5030 E. 37th St N Rv Storage/Garage Site Plan

Mark Hopp

RV Storage/Garage Site Plan

- Building will be used to house a 24 ft fishing boat and a 21 ft Riverside
 RV and a lifted Ford F150 truck. This is why I picked this size of building.
- Owned vehicles/RV's do not fit in the attached garage and have to be stored off-site.
- My house roof is 12 ft tall and a low slope roof. With the building size I cannot go with a low slope roof metal building and must go conventional. The new building will be 14 ft so I can get the required clearance for my vehicles.
- This building will be for private use only.
- Build site is flat, trees have already been removed from site.





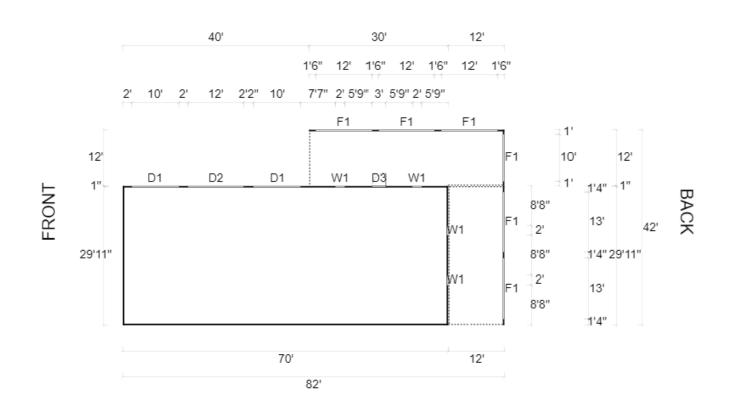


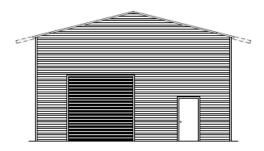
Customer Order - Jul 2, 2024						
Ship To						
Name Mark Hopp			Order # 171839	6359468311		
Install Address 5030 E. 37th N						
City Bel Aire			State KS		Zip Code 67220	
Email mhopp@txtav.com			Phone # (409) 7	70-3469	Mobile #	
Building Info	Size		Color		Anchoring & Site Preparation	
Style: Garag	е		Roof	Quaker Gray	Installation Surface:	Concrete
Roof Overhang:	42'	70' 14'/0'	Trim:	Black	Power Available	
Roof Style: A-Frame Vertic	al 42' Width	X X X 14'/9' Frame X X Leg Height	Siding:	Zinc Gray	Jobsite Level	
Gauge: 14 Gaug	е	Length	Wainscot	Black	Permits Required	
Leg Style: Standa	d				Engineering Plans Required	
Brace: Standard Brac	e				Payment Method	
Design Link & Notes						
Design Link: https://design.metalbuildingo	utfitters.com/?lng	=en-US#bfcd31c16eb58eea79	dfb1bbded0c738			
Building Images						
Perspective View	ħ	F	ont		Left Side	
	ì					
Dight Side		l R	ack			

FLOOR PLAN



LEFT SIDE





ENCLOSED GABLE END BUILDING

MAX. 30' WIDE X 16' EAVE HEIGHT WITH BOX FRAME / (UP TO) 145 M.P.H. WIND ZONE - 30 P.S.F. SNOW LOAD

FOR:

