

BLUESKY ARCHITECTURE, PC

> PO BOX 65 FRISCO, CO 80443 970-333-1210 BLUESKYARCHITECTURE.NET



PCHITECT CONTROLL CONTROL CONTROL CONTROLL CONTROL CONTROL CONTROL CONTROL CONTROL CONTROL CONTROL CONTROLL CONTROL CONTR

43 BACKLAND RESIDERINE SSS, BLUE RIVER ESTATES

E

SITE PLAN

GENERAL NOTES

- I. ALL ITEMS AND WORK SHOWN IN THESE CONSTRUCTION DOCUMENTS SHALL BE PROVIDED AND INSTALLED BY THE GENERAL CONTRACTOR OR HIS SUBCONTRACTORS UNLESS NOTED AS EXISTING OR NOT IN CONTRACT ON THE DRAWINGS.
- 2. THIS PROJECT IS GOVERNED BY THE INTERNATIONAL RESIDENTIAL CODE, 2018 EDITION ADOPTED BY BLUE RIVER, COLORADO. ALL WORK INCLUDED IN THIS CONTRACT SHALL CONFORM TO ALL NATIONAL, STATE AND LOCAL CODES, REGULATIONS AND RESTRICTIONS WHICH APPLY TO THIS PROJECT. THE GENERAL CONTRACTOR AND HIS SUBCONTRACTORS SHALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS AND REQUIRED APPROVALS. BUILDING AREAS ARE SHOWN FOR CODE PURPOSES ONLY AND SHALL BE RECALCULATED FOR ANY OTHER PURPOSES.
- 3. CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS, PROPERTY BOUNDARIES, BUILDING SETBACKS, SITE SLOPES AND UTILITY LOCATIONS, ETC. ON THE JOB SITE PRIOR TO THE BEGINNING OF ANY WORK.
- 4. THE CONTRACTOR MUST VERIFY THE BUILDING LAYOUT WITH THE ARCHITECT PRIOR TO EXCAVATION ON THE SITE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ACCURATE PLACEMENT OF ALL NEW CONSTRUCTION ON THE SITE.
- 5. IT IS THE INTENT AND MEANING OF THESE DRAWINGS THAT THE CONTRACTOR AND EACH SUBCONTRACTOR PROVIDE ALL LABOR, MATERIALS, TRANSPORTATION, SUPPLIES, EQUIPMENT, ETC., TO OBTAIN A COMPLETE JOB WITHIN THE RECOGNIZED STANDARDS OF THE INDUSTRY, AND SHALL BE RESPONSIBLE FOR THE FOLLOWING MANUFACTURERS' INSTALLATION RECOMMENDATIONS AND INSTRUCTIONS.
- 6. SUBSTITUTION OF "EQUAL" PRODUCTS WILL BE ACCEPTABLE ONLY WITH OWNER'S OR ARCHITECT'S WRITTEN APPROVAL. IF THE CONTRACTOR REQUIRES ANY CHANGES WHICH IMPACT THE PROJECT SCHEDULE OR BUDGET, HE SHALL SUBMIT A WRITTEN CHANGE ORDER REQUEST TO THE OWNER OR ARCHITECT PRIOR TO THE COMMENCEMENT OF SUCH WORK, PERFORMANCE OF SUCH WORK WITHOUT APPROVAL BY CHANGE ORDER INDICATES GENERAL CONTRACTOR'S ACKNOWLEDGMENT OF NO INCREASE IN CONTRACT SUM OR TIME, CHANGES FROM THE PLANS OR SPECIFICATIONS MADE WITHOUT CONSENT OF THE ARCHITECT ARE UNAUTHORIZED AND SHALL RELIEVE THE ARCHITECT OF RESPONSIBILTY FOR ANY AND ALL CONSEQUENCES RESULTING FROM THESE CHANGES.
- J. ANY AMBIGUITY OR DISCREPENCY DISCOVERED WITHIN THESE PLANS BY THE CONTRACTOR OR HIS SUBCONTRACTORS, SHALL BE REPORTED IMMEDIATELY TO THE ARCHITECT. FAILURE TO NOTIFY THE ARCHITECT SHALL RELIEVE THE ARCHITECT OF RESPONSIBILITY FOR ALL CONSEQUENCES.
- 8. WRITTEN DIMENSIONS ALWAYS TAKE PRECEDENCE OVER SCALED DIMENSIONS. PLAN DIMENSIONS ARE TO THE FACE OF FRAMING MEMBERS, FACE OF WOOD FURRING OR FACE OF CONCRETE WALLS UNLESS OTHERWISE NOTED. SECTION OR ELEVATION DIMENSIONS ARE AT TOP OF CONCRETE, TOP OF PLYWOOD, OR TOP OF WALL PLATES OR BEAMS UNLESS OTHERWISE NOTED.

FLOOR ELEVATIONS

MAIN FLOOR

||O'-|| 3/4" = U.S.G.S. ||OO84.6'||

LOWER FLOOR

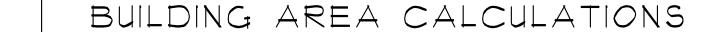
|OO'-O''| = U.S.G.S. |OO74.5'|

DOOR SCHEDULE

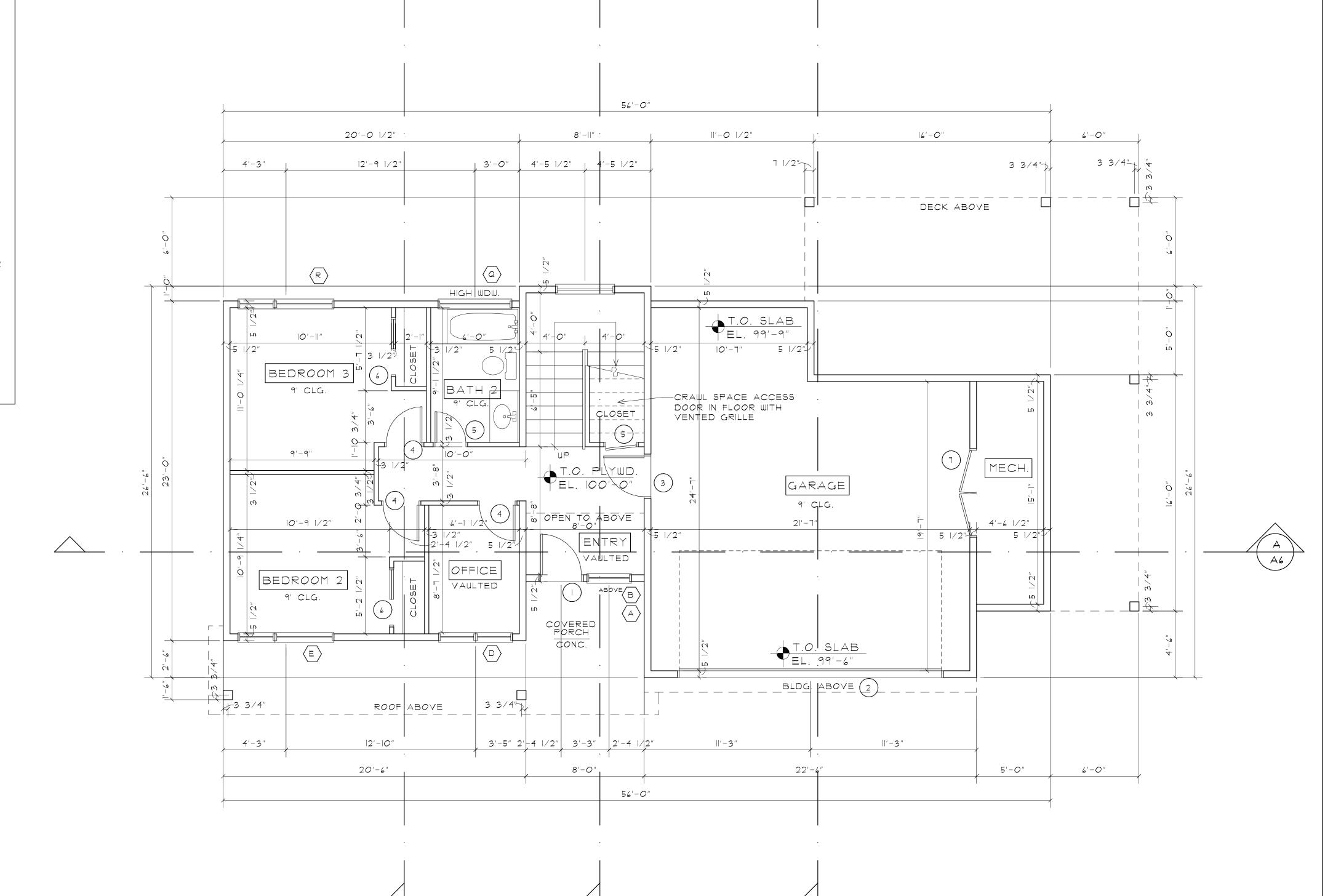
- 1									
	#	MTL.	TYPE	OPER.	HTQIW	HEIGHT	THKNES	GLASS	
	1	WOOD	SOLID	SWING	3'-0"	7'-0"	1 3/4"		CUSTOM/OWNER
	2	MOOD	GARAGE	OVHD	18'-0"	8'-0"	1 3/4"		INSUL, W/AUTO OPEN
	3	MOOD	SOLID	SWING	3'-0"	4'-8"	1 3/4"		FIRE DOOR SELF-CLOSING
	4	WOOD	PANEL	SWING	2'-8"	4′-8″	1 3/8"		
	5	WOOD	PANEL	SWING	2'-4"	₄ '-8"	1 3/8"		
-	6	WOOD	PANEL	SLIDE	4'-0"	4'-8"	1 3/8"		PAIR
	_	MOOD	PANEL	SWING	6'-0"	4′-8″	1 3/8"		PAIR
- 1									

WINDOW SCHEDULE

	$\omega \cap \langle D \rangle \cup \omega$			9 – –	
SYM	MFR ID (# UNITS)	TYPE	TINU HTQIW	UNIT HEIGHT	NOTES
А	3696	FIXED	3'-0"	8'-0"	SAFETY GLAZING
В	7530	FIXED	6'-3"	2'-6"	
С	3060	CSMT	7'-6"	5'-0"	EGRESS
D	3060(2)	CSMT	5'-0"	5'-0"	
E	3060/4860	CSMT/FIX	6'-6"	5'-0"	EGRESS
F	3018/3060/6018/6060	CSMT/FIX	7'-6"	6'-6"	EGRESS
G	3042	CSMT	2'-6"	3'-6"	
Н	4872	FIXED	4'-0"	6'-0"	
1	3018/3060/4218/4260	CSMT/FIX	6'-0"	6'-6"	
J	52102/5218	FIX/AWN	4'-4"	10'-0"	
K	5290/5218	FIX/AWN	4'-4"	9'-0"	
L	NOT USED				
M	5254/5218	FIX/AWN	4'-4"	6'-0"	
N	7254/3618(2)	FIX/AWN	6'-0"	6'-0"	
0	4896	FIXED	4'-0"	8'-0"	SAFETY GLAZING
P	3630(2)	FIXED	6'-0"	2'-6"	
Q	6018	AWNING	5'-0"	1'-6"	
R	4860/3060	FIX/CSMT	6'-6"	5'-0"	EGRESS
S	3696	SWING DR.	3'-0"	8'-0"	PATIO DOOR

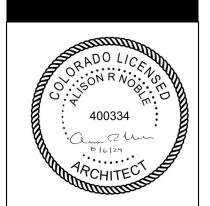


	FINISHED	UNFINISHED	TOTAL
MAIN FLOOR	8,1	0	1,178 S.F.
LOWER FLOOR	633	598	1,231 S.F.
TOTAL	1,811	598	2,409 S.F.





633 SF FINISHED 598 SF GARAGE/UNFINISHED PO BOX 65
FRISCO, CO 80443
970-333-1210
BLUESKYARCHITECTURE.NET



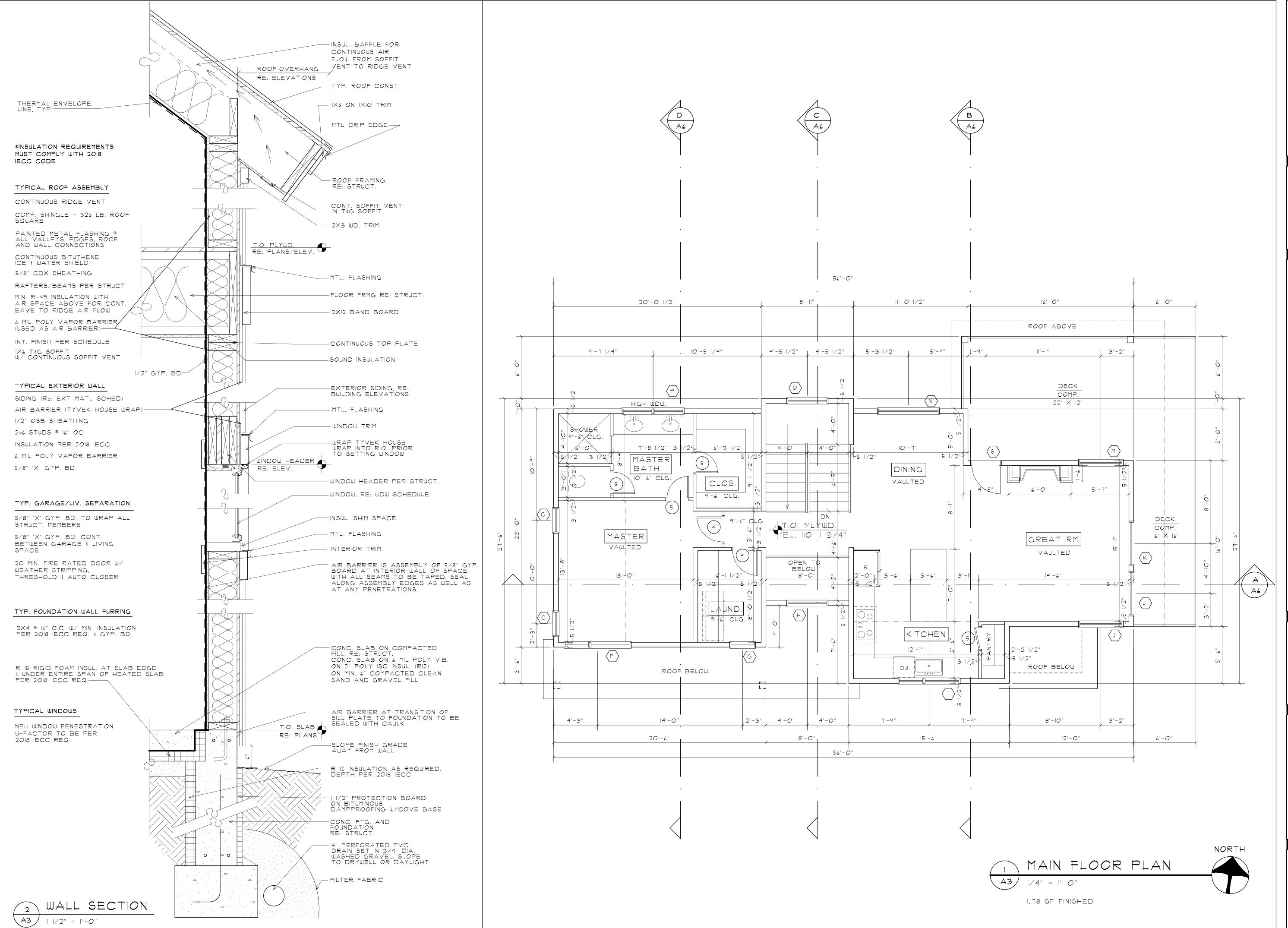
DACALAND RESIDENC

FLOOR

PLANS

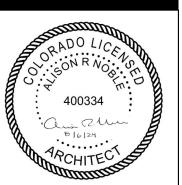
ISSUE DATE
PRELIM. 1/16/24
PERMIT 8/6/24

