STAFF REPORT

Rezoning #19CZ02 Morris Acres PUD

November 19, 2019 Town Council Meeting



All property owners within three hundred (300) feet of this rezoning were notified per UDO Sec. 2.2.11 *Public Notification*.

BACKGROUND INFORMATION:

Location: 0, 7208, & 7208B Morris Acres Road
Applicant/Owner: Kaplan Residential/Edith S. Morris
Agent: Jason Barron, Morningstar Law Group

PROJECT DESCRIPTION:

Acreage: $17.4376 \pm acres$

PINs: 0732289587, 0732382530, & 0732382709

Current Zoning: Rural Residential (RR)

Proposed Zoning: Planned Unit Development-Conditional Zoning (PUD-CZ)

Current 2045 Land Use Map: Medium Density Residential

Town Limits: 0732382709 is in the ETJ; 0732289587 & 0732382530 are in Town limits

Adjacent Zoning & Land Uses:

	Zoning	Land Use
North:	Rural Residential (RR); Medium Density- Conditional Use (MD-CU #94CU01)	Single-family residential; Vacant
South:	Planned Unit Development-Conditional Zoning (PUD-CZ #15CZ22)	Morris Acres Road; Multi-family (Flats @ 540); Single-family residential (Beaver Creek @540 Townhomes)
East:	Medium Density-Conditional Use (MD-CU #94CU01); Conservation Buffer (CB)	Single-family residential (Walden Creek); Vacant
West:	Rural Residential (RR)	Morris Acres Road; Vacant

Existing Conditions:

The subject properties are located on the north side of Morris Acres Road, just east of NC 540. There are several existing structures on the properties related to the historically rural residential setting of these properties.

Neighborhood Meeting:

The applicant conducted a neighborhood meeting for the revised request on October 29, 2019. The neighborhood meeting report is attached.

2045 LAND USE MAP:

The 2045 Land Use Map identifies the properties subject to this rezoning as Medium Density Residential. The rezoning to Planned Unit Development-Conditional Zoning, as proposed, is consistent with the Medium Density Residential classification.

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A portion of this site was identified and adopted within the Transit Oriented Development (TOD) Context Area with Advance Apex. The location is appropriate for higher medium density uses like townhomes due to the proximity to NC 540 Hwy, the adjacent Flats at 540 multi-family development, and Beaver Creek @540 townhome development, as well as proximity to a future transit corridor. TOD development typically dictates transit-supportive densities, which is a minimum of seven (7) units per acre for a circulator bus service and a minimum fifteen (15) units per acre for fixed route bus service. The Morris Acres PUD proposes a maximum density of seven (7) units per acre, contributing to an overall density that supports future transit.

PROPOSED ZONING CONDITIONS:

Limitation of Uses:

The Rezoned Lands may be used for, and only for, the uses listed immediately below. The permitted uses are subject to the limitations and regulations stated in the UDO and any additional limitations or regulations stated below. For convenience, some relevant sections of the UDO may be referenced; such references do not imply that other sections of the UDO do not apply.

Permitted Uses and Limitations:

1. Townhomes

2. Greenway

3. Recreation Facility, private

- 4. Park, active
- 5. Park, passive
- 6. Utility, minor

Permitted Design Controls:

1. Maximum Density

The PUD text indicates a maximum residential density for the project of 7.0 dwelling units per acre and no more than 122 total units.

2. Maximum Height of the Buildings and Number of Stories

Maximum height – Three (3) stories with a maximum height of 45'.

3. Minimum Building Setbacks

- From Building to Building 10'
- From Buffer/RCA 10' for Buildings; 5' for Parking Areas

4. Percentage of Built Upon Area

The UDO allows for a maximum 70% of built upon area in a PUD project and the Morris Acres PUD will not exceed that amount.

5. **Parking**

The PUD indicates that parking will be provided per the standards in the UDO.

6. Resource Conservation Area

The PUD is providing at least 20% (3.49 acres) of the total area for Resource Conservation Area and landscape buffers. The minimum RCA required is 20% (3.49 acres).



7. Buffers

Perimeter Buffers:	Required*	Proposed
Western property boundary	10' Type B	30' Type A
Eastern property boundary	20' Type B	50' Type A
Southern property boundary	30' Type B	30' Type A
Northern property boundary	10' Type B	20' Type A

^{*}based on Land Use Class 3, Townhomes

Architectural Standards:

- 1. Vinyl siding will not be used except for vinyl windows and limited decorate element use. Residential areas will utilize brick, stone, and Hardi-plank siding.
- 2. Siding materials will be varied in type and/or color on 30% of each façade on each building.
- 3. Windows that are not recessed shall be trimmed. Windows shall vary in size and/or type.
- 4. Recesses and projections shall be provided for at least 50% of each facade on each building. Building facades shall have horizontal relief achieved by the use of recesses and projections.
- 5. Four of the following decorative features shall be used on each building: decorative shake, board and batten siding, decorative porch rails and posts, shutters, decorative functional foundation and roof vents, recessed windows, decorative windows, decorative brick or stone, decorative gables, decorative cornices, or metal roofing.
- 6. A varied color palette shall be utilized throughout the development to include a minimum of three color families for siding and shall include varied trim, shutter, and accent colors complementing the siding color.
- 7. The rear and side elevations of the units that can be seen from the right-of-way shall have trim around the windows.
- 8. Front facing garage doors must have windows, decorative details, or carriage-style adornments.
- 9. Entrances for units with front-facing garages shall have a prominent covered porch/stoop area leading to the front door.
- 10. The front façade of any front-loaded garage shall not protrude farther than one foot forward of (i) the front façade of the dwelling unit, or (ii) the front porch of the dwelling unit, whichever is closer to the right-of-way from which the dwelling unit is addressed.

Additionally, the following conditions shall also apply:

- 1. A maximum of 122 residential units shall be permitted upon the property;
- 2. A fifty-foot (50') type A buffer shall be established and maintained along the eastern boundary of the subject property;
- 3. The maximum height for buildings shall be three (3) stories (forty-five feet (45'));
- 4. All buildings constructed on the property shall provide solar conduit for the installation of rooftop solar panels; and
- 5. The applicant shall provide a 6' x 6' Public Art easement to the Town of Apex along the Morris Acres Road frontage of the subject property.

Pedestrian Connectivity:

The project will provide a 10' wide side path along the north side of Morris Acres Road, consistent with the recommendations of Bike Apex. The pedestrian network will be evaluated during subdivision plan review and shall be consistent with the UDO.

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Public Facilities:

The proposed PUD shall meet all Public Facilities requirements as set forth in UDO Section 2.3.4.F.1.f. Such facilities will be designed according to sound engineering standards, and shall comply with Town of Apex Sewer and Water Master Plan and the Town of Apex Standards and Specifications.

All lots within the project will be served by the Town of Apex for water and sanitary sewer. The utility design will be finalized at the time of Master Subdivision Plan review and approval based upon available facilities adjacent to the site at that time. A conceptual utility plan is included in the PUD plan for reference. Electricity will be provided by Apex Electric. Phone, cable and gas will be provided by the developer and shall meet the Town of Apex standards as outlined in the UDO.

This PUD shall met all stormwater management requirements for quality and quantity treatment in accordance with Section 6.1.7 of the UDO, such that:

- Post development peak runoff shall not exceed pre-development peak runoff conditions for the 1 year, 10 year, 25 year, and 24-hour storm events.
- This PUD shall convey as much stormwater runoff from the site development as practical, including required Stormwater Control Measures (SCM's), to the existing 48" RCP culvert located in the southeastern corner of the site along Morris Acres Road. The direct storm drainage connection to the existing 48" RCP culvert is subject to final approval by the Town of Apex, NCDOT or any other regulatory agency. In the event that this direct storm drainage connection is not approved, then this PUD shall meet and exceed existing stormwater management requirements for quality and quantity treatment provided in Section 6.1.7 of the UDO, such that post development peak runoff shall not exceed pre-development peak runoff conditions for the 1 year, 10 year, 25 year, 100 year and 24-hour storm events.

PARKS, RECREATION, AND CULTURAL RESOURCES ADVISORY COMMISSION:

The Parks, Recreation, and Cultural Resources Advisory Commission recommended a fee-in-lieu at their May 29, 2019 meeting. The fee rate will be set based on the date of PUD approval and will be applied to the number of lots proposed at the time of Master Subdivision Plan approval.

APEX TRANSPORTATION PLAN/ACCESS and CIRCULATION:

The proposed PUD is consistent with the Apex Transportation Plan. The proposed PUD includes two (2) points of access onto Morris Acres Road, an existing 3-lane Thoroughfare on the *Thoroughfare and Collector Street Plan*. This project shall provide minimum frontage widening based on ½ of a 3-lane thoroughfare section with side path and public right-of-way dedication based on an eighty foot (80') right-of-way along Morris Acres Road. The site will promote connectivity to undeveloped property with a stub street to the north.

Roadway improvements, subject to modification and final approval by the Town of Apex and NCDOT, are part of the site plan and construction plan approval process. A traffic study has been performed as part of this PUD rezoning consistent with the Town's standards for the same. Based upon the traffic study, no off-site improvements are recommended for this development.

Wayfinding measures at the site shall be provided in an effort to facilitate the movement of vehicles and pedestrians to and within the development.

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PLANNING STAFF RECOMMENDATION:

Planning staff recommends approval of the rezoning #19CZ02 Morris Acres PUD with the conditions offered by the applicant.

PLANNING BOARD RECOMMENDATION:

The Planning Board heard this item at their November 12, 2019 meeting and unanimously voted to recommend approval of the rezoning with the conditions as proposed by the applicant.

ANALYSIS STATEMENT OF THE REASONABLENESS OF THE PROPOSED REZONING:

This Statement will address consistency with the Town's comprehensive and other applicable plans, reasonableness, and effect on public interest:

Approval of the rezoning is reasonable because the proposed Planned Unit Development-Conditional Zoning district is consistent with the Medium Density Residential land use classification on the 2045 Land Use Map.

The proposed rezoning is reasonable and in the public interest because it provides an adequate transition in the height and density from the existing multi-family and townhome uses to the south and the existing single-family residential development to the east. The proposed rezoning allows for a maximum of seven (7) dwelling units per acres, which is the minimum density needed to support Transit Oriented Development (TOD) Context Area, as adopted with Advance Apex.

PLANNED UNIT DEVELOPMENT DISTRICT AND CONDITIONAL ZONING STANDARDS: Standards

In return for greater flexibility in site design requirements, Planned Development (PD) Districts are expected to deliver exceptional quality community designs that preserve critical environmental resources; provide high quality community amenities; incorporate creative design in the layout of buildings, Resource Conservation Area and circulation; ensure compatibility with surrounding land uses and neighborhood character; provide high quality architecture; and provide greater efficiency in the layout and provision of roads, utilities, and other infrastructure. The Planned Development (PD) Districts shall not be used as a means of circumventing the Town's adopted land development regulations for routine developments.

- 1) Planned Unit Development (PUD-CZ) District
 In approving a Planned Development (PD) Zoning District designation for a PUD-CZ, the Town Council shall find the PUD-CZ district designation and PD Plan for PUD-CZ demonstrates compliance with the following standards:
 - a) Development parameters
 - (i) The uses proposed to be developed in the PD Plan for PUD-CZ are those uses permitted in Sec. 4.2.2 *Use Table*.
 - (ii) The uses proposed in the PD Plan for PUD-CZ can be entirely residential, entirely non-residential, or a mix of residential and non-residential uses, provided a minimum percentage of non-residential land area is included in certain mixed use areas as specified on the 2045 Land Use Map. The location of uses proposed by the PUD-CZ must be shown in the PD Plan

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with a maximum density for each type of residential use and a maximum square footage for each type of non-residential use.

- (iii) The dimensional standards in Sec. 5.1.3 *Table of Intensity and Dimensional Standards, Planned Development Districts* may be varied in the PD Plan for PUD-CZ. The PUD-CZ shall demonstrate compliance with all other dimensional standards of the UDO, North Carolina Building Code, and North Carolina Fire Code.
- (iv) The development proposed in the PD Plan for PUD-CZ encourages cluster and compact development to the greatest extent possible that is interrelated and linked by pedestrian ways, bikeways and other transportation systems. At a minimum, the PD Plan must show sidewalk improvements as required by the Apex Transportation Plan and the *Town of Apex Standard Specifications and Standard Details*, and greenway improvements as required by the Town of Apex Parks, Recreation, Greenways, and Open Space Plan and the Apex Transportation Plan. In addition, sidewalks shall be provided on both sides of all streets for single-family detached homes.
- v) The design of development in the PD Plan for PUD-CZ results in land use patterns that promote and expand opportunities for walkability, connectivity, public transportation, and an efficient compact network of streets. Cul-de-sacs shall be avoided unless the design of the subdivision and the existing or proposed street system in the surrounding area indicate that a through street is not essential in the location of the proposed cul-de-sac, or where sensitive environmental areas such as streams, floodplains, and wetlands would be substantially disturbed by making road connections.
- (vi) The development proposed in the PD Plan for PUD-CZ is compatible with the character of surrounding land uses and maintains and enhances the value of surrounding properties.
- (vii) The development proposed in the PD Plan for PUD-CZ has architectural and design standards that are exceptional and provide higher quality than routine developments. All residential uses proposed in a PD Plan for PUD-CZ shall provide architectural elevations representative of the residential structures to be built to ensure the Standards of this Section are met.
- b) Off-street parking and loading. The PD Plan for PUD-CZ shall demonstrate compliance with the standards of Sec. 8.3 Off-Street Parking and Loading, except that variations from these standards may be permitted if a comprehensive parking and loading plan for the PUD-CZ is submitted as part of the PD Plan that is determined to be suitable for the PUD-CZ, and generally consistent with the intent and purpose of the off-street parking and loading standards.
- c) RCA. The PD Plan for PUD-CZ shall demonstrate compliance with Sec. 8.1.2 Resource Conservation Area, except that the percentage of RCA required under Sec. 8.1.2 may be reduced by the Town Council by no more than two percent (2%) provided that:
 - (i) The PD Plan for PUD-CZ includes a non-residential component; or
 - (ii) The PD Plan for PUD-CZ has an overall density of 6 residential units per acre or more.

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- d) Landscaping. The PD Plan for PUD-CZ shall demonstrate compliance with the standards of Sec. 8.2 Landscaping, Buffering and Screening, except that variations from these standards may be permitted where it is demonstrated that the proposed landscaping sufficiently buffers uses from each other, ensures compatibility with land uses on surrounding properties, creates attractive streetscapes and parking areas and is consistent with the character of the area. In no case shall a buffer be less than one half of the width required by Sec. 8.2 or 10 feet in width, whichever is greater.
- e) Signs. Signage in the PD Plan for PUD-CZ shall demonstrate compliance with Sec. 8.7 Signs, except that the standards can be varied if a master signage plan is submitted for review and approval concurrent with the PD plan and is determined by the Town Council to be suitable for the PUD-CZ and generally consistent with the intent and purpose of the sign standards of the UDO. The master signage plan shall have design standards that are exceptional and provide for higher quality signs than those in routine developments and shall comply with Sec. 8.7.2 Prohibited Signs.
- f) Public facilities. The improvements standards and guarantees applicable to the public facilities that will serve the site shall comply with Article 7: Subdivision and Article 14: Parks, Recreation, Greenways, and Open Space.
 - (i) The PD Plan for PUD-CZ demonstrates a safe and adequate on-site transportation circulation system. The on-site transportation circulation system shall be integrated with the off-site transportation circulation system of the Town. The PD Plan for PUD-CZ shall be consistent with the Apex Transportation Plan and the *Town of Apex Standard Specifications and Standard Details* and show required right-of-way widths and road sections. A Traffic Impact Analysis (TIA) shall be required per Sec. 13.19.
 - (ii) The PD Plan for PUD-CZ demonstrates a safe and adequate on-site system of potable water and wastewater lines that can accommodate the proposed development, and are efficiently integrated into off-site potable water and wastewater public improvement plans. The PD Plan shall include a proposed water and wastewater plan.
 - (iii) Adequate off-site facilities for potable water supply, sewage disposal, solid waste disposal, electrical supply, fire protection and roads shall be planned and programmed for the development proposed in the PD Plan for PUD-CZ, and the development is conveniently located in relation to schools and police protection services.
 - (iv) The PD Plan shall demonstrate compliance with the parks and recreation requirements of Sec. Article 14: *Parks, Recreation, Greenways, and Open Space* and Sec. 7.3.1 *Privately-owned Play Lawns* if there is a residential component in the PUD-CZ.
- g) Natural resource and environmental protection. The PD Plan for PUD-CZ demonstrates compliance with the current regulatory standards of this Ordinance related to natural resource and environmental protection in Sec. 6.1 Watershed Protection Overlay District, Sec. 6.2 Flood Damage Prevention Overlay District, and Sec. 8.1 Resource Conservation.