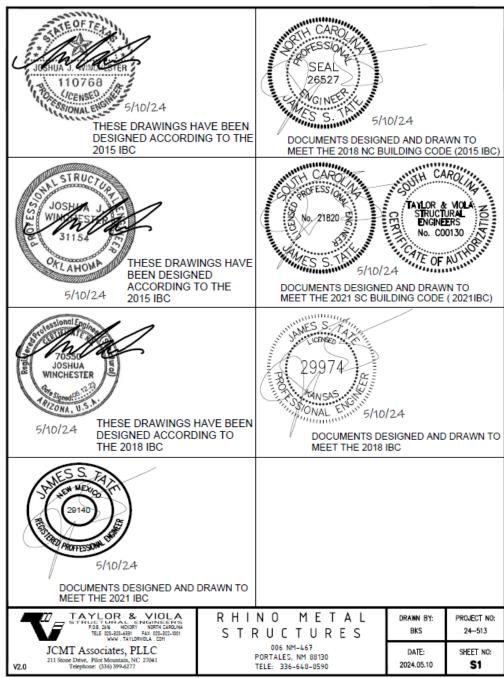
RHINO METAL STRUCTURES PORTALES, NM 88130 TELE: 336-648-8590

ISSUE DATE: MAY 10, 2024









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SHEET NUMBER		SHEET NUMBER	
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	GENERAL NOTES AND SPECIFICATIONS		OPENINGS
	SIDE AND END ELEVATIONS	S9A	TYPICAL BOX EAVE / SIDE WALL FRAMING
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S5C	TYPICAL RAFTER / COLUMN FRAME SECTIONS	5144	LEAN-TO OPTIONS
	(<16'H / >21' TO <24'W / 6:12 PITCH / 3' SOFFIT)	S14A	VOID
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	(<16'H / >25' TO <30'W)	S15	LEAN-TO CONNECTION DETAILS /
S5E	TYPICAL RAFTER / COLUMN FRAME SECTIONS	313	SINGLE COLUMN
	(<16'H / >25' TO <30'W / 3' SOFFIT)	S15A	LEAN-TO CONNECTION DETAILS /
S5F	TYPICAL RAFTER / COLUMN FRAME SECTIONS	212/1	DOUBLE COLUMN
	(<16'H / >25' TO <30'W / 6:12 PITCH)	S15B	VOID
S5G ———	TYPICAL RAFTER / COLUMN FRAME SECTIONS		
330	(<16'H / >25' TO <30'W / 6:12 PITCH / 3' SOFFIT)	S15C	VOID
S5H	TYPICAL FRAME SECTIONS		
	BOX EAVE / SINGLE COLUMN SECTION	S15D	· LEAN-TO / MAIN FRAME CONNECTION DETA
30	(<14'H)	S15E	· VOID
ee.		S15F	
S6A	BOX EAVE / SINGLE COLUMN SECTION (<14'H / 3' SOFFIT)	S15G	VOID
aen.		S15H	
S6B	BOX EAVE / SINGLE COLUMN SECTION		CONNECTION DETAIL
	(<14'H / 6:12 PITCH)	S15J	VOID
S6C	BOX EAVE / SINGLE COLUMN SECTION	S16	WERTICAL ROOF / SIDING OPTION
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S6E	BOX EAVE / DOUBLE COLUMN SECTION	S17A	END WALL HEADER OPTIONS
	(<16'H / 3' SOFFIT)		
S6F	BOX EAVE / DOUBLE COLUMN SECTION		
	(<16'H / 6:12 PITCH)		
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	(<16'H / 6:12 PITCH / 3' SOFFIT)		
S7 ———	BASE RAIL ANCHORGE / SINGLE COLUMN		
S7A	BASE RAIL ANCHORGE / SINGLE COLUMN		
	(NO SLAB)		
S7B ———	BASE RAIL ANCHORGE / SINGLE COLUMN		
375	(NO SIDING SHELF)		
S7C	BASE RAIL ANCHORGE / SINGLE COLUMN		
370	(NO SLAB / NO SIDING SHELF)		
S7D ———	BASE RAIL ANCHORGE / SINGLE COLUMN		
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	(NO SLAB)		
S7G	BASE RAIL ANCHORGE / DOUBLE COLUMN		
	(NO SIDING SHELF)		
S7H	BASE RAIL ANCHORGE / DOUBLE COLUMN		
	(NO SLAB / NO SIDING SHELF)		

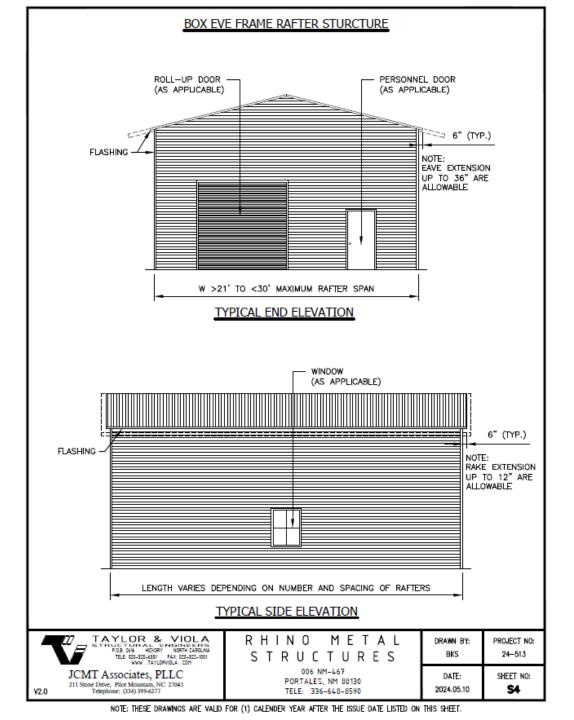


JCMT Associates, PLLC 211 Stone Drive, Pilot Mountain, NC 27041 Telephone: (336) 399-6277 RHINO METAL STRUCTURES