A2



BLUESKY ARCHITECTURE, PC

PO BOX 65 FRISCO, CO 80443 970-333-1210 BLUESKYARCHITECTURE.NET



ARCHITECTOR

WARRENTECTOR

WAR

42 B女C大L女ND REIS

TITLE

FLOOR PLANS

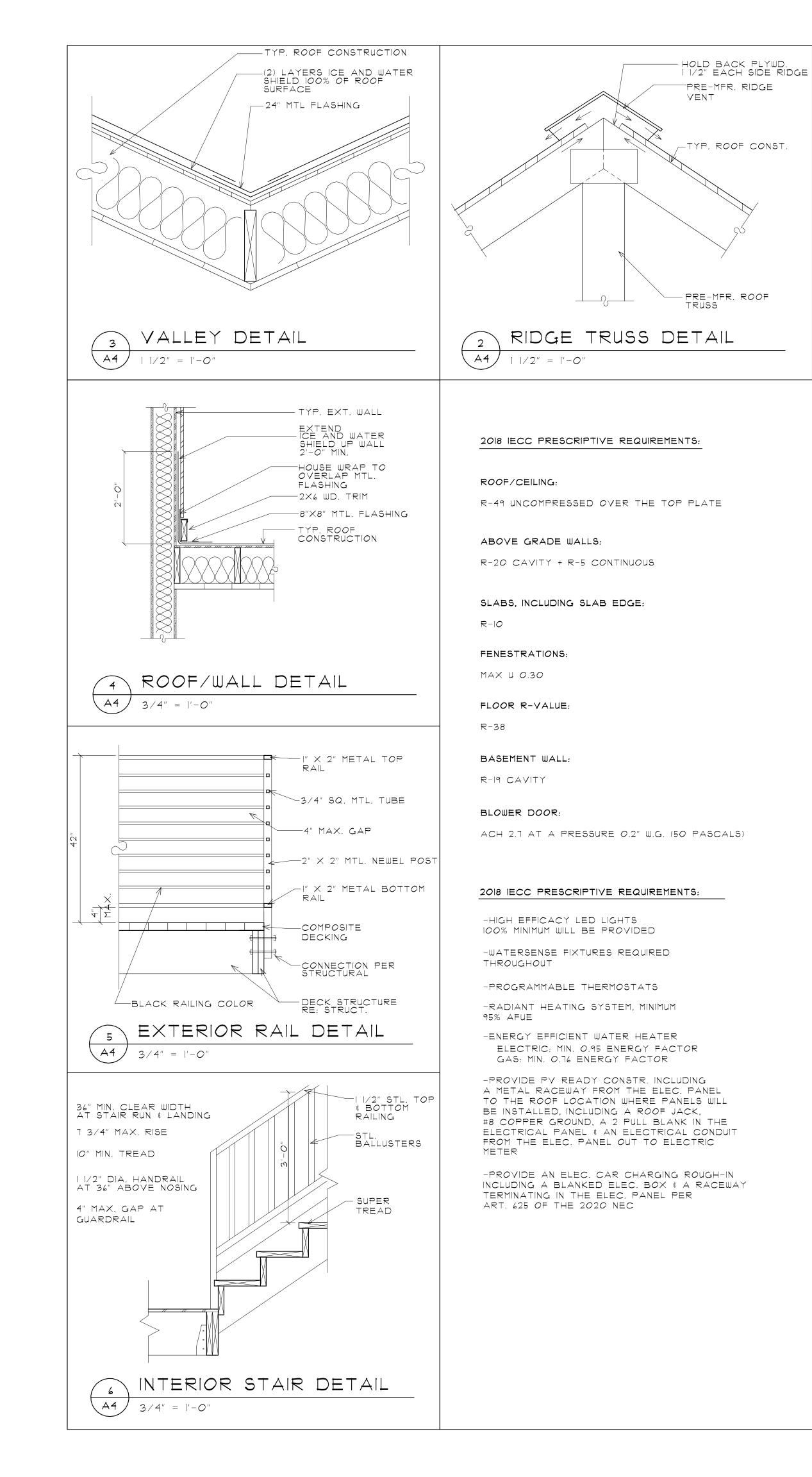
 \cup

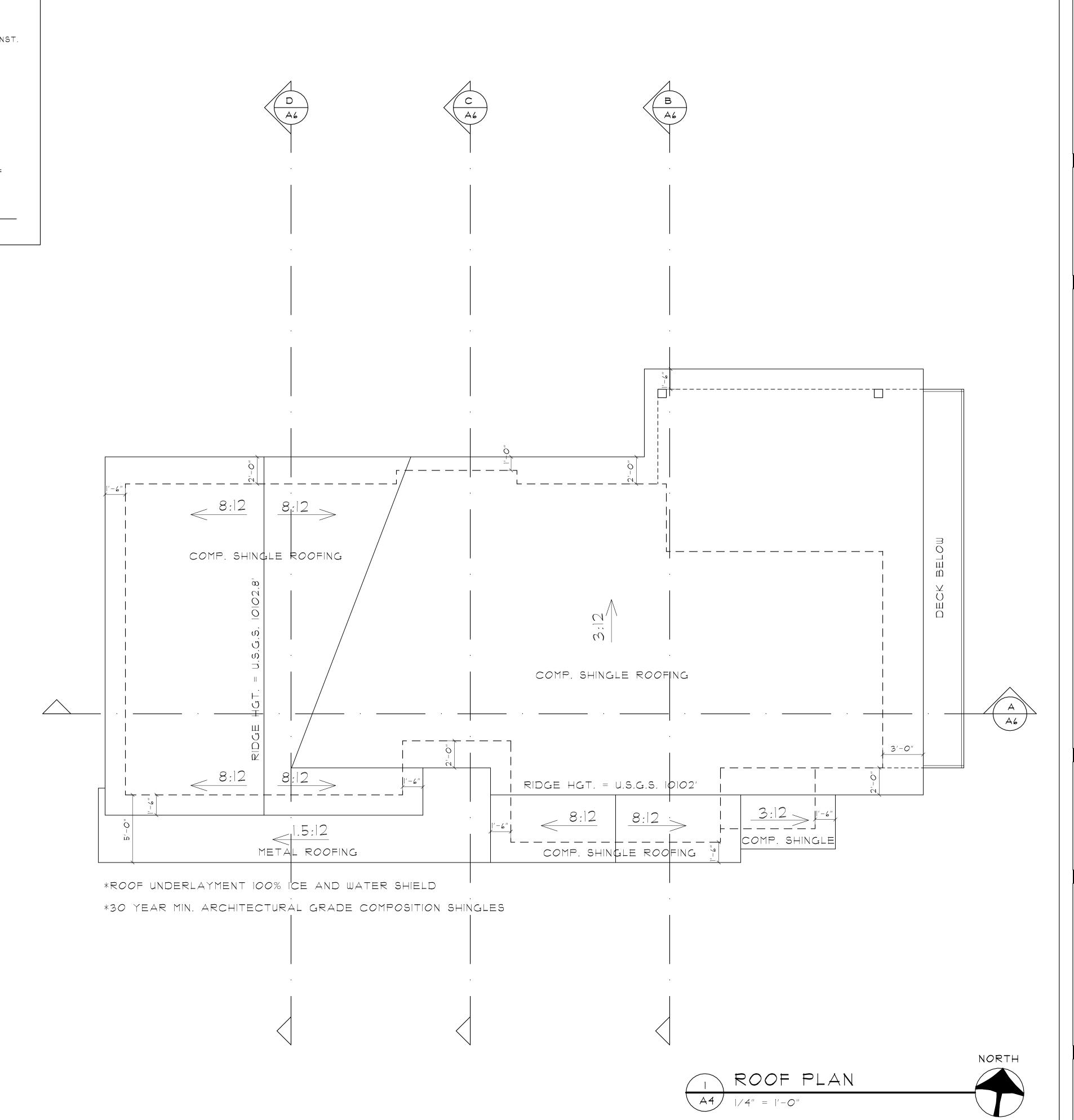
ISSUE DATE

PRELIM. 7/16/24

PERMIT 8/6/24

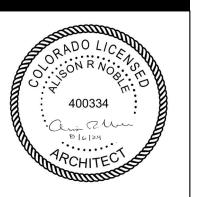
43







PO BOX 65 FRISCO, CO 80443 970-333-1210 BLUESKYARCHITECTURE.NET



BY CKLAND RESIDER

BY CKLANDER SIVER ESTATES

RIVER, COLORADO

FLOOR

PLANS

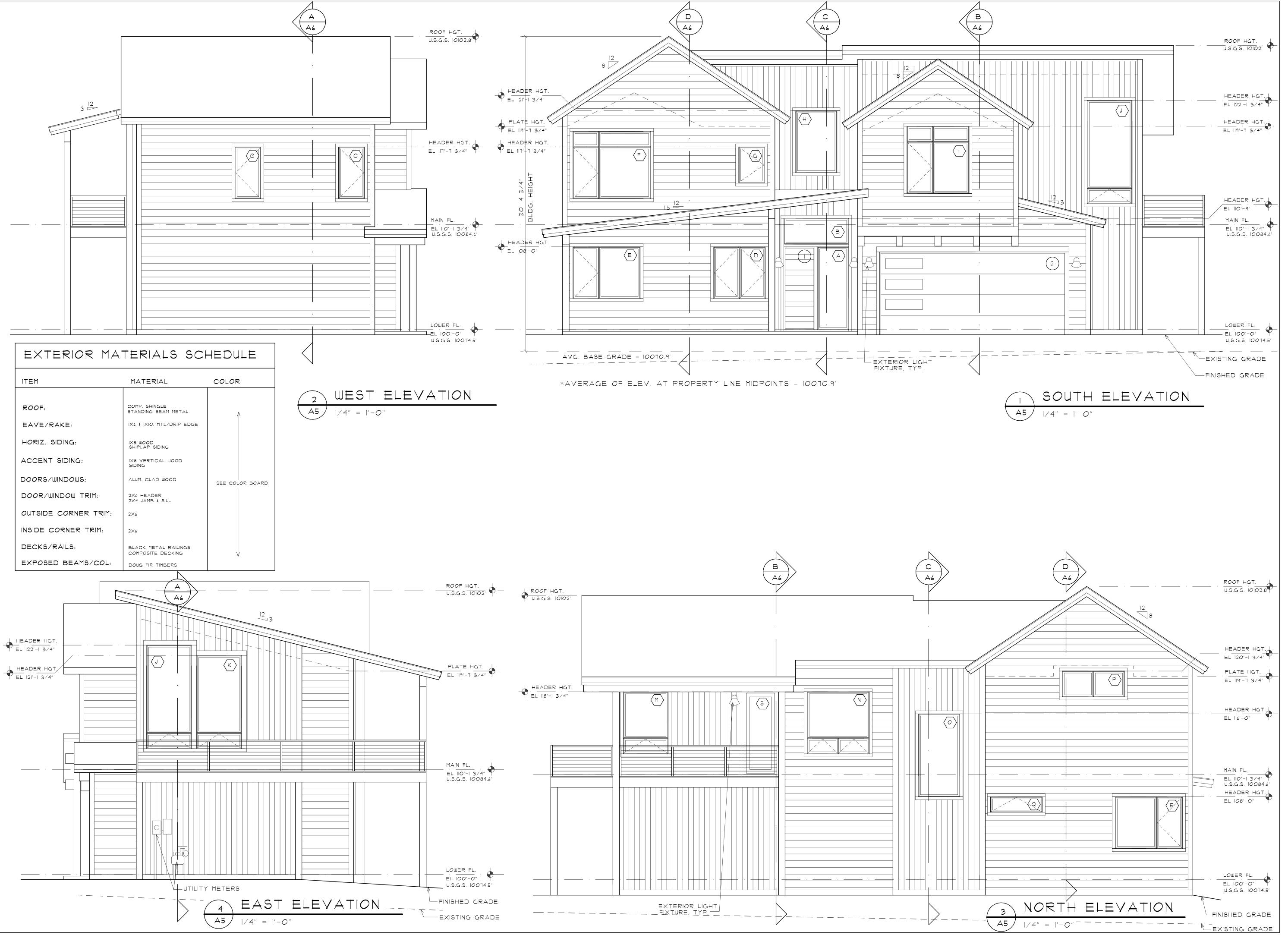
4

ISSUE DATE

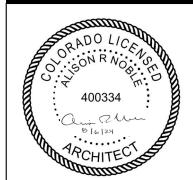
PRELIM. 1/16/24

PERMIT 8/6/24

4



PO BOX 65 FRISCO, CO 80443 970-333-1210 BLUESKYARCHITECTURE.NET

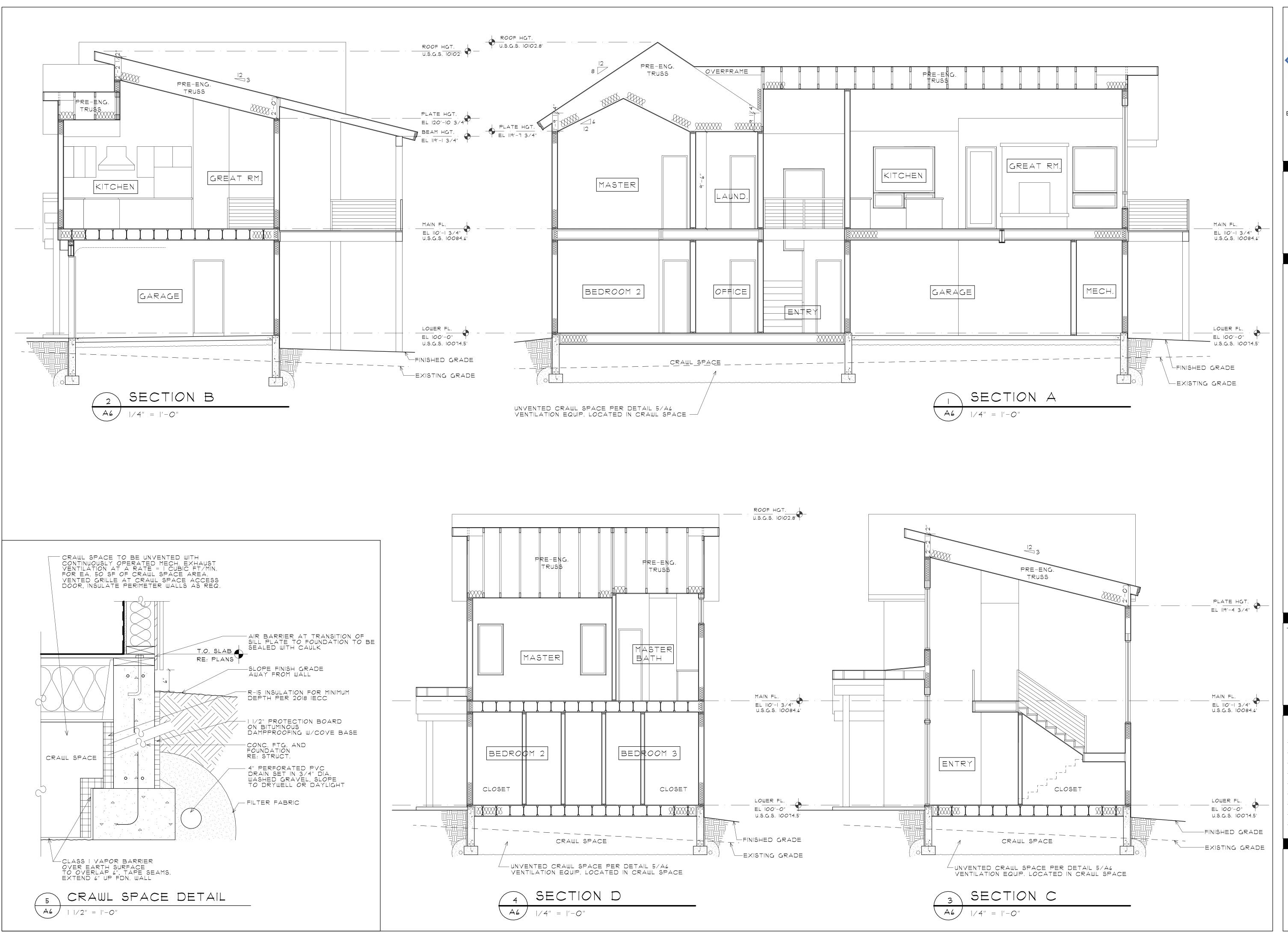




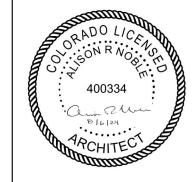
____ \mathcal{O} Ω \bigcup \mathcal{C}

TITLE BUILDING ELEVATIONS

ISSUE	DATE	
RELIM.	7/16/24	
ERMIT	8/6/24	



PO BOX 65 FRISCO, CO 80443 970-333-1210 BLUESKYARCHITECTURE.NET



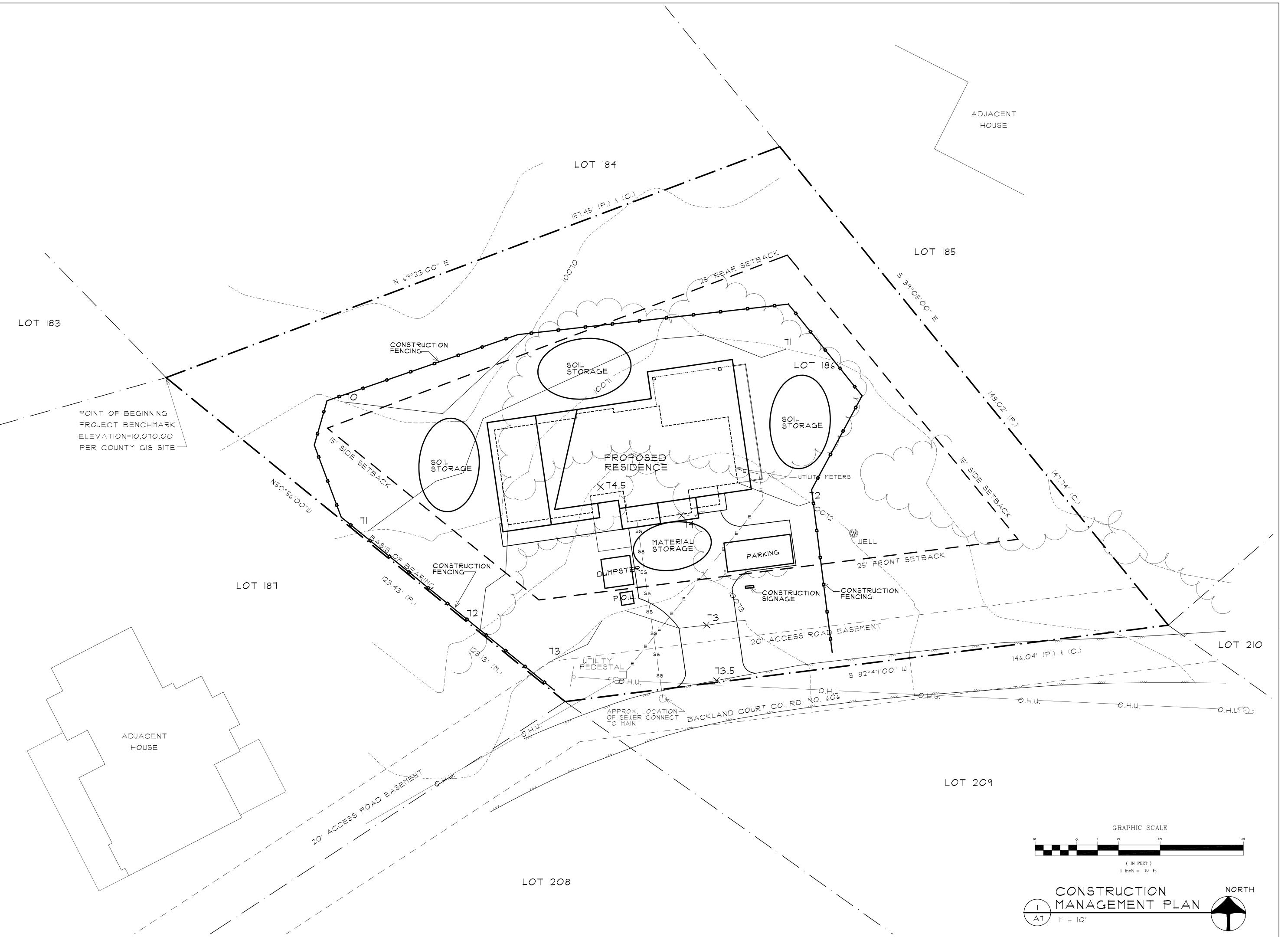


 \angle ____ \Box

TITLE BUILDING SECTIONS

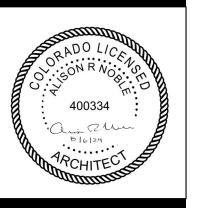
	1	
ISSUE	DATE	
RELIM.	7/16/24	
PERMIT	8/6/24	

