.41' @ 12.14 hrs Surf.Area= 715 sf Storage= 600 cf
Flood Elev= 86.30' Surf.Area= 715 sf Storage= 837 cf

Plug-Flow detention time= 164.1 min calculated for 2,496 cf (100% of inflow)
Center-of-Mass det. time= 164.4 min (908.4 - 744.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	84.00'	405 cf	21.50'W x 33.25'L x 2.33'H Field A 1,668 cf Overall - 439 cf Embedded = 1,229 cf x 33.0% Voids
#2A	84.50'	439 cf	Infiltrator H-20 Chamber x 30 Inside #1 Effective Size= 27.5"W x 14.0"H => 2.34 sf x 6.25'L = 14.6 cf Overall Size= 34.0"W x 16.0"H x 6.25'L
		845 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	84.00'	0.500 in/hr Exfiltration over Surface area
#2	Primary	84.85'	6.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 84.85' / 84.60' S= 0.0208 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Discarded OutFlow Max=0.01 cfs @ 4.96 hrs HW=84.02' (Free Discharge)
↑1=Exfiltration (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.52 cts @ 12.14 hrs HW=85.40' (Free Discharge)
↑2=Culvert (Inlet Controls 0.52 cts @ 2.64 fps)

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Summary for Pond US2: Post - Underground Storage System

Inflow Area = 4,658 sf, 100.00% Impervious, Inflow Depth = 5.86" for 25-yr event
Inflow = 0.65 cfs @ 12.07 hrs, Volume= 2,275 cf
Outflow = 0.21 cfs @ 12.86 hrs, Volume= 2,275 cf, Atten= 68%, Lag= 47.0 min
Primary = 0.21 cfs @ 12.86 hrs, Volume= 2,275 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 83.72' @ 12.36 hrs Surf.Area= 1,222 sf Storage= 515 cf

Plug-Flow detention time= 32.9 min calculated for 2,274 cf (100% of inflow)
Center-of-Mass det. time= 33.0 min (777.0 - 744.0)

Volume	Invert	Avail Storage	Storage Description
#1	83.15'	1,484 cf	18.0" Round Pipe Storage x 14 L= 60.0

Device	Routing	Invert	Outlet Devices
#1	Primary	83.15'	4.0" Round Culvert L= 100.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 83.15' / 82.10' S= 0.0105 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.09 sf

Primary OutFlow Max=0.21 cfs @ 12.86 hrs HW=83.57' (Free Discharge)
↳ 1=Culvert (Barrel Controls 0.21 cfs @ 2.42 fps)

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Time span=0.00-72.00 hrs, dt=0.04 hrs, 1801 points

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Pond B1: Post - Dry Extended Detention Basin

Peak Elev=82.46' Storage=4,335 cf Inflow=7.03 cfs 28,949 cf
Primary=2.78 cfs 25,267 cf Secondary=3,23 cfs 3,348 cf Outflow=6.01 cfs 28,615 cf

Pond BR1: Post - Bio-retention Basin-1

Peak Elev=86.09' Storage=1,567 cf Inflow=2.13 cfs 8,130 cf
Primary=2.04 cfs 5,529 cf Secondary=0.02 cfs 2,275 cf Outflow=2.06 cfs 7,804 cf

Pond BR2: Post - Bio-retention Basin-2

Peak Elev=82.56' Storage=1,140 cf Inflow=3.25 cfs 14,644 cf
Primary=2.82 cfs 11,922 cf Secondary=0.02 cfs 2,486 cf Outflow=2.84 cfs 14,408 cf

Pond BR3: Post - Bio-retention Basin-3

Peak Elev=85.10' Storage=601 cf Inflow=1.38 cfs 7,663 cf
Primary=1.20 cfs 5,986 cf Secondary=0.18 cfs 1,620 cf Outflow=1.38 cfs 7,606 cf

Pond BR4: Post - Bio-retention Basin-4

Peak Elev=84.00' Storage=324 cf Inflow=0.95 cfs 3,439 cf
Outflow=0.93 cfs 3,184 cf

Peak Elev=83.36' Inflow=0.76 cfs 4,838 cf
8.0" Round Culvert n=0.013 L=92.0' S=0.0087 '/' Outflow=0.76 cfs 4,838 cf

Peak Elev=86.38' Storage=281 cf Inflow=1.82 cfs 7,004 cf
Outflow=1.81 cfs 6,831 cf

Peak Elev=83.98' Storage=100 cf Inflow=0.78 cfs 2,914 cf
Outflow=0.77 cfs 2,838 cf

Peak Elev=82.46' Storage=343 cf Inflow=2.51 cfs 12,160 cf
Outflow=2.47 cfs 11,968 cf

Inflow=1.20 cfs 5,986 cf
Primary=1.20 cfs 5,986 cf

Inflow=6.49 cfs 33,700 cf
Primary=6.49 cfs 33,700 cf

Pond pA1: Analysis Point - Post Dev Catch Basin in Hope St.

Pond pA2: Analysis Point - Post Dev Flow to Wetland

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Subcatchment pB1: Post - Runoff to Detention Basin-1

Runoff Area=2,690 sf 0.00% Impervious Runoff Depth=5.47"
Tc=5.0 min CN=74/0 Runoff=0.40 cfs 1,225 cf

Runoff Area=12,569 sf 42.19% Impervious Runoff Depth=6.69"
Flow Length=126' Tc=10.0 min CN=74/98 Runoff=1.82 cfs 7,004 cf

Subcatchment pF1: Post - Runoff to Forebay-1

Runoff Area=4,810 sf 62.31% Impervious Runoff Depth=7.27"
Flow Length=64' Tc=8.0 min CN=74/98 Runoff=0.78 cfs 2,914 cf

Subcatchment pF2: Post - Runoff to Forebay-2

Runoff Area=2,851 sf 0.00% Impervious Runoff Depth=5.47"
Tc=5.0 min CN=74/0 Runoff=0.43 cfs 1,299 cf

Runoff Area=5,173 sf 9.22% Impervious Runoff Depth=5.73"
Tc=5.0 min CN=74/98 Runoff=0.80 cfs 2,471 cf

Subcatchment pS2: Post - Runoff to Swale-1

Runoff Area=5,708 sf 0.00% Impervious Runoff Depth=5.47"
Flow Length=453' Tc=22.1 min CN=74/0 Runoff=0.54 cfs 2,600 cf

Runoff Area=1,321 sf 0.00% Impervious Runoff Depth=5.47"
Flow Length=64' Tc=8.0 min CN=74/0 Runoff=0.18 cfs 602 cf

Subcatchment pS4: Post - Runoff to Bio-retention Basin-4

Runoff Area=1,840 sf 12.93% Impervious Runoff Depth=5.84"
Tc=5.0 min CN=74/98 Runoff=0.29 cfs 895 cf

Subcatchment pS6: Post - Runoff to Forebay-3

Runoff Area=13,640 sf 32.91% Impervious Runoff Depth=6.42"
Tc=5.0 min CN=74/98 Runoff=2.26 cfs 7,295 cf

Subcatchment pS7: Post - Flow to Hope Street

Runoff Area=16,759 sf 0.73% Impervious Runoff Depth=5.49"
Flow Length=104' Tc=30.8 min CN=74/98 Runoff=1.38 cfs 7,663 cf

Subcatchment RF1: Post - Roof Area (Units 3&4)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=8.36"
Tc=5.0 min CN=0/98 Runoff=0.51 cfs 1,781 cf

Subcatchment RF2: Post - Roof Area (Units 5&6)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=8.36"
Tc=5.0 min CN=0/98 Runoff=0.51 cfs 1,781 cf

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Subcatchment RF3: Post - Roof Area (Units 7&8)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=8.36"
Tc=5.0 min CN=0/98 Runoff=0.51 cfs 1,781 cf

Subcatchment RF4: Post- Roof Area (Units 9&10)

Runoff Area=2,102 sf 100.00% Impervious Runoff Depth=8.36"
Tc=5.0 min CN=0/98 Runoff=0.42 cfs 1,464 cf

Subcatchment RF5: Post - Roof Area (Units 1&2)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=8.36"
Tc=5.0 min CN=0/98 Runoff=0.51 cfs 1,781 cf

Reach S1: Post - Dry Swale 1

n=0.035 L=150.0' Avg. Flow Depth=0.38' Max Vel=2.59 fps Inflow=2.71 cfs 8,000 cf
S=0.0200 '/' Capacity=17.43 cfs Outflow=2.65 cfs 8,000 cf

Pond SB: Post - Settling Basin

Peak Elev=82.47' Storage=404 cf Inflow=3.41 cfs 12,837 cf
Outflow=3.41 cfs 12,618 cf

Pond US1: Post - Recharge System-1

Peak Elev=85.70' Storage=696 cf Inflow=1.01 cfs 3,561 cf
Discarded=0.01 cfs 999 cf Primary=0.73 cfs 2,562 cf Outflow=0.74 cfs 3,561 cf

Pond US2: Post - Underground Storage System

Peak Elev=83.97' Storage=828 cf Inflow=0.92 cfs 3,245 cf
4.0" Round Culvert n=0.013 L=100.0' S=0.0105 '/' Outflow=0.22 cfs 3,245 cf

Total Runoff Area = 79,687 sf Runoff Volume = 42,554 cf Average Runoff Depth = 6.41"
67.43% Pervious = 53,735 sf 32.57% Impervious = 25,952 sf

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Summary for Pond B1: Post - Dry Extended Detention Basin

Inflow Area =	57,220 sf	45.14% Impervious,	Inflow Depth = 6.07"	for 100-yr event
Inflow =	7.03 cfs @	12.12 hrs, Volume=	28,949 cf	
Outflow =	6.01 cfs @	12.22 hrs, Volume=	28,615 cf, Atten= 15%, Lag= 5.7 min	
Primary =	2.78 cfs @	12.22 hrs, Volume=	25,267 cf	
Secondary =	3.23 cfs @	12.22 hrs, Volume=	3,348 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
 Peak Elev= 82.46' @ 12.22 hrs Surf.Area= 2,281 sf Storage= 4,335 cf
 Flood Elev= 82.50' Surf.Area= 2,300 sf Storage= 4,432 cf

Plug-Flow detention time= 46.7 min calculated for 28,615 cf (99% of inflow)
 Center-of-Mass det. time= 29.7 min (888.7 - 859.0)

Volume	Invert	Avail.Storage	Storage Description
#1	80.00'	4,432 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.00	1,273	0	0
81.00	1,660	1,467	1,467
82.00	2,080	1,870	3,337
82.50	2,300	1,095	4,432
Device	Routing	Invert	Outlet Devices
#1	Secondary	82.00'	4.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50
			Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Primary	80.45'	8.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 80.45' / 79.50' S= 0.0792' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Primary	80.25'	4.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500
			Inlet / Outlet Invert= 80.25' / 79.50' S= 0.0625' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.09 sf

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Primary OutFlow Max=2.77 cfs @ 12.22 hrs HW=82.45' (Free Discharge)
 └─2=Culvert (Inlet Controls 2.17 cfs @ 6.22 fps)
 └─3=Culvert (Inlet Controls 0.60 cfs @ 6.87 fps)

Secondary OutFlow Max=3.16 cfs @ 12.22 hrs HW=82.45' (Free Discharge)
 └─1=Broad-Crested Rectangular Weir (Weir Controls 3.16 cfs @ 1.75 fps)

Summary for Pond BR1: Post - Bio-retention Basin-1

Inflow Area =	15,420 sf	34.39% Impervious,	Inflow Depth = 6.33"	for 100-yr event
Inflow =	2.13 cfs @ 12.14 hrs,	Volume= 8,130 cf		
Outflow =	2.06 cfs @ 12.14 hrs,	Volume= 7,804 cf, Atten= 3%, Lag= 0.0 min		
Primary =	2.04 cfs @ 12.14 hrs,	Volume= 5,529 cf		
Secondary =	0.02 cfs @ 12.08 hrs,	Volume= 2,275 cf		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 86.09' @ 12.14 hrs Surf.Area= 2,110 sf Storage= 1,567 cf
Flood Elev= 86.00' Surf.Area= 2,110 sf Storage= 1,567 cf

Plug-Flow detention time= 210.5 min calculated for 7,804 cf (96% of inflow)
Center-of-Mass det. time= 186.9 min (984.1 - 797.2)

Volume	Invert	Avail.Storage	Storage Description
#1	85.00'	867 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	83.00'	694 cf	Filter Material (Prismatic) Listed below (Recalc)
#3	83.75'	7 cf	2,110 cf Overall - 7 cf Embedded = 2,103 cf x 33.0% Voids 6.0" Round Pipe Storage Inside #2 L= 35.0'
1,567 cf Total Available Storage			
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
85.00	678	0	0
86.00	1,055	867	867

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.00	1,055	0	0
85.00	1,055	2,110	2,110

Device	Routing	Invert	Outlet Devices
#1 Primary	85.75'	4'0" long x 2.5' breadth Broad-Crested Rectangular Weir	
		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00	
		Coef. (English) 2.48 2.60 2.60 2.64 2.65 2.68 2.75 2.74 2.76 2.89 3.05 3.19 3.32	
#2 Device 3	83.00'	0.500 in/hr Exfiltration over Surface area	Phase-In= 0.01'
#3 Secondary	83.75'	6.0" Round Culvert L= 100.0' RCP, sq.cut end projecting, Ke= 0.500	Inlet / Outlet Invert= 83.75' / 82.85' S= 0.0090' Cc= 0.900
		n= 0.013 Corrugated PE, smooth interior,	Flow Area= 0.20 sf

Primary OutFlow Max=2.01 cfs @ 12.14 hrs HW=86.09' (Free Discharge)
 ↪**1=Broad-Crested Rectangular Weir** (Weir Controls 2.01 cfs @ 1.49 fps)

Secondary OutFlow Max=0.02 cfs @ 12.08 hrs HW=86.06' (Free Discharge)
 ↪**3=Culvert** (Passes 0.02 cfs of 0.85 cfs potential flow)
 ↪**2=Exfiltration** (Exfiltration Controls 0.02 cfs)

Summary for Pond BR2: Post - Bio-retention Basin-2

Inflow Area = 22,694 sf, 52.62% Impervious, Inflow Depth = 7.74" for 100-yr event
 Inflow = 3.25 cfs @ 12.09 hrs, Volume= 14,644 cf
 Outflow = 2.84 cfs @ 12.09 hrs, Volume= 14,408 cf, Atten= 13%, Lag= 0.0 min
 Primary = 2.82 cfs @ 12.09 hrs, Volume= 11,922 cf
 Secondary = 0.02 cfs @ 12.04 hrs, Volume= 2,486 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
 Peak Elev= 82.56' @ 12.09 hrs Surf.Area= 1,796 sf Storage= 1,140 cf
 Flood Elev= 82.50' Surf.Area= 1,796 sf Storage= 1,140 cf

Plug-Flow detention time= 113.3 min calculated for 14,408 cf (98% of inflow)
 Center-of-Mass det. time= 94.7 min (923.6 - 828.8)

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Volume	Invert	Avail.Storage	Storage Description
#1	81.35'	836 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	80.35'	292 cf	Filter Material (Prismatic) Listed below (Recalc)
#3	80.60'	12 cf	6.0" Round Pipe Storage Inside #2 L= 61.0'

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.35	563	0	0
82.00	745	425	425
82.50	898	411	836

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.35	898	0	0
81.35	898	898	898

Device	Routing	Invert	Outlet Devices
#1	Primary	82.10'	3.5' long x 2'0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Device 3	80.35'	0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'
#3	Secondary	80.60'	6.0" Round Culvert L= 62.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 80.60' / 79.00' S= 0.0258 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=2.78 cfs @ 12.09 hrs HW=82.55' (Free Discharge)
↑1=Broad-Crested Rectangular Weir (Weir Controls 2.78 cfs @ 1.76 fps)

Secondary OutFlow Max=0.02 cfs @ 12.04 hrs HW=82.51' (Free Discharge)
↑2=Culvert (Passes 0.02 cfs of 1.08 cfs potential flow)
↑2=Exfiltration (Exfiltration Controls 0.02 cfs)

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Summary for Pond BR3: Post - Bio-retention Basin-3

Inflow Area = 16,759 sf, 0.73% Impervious, Inflow Depth = 5.49" for 100-yr event
Inflow = 1.38 cfs @ 12.42 hrs, Volume= 7,663 cf
Outflow = 1.38 cfs @ 12.43 hrs, Volume= 7,606 cf, Atten= 0%, Lag= 0.6 min
Primary = 1.20 cfs @ 12.43 hrs, Volume= 5,986 cf
Secondary = 0.18 cfs @ 12.43 hrs, Volume= 1,620 cf

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 85.10' @ 12.43 hrs Surf.Area= 1,372 sf Storage= 601 cf

Plug-Flow detention time= 76.7 min calculated for 7,606 cf (99% of inflow)
Center-of-Mass det. time= 71.7 min (907.2 - 835.5)

Volume Invert Avail.Storage Storage Description

#1	84.40'	371 cf	Custom Stage Data (Prismatic)	Listed below (Recalc)
#2	83.40'	224 cf	Filter Material (Prismatic)	Listed below (Recalc)
			686 cf Overall - 6 cf Embedded	= 680 cf x 33.0% Voids
#3	83.65'	6 cf	6.0" Round Pipe Storage	Inside #2 L= 30.0'

601 cf Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
84.40	374	0	0
85.10	686	371	371

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.40	686	0	0
84.40	686	686	686

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Device	Routing	Invert	Outlet Devices
#1	Primary	84.95'	8.0' long x 2.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Device 3	85.00'	6.0" Horiz. Orifice/Grate C=0.600 Limited to weir flow at low heads
#3	Secondary	83.65'	6.0" Round Culvert L= 68.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 83.65' / 83.00' S= 0.0096' / Cc= 0.900 n= 0.013 Concrete pipe, bends & connections, Flow Area= 0.20 sf
#4	Device 3	83.40'	0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'

Primary OutFlow Max=1.20 cfs @ 12.43 hrs HW=85.10' (Free Discharge)
1=Broad-Crested Rectangular Weir (Weir Controls 1.20 cfs @ 0.99 fps)

Secondary OutFlow Max=0.18 cfs @ 12.43 hrs HW=85.10' (Free Discharge)
3=Culvert (Passes 0.18 cfs of 0.76 cfs potential flow)
2=Orifice/Grate (Weir Controls 0.17 cfs @ 1.04 fps)
4=Exfiltration (Exfiltration Controls 0.02 cfs)

Summary for Pond BR4: Post - Bio-retention Basin-4

Inflow Area = 6,131 sf, 48.88% Impervious, Inflow Depth = 6.73" for 100-yr event
Inflow = 0.95 cfs @ 12.12 hrs, Volume= 3,439 cf
Outflow = 0.93 cfs @ 12.13 hrs, Volume= 3,184 cf, Atten= 2%, Lag= 0.8 min
Primary = 0.93 cfs @ 12.13 hrs, Volume= 3,184 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 84.00' @ 12.13 hrs Surf.Area= 593 sf Storage= 324 cf
Flood Elev= 84.00' Surf.Area= 594 sf Storage= 325 cf

Plug-Flow detention time= 66.3 min calculated for 3,184 cf (93% of inflow)
Center-of-Mass det. time= 27.1 min (812.3 - 785.3)

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Volume	Invert	Avail.Storage	Storage Description
#1	83.00'	227 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	82.00'	98 cf	Filter Material (Prismatic) Listed below (Recalc)
			297 cf Overall x 33.0% Voids
		325 cf	Total Available Storage
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.00	157	0	0
84.00	297	227	227
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.00	297	0	0
83.00	297	297	297

Device	Routing	Invert	Outlet Devices
#1	Primary	83.75'	3.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.92 cfs @ 12.13 hrs HW=83.99' (Free Discharge)
 $\downarrow t_1$ =Broad-Crested Rectangular Weir (Weir Controls 0.92 cfs @ 1.26 fps)

Summary for Pond DMH: post - DMH

Inflow Area = 5,112 sf, 100.00% Impervious, Inflow Depth = 11.36" for 100-yr event
 Inflow = 0.76 cfs @ 12.14 hrs, Volume= 4,838 cf
 Outflow = 0.76 cfs @ 12.14 hrs, Volume= 4,838 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.76 cfs @ 12.14 hrs, Volume= 4,838 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Peak Elev= 83.36' @ 12.14 hrs
 Flood Elev= 85.00'