006 NM-467 PORTALES, NM 88130 TELE: 336-648-8590 DRAWN BY: PROJECT NO: 24-513

DATE: SHEET NO: 2024.05.10 \$2

DESIGN LOADS:									
IMPORTANCE FACTORS WIND SNOW SEISMIC	(1w) 1.00 (1s) 1.00 (1e) 1.00								
DEAD LOADS ROOF ROOF C	0 P.S.F.								
LIVE LOADS ROOF	20 P.S.F. VARIES - SEE NOTES								
GROUND SNOW LOAD:	VARIES - SEE NOTES 30 P.S.F. MIN.								
	MIND SPEED $\frac{V \ 145}{A/B/C}$ M.P.H. (ASCE 7–16) • DRIFT LG BEEN CA	DAD HAS NOT ALCULATED							
SEISMIC DESIGN CATAGORY	X A X B X C D								
PROVIDE THE FOLLOWING SEISMIC DE	SIGN PARAMETERS:								
OCCUPANCY CATEGORYI	VARIES BASED VARIES BASED								
SPECTRAL RESPONSE ACCELERATION	SPECTRAL RESPONSE ACCELERATION Ss ON SITE %g S1 ON SITE %g								
SITE CLASSIFICATIOND_	SITE CLASSIFICATIOND FIELD TEST _X PRESUMPTIVE . HISTORICAL DATA								
BASIC STRUCTURAL SYSTEM (CHECK	ONE)								
	X BUILDING FRAME DUAL W/ INTERMEDIATE R/C OR SPECIAL STEEL								
ANALYSIS PROCEDURE SIMP	LIFIED X EQUIVALANT LATERAL FORCE M	ODAL							
LATERAL DESIGN CONTROL?	EARTHQUAKE X WIND								
SOIL BEARING CAPACITIES: PRESUMPTIVE BEARING CAPACITIES:	<u>1,500</u> P.S.F.								
GENERAL NOTES:			_						
 MAX FRAME SPACING SHALL BE 	60"oc UNLESS NOTED OTHERWISE,								
MAX. END—WALL COLUMN SPACE	MAX. END-WALL COLUMN SPACING SHALL BE 60"oc UNLESS NOTED OTHERWISE.								
 TUBE MATERIAL SHALL BE 2-1, 	 TUBE MATERIAL SHALL BE 2-1/2" x 2-1/2" x 14 GA. 50 KSI MIN. UNLESS NOTED OTHERWISE. 								
4. ALL FASTENERS SHALL BE #12 SELF TAPPING AT 9"o.c. UNLESS NOTED OTHERWISE.									
 1,500 P.S.F. ASSUMED BEARING CAPACITY UNLESS NOTED OTHERWISE. 									
	<u> </u>								
TAYLOR & VIOLA STRUCTURAL ENGINEERS F03.28 HOUNT MORH CAROLIN TELE COL-301-331 FAX COL-321-301 WWW.TAYLORNOLA.COM	RHINO METAL	DRAWN BY:	PROJECT NO:						
	S T R U C T U R E S	BKS	24-513						
JCMT Associates, PLLC 211 Stone Drive, Pilot Mountain, NC 27041 Telephone: (336) 399-6277	PORTALES, NM 88130 TELE: 336-648-8590	DATE: 2024.05.10	SHEET NO: S3						



V2.0

DESIGN LOADS: IMPORTANCE FACTORS WIND (1w) 1.00 (1s) 1.00 SNOW SEISMIC (1e) 1.00 13 P.S.F. DEAD LOADS ROOF ROOF COLLATERAL 0 P.S.F. LIVE LOADS ROOF 20 P.S.F. VARIES - SEE NOTES GROUND SNOW LOAD: 30 P.S.F. MIN. BASIC WIND SPEED V 145 M.P.H. (ASCE 7-16) • DRIFT LOAD HAS NOT WIND LOAD: BEEN CALCULATED EXPOSURE CATAGORY A/B/C X A X B X C D SEISMIC DESIGN CATAGORY PROVIDE THE FOLLOWING SEISMIC DESIGN PARAMETERS: OCCUPANCY CATEGORY 1 VARIES BASED VARIES BASED Ss ON SITE %g S1 ON SITE %g SPECTRAL RESPONSE ACCELERATION . FIELD TEST X PRESUMPTIVE . HISTORICAL DATA SITE CLASSIFICATION ____D_ BASIC STRUCTURAL SYSTEM (CHECK ONE)

