

Date: 8/10/2023 - 12:35 PM

Design ID: 316358799911

Estimated Price: \$40,805.29

\*Today's estimated price. Future pricing may go up or down. Tax, labor, and delivery not included.

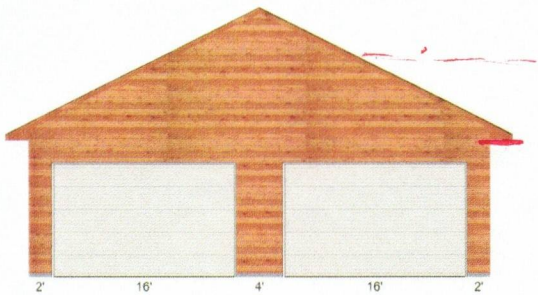
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## Dimensions

### Wall Configurations

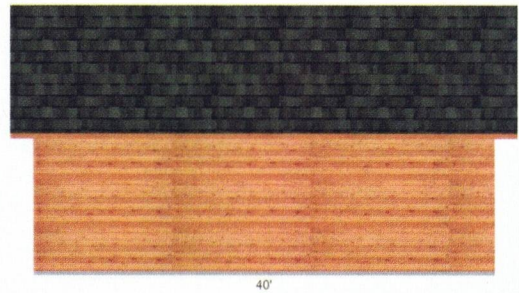
\*Some items like wainscot, gutter, gable accents, are not displayed if selected.



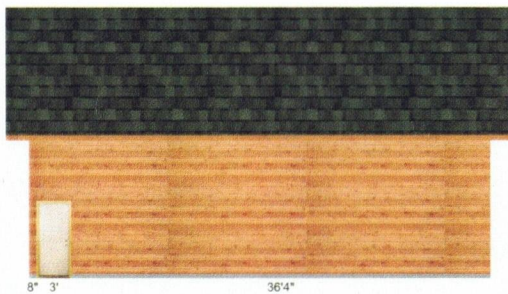
**ENDWALL B**

Ideal Door&reg; Commercial 16' x 10' White Insulated

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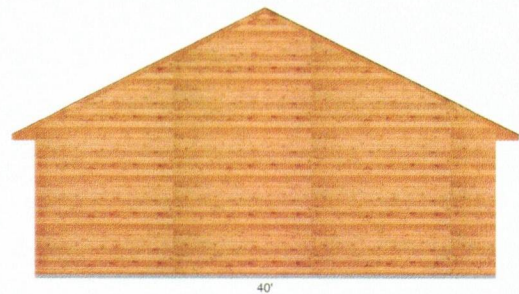


**SIDEWALL D**



**SIDEWALL C**

Mastercraft&reg; 36W x 80H Primed Steel 6-Panel



**ENDWALL A**

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**How to recall and purchase your design at home:**