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Device	Routing	Invert	Outlet Devices
#1 Primary	82.80'	8.0" Round Culvert	L= 92.0' RCP, sq cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 82.80' / 82.00' S= 0.0087' / Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=0.75 cfs @ 12.14 hrs HW=83.36' (Free Discharge)
↑=Culvert (Barrel Controls 0.75 cfs @ 3.26 fps)

Summary for Pond FB1: Post - Forebay-1

Inflow Area = 12,569 sf, 42.19% Impervious, Inflow Depth = 6.69" for 100-yr event
Inflow = 1.82 cfs @ 12.14 hrs, Volume= 7,004 cf
Outflow = 1.81 cfs @ 12.15 hrs, Volume= 6,831 cf, Atten= 1%, Lag= 0.9 min
Primary = 1.81 cfs @ 12.15 hrs, Volume= 6,831 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 86.38' @ 12.15 hrs Surf.Area= 325 sf Storage= 281 cf
Flood Elev= 86.50' Surf.Area= 350 sf Storage= 323 cf

Plug-Flow detention time= 30.9 min calculated for 6,828 cf (97% of inflow)
Center-of-Mass det. time= 15.7 min (794.3 - 778.5)

Volume	Invert	Avail Storage	Storage Description	Custom Stage Data (Prismatic) Listed below (Recalc)
#1	85.00'	323 cf		
Elevation	Surf Area (sq-ft)	Inc. Store (cubic-feet)	Cum. Store (cubic-feet)	
85.00	95	0	0	
86.00	250	173	173	
86.50	350	150	323	

Device	Routing	Invert	Outlet Devices	
#1 Primary	86.00'	3.0 long x 2.0 breadth Broad-Crested Rectangular Weir	Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32	

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Primary OutFlow Max=1.79 cfs @ 12.15 hrs HW=86.37' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 1.79 cfs @ 1.59 fps)

Summary for Pond FB2: Post - Forebay-2

Inflow Area = 4,810 sf, 62.31% Impervious, Inflow Depth = 7.27" for 100-yr event
Inflow = 0.78 cfs @ 12.11 hrs, Volume= 2,914 cf
Outflow = 0.77 cfs @ 12.12 hrs, Volume= 2,838 cf, Atten= 1%, Lag= 0.3 min
Primary = 0.77 cfs @ 12.12 hrs, Volume= 2,838 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 83.98' @ 12.12 hrs Surf.Area= 141 sf Storage= 100 cf
Flood Elev= 84.00' Surf.Area= 143 sf Storage= 103 cf

Plug-Flow detention time= 32.3 min calculated for 2,836 cf (97% of inflow)
Center-of-Mass det. time= 16.0 min (778.9 - 762.9)

Volume	Invert	Avail.Storage	Storage Description	Custom Stage Data (Prismatic) Listed below (Recalc)
#1	83.00'	103 cf		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
83.00 84.00	63 143	0 103	0 103	

Device	Routing	Invert	Outlet Devices	4.0' long x 2.0' breadth Broad-Crested Rectangular Weir
#1	Primary	83.80'		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.77 cfs @ 12.12 hrs HW=83.98' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.77 cfs @ 1.08 fps)

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Summary for Pond FB3: Post - Forebay-3

Inflow Area = 18,298 sf, 49.99% Impervious, Inflow Depth = 7.97" for 100-yr event
Inflow = 2.51 cfs @ 12.08 hrs, Volume= 12,160 cf
Outflow = 2.47 cfs @ 12.09 hrs, Volume= 11,968 cf, Atten= 1%, Lag= 0.8 min
Primary = 2.47 cfs @ 12.09 hrs, Volume= 11,968 cf

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 82.46' @ 12.09 hrs Surf.Area= 390 sf Storage= 343 cf
Flood Elev= 82.50' Surf.Area= 400 sf Storage= 358 cf

Plug-Flow detention time= 30.9 min calculated for 11,968 cf (98% of inflow)
Center-of-Mass det. time= 12.1 min (844.4 - 832.3)

Volume	Invert	Avail.Storage	Storage Description
#1	81.00'	358 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.00	121	0	0
82.00	263	192	192
82.50	400	166	358

Device	Routing	Invert	Outlet Devices
#1	Primary	82.00'	3'0" long x 2'0" breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=2.43 cfs @ 12.09 hrs HW=82.46' (Free Discharge)
↳₁=Broad-Crested Rectangular Weir (Weir Controls 2.43 cfs @ 1.77 fps)

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Summary for Pond pA1: Analysis Point - Post Dev Catch Basin in Hope St.

Inflow Area = 16,759 sf, 0.73% Impervious, Inflow Depth = 4.29" for 100-yr event
Inflow = 1.20 cfs @ 12.43 hrs, Volume= 5,986 cf
Primary = 1.20 cfs @ 12.43 hrs, Volume= 5,986 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Pond pA2: Analysis Point - Post Dev Flow to Wetland

Inflow Area = 62,928 sf, 41.05% Impervious, Inflow Depth = 6.43" for 100-yr event
Inflow = 6.49 cfs @ 12.22 hrs, Volume= 33,700 cf
Primary = 6.49 cfs @ 12.22 hrs, Volume= 33,700 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Subcatchment pB1: Post - Runoff to Detention Basin-1

Runoff = 0.40 cfs @ 12.08 hrs, Volume= 1,225 cf, Depth= 5.47"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description
2,690	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
0	98	Walkway
0	98	Roofs, HSG C
2,690	74	Weighted Average
2,690	74	100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pF1: Post - Runoff to Forebay-1

Runoff = 1.82 cfs @ 12.14 hrs, Volume= 7,004 cf, Depth= 6.69"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description
4,205	98	Paved parking, HSG C
7,266	74	>75% Grass cover, Good, HSG C
*	979	Walkway
*	119	Patio/Deck/Stairs
12,569	84	Weighted Average
7,266	74	57.81% Pervious Area
5,303	98	42.19% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
9.5	44	0.0300
0.5	82	0.0180
10.0	126	Total
		0.08
		Sheet Flow, AB
		Grass: Bermuda n= 0.410 P2= 3.30"
		Shallow Concentrated Flow, BC
		Paved Kv= 20.3 fps

Summary for Subcatchment pF2: Post - Runoff to Forebay-2

Runoff = 0.78 cfs @ 12.11 hrs, Volume= 2,914 cf, Depth= 7.27"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr 100-yr Rainfall=8.60"

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Area (sf)	CN	Description
1,813	74	>75% Grass cover, Good, HSG C
2,847	98	Paved parking, HSG C
* 150	98	Walkway
0	98	Roofs, HSG C
4,810	89	Weighted Average
1,813	74	37.69% Pervious Area
2,997	98	62.31% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
7.9	40	0.0400
Velocity (ft/sec)	Capacity (cfs)	Description
0.08		Sheet Flow, AB
		Grass: Bermuda n= 0.410 P2= 3.30"
		Shallow Concentrated Flow, BC
0.1	24	0.0200
		Paved Kv= 20.3 fps
8.0	64	Total

Summary for Subcatchment pS1: Post - Runoff to Bio-retention Basin

$$\text{Runoff} = 0.43 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 1,299 \text{ cf, Depth=} 5.47"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description
2,851	74	>75% Grass cover, Good, HSG C
*	0	Patio/Deck/Stairs
2,851	74	Weighted Average
2,851	74	100.00% Pervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
Velocity (ft/sec)	Capacity (cfs)	Description
		Direct Entry, Runoff to swale
		5.0

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Summary for Subcatchment pS2: Post - Runoff to Swale-1

Runoff = 0.80 cfs @ 12.08 hrs, Volume= 2,471 cf, Depth= 5.73"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description			
0	98	Paved parking, HSG C			
4,696	74	>75% Grass cover, Good, HSG C			
*	0	Walkway			
*	477	Patio/Deck/Stairs			
5,173	76	Weighted Average			
4,696	74	90.78% Pervious Area			
477	98	9.22% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Parking Runoff

Summary for Subcatchment pS3: Post - Flow to wetlands

Runoff = 0.54 cfs @ 12.30 hrs, Volume= 2,600 cf, Depth= 5.47"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description
5,708	74	>75% Grass cover, Good, HSG C
0	98	Paved parking, HSG C
0	70	Woods, Good, HSG C
5,708	74	Weighted Average
5,708	74	100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.2	50	0.0260	0.07		Sheet Flow, AB
8.6	183	0.0050	0.35		Grass: Bermuda n= 0.410 P2= 3.30" Shallow Concentrated Flow, BC
1.9	117	0.0400	1.00		Woodland Kv= 5.0 fps Shallow Concentrated Flow, CD
0.3	103	0.0300	5.33	26.67	Woodland Kv= 5.0 fps Channel Flow, DE Area= 5.0 sf Perim= 10.2' r= 0.49' n= 0.030 Earth, grassed & winding
22.1	453	Total			

Summary for Subcatchment pS4: Post - Runoff to Bio-retention Basin-4

Runoff = 0.18 cfs @ 12.12 hrs, Volume= 602 cf, Depth= 5.47"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description
1,321	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
*	0	Walkway
*	0	Roofs, HSG C
1,321	74	Weighted Average
1,321	74	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	40	0.0400	0.08		Sheet Flow, AB
0.1	24	0.0200	2.87		Grass: Bermuda n= 0.410 P2= 3.30" Shallow Concentrated Flow, BC
8.0	64	Total			Paved Kv= 20.3 fps

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Summary for Subcatchment pS5: Post - Runoff to Bio-retention Basin-2

$$\text{Runoff} = 0.29 \text{ cfs} @ 12.08 \text{ hrs, Volume=} 895 \text{ cf, Depth=} 5.84"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description			
0	98	Paved parking, HSG C			
1,602	74	>75% Grass cover, Good, HSG C			
*	0	Walkway			
*	238	Patio/Deck/Stairs			
0	98	Roofs, HSG C			
1,840	77	Weighted Average			
1,602	74	87.07% Pervious Area			
238	98	12.93% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pS6: Post - Runoff to Forebay-3

$$\text{Runoff} = 2.26 \text{ cfs} @ 12.07 \text{ hrs, Volume=} 7,295 \text{ cf, Depth=} 6.42"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description
3,996	98	Paved parking, HSG C
9,151	74	>75% Grass cover, Good, HSG C
*	493	Walkway
13,640	82	Weighted Average
9,151	74	67.09% Pervious Area
4,489	98	32.91% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pS7: Post - Flow to Hope Street

Runoff = 1.38 cfs @ 12.42 hrs, Volume= 7,663 cf, Depth= 5.49"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description
16,637	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
*	0	Walkways
*	122	Patio/Deck/Stairs
16,759	74	Weighted Average
16,637	74	99.27% Pervious Area
122	98	0.73% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
		(ft/sec)
30.6	60	0.0030
0.2	44	0.0450
30.8	104	Total
		0.03
		3.42
		0.51

Sheet Flow, AB
Grass: Bermuda n= 0.410 P2= 3.30"
Shallow Concentrated Flow, BC
Unpaved Kv= 16.1 fps

Summary for Subcatchment RF1: Post - Roof Area (Units 3&4)

Runoff = 0.51 cfs @ 12.07 hrs, Volume= 1,781 cf, Depth= 8.36"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

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Area (sf)	CN	Description
2,556	98	Roofs, HSG C

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF2: Post - Roof Area (Units 5&6)

Runoff = 0.51 cfs @ 12.07 hrs, Volume= 1,781 cf, Depth= 8.36"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF3: Post - Roof Area (Units 7&8)

Runoff = 0.51 cfs @ 12.07 hrs, Volume= 1,781 cf, Depth= 8.36"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF4: Post- Roof Area (Units 9&10)

Runoff = 0.42 cfs @ 12.07 hrs, Volume= 1,464 cf, Depth= 8.36"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description
2,102	98	Roofs, HSG C
2,102	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF5: Post - Roof Area (Units 1&2)

Runoff = 0.51 cfs @ 12.07 hrs, Volume= 1,781 cf, Depth= 8.36"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr 100-yr Rainfall=8.60"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

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Summary for Reach S1: Post - Dry Swale 1

Inflow Area = 20,593 sf, 28.07% Impervious, Inflow Depth = 4.66" for 100-yr event
Inflow = 2.71 cfs @ 12.11 hrs, Volume= 8,000 cf
Outflow = 2.65 cfs @ 12.15 hrs, Volume= 8,000 cf, Atten= 2%, Lag= 2.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Max. Velocity= 2.59 fps, Min. Travel Time= 1.0 min
Avg. Velocity = 0.56 fps, Avg. Travel Time= 4.4 min

Peak Storage= 158 cf @ 12.13 hrs

Average Depth at Peak Storage= 0.38'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 17.43 cfs

Custom cross-section, Length= 150.0' Slope= 0.0200 '/'
Constant n= 0.035 Earth, dense weeds
Inlet Invert= 85.00', Outlet Invert= 82.00'



Offset (feet)	Elevation (feet)	Chan.Depth (feet)	Storage (cubic-feet)	Discharge (cfs)
-3.00	1.00	0.00		
-1.00	0.00	1.00		
1.00	0.00	1.00		
3.00	1.00	0.00		
Depth (feet)	End Area (sq-ft)	Perim. (feet)	(cubic-feet)	(cfs)
0.00	0.0	2.0	0	0.00
1.00	4.0	6.5	600	17.43

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Summary for Pond SB: Post - Settling Basin

Inflow Area = 25,705 sf, 42.37% Impervious, Inflow Depth = 5.99" for 100-yr event
Inflow = 3.41 cfs @ 12.15 hrs, Volume= 12,837 cf
Outflow = 3.41 cfs @ 12.16 hrs, Volume= 12,618 cf, Atten= 0%, Lag= 0.9 min
Primary = 3.41 cfs @ 12.16 hrs, Volume= 12,618 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 82.47' @ 12.16 hrs Surf.Area= 449 sf Storage= 404 cf

Plug-Flow detention time= 34.2 min calculated for 12,618 cf (98% of inflow)
Center-of-Mass det. time= 8.4 min (918.7 - 910.3)

Volume	Invert	Avail Storage	Storage Description
#1	81.00'	416 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.00	108	0	0
82.00	331	220	220
82.50	456	197	416

Device	Routing	Invert	Outlet Devices
#1	Primary	82.00'	4.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61

Primary OutFlow Max=3.40 cfs @ 12.16 hrs HW=82.47' (Free Discharge)
↑**1=Broad-Crested Rectangular Weir** (Weir Controls 3.40 cfs @ 1.80 fps)

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Summary for Pond US1: Post - Recharge System-1

Inflow Area =	5,112 sf, 100.00% Impervious,	Inflow Depth = 8.36"	for 100-yr event
Inflow =	1.01 cfs @ 12.07 hrs,	Volume= 3,561 cf	
Outflow =	0.74 cfs @ 12.14 hrs,	Volume= 3,561 cf, Atten= 27%, Lag= 4.2 min	
Discarded =	0.01 cfs @ 2.96 hrs,	Volume= 999 cf	
Primary =	0.73 cfs @ 12.14 hrs,	Volume= 2,562 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 85.70' @ 12.14 hrs Surf.Area= 715 sf Storage= 696 cf
Flood Elev= 86.30' Surf.Area= 715 sf Storage= 837 cf

Plug-Flow detention time= 130.6 min calculated for 3,559 cf (100% of inflow)
Center-of-Mass det. time= 130.9 min (870.3 - 739.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	84.00'	405 cf	21.50'W x 33.25'L x 2.33'H Field A 1,668 cf Overall - 439 cf Embedded = 1,229 cf x 33.0% Voids
#2A	84.50'	439 cf	Infiltrator H-20 Chamber x 30 Inside #1 Effective Size= 27.5"W x 14.0"H => 2.34 sf x 6.25'L = 14.6 cf Overall Size= 34.0"W x 16.0"H x 6.25'L
		845 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	84.00'	0.500 in/hr Exfiltration over Surface area
#2	Primary	84.85'	6.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 84.85' / 84.60' S= 0.0208 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Discarded OutFlow Max=0.01 cfs @ 2.96 hrs HW=84.02' (Free Discharge)
↑1=Exfiltration (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.73 cts @ 12.14 hrs HW=85.69' (Free Discharge)
↑2=Culvert (Inlet Controls 0.73 cfs @ 3.70 fps)

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Summary for Pond US2: Post - Underground Storage System

Inflow Area = 4,658 sf, 100.00% Impervious, Inflow Depth = 8.36" for 100-yr event
Inflow = 0.92 cfs @ 12.07 hrs, Volume= 3,245 cf
Outflow = 0.22 cfs @ 12.44 hrs, Volume= 3,245 cf, Atten= 76%, Lag= 22.0 min
Primary = 0.22 cfs @ 12.44 hrs, Volume= 3,245 cf

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 83.97' @ 12.44 hrs Surf.Area= 1,255 sf Storage= 828 cf

Plug-Flow detention time= 37.3 min calculated for 3,243 cf (100% of inflow)
Center-of-Mass det. time= 37.5 min (776.9 - 739.4)

Volume	Invert	Avail Storage	Storage Description
#1	83.15'	1,484 cf	18.0" Round Pipe Storage x 14 L= 60.0

Device	Routing	Invert	Outlet Devices
#1	Primary	83.15'	4.0" Round Culvert L= 100.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 83.15' / 82.10' S= 0.0105 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.09 sf

Primary OutFlow Max=0.22 cfs @ 12.44 hrs HW=83.97' (Free Discharge)
↳ 1=Culvert (Barrel Controls 0.22 cfs @ 2.56 fps)

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Time span=0.00-72.00 hrs, dt=0.04 hrs, 1801 points

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv.

Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Pond B1: Post - Dry Extended Detention Basin

Peak Elev=80.01' Storage=14 cf Inflow=0.00 cfs 14 cf
Primary=0.00 cfs 0 cf Secondary=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf

Peak Elev=83.78' Storage=272 cf Inflow=0.10 cfs 315 cf
Primary=0.00 cfs 0 cf Secondary=0.00 cfs 54 cf Outflow=0.00 cfs 54 cf

Peak Elev=81.58' Storage=444 cf Inflow=0.22 cfs 883 cf
Primary=0.00 cfs 0 cf Secondary=0.02 cfs 809 cf Outflow=0.02 cfs 809 cf

Peak Elev=83.68' Storage=62 cf Inflow=0.01 cfs 96 cf
Primary=0.00 cfs 0 cf Secondary=0.00 cfs 39 cf Outflow=0.00 cfs 39 cf

Peak Elev=83.46' Storage=186 cf Inflow=0.07 cfs 186 cf
Outflow=0.00 cfs 0 cf

Peak Elev=82.83' Storage=187 cf Inflow=0.12 cfs 473 cf
8.0" Round Culvert n=0.013 L=92.0' S=0.0087' Outflow=0.10 cfs 300 cf

Peak Elev=86.06' Storage=81 cf Inflow=0.07 cfs 255 cf
Outflow=0.07 cfs 179 cf

Peak Elev=83.84' Storage=81 cf Inflow=0.07 cfs 255 cf
Outflow=0.07 cfs 179 cf

Peak Elev=82.07' Storage=212 cf Inflow=0.16 cfs 837 cf
Outflow=0.15 cfs 645 cf

Inflow=0.00 cfs 0 cf
Primary=0.00 cfs 0 cf

Inflow=0.02 cfs 838 cf
Primary=0.02 cfs 838 cf

Pond pA1: Analysis Point - Post Dev Catch Basin in Hope St.