BEARING WALL ____ DUAL W/ SPECTRAL MOMENT FRAME

BUILDING FRAME ____ DUAL W/ INTERMEDIATE R/C OR SPECIAL STEEL

MOMENT FRAME ____ INVERTED PENDULUM

ANALYSIS PROCEDURE SHELLED ____ FOUNDALANT LATERAL FORCE ____ MOD

ANALYSIS PROCEDURE ____ SIMPLIFIED __X EQUIVALANT LATERAL FORCE ___ MODAL

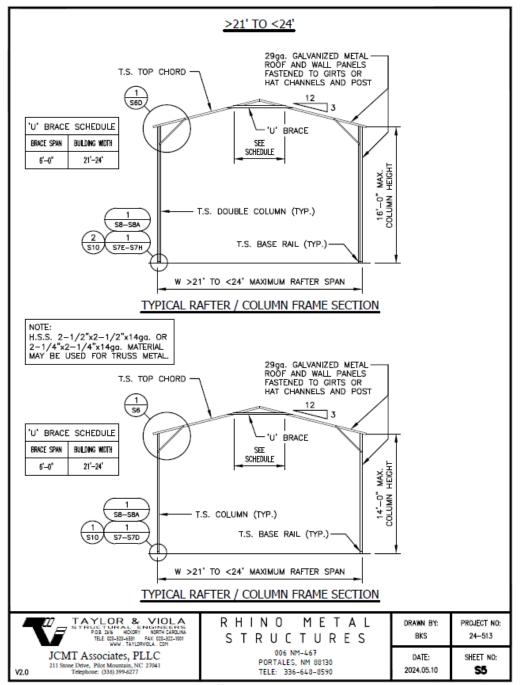
LATERAL DESIGN CONTROL? ____ EARTHQUAKE __X__ WIND

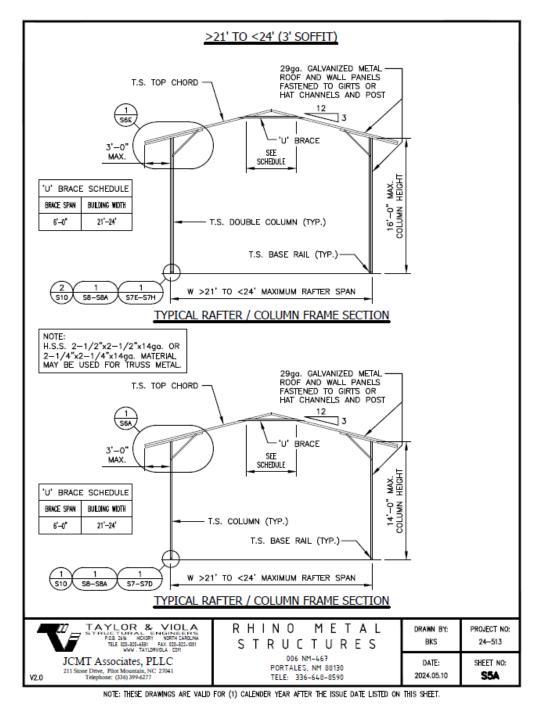
SOIL BEARING CAPACITIES:
PRESUMPTIVE BEARING CAPACITIES:

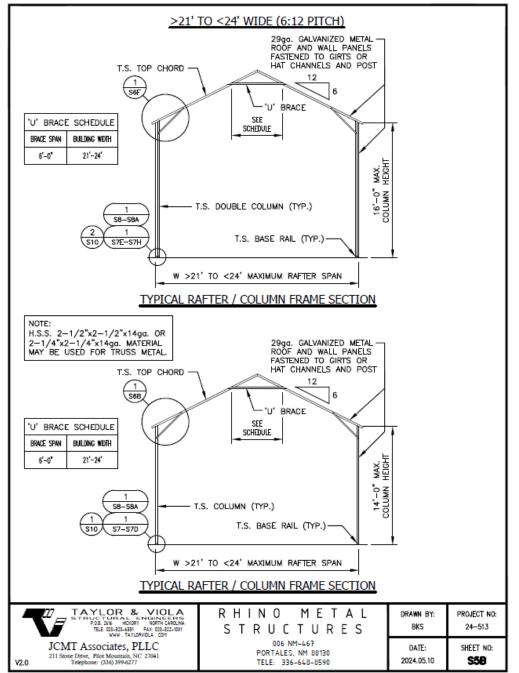
1.500 P.S.F.

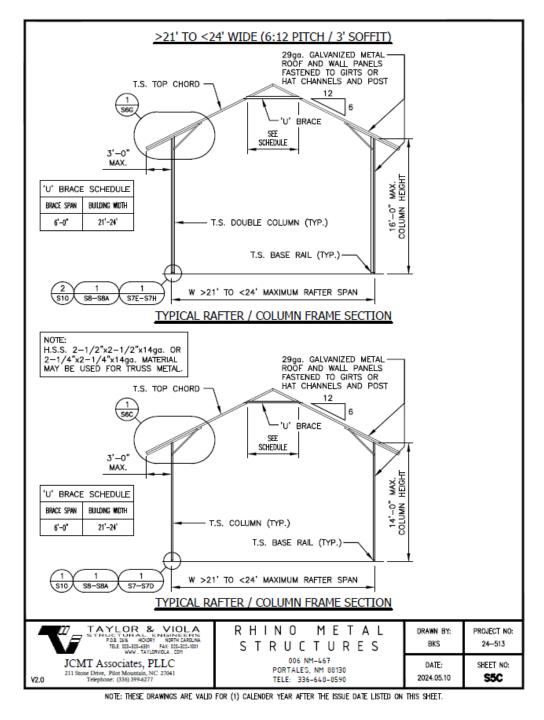
GENERAL NOTES:

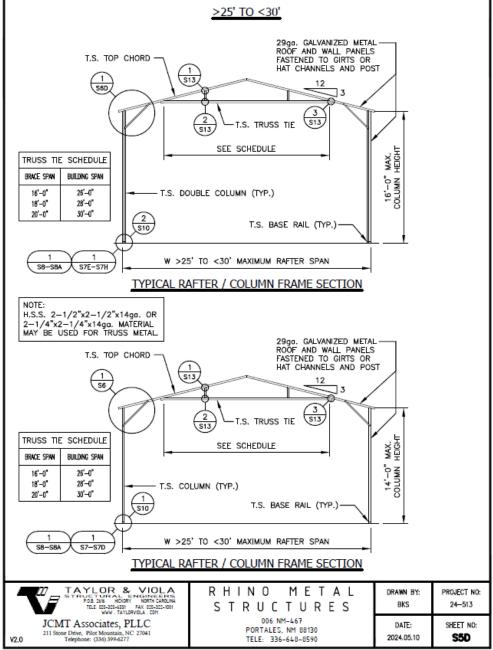
- MAX FRAME SPACING SHALL BE 60 oc UNLESS NOTED OTHERWISE.
- MAX. END—WALL COLUMN SPACING SHALL BE 60"oc UNLESS NOTED OTHERWISE.
- TUBE MATERIAL SHALL BE 2-1/2" x 2-1/2" x 14 GA. 50 KSI MIN. UNLESS NOTED OTHERWISE.
- ALL FASTENERS SHALL BE #12 SELF TAPPING AT 9"o.c. UNLESS NOTED OTHERWISE.
- 5. 1,500 P.S.F. ASSUMED BEARING CAPACITY UNLESS NOTED OTHERWISE.