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- h) Storm water management. The PD Plan shall demonstrate that the post-development rate of onsite storm water discharge from the entire site shall not exceed pre-development levels in accordance with Sec. 6.1.7 of the UDO.
- i) Phasing. The PD Plan for PUD-CZ shall include a phasing plan for the development. If development of the PUD-CZ is proposed to occur in more than one phase, then guarantees shall be provided that project improvements and amenities that are necessary and desirable for residents of the project, or that are of benefit to the Town, are constructed with the first phase of the project, or, if this is not possible, then as early in the project as is technically feasible.
- j) Consistency with 2045 Land Use Map. The PD Plan for PUD-CZ demonstrates consistency with the goals and policies established in the Town's 2045 Land Use.
- k) Complies with the UDO. The PD Plan for PUD-CZ demonstrates compliance with all other relevant portions of the UDO.

CONDITIONAL ZONING STANDARDS:

The Planning Board shall find the Planned Unit Development-Conditional Zoning (PUD-CZ) designation demonstrates compliance with the following standards. 2.3.3.F:

Legislative Considerations

The applicant shall propose site-specific standards and conditions that take into account the following considerations, which are considerations that are relevant to the legislative determination of whether or not the proposed conditional zoning district rezoning request is in the public interest. These considerations do not exclude the legislative consideration of any other factor that is relevant to the public interest.

- 1) Consistency with 2045 Land Use Map. The proposed Conditional Zoning (CZ) District use's appropriateness for its proposed location and consistency with the purposes, goals, objectives, and policies of the 2045 Land Use Map.
- 2) Compatibility. The proposed Conditional Zoning (CZ) District use's appropriateness for its proposed location and compatibility with the character of surrounding land uses.
- 3) *Zoning district supplemental standards*. The proposed Conditional Zoning (CZ) District use's compliance with Sec 4.4 *Supplemental Standards*, if applicable.
- 4) Design minimizes adverse impact. The design of the proposed Conditional Zoning (CZ) District use's minimization of adverse effects, including visual impact of the proposed use on adjacent lands; and avoidance of significant adverse impacts on surrounding lands regarding trash, traffic, service delivery, parking and loading, odors, noise, glare, and vibration and not create a nuisance.
- 5) Design minimizes environmental impact. The proposed Conditional Zoning District use's minimization of environmental impacts and protection from significant deterioration of water and air resources, wildlife habitat, scenic resources, and other natural resources.

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- 6) *Impact on public facilities.* The proposed Conditional Zoning (CZ) District use's avoidance of having adverse impacts on public facilities and services, including roads, potable water and wastewater facilities, parks, schools, police, fire and EMS facilities.
- 7) *Health, safety, and welfare.* The proposed Conditional Zoning (CZ) District use's effect on the health, safety, or welfare of the residents of the Town or its ETJ.
- 8) Detrimental to adjacent properties. Whether the proposed Conditional Zoning (CZ) District use is substantially detrimental to adjacent properties.
- 9) Not constitute nuisance or hazard. Whether the proposed Conditional Zoning (CZ) District use constitutes a nuisance or hazard due to traffic impact or noise, or because of the number of persons who will be using the Conditional Zoning (CZ) District use.
- 10) Other relevant standards of this Ordinance. Whether the proposed Conditional Zoning (CZ) District use complies with all standards imposed on it by all other applicable provisions of this Ordinance for use, layout, and general development characteristics.



May 24, 2019

Kevin Dean, PE Kimley-Horn and Associates, Inc. 421 Fayetteville St, Suite 600 Raleigh, NC 27601

Subject: Staff summary and comments for The Wayforth at Apex TIA and

Addendum, 04/30/2019

Mr. Dean:

Please review the following summary of my comments and recommendations. You may schedule a meeting with me and your client to discuss at your convenience.

Study Area

The TIA proposes to study access to the development at the following two intersections.

- Morris Acres Road and Reedybrook Crossing/North Site Driveway (unsignalized)
- Morris Acres Road and South Site Driveway (unsignalized)

The following intersections were also studied in the TIA:

- Jenks Road and Morris Acres Road (unsignalized)
- Morris Acres Road and Creekside Landing Drive (signalized)
- Morris Acres Road and US 64 Westbound (unsignalized)

Trip Generation

The proposed Wayforth at Apex development is anticipated to consist of 300 apartments. The development is anticipated to generate approximately 26 new trips entering and 74 new trips exiting the site during the weekday A.M. peak hour and 77 new trips entering and 50 new trips exiting the site during the P.M. peak hour. The development is expected to add a total of 1,634 new weekday trips to the adjacent roadway network.

Background traffic

Background traffic consists of 3% annual background traffic growth compounded to build out year 2022. Additionally 10% of site trips from the adjacent Beaver Creek Residential development (540 Townes) are included in the analysis, as the development was almost entirely built-out at the time this study was completed.

Trip Distribution and Assignment

The distribution to and from the development are as follows:

- 25% from the east on US 64
- 25% from the east on Jenks Road
- 25% from the west on Jenks Road
- 25% from the south on Creekside Landing Drive
- 50% to the east on Jenks Road
- 25% to the west on US 64
- 15% to the south on Creekside Landing Drive
- 10% to the west on Jenks Road

<u>Traffic Capacity Analysis and Recommendations</u>

Level of Service (LOS) is a grade of A through F assigned to an intersection, approach, or movement to describe how well or how poorly it operates. LOS A through D is considered acceptable for peak hour operation. LOS E or F describes potentially unacceptable operation and developers may be required to mitigate their anticipated traffic impact to improve LOS based on the Apex Unified Development Ordinance (UDO).

Tables 1 through 5 describe the levels of service (LOS) for the scenarios analyzed in the TIA. "*NA*" is shown when the scenario does not apply. The scenarios are as follows:

- Existing 2018 Existing year 2018 traffic.
- No Build 2022 Projected year (2022) with background growth, and approved development traffic from others.
- **Build 2022** Projected year (2022) with background traffic and site build-out traffic including recommended improvements where applicable.

Morris Acres Road and Reedybrook Crossing/North Site Driveway (unsignalized)

Table 1. A.M. / P.M. Unsignalized Peak Hour Levels of Service Morris Acres Road and Reedybrook Crossing/North Site Driveway						
Existing No Build 2018 Build 2						
<u>Overall</u>	<u>NA</u>	<u>NA</u>	<u>NA</u>			
Eastbound (Reedybrook Crossing)	A / B ²	A / B ²	B/B ²			
Westbound (North Site Driveway)	NA	NA	B/B ²			
Northbound (Morris Acres Road)	A/A^1	A/A^1	A/A^1			
Southbound (Morris Acres Road)	NA	NA	A/A^1			

- 1. Level of service for major street left turn movements
- 2. Level of service for minor street stop controlled approaches

TIA recommendations:

 The TIA recommends construction of a full movement stop-controlled site driveway with one lane of ingress and one lane of egress that aligns with Reedybrook Crossing at Morris Acres Road.

Apex staff recommendations:

Apex staff agree with the recommendation. All approaches at this intersection are
projected to operate at LOS B or better with no operational issues in the build condition.
Morris Acres Road already provides a left turn lane both directions at the proposed
driveway since it was constructed as a three-lane road.

Morris Acres Road and South Site Driveway (unsignalized)

Table 2. A.M. / P.M. Unsignalized Peak Hour Levels of Service Morris Acres Road and South Site Driveway				
Build 2022				
<u>Overall</u>	<u>NA</u>			
Westbound (South Site Driveway)	A / B ²			
Northbound (Morris Acres Road)	NA			
Southbound (Morris Acres Road)	A/A^1			

- 1. Level of service for major street left turn movements
- 2. Level of service for minor street stop controlled approaches

TIA recommendations:

 The TIA recommends construction of a full movement stop-controlled site driveway with one lane of ingress and one lane of egress approximately 500 feet south of Reedybrook Crossing.

Apex staff recommendations:

Apex staff agree with the recommendation. All approaches at this proposed intersection
are projected to operate at LOS B or better with no operational issues in the build
condition. Morris Acres Road already provides a left turn lane at the proposed driveway
since it was constructed as a three-lane road.

Jenks Road and Morris Acres Road (unsignalized)

Table 3. A.M. / P.M. Unsignalized Peak Hour Levels of Service Jenks Road and Morris Acres Road							
Existing No Build 2022 Build 2022							
<u>Overall</u>	<u>NA</u>	<u>NA</u>	<u>NA</u>				
Eastbound (Jenks Road)	NA	NA	NA				
Westbound (Jenks Road) A / A ¹ A / A ¹ A / A ¹							
Northbound (Morris Acres Road)	Northbound (Morris Acres Road) B/C ² B/C ²						

- 1. Level of service for major street left turn movements
- 2. Level of service for minor street stop controlled approaches

TIA recommendations:

• The TIA does not recommend any improvements at this intersection. All approaches are anticipated to operate at LOS C or better with or without the development.

Apex staff recommendations:

 Apex staff agree with the recommendation. Left turn lanes are already provided on both roadways and no additional turn lanes are recommended.

Morris Acres Road and Creekside Landing Drive (Signalized)

Table 4. A.M. / P.M. Signalized Peak Hour Levels of Service Morris Acres Road and Creekside Landing Drive						
Existing No Build 2022 Build 2022						
<u>Overall</u>	<u>A / A</u>	<u>A / A</u>	<u>A / A</u>			
Eastbound (Reedybrook Crossing)	A/A	A/A	A/A			
Northbound (Morris Acres Road)	A/A	A/A	A/A			
Southbound (Morris Acres Road)	A/A	A/B	A/B			

TIA recommendations:

 The TIA does not recommend any improvements at this signalized intersection. All approaches are anticipated to operate at LOS B or better with or without the development.

Apex staff recommendations:

• Apex staff agree with the recommendation. There are already left turn lanes provided on both roadways and no additional turn lanes are recommended.

Morris Acres Road and US 64 Westbound (unsignalized)

Table 5. A.M. / P.M. Unsignalized Peak Hour Levels of Service Morris Acres Road and US 64 Westbound						
Existing No Build 2022 Build 2022						
<u>Overall</u>	<u>NA</u>	<u>NA</u>	<u>NA</u>			
Westbound (US 64)	NA	NA	NA			
Southbound (Morris Acres Road)	D/D^1	E/E¹	E/E¹			

1. Level of service for minor street stop controlled approaches

TIA recommendations:

The TIA does not recommend any improvements at this channelized intersection.

Apex staff recommendations:

The right-out onto westbound US-64 is anticipated to operate at LOS E with average delays of over 45 seconds per vehicle and 95th percentile queues of 5 vehicles in both the A.M. and P.M. peak hours. Staff concurs with no roadway improvements being

required of this site approval according to the requirements of the UDO. However, the Town or NCDOT should consider future construction of an 800-foot long acceleration lane with a 300-foot taper on US 64 and lengthening of the right-turn deceleration lane. Town staff have already submitted these improvements to NCDOT for consideration of funding under one of their grant programs. The acceleration lane will eliminate the stop condition and the delays associated with the stop on Morris Acres Road while the longer deceleration lane will provide improved operations for the exit from US 64.

Please coordinate with the NCDOT District Engineer's Office concerning recommended improvements. Town staff will be available for meetings with NCDOT staff to discuss improvements on state maintained roadways as needed. All recommendations are subject to review by Town Council prior to approval.

Sincerely,

Serge Grebenschikov

Traffic Engineer 919-372-7448



PLANNED UNIT DEVELOPMENT APPLICATION							
This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.							
Application	#: 19CZ02			Submittal Dat	e:	2/1/2019	
Fee Paid	\$ 2370.00			Check #			
PETITION 1	O AMEND THE OFFICIAL	ZONING DISTRICT	MAP				
Project Nar	M : A DI		NAME OF TAXABLE PARTY.				
Address(es)	0.7000 1.70		es Ros	ad			
	0732-28-9587; 0732-38-2530; and 0732-38-2709						
PIN(S)	102 20 0001, 0102-0	50-2550, and 0	702-00	2103		1-	7.44
Current 7or	ning: RR		D		PUD-CZ		
Current Zor		Medium Dens	_	sed Zoning:	1 00-02	-	
	30 LUM Designation:	Medium Dens					
	2030 LUM Designation: e next page for LUM amen		nty 1163	nucritiai			
	on of the project is shown		nore stri	pes on the 20	30 Land Use	Map) provide	e the following:
Are	ea classified as mixed use:			Ac	reage:		
	ea proposed as non-resider	ntial develonment:			reage:		
	rcent of mixed use area pro	•	lontial:		rcent:		
- 1 - 20		sposed as non resid	Cittai.	16			
Applicant I				-		والتصورو	
Name:	Kaplan Residential			7-7-7-4-7-4-7-4-7-4-7-4-7-4-7-4-7-4-7-4			
Address:	1111 Kane Concour						
City:	Bay Harbor Islands		State:	FL		Zip:	33154
Phone:	305.901.2202	E	E-mail:				
Owner Info	rmation			4 3 2 1			37 1 3 17
Name:	Edith S Morris						
Address:	7208 Morris Acres F	Road					
City:	Apex	S	State:	NC		Zip:	27523
Phone:			E-mail:				
Agent Infor						1 7 - 7	
Name:	Jason Barron	4104 500					
Address:	421 Fayetteville Str	eet Ste 530					
City:	Raleigh		State:	NC		Zip:	27601
Phone:	919-590-0371		E-mail:	jbarron@r		arlawgrou	p.com
Other conta	Nil Ghosh - no	ghosh@morning	gstarla	wgroup.co	m		
			· · · -				

PLANNED UNIT DEVELOPMENT APPLICAT	TION		- 52 17 17 17 17				
Application #: 19CZ02		Submittal Date:	2/1/2019				
2030 LAND USE MAP AMENDMENT (if a	pplicable)						
The applicant does hereby respectfully request, the following facts are shown:	uest the Town	n Council amend the 2030 Land U	se Map. In support of this				
The area sought to be amended on the 20	030 Land Use	Map is located at:					
0, 7208, and 7208B Morris Ac	res Road						
		4-1-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-					
Current 2030 Land Use Classification:	Mediun	n Density Residential					
Proposed 2030 Land Use Classification:	Mediun	ledium Density Residential					
What conditions justify the passage of the classifications of the subject area in addit		-	iscuss the existing use				

CERTIFIED LIST OF NEIGHBORING PROPERTY OWNERS

Application #:	19CZ02	Submittal Date:	2/1/2019	
----------------	--------	-----------------	----------	--

Provide a certified list of property owners subject to this application and all property owners within 300' of the subject property and HOA Contacts.