Pond pA2: Analysis Point - Post Dev Flow to Wetland

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Subcatchment pB1: Post - Runoff to Detention Basin-1

Runoff Area=2,690 sf 0.00% Impervious Runoff Depth=0.06"
Tc=5.0 min CN=74/0 Runoff=0.00 cfs 14 cf

Runoff Area=12,569 sf 42.19% Impervious Runoff Depth=0.45"
Flow Length=126' Tc=10.0 min CN=74/98 Runoff=0.12 cfs 473 cf

Subcatchment pF1: Post - Runoff to Forebay-2

Runoff Area=4,810 sf 62.31% Impervious Runoff Depth=0.64"
Flow Length=64' Tc=8.0 min CN=74/98 Runoff=0.07 cfs 255 cf

Subcatchment pS1: Post - Runoff to Bio-retention Basin

Runoff Area=2,851 sf 0.00% Impervious Runoff Depth=0.06"
Tc=5.0 min CN=74/0 Runoff=0.00 cfs 15 cf

Runoff Area=5,173 sf 9.22% Impervious Runoff Depth=0.15"
Tc=5.0 min CN=74/98 Runoff=0.01 cfs 63 cf

Subcatchment pS2: Post - Runoff to Swale-1

Runoff Area=5,708 sf 0.00% Impervious Runoff Depth=0.06"
Flow Length=453' Tc=22.1 min CN=74/0 Runoff=0.00 cfs 29 cf

Subcatchment pS3: Post - Flow to wetlands

Runoff Area=1,321 sf 0.00% Impervious Runoff Depth=0.06"
Flow Length=64' Tc=8.0 min CN=74/0 Runoff=0.00 cfs 7 cf

Subcatchment pS4: Post - Runoff to Bio-retention Basin-4

Runoff Area=1,840 sf 12.93% Impervious Runoff Depth=0.18"
Tc=5.0 min CN=74/98 Runoff=0.01 cfs 28 cf

Subcatchment pS5: Post - Runoff to Bio-retention Basin-2

Runoff Area=13,640 sf 32.91% Impervious Runoff Depth=0.37"
Tc=5.0 min CN=74/98 Runoff=0.12 cfs 416 cf

Subcatchment pS7: Post - Flow to Hope Street

Runoff Area=16,759 sf 0.73% Impervious Runoff Depth=0.07"
Flow Length=104' Tc=30.8 min CN=74/98 Runoff=0.01 cfs 96 cf

Subcatchment RF1: Post - Roof Area (Units 3&4)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=0.99"
Tc=5.0 min CN=0/98 Runoff=0.07 cfs 210 cf

Subcatchment RF2: Post - Roof Area (Units 5&6)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=0.99"
Tc=5.0 min CN=0/98 Runoff=0.07 cfs 210 cf

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Subcatchment RF3: Post - Roof Area (Units 7&8)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=0.99"
Tc=5.0 min CN=0/98 Runoff=0.07 cfs 210 cf

Subcatchment RF4: Post- Roof Area (Units 9&10)

Runoff Area=2,102 sf 100.00% Impervious Runoff Depth=0.99"
Tc=5.0 min CN=0/98 Runoff=0.05 cfs 173 cf

Subcatchment RF5: Post - Roof Area (Units 1&2)

Runoff Area=2,556 sf 100.00% Impervious Runoff Depth=0.99"
Tc=5.0 min CN=0/98 Runoff=0.07 cfs 210 cf

Avg. Flow Depth=0.01' Max Vel=0.36 fps Inflow=0.01 cfs 63 cf
n=0.035 L=150.0' S=0.0200 '/' Capacity=17.43 cfs Outflow=0.01 cfs 63 cf

Pond SB: Post - Settling Basin

Peak Elev=81.65' Storage=117 cf Inflow=0.01 cfs 117 cf
Outflow=0.00 cfs 0 cf

Pond US1: Post - Recharge System-1

Discarded=0.01 cfs 420 cf Primary=0.00 cfs 0 cf Outflow=0.01 cfs 420 cf

Pond US2: Post - Underground Storage System

Peak Elev=83.32' Storage=90 cf Inflow=0.12 cfs 383 cf
4.0" Round Culvert n=0.013 L=100.0' S=0.0105 '/' Outflow=0.06 cfs 383 cf

Total Runoff Area = 79,687 sf Runoff Volume = 2,408 cf Average Runoff Depth = 0.36"
67.43% Pervious = 53,735 sf 32.57% Impervious = 25,952 sf

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Summary for Pond B1: Post - Dry Extended Detention Basin

Inflow Area = 57,220 sf, 45.14% Impervious, Inflow Depth = 0.00" for WQv event
Inflow = 0.00 cfs @ 12.40 hrs, Volume= 14 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 80.01' @ 24.32 hrs Surf.Area= 1,277 sf Storage= 14 cf
Flood Elev= 82.50' Surf.Area= 2,300 sf Storage= 4,432 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description	#1	80.00'	4,432 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)				
80.00	1,273	0	0				
81.00	1,660	1,467	1,467				
82.00	2,080	1,870	3,337				
82.50	2,300	1,095	4,432				
Device	Routing	Invert	Outlet Devices	#1	Secondary	82.00'	4.0' Long x 2.0' breadth Broad-Crested Rectangular Weir
							Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50
							Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Primary	80.45'	8.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500				Inlet / Outlet Invert= 80.45' / 79.50' S= 0.0792' Cc= 0.900
#3	Primary	80.25'	4.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500				n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
							Inlet / Outlet Invert= 80.25' / 79.50' S= 0.0625' Cc= 0.900
							n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.09 sf

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Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=80.00' (Free Discharge)
↳ 2=Culvert (Controls 0.00 cfs)
↳ 3=Culvert (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=80.00' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond BR1: Post - Bio-retention Basin-1

Inflow Area =	15,420 sf	34.39% Impervious,	Inflow Depth = 0.25"	for WQv event
Inflow =	0.10 cfs @ 12.22 hrs,	Volume= 315 cf		
Outflow =	0.00 cfs @ 20.81 hrs,	Volume= 54 cf, Atten= 97%, Lag= 515.6 min		
Primary =	0.00 cfs @ 0.00 hrs,	Volume= 0 cf		
Secondary =	0.00 cfs @ 20.81 hrs,	Volume= 54 cf		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 83.78' @ 20.81 hrs Surf.Area= 1,055 sf Storage= 272 cf
Flood Elev= 86.00' Surf.Area= 2,110 sf Storage= 1,567 cf

Plug-Flow detention time= 603.4 min calculated for 54 cf (17% of inflow)
Center-of-Mass det. time= 442.2 min (1,338.2 - 895.9)

Volume	Invert	Avail.Storage	Storage Description
#1	85.00'	867 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	83.00'	694 cf	Filter Material (Prismatic) Listed below (Recalc)
#3	83.75'	7 cf	2,110 cf Overall - 7 cf Embedded = 2,103 cf x 33.0% Voids 6.0" Round Pipe Storage Inside #2 L= 35.0'
1,567 cf Total Available Storage			
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
85.00	678	0	0
86.00	1,055	867	867

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Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.00	1,055	0	0
85.00	1,055	2,110	2,110

Device	Routing	Invert	Outlet Devices
#1 Primary	85.75'	4'0" long x 2.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 Coef. (English) 2.48 2.60 2.60 2.64 2.65 2.68 2.75 2.74 2.76 2.89 3.05 3.19 3.32	

#2 Device 3 83.00'
#3 Secondary 83.75'
0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'
6.0" Round Culvert L= 100.0' RCP, sq.cut end projecting, Ke= 0.500
Inlet / Outlet Invert= 83.75' / 82.85' S= 0.0090' Cc= 0.900
n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=83.00' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 20.81 hrs HW=83.78' (Free Discharge)
↳ 3=Culvert (Barrel Controls 0.00 cfs @ 0.68 fps)
↳ 2=Exfiltration (Passes 0.00 cfs of 0.01 cfs potential flow)

Summary for Pond BR2: Post - Bio-retention Basin-2

Inflow Area = 22,694 sf, 52.62% Impervious, Inflow Depth = 0.47" for WQv event
Inflow = 0.22 cfs @ 12.10 hrs, Volume= 883 cf
Outflow = 0.02 cfs @ 14.55 hrs, Volume= 809 cf, Atten= 92%, Lag= 147.1 min
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Secondary = 0.02 cfs @ 14.55 hrs, Volume= 809 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 81.58' @ 14.55 hrs Surf.Area= 1,526 sf Storage= 444 cf
Flood Elev= 82.50' Surf.Area= 1,796 sf Storage= 1,140 cf

Plug-Flow detention time= 339.0 min calculated for 809 cf (92% of inflow)
Center-of-Mass det. time= 291.8 min (1,161.9 - 870.1)

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Volume	Invert	Avail.Storage	Storage Description
#1	81.35'	836 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	80.35'	292 cf	Filter Material (Prismatic) Listed below (Recalc)
#3	80.60'	12 cf	6.0" Round Pipe Storage Inside #2 L= 61.0'

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.35	563	0	0
82.00	745	425	425
82.50	898	411	836

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
80.35	898	0	0
81.35	898	898	898

Device	Routing	Invert	Outlet Devices
#1	Primary	82.10'	3.5' long x 2' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2	Device 3	80.35'	0.500 in/hr Exfiltration over Surface area Phase-In= 0.01'
#3	Secondary	80.60'	6.0" Round Culvert L= 62.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 80.60' / 79.00' S= 0.0258 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=80.35' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=0.02 cfs @ 14.55 hrs HW=81.58' (Free Discharge)
↳ 2=Exfiltration (Exfiltration Controls 0.02 cfs)
↳ 3=Culvert (Passes 0.02 cfs of 0.81 cfs potential flow)

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Summary for Pond BR3: Post - Bio-retention Basin-3

Inflow Area =	16,759 sf,	0.73% Impervious, Inflow Depth = 0.07"	for WQv event
Inflow =	0.01 cfs @ 12.77 hrs, Volume=	96 cf	
Outflow =	0.00 cfs @ 18.37 hrs, Volume=	39 cf, Atten= 71%, Lag= 335.9 min	
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf	
Secondary =	0.00 cfs @ 18.37 hrs, Volume=	39 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Peak Elev= 83.68' @ 18.37 hrs Surf.Area= 686 sf Storage= 62 cf

Plug-Flow detention time= 491.2 min calculated for 39 cf (41% of inflow)
 Center-of-Mass det. time= 298.9 min (1,289.2 - 990.3)

Volume Invert Avail.Storage Storage Description

#1	84.40'	371 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	83.40'	224 cf	Filter Material (Prismatic) Listed below (Recalc)
		686 cf Overall - 6 cf Embedded = 680 cf x 33.0% Voids	
#3	83.65'	6 cf	6.0" Round Pipe Storage Inside #2 L= 30.0'

601 cf Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
84.40	374	0	0
85.10	686	371	371

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.40	686	0	0
84.40	686	686	686

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Device	Routing	Invert	Outlet Devices
#1 Primary	84.95'	8.0' long x 2.0' breadth Broad-Crested Rectangular Weir	Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32
#2 Device 3	85.00'	6.0" Horiz. Orifice/Grate	C=0.600 Limited to weir flow at low heads
#3 Secondary	83.65'	6.0" Round Culvert	L= 68.0' RCP, sq cut end projecting, Ke= 0.500
#4 Device 3	83.40'	0.500 in/hr Exfiltration over Surface area	Inlet / Outlet Invert= 83.65' / 83.00' S= 0.0096' / Cc= 0.900 n= 0.013 Concrete pipe, bends & connections, Flow Area= 0.20 sf Phase-In= 0.01'

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=83.40' (Free Discharge)
1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 18.37 hrs HW=83.68' (Free Discharge)
3=Culvert (Barrel Controls 0.00 cfs @ 0.60 fps)
2=Orifice/Grate (Controls 0.00 cfs)
4=Exfiltration (Passes 0.00 cfs of 0.01 cfs potential flow)

Summary for Pond BR4: Post - Bio-retention Basin-4

Inflow Area = 6,131 sf, 48.88% Impervious, Inflow Depth = 0.36" for WQv event
Inflow = 0.07 cfs @ 12.12 hrs, Volume= 186 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 83.46' @ 25.16 hrs Surf.Area= 519 sf Storage= 186 cf
Flood Elev= 84.00' Surf.Area= 594 sf Storage= 325 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= (not calculated: no outflow)

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Volume	Invert	Avail.Storage	Storage Description
#1	83.00'	227 cf	Custom Stage Data (Prismatic) Listed below (Recalc)
#2	82.00'	98 cf	Filter Material (Prismatic) Listed below (Recalc)
			297 cf Overall x 33.0% Voids
		325 cf	Total Available Storage
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
83.00	157	0	0
84.00	297	227	227
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
82.00	297	0	0
83.00	297	297	297

Device	Routing	Invert	Outlet Devices
#1	Primary	83.75'	3.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=82.00' (Free Discharge)
 $\downarrow t_1$ =Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond DMH: post - DMH

Inflow Area = 5,112 sf, 100.00% Impervious, Inflow Depth = 0.13" for WQv event
 Inflow = 0.00 cfs @ 20.81 hrs, Volume= 54 cf
 Outflow = 0.00 cfs @ 20.81 hrs, Volume= 54 cf, Atten= 0%, Lag= 0.0 min
 Primary = 0.00 cfs @ 20.81 hrs, Volume= 54 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Peak Elev= 82.83' @ 20.81 hrs
 Flood Elev= 85.00'

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Device	Routing	Invert	Outlet Devices
#1 Primary	82.80'	8.0" Round Culvert	L= 92.0' RCP, sq cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 82.80' / 82.00' S= 0.0087' / Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf

Primary OutFlow Max=0.00 cfs @ 20.81 hrs HW=82.83 (Free Discharge)
↑=Culvert (Barrel Controls 0.00 cfs @ 0.63 fps)

Summary for Pond FB1: Post - Forebay-1

Inflow Area = 12,569 sf, 42.19% Impervious, Inflow Depth = 0.45" for WQv event
Inflow = 0.12 cfs @ 12.14 hrs, Volume= 473 cf
Outflow = 0.10 cfs @ 12.22 hrs, Volume= 300 cf, Atten= 14%, Lag= 4.9 min
Primary = 0.10 cfs @ 12.22 hrs, Volume= 300 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 86.06' @ 12.22 hrs Surf.Area= 261 sf Storage= 187 cf
Flood Elev= 86.50' Surf.Area= 350 sf Storage= 323 cf

Plug-Flow detention time= 199.3 min calculated for 300 cf (64% of inflow)
Center-of-Mass det. time= 89.4 min (891.5 - 802.1)

Volume	Invert	Avail Storage	Storage Description	Custom Stage Data (Prismatic) Listed below (Recalc)
#1	85.00'	323 cf		
Elevation	Surf Area (sq-ft)	Inc. Store (cubic-feet)	Cum. Store (cubic-feet)	
85.00	95	0	0	
86.00	250	173	173	
86.50	350	150	323	

Device	Routing	Invert	Outlet Devices	
#1 Primary	86.00'	3.0 long x 2.0 breadth Broad-Crested Rectangular Weir	Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32	

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Primary OutFlow Max=0.09 cfs @ 12.22 hrs HW=86.05' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.09 cfs @ 0.59 fps)

Summary for Pond FB2: Post - Forebay-2

Inflow Area =	4,810 sf,	62.31% Impervious,	Inflow Depth = 0.64"	for WQv event
Inflow =	0.07 cfs @ 12.11 hrs,	Volume= 255 cf		
Outflow =	0.07 cfs @ 12.12 hrs,	Volume= 179 cf, Atten= 2%, Lag= 0.7 min		
Primary =	0.07 cfs @ 12.12 hrs,	Volume= 179 cf		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs / 2
Peak Elev= 83.84' @ 12.12 hrs Surf.Area= 130 sf Storage= 81 cf
Flood Elev= 84.00' Surf.Area= 143 sf Storage= 103 cf

Plug-Flow detention time= 164.5 min calculated for 179 cf (70% of inflow)
Center-of-Mass det. time= 69.0 min (860.4 - 791.4)

Volume	Invert	Avail.Storage	Storage Description	Custom Stage Data (Prismatic) Listed below (Recalc)
#1	83.00'	103 cf		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	

Device	Routing	Invert	Outlet Devices	4.0' long x 2.0' breadth Broad-Crested Rectangular Weir
#1	Primary	83.80'		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.07 cfs @ 12.12 hrs HW=83.84' (Free Discharge)
↳ 1=Broad-Crested Rectangular Weir (Weir Controls 0.07 cfs @ 0.48 fps)

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Summary for Pond FB3: Post - Forebay-3

Inflow Area = 18,298 sf, 49.99% Impervious, Inflow Depth = 0.55" for WQv event
Inflow = 0.16 cfs @ 12.08 hrs, Volume= 83 cf
Outflow = 0.15 cfs @ 12.12 hrs, Volume= 645 cf, Atten= 4%, Lag= 1.9 min
Primary = 0.15 cfs @ 12.12 hrs, Volume= 645 cf

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 82.07' @ 12.12 hrs Surf.Area= 283 sf Storage= 212 cf
Flood Elev= 82.50' Surf.Area= 400 sf Storage= 358 cf

Plug-Flow detention time= 167.3 min calculated for 645 cf (77% of inflow)
Center-of-Mass det. time= 64.7 min (900.2 - 835.5)

Volume	Invert	Avail.Storage	Storage Description
#1	81.00'	358 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.00	121	0	0
82.00	263	192	192
82.50	400	166	358

Device	Routing	Invert	Outlet Devices
#1	Primary	82.00'	3'0" long x 2'0" breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32

Primary OutFlow Max=0.15 cfs @ 12.12 hrs HW=82.07' (Free Discharge)
↳₁=Broad-Crested Rectangular Weir (Weir Controls 0.15 cfs @ 0.69 fps)

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Summary for Pond pA1: Analysis Point - Post Dev Catch Basin in Hope St.

Inflow Area = 16,759 sf, 0.73% Impervious, Inflow Depth = 0.00" for WQv event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Pond pA2: Analysis Point - Post Dev Flow to Wetland

Inflow Area = 62,928 sf, 41.05% Impervious, Inflow Depth = 0.16" for WQv event
Inflow = 0.02 cfs @ 12.72 hrs, Volume= 838 cf
Primary = 0.02 cfs @ 12.72 hrs, Volume= 838 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs

Summary for Subcatchment pB1: Post - Runoff to Detention Basin-1

Runoff = 0.00 cfs @ 12.40 hrs, Volume= 14 cf, Depth= 0.06"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description
2,690	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
*	0	Walkway
*	0	Roofs, HSG C
2,690	74	Weighted Average
2,690	74	100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pF1: Post - Runoff to Forebay-1

Runoff = 0.12 cfs @ 12.14 hrs, Volume= 473 cf, Depth= 0.45"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description
4,205	98	Paved parking, HSG C
7,266	74	>75% Grass cover, Good, HSG C
*	979	Walkway
*	119	Patio/Deck/Stairs
12,569	84	Weighted Average
7,266	74	57.81% Pervious Area
5,303	98	42.19% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
9.5	44	0.0300
0.5	82	0.0180
10.0	126	Total
		0.08
		Sheet Flow, AB
		Grass: Bermuda n= 0.410 P2= 3.30"
		Shallow Concentrated Flow, BC
		Paved Kv= 20.3 fps

Summary for Subcatchment pF2: Post - Runoff to Forebay-2

Runoff = 0.07 cfs @ 12.11 hrs, Volume= 255 cf, Depth= 0.64"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr WQv Rainfall=1.20"

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Area (sf)	CN	Description
1,813	74	>75% Grass cover, Good, HSG C
2,847	98	Paved parking, HSG C
*	150	Walkway
0	98	Roofs, HSG C
4,810	89	Weighted Average
1,813	74	37.69% Pervious Area
2,997	98	62.31% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
7.9	40	0.0400
Velocity (ft/sec)	Capacity (cfs)	Description
0.08		Sheet Flow, AB
		Grass: Bermuda n= 0.410 P2= 3.30"
		Shallow Concentrated Flow, BC
0.1	24	0.0200
		Paved Kv= 20.3 fps
8.0	64	Total

Summary for Subcatchment pS1: Post - Runoff to Bio-retention Basin

Runoff = 0.00 cfs @ 12.40 hrs, Volume= 15 cf, Depth= 0.06"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description
2,851	74	>75% Grass cover, Good, HSG C
*	0	Patio/Deck/Stairs
2,851	74	Weighted Average
2,851	74	100.00% Pervious Area
Tc (min)	Length (feet)	Velocity (ft/sec)
5.0		
Capacity (cfs)	Description	Direct Entry, Runoff to swale

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Summary for Subcatchment pS2: Post - Runoff to Swale-1

Runoff = 0.01 cfs @ 12.07 hrs, Volume= 63 cf, Depth= 0.15"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description			
0	98	Paved parking, HSG C			
4,696	74	>75% Grass cover, Good, HSG C			
*	0	Walkway			
*	477	Patio/Deck/Stairs			
5,173	76	Weighted Average			
4,696	74	90.78% Pervious Area			
477	98	9.22% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Parking Runoff