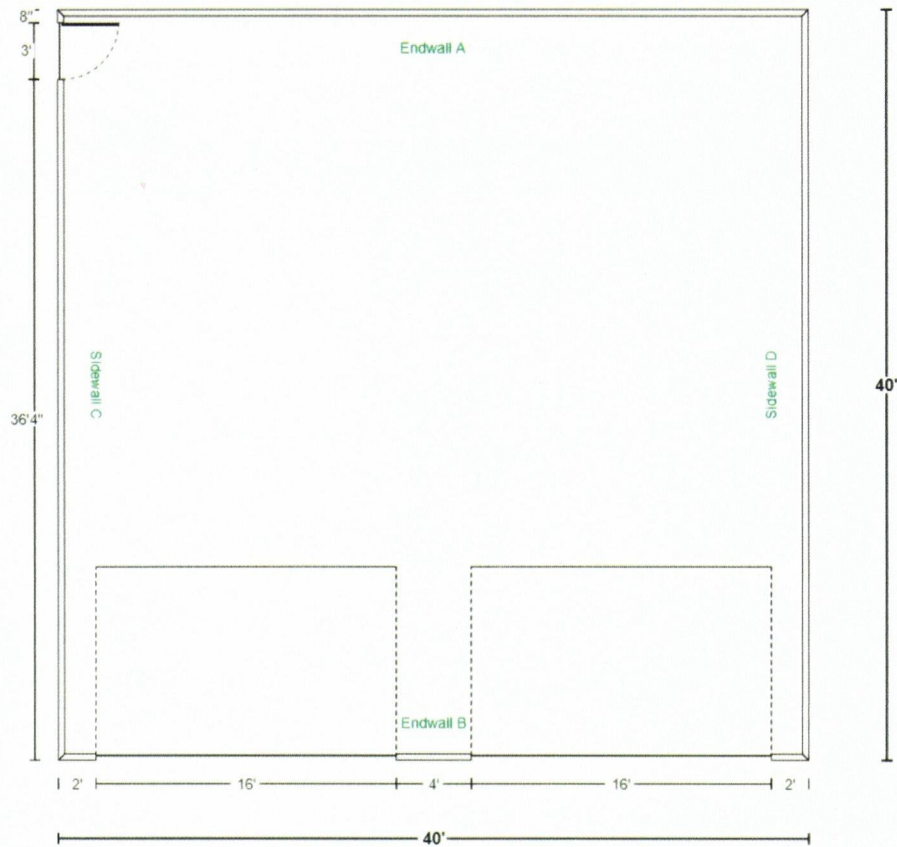
OR

1. On Menards.com, enter "Design & Buy" in the search bar
2. Select the Garage Designer
3. Recall your design by entering Design ID: 316358799911
4. Follow the on-screen purchasing instructions

**How to purchase your design at the store:**

1. Enter Design ID: 316358799911 at the Design-It Center Kiosk in the Building Materials Department
2. Follow the on-screen purchasing instructions

**Garage Image**



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**Floor type (concrete, dirt, gravel) is NOT included in estimated price. The floor type is used in the calculation of materials needed. Labor, foundation, steel beams, paint, electrical, heating, plumbing, and delivery are also NOT included in estimated price. This is an estimate. It is only for general price information. This is not an offer and there can be no legally binding contract between the parties based on this estimate. The prices stated herein are subject to change depending upon the market conditions. The prices stated on this estimate are not firm for any time period unless specifically written otherwise on this form. The availability of materials is subject to inventory conditions.**

MENARDS IS NOT RESPONSIBLE FOR ANY LOSS INCURRED BY THE GUEST WHO RELIES ON PRICES SET FORTH HEREIN OR ON THE AVAILABILITY OF ANY MATERIALS STATED HEREIN. All information on this form, other than price, has been provided by the guest and Menards is not responsible for any errors in the information on this estimate, including but not limited to quantity, dimension and quality. Please examine this estimate carefully.

MENARDS MAKES NO REPRESENTATIONS, ORAL, WRITTEN OR OTHERWISE THAT THE MATERIALS LISTED ARE SUITABLE FOR ANY PURPOSE BEING CONSIDERED BY THE GUEST. BECAUSE OF WIDE VARIATIONS IN CODES, THERE ARE NO REPRESENTATIONS THAT THE MATERIALS LISTED HEREIN MEET YOUR CODE REQUIREMENTS. THE PLANS AND/OR DESIGNS PROVIDED ARE NOT ENGINEERED. LOCAL CODE OR ZONING REGULATIONS MAY REQUIRE SUCH STRUCTURES TO BE PROFESSIONALLY ENGINEERED AND CERTIFIED PRIOR TO CONSTRUCTION.

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## Materials

### Building Info

Building Location Zip Code:	56472
Building Width:	40'
Building Length:	40'
Building Height:	12'
Curb:	Poured Curb
Curb Height:	4"
Foundation Type:	Thickened Slab
Wall Framing Stud:	2 x 6
Roof Framing:	Truss Construction
Truss Type:	Common
Roof Pitch:	6/12 Pitch
Eave Overhang:	24"
Gable Overhang:	24"
Custom Garage Plan:	No I do not need a custom building plan

### Wall Info

Siding Material Types:	Cedar Bevel Lap
Accent Material Type:	None
Wainscot Material Type:	None
Wall Sheathing:	7/16 x 4 x 8 OSB(Oriented Strand Board)
House Wrap:	Kimberly-Clark BLOCK-IT®9'x75'House Wrap
Gable Vents:	None

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## Roof Info

Roof Sheathing:	1/2 x 4 x 8 OSB(Oriented Strand Board)
Roofing Material Type:	Architectural Shingle
Architectural Roofing:	Owens Corning® TruDefinition® Duration® Limited Lifetime Warranty Architectural Shingles (32.8 sq. ft.), Color: Estate Gray
Roof Underlayment:	Owens Corning® ProArmor® Synthetic Roofing Underlayment 42" x 286' (1,000 sq. ft.)
Ice and Water Barrier:	Owens Corning® WeatherLock® G Granulated Self-Sealing Ice and Water Barrier 3' x 66.7'(200sq.ft)
Fascia Material Type:	Cedar Fascia
Fascia:	1 x 6 Red Cedar Board
Soffit Material Type:	Cedar Soffit
Soffit:	3/8 x 4 x 8 Textured No Groove Plywood
Roof Edge:	12' Aluminum Style D Roof Edging, Color: White
Eave Vent:	None
Gutter Material Type:	None
Ridge Vent:	None
Roof Vents:	None