	Owner's Name	PIN		Owner's Name	PIN
		0732-37-1960;		4-141-0-0-3700 (300) (300)	
		0732-37-4927;			
		0732-37-5626;			
		0732-38-0119;			
		0732-38-1079;		MADHVANI, VIRAT K MADHVANI,	
1	540 TOWNES HOA, INC	0732-38-2217	32	KAJAL V	0732-37-3978
	·		-	MAGNOLIA PROPERTY	
2	ADDEN, NICOLE	0732-39-8073	33	MANAGEMENT LLC	0732-37-4900
3	ALPS LP	0732-38-7152	34	MCSWAIN, CHASE ALEXANDER	0732-38-1102
	ANKNEY, CHRISTINA L ANKNEY, JOHN		-	MISTRY, DHANSUKH MISTRY, SHILA	
4	ASTOR	0732-38-8153	35	D	0732-38-1071
		·	-		0732-28-9587;
		0732-38-7570;			0732-38-2530;
5	APEX TOWN OF	0732-39-7559	36	MORRIS, EDITH S	0732-38-2709
			-		0732-29-5482;
6	BARK, GI CHAN	0732-38-1244	37	MORRIS, WILMA LEE	0732-29-8556
			-	MULCAHY, JOHN M MULCAHY,	0.02 23 0330
7	BEAVER CREEK CROSSING LLC	0732-28-6392	38	MICHELE A	0732-39-6483
					0732-29-5017;
8	BECKER, GARY A BECKER, BARBARA J	0732-38-9017	39	NC DOT TRNPK AUTHORITY	0732-28-4334
_	BUTTERWORTH, JAMES D	0,02 00 001,	-	THE SECTION AND THE SECTION AN	0732 20-4334
9	BUTTERWORTH, BARBARA A	0732-39-8254	40	PARKER, DAVID PARKER, ROBYN	0732-38-7613
	CAUTHEN, JOHNSON JR CAUTHEN,	0732 33 0234	- 10	PETERSON, DAVID R PETERSON, GAIL	0/32-36-7013
10	DEBORAH	0732-39-6202	41	C	0722 20 0244
10	DEBOTALL	0732-33-0202	. 41	PULIJALA, DHEERAJ KUMAR	0732-39-8344
11	CHEN, WANLING	0732-38-2191	42	PULIJALA, CHAITANYA	0732-38-3061
12	CHING, LEUNG YIN	0732-38-9347	43	PYNE, CRAIG A GALIEN, KIMBERLY L	0732-39-6402
		0,02 00 00 11	. '-	RAJARAM, NARAYAN K	0732-33-0402
13	CHO, EUNA K CHO, REX H	0732-38-2024	44	UTHAMARAJAN, ARTHI	0732-38-1216
20	CITISIDE AT BEAVER CREEK	0732 30 2024		OTTAWAWOAN, ARTTI	0732-36-1210
	CROSSING HOA INC				
14	(Charleston Mngmt)	0732-37-7766	45	RAMSEY, FRANCES B	0722 20 21 47
14	DIAZ, CYNTHIA I COLON CADENA,	0732-37-7700	. 43	TAIVISET, TRAIVES B	0732-38-2147
15	ARGYL I RAMIREZ	0732-38-0249	46	SAFIAN, DAVID SAFIAN, MICHELLE	0722 20 0200
13	FALKANGER, JEFFREY J FALKANGER,	0732-36-0243	. 40	SAFIAN, DAVID SAFIAN, MICHELLE	0732-38-8289
16	KERRY C	0722 20 7014	47	CARTORI ICANETTE	0700 00 0400
10	KERKY C	0732-39-7014	47	SARTORI, JEANETTE	0732-28-9182
17	CAVIES ANTHONY DADON	0722 20 0260	4.0	SIMMONS, RYAN KENNETH	
17	GAYLES, ANTHONY DARON	0732-38-0268	. 48	SIMMONS, KRYSTAL MARIE	0732-38-7292
18	GREEN ACRES OF APEX LLC	0732-39-3853	49	SINGH, ISHA	0732-38-1092
	GROSSER, DONALD B JR GROSSER,			SMITH, DERMOT J SMITH, JENNIFER	
19	CYNTHIA S	0732-38-9588	50	R	0732-38-2164
	HARPER, PAUL MARK HARPER,		•		
20	RENAE KEY	0732-39-6197	51	ST AMANT, STEVEN	0732-38-1272
	HOUSTON, MICHAEL J HOUSTON,			STEVENS, GREGORY W STEVENS,	
21	KRISTIN A	0732-38-0310	52	YOKO FUSE	0732-38-7923
22	ISAACS, DANIEL J	0732-48-0708	53	V & V PROPERTY GROUP LLC	0732-37-3986
23	KAPLAN, PETER KAPLAN, ERIN B	0732-39-7472	54	VACCA, STACY ELLEN	0732-38-1121
23	KOESTER, JOHN D KOESTER,	0102 05 1712		VOJTICEK, BRANDON M VOJTICEK,	0/32-30-1121
24	JOHANNA P	0732-38-9603	55	LEIGH ANN	0732-38-7723
25	LAO, TERENCE LAO, CATHERINE	0732-38-3044	56	WARD, JUDITH F	0732-48-0456
	, ,			,	J, JL 40 0430

	LAXMANA, RAJINEESH KUMAR		-		
	VUMMIDISINGH LAXMANA, SREE				
26	HARSHITHA VUMMIDISINGH	0732-28-9185	_ 57	WEISS, GEOFFREY L	0732-38-7823
27	LIN, SEN	0732-38-0174	_ 58	WEST, DONALD EUGENE II	0732-38-0287
20	THE VINCHE VINCHIA	0722 20 2440		WILLIAMS, STACEY D WILLIAMS,	
28	LIU, XINGJUN XING, JUN	0732-38-2119	_ 59	JOHN C	0732-39-8164
29	LU, HAIRONG	0732-38-1049	_ 60	WRIGHT, STEVEN C	0732-38-3018
30	LU, XIAOYUAN WAN, PENG	0732-37-3993	61	ZHANG, DONG	0732-38-2173
31	LUO, JING OUYANG, WEN	0732-38-1281			
1. Ja	son L. Barran	م ماه د گذشت	اعتملت	is an assumate listing of all	
/	.)		this	is an accurate listing of all prope	rty owners and
property	owners within 300' of the subject	ct property.			
	11.1 10 2019				
Date:	October 18, 2019	By:		//	
			//		
COUNT	Y OF WAKE STATE OF NORTH CAI	ROLINA			
Sworn	and subscribed before me,	wat. Zo.	rib	, a Notary Public for the	e above State and
	, on this the 30^{4} day of 0	ا مادادها	20	19	
Country	, or this the day or	CX 010 (2)	-, 29		
			(_	Para U. En	
				Notary Public	
SEAL				Paula H. Zorio	
				Print Name	
		M	ly Cor	mmission Expires: <u>علما</u>	24
				30	
				PAULA H. ZO	RIO
				PAULA II. 20	110

PAULA H. ZORIO NOTARY PUBLIC WAKE COUNTY, N.C.

DEVELOPMENT NAME APPROVAL APPLICATION

Application #:

191202

Submittal Date:

2/1/19

Fee for Initial Submittal: No Charge

Fee for Name Change after Approval: \$500*

Purpose

To provide a consistent and clearly stated procedure for the naming of subdivisions and/or developments and entrance roadways (in conjunction with *Town of Apex Address Policy*) so as to allow developers to define and associate the theme or aesthetics of their project(s) while maintaining the Town's commitment to preserving the quality of life and safety for all residents of Apex proper and extraterritorial jurisdiction.

Guidelines

- ✓ The subdivision/development name shall not duplicate, resemble, or present confusion with an existing subdivision/development within Apex corporate limits or extraterritorial jurisdiction except for the extension of an existing subdivision/development of similar or same name that shares a continuous roadway.
- ✓ The subdivision/development name shall not resemble an existing street name within Apex corporate limits or extraterritorial jurisdiction unless the roadway is a part of the subdivision/development or provides access to the main entrance.
- ✓ The entrance roadway of a proposed subdivision/development shall contain the name of the subdivision/development where this name does not conflict with the Town of Apex Road Name Approval Application and Town of Apex Address Policy guidelines.
- ✓ The name "Apex" shall be excluded from any new subdivision/development name.
- Descriptive words that are commonly used by existing developments will be scrutinized more seriously in order to limit confusion and encourage distinctiveness. A list of commonly used descriptive words in Apex's jurisdiction is found below.
- ✓ The proposed subdivision/development name must be requested, reviewed and approved during preliminary review by the Town.
- √ A \$500.00 fee will be assessed to the developer if a subdivision/development name change is requested
 after official submittal of the project to the Town.*

*The imposed fee offsets the cost of administrative changes required to alleviate any confusion for the applicant, Planning staff, other Town departments, decision-making bodies, concerned utility companies and other interested parties. There is no charge for the initial name submittal.

Existing Development Titles, Recurring

	Residential	Non-Residential
10 or more	Creek, Farm(s), Village(s),	Center/Centre
6 to 9	Crossing(s), Park, Ridge, Wood(s)	Commons, Park
3 to 5	Acres, Estates, Glen(s), Green*, Hills	Crossing(s), Plaza, Station, Village(s)

^{*}excludes names with Green Level

Application #: $19C202$ S	Submittal Date: $2/1/19$							
Proposed Subdivision/Development Information								
Description of location: 0, 7208, and 7208B Morris Acres F	Description of location: 0, 7208, and 7208B Morris Acres Rd							
Nearest intersecting roads: Morris Acres Road at Reedybr	rook Crsg							
Wake County PIN(s): 0732-28-9587; 0732-38-2530; and 07	32-38-2709							
Township: White Oak								
Contact Information (as appropriate)								
Contact person: Jason Barron								
Phone number: 919-590-0371 Fax number	er:							
Address: 421 Fayetteville Street Ste 530, Raleigh, NC 276								
E-mail address: jbarron@morningstarlawgroup.com								
-								
Owner:								
Phone number: Fax number								
Address:								
E-mail address:								
Proposed Subdivision/Development Name								
1 st Choice:								
2 nd Choice (Optional):]							
Town of Apex Staff Approval:								
Town of Apex Planning Department Staff	Date							

DEVELOPMENT NAME APPROVAL APPLICATION

STREET NAME APPROVAL APPLICATION 19CZ02 2/1/2019 Application #: Submittal Date: Wake County Approval Date: **Guidelines:** No names duplicating or sounding similar to existing road names Avoid difficult to pronounce names No individuals' names • Avoid proper names of a business, e.g. Hannaford Drive Limit names to 14 characters in length No directionals, e.g. North, South, East, West No punctuation marks, e.g. periods, hyphens, apostrophes, etc. · Avoid using double suffixes, e.g. Deer Path Lane All names must have an acceptable suffix, e.g. Street, Court, Lane, Path, etc. Use only suffixes which are Town of Apex approved · Town of Apex has the right to deny any street name that is determined to be inappropriate Information: Description of location: 0, 7208, and 7208B Morris Acres Rd Nearest intersecting roads: Morris Acres Road at Reedybrook Crsg Wake County PIN(s): 0732-28-9587; 0732-38-2530; and 0732-38-2709 Township: White Oak Contact information (as appropriate) Contact person: Jason Barron Phone number: 919-590-0371 Fax number: Address: 421 Fayetteville Street | Ste 530, Raleigh, NC 27601 E-mail address: jbarron@morningstarlawgroup.com Edith S Morris Owner:

Phone number:

Address: 0, 7208 and 7208B Morris Acres Road

E-mail address:

Fax number:

Application #: Submittal Date: # of roads to be named: Please submit twice as many road names as needed, with preferred names listed first. Proposed road names should be written exactly as one would want them to appear. Town of Apex Planning Department staff will send all approved street names to the Wake County GIS Department for county approval. Please allow several weeks for approval. Upon approval Wake County GIS – Street Addressing will inform you of the approved street names. Example: Road Name Suffix Hunter Street 1 Nathan Drive 11 2 12 3 13 4 14 5 ______ 15 _____ 6 16 7 ______ 17 _____ 8 18 9 ______ 19 ______ 10 20 TOWN OF APEX STAFF APPROVAL Town of Apex Staff Approval Date WAKE COUNTY STAFF APPROVAL: GIS certifies that names indicated by checkmark ☑ are approved. Please disregard all other names. Comments: Wake County GIS Staff Approval Date

STREET NAME APPROVAL APPLICATION

TOWN OF APEX UTILITIES OFFER AND AGREEMENT

Application #:		Submittal Date:	
	P.O. Bo	Town of Apex 73 Hunter Street ox 250 Apex, NC 27502 919-249-3400 ROLINA CUSTOMER SELECTION AGREEMENT	
	0, 7208, and 7208B Morris Ac	cres Rd	
		(the "Premises")	
you accept the Tow the Town. Kaplan Residenti Fown of Apex (the	on's offer, please fill in the blanks or al, the undersigned	electric utilities on the terms described in this Offer & Agreement. If in this form and sign and we will have an Agreement once signed by dicustomer ("Customer") hereby irrevocably chooses and selects the supplier for the Premises. Permanent service to the Premises will be	
		by Customer at the Premises shall be subject to, and in accordance ce regulations, policies, procedures and the Code of Ordinances of the	e
the requested serv	ice. By signing this Agreement the i	upon this Agreement, will take action and expend funds to provide undersigned signifies that he or she has the authority to select the porary power, for the Premises identified above.	
•	onal terms and conditions to this Aputes the entire agreement of the pa	agreement are attached as Appendix 1. If no appendix is attached thi arties.	S
Acceptanc	e of this Agreement by the Town co	onstitutes a binding contract to purchase and sell electric power.	
Please not supplier for the Pre		ral Statute §160A-332, you may be entitled to choose another electric	С
	eptance of this Agreement, the Tow iises and looks forward to working v	on of Apex Electric Utilities Division will be pleased to provide electric with you and the owner(s).	
ACCEPTED:			
CUSTOMER:		TOWN OF APEX	
BY:		BY:	
	Authorized Agent	Authorized Agent	
DATE:		DATE:	_

AGENT A	A UTHORIZATIO	N FORM				
Application #: Edith S Morris			Submittal Date:	is the owner of the property for which the attached		
			is the owner of the property for			
applica	tion is being su	bmitted:				
	Land Use An Rezoning Site Plan Subdivision Variance Other:	nendment				
The agen	erty is located a	ct is: Jason E				
☐ I am	ime:	Jason Barron	d will be acting as my own agent			
Address:		421 Fayettevill	le Street Ste 530, Raleigh, NC 27601			
Telephor	ne Number:	919-590-0371				
Fax Num	ber:					
E-Mail A	ddress:	jbarron@morn	ingstarlawgroup.com			
		Signature(s) of Edith	of Owner(s) L. S. Marris Type or print name	1-28-2019 Date		
	-		Type or print name	Date		
		× 	Type or print name	Date		

Attach additional sheets if there are additional owners.

*Owner of record as shown on the latest equalized assessment rolls of Wake County. (An option to purchase does not constitute ownership). If ownership has been recently transferred, a copy of the deed must accompany this authorization.