A



BLUESKY ARCHITECTURE, PC

PO BOX 65 FRISCO, CO 80443 970-333-1210 BLUESKYARCHITECTURE.NET



LE

CONSTRUCTION
MANAGEMENT
PLAN

	1	
SSUE	DATE	
.IM.	7/16/24	
11⊤	8/6/24	

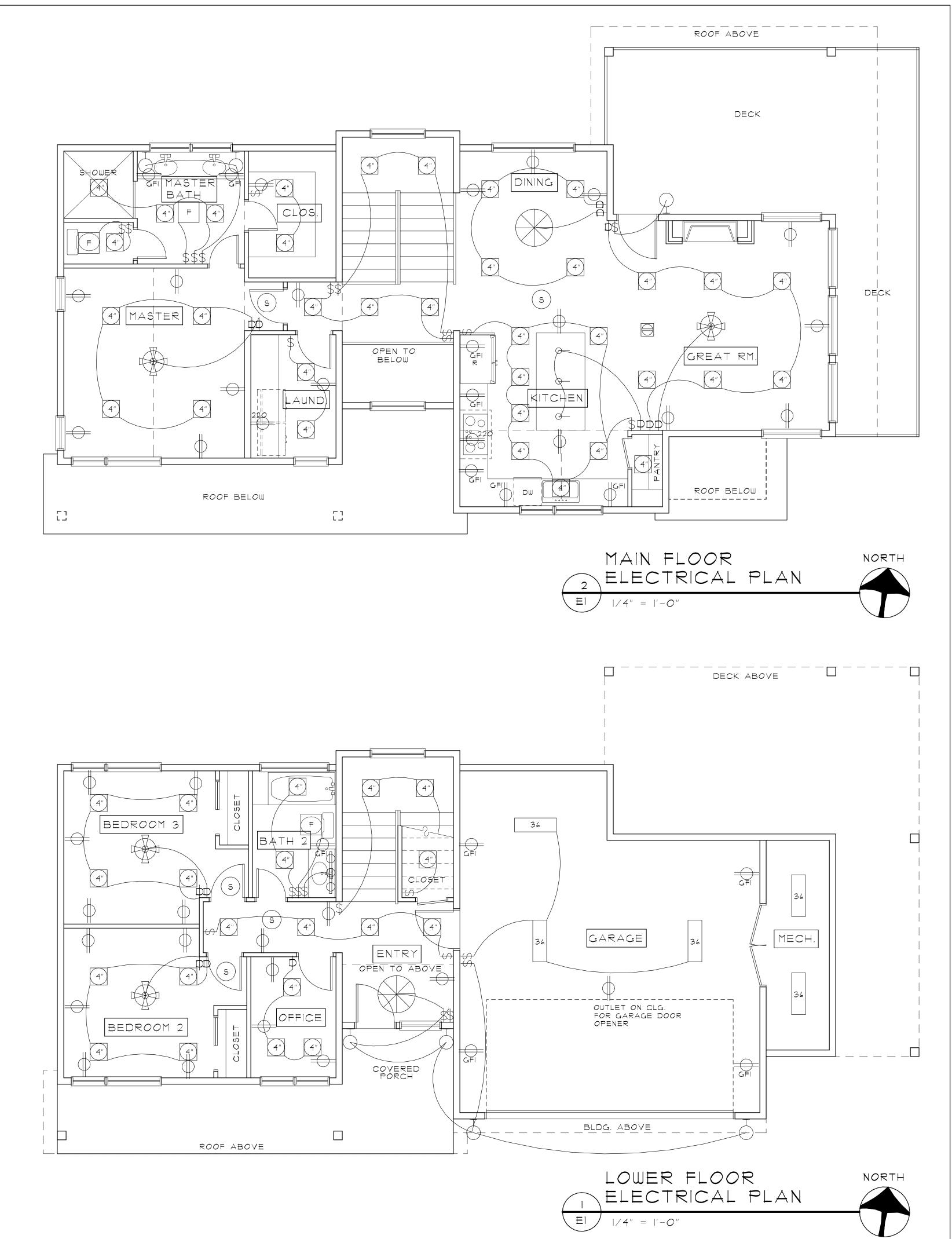
 \triangle \neg

ELECTRICAL SYMBOLS RECESSED CAN LIGHT CEILING MOUNTED FLOUR. FIXTURE (SPECIFY SIZE LIGHT FIXTURE (SPECIFY SIZE IN INCHES) IN INCHES) RECESSED CAN LIGHT FIXTURE DIRECTIONAL FLUSH CEILING MOUNTED FOR ART LIGHT FIXTURE RECESSED CAN LIGHT CEILING MOUNTED WEATHER PROOF PENDANT TASK LIGHTING CHANDELIER CEILING FAN WITH LIGHT LIGHT FIXTURE CEILING MOUNTED WALL MOUNTED SCONCE BATH VENT WALL MOUNTED VANITY UNDER COUNTER LIGHT FIXTURE FLOUR, LIGHT FIXTURE DOUBLE RECEPTACLE CLOS. WALL LIGHT MOTION SENSOR FIREPLACE FAN DOUBLE RECEPTACLE GFI OUTLET W/ GROUND FAULT INTERRUPTER HARD WIRED CO/SMOKE DETECTOR 220 VOLT RECEPTACLE 220 OUTLET GARBAGE DISPOSAL W/ AIR SWITCH SPECIAL PURPOSE OUTLET (DW, MICRO, REF) RECEPTACLE FOR × CHRISTMAS LIGHTS SWITCH, SINGLE WIRING DIMMER, SINGLE DUPLEX FLOOR

*ALL ELECTRICAL TO BE IN ACCORDANCE WITH 2020 NEC CODE REQUIREMENTS.

STAIR LIGHTING

OUTLET





PO BOX 65 FRISCO, CO 80443 970-333-1210 BLUESKYARCHITECTURE.NET

ELEC. PLANS

ISSUE DATE
PERMIT 8/6/24

Εl

OF I

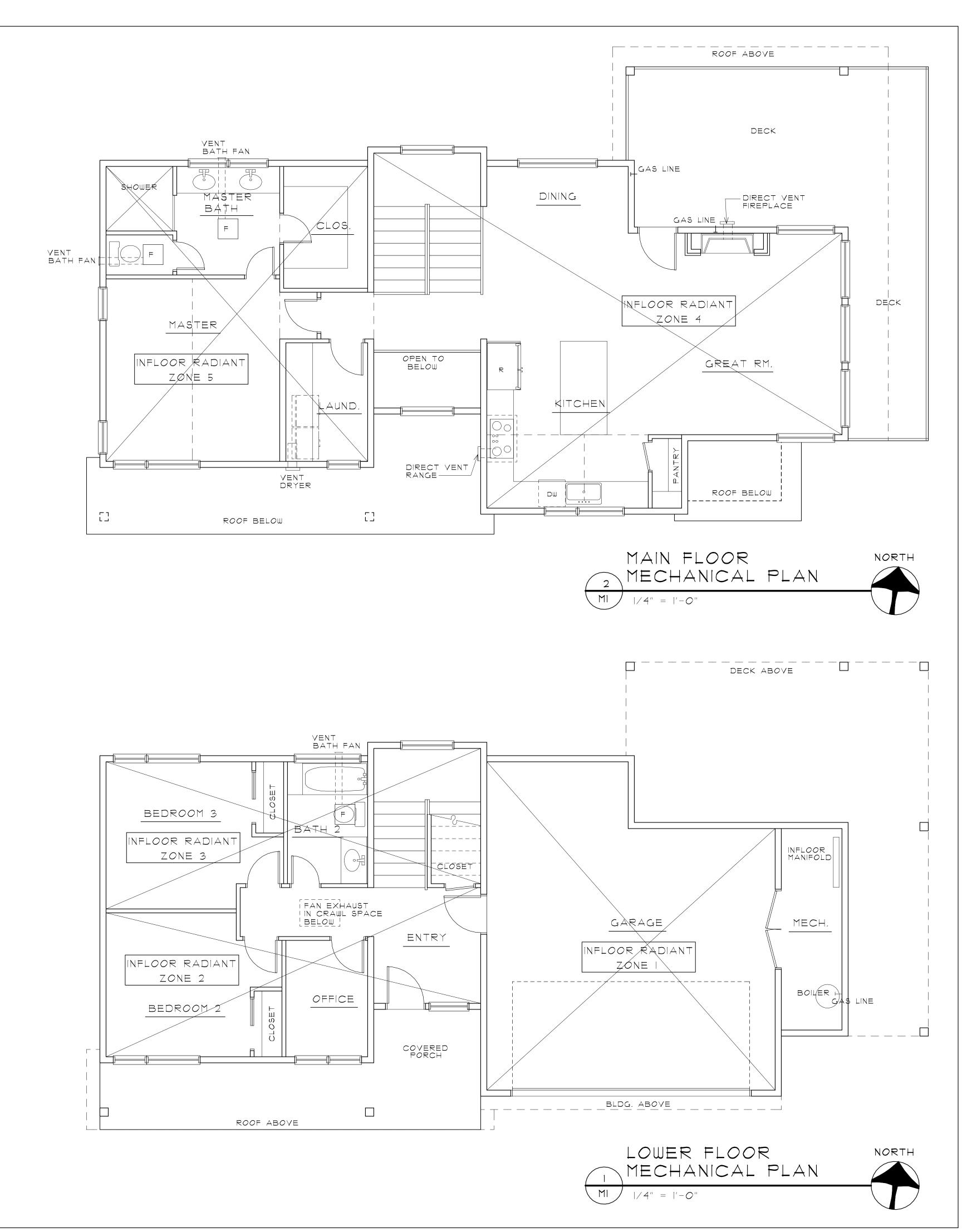
MECHANICAL SYSTEM DESIGN

- I. IN-FLOOR RADIANT HEAT WITH A TRIANGLE TUBE INSTINCT IIO MBH 95% EFFICIENCY WALL MOUNT BOILER WITH 80 GALLON SIDEARM WATER TANK.
- 2. EQUIPMENT AND SYSTEM CONTROLS: HONEY WELL CONTROL BOARDS WITH HONEY WELL LCD PROGRAMMABLE THERMOSTATS
- 3. DUCTS WITH HAVE ALL SEAMS FOIL TAPED
- 4. 1/2" PIPE INSULATION ON ALL HOT AND COLD DOMESTIC WATER LINES AND HEAT LINE MAINS.
- 5. WHOLE HOUSE VENTILATION SYSTEM TO INCLUDE:
 -OUTDOOR AIR AT A CONTINUOUS RATE OF 95 CFM. (2,242 SF W/ 3 BEDROOMS.)
 -LOCAL EXHAUST RATES FOR KITCHENS TO BE 100 CFM INTERMITTENT OR 25 CFM CONT.
 -LOCAL EXHAUST RATES FOR BATHROOMS TO BE MECHANICAL EXHAUST CAPACITY OF 50 CFM INTERMITTENT OR 20 CFM CONTINUOUS

*ALL ELECTRICAL TO BE IN ACCORDANCE WITH 2020 NEC CODE REQUIREMENTS.

*ALL COMBUSTION AIR FOR APPLIANCES TO BE DRAWN FROM OUTSIDE OF BLDG.

*MECHANICAL CONTRACTOR TO COORDINATE WITH ELECTRICAL CONTRACTOR LOCATION AND SIZE OF CIRCUITS REQUIRED



BLUESKY PO BOX 65

PO BOX 65 FRISCO, CO 80443 970-333-1210 BLUESKYARCHITECTURE.NET

THUSION WEST

MECH.
PLANS

ISSUE DATE
PERMIT 8/6/24



OF I

general structural notes

DESIGN LIVE LOADS:

Governing Jurisdiction: Blue River, Colorado

Design loads are per the 2018 International Residential Code (IRC / the Code) unless noted otherwise.

 Risk category (from ASCE 7 Table 1-1) Floors

 Exterior Decks 125psf Ground Snow 100 psf Wind speed (V_{IJII}) 115 mph, Exposure C Seismic Category B

STRUCTURAL ERECTION, BRACING, OBSERVATION, and SHOP DRAWINGS

• The structural drawings illustrate the completed structure with all elements in their final positions, properly supported and braced. The Contractor, in the proper sequence, shall provide proper shoring and bracing as may be required to achieve the final

- These plans have been engineered for construction at one specific building site by 40th Parallel Structural Engineering (Engineer of Record — "EOR"). Builder assumes ALL responsibility for use of these plans at Any Other building site. Plans shall not be used for construction at any other building site without specific review by the engineer.
- Observations of foundation reinforcing, or framing required by the Owner, lender, insurer, building department or any other party will be accomplished by the engineer at the Owner's expense. At least 48 hours advance notice is requested
- Contractor is responsible for complying with all applicable Special Inspections requirements of the Code. Special Inspections and Testing shall be performed by a qualified Special Inspector, retained by the Owner, in accordance with the applicable
- sections of IBC Chapter 17. • Fabricator and /or supplier of structural components (such as structural steel) and performance-specified components (such as prefabricated wood trusses) shall submit shop and erection drawings for architect and engineer review. Submit PDF files for each
- drawing to the architect and engineer. Allow five working days for review. • DEFLECTION TOLERANCE: Unless noted otherwise, beams/headers are designed for the code minimum deflection criteria. General Contractor to notify EOR in writing of any Window/Door openings (or other finish elements) that require a stricter deflection tolerance.

<u>DEFERRED SUBMITTALS:</u>

• Non-structural items that the building official requires to be reviewed or designed by the "engineer of record" and that are not included in these drawings shall be submitted as a Deferred Submittal. Documents for deferred submittal shall be submitted to EOR, the Architect, and the owner for review prior to construction. Common Deferred Submittal items are: Stair Stringer design and connections, Handrail and rail post at floor openings and raised decks, Solar Racking System, and connections to structure. EOR's review of these items will be for structural support only, based on the current IRC and IBC code Live Loads.

SOILS AND FOUNDATION

• Soils Report: 24-1121 by Best Engineering Solutions and Technologies dated June 29, 2024.

- The soils report is hereby referenced, and all recommendations and precautions contained in that report shall be adhered to by the Owner and Contractor except where otherwise specifically noted.
- Spread footings shall be as follows except as noted on plans:
- * Maximum allowable soil bearing pressure: 2000 psf
- * Place footings on undisturbed natural soil or compacted structural fill tested and approved by the soils engineer. * Open Hole Observation shall be performed by the Soils Engineer to confirm soil bearing conditions prior to footing construction. See Soils Report for any over-excavation/compaction requirements. Provide a copy of the Open Hole Observation Report to
- EOR. The cost of soils engineering shall be at the Owner's expense * Center all continuous footings under foundation walls and isolated footings under piers or columns unless noted otherwise.
- * Minimum frost depth = 40" from finished grade to bottom of footing unless noted otherwise on the plans.
- Design lateral soil pressures (equivalent fluid pressures) are as follows:
- * At-Rest Pressure: 50 pcf.
- * Active Pressure: 45 pcf.
- Slabs must be in place prior to backfilling around basements, or the contractor shall provide adequate shoring and bracing
- Provide perimeter drain system per soils engineer recommendations, unless specifically not required by soils engineer. Extend perimeter drain to daylight or sump. All drain systems and foundation damp-proofing shall be inspected by the soil engineer for compliance with their recommendations.
- Non-structural slabs on grade shall be by others but shall not be less than 4" deep welded wire fabric or #4 @ 18" each way in accordance with the soils engineer and shall be separated from the foundation with slip joints and from framing and other structure to allow free-floating movement of the slab with soil.
- Placement of any significant thickness of fill beneath footings, slabs, and pavement should be tested to verify proper compaction is obtained in accordance with the soils engineer's recommendations. Backfill all but the top two feet (2'-0) of all basement and site retaining walls with free draining granular material except where backfill with on-site soils is specifically allowed on plans.
- Slope compacted grade away from building per soils report, or minimum 1:10 slope for first 10 ft.