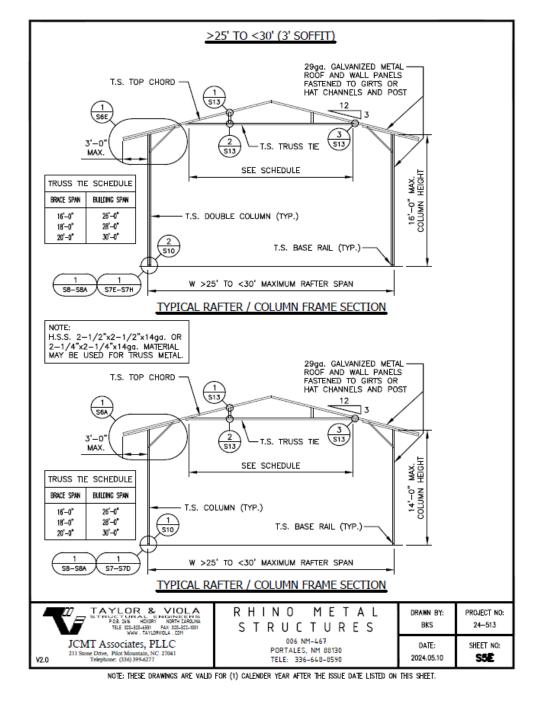


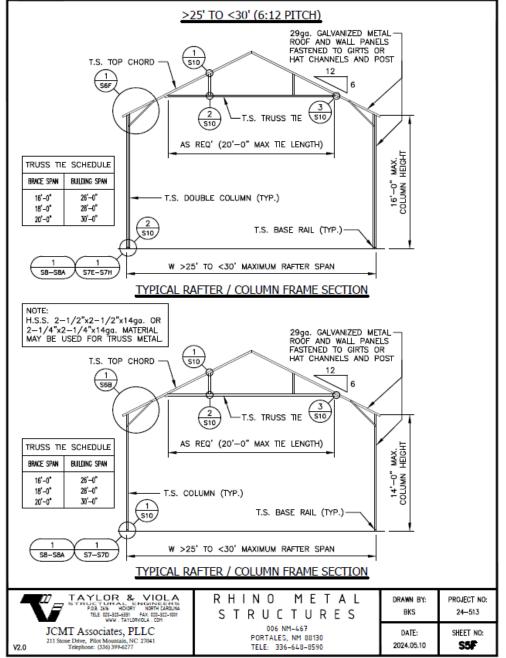


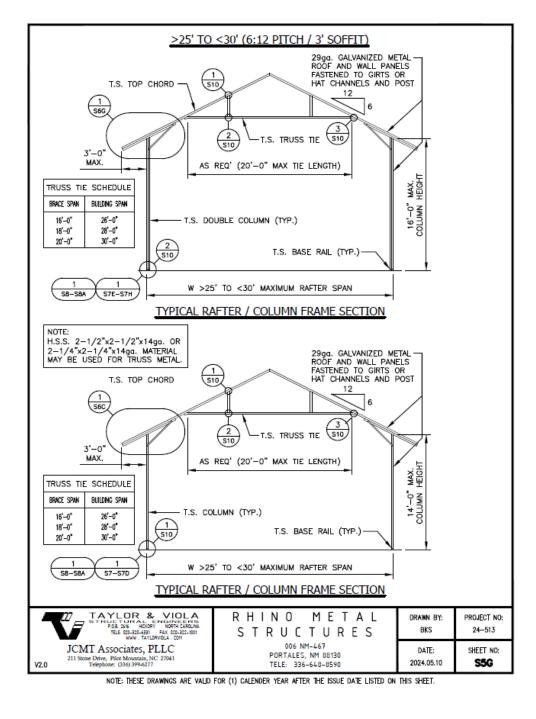


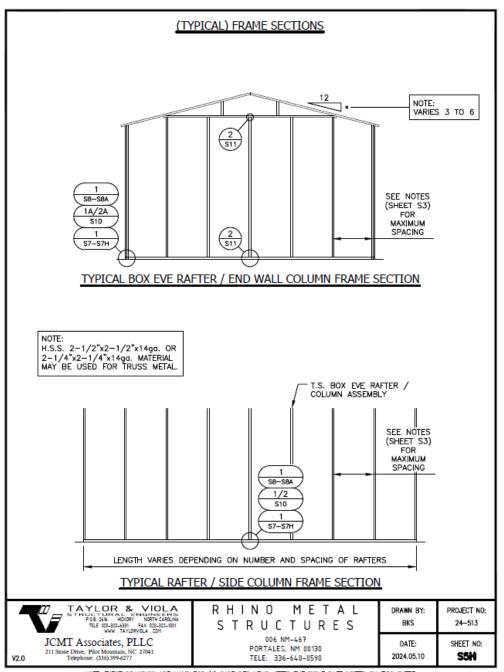


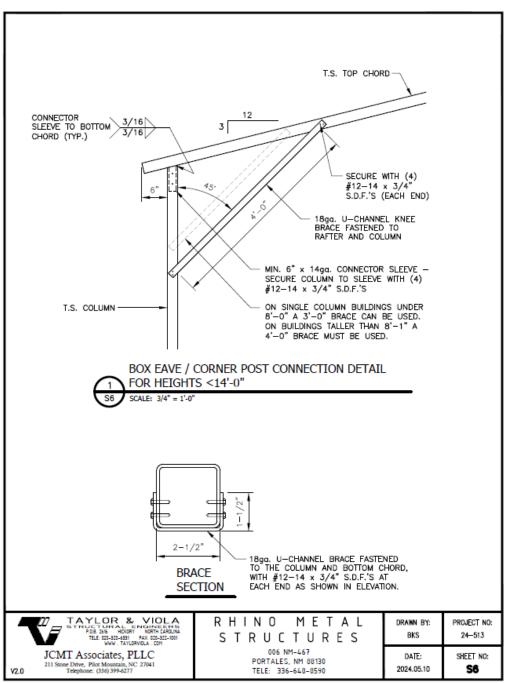


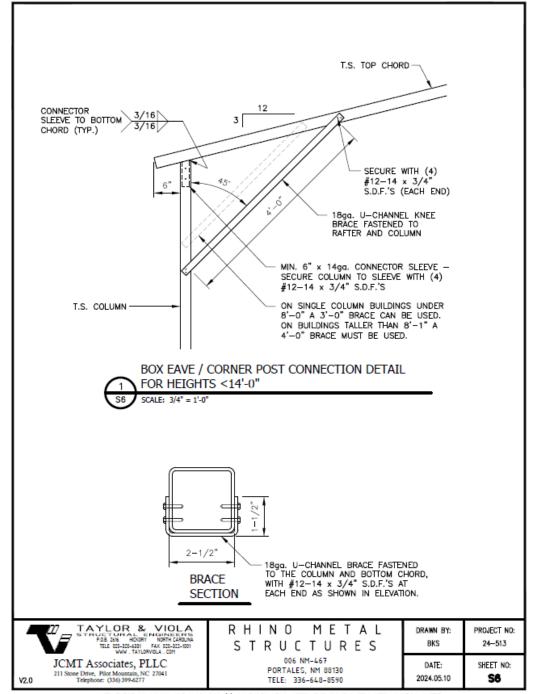


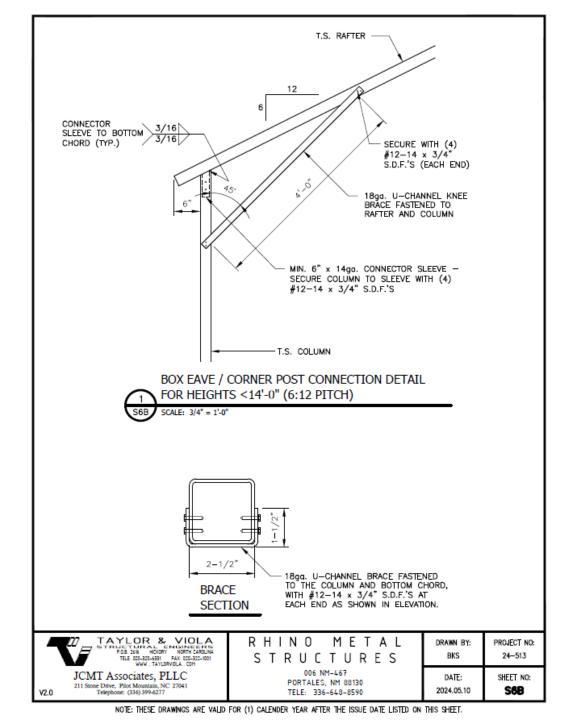


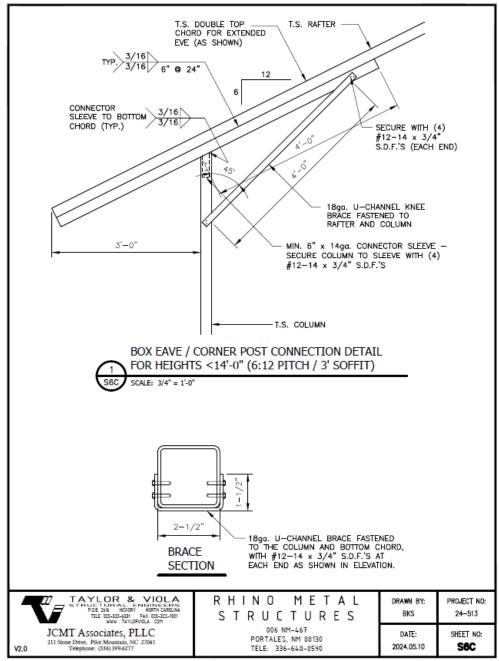


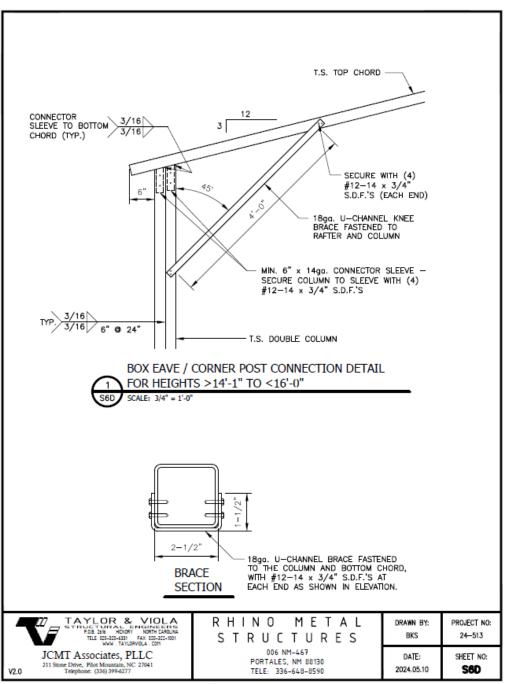




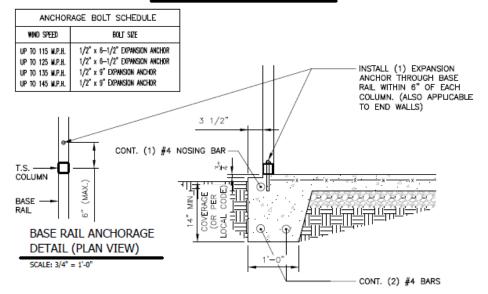


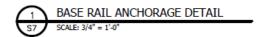






CONCRETE BASE RAIL ANCHORAGE





GENERAL NOTES:

ALL CONCRETE MONOLITHIC SLAB DESIGN BASED ON MINIMUM SOIL BEARING CAPACITY OF 2,000 P.S.F.

CONCRETE:

CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3,000 P.S.I. AT 28 DAYS.

COVER OVER REINFORCING STEEL:

FOR FOUNDATIONS, MINIMUM CONCRETE COVER OVER REINFORCING BARS SHALL BE PER ACI-318: 3" IN FOUNDATIONS WHERE THE CONCRETE IS CAST AGAINST AND PERMANENTLY IN CONTACT WITH THE EARTH OR EXPOSED TO THE EARTH AND WEATHER AND 1-1/2" ELSEWHERE.

REINFORCING STEEL:

THE TURNDOWN REINFORCING STEEL SHALL BE ASTM A615 GRADE 60. THE SLAB REINFORCEMENT SHALL BE WELDED WIRE FABRIC MEETING ASTM A185 OR FIBERGLASS FIBER REINFORCEMENT.

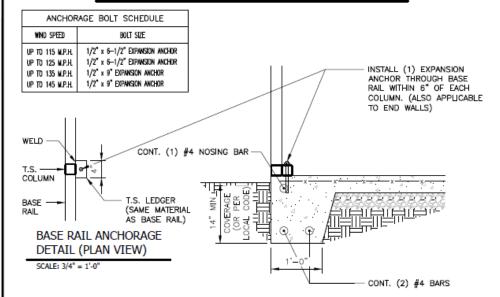
REINFORCEMENT MAT BE BENT IN THE SHOP OF THE FIELD PROVIDED:

- REINFORCEMENT IS BENT COLD.
- THE DIAMETER OF THE BEND, MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX—BAR DIAMETERS.
- REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT.