## Openings

Service Door:	Mastercraft® 36W x 80H Primed Steel 6-Panel
Overhead Door:	Ideal Door® Commercial 16' x 10' White Insulated
Overhead Door:	Ideal Door® Commercial 16' x 10' White Insulated
Overhead Door Trim Type:	Vinyl
Vinyl Trim Color:	White

## Additional Options

Ceiling Insulation:	Blow-in Cellulose
Ceiling Insulation R Value:	R13 INSULMAX® Blow-in Cellulose Insulation
Wall Insulation:	R-19 Unfaced Fiberglass Insulation 6-1/4" x 15" x 39.2' - 48.96 sq ft
Ceiling Finish:	Cut to Length Pro-Rib® Steel Panel
Wall Finish:	Cut to Length Pro-Rib® Steel Panel
Hydronic Radiant Heat:	No
Anchor bolt:	Grip Fast® 1/2 x 10 HDG Anchor Bolt w/ Nut & Washer
Framing Fasteners:	Grip Fast® 3-1/4 16D Vinyl-Coated Smooth Shank Sinker Nail - 5 lb. Box
Sheathing Fasteners:	Grip Fast® 2-1/2 8D Vinyl-Coated Smooth Shank Sinker Nail - 5 lb. Box
Roofing/Shingle Fasteners:	Grip Fast® 1-1/4 Electro-Galvanized Coil Roofing Nails - 7,200 Count
Truss Fastener:	FastenMaster® TimberLOK® 5/16 x 6 Hex Drive Black Hex Head Timber Screw - 50 Count
Overhead Opening Hardware:	No

For other design systems search "Design & Buy" on Menards.com

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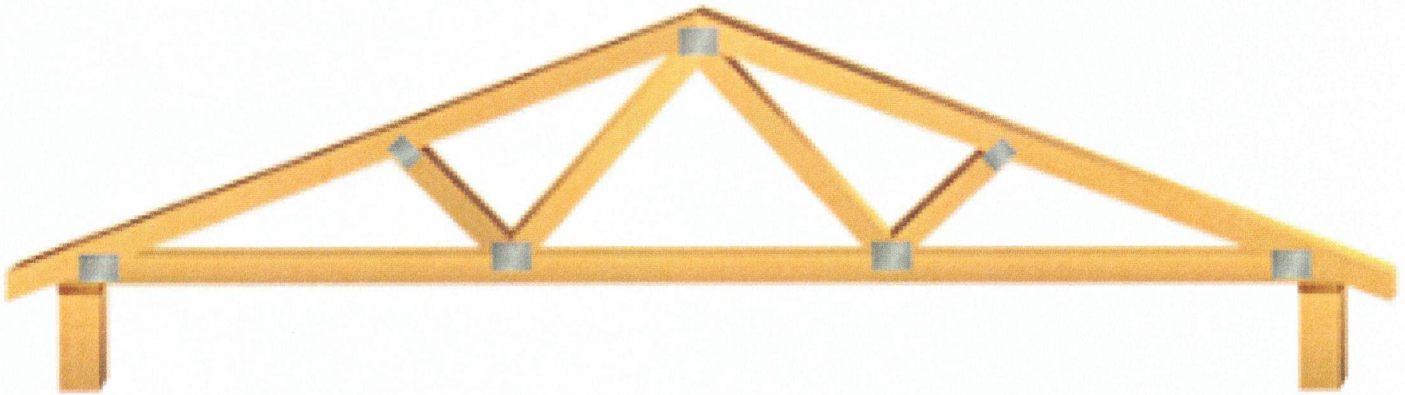
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## Helpful Hints for Garage Construction

- Studs are estimated 16 inches on center with single treated bottom plate and double top plate.
- For 10- and 12-foot-tall buildings studs should be cut for an approximate 10- or 12-foot plate height.
- If steel is estimated (Pro-Rib or Pro-Snap), the steel lengths should be verified based off the actual framing. Plate height (stud length), truss heel and other framing should be confirmed. Steel is estimated to the inch, make sure the lengths are accurate based on final overall building design.
- Trusses included are estimated at 2 feet on center spacing. The design is based on the zip code provided, design and loading should be verified.
- Trusses should not be cut or modified with the exception of trimming the truss tails to the correct overhang.
- The bottom chord is designed to support standard ceiling and insulation materials.
- Dropped end trusses are estimated with 18 inch and 24 inch gable overhangs.