CONCRETE AND REINFORCEMENT:

- Concrete shall conform to applicable provisions of ACI 301 and 318.
- All concrete shall have 3500 psi compressive strength at 28 days (f'c).
- All cement shall be Type I/II. Concrete with High Weathering Potential such as porches, carport slabs, and garage floor slabs shall be air entrained (air content between 5% and 7%) per R402.2.
- Concrete Exposed to High Sulfates shall be TYPE V cement or any hydraulic cement meeting the requirements of Type HS per ASTM C595, W/C ratio of .45 max
- All rebar shall be ASTM A615 grade 60 fabricated and placed per ACl Manual of Standard Practice (ACl 315) and Welded Wire Fabric (WWF) shall be ASTM A185. Keep reinforcement clean and free of dirt, oil, and scale.
- Welded Wire Fabric: 4x4-W2.9 x W2.9 ASTM-A1064 Grade 70 Weldable rebar shall be ASTM A706 rebar and is typically stamped with a "w" - rebar welded to the embed plate without a
- "W" stamp will be rejected and may delay the concrete pour. • Place rebar 3" clear of soil and 1½" clear of forms except as noted. Splice bars 50 diameters (18" for #3, 24" for #4,
- 32" for #5). Add (2) #5 or (3) #4 bars around openings with 32" straight extensions at corners of the opening. Place concrete continuously without horizontal cold joints.
- Unless noted otherwise, threaded rods, rebar dowels, and similar anchors noted on drawings as fastened to concrete with epoxy shall have specified embedment in a hole 1/8" larger in diameter than the anchor, prepared and installed in accordance with manufacturer's installation instructions, using Simpson "SET3G" adhesive (ESR-4057). Clean the hole with a steel wire brush and compressed air per the manufacturer's recommendations. Temperatures must be above 40° for 48
- hour period for installation unless noted by the manufacturer. • A Concrete Encased Electrode (CEE, or "Ufer rod") is required and shall be field located near the electrical service by the contractor. The CEE shall be per governing jurisdiction and per the NEC code, 40TH PARALLEL can observe the CEE if required by the building official but does not accept responsibility for its design or detailing; the contractor shall notify
- COLD WEATHER CONCRETE PLACEMENT: * Concrete may be poured with no protection if the average air temperature is above 40°F for the next 3 days. Average air
- temperature is defined as the average between the daily high and low temps. * No concrete is allowed to be poured when the average air temperature is below 25%.
- * When the air temperature is below 40F, the concrete temperature at placement should be 65F to 85F
- * Contractor to use blankets, windbreaks, higher strength concrete, and accelerating and/or water-reducing admixtures per ASTM C 494 C & E as required to protect concrete from freezing during curing.
- * No Chloride Admixtures shall be added to concrete without 40th Parallel Structural Engineering's written approval

STRUCTURAL STEEL:

Steel material shall be as follows unless specifically noted otherwise:

40TH PARALLEL of the applicable CEE requirements.

- * Structural Beams.: ASTM A992 * Angles, misc.: ASTM A36
- * Anchor Bolts: ASTM A307 or A36 with a minimum 7" embedment depth unless noted * Standard pipe columns: ASTM A 53, Grade B, 35 ksi.
- * Tube steel (HSS): ASTM A500, Grade B, 46 ksi
- * Connector bolts: ASTM A325 FOR STEEL TO STEEL, ASTM A307 FOR WOOD TO STEEL
- * Expansion Anchors Simpson Strong-Bolt 2 or equivalent • All structural steel shall be fabricated and erected per the current edition of AISC Steel Construction Manual.
- Welding shall be by qualified welders. Use E70XX electrodes and 3/16" fillet welds unless noted otherwise.
- Adjustable caps <u>NOT</u> allowed on columns UNO on plans.
- Non-shrink grout beneath column base and beam bearing plates shall be non-metallic with minimum compressive strength of Steel Beam Framing:
- * Typical Wood Nailer: 2x nailer to match beam width. Connect to the top flange with construction adhesive and ½" diameter thru-bolts at 24"oc, staggered.
- * At flush steel beams which receive top mounted hangers, rip 2x nailer to exact beam width plus 3/8" and attach to top flange with construction adhesive and 1/2" diameter machine bolts @ 32", staggered. Plate shall overhang beam flange (at least 1/8" but not more than 1/4") on both sides to prevent hangers from contacting steel beam. Set joists in hangers with adhesive.
- * At flush steel beams which receive top mounted hangers, rip 2x nailer to exact beam width plus 3/8" Plate shall overhang
- beam flange (at least 1/8" but not more than 1/4") on both sides to prevent hangers from contacting steel beam. * Blocked steel beam webs: Block beam web with solid 2x blocking, bear tight to bottom flange, and glue & bolt to flange with 1/2" thru-bolts at 24"oc unless noted otherwise. Use a minimum of 3 - 1/2" thru-bolts.
- * At dropped steel beams bearing on built-up studs, bearing beams on stud end grain. Install 2x6 vertical blocking between beam flanges and nail king studs thereto with at least (6) 16d nails on each side unless noted otherwise.
- * 2 -#14 Self-Drilling Screws (TB1460 screws by SIMPSON or equivalent) may be installed in place of 1 ½" thru-bolt. Use length as required to penetrate steel member.
- * All beams shall have full-depth web stiffeners on each side of the webs above and below columns.

All framing and details not specifically specified shall comply with the prescriptive (non-engineered) requirements of the

- International Residential Code.
- Sawn Lumber and Timbers:
- * Nominal 2x and 3x lumber shall be Douglas-Fir #2 or better. * Sill plates and ledgers in contact with concrete or masonry shall be preservative treated with Micronized Copper Azole
- (Southern Yellow Pine lumber), or "Strandguard" LSL. ACQ treatment is NOT acceptable. * All deck framing lumber to be Treated Lumber (with MCA-C treatment or another equivalent), Southern Yellow Pine,
- GRADE #1, not incised.
- * Field cut ends, notches and drilled holes of pressure preservative treated wood is to be re-treated in the field in accordance with IRC Section R317.1.1 and AWPA M4.

- Engineered Wood: * Laminated Veneer Lumber (LVL): Manufactured 1-3/4" width with Fb=2,600 psi, F=2,000,000 psi, Fv=285 psi.
- * LSL Rim Joists: Manufactured 1-1/4" laminated strand lumber Trus Joist by Weyerhaeuser or equivalent.
- * Glued, laminated framing members (Glulams) per ANSI Standard A190.1-92. Mark members with an AITC Quality Stamp and furnish an AITC Certificate of Conformance. Doug Fir, Fb = 2400 psi, E = 1,800,000. Combination symbols as follows:
- * Simple span beams: 24FV4 with ZERO camber or 2400 foot camber or ZERO camber except as noted on plan.
- * Multiple span beams, continuous beams, and cantilevered beams: 24FV8, ZERO camber. * Columns: Combination #2.
- * Alaskan Yellow Cedar Glu-lam beams, Fb = 2000 psi, E = 1,500,000, 20F-V12 unless noted otherwise.
- * Treated LVL to be "PWT treated" 2.0E, 2800 Fb LVL (1 ¾" wide UNO) Plywood and OSB Sheathing:
- * Floor sheathing shall be T&G Sturd-I-Floor (23/32" min) with 24" oc APA rating. Lay panels perpendicular to framing members and stagger joints with at least 2-spans per panel. Glue and nail with 10d ring-shank nails @ 6" along edges
- * Wall sheathing shall be 7/16" minimum sheathing with 24/16 APA rating. Block unsupported edges and nail sheathing to all studs, plates, and nails with 10d nails @ 6" along edges & @ 12" in fields. Note additional requirements for shear walls as shown on the plans.
- * "Roof" sheathing shall be 19/32" minimum with 40/20 APA rating. Lay panels perpendicular to framing members and stagger joints with at least 2-spans per panel. Nail with 10d ring-shank nails @ 6" along edges & @ 12" in fields, UNO. Wall, Floor, and Roof Framing:
- * Anchor all roof rafters, joists, and trusses to beams and walls at bearing points with metal framing anchors, doubled within 4'-0" of corners and at hips. Install full height blocking between rafters/joists/trusses at all bearings unless noted
- * Exterior walls shall be 2x6 @ 16" unless noted otherwise on the plans. All wall studs shall be continuous from floor to
- * All wood posts and columns shall be supported with posts of equal size at all walls below and with squash blocking in all platform levels to transfer the load to the foundation.
- * All prefabricated plywood Web |-type joists shall be installed per the manufacturer's recommendations. See manufacturer's literature for web-stiffener installation, and minimum connections at bearing locations. Do not cut or notch chords in any
- manner. Holes in the joist webs shall not exceed the manufacturer's published limit criteria. * Pre-engineered I-Joists shall be continuous over intermediate bearings as is possible and shown on the plans. Block between joists under bearing walls and over interior shear walls.
- * Layout joists to avoid plumbing and other floor penetrations. * All beams shall be braced against rotation at points of bearing. Dry pack grout all beam pockets in concrete full after beams
- are set. Wrap wood beams with ice and water shield in beam pockets. * At built-up stud columns, nail all laminations with 16d @ 12" full height, staggered.
- * At dropped wood beams bearing on built-up studs, bear beams on stud end grain. Form a pocket by extending king studs to the wall top plate on each side of the beam. Nail king studs to beam with at least (6) 16d nails each side unless noted

Roof Trusses:

FASTENERS:

otherwise.

- * Pre-engineered, prefabricated trusses shall be designed for the fabricator by a Professional Engineer Registered in the State of construction and shall comply with the Code and the Truss Plate Institute Requirements. Manufacture and installation of trusses shall comply with ANSI/TPI 1 "National Design Standard for Metal-Plate-Connected Wood Truss Construction", TPI HIB "Commentary and Recommendations for Handling Installing and Bracing Metal Plate Connected Wood Trusses", and TPI DSB "Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses".
- * Trusses shall be designed for loads per plan. * Maximum allowable deflections shall be as follows:
- * Truss Maximum deflection under full Live load: Span / 360 * Truss Maximum deflection under full Dead + Live load: Span / 240 or ¾", whichever is lesser * Lower chord of gable end trusses shall be anchored to wall plate with framing anchors at 6'-0 on center and laterally braced
- to roof framing at 6'-0 on center maximum spacing, or as required by the fabricator. * Solid block between trusses at bearings.
- * Unless otherwise indicated, trusses shall be designed for perpendicular to grain bearing on Hem Fir plates (405 psi). End grain bearing is not allowed unless accepted in writing by EOR. Design truss bearings for bearing blocks or Truss Bearing Enhancers
- as required to compensate for overstresses. Specify size, species, and nailing for bearing blocks. * All truss-to-truss connections shall be specified by the truss supplier unless specifically noted on the drawings.

WOOD FRAMING: HARDWARE, CONNECTORS, AND FASTENERS:

- Metal connectors shall be by Simpson Strong—Tie and installed with nailing to achieve maximum rated capacity unless noted otherwise. Note that heavy-duty and skewed hangers may require a special order. See the current Simpson catalog or "Installer's Pocket Guide" for required nailing. NOTE THAT MOST HEAVY HANGERS REQUIRE 16d COMMON NAILS (.162x31/y"). "Sinkers", 12d common nails, and short "hanger nails" are NOT acceptable and WLL HAVE TO BE PULLED AND REPLACED.
- All connectors shall meet the recommendations of the pressure-treated wood manufacturer. Connectors in exposed applications shall not be less than Hot Dipped Galvanized (HDG) or Stainless Steel (SS). All screws, nails, and bolts shall match the material and coating of hangers and other connectors. DO NOT mix stainless with
- galvanized products. • Straps shall not be installed with fasteners into end grain in 2x members or any narrow face of LVL members.
- * Nails designated as "8d" nails on the plan, shall be 8d ring shank gun nails (0.113" diameter x 2 ½" long) unless noted
- * Nails designated as "10d" nails on the plan, shall be 10d ring shank gun nails (0.131" diameter x 2 ½" long) unless noted * Framing nails in 2x lumber shall be 12d common nails (0.131" diameter x 3 ¼" long) unless noted otherwise. These nails
- are commonly referred to as "short sixteens" or "16d gun nails". Nails called out as "16d" on plan shall be 12d
- * 16d Common nails shall be 0.162" diameter x 3 ½" long and may not be substituted. * "Ramset Pins" indicate .145" diameter powder actuated drive pins. Use appropriate length to penetrate steel material per
- * Epoxy Anchors in wood: Unless noted otherwise, threaded rods fastened to timbers with epoxy shall have specified embedment in a hole 1/8" larger in diameter than the anchor. Clean out the hole with a steel wire brush and compressed air, and fill the
- hole 2/3 full with Simpson "SET" adhesive prior to installing the rod. * SDS SCREWS -1/4" screws with length indicated, per SIMPSON STRONG TIE. NO SUBSTITUTIONS.
- * TIMBERLOK or LEDGERLOK Screws per Fastenmaster, pre-drilling w/ 1/8" bit is acceptable if required. Substituting SDS screws of the same length is acceptable.
- * Lead holes for lag screws shall be 60% to 70% of lag shank diameter in compliance with AITC criteria



40th Parallel Structural Engineering 263 2nd Avenue Suite 106A Niwot, Colorado

PM: Dana Michel, PE Email: dana@40thParallelSE.com PM: Daniel Belleau, PE Email: daniel@40thParallelSE.com

S

S Ш ш $\mathbf{\Omega}$ S III ()A E E E E 6WI ∞

DBM24118 8/6/2024 DBM DESIGN BY REVSIONS: No. Description

 \bigcirc



40PDBM24118_2024.08.06

SHEET NO.

Secure ESeal#

CONNECTION POINT FOR CONCRETE ENCASED ELECTRODE ("UFER GROUND") PER CONTRACTOR/ARCH. UFER GROUND TO BE INSTALLED

PER NEC OR CITY CODE REQUIREMENTS, BY FOUNDATION SUB

OBSERVE DURING SITE VISIT.

<u>TYP SLAB-ON-GRADE:</u>

CONTRACTOR. CONTRACTOR REMIND BLDG INSPECTOR/ENGINEER TO

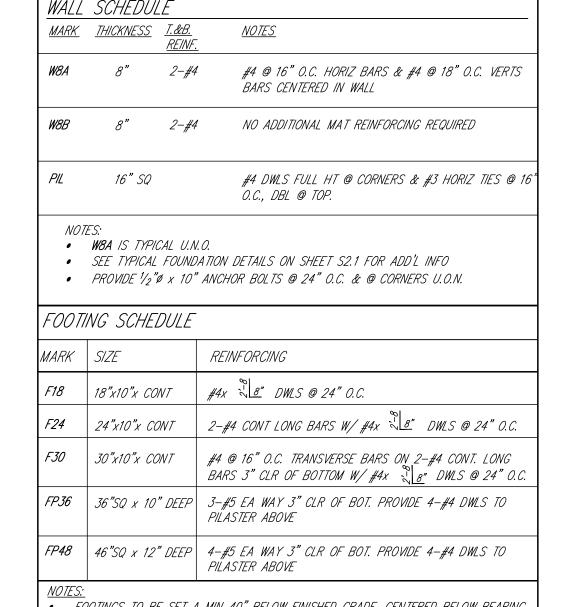
4" MIN. CONCRETE SLAB-ON-GRADE W/ 6x6-W2.9 x W2.9 W.W.F. ON COMPACTED STRUCTURAL FILL PER SOILS REPORT. FOR BETTER CRACK

CONTROL, USE 5" SLAB W/ #4 @ 18 EA WAY, 3" CLR BOTTOM. CUT OR

TROWEL CRACK CONTROL JOINTS AT 12' MAX EA WAY W/ 100 SQ FT MAX

JOINT-FREE AREAS. PROVIDE VAPOR BARRIER & RADON MITIGATION PER

<u>STEEL COLUMN SCHEDULE & CAP PL & BASE PL "BP"</u> SCOL3.5 — HSS3 ½"x3 ½"x½, STL COL W/ CAP PL PER **50/S2.3**



MAIN FLOOR FRAMING & FOUNDATION PLAN

1/4" = 1'-0

• FLOOR SHTG TO BE MIN 3/4" T. & G. APA RATED SHTG W/ 10d NAILS @ 6"o.c. @ EDGES & 12"o.c. IN

• PROVIDE 1 1/4" x JST DEPTH LSL RIM MAT'L @ FLOOR PERIM. CONT. U.O.N. • "11J-1" TO BE 11⁷/8 BCI 6000 SERIES OR EQUIVALENT

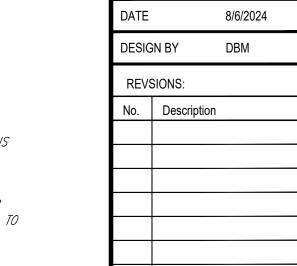
1) • PROVIDE MIN. 2-2x6 VERT. BLKG (SQUASH BLKG) THRU - FLOOR SYSTEM @ COL ABV LOCS. NOTED THUS • <u>TREATED "TRTD LVL"</u> TO BE "PWT TREATED" 2.0E, 2800 FB LVL DECK JOIST AND BEAMS • FLOAT NON-LOAD BRG PARTITION WALLS ABOVE SLAB ON GRADE PER GEOTECH. ENGR.

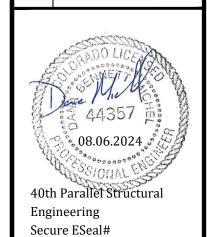
 DO NOT BACKFILL BASEMENT WALLS PRIOR TO BASEMENT SLAB INSTALLATION • INDICATES STHD14RJ HOLDDOWN @ 2-2x6 STUDS ABOVE. INSTALL TYPICAL FOUNDATION DETAILS & PER MANUF RECOMMENDATIONS W/ FULL 16d NAILING. CONTRACTOR TO VERIFY LOCS W/ ARCH DWGS PRIOR TO CONCRETE POUR. RE: 6/S2.1

• INDICATES STEP TOP OF WALL (T.O.W.)