Summary for Subcatchment pS3: Post - Flow to wetlands

Runoff = 0.00 cfs @ 12.67 hrs, Volume= 29 cf, Depth= 0.06"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description
5,708	74	>75% Grass cover, Good, HSG C
0	98	Paved parking, HSG C
0	70	Woods, Good, HSG C
5,708	74	Weighted Average
5,708	74	100.00% Pervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.2	50	0.0260	0.07		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
8.6	183	0.0050	0.35		Shallow Concentrated Flow, BC
					Woodland Kv= 5.0 fps
1.9	117	0.0400	1.00		Shallow Concentrated Flow, CD
					Woodland Kv= 5.0 fps
0.3	103	0.0300	5.33	26.67	Channel Flow, DE
					Area= 5.0 sf Perim= 10.2' r= 0.49'
					n= 0.030 Earth, grassed & winding
22.1	453	Total			

Summary for Subcatchment pS4: Post - Runoff to Bio-retention Basin-4

Runoff = 0.00 cfs @ 12.45 hrs, Volume= 7 cf, Depth= 0.06"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description			
1,321	74	>75% Grass cover, Good, HSG C			
*	0	Paved parking, HSG C			
*	0	Walkway			
*	0	Roofs, HSG C			
1,321	74	Weighted Average			
1,321	74	100.00% Pervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.9	40	0.0400	0.08		Sheet Flow, AB
					Grass: Bermuda n= 0.410 P2= 3.30"
0.1	24	0.0200	2.87		Shallow Concentrated Flow, BC
					Paved Kv= 20.3 fps
8.0	64	Total			

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Summary for Subcatchment pS5: Post - Runoff to Bio-retention Basin-2

$$\text{Runoff} = 0.01 \text{ cfs} @ 12.07 \text{ hrs, Volume=} 28 \text{ cf, Depth=} 0.18"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description			
0	98	Paved parking, HSG C			
1,602	74	>75% Grass cover, Good, HSG C			
*	0	Walkway			
*	238	Patio/Deck/Stairs			
0	98	Roofs, HSG C			
1,840	77	Weighted Average			
1,602	74	87.07% Pervious Area			
238	98	12.93% Impervious Area			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pS6: Post - Runoff to Forebay-3

$$\text{Runoff} = 0.12 \text{ cfs} @ 12.07 \text{ hrs, Volume=} 416 \text{ cf, Depth=} 0.37"$$

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description
3,996	98	Paved parking, HSG C
9,151	74	>75% Grass cover, Good, HSG C
*	493	Walkway
13,640	82	Weighted Average
9,151	74	67.09% Pervious Area
4,489	98	32.91% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, AB

Summary for Subcatchment pS7: Post - Flow to Hope Street

Runoff = 0.01 cfs @ 12.77 hrs, Volume= 96 cf, Depth= 0.07"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description
16,637	74	>75% Grass cover, Good, HSG C
*	0	Paved parking, HSG C
*	0	Walkways
*	122	Patio/Deck/Stairs
16,759	74	Weighted Average
16,637	74	99.27% Pervious Area
122	98	0.73% Impervious Area
Tc (min)	Length (feet)	Slope (ft/ft)
		(ft/sec)
30.6	60	0.0030
0.2	44	0.0450
30.8	104	Total
		0.03
		3.42
		0.07

Sheet Flow, AB
 Grass: Bermuda n= 0.410 P2= 3.30"
Shallow Concentrated Flow, BC
 Unpaved Kv= 16.1 fps

Summary for Subcatchment RF1: Post - Roof Area (Units 3&4)

Runoff = 0.07 cfs @ 12.07 hrs, Volume= 210 cf, Depth= 0.99"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Type III 24-hr WQv Rainfall=1.20"

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Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF2: Post - Roof Area (Units 5&6)

Runoff = 0.07 cfs @ 12.07 hrs, Volume= 210 cf, Depth= 0.99"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF3: Post - Roof Area (Units 7&8)

Runoff = 0.07 cfs @ 12.07 hrs, Volume= 210 cf, Depth= 0.99"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

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Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF4: Post- Roof Area (Units 9&10)

Runoff = 0.05 cfs @ 12.07 hrs, Volume= 173 cf, Depth= 0.99"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description
2,102	98	Roofs, HSG C
2,102	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

Summary for Subcatchment RF5: Post - Roof Area (Units 1&2)

Runoff = 0.07 cfs @ 12.07 hrs, Volume= 210 cf, Depth= 0.99"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Type III 24-hr WQv Rainfall=1.20"

Area (sf)	CN	Description
2,556	98	Roofs, HSG C
2,556	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Roof Runoff

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Summary for Reach S1: Post - Dry Swale 1

Inflow Area = 20,593 sf, 28.07% Impervious, Inflow Depth = 0.04" for WQv event
Inflow = 0.01 cfs @ 12.07 hrs, Volume= 63 cf
Outflow = 0.01 cfs @ 12.25 hrs, Volume= 63 cf, Atten= 20%, Lag= 10.6 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Max. Velocity= 0.36 fps, Min. Travel Time= 7.0 min
Avg. Velocity = 0.28 fps, Avg. Travel Time= 9.0 min

Peak Storage= 4 cf @ 12.13 hrs

Average Depth at Peak Storage= 0.01'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 17.43 cfs

Custom cross-section, Length= 150.0' Slope= 0.0200 '/'
Constant n= 0.035 Earth, dense weeds
Inlet Invert= 85.00', Outlet Invert= 82.00'



Offset (feet)	Elevation (feet)	Perim. (feet)	Chan.Depth (feet)	Storage (cubic-feet)	Discharge (cfs)
-3.00	1.00	2.0	0.00	0	0.00
-1.00	0.00	1.00	0.00	600	17.43
1.00	0.00	1.00	0.00	0	0.00
3.00	1.00	2.0	0.00	0	0.00

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Summary for Pond SB: Post - Settling Basin

Inflow Area = 25,705 sf, 42.37% Impervious, Inflow Depth = 0.05" for WQv event
Inflow = 0.01 cfs @ 12.25 hrs, Volume= 117 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf

Routing by Stor-Ind method, Time Span=0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 81.65' @ 71.00 hrs Surf.Area= 253 sf Storage= 117 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail Storage	Storage Description
#1	81.00'	416 cf	Custom Stage Data (Prismatic) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
81.00	108	0	0
82.00	331	220	220
82.50	456	197	416

Device	Routing	Invert	Outlet Devices
#1	Primary	82.00'	4.0' long x 2.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61 2.61

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=81.00' (Free Discharge)
↑**1=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

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Summary for Pond US1: Post - Recharge System-1

Inflow Area =	5,112 sf, 100.00% Impervious, Inflow Depth = 0.99"	for WQv event
Inflow =	0.13 cfs @ 12.07 hrs, Volume= 420 cf	
Outflow =	0.01 cfs @ 11.20 hrs, Volume= 420 cf, Atten= 94%, Lag= 0.0 min	
Discarded =	0.01 cfs @ 11.20 hrs, Volume= 420 cf	
Primary =	0.00 cfs @ 0.00 hrs, Volume= 0 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
Peak Elev= 84.61' @ 13.61 hrs Surf.Area= 715 sf Storage= 181 cf
Flood Elev= 86.30' Surf.Area= 715 sf Storage= 837 cf

Plug-Flow detention time= 185.9 min calculated for 420 cf (100% of inflow)
Center-of-Mass det. time= 185.8 min (966.9 - 781.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	84.00'	405 cf	21.50'W x 33.25'L x 2.33'H Field A 1,668 cf Overall - 439 cf Embedded = 1,229 cf x 33.0% Voids
#2A	84.50'	439 cf	Infiltrator H-20 Chamber x 30 Inside #1 Effective Size= 27.5"W x 14.0"H => 2.34 sf x 6.25'L = 14.6 cf Overall Size= 34.0"W x 16.0"H x 6.25'L
		845 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	84.00'	0.500 in/hr Exfiltration over Surface area
#2	Primary	84.85'	6.0" Round Culvert L= 12.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 84.85' / 84.60' S= 0.0208 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Discarded OutFlow Max=0.01 cfs @ 11.20 hrs HW=84.02' (Free Discharge)
↑1=Exfiltration (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.00 cts @ 0.00 hrs HW=84.00' (Free Discharge)
↑2=Culvert (Controls 0.00 cfs)

22024 Hope Street HydroCAD 6-13-22

Prepared by {enter your company name here}
HydroCAD® 10.00 s/n 05085 © 2013 HydroCAD Software Solutions LLC

Type III 24-hr WQv Rainfall=1.20"
Printed 12/9/2022
Page 158

Summary for Pond US2: Post - Underground Storage System

Inflow Area = 4,658 sf, 100.00% Impervious, Inflow Depth = 0.99" for WQv event
 Inflow = 0.12 cfs @ 12.07 hrs, Volume= 383 cf
 Outflow = 0.06 cfs @ 12.22 hrs, Volume= 383 cf, Atten= 53%, Lag= 8.8 min
 Primary = 0.06 cfs @ 12.22 hrs, Volume= 383 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.04 hrs
 Peak Elev= 83.32' @ 12.22 hrs Surf.Area= 790 sf Storage= 90 cf

Plug-Flow detention time= 41.7 min calculated for 382 cf (100% of inflow)
 Center-of-Mass det. time= 41.9 min (823.0 - 781.1)

Volume	Invert	Avail Storage	Storage Description
#1	83.15'	1,484 cf	18.0" Round Pipe Storage x 14 L= 60.0

Device	Routing	Invert	Outlet Devices
#1	Primary	83.15'	4.0" Round Culvert L= 100.0' RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 83.15' / 82.10' S= 0.0105 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.09 sf

Primary OutFlow Max=0.06 cfs @ 12.22 hrs HW=83.32' (Free Discharge)
 ↴_{1=Culvert} (Barrel Controls 0.06 cfs @ 1.90 fps)

LONGFIELD ESTATES
 1200 & 1202 Hope Street - Bristol, RI
 STORMWATER MANAGEMENT
STORMWATER RUNOFF VOLUME & FLOW RATES SUMMARY

SDE Job No.: 22024
 Prepared by: SJE

Date: 6/20/2022 , Revised: 12/9/2022
 Checked by: MER

PRE-DEVELOPMENT						
Analysis Points (Subcatchment/Pond)	1-yr			2-yr		
	Rate (cfs)	Vol. (cf)	Rate (cfs)	Vol. (cf)	Rate (cfs)	Vol. (cf)
ea1	0.20	682	0.25	944	0.58	1,895
ea2	1.08	5,293	1.53	7,233	3.15	14,295
Totals	1.28	5,975	1.78	8,177	3.73	16,190

POST-DEVELOPMENT						
Analysis Points (Subcatchment/Pond)	1-yr			2-yr		
	Rate (cfs)	Vol. (cf)	Rate (cfs)	Vol. (cf)	Rate (cfs)	Vol. (cf)
pa1	0.02	126	0.16	501	0.52	1,964
pa2	0.39	6,541	0.76	8,631	2.02	15,811
Totals	0.41	6,667	0.92	9,132	2.54	17,775

Notes:

Totals are the summations of the Analysis points values (Overall combine values leaving the site).

Analysis points **ea1** & **pa1** are pre-development and post-development comparisons. (Site's runoff flow toward Hope Street)

Analysis points **ep1** & **pp1** are pre-development and post-development comparisons. (Site's runoff flow to the existing wetland)

Refer to Hydrocad® calculations for additional information.

LONGFIELD ESTATES
 1200 & 1202 Hope Street - Bristol, RI
 STORMWATER MANAGEMENT
STORMWATER RECHARGE CALCULATION WORKSHEET

SDE Job No.: 22024
 Prepared by: SJE

Date: 6/20/2022 , Revised: 12/9/2022
 Checked by: MER

Hydrologic Group	A	B	C	D	ReV (cf)
Target Runoff Depth Factor (Inches of Runoff)	0.60	0.35	0.25	0.10	
Total Increase of Impervious Areas, (sf):					
Proposed Driveway & Parking			13,626		
Proposed Buildings			10,224		
X inches of runoff, (cf)	0	0	497	0	497
Total Recharge Volume Required = 1" x F x Ai / 12 , (ac-ft.)					Total ReV (cf) = 497

Structure	Infiltration Rate (inches / hour)	Recharged Volume (cu.ft.)	ReV Volume Below Outlet Structure (cu.ft.)
Underground System-1	0.50	764	319
Total Recharge Volume Provided, (cf) = 765			319

Note:

1. System Infiltration rates are derived from the 1982 Rawls rates based on field observed soil texture (HSG C).
2. Recharged volume are calculated utilizing Simple Dynamic Method : Automated
3. Calculations are based on 1-Yr , 24-Hr storm event.

LONGFIELD ESTATES
 1200 & 1202 Hope Street - Bristol, RI
 STORMWATER MANAGEMENT
WATER QUALITY TREATMENT VOLUME CALCULATION WORKSHEET

SDE Job No.: 22024
 Prepared by: SJE

Date: 6/20/2022 , Revised: 12/9/2022
 Checked by: MER

Impervious Area	Area sf		X in. of runoff 1.0 cf		1.2 inch rainfall cf
Total Impervious Area Excluding Roof	13,626		1,136		
Building Roof Area	12,326		1,027		
1.2 inch rainfall total runoff					2,575
Total Water Quality Volume , (cf) =			2,163		2,575
System Water Quality Volume (volumes below lowest outlet device)	Impervious Area to BMP	WQv	Treatment Required (cf)	Pretreatment Provided (cf)	Extended Treatment (cf)
(FB1) Forebay-1 =	5,303 sf	442.0	25% = 111	173	
(BR1) Bio-Retention Basin-1 =			75% = 331		1,315
(FB3) Forebay-3 =	9,147 sf	762.0	25% = 191	192	
(BR2) Bio-Retention Basin-2 =	2,794 sf	233.0	75% = 572		Required = 805 Provided = 805
(BR3) Bio-Retention Basin-3 =	122 sf	10.0	100% = 10		503
(US1) Underground System-1 =	5,112 sf	426.0	100% = 426		WQv storm contained & infiltrated (426 cf)
(FB2) Forebay-2 =	2,997 sf	250.0	25% = 63	76	
(BR4) Bio-Retention Basin-4 =			75% = 187		255
Total =	25,475 sf			441	3,304
Total Water Quality Volume Available , (cf) =				3,745	

Note:

1. Total water quality volume (WQV) in the 1.2 inch rainfall event is the inflow volume flowing toward the drainage system.
2. Recharged volume are calculated utilizing Simple Dynamic Method : Automated
3. Refer to HydroCAD report for additional information.
4. Inflow is roof runoff and/or deck/patio runoff that directly flows into bio-retention basins 2&3
5. Inflow is roof runoff only ¹⁵
6. 477 sf of deck/patio area discharges into the dry swale before entering the extended detention basin.

LONGFIELD ESTATES
 1200 & 1202 Hope Street - Bristol, RI
 STORMWATER MANAGEMENT
TSS REMOVAL CALCULATION WORKSHEET

SDE Job No.: 22024
 Prepared by: SJE

Date: 6/20/2022 , Revised: 12/9/2022
 Checked by: MER

Design Point PA1 - Hope Street

A BMP	B TSS Removal Rate	C Starting TSS Load*	D Amount Removed (BxC)	E Remaining Load (C-D)
Bio-retention Basin	90.0%	1.000	0.900	0.100
	0.0%	0.100	0.000	0.100
		0.100	0.000	0.100
Total TSS Removal =			90.0%	

Design Point PA2 - Existing Wetland

A BMP	B TSS Removal Rate	C Starting TSS Load*	D Amount Removed (BxC)	E Remaining Load (C-D)
Forebay	25.0%	1.000	0.250	0.750
Bio-retention Basin	90.0%	0.750	0.675	0.075
		0.075	0.000	0.075
Total TSS Removal =			92.5%	



STATE OF RHODE ISLAND AND PROVIDENCE PLANTATIONS

Department of Environmental Management

Office of Water Resources

Onsite Wastewater Treatment System Program



Site Evaluation Form

Part A - Soil Profile Description

Application Number _____

Property Owner: 1200 HOPE STREET, L.L.C.Property Location: BRISTOL 1200 HOPE STREETDate of Test Hole: 10 DECEMBER, 2010Soil Evaluator: N. LETENDRELicense Number: D4019Weather: SUNNY 36°F 0-5 mphShaded: Yes No Time: 10:30

TH 1 Horizon	Depth	Horizon Boundaries		Soil Colors		Re-Dox Features	Ab. S. Contr.	Texture	Structure	Consistence	Soil Category
		Dist	Topo	Matrix	Re-Dox Features						
A	6	g	s	10YR 3/3				FSL	1fgr	fr	4
Bw ₁	12	g	s	10YR 3/6				SL	1fsbk	fr	3
Bw ₂	20	c	w	10YR 4/4				SL	1fsbk	fr	3
C	30	c	w	2.5Y 9/2	2.5Y 5/6	f	f	SL	omm	fr	6
Cd	52			2.5Y 4/1	10YR 5/6	c	d	FSL	omm	Vfi	9
TH 2 Horizon	Depth	Horizon Boundaries		Soil Colors		Re-Dox Features	Ab. S. Contr.	Texture	Structure	Consistence	Soil Category
		Dist	Topo	Matrix	Re-Dox Features						
A	10	c	w	10YR 3/3				ch FSL	1fgr	fr	4
Bw	16	c	w	10YR 4/6	10YR 5/8	@12	f	SL	1fsbk	fr	3
C	50			2.5Y 4/1	10YR 5/6	c	d	FSL	omm	fr	7

TH 1 Soil Class A Total Depth 52" Impervious/Limiting Layer Depth N/A (og) GW Seepage Depth N/A SHWT 24" (og)TH 2 Soil Class A Total Depth 50" Impervious/Limiting Layer Depth N/A (og) GW Seepage Depth 38" SHWT 12" (og)

Comments: _____

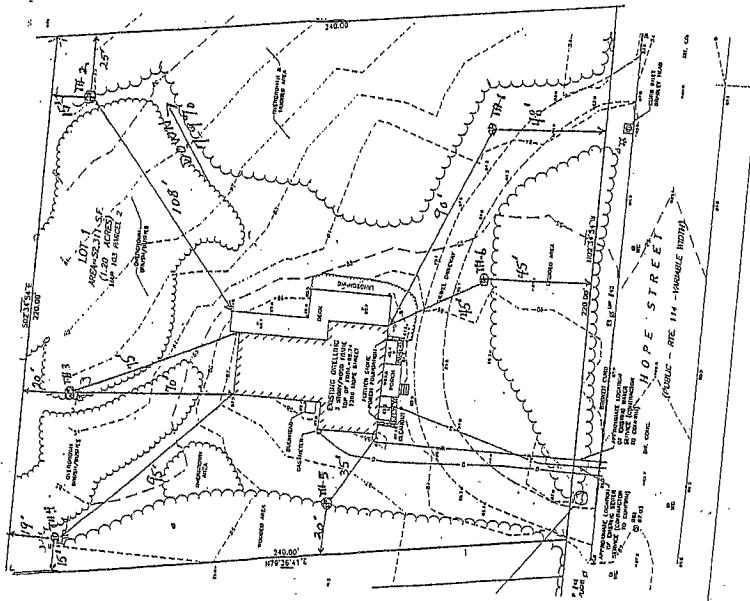
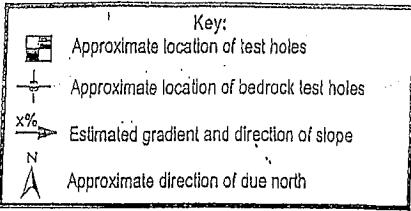
Part B

Site Evaluation – to be completed by Soil Evaluator or Class II or III Designer

Please use the area below to locate:

1. Test holes and bedrock test holes,
 2. Approximate direction of due north,
 3. Offsets from all test holes to fixed points such as street, utility pole, or other permanent, marked object.*

*** OFFSETS MUST BE SHOWN**



1. Relief and Slope: _____

2. Presence of any watercourse, wetlands or surface water bodies, within 200 feet of test holes? If yes, locate on above sketch. NO YES

3. Restrictive Layer or Bedrock within 4' below original ground within 25 feet of test hole? Provide all test hole locations & depths above. NO YES

4. Presence of existing or proposed private drinking water wells within 200 feet of test holes? If yes, locate on above sketch. NO YES

5. Public drinking water wells within 500 feet of test holes? If yes, locate on above sketch. NO YES

6. Is site within the watershed of a public drinking water reservoir or other critical area defined in Rule 38? NO YES

7. Has soil been excavated from or fill deposited on site? If yes, locate on above sketch. SITE IS PREVIOUSLY DEVELOPED NO YES

8. Site's potential for flooding or ponding: NONE SLIGHT MODERATE SEVERE

9. Landscape position: SHOULDER SLOPE

10. Vegetation: DISTURBANCE VARIETY

11. Indicate approximate location of property lines and roadways.

12. Additional comments, site constraints or additional information regarding site: _____

Certification

The undersigned hereby certifies that all information on this application and accompanying forms, submittals and sketches are true and accurate and that I have been authorized by the owner(s) to conduct these necessary field investigations and submit this request.