## Menards Building Checklist Planning

- Get a permit. Check restrictions, building codes or local zoning to make sure your design complies with all requirements.
- Contact local utilities to ensure construction will not disturb any electrical, cable or plumbing.
- If necessary, hire a professional to help with planning and construction.
- Consider site conditions including soil type, grade, and runoff before finalizing your design.
- Material estimates provided can be changed to meet your needs.
- Menards offers professional delivery of materials. Delivery is extra based on the distance from your local Menards store to your building site.
- Practice good safety habits, use PPE including eye protection & dust masks during construction.
- Make sure to follow good building practice and all manufacturer's instructions. Use all the hardware and fasteners recommended.

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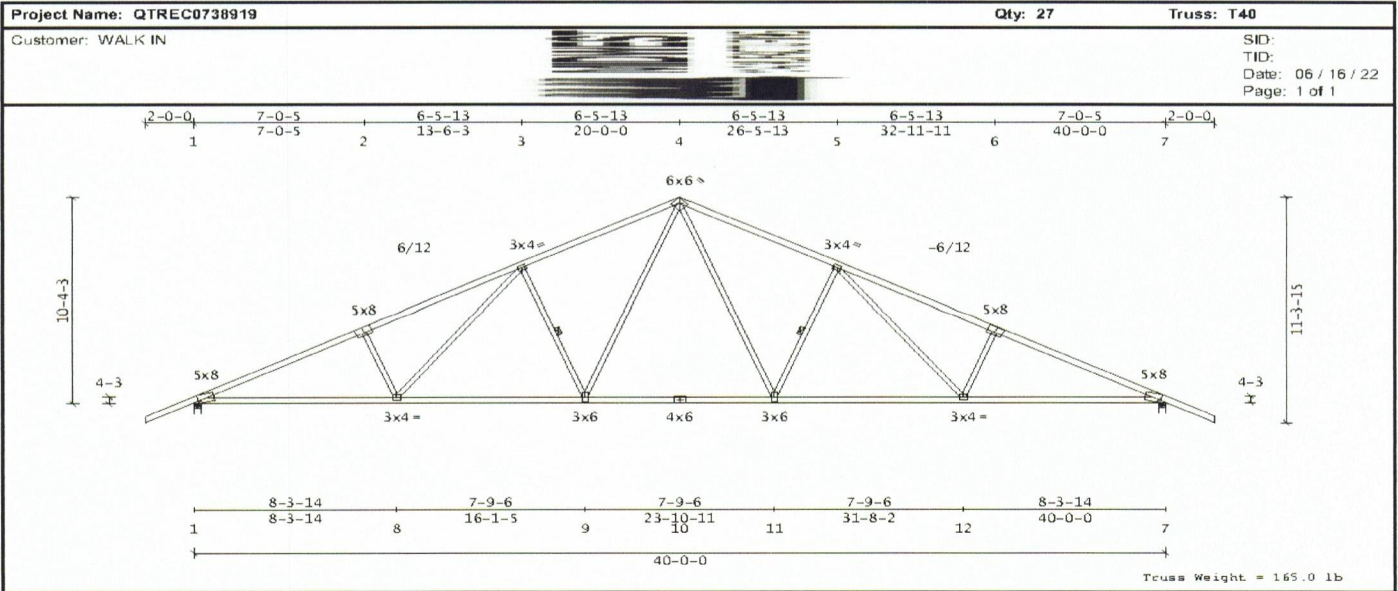
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# Design & Buy™

## GARAGE



Code/Design: IRC-2018/TPI-2014	-----Snow Load Specs-----	-----Wind Load Specs-----	-----Additional Design Checks-----
PSF Live Dead Dur Factors	ASCE7-16 Ground Snow (Pg) = 60.0 psf	ASCE7-16 Wind Speed (V) = 115 mph	10 psf Non-Concurrent SCLL: Yes
TC 42.0 7.0 Live Wind Snow	Risk Cat: II Terrain Cat: B	Risk Cat: II Exposure Cat: B	20 psf BC Limited Storage: Yes
BC 0.0 10.0 Lum 1.15 1.60 1.15	Roof Exposure: Fully Exposed	Bldg Dims: L = 0.0 ft S = 0.0 ft	200 lb BC Accessible Ceiling: No
Total 59.0 Plt 1.15 1.60 1.15	Thermal Condition: Cold	M.R.H(h) = 15.0 ft Kzt = 1.0	300 lb TC Maintenance Load: No
Spacing: 2-00-00 o.c. Piles: 1	Ventilated (1.1)	Bldg Enclosure: Enclosed	2000 lb TC Safe Load: No
Repetitive Member Increase: Yes	Unobstructed Slippery Roof: No	Wind DL(psf): TC = 4.2 BC = 6.0	Unbalanced TCLL: Yes
Green Lumber: No Wet Service: No	Low-Slope Minimum (P <sub>min</sub> ): No	End Vertical Exposed: L = Yes R = Yes	
Fib Tolerance: 15% Creep (K <sub>cc</sub> ) = 2.0	Unbalanced Snow Loads: Yes	Wind Uplift Reporting: ASCE7 MWFRS	
OH Soffit Load: 1.0 psf	Rain Surcharge: No Ice Dam Chk: Yes	C&C End Zone: 4-00-00	