• SEE S1.0 FOR GENERAL STRUCTURAL NOTES

△ STEEL EMBED PL. RE: 65/S2.3 EX BOTTOM OF WALL (B.O.W.) = TOP OF FOOTING (T.O.FTG.)
TOP OF STEEL BMS NOTED THUS (ELEV)





40PDBM24118_2024.08.06

40th Parallel Structural Engineering 263 2nd Avenue Suite 106A

Fm. Dana midnet, FE Email: dana@40thParallelSE.com PM: Daniel Belleau, PE Email:daniel@40thParallelSE.com

RIVER

 \mathbf{m}

BLUE RIVER, (

Ш

186WILDE

0

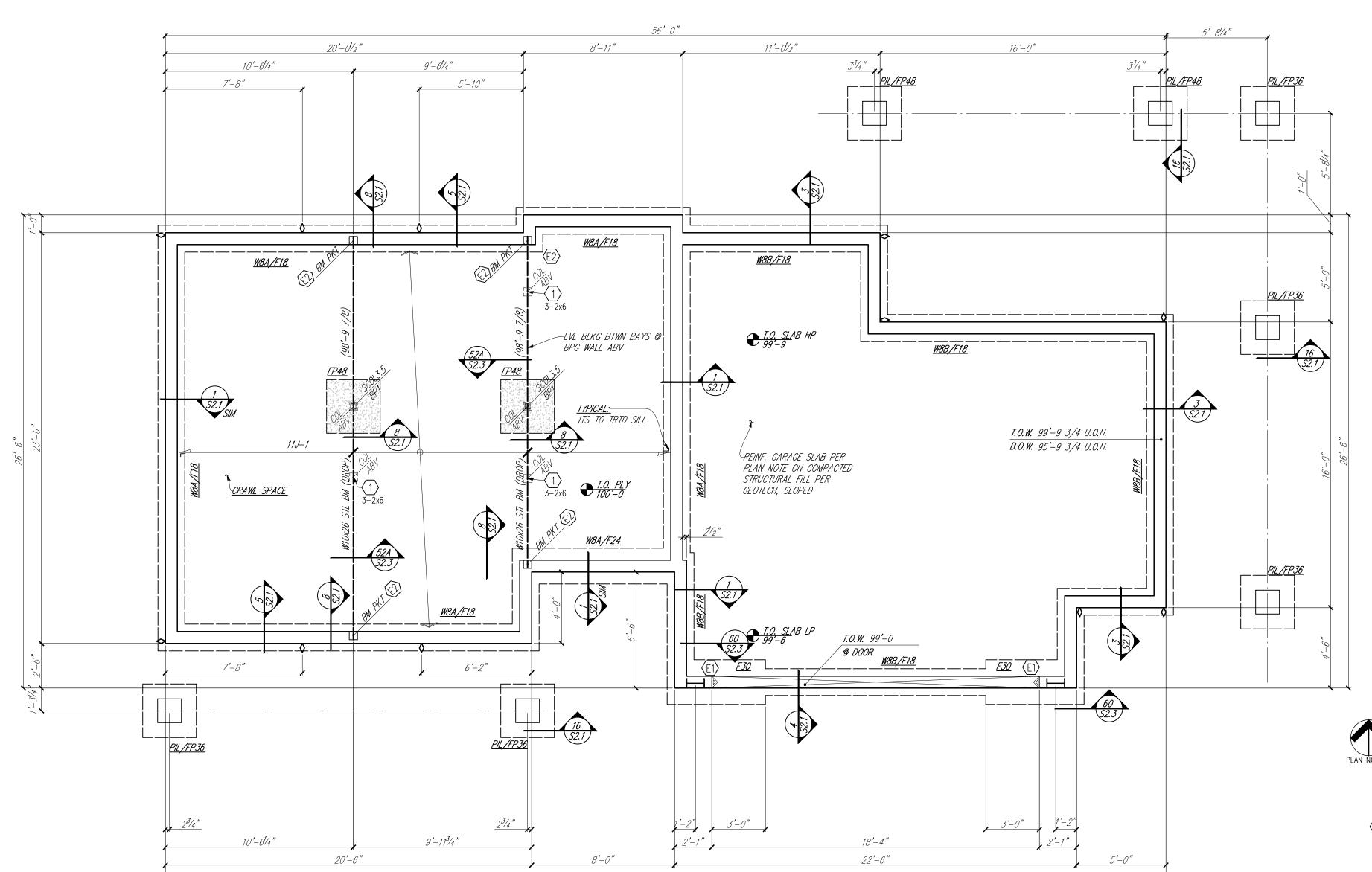
DBM24118

AND

Niwot, Colorado

PM: Dana Michel, PE

SHEET NO. S1.1



• 2x TRTD DECK JST

• LVL BM TO CONC. WALL (**LOCS) LVL BEAMS TO FLUSH BM

• Metal connectors shall be by Simpson Strong Tie and installed with nailing to achieve maximum rated capacity unless noted otherwise. Note that heavy duty and skewed hangers may require special order. See current Simpson catalog or "Installer's Pocket Guide" for required nailing. NOTE THAT MOST HEAVY HANGERS REQUIRE 16d COMMON NAILS (.162x3½"). "Sinkers", 12d common nails, and short "hanger nails" are NOT acceptable and WILL HAVE TO BE PULLED AND REPLACED.

STEEL FRAMING NOTES

B1) TYPICAL @ STL BMS NAILER: RE: 52A/S23

B2 TYPICAL @ BLOCKED BEAM WEB: RE:52B/S2.3

BP1 – BASE PLATE PER **55/S2.3**

<u>MARK</u>	<u>THICKNESS</u>	<u>T.&B.</u> REINF.	<u>NOTES</u>
W8A	8"	2-#4	#4 @ 16" O.C. HORIZ BARS & #4 @ 18" O.C. VERTS BARS CENTERED IN WALL
W8B	8"	2-#4	NO ADDITIONAL MAT REINFORCING REQUIRED
PIL	16" SQ		#4 DWLS FULL HT @ CORNERS & #3 HORIZ TIES @ O.C., DBL @ TOP.
•		FOUNDATION Ø x 10" ANC	N DETAILS ON SHEET S2.1 FOR ADD'L INFO HOR BOLTS @ 24" O.C. & @ CORNERS U.O.N.
MARK	SIZE		EINFORCING
F18	18"x10"x CO	NT #4.	x ~ DWLS @ 24" O.C.
			σ.I

• FOOTINGS TO BE SET A MIN 40" BELOW FINISHED GRADE, CENTERED BELOW BEARING ELEMENTS, ON COMPACTED STRUCTURAL FILL OR UNDISTURBED SOIL REVIEWED & APPROVED BY GEOTECHNICAL ENGINEER OF RECORD. • F18 WALL FOOTING IS TYPICAL U.O.N.

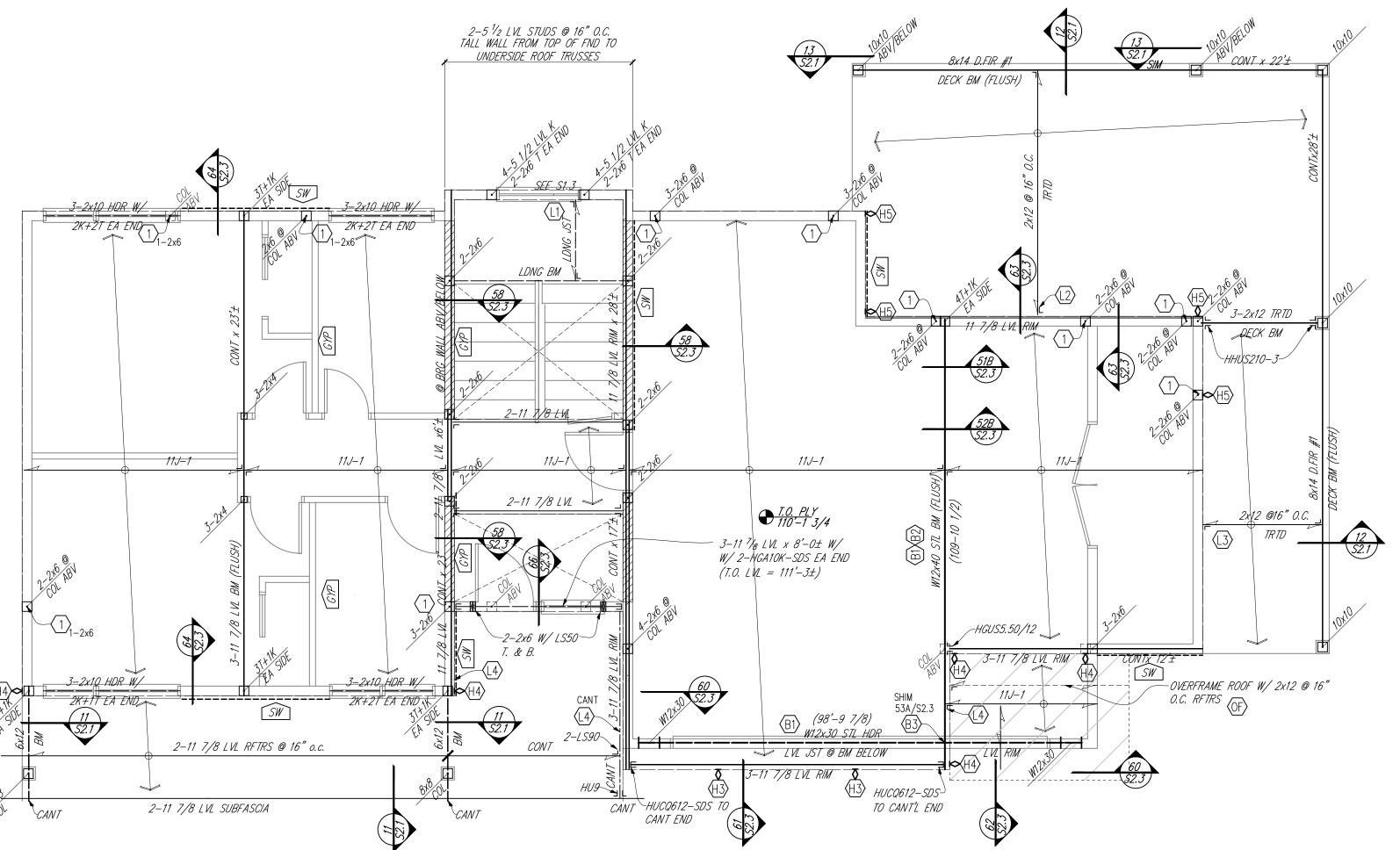
• CONTRACTOR TO VERIFY MIN. 2000 PSF SOIL BEARING CAPACITY W/ A QUALIFIED SOILS ENGINEER & 55 PCF LATER FLUID PRESSURE PRIOR TO FTG CONSTRUCTION.

ELEV 100'-0 = 10073.0' USGS

LEDGER SCHEDULE

- $\langle L1 \rangle$ 2x10 LEDGER W/ 2-1/4" ϕ x3 $^{\prime}$ / $_2$ " SDS SCREWS @ 16" O.C.. FLUSH FRAME LNDG JSTS W/ LS70.
- (L2) 2x12 TRTD LEDGER W/4-1/4"øx4" SDS SCREWS @ 8"O.C. FLUSH FRAME JST W/LUS212
- 2x12 TRTD LEDGER W/ 3-1/4"øx4" SDS SCREWS @ 16" O.C. FLUSH FRAME JST W/ LUS212
- (L4) 11 ⁷/₈ LVL LEDGER W/ 3-1/4"øx4" SDS SCREWS @ 16" O.C. TRHU WALL SHTG TO EA STUD. FLUSH FRAME RFTRS W/ 2-LS90.(6 SCREWS) @ CORNER & CANTILEVER TO SUBFASCIA @ LOCS
- (L5) LET IN 2x RFTRS & BEAR ON 2x STUB COL. SISTER TO TRUSSES PER XX/S2.x

SHEAR WALL SCHEDULE	TIE DOWN & STRAP SCHEDULE			
TYPE MARK NAILING: NOTES:	► ⟨H1⟩ STHD14RJ HOLDDOWN @ 2−2x6 STUDS ABOVE. INSTALL TYPICAL FOUNDATION DETAILS & PER MANUF RECOMMENDATIONS W/ FULL 16d NAILING. CONTRACTOR TO VERIFY LOCS W/ ARCHDWGS PRIOR TO CONCRETE			
"PLY" TYPICAL @ 10d @ 6"oc @ HX VERTICAL STRAP AT ENDS PER SCHD EXTERIOR WALLS PANEL EDGES & 12" U.O.N. O.C. IN FIELDS	POUR. RE: 6/S2.1			
"PLY" @ LOCS SW PANEL EDGES & 12" O.C. IN FIELDS SW PANEL EDGES & 12"	■ (H2) HDU5-SDS HOLD DOWN TO ⁵ / ₈ "ø ALL THREAD SET IN ³ / ₄ "ø HOLE. CLEANED, BRUSHED & FILLED W/ SIMPSON "SET 3G" EPOXY PER MANUF. <u>SPECIAL INSPECTION</u> REQUIRED RE: 7/S2.1 ■ (H3) 2-ST22 VERTICAL TIE DOWN FROM FLOOR RIM BM TO 2-2x STUDS @ ENDS OF SHEAR WALL ABOVE			
"GYP" @ LOCS GYP SCREWS @ 4" oc @ PANEL EDGES & 4" O.C. IN FIELDS	► ⟨H4⟩ MSTC66B3 VERT STRAP HOOKED TO UNDERSIDE OF TOP FLANGE OF STL BM W/1/8" FILLET WELD 2 SIDES.			
FRAMING: 2x6 @ 16" o.c. HEM FIR #2 OR BETTER W/ 2x6 VERTICAL BLOCKING @ ALL ALL UNSUPPORTED PANEL EDGES SHEATHING: "PLY" - 7/16" APA RATED OSB OR PLY SHTG "GYP - 5/8" DRYWALL SHEATHING NAILS: "10d" = 0.131"øx 2 1/2" MIN W/ SPACING @ PANEL EDGES PER SCHEDULE & @ 8" O.C. IN FIELDS. INCLUDING FULL LENGTH OF KING STUDS @ OPENINGS. LAP FLOOR RIMS 3" MIN U.O.N. NAIL HEADS SHALL BE DRIVEN FLUSH WITHOUT BREAKING SURFACE OF SHTG; NON-CONFORMING SHTG & NAILING SHALL BE REPLACED. GYP "SCREWS": #6x1 1 7/8" TYPE S OR TYPE W/ DRYWALL SCREWS STEEL STRAPS: APPLY STRAPS OVER FACE OF WALL SHTG. W/ NAILING PER SCHD. STRAP & SHEAR WALL INSPECTION: CONTACT E.O.R. OR 3RD PARTY INSPECTOR FOR VISUAL OBSERVATION PRIOR TO INSTALLING BUILDING WRAP & FINISHES. PENETRATIONS & NOTCHES: NO MECH OR PLUMBING HOLES OR NOTCHES IN SHEATHING OR IN TOP OR BTM PLATES UNLESS NOTED OR PRIOR WRITTEN APPROVAL.	- (H5) 2-CS14x5'-0 VERT STRAP FROM STUDS SHOWN TO STUDS @ END OF SHEAR WALL BELOW TO W/ 14-10d NAILS EA END & 6-NAILS TO RIIM			



TYP FRAMING MEMBERS CONNECTION SCHEDULE UNLESS NOTED OTHERWISE, CONNECT FRAMING MEMBERS AS FOLLOWS: TYP CONNECTION, UNO: MEMBER/CONDITION:

- FLOOR JST TO FLUSH BM • FLOOR JST TO SILL PL
- 2x TRTD DECK JST • LVL BM TO CONC. WALL (**LOCS) LVL BEAMS TO FLUSH BM
- RFTR (SLOPED) • RFTR TIE DOWN • ROOF TRUSS TIE DOWN
- HU SERIES W/1/4"øx2" TITEN CONC. SCREWS TO FND HHUS-SERIES HANGERS W/ 16d x 0.162"x3 1/2" NAILS LRU212-Z U.O.N. W/ 10d x 0.148"x2 1/2" NAILS HGA10K-SDS

LUS U.O.N.

H2.5A (DBL @ CORNERS) W/ 10d x 0.148"x11/2" NAILS

IUS—SERIES W/ 10d x 0.148"x3" NAILS

MIT W/ 10d x 0.148"x1 1/2" NAILS TO SILL

Metal connectors shall be by Simpson Strong Tie and installed with nailing to achieve maximum rated capacity unless noted otherwise. Note that heavy duty and skewed hangers may require special order. See current Simpson catalog or "Installer's Pocket Guide" for required nailing. NOTE THAT MOST HEAVY HANGERS REQUIRE 16d COMMON NAILS (.162x3½"). "Sinkers", 12d common nails, and short "hanger nails" are NOT acceptable and WILL HAVE TO BE PULLED AND REPLACED.