	OTICE OF NEIGHBORHOOD I		
or dis	document is a public record under the North Carolina Publ closed to third parties. 8/19	ic Records Act and may be pub	olished on the Town's website
Dat	re e		
	Neighbor: are invited to a neighborhood meeting to review ar	nd discuss the development	t proposal at
0, 7	208 & 7208B Morris Acres Road	0732-28-9587; 0732-3	8-2530; and 0732-38-2709
	Address(es)	P	PIN(s)
oppo subr Deve	hborhood organizations before the submittal of an ortunity to raise questions and discuss any concerns nitted. Once an application has been submitted to elopment Map or the Apex Development Regularization. V.apexnc.org. Eighborhood Meeting is required because this proje	about the impacts of the pot the Town, it may be traceout located on the Tow	roject before it is officially ked using the <u>Interactive</u> vn of Apex website a
Ар	olication Type		Approving Authority
V	Rezoning (including Planned Unit Development)		Town Council
	Major Site Plan		Town Council (QJPH*)
	Special Use Permit		Town Council (QJPH*)
▼	Residential Master Subdivision Plan (excludes exe	mpt subdivisions)	Technical Review Committee (staff)
*(Quasi-Judicial Public Hearing: The Town Council can	not discuss the project prio	r to the public hearing.
	following is a description of the proposal (also see a wresidential community including up to 122 townhome		1 17
Est	mated submittal date: November 1		
	ETING INFORMATION:		
	martin Oursella marra/a). Edith S Morris		

Property Owner(s) name(s):

Applicant(s):

Kaplan Residential

Contact information (email/phone):

jbarron@morningstarlawgroup.com/919-590-0371

Meeting Address:

6175 Old Jenks Road, Apex, NC 27523

Date of meeting**:

10/29/19

Time of meeting**:

6:15PM

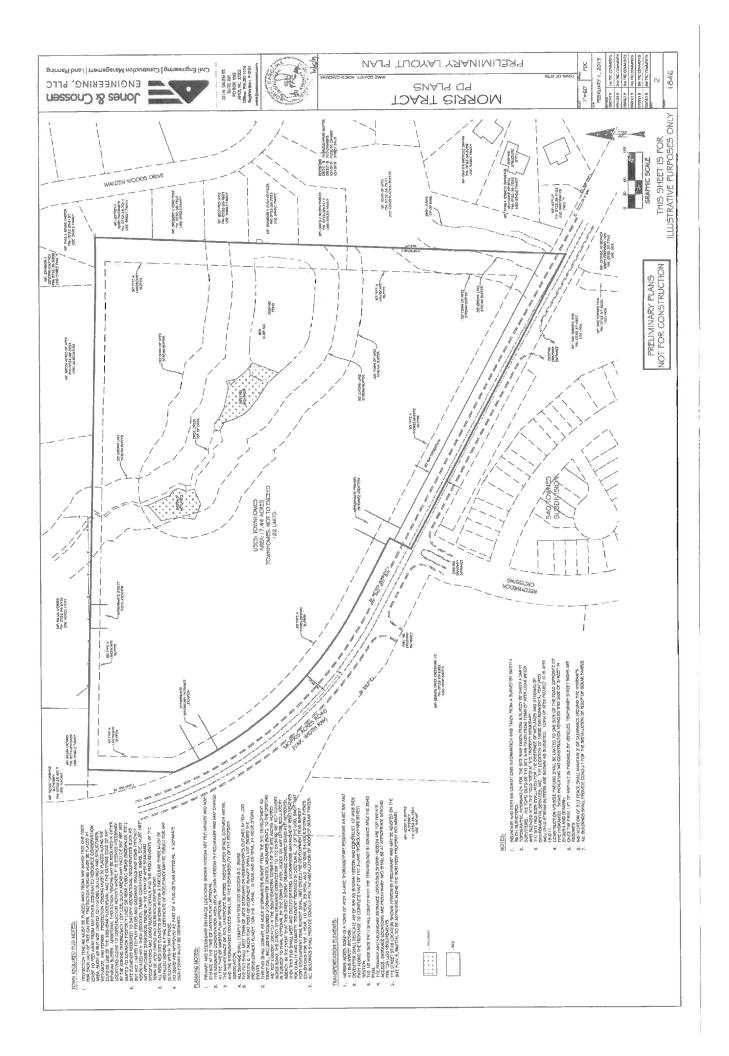
MEETING AGENDA TIMES:

Welcome: 6:15 PM - 6:18 PM

Project Presentation: 6:18 PM - 6:25 PM

Question & Answer: 6:25 PM - end

^{**}Meetings shall occur between 5:00 p.m.-9:00 p.m. on a Monday through Thursday (excluding Town recognized holidays). If you have questions about the general process for this application, please contact the Planning Department at 919-249-3426. You may also find information about the Apex Planning Department and on-going planning efforts at http://www.apexnc.org/180/Planning.



SUMMARY OF DISCUSSION FROM THE NEIGHBORHOOD MEETING This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Property Owner(s) name(s):	Edith Morris		
Applicant(s):	Kaplan Residential c/o Jason Barron (jbarron@morningstarlawgroup.com)		
Contact information (email/phone):			
Meeting Address:	Hope Chapel, 6175 Old Jenks Road, Apex		
Date of meeting: October 29, 201	9 Time of meeting: 6:15 p.m.		
below (attach additional sheets, if new any concerns. The response should no	ments and your response from the Neighborhood Meeting in the spaces cessary). Please state if/how the project has been modified in response to ot be "Noted" or "No Response". There has to be documentation of what was given and justification for why no change was deemed warranted.		
	en the decision was made to develop at a much lower density.		
	s. The apartments allowed Site to be compressed, because the units		
	se plan requires more land area.		
Question/Concern #2: When do you plan to provide elevation	ons?		
Applicant's Response: At Master Subdivision submittal.	Neighborhood meeting is required at that time.		
Question/Concern #3: What is type of buffer is a Type A Bu	ıffer?		
Applicant's Response: Most dense buffer required per A vegetated areas.	Apex UDO. Supplemental plantings may be required in existing		
Question/Concern #4: Which building heights will be consid	lered adjacent to existing homes?		
Applicant's Response: Maximum building height is 45'.			

SUMMARY OF DISCUSSION FROM THE NEIGHBORHOOD MEETING

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties. **Edith Morris** Property Owner(s) name(s): Applicant(s): Kaplan Residential c/o Jason Barron (jbarron@morningstarlawgroup.com) Contact information (email/phone): **Meeting Address:** Hope Chapel, 6175 Old Jenks Road, Apex Date of meeting: October 29, 2019 Time of meeting: 6:15 p.m. Please summarize the questions/comments and your response from the Neighborhood Meeting in the spaces below (attach additional sheets, if necessary). Please state if/how the project has been modified in response to any concerns. The response should not be "Noted" or "No Response". There has to be documentation of what consideration the neighbor's concern was given and justification for why no change was deemed warranted. Question/Concern #1: Discussion on traffic Applicant's Response: Entrance will be across from Apartments on other side of street. Question/Concern #2: Discussion on sewer Applicant's Response: We have to tie into existing sewer, will know at time of Site Plan if will be permitted to bury the line under the creek. Question/Concern #3: Question raised about 50' buffer adjacent to existing homes Applicant's Response: Yes, 50' Type A Buffer adjacent to all existing single family homes. Question/Concern #4: Applicant's Response:

NEIGHBORHOOD MEETING SIGN-IN SHEET

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Meeting Addres	Hope Chapel, 6175 Old Jenks Road, Apex		
Date of meeting	g: October 29, 2019	Time of meeting: 6:15pm	
Property Owner	r(s) name(s): Edith Morris		
Applicant(s): _	Kaplan Residential		

Please <u>print</u> your name below, state your address and/or affiliation with a neighborhood group, and provide your phone number and email address. Providing your name below does not represent support or opposition to the project; it is for documentation purposes only.

	NAME/ORGANIZATION	ADDRESS	PHONE #	EMAIL	SEND PLANS & UPDATES
1.	ANNA COITEN	1602 SHEPHERDS 94			
2.	haren Arbramson	2107 Ock Stream Ln.			
	Matt Abramen	11			-
4.	Susan Cormier	1509 Poets Glade Dr.			
5.	Parti Edewords	1512 Poets Glade Dr.			
6.	Andrew George	2314 Walde Creek Dr			
7.	Eem Kadan	2524 Walden woods			<u>-</u>
8.	Varda Carlaces	2416 Flints Pond Cir			-
9.	Carl Aterson	2502 战战			
10.	Michelemulcaty	2526 warden woods			
11.	Olbi Courther	2521 Wolder woods			
12.	DonGrosser	2503 Valden Woods			
13.	Dan Iraay	2500 Malder Moods			
14.	RYAM SIMMONS	2508 FLINTS POND CIR			

Use additional sheets, if necessary.

NEIGHBORHOOD MEETING SIGN-IN SHEET

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Meeting Address:	Hope Chapel, 6175 Old Jenks Road, Apex				
Date of meeting:	October 29, 2019	Time of meeting:	6:15pm		
Property Owner(s) name(s): Edith Morris					
Applicant(s): Kap	olan Residential				

Please <u>print</u> your name below, state your address and/or affiliation with a neighborhood group, and provide your phone number and email address. Providing your name below does not represent support or opposition to the project; it is for documentation purposes only.

	NAME/ORGANIZATION	ADDRESS	PHONE #	EMAIL	SEND PLANS & UPDATES
1.	Doug Keiter	2213 Colony Woods Dr			
2.	Marka Bence Harper	2519 Wilden Woods Dr			
3.	John Koester	2505 walder woods D			
4.	John Williams	2518 Worlder Woods)			-
5.	Doug Dickwann	1 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1			
6.	V 0			10.0000 10 10 11 10 11 10 11	
7.					
8.					
9.					
10.					
11.					
12.					
13.		****			
14.		**************************************			
<u></u>	dia a da la casa de la				

Use additional sheets, if necessary.





Morris Acres PUD

PD PLAN

APEX, NORTH CAROLINA

Submitted: February 2019

Revised: February 28, 2019

Revised: May 10, 2019

Revised: June 7, 2019

Revised: June 27, 2019

Revised: July 31, 2019

Revised: August 6, 2019

Revised: August 23, 2019

Revised: October 30, 2019

PREPARED BY:



Section 1: Table of Contents - PUD Text

Section 1: Table of Contents

Section 2: Vicinity Map

Section 3: Project Data

Section 4: Purpose Statement

Section 5: Permitted Uses

Section 6: Design Controls

Section 7: Architectural Controls

Section 8: Parking and Loading

Section 9: Signage

Section 10: Natural Resource and Environmental Data

Section 11: Stormwater Management

Section 12: Parks and Recreation

Section 13: Public Facilities

Section 14: Phasing Plan

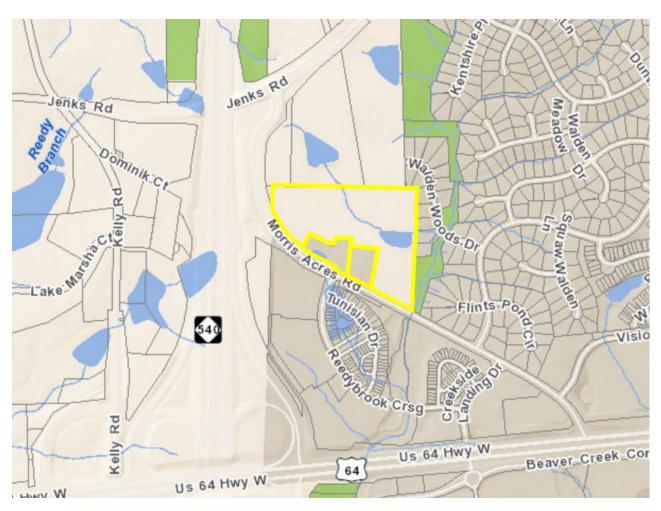
Section 15: Consistency with 2045 Land Use Plan

Section 16: Compliance with UDO

Section 17: Compliance with Apex Bicycle Plan

Section 18: Public Art

Section 2: Vicinity Map



The Morris Acres PUD is located in the Town of Apex, east of I-540, south of Jenks Road, and north of US-64. The properties are located on the north side of Morris Acres Road. To the north are large tracts of agricultural land with frontage on Jenks Road. Directly south is Morris Acres Road, and across Morris Acres Road is the 540 Townes Townhome community. East of the project site is the large Walden Creek single-family neighborhood built in the late 90's and early 2000's. Morris Acres Road and I-540 are directly west of the project.

Section 3: Project Data

A. Name of Project:

Morris Acres PUD

B. Property Owners:

Edith S. Morris

Prepared By:

Jason Barron, Partner Morningstar Law Group 421 Fayetteville St | Ste 530 Raleigh, NC 27601

C. Current Zoning Designation:

Rural Residential (RR)

D. Proposed Zoning Designation:

Planned Unit Development – Conditional Zoning (PUD-CZ)

E. Current 2045 Land Use Map Designation:

Medium Density Residential (< 7 units/acre)

F. Proposed 2045 Land Use Map Designation:

Medium Density Residential (< 7 units/acre)

G. Proposed Use

Up to 122 dwelling units and associated open space, recreational amenities and infrastructure.

H. Size of Project

Wake County Tax Identification Number	Acreage
0732-28-9587; 0732-38-2530; and 0732-	17.44
38-2709	acres

Section 4: Purpose Statement

The Morris Acres PUD development will be a townhome community with buildings that are up to three (3) stories in height. The maximum building height shall be forty-five feet (45') measured to the top of any pitched roof. For a site adjacent to existing four-story apartments and NC540 to the south and west, but also adjacent to single family dwellings to the east, the townhome style development at three stories and seven (7) dwelling units per acre is intended to provide a transition between the existing uses. Additionally, the property abuts a future transit corridor anticipated to be located along Morris Acres Road, so medium density use at seven (7) dwelling units per acre is advisable.

A fifty-foot (50') Type A buffer shall be established along the eastern boundary of the subject property to appropriately buffer the townhome units from the existing residential community and further bolster transitions.

This concept is consistent with the Town's stated PUD goal to provide site specific, high quality neighborhoods that exhibit natural feature preservation as well as compatibility with, and connectivity to, surrounding land uses. More specifically, this plan will:

- Allow uses that are compatible with Section 4.2.2, Use Table of the UDO
- Provide for the preservation of existing environmentally sensitive areas.
- Provide for site specific and appropriate stormwater controls that exceed the requirements of the UDO.
- Provide appropriate buffering and screening from the proposed use to the existing residential areas.
- Offer medium density near interstate I-540 in an area where there are not many options for the same.
- Provide development densities that support the Town's future transit corridor planning.
- Provide residential densities that
- Demonstrate dimensional standards that are consistent with the UDO, and where variations occur, said variations will be included herein and subject to Council approval.
- Provide a high-quality community that is linked by a network of connected streets and pedestrian sidewalks that promotes connectivity, walkability and healthy lifestyles.
- Exhibit character and quality that is compatible with surrounding communities, which is expected to enhance the value of surrounding land uses.
- Provide open space and walkable trails to promote pedestrian activity, while appropriately buffering adjacent residential areas

All site-specific standards and conditions of this PUD Plan shall be consistent with all Conditional Zoning (CZ) District standards set forth in the UDO Section 2.3.3, *Conditional Zoning Districts* and UDO Section 2.3.4.F.1, *Planned Unit Development (PUD-CZ) District*. The proposed PUD will provide a development density that is

consistent with principles found throughout the recently updated Advance Apex 2045. Through various policies, the Peak Plan 2030 works to ensure that there are appropriate transitions between uses. The proposed PUD Plan does just that, by transitioning from I-540 on the west to the single-family Walden Creek community east of the site. Thus, the plan is consistent with several policies contained within the Peak Plan 2030.

Section 5: Permitted Uses

The development will only include residential and supporting uses. Specifically, the permitted uses include:

- Townhomes
- Greenway
- Recreation Facility, private
- Park, active
- Park, passive
- Utility, minor

Additionally, the following conditions shall also apply:

- A. A maximum of 122 residential units shall be permitted upon the property.
- B. A fifty-foot (50') Type A Duffer shall be established and maintained along the eastern boundary of the subject property.
- C. The maximum height for buildings shall be three (3) stories (forty-five feet (45')).
- D. All buildings constructed on the property shall provide solar conduit for the installation of rooftop solar panels.

Section 6: Proposed Design Controls

A. Maximum Non-Residential Densities (SF per non-residential use) This PUD does not provide for any non-residential land uses (see Section 5, *Permitted Uses*).

B. Residential Densities and Design Controls

Density - The overall gross density shall not exceed 7.0 units per acre.

Design Controls – Dimensional standards below shall apply to all residential uses, and at a minimum, will comply with the following:

Maximum Density: 7.0 Units/Acre

(includes RCA and rights-of-way)

Maximum Number of Units: 122
Maximum Built-Upon Area: 70%
Minimum Lot Size: n/a
Minimum Lot Width: n/a

Maximum Building Height: three (3) stories (45')

Note: Porches, patios, decks and other accessory structures may encroach into building setbacks as allowed by the Town of Apex UDO.

Minimum Building Setbacks:

- From Building to Building: 10 feet

From Buffer/RCA:
 10 feet for Buildings

5 feet for Parking Areas

C. Buffers

Perimeter Buffers

North boundary: 20-foot Type A South boundary (Morris Acres Road): 30-foot Type A West boundary (along 0732-29-5017): 30-foot Type A East boundary 50-foot Type A

Note: Where perimeter buffers coincide with stream buffers or 100-year floodplain, existing vegetation will be used to meet the buffer width and opacity.

Thoroughfare Buffers

As depicted on the PD Plan, a 30' Type A Buffer shall be established along Morris Acres Road.

Section 7: Proposed Architectural Controls

The proposed development offers the following architectural controls to ensure a consistency of character throughout the development, while allowing for enough variety to create interest and avoid monotony. Changes to the exterior materials, roof, windows, doors, process, trim, etc. are allowable with administrative approval at the staff level. Further details shall be provided at the time of Master Subdivision submittal. The following conditions shall apply:

- **A.** Vinyl siding will not be used except for vinyl windows and limited decorative element use. Residential areas will utilize brick, stone, and Hardi-plank siding.
- **B.** Siding materials will be varied in type and/or color on 30% of each façade on each building.
- **C.** Windows that are not recessed shall be trimmed. Windows shall vary in size and/or type.
- **D.** Recesses and projections shall be provided for at least 50% of each facade on each building. Building facades shall have horizontal relief achieved by the use of recesses and projections.
- **E.** Four of the following decorative features shall be used on each building: decorative shake, board and batten siding, decorative porch rails and posts, shutters, decorative functional foundation and roof vents, recessed windows, decorative windows, decorative brick or stone, decorative gables, decorative cornices, or metal roofing.
- **F.** A varied color palette shall be utilized throughout the development to include a minimum of three color families for siding and shall include varied trim, shutter, and accent colors complementing the siding color.
- **G.** The rear and side elevations of the units that can be seen from the right-of-way shall have trim around the windows.
- **H.** Front facing garage doors must have windows, decorative details, or carriagestyle adornments.
- **I.** Entrances for units with front-facing garages shall have a prominent covered porch/stoop area leading to the front door.
- **J.** The front façade of any front-loaded garage shall not protrude farther than one foot forward of (i) the front façade of the dwelling unit, or (ii) the front porch of the dwelling unit, whichever is closer to the right-of-way from which the dwelling unit is addressed.