<b>Material Summary</b> TC 2x4 SPF 1650/1.5 PSF 2x4 SPF 2100/1.8 2-4 4-6 BC 2x4 SPF 1650/1.5 Webs 2x3 SPF Stud 2x3 SPF #2 4-11 9-4	<b>Reaction Summary</b> Lu(max) = 22-00-00 -----Reaction Summary (Lbs)----- Jnt --X- Loc- React -Up- --Width- -Reqd -Mat PSI 1 01-12 2560 0 03-08 04-00** DPL 425 7 39-10-04 2560 0 03-08 04-00** DPL 425 Max Horiz = -140 / +140 at Joint 1 (**) indicates Req'd Width > actual Width; enhancement may be required. Building Designer to provide adequate bearing size or enhancement.	<b>Deflection Summary</b> TrussSpan Limit Actual (in) Location Vert LL L/240 L/999 (-0.39) 11-12 Vert DL L/90 L/999 (-0.18) 11-12 Vert CR L/180 L/833 (-0.57) 11-12 Horiz LL 0.75in ( 0.14) @Jt 7 Horiz CR 1.25in ( 0.20) @Jt 7 Ohng CR 2L/180 2L/982 (-0.05) 1-1 Ohng CR 2L/180 2L/982 (-0.05) 7-7																																																																																												
<b>Member Forces Summary</b> Max CSI in TC PANEL 1 - 2 0.83 Max CSI in BC PANEL 1 - 8 0.76 Max CSI in Web 3 - 9 0.77	<b>Loads Summary</b> This truss has been designed for the effects of an unbalanced top chord live load occurring at [20-00-00] using a 1.00 Full and 0.00 Reduced load factor. See Loadcase Report for loading combinations and additional details.	<b>Bracing Data Summary</b> -----Bracing Data----- Chords: continuous except where shown ----- Web Bracing ----- CLR ----- Single: 3- 9 11- 5 Continuous Restraint Bracing Req'd See BCSI-83 3.0																																																																																												
<table border="1"> <thead> <tr> <th>Mem</th> <th>Ten</th> <th>Comp</th> <th>CSI</th> </tr> </thead> <tbody> <tr><td>TC OH- 1</td><td>92</td><td>0</td><td>0.37</td></tr> <tr><td>1- 2</td><td>495</td><td>4423</td><td>0.83</td></tr> <tr><td>2- 3</td><td>537</td><td>4193</td><td>0.74</td></tr> <tr><td>3- 4</td><td>495</td><td>3224</td><td>0.80</td></tr> <tr><td>4- 5</td><td>495</td><td>3224</td><td>0.80</td></tr> <tr><td>5- 6</td><td>538</td><td>4193</td><td>0.74</td></tr> <tr><td>6- 7</td><td>495</td><td>4423</td><td>0.83</td></tr> <tr><td>7- OH</td><td>92</td><td>0</td><td>0.37</td></tr> <tr><td>BC 1- 8</td><td>3833</td><td>335</td><td>0.76</td></tr> <tr><td>7-12</td><td>3833</td><td>343</td><td>0.76</td></tr> <tr><td>8- 9</td><td>3106</td><td>212</td><td>0.75</td></tr> <tr><td>9-10</td><td>2320</td><td>67</td><td>0.63</td></tr> <tr><td>10-11</td><td>2320</td><td>67</td><td>0.63</td></tr> <tr><td>11-12</td><td>3106</td><td>214</td><td>0.75</td></tr> <tr><td>Web 2- 8</td><td>191</td><td>571</td><td>0.28</td></tr> <tr><td>3- 8</td><td>861</td><td>98</td><td>0.52</td></tr> <tr><td>3- 9</td><td>249</td><td>1220</td><td>0.77</td></tr> <tr><td>4- 9</td><td>1397</td><td>151</td><td>0.48</td></tr> <tr><td>4-11</td><td>1397</td><td>151</td><td>0.48</td></tr> <tr><td>5-11</td><td>249</td><td>1220</td><td>0.77</td></tr> <tr><td>5-12</td><td>861</td><td>98</td><td>0.52</td></tr> <tr><td>6-12</td><td>191</td><td>571</td><td>0.28</td></tr> </tbody> </table>	Mem	Ten	Comp	CSI	TC OH- 1	92	0	0.37	1- 2	495	4423	0.83	2- 3	537	4193	0.74	3- 4	495	3224	0.80	4- 5	495	3224	0.80	5- 6	538	4193	0.74	6- 7	495	4423	0.83	7- OH	92	0	0.37	BC 1- 8	3833	335	0.76	7-12	3833	343	0.76	8- 9	3106	212	0.75	9-10	2320	67	0.63	10-11	2320	67	0.63	11-12	3106	214	0.75	Web 2- 8	191	571	0.28	3- 8	861	98	0.52	3- 9	249	1220	0.77	4- 9	1397	151	0.48	4-11	1397	151	0.48	5-11	249	1220	0.77	5-12	861	98	0.52	6-12	191	571	0.28	<b>Notes</b> Plates designed for Cq at 0.85 and Rotational Tolerance of 10.0 degrees Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Continuous Lateral Restraint (CLR) crows require diagonal bracing per D-WESCLRBACE. Alternatively, see D-WESREINFORCE.	<b>Plate offsets (X, Y):</b> (None unless indicated below) Jnt1 (02-10,01-08), Jnt2 (-00-07,00-14), Jnt4 (0,-00-05), Jnt6 (00-07,00-14), Jnt7 (-02-10,01-08), Jnt10 (0,00-04)
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		<b>Joint Stress Index (JSI):</b> Jnt1 (0.97), Jnt2 (0.88), Jnt3 (0.98), Jnt4 (0.94), Jnt5 (0.98), Jnt6 (0.88), Jnt7 (0.97), Jnt8 (0.90), Jnt9 (0.94), Jnt10 (0.99), Jnt11 (0.94), Jnt12 (0.90)																																																																																												