STEEL FRAMING NOTES

- (B1) <u>TYPICAL @ STL BMS NAILER: RE: 52A/S23</u>
- B2) TYPICAL @ BLOCKED BEAM WEB: RE:52B/S2.3
- B3 TYPICAL @ STL BM TO STL BM CONN NOTED: RE: 53A/S2.3 & 53B/S2.3

(B4) <u>NOT USED</u>

- B5 PRE-DRILL BM FLANGES FOR SDS CONN TO WALL PL RE: 52A/S2.3
- B6 STEEL END PL TO STUDS: RE: 56/S2.3

STEEL COLUMN SCHEDULE & CAP PL & BASE PL "BP" SCOL3.5 - HSS3 1/2"x3 1/2"x1/4 STL COL W/ CAP PL PER 50/S2.3 SCOL4 - HSS4x4x³/8" STL COL W/ CAP PL PER 50/S2.3 SCOL5 - HSS5x5x³/₈" STL COL W/ CAP PL PER 50/S2.3

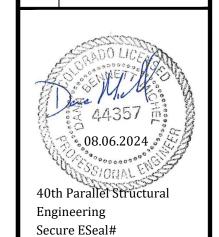
BP1 – BASE PLATE PER **55/S2.3** BP2 – BASE PLATE PER 55/S2.3

BP3 – BASE PLATE PER **55/S2.3** BP4 - WELD COL TO TOP OF STL BM PER 57/S2.3

UPPER FLOOR & LOWER ROOF FRAMING PLAN

1/4" = 1'-0

- HEADERS TO BE 2-2x10 DOUG. FIR #2 INSULATED HEADERS W/ 1-2x6 KING (K) + 1-2x6 TRIM (T) EA. END & 1-2x6 PL T. & B. OR MORE AS NOTED. RE: 54/S2.3
- WG LOWER LVL HDR (ELEVATION VARIES PER ARCH) W/ 5 1/2" LVL PL TOP & BOTTOM. PROVIDE LS50 EA END TO FULL HT KING STUDS. RE: 54/S2.3 • FLOOR SHTG TO BE MIN 3/4" T. & G. APA RATED SHTG W/ 10d NAILS @ 6"o.c. @ EDGES & 12"o.c. IN
- ROOF SHTG TO BE MIN. 5/8" APA RATED (40/20) OSB SHTG W/ 10d RINGSHANK NAILS @ 6"oc @ EDGES &
- 12"oc IN FIELDS
- PROVIDE 1 1/4" x JST DEPTH LSL RIM MAT'L @ FLOOR PERIM. CONT. U.O.N. • "11J" TO BE 11⁷/₈ BCI 6000 SERIES OR EQUIVALENT
- "RFTR" TO BE 2x12 @ 16" O.C. W/ LRU212Z U.O.N. • "LDNG JST" TO BE 2x10 @ 16" O.C. W/ LUS210 TO LEDGER
- "LDNG BM" TO BE 2-9 1/2" LVL BM
- 1) PROVIDE MIN. 2—2x6 VERT. BLKG (SQUASH BLKG) THRU FLOOR SYSTEM @ COL ABV LOCS. NOTED THUS
- <u>TREATED "TRTD LVL"</u> TO BE "PWT TREATED" 2.0E, 2800 FB LVL DECK JOIST AND BEAMS
- TOP OF STEEL AS NOTED (ELEV)
- FLOAT NON-LOAD BRG PARTITION WALLS ABOVE SLAB ON GRADE PER GEOTECH. ENGR.
- SEE S1.0 FOR GENERAL STRUCTURAL NOTES



40PDBM24118_2024.08.06

SHEET NO. S1.2

40th Parallel Structural Engineering 263 2nd Avenue Suite 106A Niwot, Colorado

PM: Dana Michel, PE Email: dana@40thParallelSE.com PM: Daniel Belleau, PE Email: daniel@40thParallelSE.com

STATE BACKL ERNESS, E BLUE RIVER, (43 186WILDE

DESIGN BY

REVSIONS:

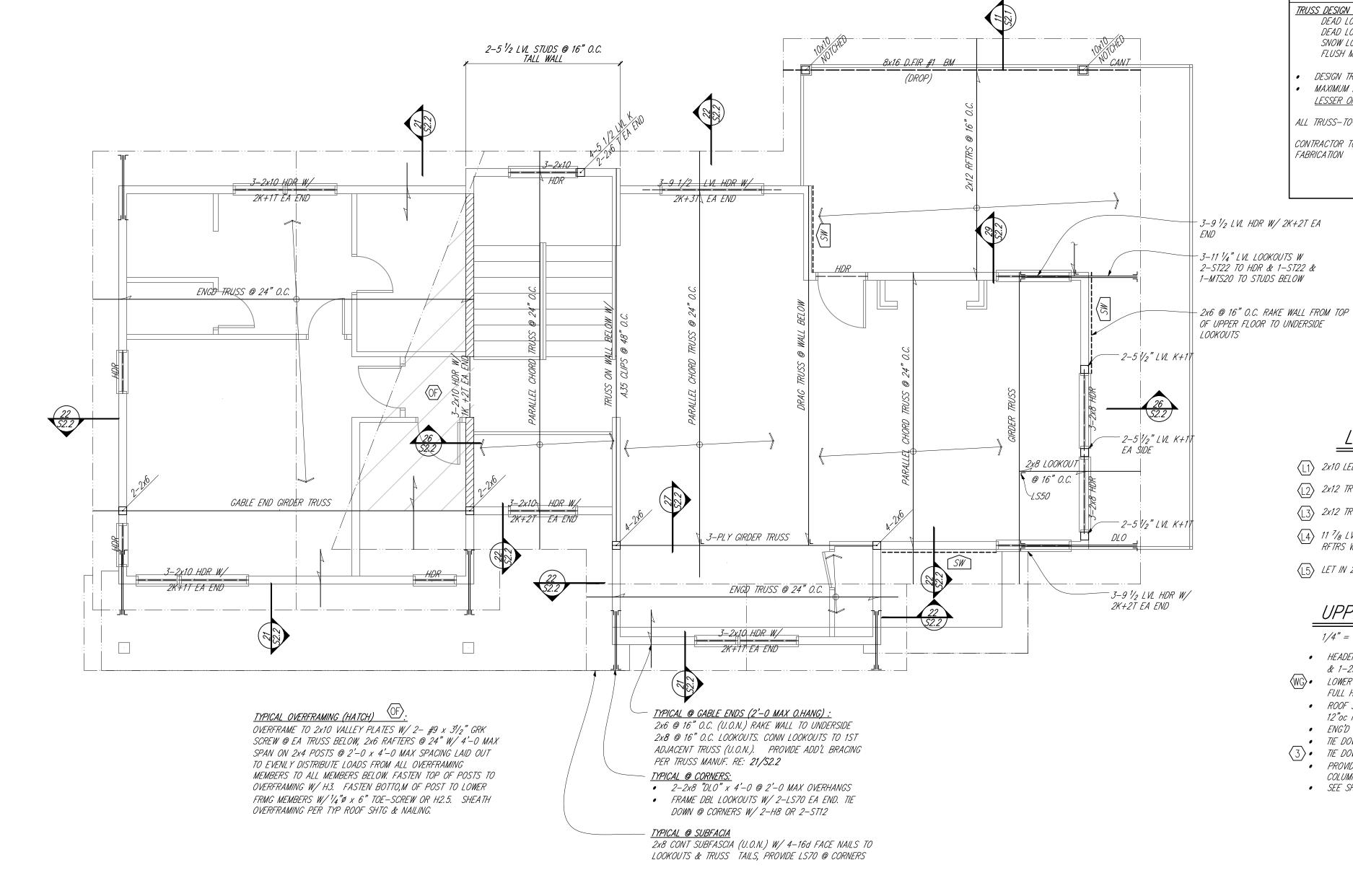
No. Description

DBM24118

8/6/2024

DBM

UNLESS NOTED OR PRIOR WRITTEN APPROVAL.



SNOW LOAD TOP CHORD =

TRUSS SUPPLIER NOTE:

TRUSS DESIGN LOADS ARE AS FOLLOWS:

DEAD LOAD TOP CHORD =

DEAD LOAD BOTTOM CHORD =

FLUSH MOUNT SOLAR PV ARRAY =

DESIGN TRUSSES FOR MAXIMUM LIVE LOAD DEFLECTION OF L/360.

MAXIMUM DEFLECTION UNDER <u>TOTAL LOAD</u> (DEAD LOAD PLUS FULL LIVE LOADS) SHALL BE THE <u>LESSER OF L/240 OR 3/4".</u>

10 PSF

10 PSF

100 PSF

3 PSF

ALL TRUSS-TO-TRUSS CONNECTIONS SHALL BE DESIGNED BY TRUSS SUPPLIER.

CONTRACTOR TO PROVIDE ENGINEERED TRUSS SHOP DRAWINGS FOR E.O.R. TO REVIEW PRIOR TO FABRICA TION

LEDGER SCHEDULE

- $\langle L1 \rangle$ 2x10 LEDGER W/ 2-1/4" ϕ x3 $^{\prime}$ /2" SDS SCREWS @ 16" O.C.. FLUSH FRAME LNDG JSTS W/ LS70.
- (2) 2x12 TRTD LEDGER W/ 4-1/4"øx4" SDS SCREWS @ 8"O.C. FLUSH FRAME JST W/ LUS212
- (3) 2x12 TRTD LEDGER W/ 3-1/4"øx4" SDS SCREWS @ 16" O.C. FLUSH FRAME JST W/ LUS212
- (L4) 11 ⁷/₈ LVL LEDGER W/ 3-1/4"Øx4" SDS SCREWS @ 16" O.C. TRHU WALL SHTG TO EA STUD. FLUSH FRAME RFTRS W/ 2-LS90.(6 SCREWS) @ CORNER & CANTILEVER TO SUBFASCIA @ LOCS
- (15) LET IN 2x RFTRS & BEAR ON 2x STUB COL. SISTER TO TRUSSES PER XX/S2.x

UPPER ROOF FRAMING PLAN

1/4" = 1'-0

• HEADERS TO BE 2-2x10 DOUG. FIR #2 INSULATED HEADERS W/ 1-2x6 KING (K) + 1-2x6 TRIM (T) EA. END & 1–2x6 PL T. & B. OR MORE AS NOTED. RE: 54/S2.3 (WG) • LOWER LVL HDR (ELEVATION VARIES PER ARCH) W/ 5 1/2" LVL PL TOP & BOTTOM. PROVIDE LS50 EA END TO

FULL HT KING STUDS. RE: 54/S2.3 • ROOF SHTG TO BE MIN. 5/8" APA RATED (40/20) OSB SHTG W/ 10d RINGSHANK NAILS @ 6"oc @ EDGES &

 ENG'D TRUSS DESIGN, TRUSS TO TRUSS CONNECTIONS, AND BRACING DESIGN BY TRUSS MANUF. • TIE DOWN TRUSSES, RAFTERS, AND LOOKOUTS @ BRG W/ H2.5A TIE, (DBL @ CORNERS) OR AS NOTED.

(3) • TIE DOWN GIRDER TRUSSES & ROOF BEAMS @ BRG W/ ST18 OR MTS18 <u>2 SIDES U.O.N.</u> • PROVIDE W/ 2-1/4"øx4 1/2" SCREWS @ 16" O.C. STAGGERED (8" O.C. EQUIVALENT) EA SIDE OF BUILT UP

• SEE SHEET **S1.01** FOR GENERAL STRUCTURAL NOTES



40th Parallel Structural Engineering 263 2nd Avenue Suite 106A Niwot, Colorado

PM: Dana Michel, PE Email: dana@40thParallelSE.com PM: Daniel Belleau, PE Email: daniel@40thParallelSE.com

S Ш ODR RIVER AND \mathbf{m} ()BACERINES BLUE RI 3 186WILDE 0

REVSIONS: No. Description

DESIGN BY

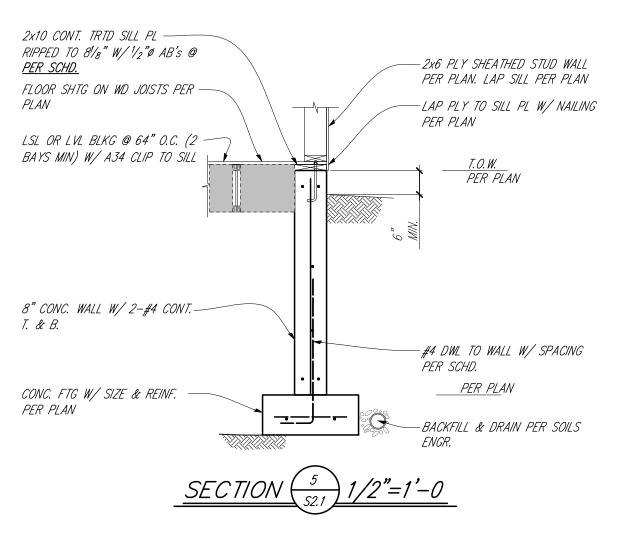
DBM24118

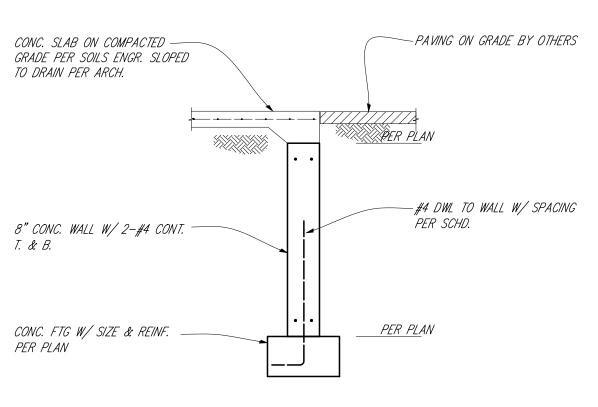
8/6/2024

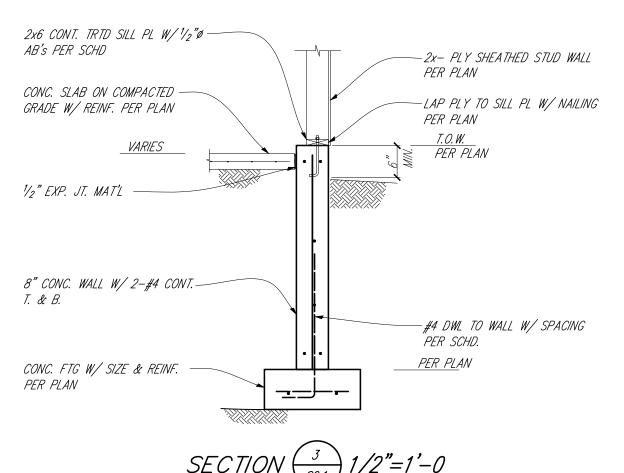
DBM



SHEET NO. S1.3

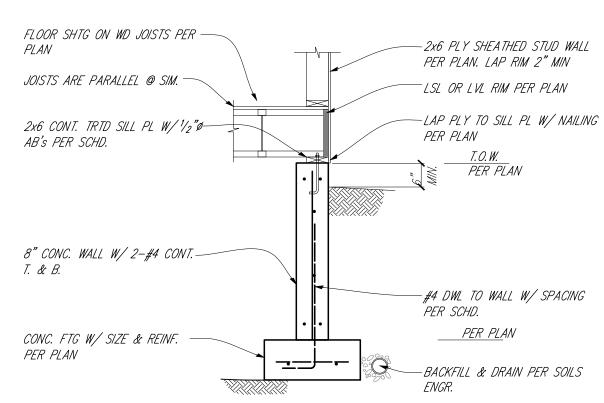






- PROVIDE BLOCKING BTWN BAYS

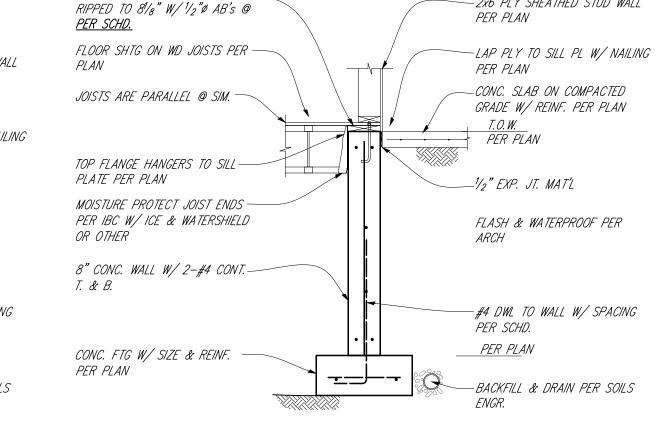
-W STL BM PER PLAN W/ 2x



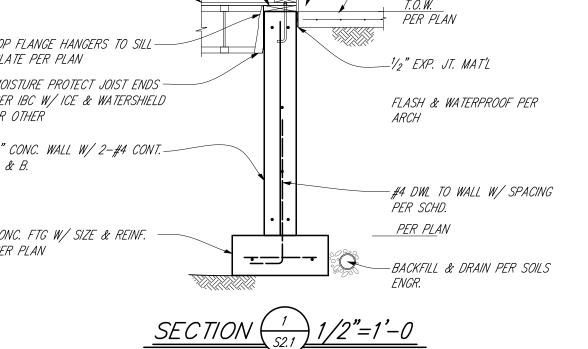
3"± (2" MIN)