Part A prepared by:

Signature

License

Part B prepared by:

Signature

License #

DO NOT WRITE IN THIS SPACE

Witnessed Soil Evaluation Decision:

Concur

Inconclusive.

Disclaimer:

Unwitnessed Soil Evaluations Decision:

Accept

Inconclusive

Disclaim

Wet Season Determination required

Additional Field Review Required

Explanation:

Signature Authorized Agent

Date



STATE OF RHODE ISLAND AND PROVIDENCE PLANTATIONS

Department of Environmental Management

Office of Water Resources

Onsite Wastewater Treatment System Program



Site Evaluation Form

Part A - Soil Profile Description

Application Number _____

Property Owner: 1200 HOPE STREET, L.L.C.Property Location: BRISTOL 1200 HOPE STREETDate of Test Hole: 10 DECEMBER, 2012Soil Evaluator: N. LETENDRELicense Number: D4019Weather: SUNNY 36°F 0-5 mphShaded: Yes No Time: 10:30

TH <u>3</u> Horizon	Depth	Horizon Boundaries		Soil Colors		Re-Dox Features	Ab. S. Cont.	Texture	Structure	Consistence	Soil Category
		Dist	Topo	Matrix							
FILL	-8 to 0										
A	10	C	S	10YR $\frac{3}{4}$				FSL	1fsbk	fr	4
Bw ₁	17	g	w	10YR $\frac{4}{2}$				SL	1fsbk	fr	3
Bw ₂	22	c	s	10YR $\frac{4}{4}$	@24			SL	1fsbk	fr	3
C	26	c	w	2.5Y $\frac{4}{2}$ 2.5Y $\frac{4}{4}$	f	f	g LS	omm	fr	6	
2C	42			2.5Y $\frac{4}{1}$ 10YR $\frac{5}{6}$	c	i	d	FSL	omm	fr	7
TH <u>4</u> Horizon	Depth	Horizon Boundaries		Soil Colors		Re-Dox Features	Ab. S. Cont.	Texture	Structure	Consistence	Soil Category
		Dist	Topo	Matrix							
A _p	10	c	w	10YR $\frac{3}{3}$				FSL	1fgr	vfr	4
Bw ₁	16	g	w	10YR $\frac{4}{6}$				SL	1fsbk	fr	3
Bw ₂	21	g	w	10YR $\frac{5}{4}$	@24			SL	1fsbk	fr	3
C	36	c	i	2.5Y $\frac{6}{4}$ 10YR $\frac{5}{6}$	e	i	d	FSL	omm	fr	7
2C	57			5Y $\frac{4}{2}$ 10YR $\frac{5}{6}$	c	i	d	v g SL	6mm	fip fir	8

TH 3 Soil Class A Total Depth 50" Impervious/Limiting Layer Depth N/A (og) GW Seepage Depth N/A SHWT 24" (og)TH 4 Soil Class A Total Depth 57" Impervious/Limiting Layer Depth N/A (og) GW Seepage Depth N/A SHWT 24" (og)

Comments:

Part B

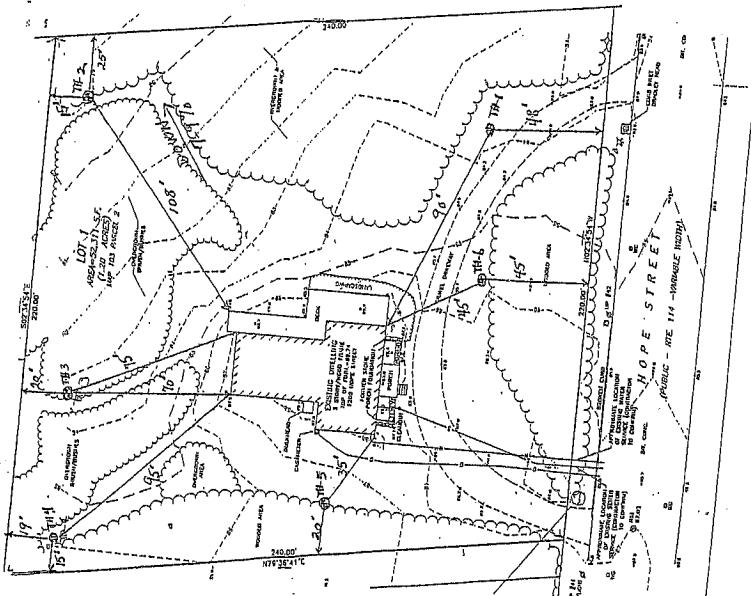
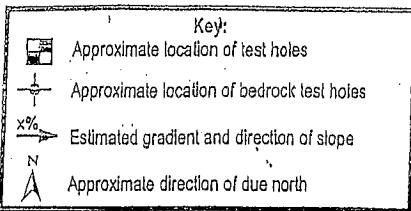
Site Evaluation – to be completed by Soil Evaluator or Class II or III Designer

Please use the area below to locate:

- Please use the area below to locate:

 1. Test holes and bedrock test holes,
 2. Approximate direction of due north,
 3. Offsets from all test holes to fixed points such as street, utility pole, or other permanent, marked object.*

* OFFSETS MUST BE SHOWN



1. Relief and Slope: _____

2. Presence of any watercourse, wetlands or surface water bodies, within 200 feet of test holes? If yes, locate on above sketch. NO YES

3. Restrictive Layer or Bedrock within 4' below original ground within 25 feet of test hole? Provide all test hole locations & depths above. NO YES

4. Presence of existing or proposed private drinking water wells within 200 feet of test holes? If yes, locate on above sketch. NO YES

5. Public drinking water wells within 500 feet of test holes? If yes, locate on above sketch. NO YES

6. Is site within the watershed of a public drinking water reservoir or other critical area defined in Rule 38? NO YES

7. Has soil been excavated from or fill deposited on site? If yes, locate on above sketch. SITE IS PREVIOUSLY DEVELOPED NO YES

8. Site's potential for flooding or ponding: NONE SLIGHT MODERATE SEVERE

9. Landscape position: SHOULDER SLOPE

10. Vegetation: DISTURBANCE VARIETY

11. Indicate approximate location of property lines and roadways.

12. Additional comments, site constraints or additional information regarding site: _____

Certification

The undersigned hereby certifies that all information on this application and accompanying forms, submittals and sketches are true and accurate and that I have been authorized by the owner(s) to conduct these necessary field investigations and submit this request.

Part A prepared by:

Signature

Licença

Part B prepared by:

Signature

License 并

DO NOT WRITE IN THIS SPACE

Witnessed Soil Evaluation Decision:

Concur

Inconclusive

Disclaimer

Unwitnessed Soil Evaluations Decision:

Accept

Inconclusive

Disclaim

Wet Season Determination required

Additional Field Review Required

Explanation:

Signature Authorized Agent

Date



STATE OF RHODE ISLAND AND PROVIDENCE PLANTATIONS
 Department of Environmental Management
 Office of Water Resources
 Onsite Wastewater Treatment System Program



Site Evaluation Form

Part A - Soil Profile Description

Application Number _____

Property Owner: 1200 HOPE STREET, L.L.C.Property Location: PROVIDENCE 1200 HOPE STREETDate of Test Hole: 10 DECEMBER, 2018Soil Evaluator: N. LETENDRE License Number: D 4019Weather: SUNNY 36°F 0-5 mph Shaded: Yes No Time: 10:30

TH 5 Horizon	Depth	Horizon Boundaries		Soil Colors		Re-Dox Features	Ab. S. Contr.	Texture	Structure	Consistence	Soil Category
		Dist	Topo	Matrix							
FILL	-30 to 0										
C	12	c	b	2.5Y 6/4 @ 0 10YR 5/8	c	1 d	FSL	omm	fr	7	
C	18			5Y 4/1 10YR 5/6	c	1 d	vgsL	omm	fr	6	
TH 6 Horizon	Depth	Horizon Boundaries		Soil Colors		Re-Dox Features	Ab. S. Contr.	Texture	Structure	Consistence	Soil Category
		Dist	Topo	Matrix							
Ao	12	c	s	10YR 3/3				FSL	fgr	Vfr	4
Bw ₁	20	g	i	10YR 4/6				vcbSL	fsbk	fr	3
Bw ₂	29	g	s	10YR 5/4				cb SL	fsbk	fr	3
C	38	c	s	2.5Y 5/3 @ 3.2 10YR 5/6	f	1 d	SL	omm	fr	6	
Cd	56			2.5Y 4/1 10YR 5/6	c	1 d	gSL	omm	fi	8	

TH 5 Soil Class A Total Depth 48" Impervious/Limiting Layer Depth N/A (og) GW Seepage Depth N/A SHWT 0 (og)TH 6 Soil Class A Total Depth 56" Impervious/Limiting Layer Depth N/A (og) GW Seepage Depth N/A SHWT 32" (og)

Comments: _____

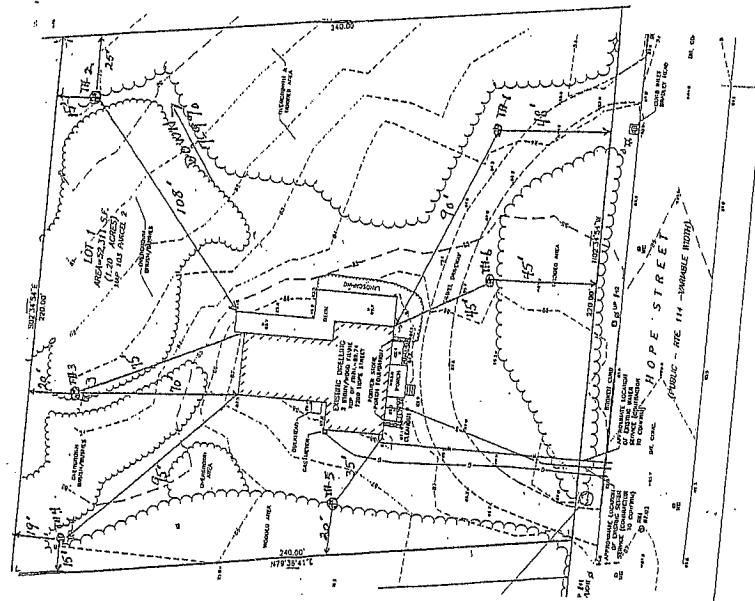
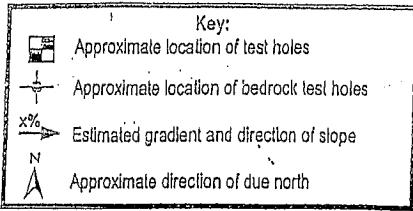
Part B

Site Evaluation – to be completed by Soil Evaluator or Class II or III Designer

Please use the area below to locate:

1. Test holes and bedrock test holes,
 2. Approximate direction of due north,
 3. Offsets from all test holes to fixed points such as street, utility pole, or other permanent, marked object.*

*** OFFSETS MUST BE SHOWN**



1. Relief and Slope: _____

2. Presence of any watercourse, wetlands or surface water bodies, within 200 feet of test holes? If yes, locate on above sketch. NO YES

3. Restrictive Layer or Bedrock within 4' below original ground within 25 feet of test hole? Provide all test hole locations & depths above. NO YES

4. Presence of existing or proposed private drinking water wells within 200 feet of test holes? If yes, locate on above sketch. NO YES

5. Public drinking water wells within 500 feet of test holes? If yes, locate on above sketch. NO YES

6. Is site within the watershed of a public drinking water reservoir or other critical area defined in Rule 38? NO YES

7. Has soil been excavated from or fill deposited on site? If yes, locate on above sketch. SITE IS PREVIOUSLY DEVELOPED NO YES

8. Site's potential for flooding or ponding: NONE SLIGHT MODERATE SEVERE

9. Landscape position: SHOULDER SLOPE

10. Vegetation: DISTURBANCE VARIETY

11. Indicate approximate location of property lines and roadways.

12. Additional comments, site constraints or additional information regarding site: _____

Certification

The undersigned hereby certifies that all information on this application and accompanying forms, submittals and sketches are true and accurate and that I have been authorized by the owner(s) to conduct these necessary field investigations and submit this request.

Part A prepared by:

Signature

License #

Part B prepared by:

Signature

License #

DO NOT WRITE IN THIS SPACE

Witnessed Soil Evaluation Decision:

Concur

Inconclusive

Disclaimer:

Unwitnessed Soil Evaluations Decision:

Accept

Inconclusive

Disclaimer

Wet Season Determination required

Additional Field Review Required

Explanation:

Signature Authorized Agent

Date

APPENDIX A: STORMWATER MANAGEMENT PLAN CHECKLIST AND LID PLANNING REPORT – STORMWATER DESIGN SUMMARY

PROJECT NAME Longfield Estates	(RIDEML USE ONLY)
TOWN Bristol	STW/WQC File #:
BRIEF PROJECT DESCRIPTION: Construction of 4 residential structures with associated improvements and renovations of an existing dwelling.	Date Received:

Stormwater Management Plan (SMP) Elements – Minimum Standards

When submitting a SMP,¹ submit **four separately bound documents**: Appendix A Checklist; Stormwater Site Planning, Analysis and Design Report with Plan Set/Drawings; Soil Erosion and Sediment Control (SESC) Plan, and Post Construction Operations and Maintenance (O&M) Plan. Please refer to [Suggestions to Promote Brevity](#).

Note: All stormwater construction projects **must create** a Stormwater Management Plan (SMP). However, not every element listed below is required per the [RIDEML Stormwater Rules](#) and the [RIPDES Construction General Permit \(CGP\)](#). This checklist will help identify the required elements to be submitted with an Application for Stormwater Construction Permit & Water Quality Certification.

PART 1. PROJECT AND SITE INFORMATION

PROJECT TYPE (Check all that apply)

<input checked="" type="checkbox"/> Residential	<input type="checkbox"/> Commercial	<input type="checkbox"/> Federal	<input type="checkbox"/> Retrofit	<input type="checkbox"/> Restoration
<input checked="" type="checkbox"/> Road	<input checked="" type="checkbox"/> Utility	<input checked="" type="checkbox"/> Fill	<input type="checkbox"/> Dredge	<input type="checkbox"/> Mine
<input type="checkbox"/> Other (specify):				

SITE INFORMATION

<input checked="" type="checkbox"/> Vicinity Map
--

INITIAL DISCHARGE LOCATION(S): The WQ_v discharges to: (You may choose more than one answer if several discharge points are associated with the project.)

<input checked="" type="checkbox"/> Groundwater	<input checked="" type="checkbox"/> Surface Water	<input type="checkbox"/> MS4
<input type="checkbox"/> GAA	<input checked="" type="checkbox"/> Isolated Wetland	<input type="checkbox"/> RIDOT
<input type="checkbox"/> GA	<input type="checkbox"/> Named Waterbody	<input type="checkbox"/> RIDOT Alteration Permit is Approved
<input checked="" type="checkbox"/> GB	<input type="checkbox"/> Unnamed Waterbody Connected to Named Waterbody	<input type="checkbox"/> Town
		<input type="checkbox"/> Other (specify):

ULTIMATE RECEIVING WATERBODY LOCATION(S): Include pertinent information that applies to both WQ_v and flow from larger storm events including overflows. Choose all that apply, and repeat table for each waterbody.

<input checked="" type="checkbox"/> Groundwater or Disconnected Wetland	<input type="checkbox"/> SRWP		
<input type="checkbox"/> Waterbody Name:	<input type="checkbox"/> Coldwater	<input type="checkbox"/> Warmwater	<input type="checkbox"/> Unassessed
<input type="checkbox"/> Waterbody ID:	<input type="checkbox"/> 4 th order stream of pond 50 acres or more		
<input type="checkbox"/> TMDL for:	<input type="checkbox"/> Watershed of flood prone river (e.g., Pocasset River)		
<input type="checkbox"/> Contributes to a priority outfall listed in the TMDL	<input type="checkbox"/> Contributes stormwater to a public beach		
<input type="checkbox"/> 303(d) list – Impairment(s) for:	<input type="checkbox"/> Contributes to shellfishing grounds		

¹ Applications for a Construction General Permit that do not require any other permits from RIDEML and will disturb less than 5 acres over the entire course of the project do not need to submit a SMP. The Appendix A checklist must still be submitted.

PROJECT HISTORY

<input type="checkbox"/> RIDEM Pre- Application Meeting	Meeting Date:	<input type="checkbox"/> Minutes Attached
<input type="checkbox"/> Municipal Master Plan Approval	Approval Date:	<input type="checkbox"/> Minutes Attached
<input type="checkbox"/> Subdivision Suitability Required	Approval #:	
<input type="checkbox"/> Previous Enforcement Action has been taken on the property	Enforcement #:	

FLOODPLAIN & FLOODWAY See [Guidance Pertaining to Floodplain and Floodways](#)

<input type="checkbox"/> Riverine 100-year floodplain: FEMA FLOODPLAIN FIRMETTE has been reviewed and the 100-year floodplain is on site	
<input type="checkbox"/> Delineated from FEMA Maps	
NOTE: Per Rule 250-RICR-150-10-8-1.1(B)(5)(d)(3), provide volumetric floodplain compensation calculations for cut and fill/displacement calculated by qualified professional	
<input type="checkbox"/> Calculated by Professional Engineer	
<input type="checkbox"/> Calculations are provided for cut vs. fill/displacement volumes proposed within the 100-year floodplain	Amount of Fill (CY): Amount of Cut (CY):
<input type="checkbox"/> Restrictions or modifications are proposed to the flow path or velocities in a floodway	
<input type="checkbox"/> Floodplain storage capacity is impacted	
<input type="checkbox"/> Project area is not within 100-year floodplain as defined by RIDEM	

CRMC JURISDICTION

<input type="checkbox"/> CRMC Assent required
<input type="checkbox"/> Property subject to a Special Area Management Plan (SAMP). If so, specify which SAMP:
<input type="checkbox"/> Sea level rise mitigation has been designed into this project

LUHPPPL IDENTIFICATION - MINIMUM STANDARD 8:

1. OFFICE OF Land Revitalization and Sustainable Materials Management (OLRSMM)

<input type="checkbox"/> Known or suspected releases of HAZARDOUS MATERIAL are present at the site (Hazardous Material is defined in Rule 1.4(A)(33) of 250-140-30-1 of the RIDEM Rules and Regulations for Investigation and Remediation of Hazardous Materials (the Remediation Regulations))	RIDEM CONTACT:
<input type="checkbox"/> Known or suspected releases of PETROLEUM PRODUCT are present at the site (Petroleum Product as defined in Rule 1.5(A)(84) of 250-140-25-1 of the RIDEM Rules and Regulations for Underground Storage Facilities Used for Regulated Substances and Hazardous Materials)	
<input type="checkbox"/> This site is identified on the RIDEM Environmental Resources Map as one of the following regulated facilities	SITE ID#:
<input type="checkbox"/> CERCLIS/Superfund (NPL)	
<input type="checkbox"/> State Hazardous Waste Site (SHWS)	
<input type="checkbox"/> Environmental Land Usage Restriction (ELUR)	
<input type="checkbox"/> Leaking Underground Storage Tank (LUST)	
<input type="checkbox"/> Closed Landfill	

Note: If any boxes in 1 above are checked, the applicant must contact the RIDEM OLRSMM Project Manager associated with the Site to determine if subsurface infiltration of stormwater is allowable for the project. Indicate if the infiltration corresponds to "Red," "Yellow" or "Green" as described in Section 3.2.8 of the RISDISM Guidance (Subsurface Contamination Guidance). Also, note and reference approval in PART 3, Minimum Standard 2: Groundwater Recharge/Infiltration.