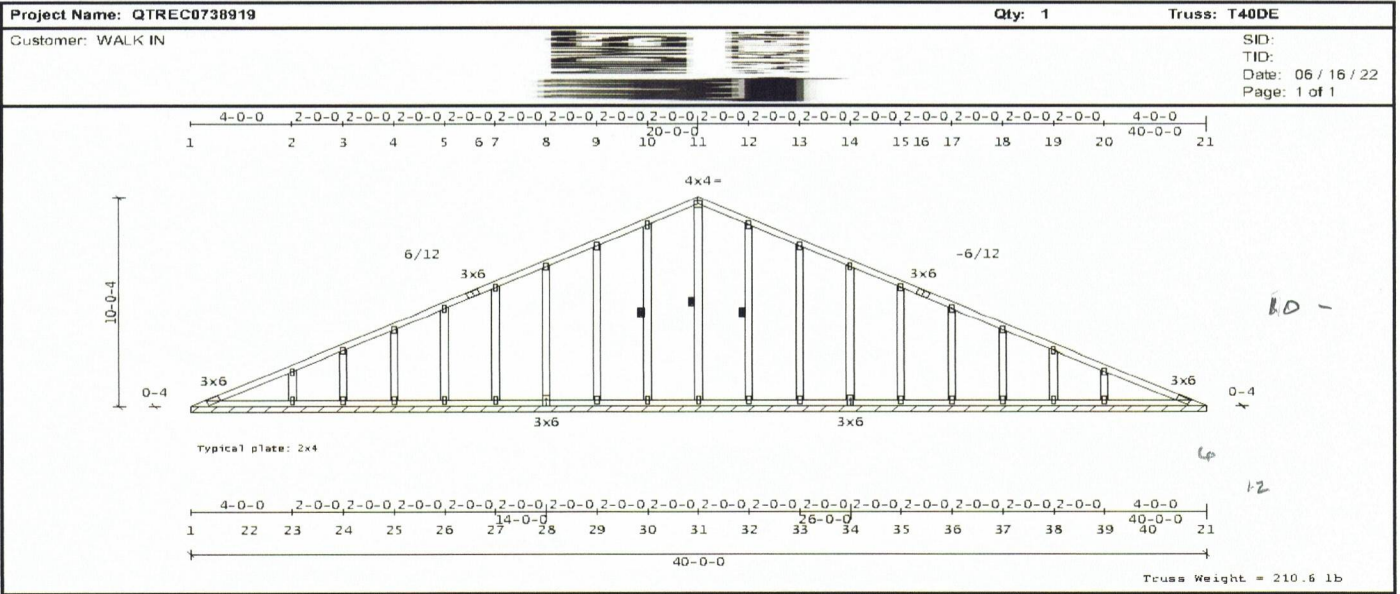
NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "#8 15", which indicates a high tension 18 gauge plate.