FIELD VERIFY EXIST. SOUND

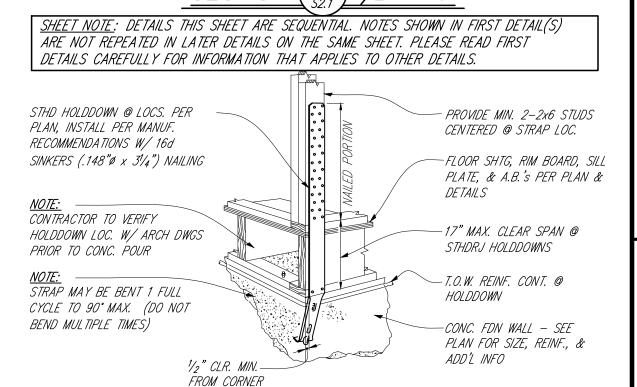
STABLE CONC. WALL IN AREA OF

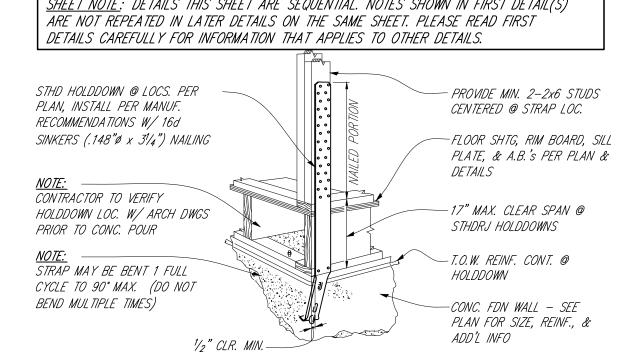


2x10 CONT. TRTD SILL PL _



-2x6 PLY SHEATHED STUD WALL

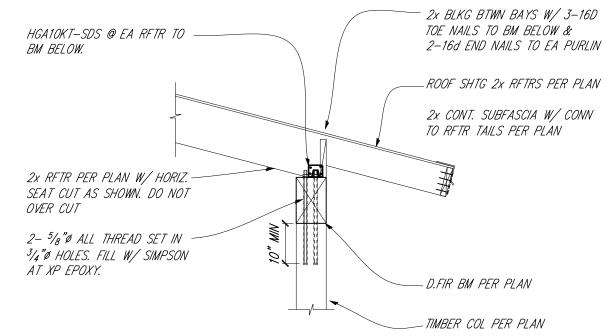


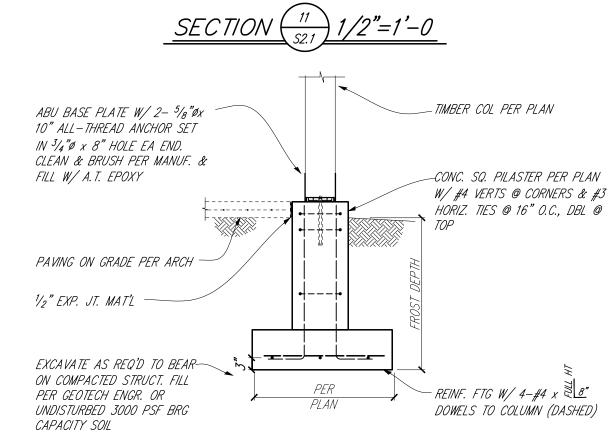


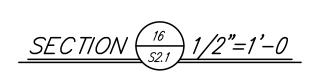


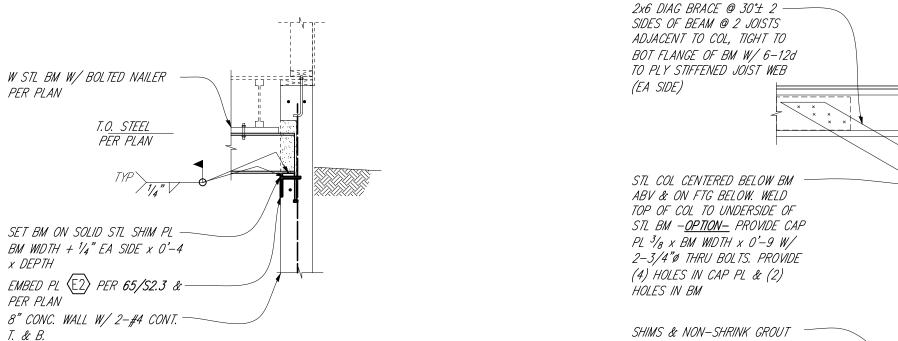
• DO NOT WETSET HOLDDOWN. USE STRAPMATE (BY SIMPSON) OR OTHER METHOD TO SECURE HOLDOWN TO FORMS PRIOR TO CONC POUR

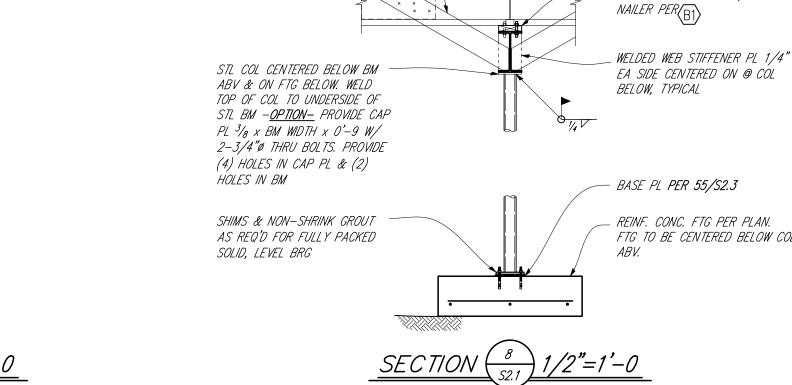
 NAIL STRAP FROM BOTTOM UP • STRAP MAY BE NAILED THRU PLY SHTG TO STUDS, OR DIRECTLY TO STUDS

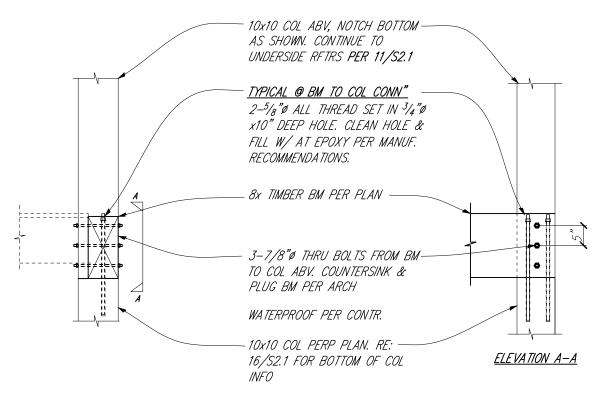


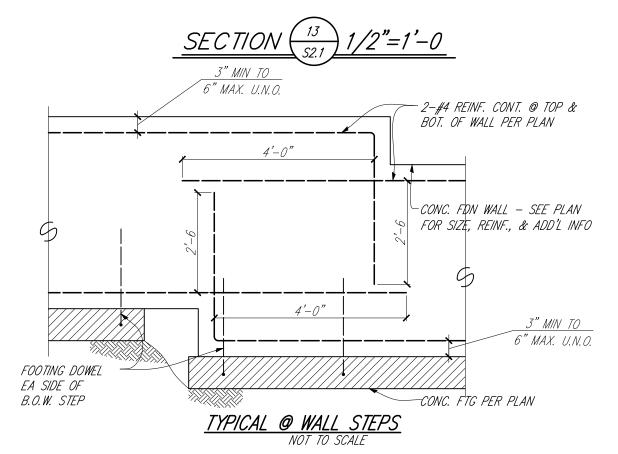


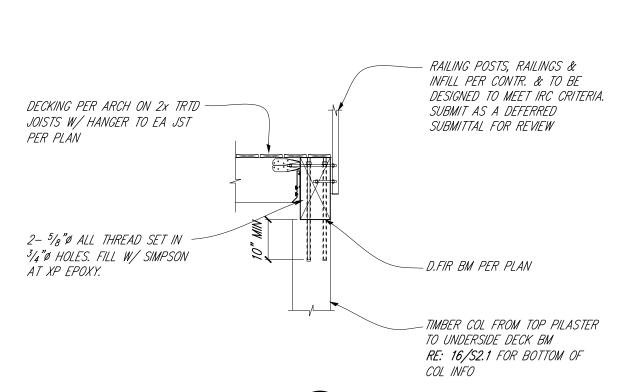




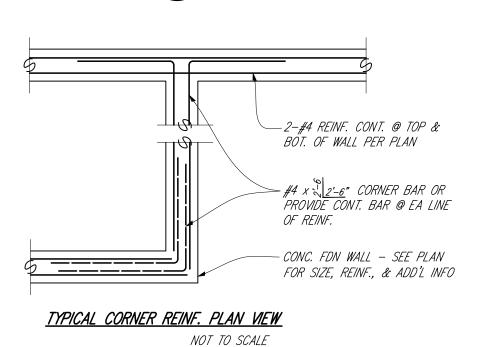


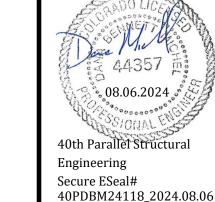






<u>SECTION</u>





40th Parallel Structural Engineering

263 2nd Avenue Suite 106A

Email: dana@40thParallelSE.com

PM: Daniel Belleau, PE Email: daniel@40thParallelSE.com

ATE

S

Ш

R

 \mathbf{m}

Z v, ⊞

BAC ERNES BLUE RI

186WILDE

LOT

DBM24118

8/6/2024

DBM

() ()

区证

 \triangleleft

3

4

DESIGN BY

REVSIONS:

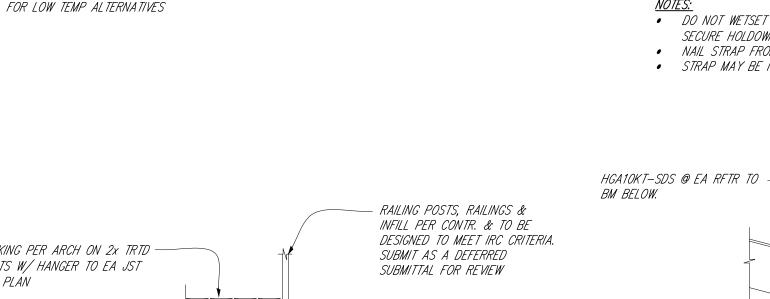
No. Description

Niwot, Colorado

PM: Dana Michel, PE

SHEET NO. S2.1

NEW HOLDDOWN ANCHOR TYPICAL HDU4-SDS2.5 HOLDDOWN REINF. CONC. FTG PER PLAN. FTG TO BE CENTERED BELOW COL <u>EPOXY NOTES:</u> • CLEAN OUT HOLE W/ BRUSH AND COMPRESSED AIR TO REMOVE DUST PER MANUF. RECOMMENDA TIONS • BASE MATERIAL MUST HAVE MIN 40° TEMP FOR 72 HOURS FOR SET EPOXY. CONTACT E.O.R.



-2-2x STUD WALL @ ENDS OF

-HDU4-SDS2.5 HOLDDOWN W/

SDS SCREWS TO STUD COL @

⁵/8"Ø ALL—THREAD ANCHOR

(GRADE 36 OR A307) SET IN

3/4"ø x 10" HOLE W/ SET 3G

EPOXY- LOCATE AS CLOSE TO

CENTER OF WALL AS PE.O.R.IBLE

S2.1/

& MIN. 6" FROM CORNERS

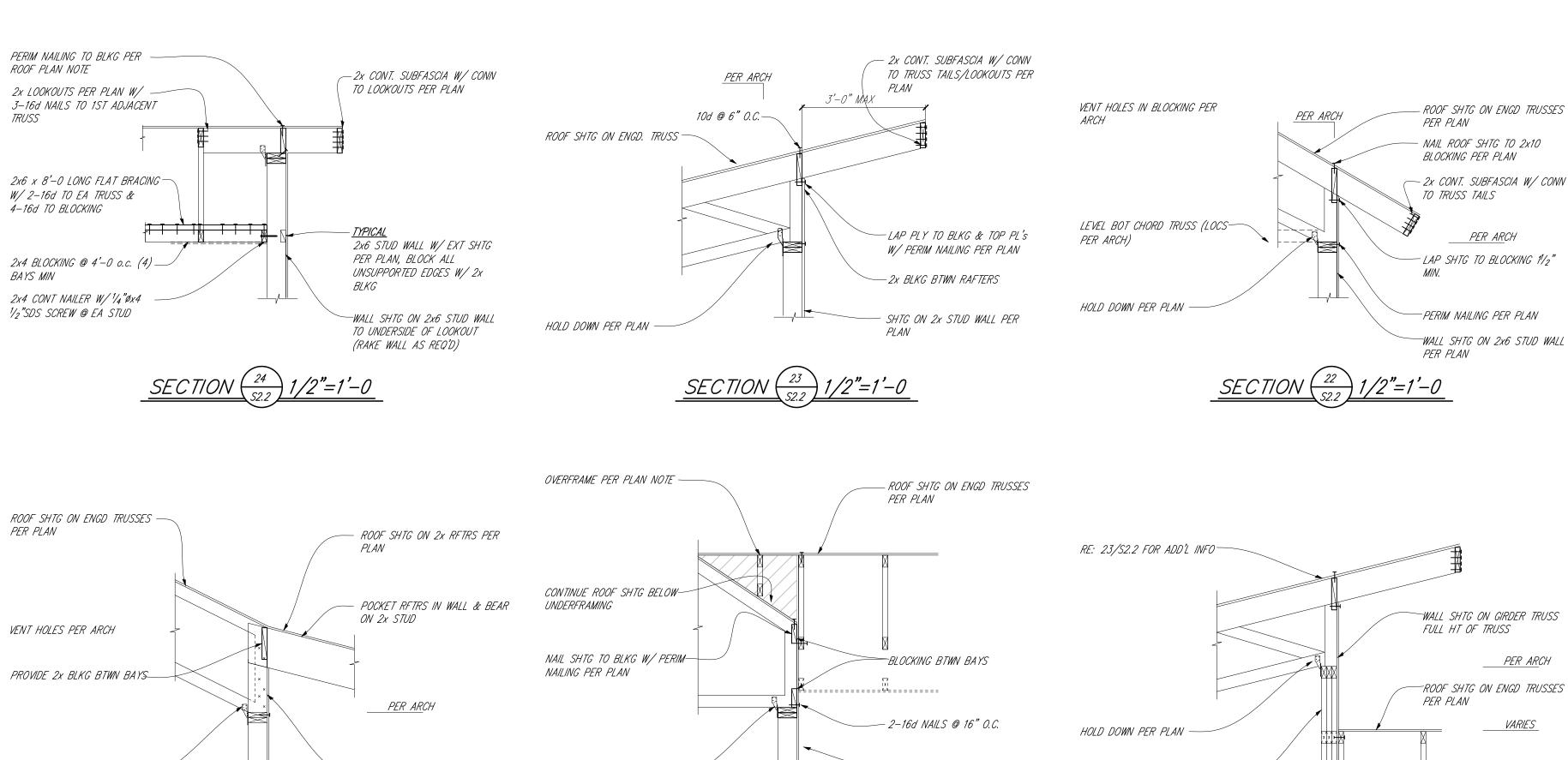
SHEAR WALL.

LOCS. PER PLAN

<u>CONTRACTOR NOTE:</u> SPECIAL INSPECTION OF

- PROVIDE MIN. 48 HR NOTICE TO E.O.R.

POST-INSTALLED HOLDDOWN ANCHORS IS REQ'D



— SISTER 2x6 STUD TO EA TRUSS W/ 4–12D NAILS

HOLD DOWN PER PLAN -

HOLD DOWN PER PLAN -

-WALL SHTG ON 2x6 STUD WALL

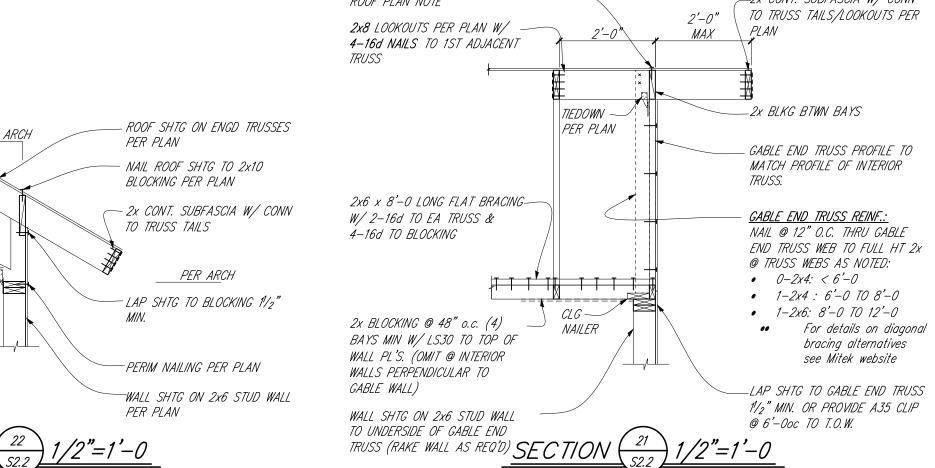
FULL HT TO UPPER TRUSSES.

PER PLAN. CONTINUE WALL SHTG

GIRDER SCISSOR TRUSS W/ TOP-

CHORD TO UNDERSIDE UPPER

ROOF TRUSSES.