Section 8: Parking and Loading

Parking for the development shall be per Town of Apex UDO.

Section 9: Signage

All signage for this PUD shall comply with Section 8.7, Signs, of the Town of Apex UDO.

Section 10: Natural Resource and Environmental Data

A. River Basins and Watershed Protection Overlay Districts

The project is located within the Beaver Creek drainage basin, which is within the Cape Fear River Basin.

B. Resource Conservation Areas (RCA) - Required and Provided

This PUD will be subject to, and meet the requirements of Section 8.1.2 of the UDO, Resource Conservation Area and Section 2.3.4, Planned Development Districts.

The Site is located on the east side of the 540 corridor and therefore is required to preserve a minimum of 20% Resource Conservation Area (RCA). Designated RCA areas will be consistent with the items listed in Section 8.1.2(B) of the Town's UDO. Preserved streams, wetlands, and associated riparian buffers provide the primary RCA's throughout the site. Additional RCA area provided include stormwater management areas, perimeter buffers, and greenway trails within the walkable community.

C. Any historic structures present

As confirmed by the North Carolina State Historic Preservation Office and Capital Area Preservation, Inc. there are no historic structures present within the project boundary.

Section 11: Stormwater Management

- A. This PUD shall meet all stormwater management requirements for quality and quantity treatment in accordance with Section 6.1.7 of the UDO, such that:
- Post development peak runoff shall not exceed pre-development peak runoff conditions for the 1 year, 10 year, and 25 year 24-hour storm events.
- B. This PUD shall convey as much stormwater runoff from the site development as practical, including required Stormwater Control Measures (SCM'), to the existing 48" RCP culvert located in the southeastern corner of the site along Morris Acres Road. The direct storm drainage connection to the existing 48" RCP culvert is subject to final approval by the Town of Apex, NCDOT or any other regulatory agency. In the event that this direct storm drainage connection is not approved, then this PUD shall meet and exceed existing stormwater management requirements for quality and quantity treatment provided in Section 6.1.7 of the UDO, such that post development peak runoff shall not exceed pre-development peak runoff conditions for the 1 year, 10 year, 25 year, and 100 year 24-hour storm events.

Section 12: Parks and Recreation

The Parks, Recreation, and Cultural Resources Advisory Commission recommended a fee-in-lieu for the project at the May 29, 2019 meeting.

Section 13: Public Facilities

The proposed PUD shall meet all Public Facilities requirements as set forth in UDO

Section 2.3.4(F)(1)(f) and be designed according to sound engineering standards, and shall comply with Town of Apex Sewer and Water Master Plan and the Town of Apex Standards and Specifications. Specifically, road and utility infrastructure shall be as follows:

General Roadway Infrastructure

Developer shall provide minimum frontage widening based on ½ of a 3-lane thoroughfare section with side path and public right-of-way dedication based on an eighty foot (80') right-of-way along Morris Acres Road. The road network will promote connectivity wherever possible to adjacent neighborhoods and undeveloped property. Further, cul-de-sacs will be avoided except where environmental features make through streets unfeasible. Sidewalks will be provided on both sides of streets internal to the site and along street frontage.

Please refer to the concept plan of the PUD plan for proposed access points, stub streets and planned vehicular connectivity. All access and circulation is conceptual and will be finalized at the time of Master Subdivision review and approval.

• Transportation Improvements

 Roadway improvements are subject to modification and final approval by the Town of Apex and NCDOT as part of the Master Subdivision and construction plan approval process. A traffic study has been performed as part of this PUD rezoning consistent with the Town's standards for the same. Based upon the traffic study, no offsite improvements are recommended for this development.

Wayfinding Improvements

Wayfinding measures at the site shall be provided in an effort to facilitate the movement of vehicles and pedestrians to and within the development.

Water and Sanitary Sewer

All lots within the project will be served by the Town of Apex for water and sanitary sewer. The utility design will be finalized at the time of Master Subdivision review and approval based upon available facilities adjacent to the site at that time. A conceptual utility plan is included in the PUD plan for reference.

Other Utilities

Electricity will be provided by Apex Electric. Phone, cable and gas will be provided by the developer and shall meet the Town of Apex standards as outlined in the UDO.

Section 14: Phasing Plan

This PUD may be completed in up to three (3) phases, with construction anticipated to begin in 2020. Project phasing will be planned to ensure the points of access are

provided in accordance with the UDO.

Section 15: Consistency with the 2045 Land Use Map

The proposed land use is consistent with the 2045 Land Use Map.

Section 16: Compliance with the UDO

The development standards adopted for this PUD are in compliance with those set forth in the current version of the Town's Unified Development Ordinance (UDO). Any deviations from UDO requirements have been specifically defined within this document.

Section 17: Compliance with Comprehensive Transportation Plan and Bicycle Plan

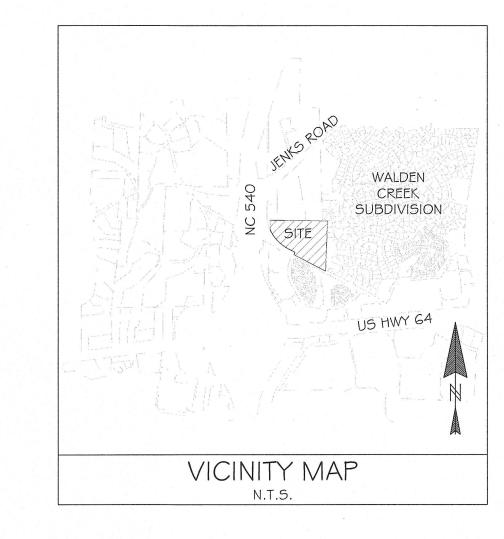
Master Subdivision plans for any development to be made pursuant to this amendment to the Official Zoning District Map shall comply with the adopted Comprehensive Transportation Plan in effect at the time of the Master Subdivision Plan approval as provided for in the Unified Development Ordinance. Further, development of the Property shall be consistent with the Town's adopted Bicycle Plan.

Section 18: Public Art

The applicant shall provide a 6' x 6' Public Art easement to the Town of Apex along the Morris Acres Road frontage of the subject property. The precise location for this easement will be determined at the time of Master Subdivision review.

MORRIS TRACT PLANNED UNIT DEVELOPMENT





PROJECT NAME	MORRIS TRACT
PREPARER'S CONTACT INFORMATION	JONES & CNOSSEN ENGINEERING, PLLC P.O. BOX 1062 APEX, NORTH CAROLINA 27502 PHONE - (919) 387-1174 FAX - (919) 387-3375 CONTACT PERSON - PETER D. CNOSSEN
OWNER / DEVELOPER CONTACT INFORMATION	KAPLAN RESIDENTIAL I I I KANE CONCOURSE, SUITE 302 BAY HARBOUR, FLORIDA 33154 PHONE - (305) 901-2203 CONTACT PERSON - MORRIS KAPLAN
CURRENT ZONING	RR
CURRENT 2045 LAND USE MAP DESIGNATION	MEDIUM DENSITY RESIDENTIAL
PROPOSED ZONING DESIGNATION	PUD-CZ
PROPOSED 2045 LAND USE MAP DESIGNATION	MEDIUM DENSITY RESIDENTIAL
WAKE COUNTY PINS 0732:	38-2709, 38-2530, 28-9587
TOTAL PROJECT AREA	17.44 ACRES
AREA IN MORRIS ACRES ROAD R/W DEDICATION	0.64 ACRE
NET SITE AREA	16.80 ACRES
MAXIMUM NUMBER OF UNITS	I 22 UNITS (7.0 UNITS/ACRE)
PROPOSED NUMBER OF UNITS	I 22 UNITS (7.0 UNITS/ACRE)
REQUIRED RCA / BUFFER AREA	3.49 ACRES (20.0%)
PROVIDED RCA / BUFFER AREA	5.11 ACRES (29.3%)
MAXIMUM BUILT UPON AREA FOR PUD	70% OR 12.21 ACRES
MAXIMUM BUILDING HEIGHT	45' (3-STORIES)
OFF STREET PARKING	TOWN OF APEX UDO REQUIREMENTS
PUBLIC RECREATION REQUIREMENT	TOWNHOMES
WATERSHED INFORMATION	PRIMARY; BEAVER CREEK BASIN
HISTORIC STRUCTURE?	NO
FEMA FLOODPLAIN INFORMATION	MAP #3720073300J - PROJECT IS NOT WITHIN I OO YEAR FLOODPLAIN