2. PER MINIMUM STANDARD 8 of RICR 8.14.C.1-6 "LUHPPPLS," THE SITE IS/HAS:

<input type="checkbox"/> Industrial Site with RIPDES MSGP, except where No Exposure Certification exists. http://www.dem.ri.gov/programs/water/permits/ripdes/stormwater/status.php	
<input type="checkbox"/> Auto Fueling Facility (e.g., gas station)	
<input type="checkbox"/> Exterior Vehicles Service, Maintenance, or Equipment Cleaning Area	

<input type="checkbox"/>	Road Salt Storage and Loading Areas (exposed to rainwater)	
<input type="checkbox"/>	Outdoor Storage and Loading/Unloading of Hazardous Substances	
3. STORMWATER INDUSTRIAL PERMITTING		
<input type="checkbox"/>	The site is associated with existing or proposed activities that are considered Land Uses with Higher Potential Pollutant Loads (LUHPPLS) (see RICR 8.14.C)	Activities: Sector:
<input type="checkbox"/>	Construction is proposed on a site that is subject to <u>THE MULTI-SECTOR GENERAL PERMIT (MSGP) UNDER RULE 31(B)15 OF THE RIPDES REGULATIONS.</u>	MSGP permit #
<input type="checkbox"/>	Additional stormwater treatment is required by the MSGP Explain:	

REDEVELOPMENT STANDARD – MINIMUM STANDARD 6

<input checked="" type="checkbox"/>	Pre Construction Impervious Area	
<input checked="" type="checkbox"/>	Total Pre-Construction Impervious Area (TIA): 5,106 sf	
<input checked="" type="checkbox"/>	Total Site Area (TSA): 76,705 sf	
<input checked="" type="checkbox"/>	Jurisdictional Wetlands (JW): 48 sf	
<input checked="" type="checkbox"/>	Conservation Land (CL)	
<input type="checkbox"/>	Calculate the Site Size (defined as contiguous properties under same ownership)	
<input checked="" type="checkbox"/>	Site Size (SS) = (TSA) – (JW) – (CL)	
<input checked="" type="checkbox"/>	(TIA) / (SS) = 0.067	<input type="checkbox"/> (TIA) / (SS) >0.4?
<input type="checkbox"/>	YES, Redevelopment	

PART 2. LOW IMPACT DEVELOPMENT ASSESSMENT – MINIMUM STANDARD 1
(NOT REQUIRED FOR REDEVELOPMENT OR RETROFITS)
This section may be deleted if not required.

Note: A written description must be provided specifying why each method is not being used or is not applicable at the Site.
Appropriate answers may include:

- Town requires ... (state the specific local requirement)
- Meets Town's dimensional requirement of ...
- Not practical for site because ...
- Applying for waiver/variance to achieve this (pending/approved/denied)
- Applying for waiver/variance to seek relief from this (pending/approved/denied)

A) PRESERVATION OF UNDISTURBED AREAS, BUFFERS, AND FLOODPLAINS	IF NOT IMPLEMENTED, EXPLAIN HERE
<input checked="" type="checkbox"/> Sensitive resource areas and site constraints are identified (required) <input checked="" type="checkbox"/> Local development regulations have been reviewed (required) <input checked="" type="checkbox"/> All vegetated buffers and coastal and freshwater wetlands will be protected during and after construction <input type="checkbox"/> Conservation Development or another site design technique has been incorporated to protect open space and pre-development hydrology. Note: If Conservation Development has been used, check box and skip to Subpart C <input checked="" type="checkbox"/> As much natural vegetation and pre-development hydrology as possible has been maintained	

<p>B) LOCATE DEVELOPMENT IN LESS SENSITIVE AREAS AND WORK WITH THE NATURAL LANDSCAPE CONDITIONS, HYDROLOGY, AND SOILS</p> <ul style="list-style-type: none"> <input type="checkbox"/> Development sites and building envelopes have been appropriately distanced from wetlands and waterbodies <input type="checkbox"/> Development and stormwater systems have been located in areas with greatest infiltration capacity (e.g., soil groups A and B) <input type="checkbox"/> Plans show measures to prevent soil compaction in areas designated as Qualified Pervious Areas (QPA's) <input type="checkbox"/> Development sites and building envelopes have been positioned outside of floodplains <input checked="" type="checkbox"/> Site design positions buildings, roadways and parking areas in a manner that avoids impacts to surface water features <input checked="" type="checkbox"/> Development sites and building envelopes have been located to minimize impacts to steep slopes ($\geq 15\%$) <input type="checkbox"/> Other (describe): 	
<p>C) MINIMIZE CLEARING AND GRADING</p> <ul style="list-style-type: none"> <input checked="" type="checkbox"/> Site clearing has been restricted to <u>minimum area needed</u> for building footprints, development activities, construction access, and safety. <input checked="" type="checkbox"/> Site has been designed to position buildings, roadways, and parking areas in a manner that minimizes grading (cut and fill quantities) <input type="checkbox"/> Protection for stands of trees and individual trees and their root zones to be preserved has been specified, and such protection extends at least to the tree canopy drip line(s) <input type="checkbox"/> Plan notes specify that public trees removed or damaged during construction shall be replaced with equivalent 	
<p>D) REDUCE IMPERVIOUS COVER</p> <ul style="list-style-type: none"> <input type="checkbox"/> Reduced roadway widths (≤ 22 feet for ADT ≤ 400; ≤ 26 feet for ADT 400 - 2,000) <input type="checkbox"/> Reduced driveway areas (length minimized via reduced ROW width (≤ 45 ft.) and/or reduced (or absolute minimum) front yard setback; width minimized to ≤ 9 ft. wide one lane; ≤ 18 ft. wide two lanes; shared driveways; pervious surface) <input type="checkbox"/> Reduced building footprint: Explain approach: <input type="checkbox"/> Reduced sidewalk area (≤ 4 ft. wide; one side of the street; unpaved path; pervious surface) <input type="checkbox"/> Reduced cul-de-sacs (radius < 45 ft; vegetated island; alternative turn-around) <input type="checkbox"/> Reduced parking lot area: Explain approach <input type="checkbox"/> Use of pervious surfaces for driveways, sidewalks, parking areas/overflow parking areas, etc. <input type="checkbox"/> Minimized impervious surfaces (project meets or is less than maximum specified by Zoning Ordinance) <input type="checkbox"/> Other (describe): 	
<p>E) DISCONNECT IMPERVIOUS AREA</p> <ul style="list-style-type: none"> <input type="checkbox"/> Impervious surfaces have been disconnected, and runoff has been diverted to QPAs to the maximum extent possible <input type="checkbox"/> Residential street edges allow side-of-the-road drainage into vegetated open swales <input type="checkbox"/> Parking lot landscaping breaks up impervious expanse AND accepts runoff <input type="checkbox"/> Other (describe): 	
<p>F) MITIGATE RUNOFF AT THE POINT OF GENERATION</p> <ul style="list-style-type: none"> <input checked="" type="checkbox"/> Small-scale BMPs have been designated to treat runoff as close as possible to the source 	

G) PROVIDE LOW-MAINTENANCE NATIVE VEGETATION <ul style="list-style-type: none"> <input type="checkbox"/> Low-maintenance landscaping has been proposed using native species and cultivars <input type="checkbox"/> Plantings of native trees and shrubs in areas previously cleared of native vegetation are shown on site plan <input type="checkbox"/> Lawn areas have been limited/minimized, and yards have been kept undisturbed to the maximum extent practicable on residential lots 	
H) RESTORE STREAMS/WETLANDS <ul style="list-style-type: none"> <input type="checkbox"/> Historic drainage patterns have been restored by removing closed drainage systems, daylighting buried streams, and/or restoring degraded stream channels and/or wetlands <input type="checkbox"/> Removal of invasive species <input type="checkbox"/> Other 	

PART 3. SUMMARY OF REMAINING STANDARDS

GROUNDWATER RECHARGE – MINIMUM STANDARD 2		
YES	NO	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	The project has been designed to meet the groundwater recharge standard.
<input type="checkbox"/>	<input type="checkbox"/>	If “No,” the justification for groundwater recharge criterion waiver has been explained in the Narrative (e.g., threat of groundwater contamination or physical limitation), if applicable (see RICR 8.8.D);
<input type="checkbox"/>	<input type="checkbox"/>	Your waiver request has been explained in the Narrative, if applicable.
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Is this site identified as a Regulated Facility in Part 1, Minimum Standard 8: LUHPPL Identification?
<input type="checkbox"/>	<input type="checkbox"/>	If “Yes,” has approval for infiltration by the OLRSMM Site Project Manager, per Part 1, Minimum Standard 8, been requested?

TABLE 2-1: Summary of Recharge (see RISDISM Section 3.3.2)

(Add or Subtract Rows as Necessary)

Design Point	Impervious Area Treated (sq ft)	Total Rev Required (cu ft)	LID Stormwater Credits (see RISDISM Section 4.6.1)	Recharge Required by Remaining BMPs (cu ft)	Recharge Provided by BMPs (cu ft)
			Portion of Rev directed to a QPA (cu ft)		
DP-1:	23,850	497		497	764
DP-2:					
DP-3:					
DP-4:					
TOTALS:					

Notes:

1. Only BMPs listed in RISDISM Table 3-5 “List of BMPs Acceptable for Recharge” may be used to meet the recharge requirement.
 2. Recharge requirement must be satisfied for each waterbody ID.
- Indicate where the pertinent calculations and/or information for the above items are provided (i.e., name of report/document, page numbers, appendices, etc.):

See Appendix 2 of the Stormwater Management Report

WATER QUALITY – MINIMUM STANDARD 3		
YES	NO	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Does this project meet or exceed the required water quality volume WQ _v (see RICR 8.9.E-I)?
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Is the proposed final impervious cover greater than 20% of the disturbed area (see RICR 8.9.E-I)?
<input checked="" type="checkbox"/>	<input type="checkbox"/>	If “Yes,” either the Modified Curve Number Method or the Split Pervious/Impervious method in Hydro-CAD was used to calculate WQ _v ; or,
<input type="checkbox"/>	<input type="checkbox"/>	If “Yes,” either TR-55 or TR-20 was used to calculate WQ _v ; and,
<input type="checkbox"/>	<input type="checkbox"/>	If “No,” the project meets the minimum WQ _v of 0.2 watershed inches over the entire disturbed area.
<input type="checkbox"/>	<input type="checkbox"/>	Not Applicable
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Does this project meet or exceed the ability to treat required water quality flow WQ _f (see RICR 8.9.I.1-3)?
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Does this project propose an increase of impervious cover to a receiving water body with impairments? If “Yes,” please indicate below the method that was used to address the water quality requirements of no further degradation to a low-quality water.
<input type="checkbox"/>	<input checked="" type="checkbox"/>	RICR 8.36. A Pollutant Loading Analysis is needed and has been completed.
<input type="checkbox"/>	<input type="checkbox"/>	The Water Quality Guidance Document (Water Quality Goals and Pollutant Loading Analysis Guidance for Discharges to Impaired Waters) has been followed as applicable.
<input type="checkbox"/>	<input checked="" type="checkbox"/>	BMPs are proposed that are on the approved technology list . If “Yes,” please provide all required worksheets from the manufacturer.
<input type="checkbox"/>	<input type="checkbox"/>	Additional pollutant-specific requirements and/or pollutant removal efficiencies are applicable to the site as the result of a TMDL, SAMP, or other watershed-specific requirements. If “Yes,” please describe:

TABLE 3-1: Summary of Water Quality (see RICR 8.9)

Design Point and WB ID	Impervious area treated (sq ft)	Total WQ_v Required (cu ft)	LID Stormwater Credits (see RICR 8.18)	Water Quality Treatment Remaining (cu ft)	Water Quality Provided by BMPs (cu ft)
			WQ_v directed to a QPA (cu ft)		
DP-1:	25,952	2,163		2,163	3,745
DP-2:					
DP-3:					
DP-4:					
TOTALS:					

Notes:

- Only BMPs listed in RICR 8.20 and 8.25 or the Approved Technologies List of BMPs is Acceptable for Water Quality treatment.
- For each Design Point, the Water Quality Volume Standard must be met for each Waterbody ID.

<input checked="" type="checkbox"/> YES	This project has met the setback requirements for each BMP.
<input type="checkbox"/> NO	If “No,” please explain:
<input checked="" type="checkbox"/> Indicate where the pertinent calculations and/or information for the above items are provided (i.e., name of report/document, page numbers, appendices, etc.):	
See Appendix 2 of the Stormwater Management Report	

CONVEYANCE AND NATURAL CHANNEL PROTECTION (RICR 8.10) – MINIMUM STANDARD 4		
YES	NO	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Is this standard waived? If “Yes,” please indicate one or more of the reasons below:
		<input type="checkbox"/> The project directs discharge to a large river (i.e., 4th-order stream or larger. See RISDISM Appendix I for State-wide list and map of stream orders), bodies of water >50.0 acres in surface area (i.e., lakes, ponds, reservoirs), or tidal waters. <input checked="" type="checkbox"/> The project is a small facility with impervious cover of less than or equal to 1 acre. <input type="checkbox"/> The project has a post-development peak discharge rate from the facility that is less than 2 cfs for the 1-year, 24-hour Type III design storm event (prior to any attenuation). (<u>Note:</u> LID design strategies can greatly reduce the peak discharge rate).
<input type="checkbox"/>	<input type="checkbox"/>	Conveyance and natural channel protection for the site have been met. If ‘No,’ explain why:

TABLE 4-1: Summary of Channel Protection Volumes (see RICR 8.10)

Design Point	Receiving Water Body Name	Coldwater Fishery? (Y/N)	Total CPv Required (cu ft)	Total CPv Provided (cu ft)	Average Release Rate Modeled in the 1-yr storm (cfs)
DP-1:					
DP-2:					
DP-3:					
DP-4:					
TOTALS:					
<u>Note:</u> The Channel Protection Volume Standard must be met in each waterbody ID.					
<input type="checkbox"/> YES <input type="checkbox"/> NO	The CPv is released at roughly a uniform rate over a 24-hour duration (see examples of sizing calculations in Appendix D of the RISDISM).				
<input type="checkbox"/> YES <input type="checkbox"/> NO	Do additional design restrictions apply resulting from any discharge to cold-water fisheries; If “Yes,” please indicate restrictions and solutions below.				
<input type="checkbox"/> Indicate below where the pertinent calculations and/or information for the above items are provided (i.e., name of report/document, page numbers, appendices, etc.).					

OVERBANK FLOOD PROTECTION (RICR 8.11) AND OTHER POTENTIAL HIGH FLOWS – MINIMUM STANDARD 5		
YES	NO	
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Is this standard waived? If yes, please indicate one or more of the reasons below:
		<input type="checkbox"/> The project directs discharge to a large river (i.e., 4th-order stream or larger. See Appendix I for state-wide list and map of stream orders), bodies of water >50.0 acres in surface area (i.e., lakes, ponds, reservoirs), or tidal waters. <input type="checkbox"/> A Downstream Analysis (see RICR 8.11.D and E) indicates that peak discharge control would not be beneficial or would exacerbate peak flows in a downstream tributary of a particular site (e.g., through coincident peaks).
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Does the project flow to an MS4 system or subject to other stormwater requirements? If "Yes," indicate as follows:
		<input type="checkbox"/> RIDOT <input type="checkbox"/> Other (specify):
<p><u>Note:</u> The project could be approved by RIDEM but not meet RIDOT or Town standards. RIDOT's regulations indicate that post-volumes must be less than pre-volumes for the 10-yr storm at the design point entering the RIDOT system. If you have not already received approval for the discharge to an MS4, please explain below your strategy to comply with RIDEM and the MS4.</p>		
		Indicate below which model was used for your analysis. <input type="checkbox"/> TR-55 <input type="checkbox"/> TR-20 <input type="checkbox"/> HydroCAD <input type="checkbox"/> Bentley/Haestad <input type="checkbox"/> Intellisolve <input type="checkbox"/> Other (Specify):
YES	NO	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Does the drainage design demonstrate that flows from the 100-year storm event through a BMP will safely manage and convey the 100-year storm? If "No," please explain briefly below and reference where in the application further documentation can be found (i.e., name of report/document, page numbers, appendices, etc.):
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Do off-site areas contribute to the sub-watersheds and design points? If "Yes," <input type="checkbox"/> Are the areas modeled as "present condition" for both pre- and post-development analysis? <input type="checkbox"/> Are the off-site areas shown on the subwatershed maps?
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Does the drainage design confirm safe passage of the 100-year flow through the site for off-site runoff?
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Is a Downstream Analysis required (see RICR 8.11.E.1)?
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Calculate the following:
	<input checked="" type="checkbox"/>	Area of disturbance within the sub-watershed (areas): 1.76+/- acres
	<input checked="" type="checkbox"/>	Impervious cover (%): 32.6%
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Is a dam breach analysis required (earthen embankments over six (6) feet in height, or a capacity of 15 acre-feet or more, and contributes to a significant or high hazard dam)?
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Does this project meet the overbank flood protection standard?

Stormwater Management, Design, and Installation Rules (250-RICR-150-10-8)

Table 5-1 Hydraulic Analysis Summary								
Subwatershed (Design Point)	1.2" Peak Flow (cfs) **		1-yr Peak Flow (cfs)		10-yr Peak Flow (cfs)		100-yr Peak Flow (cfs)	
	Pre (cfs)	Post (cfs)	Pre (cfs)	Post (cfs)	Pre (cfs)	Post (cfs)	Pre (cfs)	Post (cfs)
A1:	0.01	0.00	0.20	0.02	0.58	0.52	1.36	1.20
A2:	0.09	0.02	1.08	0.39	3.15	2.02	7.34	6.49
TOTALS:								

** Utilize modified curve number method or split pervious /impervious method in HydroCAD.

Note: The hydraulic analysis must demonstrate no impact to each individual subwatershed DP unless each DP discharges to the same wetland or water resource.