Midwest Manufacturing  
 5311 Kane Road  
 Eau Claire, WI 54703  
 (715) 876-5555  
 midwestmanufacturing.com

# Design & Buy™

## GARAGE



<b>Code/Design:</b> IRC-2018/PP1-2014 <b>PSF Live Dead:</b> 42.0 7.0 <b>TC:</b> 0.0 10.0 <b>Total:</b> 59.0 <b>Spacing:</b> 2'-00" o.c. <b>Repetitive Member Increase:</b> Yes <b>Green Lumber:</b> No <b>Fab Tolerance:</b> 15% Creep (Kcr) = 2.0 <b>OH Soffit Load:</b> 1.0 psf	<b>Snow Load Specs:</b> ASCE7-15 Ground Snow (Pg) = 60.0 psf Risk Cat: II Terrain Cat: B Roof Exposure: Fully Exposed Thermal Condition: Cold Ventilated (I.1) Unobstructed Slippery Roof: No Low-Slope Minimums (P6min): No Unbalanced Snow Loads: Yes Rain Surcharge: No Ice Dam Chk: Yes	<b>Wind Load Specs:</b> ASCE7-15 Wind Speed (V) = 115 mph Risk Cat: II Exposure Cat: B Sldg Dims: L = 0.0 Et B = 0.0 Et M.R.H(h) = 15.0 Et Kzt = 1.0 Sldg Enclosure: Enclosed Wind Dir (psf): TC = 4.2 BC = 6.0 End Vertical Exposed: L = Yes R = Yes Wind Uplift Reporting: ASCE7 MWFRS C&C End Zone: 4-00-00	<b>Additional Design Checks:</b> 10 psf Non-Concurrent BCLL: Yes 20 psf BC Limited Storage: Yes 200 lb BC Accessible Ceiling: No 300 lb TC Maintenance Load: No 2000 lb TC Safe Load: No Unbalanced TCLL: Yes
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<b>Material Summary</b> TC 2x4 SPF #2 BC 2x4 SPF #2 Webs 2x4 SPF Stud J2-12 33-13 2x4 SPF #2 29-9 30-10 31-11	<b>Reaction Summary</b> Reactions not shown: down < 400 and up < 150 --- Reaction Summary (plf) --- Jnt-Jnt React -Up- --Width- 1- 21 118 0.40-00-00 Max Horiz = -135 / +135 at Joint 31	<b>Deflection Summary</b> Truss Span Limit Actual (in) Location Vert DL L/240 L/999 (-0.00) 1-22 Vert LL L/90 L/999 (-0.00) 1-22 Vert CR L/180 L/999 (-0.01) 1-22 Horiz LL 0.75in (0.01) @J21 Horiz CR 1.25in (0.01) @J21																																																																																																				
<b>Member Forces Summary</b> Max CSI in TC PANEL 1 - 2 0.17 Max CSI in BC PANEL 1 - 22 0.11 Max CSI in Web 29 - 9 0.32	<b>Loads Summary</b> This truss has been designed for the effects of an unbalanced top chord live load occurring at [20-00-00] using a 1.00 Full and 0.00 Reduced load factor. See Loadcase Report for loading combinations and additional details.	<b>Bracing Data Summary</b> -----Bracing Data----- Chords: continuous except where shown ----- Web Bracing ----- CLR Single: 30-10 31-11 32-12 Continuous Restraint Bracing Req'd See BCSI-B3 3.0																																																																																																				
<table border="1"> <thead> <tr> <th>Mem</th> <th>Ten</th> <th>Comp</th> <th>CSI</th> </tr> </thead> <tbody> <tr><td>TC 1-6</td><td>57</td><td>111</td><td>0.17</td></tr> <tr><td>6-11</td><td>182</td><td>98</td><td>0.13</td></tr> <tr><td>11-16</td><td>182</td><td>98</td><td>0.13</td></tr> <tr><td>16-21</td><td>57</td><td>111</td><td>0.17</td></tr> <tr><td>BC 1-28</td><td>134</td><td>62</td><td>0.11</td></tr> <tr><td>21-34</td><td>134</td><td>62</td><td>0.11</td></tr> <tr><td>28-34</td><td>134</td><td>62</td><td>0.03</td></tr> <tr><td>Web 