PERMITTED USE:
TOWNHOMES GREENWAY RECREATION FACILITY, PRIVATE PARK, ACTIVE PARK, PASSIVE UTILITY, MINOR

MINIMUM BUILDING SETBACKS				
FROM BUILDING TO BUILDING	10'			
FROM BUFFER/RCA	I O' FOR BUILDINGS 5' FOR PARKING AREAS			

PD PLAN - DRAWING SHEET INDEX

COVER SHEET

PRELIMINARY LAYOUT PLAN EXISTING CONDITIONS PLAN PRELIMINARY UTILITY PLAN

RCA # SETBACKS PUD CHANGES PUD USE CHANGE

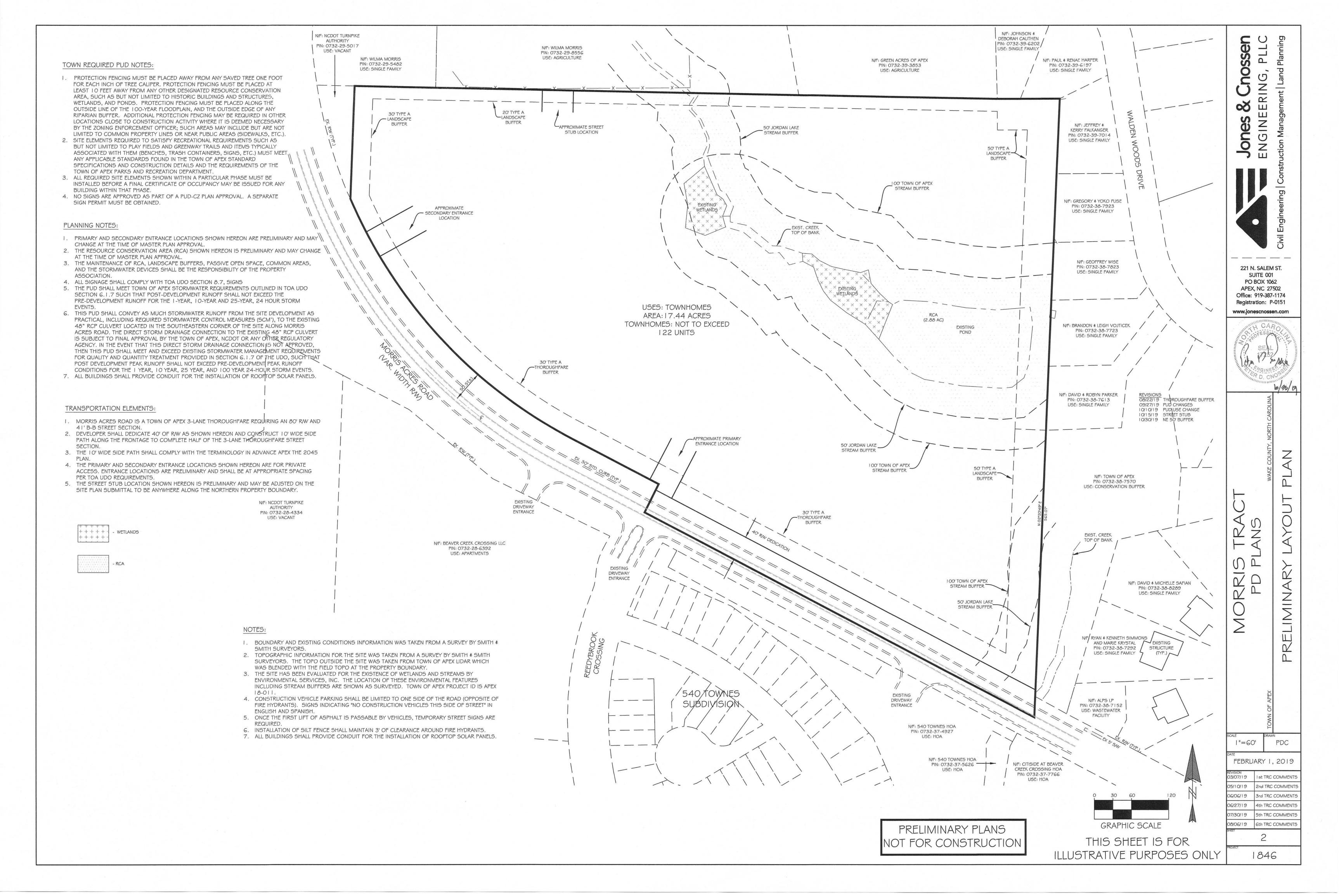
PRELIMINARY PLANS NOT FOR CONSTRUCTION

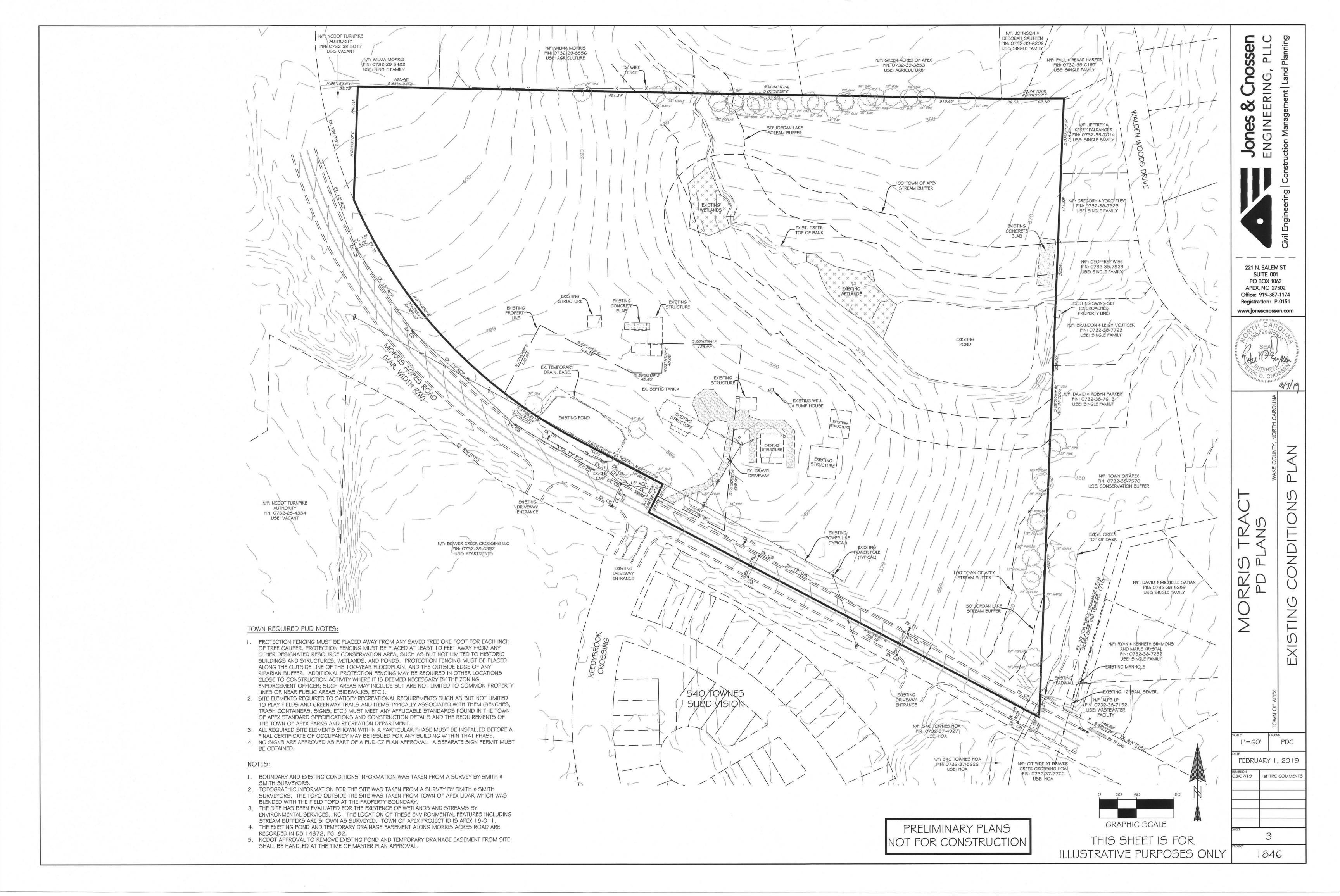
THIS SHEET IS FOR ILLUSTRATIVE PURPOSES ONLY

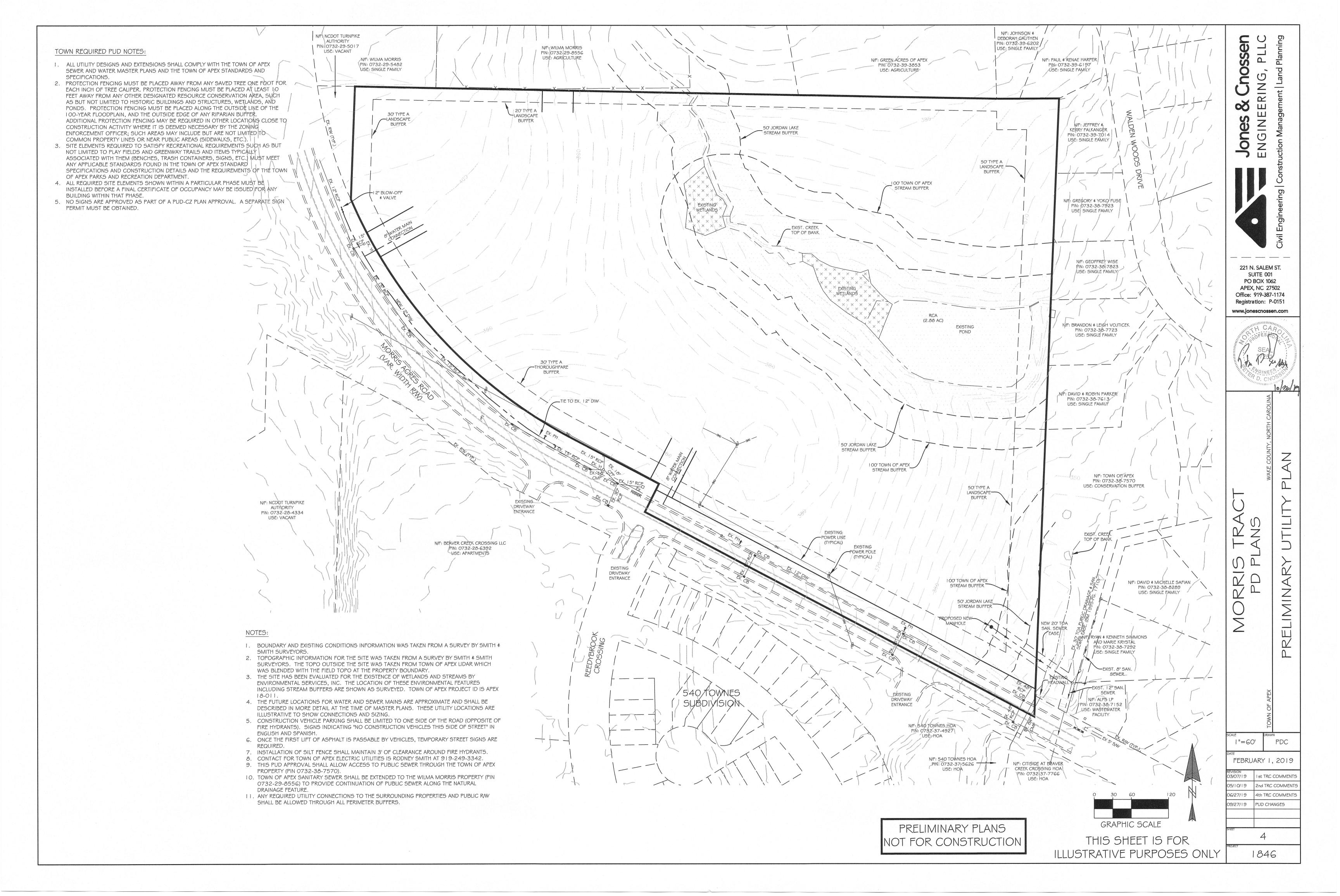
FEBRUARY 1, 2019 05/10/19 | 2nd TRC COMMENTS 96/27/19 4th TRC COMMENTS 7/30/19 5th TRC COMMENTS 08/06/19 6th TRC COMMENTS

221 N. SALEM ST. SUITE 001 PO BOX 1062

1846







Traffic Impact Analysis

The Wayforth at A pex A pex, NC

Prepared for:

Kaplan Residential

 \times K imley-Horn and Associates, Inc. 2019



Traffic Impact Analysis for The Wayforth at Apex Apex, North Carolina

Prepared for: K aplan R esidential Bay Harbor, FL

Prepared by:
K imley-Horn and Associates, Inc.
NC License #F-0102
421 Fayetteville Street, Suite 600
Raleigh, NC 27601
(919) 677-2000

J anuary 2019 013249000







Executive Summary

Kimley-Horn and Associates, Inc. has performed a Traffic Impact Analysis for The Wayforth at Apex, a proposed apartment project located on the east side of Morris Acres Road between Jenks Road and Walden Creek Drive in Apex, North Carolina. The property is currently occupied by a few single-family homes and as currently envisioned will consist of approximately 300 apartments. The development is proposed to be accessed via three driveways on Morris Acres Road, and build-out of the project is anticipated in the year 2022.

This report presents trip generation, distribution, traffic analyses, and recommendations for transportation improvements required to meet anticipated traffic demands in conjunction with the development. The traffic conditions studied include the existing (2018) traffic condition as well as the projected (2022) background and build-out traffic conditions.

As shown in Table ES-1, the proposed development has the potential to generate 1,634 new trips during a typical weekday with 100 new trips during the AM peak hour and 127 new trips during the PM peak hour.

	Table ES-1 ПЕ Traffic Generation (Vehicles)								
Land Use	Land Use Ir		Intensity		ily	AM F			Peak our
Code				In	Out	In	Out	In	Out
221	Multifamily Housing (Mid-Rise)	300	d.u.	817	817	26	74	77	50

Capacity analyses were performed using Synchro V ersion 9.2 software. Table ES-2 summarizes the operation of the study intersections for the AM and PM peak hour traffic conditions.

Table ES-2 Level-of-Service Summary					
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)			
J enks R oad at Morris A cres R oad (Unsignalized)					
Existing (2018) Traffic	NB ⁻ B (11.3) WBL ⁻ A (7.8)	NB ⁻ B (14.6) WBL ⁻ A (8.1)			
Background (2022) Traffic	NB ⁻ B (11.8) WBL ⁻ A (7.9)	NB ⁻ C (16.6) WBL ⁻ A (8.2)			
Build-out (2022) Traffic	NB ⁻ B (11.9) WBL ⁻ A (7.9)	NB ⁻ C (17.6) WBL ⁻ A (8.3)			

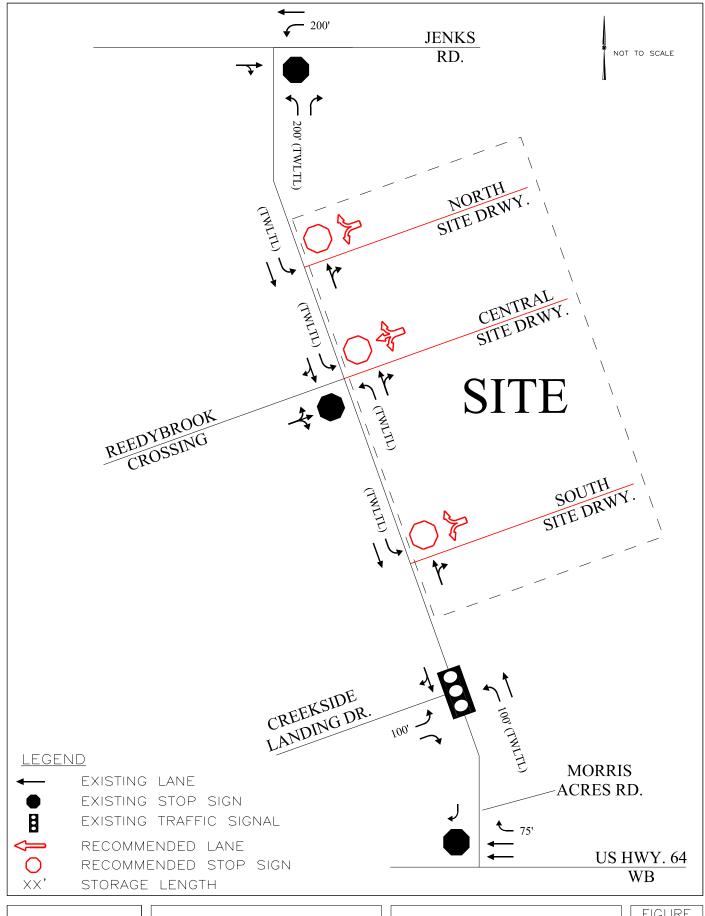


Table ES-2 (cont.) Level-of-Service Summary					
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)			
Morris Acres Road at Reedybrook Crossing/Central Site Driveway (Unsignalized)					
Existing (2018) Traffic	EB ⁻ A (9.4) NBL ⁻ A (7.3)	EB ⁻ B (10.2) NBL ⁻ A (7.6)			
Background (2022) Traffic	EB ⁻ A (9.5) NBL ⁻ B (7.3)	EB ⁻ B (10.4) NBL ⁻ A (7.7)			
Build-out (2022) Traffic	EB ⁻ B (10.2) WB ⁻ B (10.2) NBL ⁻ A (7.4) SBL ⁻ A (7.6)	EB ⁻ B (12.6) WB ⁻ B (12.5) NBL ⁻ A (7.7) SBL ⁻ A (8.0)			
Morris A cres R oad at C ree	ekside Landing Drive (Sig	nalized)			
Existing (2018) Traffic	A (4.6)	A (6.7)			
Background (2022) Traffic	A (4.7)	A (7.1)			
Build-out (2022) Traffic	A (5.0)	A (7.8)			
US 64 Westbound at Mo	rris A cres R oad (Unsigna	lized)			
Existing (2018) Traffic	SB ⁻ D (27.9)	SB ⁻ D (28.8)			
Background (2022) Traffic	SB ⁻ E (41.8)	SB ⁻ E (43.6)			
Build-out (2022) Traffic	SB ⁻ E (49.4)	SB ⁻ E (48.8)			
Morris A cres R oad at Nor	th Site Driveway (Unsign	nalized)			
Build-out (2022) Traffic	WB ⁻ A (9.8) SBL ⁻ A (7.7)	WB ⁻ B (10.6) SBL ⁻ A (8.0)			
M orris A cres R oad at Sou	ıth Site Driveway (Unsign	alized)			
Build-out (2022) Traffic	WB ⁻ A (9.7) SBL ⁻ A (7.6)	WB ⁻ B (10.8) SBL ⁻ A (8.0)			

With the exception of southbound Morris Acres Road at US 64 Westbound, analyses indicate that all of the study intersections are expected to operate at an acceptable LOS at project build-out with only minor increases in delays and queues associated with the addition of site traffic. The intersection of US 64 Westbound at Morris Acres road is expected to operate with moderate delays on Morris Acres Road in the year 2022 with or without the proposed project in place.

No roadway improvements are recommended to be performed to accommodate projected site traffic volumes.

The build-out roadway laneage is shown on Figure ES-1.



THE WAYFORTH AT APEX APEX, NC TRAFFIC CAPACITY ANALYSIS

BUILD-OUT ROADWAY LANEAGE FIGURE ES-1



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1.0 Introduction

Kimley-Horn and Associates, Inc. has performed a Traffic Impact Analysis for The Wayforth at Apex, a proposed apartment project located on the east side of Morris Acres Road between Jenks Road and Walden Creek Drive in Apex, North Carolina. The property is currently occupied by a few single-family homes and as currently envisioned will consist of approximately 300 apartments. The development is proposed to be accessed via three driveways on Morris Acres Road, and build-out of the project is anticipated in the year 2022.

This report presents trip generation, distribution, traffic analyses, and recommendations for transportation improvements required to meet anticipated traffic demands in conjunction with the development. The traffic conditions studied include the existing (2018) traffic condition as well as the projected (2022) background and build-out traffic conditions.

Town of A pex and North Carolina Department of Transportation (NCDOT) transportation staff provided background data and were consulted regarding the elements to be covered in this analysis. The approved Memorandum of Understanding is included in the Appendix of this report.



2.0 Inventory

2.1 Study Area

The study area for this development in includes the following intersections:

- é Jenks Road at Morris Acres Road
- é Morris Acres Road at Reedybrook Crossing/Central Site Driveway
- é Morris Acres Road at Creekside Landing Drive
- é Morris Acres Road at US 64 Westbound
- é Morris A cres Road at North Site Driveway
- é Morris Acres Road at South Site Driveway

Figure 1 shows the site location. The preliminary site plan is shown on Figure 2.

2.2 Existing Conditions

The Wayforth at Apex development is proposed to be located generally east of Morris Acres Road between Jenks Road and Walden Creek Drive in Apex, North Carolina. Roadways in the study area include US 64, Jenks Road, Creekside Landing Drive, and Morris Acres Road (formerly Green Level Church Road). The existing roadway laneage is shown in Figure 3.

US 64 is a 4-lane divided highway with a posted speed limit of 55 mph near Morris Acres Road. On US 64 Westbound, the reported 2017 average daily traffic (ADT) volume was approximately 31,000 vehicles per day (vpd) west of Morris Acres Road. US 64 is designated as a freeway on the Town of Apex Thoroughfare and Collector Street Plan.

Jenks Road is a 2-lane undivided roadway with a posted speed limit of 45 mph in the vicinity of Morris Acres Road. The estimated 2018 ADT volume is approximately 6,000 vpd west of Morris Acres Road. Jenks Road is designated to be a 3-lane thoroughfare section per the Town of Apex Thoroughfare and Collector Street Plan.

Creekside Landing Drive is a 2-lane undivided roadway with a posted speed limit of 25 mph. The estimated 2018 ADT volume is approximately 3,500 vpd. Morris Acres Road is designated to be a minor collector on the Town of Apex Thoroughfare and Collector Street Plan.