Indicate as follows where the pertinent calculations and/or information for the items above are provided	Name of report/document, page numbers, appendices, etc.
Existing conditions analysis for each subwatershed, including curve numbers, times of concentration, runoff rates, volumes, and water surface elevations showing methodologies used and supporting calculations.	See Appendix 2 of the Stormwater Management Report
Proposed conditions analysis for each subwatershed, including curve numbers, times of concentration, runoff rates, volumes, water surface elevations, and routing showing the methodologies used and supporting calculations.	See Appendix 2 of the Stormwater Management Report
Final sizing calculations for structural stormwater BMPs, including contributing drainage area, storage, and outlet configuration.	See Appendix 2 of the Stormwater Management Report
Stage-storage, inflow and outflow hydrographs for storage facilities (e.g., detention, retention, or infiltration facilities).	See Appendix 2 of the Stormwater Management Report

Table 5-2 Summary of Best Management Practices

DP #	BMP ID	BMP Type (e.g., bioretention, tree filter)	BMP Functions					Bypass Type	Horizontal Setback Criteria are met per RICR 8.21.B.10, 8.22.D.11, and 8.35.B.4		
			Pre-Treatment (Y/N/NA)	Re _v	WQ _v	CP _v (Y/N/NA)	Overbank Flood Reduction (Y/N/NA)		Yes/No	Technical Justification (Design Report page number)	Distance Provided
pA2	US1	Underground Infiltration System		764	426			N/a	Y		
pA2	FB1	Sediment Forebay-1	173					N/a	Y		
pA2	FB2	Sediment Forebay-2	76					N/a	Y		
pA2	FB3	Sediment Forebay-3	192					N/a	Y		

Stormwater Management, Design, and Installation Rules (250-RICR-150-10-8)

Table 5-2 Summary of Best Management Practices

DP #	BMP ID	BMP Type (e.g., bioretention, tree filter)	BMP Functions				Bypass Type	Horizontal Setback Criteria are met per RICR 8.21.B.10, 8.22.D.11, and 8.35.B.4		
			Pre-Treatment (Y/N/ NA)	Re _v	WQ _v	CP _v (Y/N/ NA)		External (E) Internal (I) or NA	Yes/ No	Technical Justification (Design Report page number)
pA2	BR1	Bio-retention Basin-1			1315			N/a	Y	
pA2	BR2	Bio-retention Basin-2			805			N/a	Y	
pA1	BR3	Bio-retention Basin-3			503			N/a	Y	
pA1	BR4	Bio-retention Basin-4			255			N/a	Y	
TOTALS:										

Table 5.3 Summary of Soils to Evaluate Each BMP

DP #	BMP ID	BMP Type (e.g., bioretention, tree filter)	Soils Analysis for Each BMP						Exfiltration Rate Applied (in/hr)	
			Test Pit ID# and Ground Elevation		SHWT Elevation (ft)	Bottom of Practice Elevation* (ft)	Separation Distance Provided (ft)	Hydrologic Soil Group (A, B, C, D)		
			Primary	Secondary						
pA2	US1	Underground Infiltration System	TH-3	TH-4	84.0	82.0	2'	C	.27	
pA2	FB1	Sediment Forebay-1	TH-3	TH-4	85.5	85.0	Lined	C	N/a	
pA2	FB2	Sediment Forebay-2	TH-2	TH-3	81.0	83.0	Lined	C	N/a	
pA2	FB3	Sediment Forebay-3	TH-1	TH-6	80.5	80.3	Lined	c	N/a	
pA2	BR1	Bio-retention Basin-1	TH-3	TH-4	85.5	85.0	Lined	C	N/a	
pA2	BR2	Bio-retention Basin-2	TH-2	TH-1	81.0	80.0	Lined	C	N/a	
pA1	BR3	Bio-retention Basin-3	TH-1	TH-6	84.4	82.7	Lined	C	N/a	
pA1	BR4	Bio-retention Basin-4	TH-2	TH-3	81.0	83.0	2'	C	N/a	
pA2	B1	Dry Extended Detention Basin	TH-2	TH-3	80.0	80.0	0'	C	N/a	
TOTALS:										

* For underground infiltration systems (UICs) bottom equals bottom of stone, for surface infiltration basins bottom equals bottom of basin, for filters bottom equals interface of storage and top of filter layer

LAND USES WITH HIGHER POTENTIAL POLLUTANTS LOADS (LUHPPLs) – MINIMUM STANDARD 8									
YES	NO	N/A							
<input type="checkbox"/>	<input type="checkbox"/>	<input checked="" type="checkbox"/>	Describe any LUHPPLs identified in Part 1, Minimum Standard 8, Section 2. If not applicable, continue to Minimum Standard 9.						
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Are these activities already covered under an MSGP? If “No,” please explain if you have applied for an MSGP or intend to do so?						
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	List the specific BMPs that are proposed for this project that receive stormwater from LUHPPL drainage areas. These BMP types must be listed in RISDISM Table 3-3, “Acceptable BMPs for Use at LUHPPLs.” Please list BMPs:						
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Additional BMPs, or additional pretreatment BMP’s if any, that meet RIPDES MSGP requirements; Please list BMPs:						

Stormwater Management, Design, and Installation Rules (250-RICR-150-10-8)

			Indicate below where the pertinent calculations and/or information for the above items are provided (i.e., name of report/document, page numbers, appendices, etc.).
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ILLICIT DISCHARGES – MINIMUM STANDARD 9

Illicit discharges are defined as unpermitted discharges to Waters of the State that do not consist entirely of stormwater or uncontaminated groundwater, except for certain discharges identified in the RIPDES Phase II Stormwater General Permit.

YES	NO	N/A	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Have you checked for illicit discharges?
<input type="checkbox"/>	<input checked="" type="checkbox"/>	<input type="checkbox"/>	Have any been found and/or corrected? If "Yes," please identify.
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Does your report explain preventative measures that keep non-stormwater discharges out of the Waters of the State (during and after construction)?

SOIL EROSION AND SEDIMENT CONTROL (SESC) – MINIMUM STANDARD 10

YES	NO	N/A	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Have you included a Soil Erosion and Sediment Control Plan Set and/or Complete Construction Plan Set?
<input checked="" type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	Have you provided a separately-bound document based upon the SESC Template ? If yes, proceed to Minimum Standard 11 (the following items can be assumed to be addressed).
If "No," include a document with your submittal that addresses the following elements of an SESC Plan:			
<input type="checkbox"/> Soil Erosion and Sediment Control Plan Project Narrative, including a description of how the fifteen (15) Performance Criteria have been met: <input type="checkbox"/> Provide Natural Buffers and Maintain Existing Vegetation <input type="checkbox"/> Minimize Area of Disturbance <input type="checkbox"/> Minimize the Disturbance of Steep Slopes <input type="checkbox"/> Preserve Topsoil <input type="checkbox"/> Stabilize Soils <input type="checkbox"/> Protect Storm Drain Inlets <input type="checkbox"/> Protect Storm Drain Outlets <input type="checkbox"/> Establish Temporary Controls for the Protection of Post-Construction Stormwater Control Measures <input type="checkbox"/> Establish Perimeter Controls and Sediment Barriers <input type="checkbox"/> Divert or Manage Run-On from Up-Gradient Areas <input type="checkbox"/> Properly Design Constructed Stormwater Conveyance Channels <input type="checkbox"/> Retain Sediment On-Site <input type="checkbox"/> Control Temporary Increases in Stormwater Velocity, Volume, and Peak Flows <input type="checkbox"/> Apply Construction Activity Pollution Prevention Control Measures <input type="checkbox"/> Install, Inspect, and Maintain Control Measures and Take Corrective Actions <input type="checkbox"/> Qualified SESC Plan Preparer's Information and Certification <input type="checkbox"/> Operator's Information and Certification; if not known at the time of application, the Operator must certify the SESC Plan upon selection and prior to initiating site activities <input type="checkbox"/> Description of Control Measures, such as Temporary Sediment Trapping and Conveyance Practices, including design calculations and supporting documentation, as required			

STORMWATER MANAGEMENT SYSTEM OPERATION, MAINTENANCE, AND POLLUTION PREVENTION PLAN – MINIMUM STANDARDS 7 AND 9

Operation and Maintenance Section

YES	NO	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Have you minimized all sources of pollutant contact with stormwater runoff, to the maximum extent practicable?

Stormwater Management, Design, and Installation Rules (250-RICR-150-10-8)

<input checked="" type="checkbox"/>	<input type="checkbox"/>	Have you provided a separately-bound Operation and Maintenance Plan for the site and for all of the BMPs, and does it address each element of RICR 8.17 and RISDISM Appendix C and E?
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Lawn, Garden, and Landscape Management meet the requirements of RISDISM Section G.7? If "No," why not?
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Is the property owner or homeowner's association responsible for the stormwater maintenance of all BMP's? If "No," you must provide a legally binding and enforceable maintenance agreement (see RISDISM Appendix E, page 26) that identifies the entity that will be responsible for maintenance of the stormwater. Indicate where this agreement can be found in your report (i.e., name of report/document, page numbers, appendices, etc.).
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Do you anticipate that you will need legal agreements related to the stormwater structures? (e.g. off-site easements, deed restrictions, covenants, or ELUR per the Remediation Regulations). If "Yes," have you obtained them? Or please explain your plan to obtain them:
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Is stormwater being directed from public areas to private property? If "Yes," note the following: <u>Note:</u> This is not allowed unless a funding mechanism is in place to provide the finances for the long-term maintenance of the BMP and drainage, or a funding mechanism is demonstrated that can guarantee the long-term maintenance of a stormwater BMP by an individual homeowner.

Pollution Prevention Section

<input type="checkbox"/>	<input checked="" type="checkbox"/>	Designated snow stockpile locations?
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Trash racks to prevent floatables, trash, and debris from discharging to Waters of the State?
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Asphalt-only based sealants?
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Pet waste stations? (<u>Note:</u> If a receiving water has a bacterial impairment, and the project involves housing units, then this could be an important part of your pollution prevention plan).
<input type="checkbox"/>	<input checked="" type="checkbox"/>	Regular sweeping? Please describe:
<input type="checkbox"/>	<input checked="" type="checkbox"/>	De-icing specifications, in accordance with RISDISM Appendix G. (NOTE: If the groundwater is GAA, or this area contributes to a drinking water supply, then this could be an important part of your pollution prevention plan).
<input type="checkbox"/>	<input checked="" type="checkbox"/>	A prohibition of phosphate-based fertilizers? (<u>Note:</u> If the site discharges to a phosphorus impaired waterbody, then this could be an important part of your pollution prevention plan).

PART 4. SUBWATERSHED MAPPING AND SITE-PLAN DETAILS

Existing and Proposed Subwatershed Mapping (REQUIRED)

YES	NO	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Existing and proposed drainage area delineations
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Locations of all streams and drainage swales
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Drainage flow paths, mapped according to the DEM <i>Guidance for Preparation of Drainage Area Maps</i> (included in RISDISM Appendix K)
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Complete drainage area boundaries; include off-site areas in both mapping and analyses, as applicable
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Logs of borings and/or test pit investigations along with supporting soils/geotechnical report
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Mapped seasonal high-water-table test pit locations
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Mapped locations of the site-specific borings and/or test pits and soils information from the test pits at the locations of the BMPs
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Mapped locations of the BMPs, with the BMPs consistently identified on the Site Construction Plans
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Mapped bedrock outcrops adjacent to any infiltration BMP
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Soils were logged by a:
	<input checked="" type="checkbox"/>	DEM-licensed Class IV soil evaluator

Stormwater Management, Design, and Installation Rules (250-RICR-150-10-8)

	Name: N. Letendre
<input type="checkbox"/>	RI-registered P.E. Name:

Subwatershed and Impervious Area Summary				
Subwatershed (area to each design point)	First Receiving Water ID or MS4	Area Disturbed (units)	Existing Impervious (units)	Proposed Impervious (units)
DP-1:				
DP-2:				
DP-3:				
DP-4:				
TOTALS:				

Stormwater Management, Design, and Installation Rules (250-RICR-150-10-8)

Site Construction Plans (Indicate that the following applicable specifications are provided)		
YES	NO	
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Existing and proposed plans (scale not greater than 1" = 40') with North arrow
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Existing and proposed site topography (with 1 or 2-foot contours); 10-foot contours accepted for off-site areas
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Boundaries of existing predominant vegetation and proposed limits of clearing
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Site Location clarification
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Location and field-verified boundaries of resource protection areas such as: <ul style="list-style-type: none"> ▶ freshwater and coastal wetlands, including lakes and ponds ▶ coastal shoreline features Perennial and intermittent streams, in addition to Areas Subject to Storm Flowage (ASSFs)
<input checked="" type="checkbox"/>	<input type="checkbox"/>	All required setbacks (e.g., buffers, water-supply wells, septic systems)
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Representative cross-section and profile drawings, and notes and details of structural stormwater management practices and conveyances (i.e., storm drains, open channels, swales, etc.), which include: <ul style="list-style-type: none"> ▶ Location and size of the stormwater treatment practices (type of practice, depth, area). Stormwater treatment practices (BMPs) must have labels that correspond to RISDISM Table 5-2; ▶ Design water surface elevations (applicable storms); ▶ Structural details of outlet structures, embankments, spillways, stilling basins, grade-control structures, conveyance channels, etc.; ▶ Existing and proposed structural elevations (e.g., invert of pipes, manholes, etc.); ▶ Location of floodplain and, if applicable, floodway limits and relationship of site to upstream and downstream properties or drainage that could be affected by work in the floodplain; ▶ Planting plans for structural stormwater BMPs, including species, size, planting methods, and maintenance requirements of proposed planting
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Logs of borings and/or test pit investigations along with supporting soils/geotechnical report and corresponding water tables
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Mapping of any OLRSMM-approved remedial actions/systems (including ELURs)
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Location of existing and proposed roads, buildings, and other structures including limits of disturbance; <ul style="list-style-type: none"> ▶ Existing and proposed utilities (e.g., water, sewer, gas, electric) and easements; ▶ Location of existing and proposed conveyance systems, such as grass channels, swales, and storm drains, and location(s) of final discharge point(s) (wetland, waterbody, etc.); ▶ Cross sections of roadways, with edge details such as curbs and sidewalks; ▶ Location and dimensions of channel modifications, such as bridge or culvert crossings
<input checked="" type="checkbox"/>	<input type="checkbox"/>	Locations, cross sections, and profiles of all stream or wetland crossings and their method of stabilization

MOUNDSOLV
GROUNDWATER MOUNDING ANALYSIS
FOR A SLOPING WATER-TABLE AQUIFER
ZLOTNIK ET AL. (2017) SOLUTION

Solution Method

**Zlotnik et al. (2017) transient solution for a rectangular source
 (linearization method 1)**

Site Description

Aquifer Data

Property	Value
Horizontal hydraulic conductivity, K (ft/d)	2.5
Specific yield, S_y	0.2
Initial saturated thickness, h_0 (ft)	10
Maximum allowable water-table rise, σ (ft)	2
Dip, i (ft/ft)	0.05
Slope rotation from x axis, γ (°)	0

Recharge Sources

Property	Source 1
X coordinate at center, X (ft)	0
Y coordinate at center, Y (ft)	0
Dimension along x^* axis, L (ft)	21.5
Dimension along y^* axis, W (ft)	33.25
Rotation from slope direction, ϕ (°)	0
Recharge rate, Q (ft ³ /d)	357.4375
Infiltration rate, q (ft/d)	0.5

Monitoring Points

Elapsed Time, $t = 1$ d

Name	x (ft)	y (ft)	s (ft)	h (ft)	z (ft)

Source 1 0	0	1.561	11.56 0
--------------	---	-------	-----------

Time Series Data

Time (d)	Source 1	
	s (ft)	h (ft)
0	0	10
0.002916	0.00729	10.01
0.006561	0.0164	10.02
0.01112	0.02779	10.03
0.01681	0.04203	10.04
0.02393	0.05983	10.06
0.03283	0.08207	10.08
0.04395	0.1099	10.11
0.05786	0.1445	10.14
0.07524	0.1876	10.19
0.09696	0.2408	10.24
0.1241	0.3058	10.31
0.1581	0.3843	10.38
0.2005	0.4776	10.48
0.2535	0.5868	10.59
0.3198	0.7122	10.71
0.4027	0.8537	10.85
0.5063	1.011	11.01
0.6358	1.182	11.18
0.7977	1.366	11.37
1	1.561	11.56

Profile Data

**Profile Along X* Axis for
Source 1 at Elapsed Time, t =
1 d**

x* (ft) s (ft) h (ft) z (ft)

-34	0.05054	8.351	-1.7
-32.64	0.06272	8.431	-1.632
-31.28	0.07742	8.513	-1.564
-29.92	0.09508	8.599	-1.496
-28.56	0.1162	8.688	-1.428
-27.2	0.1413	8.781	-1.36
-25.84	0.1709	8.879	-1.292
-24.48	0.2058	8.982	-1.224
-23.12	0.2466	9.091	-1.156
-21.76	0.294	9.206	-1.088
-20.4	0.349	9.329	-1.02
-19.04	0.4124	9.46	-0.952
-17.68	0.4851	9.601	-0.884
-16.32	0.5681	9.752	-0.816
-14.96	0.6624	9.914	-0.748
-13.6	0.7688	10.09	-0.68
-12.24	0.8886	10.28	-0.612
-10.88	1.023	10.48	-0.544
-9.52	1.156	10.68	-0.476
-8.16	1.27	10.86	-0.408
-6.8	1.363	11.02	-0.34
-5.44	1.438	11.17	-0.272
-4.08	1.495	11.29	-0.204
-2.72	1.534	11.4	-0.136
-1.36	1.556	11.49	-0.068
0	1.561	11.56	0
1.36	1.55	11.62	0.068
2.72	1.522	11.66	0.136
4.08	1.478	11.68	0.204
5.44	1.416	11.69	0.272
6.8	1.337	11.68	0.34
8.16	1.24	11.65	0.408

9.52	1.123	11.6	0.476
10.88	0.9873	11.53	0.544
12.24	0.8526	11.46	0.612
13.6	0.733	11.41	0.68
14.96	0.6275	11.38	0.748
16.32	0.5349	11.35	0.816
17.68	0.4539	11.34	0.884
19.04	0.3834	11.34	0.952
20.4	0.3224	11.34	1.02
21.76	0.2699	11.36	1.088
23.12	0.2249	11.38	1.156
24.48	0.1865	11.41	1.224
25.84	0.1539	11.45	1.292
27.2	0.1264	11.49	1.36
28.56	0.1033	11.53	1.428
29.92	0.08403	11.58	1.496
31.28	0.06799	11.63	1.564
32.64	0.05472	11.69	1.632
34	0.04382	11.74	1.7

The axes of Source 1 (x^* , y^*) are rotated 0° from the axes of mapping coordinate system (x , y)

**Profile Along Y* Axis for
Source 1 at Elapsed Time, t
= 1 d**

y* (ft)	s (ft)	h (ft)	z (ft)
-34	0.08893	10.09	0
-32.64	0.1089	10.11	0
-31.28	0.1327	10.13	0
-29.92	0.161	10.16	0
-28.56	0.1946	10.19	0
-27.2	0.2343	10.23	0
-25.84	0.2809	10.28	0

-24.48	0.3355	10.34	0
-23.12	0.3994	10.4	0
-21.76	0.4736	10.47	0
-20.4	0.5598	10.56	0
-19.04	0.6594	10.66	0
-17.68	0.7741	10.77	0
-16.32	0.9045	10.9	0
-14.96	1.028	11.03	0
-13.6	1.134	11.13	0
-12.24	1.225	11.23	0
-10.88	1.303	11.3	0
-9.52	1.368	11.37	0
-8.16	1.422	11.42	0
-6.8	1.467	11.47	0
-5.44	1.502	11.5	0
-4.08	1.528	11.53	0
-2.72	1.547	11.55	0
-1.36	1.558	11.56	0
0	1.561	11.56	0
1.36	1.558	11.56	0
2.72	1.547	11.55	0
4.08	1.528	11.53	0
5.44	1.502	11.5	0
6.8	1.467	11.47	0
8.16	1.422	11.42	0
9.52	1.368	11.37	0
10.88	1.303	11.3	0
12.24	1.225	11.23	0
13.6	1.134	11.13	0
14.96	1.028	11.03	0
16.32	0.9045	10.9	0
17.68	0.7741	10.77	0

19.04	0.6594	10.66	0
20.4	0.5598	10.56	0
21.76	0.4736	10.47	0
23.12	0.3994	10.4	0
24.48	0.3355	10.34	0
25.84	0.2809	10.28	0
27.2	0.2343	10.23	0
28.56	0.1946	10.19	0
29.92	0.161	10.16	0
31.28	0.1327	10.13	0
32.64	0.1089	10.11	0
34	0.08893	10.09	0

The axes of Source 1 (x^* , y^*) are rotated 0° from the axes of mapping coordinate system (x , y)

Sensitivity Data

Source 1, $x=0$ ft, $y=0$ ft

Parameter	Water-Table Rise (ft)			
	Multiplier	K	Sy	h_o
0.8	1.71	1.765	1.71	1.561
0.82	1.694	1.742	1.694	1.561
0.84	1.678	1.719	1.678	1.561
0.86	1.662	1.698	1.662	1.561
0.88	1.647	1.677	1.647	1.561
0.9	1.632	1.656	1.632	1.561
0.92	1.617	1.636	1.617	1.561
0.94	1.603	1.617	1.603	1.561
0.96	1.589	1.598	1.589	1.561
0.98	1.575	1.579	1.575	1.561
1	1.561	1.561	1.561	1.561
1.02	1.548	1.544	1.548	1.561
1.04	1.535	1.527	1.535	1.561

1.06	1.522	1.51	1.522	1.561
1.08	1.51	1.494	1.51	1.561
1.1	1.497	1.478	1.497	1.561
1.12	1.485	1.462	1.485	1.561
1.14	1.473	1.447	1.473	1.561
1.16	1.462	1.432	1.462	1.561
1.18	1.45	1.417	1.45	1.561
1.2	1.439	1.403	1.439	1.561

Notation

h is water-table elevation above datum¹

h_0 is aquifer saturated thickness prior to mounding

i is dip of aquifer

K is horizontal hydraulic conductivity

L is dimension of recharge source parallel to x^* axis

q is infiltration rate ($= Q / L \cdot W$)