2-23</td><td>126</td><td>279</td><td>0.06</td></tr> <tr><td>3-24</td><td>57</td><td>169</td><td>0.04</td></tr> <tr><td>4-25</td><td>69</td><td>200</td><td>0.06</td></tr> <tr><td>5-26</td><td>66</td><td>195</td><td>0.09</td></tr> <tr><td>7-27</td><td>66</td><td>208</td><td>0.14</td></tr> <tr><td>8-28</td><td>65</td><td>209</td><td>0.28</td></tr> <tr><td>9-29</td><td>70</td><td>311</td><td>0.32</td></tr> <tr><td>10-30</td><td>59</td><td>328</td><td>0.12</td></tr> <tr><td>11-31</td><td>41</td><td>159</td><td>0.07</td></tr> <tr><td>12-32</td><td>59</td><td>328</td><td>0.12</td></tr> <tr><td>13-33</td><td>70</td><td>311</td><td>0.32</td></tr> <tr><td>14-34</td><td>65</td><td>299</td><td>0.28</td></tr> <tr><td>15-35</td><td>66</td><td>208</td><td>0.14</td></tr> <tr><td>17-36</td><td>66</td><td>195</td><td>0.09</td></tr> <tr><td>18-37</td><td>69</td><td>200</td><td>0.06</td></tr> <tr><td>19-38</td><td>57</td><td>169</td><td>0.04</td></tr> <tr><td>20-39</td><td>126</td><td>279</td><td>0.06</td></tr> </tbody> </table>	Mem	Ten	Comp	CSI	TC 1-6	57	111	0.17	6-11	182	98	0.13	11-16	182	98	0.13	16-21	57	111	0.17	BC 1-28	134	62	0.11	21-34	134	62	0.11	28-34	134	62	0.03	Web 2-23	126	279	0.06	3-24	57	169	0.04	4-25	69	200	0.06	5-26	66	195	0.09	7-27	66	208	0.14	8-28	65	209	0.28	9-29	70	311	0.32	10-30	59	328	0.12	11-31	41	159	0.07	12-32	59	328	0.12	13-33	70	311	0.32	14-34	65	299	0.28	15-35	66	208	0.14	17-36	66	195	0.09	18-37	69	200	0.06	19-38	57	169	0.04	20-39	126	279	0.06	<b>Notes</b> Gable webs are attached with min. 1x3 20 ga. plates. The max. rake overhang = 1/2 the truss spacing. If this truss is exposed to wind loads perpendicular to the plane of the truss, it must be braced according to a standard detail matching the wind criteria shown, or according to the Construction Documents and/or BCSI - B3. Plates designed for Cq at 0.85 and Rotational Tolerance of 10.0 degrees Plates located at TC pitch breaks meet the prescriptive minimum size requirement to transfer unblocked diaphragm loads across those joints. Continuous Lateral Restraint (CLR) cove requires diagonal bracing per D-WEBCLRSRACE. Alternatively, see D-WEBREINFORCE.	<b>Plate offsets (X, Y):</b> (None unless indicated below) Jnt28(0, -00-08), Jnt34(0, -00-08)
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<b>Joint Stress Index (JSI):</b> Jnt1(0.43), Jnt2(0.49), Jnt3(0.49), Jnt4(0.49), Jnt5(0.49), Jnt6(0.33), Jnt7(0.49), Jnt8(0.49), Jnt9(0.49), Jnt10(0.49), Jnt11(0.49), Jnt12(0.49), Jnt13(0.49), Jnt14(0.49), Jnt15(0.49), Jnt16(0.33), Jnt17(0.49), Jnt18(0.49), Jnt19(0.49), Jnt20(0.49), Jnt21(0.43), Jnt22(0.49), Jnt24(0.49), Jnt25(0.49), Jnt26(0.49), Jnt27(0.49), Jnt28(0.65), Jnt29(0.49), Jnt30(0.49), Jnt31(0.49), Jnt32(0.49), Jnt33(0.49), Jnt34(0.65), Jnt35(0.49), Jnt36(0.49), Jnt37(0.49), Jnt38(0.49), Jnt39(0.49)																																																																																																						

NOTICE A copy of this design shall be furnished to the erection contractor. The design of this individual truss is based on design criteria and requirements supplied by the Truss Manufacturer and relies upon the accuracy and completeness of the information set forth by the Building Designer. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown. See the cover page and the "Important Information & General Notes" page for additional information. All connector plates shall be manufactured by Simpson Strong-Tie Company, Inc in accordance with ESR-2762. All connector plates are 20 gauge, unless the specified plate size is followed by a "-18" which indicates an 18 gauge plate, or "58 18", which indicates a high tension 18 gauge plate.



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