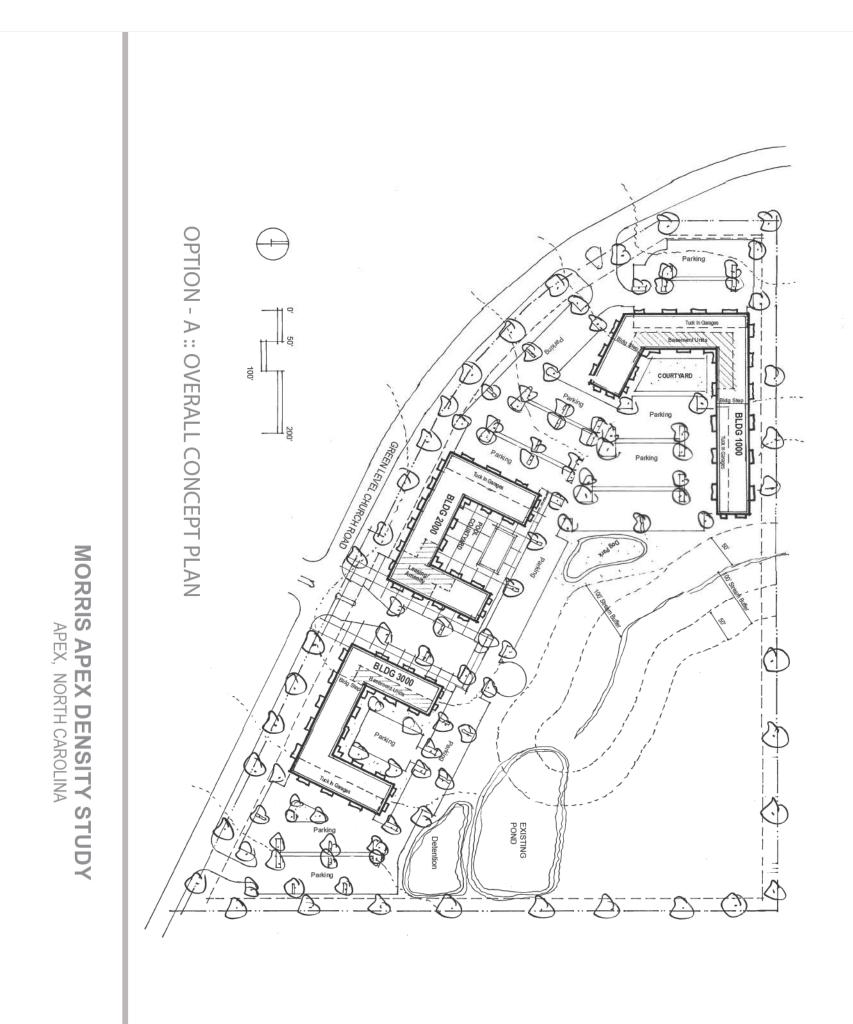
Morris Acres Road (formerly Green Level Church Road) is a 3-lane undivided roadway in the vicinity of the site with a posted speed limit of 45 mph. The estimated 2018 ADT volume is approximately 3,000 vpd at Jenks Road. Morris Acres Road has already been widened to the designated 3-lane thoroughfare per the Town of Apex Thoroughfare and Collector Street Plan.



THE WAYFORTH AT APEX APEX, NC TRAFFIC CAPACITY ANALYSIS

SITE LOCATION

FIGURE 1





PROJECT DATA

RESIDENTIAL - 1,000 SF Average

BUILDING 1000 BUILDING 2000 BUILDING 3000 TOTAL

- 102 Units (4/5 Story)
- 88 Units (4/5 Story)
- 113 Units (4/5 Story)
- 303 Units

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PARKING
SURFACE PARKING
TUCK UNDER GARAGES
TOTAL

408 SPACES 47 SPACES 455 SPACES (1.5 Spaces/Unit)

Leasing and Amenity in

n Bldg 2000 - 7,500 SF

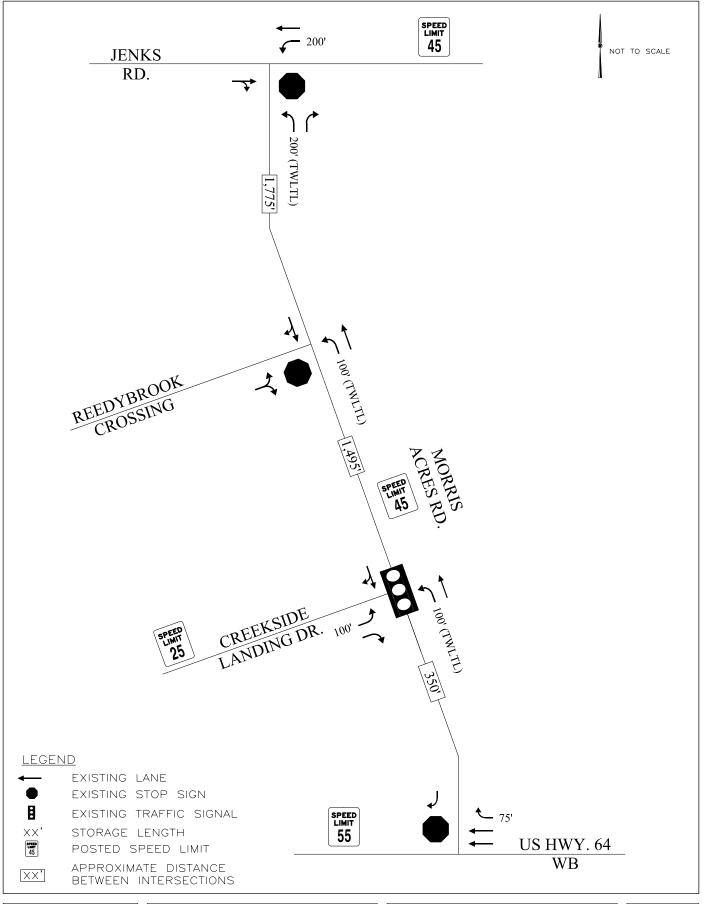
Kimley » Horn

THE WAYFORTH AT APEX APEX, NC TRAFFIC CAPACITY ANALYSIS

PROPOSED SITE PLAN

FIGURE

2



THE WAYFORTH AT APEX APEX, NC TRAFFIC CAPACITY ANALYSIS

EXISTING ROADWAY LANEAGE

FIGURE

3



3.0 Traffic Generation

The traffic generation potential of the proposed development was determined using the traffic generation rates published in Trip Generation (Institute of Transportation Engineers, Tenth Edition, 2017). As currently envisioned the development will consist of approximately 300 apartments. Table 3.0 summarizes the estimated traffic generation for the proposed development.

Table 3.0 ITE Traffic Generation (Vehicles)												
Land Use	Land Use	Intensity _		Intensity		Da	Daily		AM Peak Hour		PM Peak Hour	
Code				In	Out	In	Out	In	Out			
221	Multifamily Housing (Mid-Rise)	300	d.u.	817	817	26	74	77	50			

Table 3.0 shows the proposed development has the potential to generate 1,634 new trips during a typical weekday with 100 new trips during the AM peak hour and 127 new trips during the PM peak hour.

Detailed trip generation calculations are included in the Appendix of this report.



4.0 Site Traffic Distribution

The projected site-generated trips were assigned to the surrounding roadway network. The directional distribution and assignment for this development were based on a review of surrounding land uses and traffic patterns in the study area. As the intersection of Morris Acres Road at US 64 Westbound is limited to right-in/right-out access, separate inbound and outbound distributions were developed for the site in conjunction with Town of Apex staff to account for anticipated travel paths.

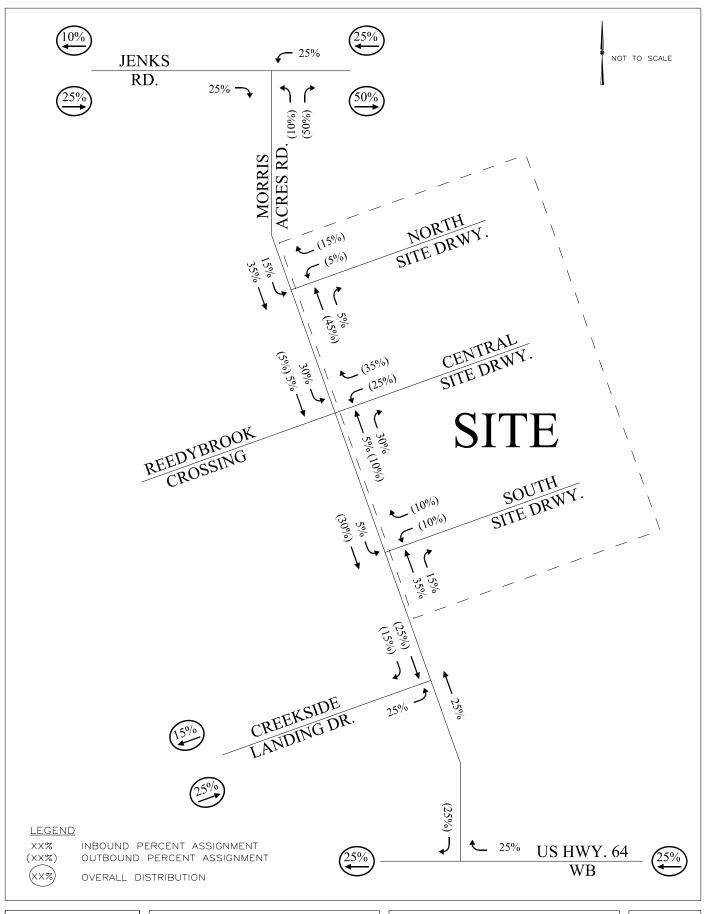
The inbound site traffic distribution used for the site was:

- é 25% from the east on US 64
- é 25% from the east on Jenks Road
- é 25% from the west on Jenks Road
- é 25% from the south on Creekside Landing Drive

The outbound site traffic distribution used for the site was:

- é 50% to the east on Jenks Road
- é 25% to the west on US 64
- é 15% to the south on Creekside Landing Drive
- é 10% to the west on Jenks Road

The site traffic distribution and percent assignment for site are shown on Figure 4.



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SITE TRAFFIC DISTRIBUTION AND PERCENT ASSIGNMENT

FIGURE 4



5.0 Projected Traffic Volumes

5.1 Existing Traffic

AM peak hour (7:00 to 9:00 AM) and PM peak hour (4:00 to 6:00 PM) turning movement counts were performed at the following intersections:

خ	J enks R oad at Morris A cres R oad	October 23, 2018
¿	Morris Acres Road at Creekside Landing Drive	October 23, 2018
;	Morris Acres Road at US 64 Westbound	October 23, 2018

The existing AM and PM peak hour traffic volumes are shown on Figures 5 and 6, and the traffic count data are included in the Appendix. No turning movement counts were performed at the intersection of Morris Acres Road at Reedybrook Crossing. However, as the development is approximately 90% occupied (discussed below), existing volumes onto/off of Reedybrook Crossing were assumed to be equal to 90% of the site traffic volume on those movements as indicated in the TIA for that project.

5.2 Historic Growth Traffic

Historic growth traffic is the increase in traffic due to usage increases and non-specific growth throughout the area. An annual growth rate of 3% was applied to the existing volumes up to the year 2022. Background growth calculations are detailed on intersection spreadsheets in the Appendix of this report.

5.3 Approved Development Traffic

Approved development traffic is generated by approved but not yet constructed projects in the vicinity of the proposed project. Based on discussions with the Town of Apex, the Beaver Creek Phase 4 Residential (540 Townes) project was the only development identified for inclusion in the analysis as background traffic.

Per the Beaver Creek Residential Development TIA (Stantec, August 2015), the project proposes the construction of approximately 300 apartments and 50 townhomes along Morris Acres Road (Green Level Church Road) north of US 64 with a build-out year of 2020. As the development was almost entirely built-out when traffic counts were performed, only 10% of site trips from this development were included at off-site intersections as background traffic.

Background traffic volumes consisting of existing, historic growth, and approved development traffic, are shown on Figures 5 and 6 for the AM and PM peak hours, respectively.

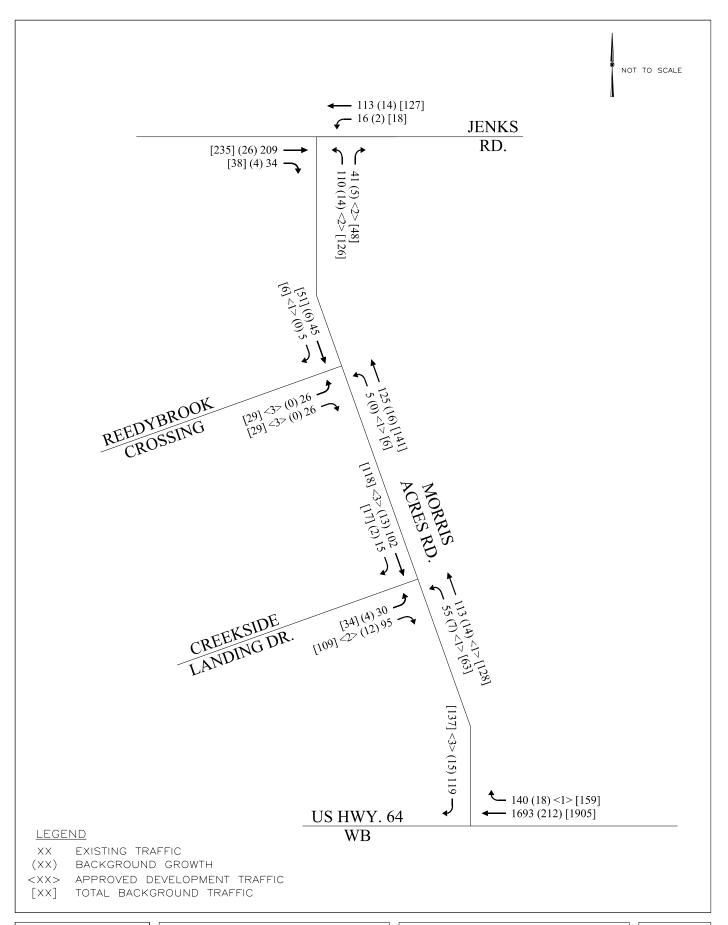


5.4 Site Traffic

The projected site traffic was generated and assigned to the adjacent roadway network according to the distribution discussed previously in Section 4.0. The site traffic volumes for the AM and PM peak hours are shown in Figures 7 and 8, respectively.

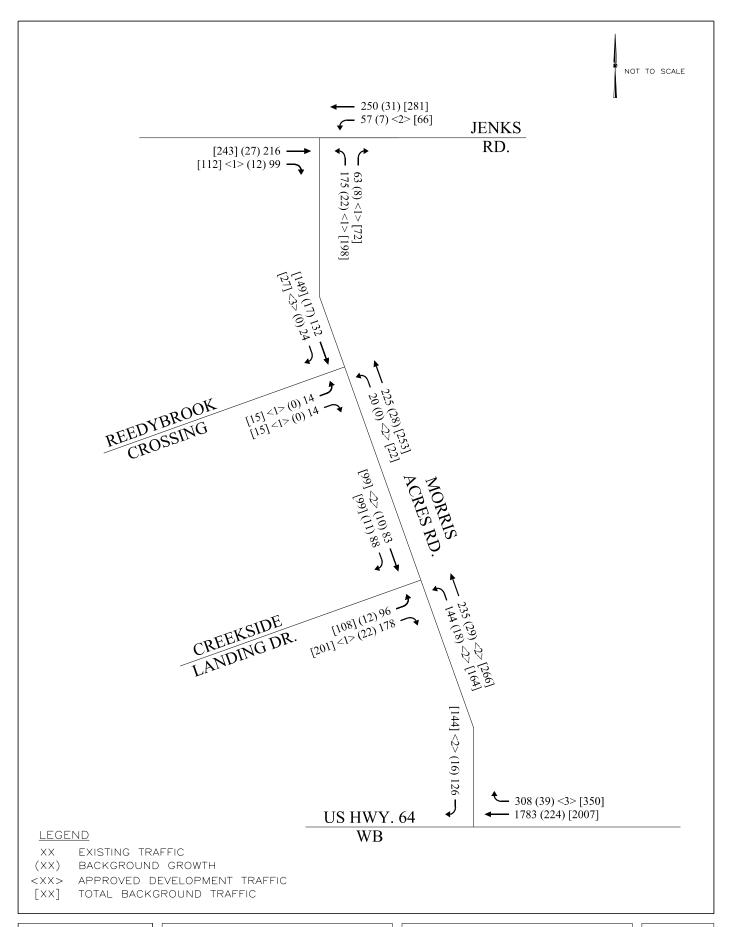
5.5 Build-Out Traffic

To obtain the projected (2022) build-out traffic volumes, the projected site traffic were added to the projected (2022) background traffic. Traffic volume calculations are detailed in intersection spreadsheets in the Appendix of this report. Figures 7 and 8 show the projected (2022) AM and PM peak hour build-out traffic volumes, respectively.