NOTE: THESE DRAWINGS ARE VALID FOR (1) CALENDER YEAR AFTER THE ISSUE DATE LISTED ON THIS SHEET.

CONCRETE BASE RAIL ANCHORAGE (LEAN-TO / NO LEDGE)





GENERAL NOTES:

ALL CONCRETE MONOLITHIC SLAB DESIGN BASED ON MINIMUM SOIL BEARING CAPACITY OF 2,000 P.S.F.

CONCRETE:

CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3,000 P.S.I. AT 28 DAYS.

COVER OVER REINFORCING STEEL:

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REINFORCEMENT MAT BE BENT IN THE SHOP OF THE FIELD PROVIDED:

- REINFORCEMENT IS BENT COLD.
- THE DIAMETER OF THE BEND, MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX—BAR DIAMETERS.
- REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT.

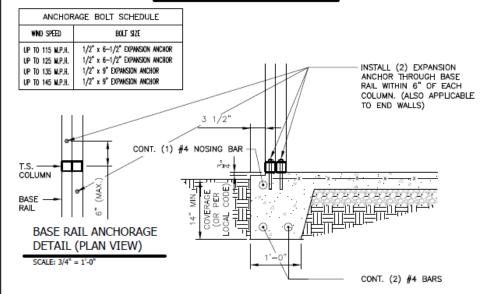


CONCRETE BASE RAIL ANCHORAGE (CLIP ANGLE ATTACHMENT OPTION) ANCHORAGE BOLT SCHEDULE WND SPEED BOLT SIZE 1/2" x 6-1/2" EXPANSION ANCHOR UP TO 115 M.P.H. 1/2" x 6-1/2" EXPANSION ANCHOR UP TO 125 M.P.H. 1/2" x 9" EXPANSION ANCHOR UP TO 135 M.P.H. UP TO 145 M.P.H. 1/2" x 9" EXPANSION ANCHOR INSTALL (1) EXPANSION ANCHOR THROUGH 4" CLIP ANGLE (WITHIN 6" OF EACH COLUMN - ALSO APPLICABLE TO END WALLS) T.S. BASE RAIL (CONT.) COLUMN 2" x 2" x 4" 16ga. CLIP ANGLE. 2" x 2" x 4" 16ga. CLIP ANGLE. BASE ATTACH TO SIDE OF BASE RAIL ATTACH TO SIDE OF BASE RAIL RAIL W/ (6) #12 x 3/4" S.D.F. W/ (6) #12 x 3/4" S.D.F. BASE RAIL ANCHORAGE DETAIL (PLAN VIEW) SCALE: 3/4" = 1'-0" BASE RAIL ANCHORAGE DETAIL (CLIP ANGLE ATTACHMENT OPTION) SCALE: 3/4" = 1'-0" TAYLOR & VIOLA STRUCTURAL ENGINEERS P.OB. 256 HOXORY NORTH CAROLINA TELE 001-301-5331 FAX 001-302-1001 RHINO METAL DRAWN BY: PROJECT NO: BKS 24-513 STRUCTURES WWW . TAYLORVIOLA . COM 006 NM-467 JCMT Associates, PLLC DATE: SHEET NO: PORTALES, NM 88130 211 Stone Drive. Pilot Mountain, NC 27041 2024.05.10 S7D Telephone: (336) 399-6277 TELE: 336-648-8590

NOTE: THESE DRAWINGS ARE VALID FOR (1) CALENDER YEAR AFTER THE ISSUE DATE LISTED ON THIS SHEET.

V2.0

CONCRETE BASE RAIL ANCHORAGE





GENERAL NOTES:

ALL CONCRETE MONOLITHIC SLAB DESIGN BASED ON MINIMUM SOIL BEARING CAPACITY OF 2,000 P.S.F.

CONCRETE SHALL HAVE A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 3,000 P.S.I. AT 28 DAYS.

COVER OVER REINFORCING STEEL:

FOR FOUNDATIONS, MINIMUM CONCRETE COVER OVER REINFORCING BARS SHALL BE PER ACI-318: 3" IN FOUNDATIONS WHERE THE CONCRETE IS CAST AGAINST AND PERMANENTLY IN CONTACT WITH THE EARTH OR EXPOSED TO THE EARTH AND WEATHER AND 1-1/2" ELSEWHERE.

REINFORCING STEEL:

THE TURNDOWN REINFORCING STEEL SHALL BE ASTM A615 GRADE 60. THE SLAB REINFORCEMENT SHALL BE WELDED WIRE FABRIC MEETING ASTM A185 OR FIBERGLASS FIBER REINFORCEMENT.

REINFORCEMENT MAT BE BENT IN THE SHOP OF THE FIELD PROVIDED:

- REINFORCEMENT IS BENT COLD.
- 2. THE DIAMETER OF THE BEND, MEASURED ON THE INSIDE OF THE BAR, IS NOT LESS THAN SIX-BAR DIAMETERS.
- REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT.