PER ARCH

_____VARIES

ROOF SHTG ON ENGD TRUSSES

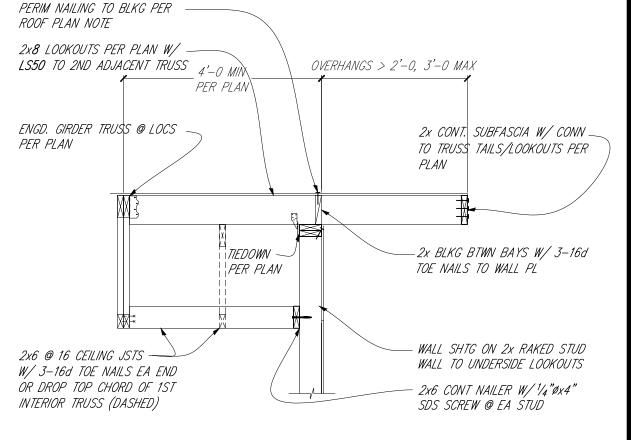
PERIM NAILING TO BLKG PER ——

PER PLAN

ROOF PLAN NOTE

https://www.mitek-us.com/resources/engineering/Gable-End-Details/

_2x CONT. SUBFASCIA W/ CONN



STATE Ш ODR RIVER AND BLL 43 BACKL, 186WILDERNESS, B LOT

40th Parallel Structural Engineering 263 2nd Avenue Suite 106A

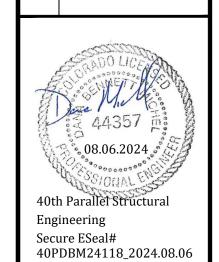
Email: dana@40thParallelSE.com

PM: Daniel Belleau, PE Email: daniel@40thParallelSE.com

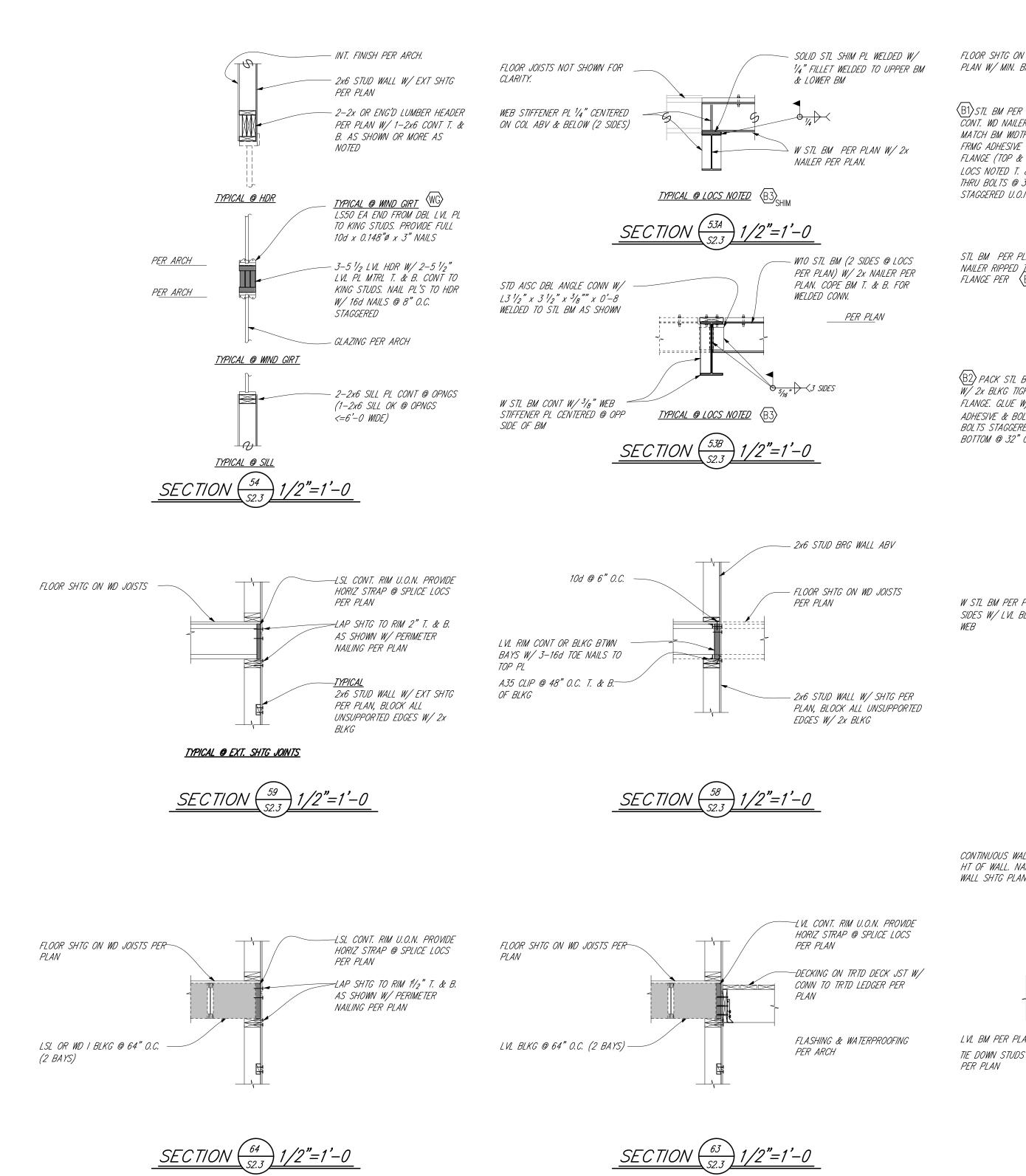
Niwot, Colorado

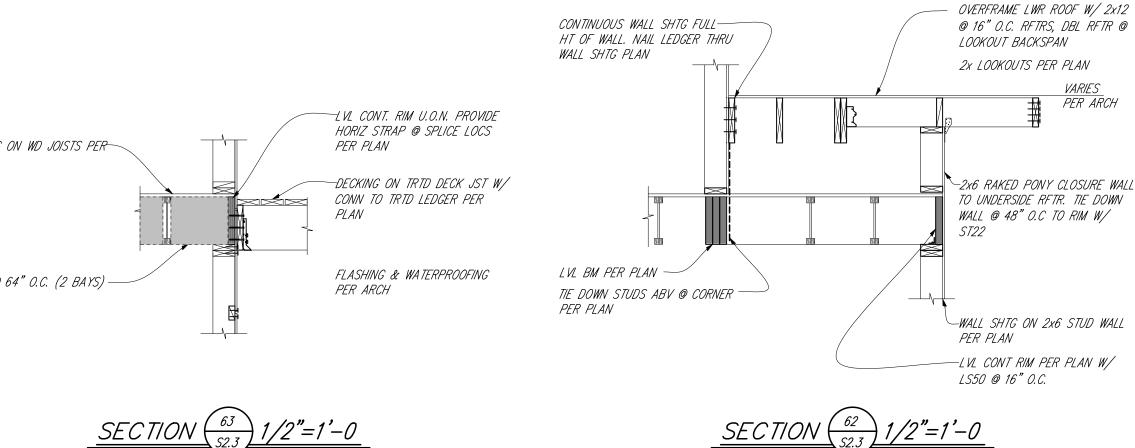
PM: Dana Michel, PE

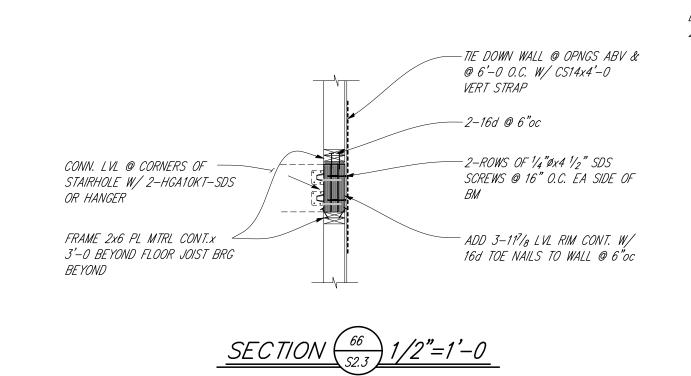
DBM24118 DESIGN BY REVSIONS: No. Description

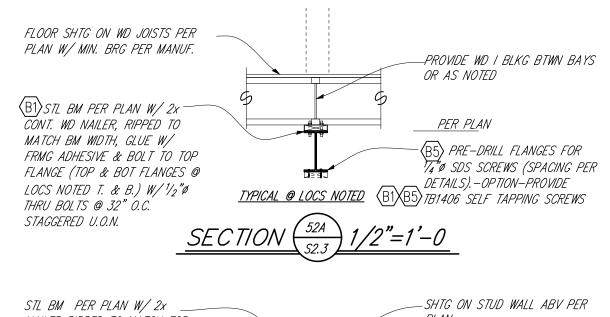


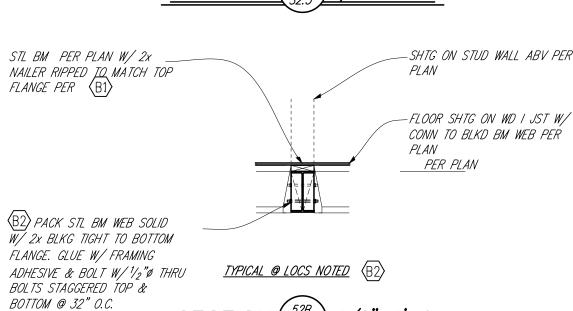
SHEET NO.

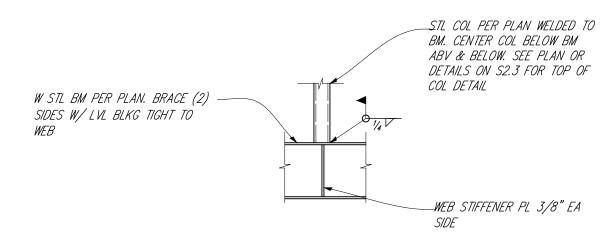






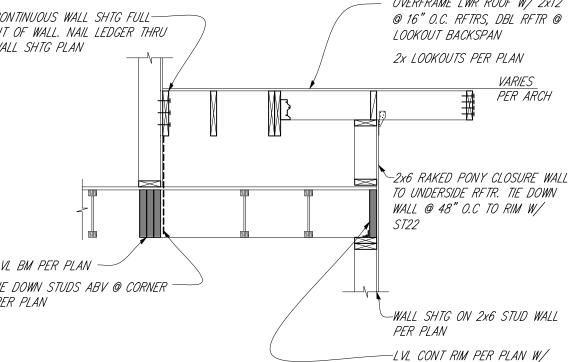


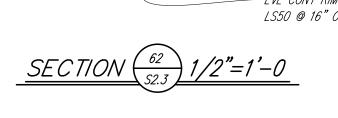


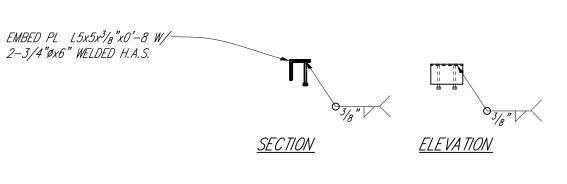


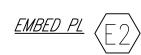
TYPICAL @ LOCS NOTED "BP4"

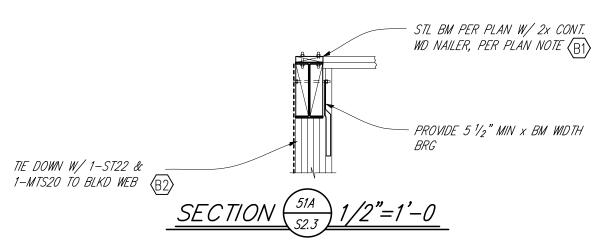
SECTION (57)

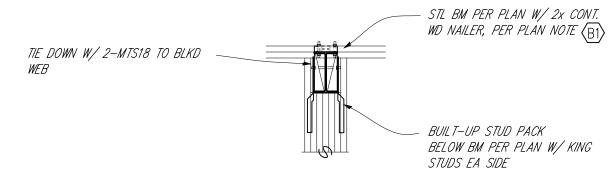




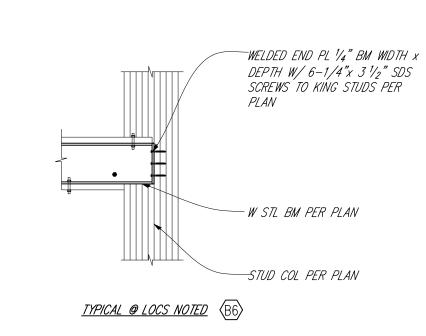


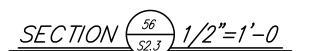


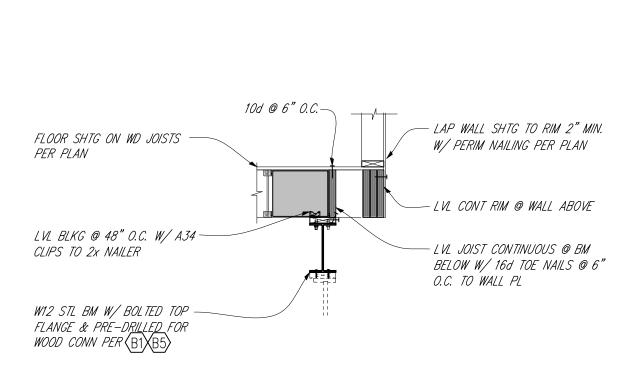


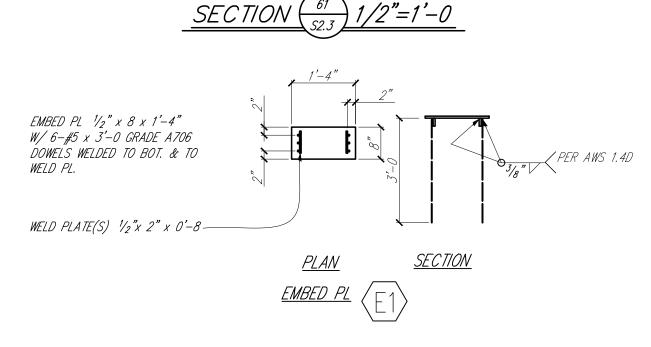


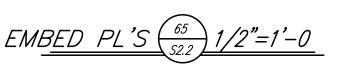




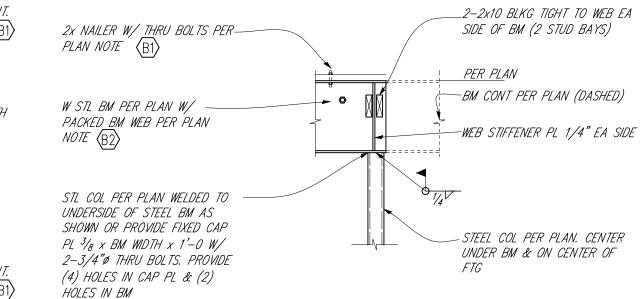








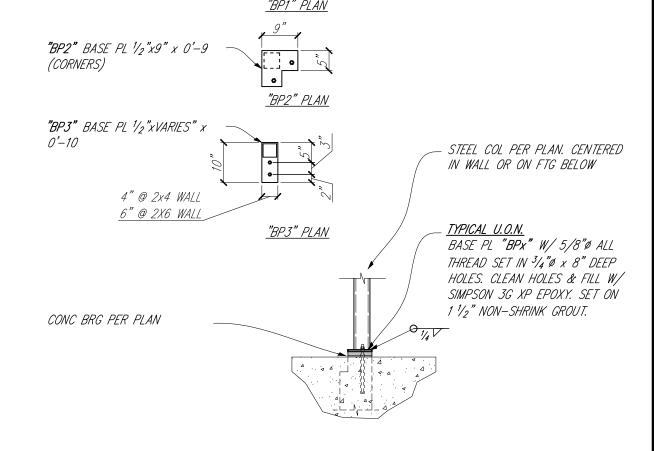
- E.O.R. TO REVIEW EMBED PL SHOP WELDS PRIOR TO INSTALLATION • E.O.R. TO REVIEW FRAME FIELD WELDS PRIOR TO COVERING W/ WOOD FRAMING
- FIELD MEASURE EMBED LOCATIONS, HEIGHTS, LEVELNESS PRIOR TO COLUMN FABRICATION. DO NOT
- SHIM CONNECTIONS W/OUT PRIOR WRITTEN NOTIFICATION BY E.O.R. WELDABLE REBAR IS TYPICALLY STAMPED WITH A "W" — REBAR WELDED TO THE EMBED PL
- WITHOUT A "W" STAMP WILL BE REJECTED AND MAY DELAY THE CONCRETE POUR.
- TIE EMBED PL'S IN FORMS PRIOR TO POUR DO NOT WET SET.

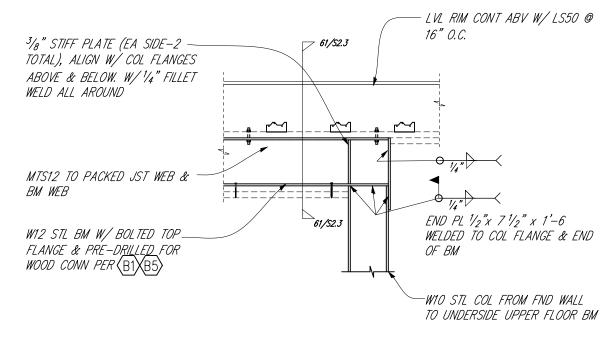


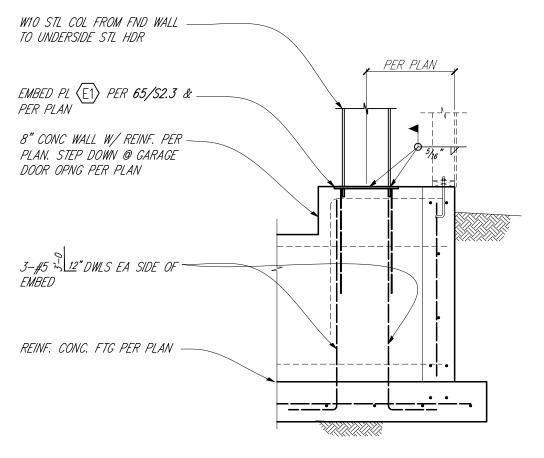


"BP1" BASE PL 1/2"x5 1/2" x

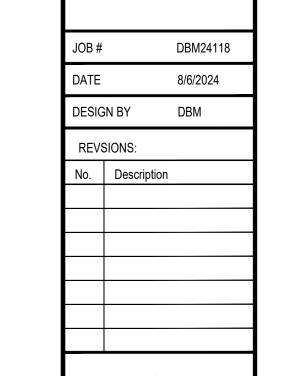
0'-10











40th Parallel Structural Engineerin 263 2nd Avenue Suite 106A

Email: dana@40thParallelSE.com

PM: Daniel Belleau, PE Email: daniel@40thParallelSE.com

S

Ш

 \mathbf{m}

BACKL ERNESS, E BLUE RIVER, (

Ш

186WILDE

O

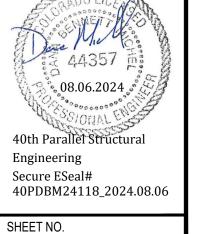
ODR RIVER

 \triangleleft

43

Niwot, Colorado

PM: Dana Michel, PE



S2.3