Q is recharge rate

s is water-table rise above static water table

S_y is specific yield

t is time since start of recharge

t_0 is time when recharge stops

W is dimension of recharge source parallel to y^* axis

x, y are mapping Cartesian coordinate axes

x^*, y^* are recharge source Cartesian coordinate axes

z is elevation above datum¹

γ is angle between x axis and dip direction

ϕ is angle between dip direction and x^* axis of recharge source

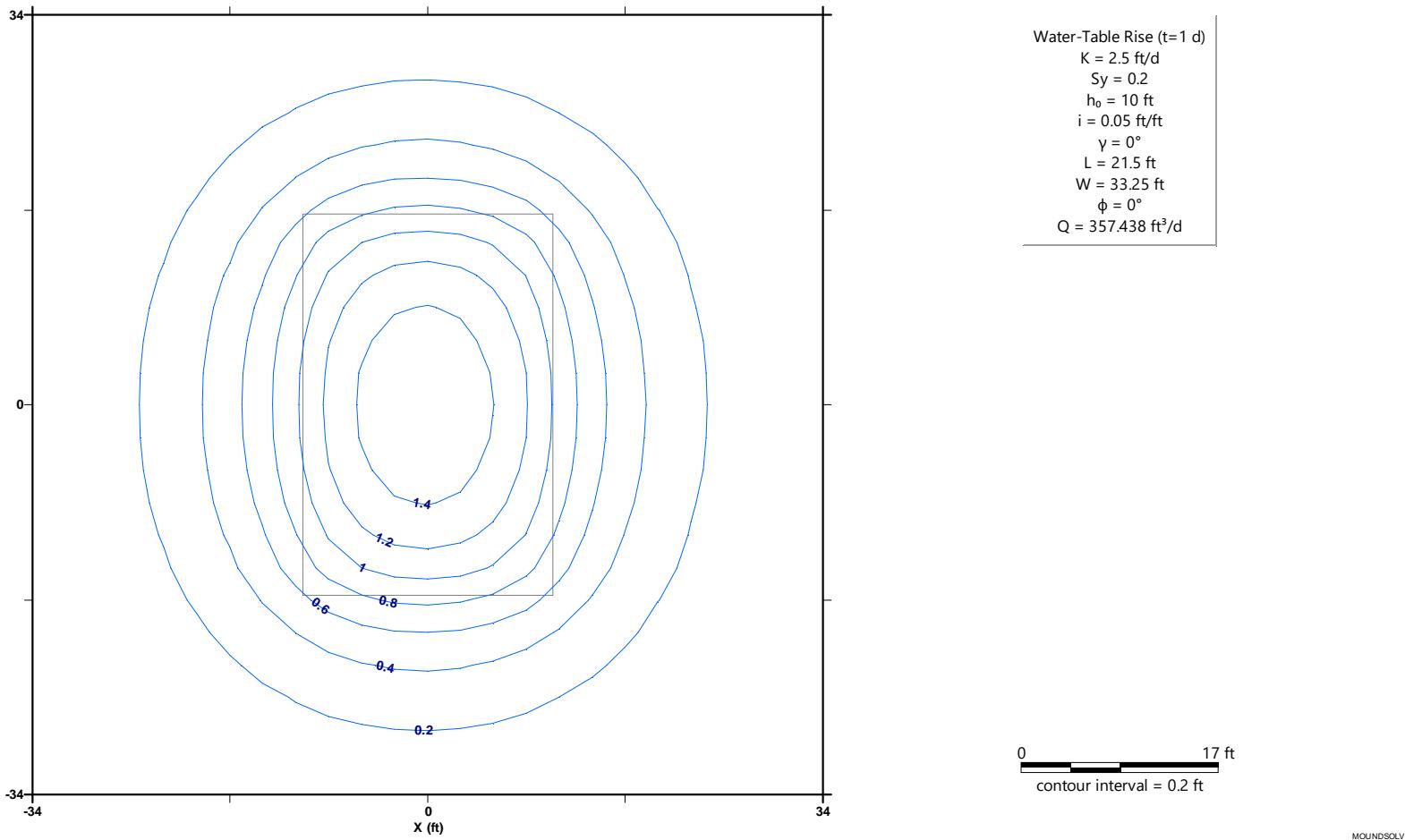
σ is maximum acceptable water-table rise

¹Elevation datum is the base of aquifer beneath the center of primary recharge source

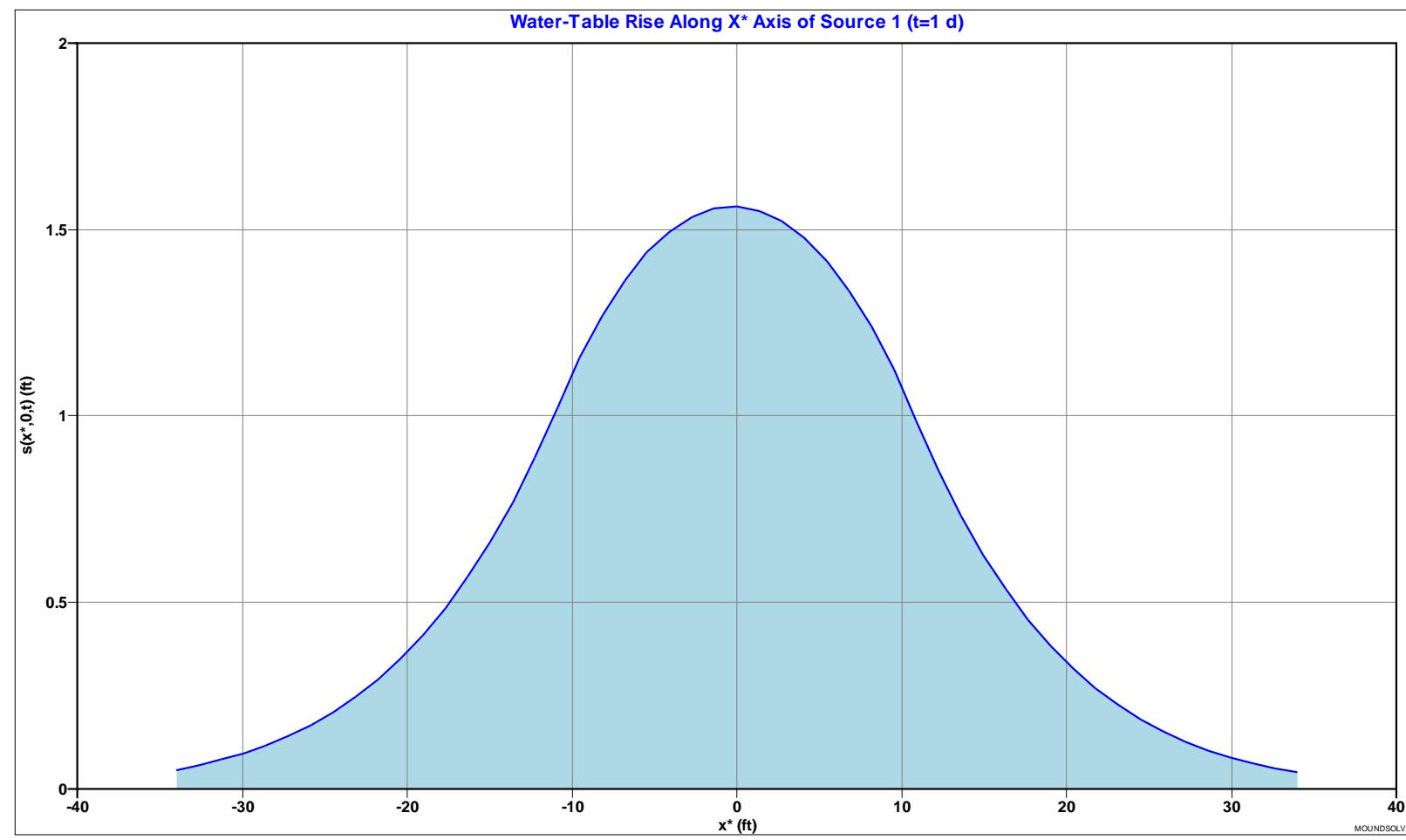
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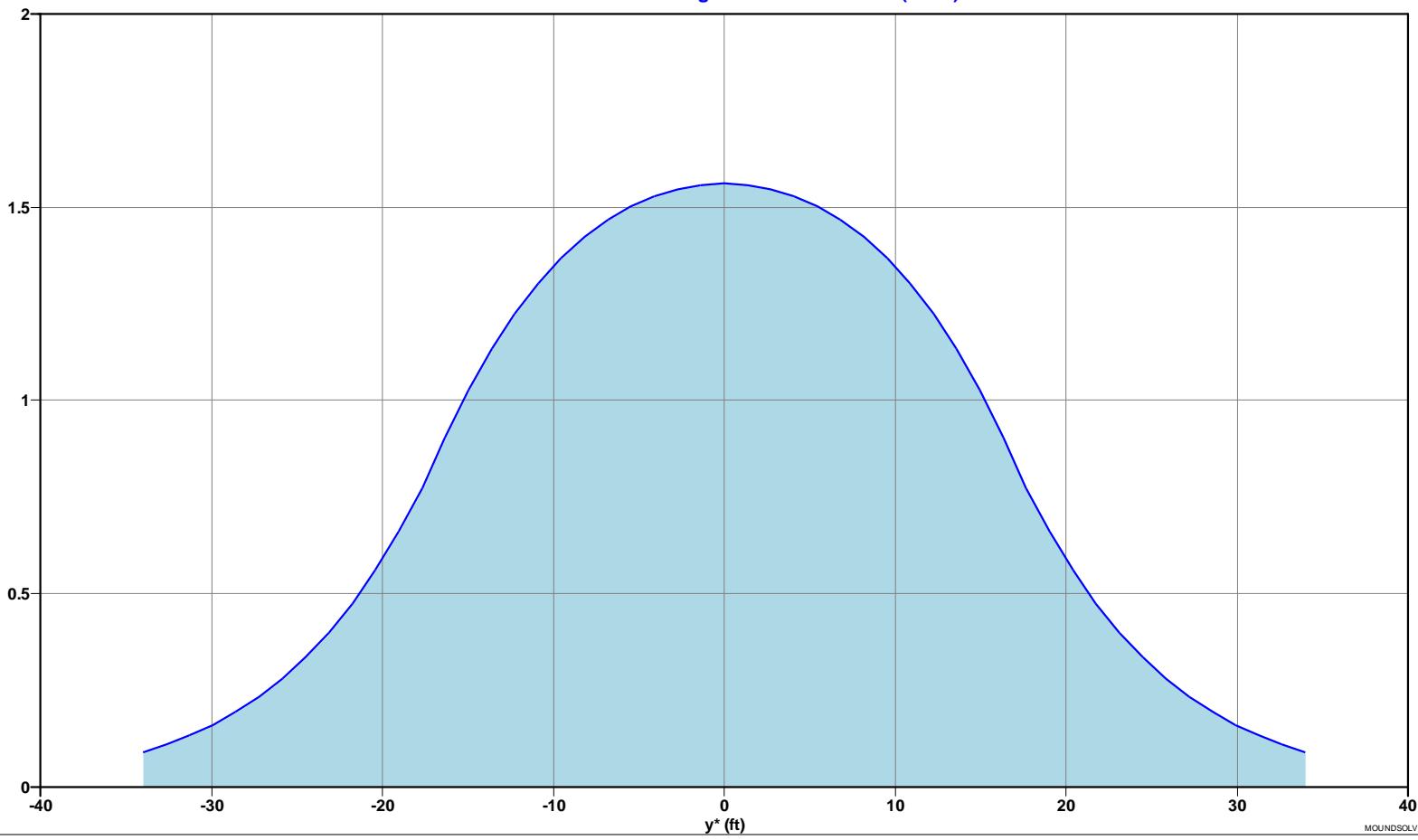
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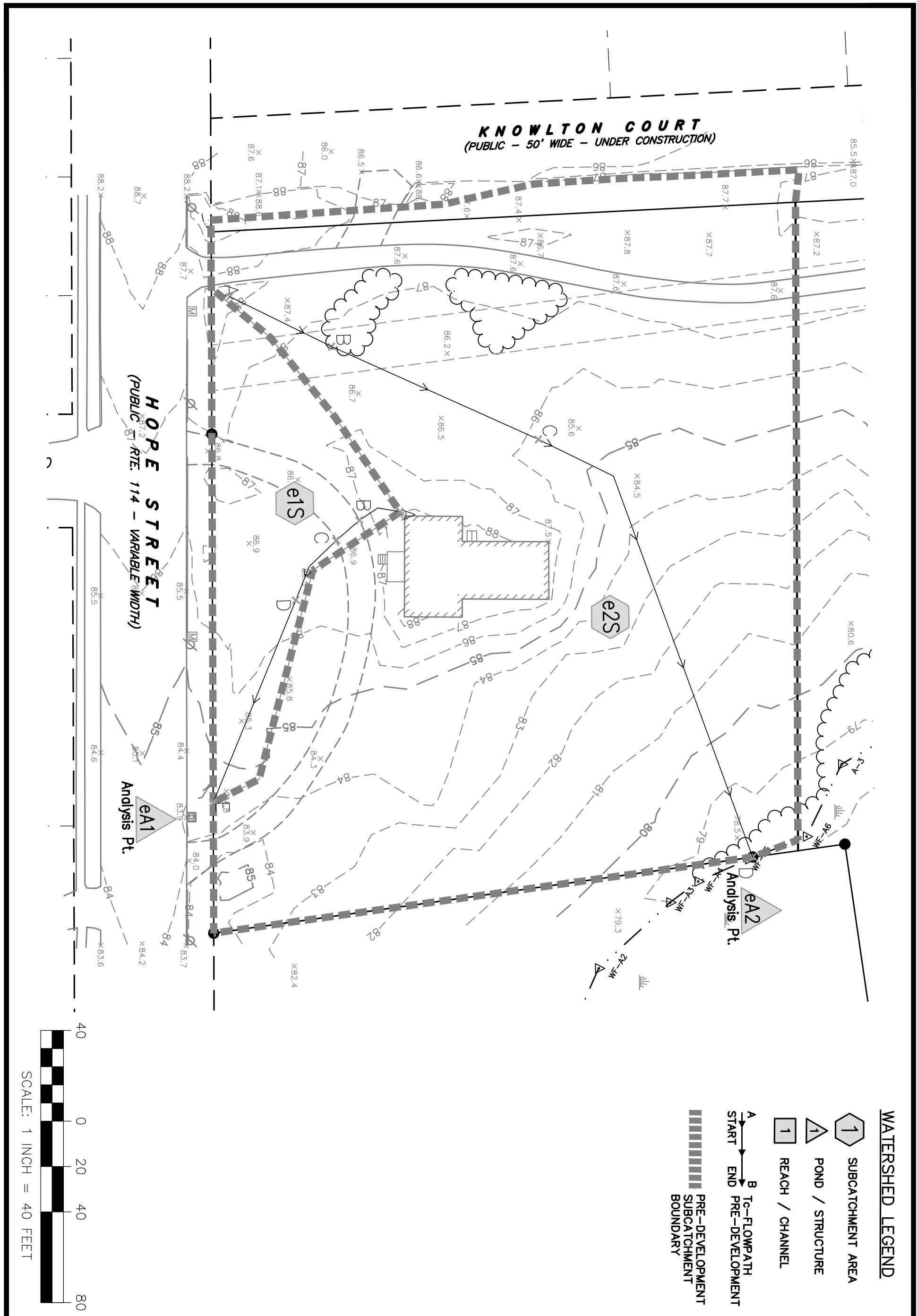


Water-Table Rise Along X* Axis of Source 1 (t=1 d)



Water-Table Rise Along Y* Axis of Source 1 (t=1 d)





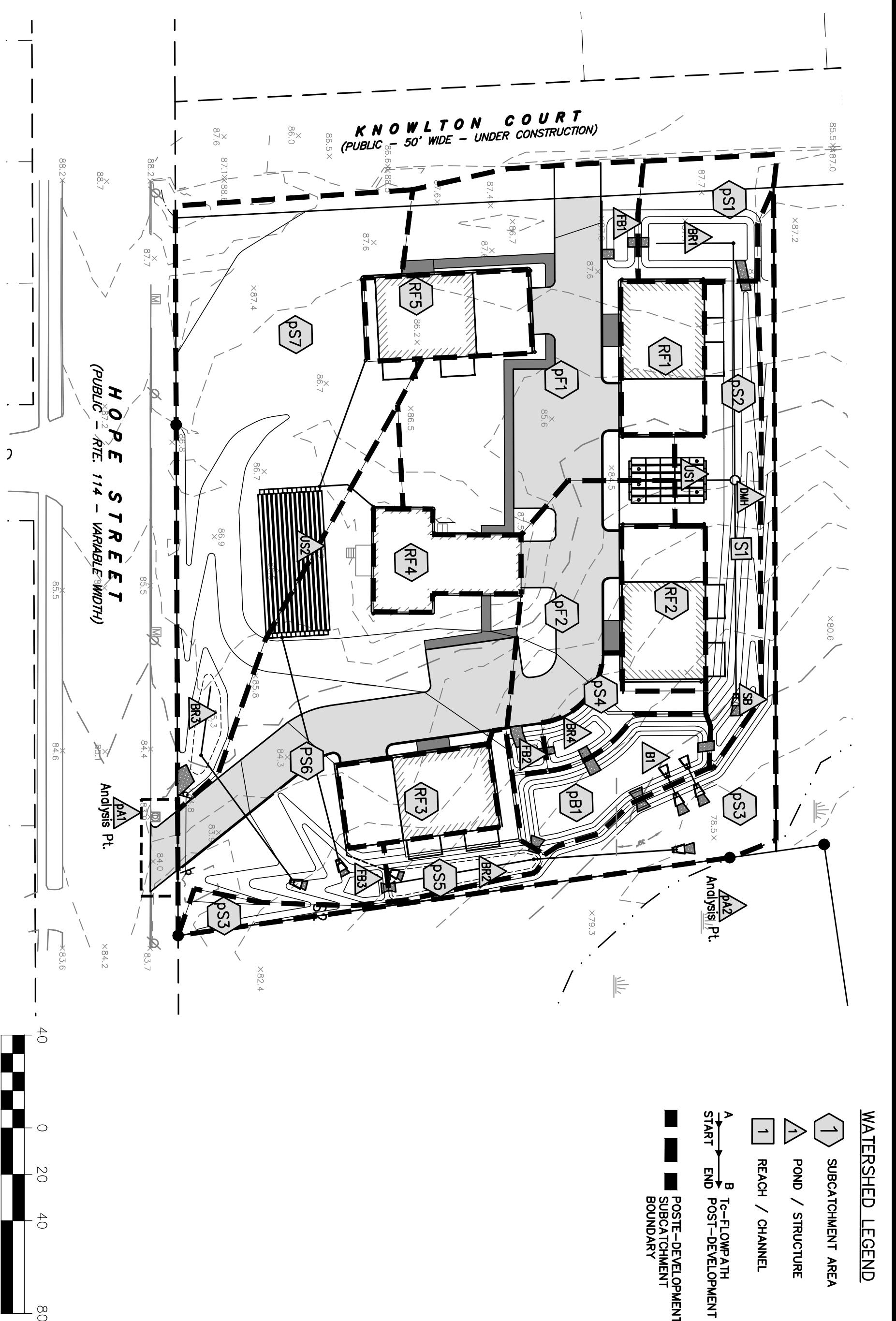
PRE DEVELOPMENT WATERSHED PLAN

1200 & 1202 HOPE STREET
ASSESSOR'S MAP 103, LOT 2 & 14
BRISTOL, RHODE ISLAND

DATE: JUNE 15, 2022	REV. DATE:
PROJ.#: 22024	SCALE : 1" = 40'
DRAWN BY: SJE	CHECK BY: MER
ISSUED FOR: PERMITTING	PREPARED FOR: EXPRESS REALTY TRUST, LLC



207 HIGH POINT AVE.
UNIT 6
Portsmouth, RI 02871
T: 401-354-2050
F: 401-369-9775
WWW.SDE-LDEC.COM



POST DEVELOPMENT WATERSHED PLAN

1200 & 1202 HOPE STREET
ASSESSOR'S MAP 103, LOT 2 & 14
BRISTOL, RHODE ISLAND



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