THE WAYFORTH AT APEX APEX, NC TRAFFIC CAPACITY ANALYSIS

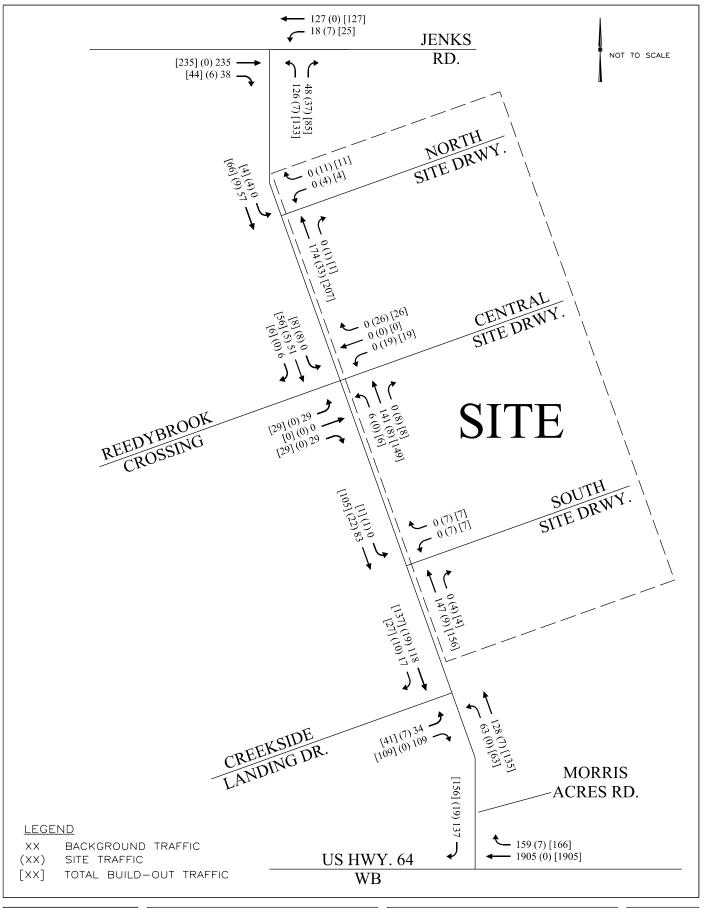
EXISTING AND PROJECTED (2022) BACKGROUND AM PEAK HOUR TRAFFIC VOLUMES FIGURE 5



THE WAYFORTH AT APEX APEX, NC TRAFFIC CAPACITY ANALYSIS

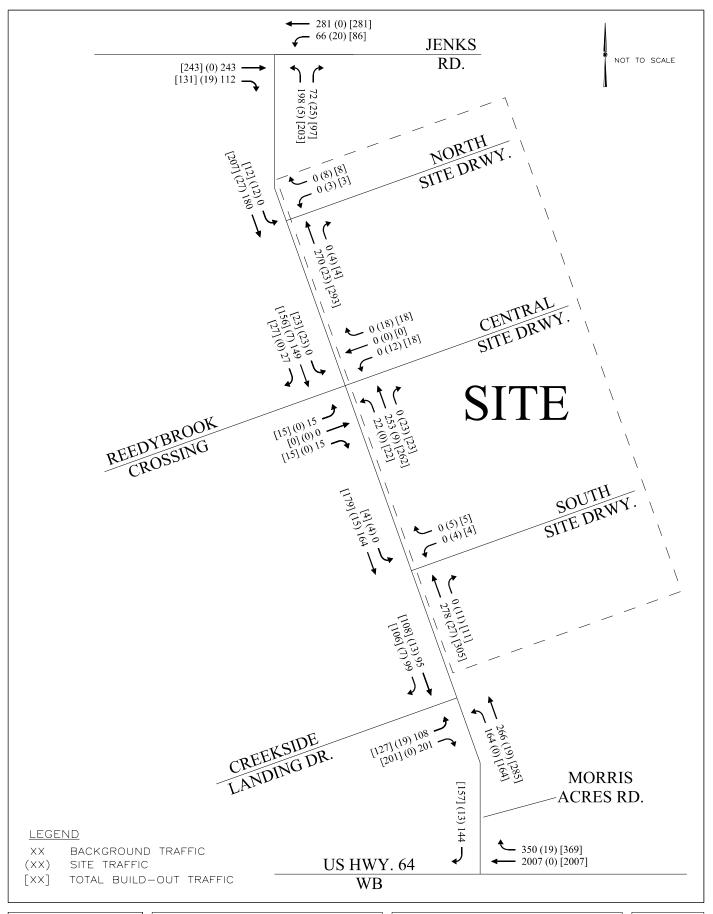
EXISTING AND PROJECTED (2022) BACKGROUND PM PEAK HOUR TRAFFIC VOLUMES FIGURE

6



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PROJECTED (2022) BUILD-OUT AM PEAK HOUR TRAFFIC VOLUMES FIGURE 7



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PROJECTED (2022) BUILD-OUT PM PEAK HOUR TRAFFIC VOLUMES FIGURE 8



6.0 Capacity Analysis

Capacity analyses (see A ppendix) were performed for the A M and PM peak hours for the existing traffic condition and the projected (2022) background and build-out traffic conditions using Synchro V ersion 9.2 software to determine the operating characteristics of the adjacent road network and the impacts of the proposed project.

Capacity is defined as the maximum number of vehicles that can pass over a particular road segment or through a particular intersection within a set time duration. Capacity is combined with Level-of-Service (LOS) to describe the operating characteristics of a road segment or intersection. LOS is a qualitative measure that describes operational conditions and motorist perceptions within a traffic stream. The Highway Capacity Manual defines six levels of service, LOS A through LOS F, with A representing the shortest average delays and F representing the longest average delays. LOS D is the typically accepted standard for signalized intersections in urbanized areas. For signalized intersections, LOS is defined for the overall intersection operation.

For unsignalized intersections, only the movements that must yield right-of-way experience control delay. Therefore, LOS criteria for the overall intersection is not reported by Synchro V ersion 9.2 or computable using methodology published in the Highway Capacity Manual. It is typical for stop sign controlled side streets and driveways intersecting major streets to experience long delays during peak hours, while the majority of the traffic moving through the intersection on the major street experiences little or no delay. Table 6.0-A lists the LOS control delay thresholds published in the Highway Capacity Manual for signalized and unsignalized intersections.

	Table 6.0-A						
	Level-of-Service Control Delay Thresholds						
Level-of-	Signalized Intersections ⁻	Unsignalized Intersections ⁻					
Service	Control Delay Per Vehicle	Average Control Delay					
Service	[sec/veh]	[sec/veh]					
А	Ħ10	Ħ10					
В	> 10 ⁻ 20	> 10 ⁻ 15					
С	> 20 ⁻ 35	> 15 ⁻ 25					
D	> 35 ⁻ 55	> 25 ⁻ 35					
E > 55 ⁻ 80		> 35 ⁻ 50					
F	> 80	> 50					

Existing peak hour factors (PHF) were used at all existing intersections for all conditions except at new intersections, where a PHF of 0.90 was used. The existing signal plan provided by NCDOT was referenced to obtain signal timings, which were not adjusted as part of this analysis, and right-turns on red were allowed where currently allowed.

Capacity analyses were performed for the existing (2018) traffic condition and the projected (2022) background and build-out traffic conditions for the following intersections:

- é Jenks Road at Morris Acres Road
- é Morris Acres Road at Reedybrook Crossing/Central Site Driveway
- é Morris Acres Road at Creekside Landing Drive
- é Morris Acres Road at US 64 Westbound
- é Morris Acres Road at North Site Driveway
- é Morris Acres Road at South Site Driveway

Table 6.0-B summarizes the LOS and delay (seconds per vehicle) for all of the study intersections for the existing (2018) traffic condition and the projected (2022) background and build-out traffic conditions. All capacity analyses are included in the Appendix and are briefly summarized in the following sub-sections.

Table 6.0-B		
Level-of-Service Summary		
Condition	AM Peak Hour	PM Peak Hour
	LOS (Delay)	LOS (Delay)
J enks R oad at Morris A cres R oad (Unsignalized)		
Existing (2018) Traffic	NB ⁻ B (11.3) WBL ⁻ A (7.8)	NB ⁻ B (14.6) WBL ⁻ A (8.1)
Background (2022) Traffic	NB ⁻ B (11.8) WBL ⁻ A (7.9)	NB ⁻ C (16.6) WBL ⁻ A (8.2)
Build-out (2022) Traffic	NB ⁻ B (11.9) WBL ⁻ A (7.9)	NB ⁻ C (17.6) WBL ⁻ A (8.3)
Morris Acres Road at Reedybrook Crossing/Central Site Driveway (Unsignalized)		
Existing (2018) Traffic	EB - A (9.4)	EB - B (10.2)
Background (2022) Traffic	NBL ⁻ A (7.3) EB ⁻ A (9.5)	NBL ⁻ A (7.6) EB ⁻ B (10.4)
	NBL - B (7.3)	NBL A (7.7)
Build-out (2022) Traffic	EB - B (10.2)	EB ⁻ B (12.6)
	WB ⁻ B (10.2) NBL ⁻ A (7.4)	WB ⁻ B (12.5) NBL ⁻ A (7.7)
	SBL - A (7.4)	SBL - A (8.0)
Morris Acres Road at Creekside Landing Drive (Signalized)		
Existing (2018) Traffic	A (4.6)	A (6.7)
Background (2022) Traffic	A (4.7)	A (7.1)
Build-out (2022) Traffic	A (5.0)	A (7.8)
US 64 Westbound at Morris Acres Road (Unsignalized)		
Existing (2018) Traffic	SB ⁻ D (27.9)	SB ⁻ D (28.8)
Background (2022) Traffic	SB ⁻ E (41.8)	SB ⁻ E (43.6)
Build-out (2022) Traffic	SB ⁻ E (49.4)	SB ⁻ E (48.8)



Table 6.0-B (cont.)									
Level-of-S	ervice Summary								
Condition AM Peak Hour PM Peak H LOS (Delay) LOS (Dela									
Morris A cres R oad at North Site Driveway (Unsignalized)									
Build-out (2022) Traffic	WB ⁻ A (9.8) SBL ⁻ A (7.7)	WB ⁻ B (10.6) SBL ⁻ A (8.0)							
Morris Acres Road at South Site Driveway (Unsignalized)									
Build-out (2022) Traffic WB ¯ A (9.7) WB ¯ B (10.8) SBL ¯ A (7.6) SBL ¯ A (8.0)									



6.1 Jenks Road at Morris Acres Road

A nalyses indicate that the unsignalized intersection of Jenks Road at Morris Acres Road currently operates with short delays on the minor street approach (Morris Acres Road) in both the AM and PM peak hours. The intersection is expected to continue to operate with short delays and queues in the year 2022 with or without the proposed project in place, and no roadway improvements are recommended to accommodate projected site traffic.

Table 6.1 summarizes the operation of the intersection of Jenks Road at Morris Acres Road for the existing (2018) and projected (2022) background and build-out traffic conditions.

Table 6.1 Level-of-Service J enks Road at Morris Acres Road (Unsignalized)									
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)							
Existing (2018) Traffic	NB ⁻ B (11.3) WBL ⁻ A (7.8)	NB ⁻ B (14.6) WBL ⁻ A (8.1)							
Background (2022) Traffic	NB ⁻ B (11.8) WBL ⁻ A (7.9)	NB ⁻ C (16.6) WBL ⁻ A (8.2)							
Build-out (2022) Traffic	NB ⁻ B (11.9) WBL ⁻ A (7.9)	NB ⁻ C (17.6) WBL ⁻ A (8.3)							



6.2 Morris Acres Road at Reedybrook Crossing/Central Site Driveway

Analyses indicates that the intersection of Morris Acres Road at Reedybrook Crossing currently operates with short delays on the minor street approach (Reedybrook Crossing) in both the AM and PM peak hours, and the intersection is expected to continue to operate with short delays in the background traffic condition.

The Wayforth at A pex proposes to construct a site driveway aligning with Reedybrook Crossing, providing one ingress lane and one egress lane. A nalyses indicate that at project build-out both minor street approaches (Reedybrook Crossing and the Central Site Driveway) are expected to operate with short delays and queues. No roadway improvements are recommended to be performed as part of this development

Table 6.2 summarizes the operation of the intersection of Morris Acres Road at Reedybrook Crossing/Central Site Driveway for the existing (2018) and projected (2022) background and build-out traffic conditions.

Table 6.2 Level-of-Service										
Morris Acres Road at Reedybrook Crossing/Central Site Driveway (Unsignalized)										
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)								
Existing (2018) Traffic	EB ⁻ A (9.4) NBL ⁻ A (7.3)	EB ⁻ B (10.2) NBL ⁻ A (7.6)								
Background (2022) Traffic	EB ⁻ A (9.5) NBL ⁻ B (7.3)	EB ⁻ B (10.4) NBL ⁻ A (7.7)								
Build-out (2022) Traffic	EB ⁻ B (10.2) WB ⁻ B (10.2) NBL ⁻ A (7.4) SBL ⁻ A (7.6)	EB ⁻ B (12.6) WB ⁻ B (12.5) NBL ⁻ A (7.7) SBL ⁻ A (8.0)								



6.3 Morris Acres Road at Creekside Landing Drive

Analyses indicate that the signalized intersection of Morris Acres Road at Creekside Landing Drive currently operates at LOS A in both the AM and PM peak hours. The intersection is expected to continue to operate at LOS A in the year 2022 with or without the proposed project in place, and no queuing issues are expected at this intersection. No roadway improvements are recommended to be performed at this intersection to accommodate projected site traffic volumes.

Table 6.3 summarizes the operation of the intersection of Morris Acres Road at Creekside Landing Drive for the existing (2018) and projected (2022) background and build-out traffic conditions.

Table 6.3 Level-of-Service Morris Acres Road at Creekside Landing Drive (Signalized)										
Condition AM Peak Hour PM Peak HOS (Delay) LOS (Delay)										
Existing (2018) Traffic	A (4.6)	A (6.7)								
Background (2022) Traffic	A (4.7)	A (7.1)								
Build-out (2022) Traffic	A (5.0)	A (7.8)								



6.4 Morris Acres Road at US 64 Westbound

Analyses indicate that the unsignalized intersection of Morris Acres Road at US 64 Westbound currently operates with moderate delays on the minor street approach (Morris Acres Road) in both the AM and PM peak hours. The intersection is expected to continue to operate with moderate delays on the minor street approach in the year 2022 with or without the proposed project in place. As only slight increases in queues and delays are expected at this intersection with the addition of site traffic, no roadway improvements are recommended to be performed at this intersection.

Table 6.4 summarizes the operation of the intersection of Morris Acres Road at US 64 Westbound for the existing (2018) and projected (2022) background and build-out traffic conditions.

Table 6.4 Level-of-Service Morris Acres Road at US 64 Westbound (Unsignalized)										
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)								
Existing (2018) Traffic	SB ⁻ D (27.9)	SB ⁻ D (28.8)								
Background (2022) Traffic	SB ⁻ E (41.8)	SB ⁻ E (43.6)								
Build-out (2022) Traffic	SB ⁻ E (49.4)	SB ⁻ E (48.8)								



6.5 Morris Acres Road at North Site Driveway

A full-movement site driveway is proposed to be constructed on Morris Acres Road approximately 750 feet north of Reedybrook Crossing. Analyses indicate that the intersection is expected to operate with short delays and queues on the minor street approach (North Site Driveway) at project build-out. No roadway improvements are recommended to be performed as part of this development

Table 6.5 summarizes the operation of the intersection of Morris Acres Road at North Site Driveway for the projected (2022) build-out traffic condition.

Table 6.5 Level-of-Service										
Morris Acres Road at North Site Driveway (Unsignalized)										
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)								
Build-out (2022) Traffic	WB ⁻ A (9.8) SBL ⁻ A (7.7)	WB ⁻ B (10.6) SBL ⁻ A (8.0)								



6.6 Morris Acres Road at South Site Driveway

A full-movement site driveway is proposed to be constructed on Morris Acres Road approximately 500 feet south of Reedybrook Crossing. Analyses indicate that the intersection is expected to operate with short delays and queues on the minor street approach (South Site Driveway) at project build-out. No roadway improvements are recommended to be performed as part of this development

Table 6.6 summarizes the operation of the intersection of Morris Acres Road at South Site Driveway for the projected (2022) build-out traffic condition.

Table 6.6 Level-of-Service										
Morris Acres Road at South Site Driveway (Unsignalized)										
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)								
Build-out (2022) Traffic	WB ⁻ A (9.7) SBL ⁻ A (7.6)	WB ⁻ B (10.8) SBL ⁻ A (8.0)								

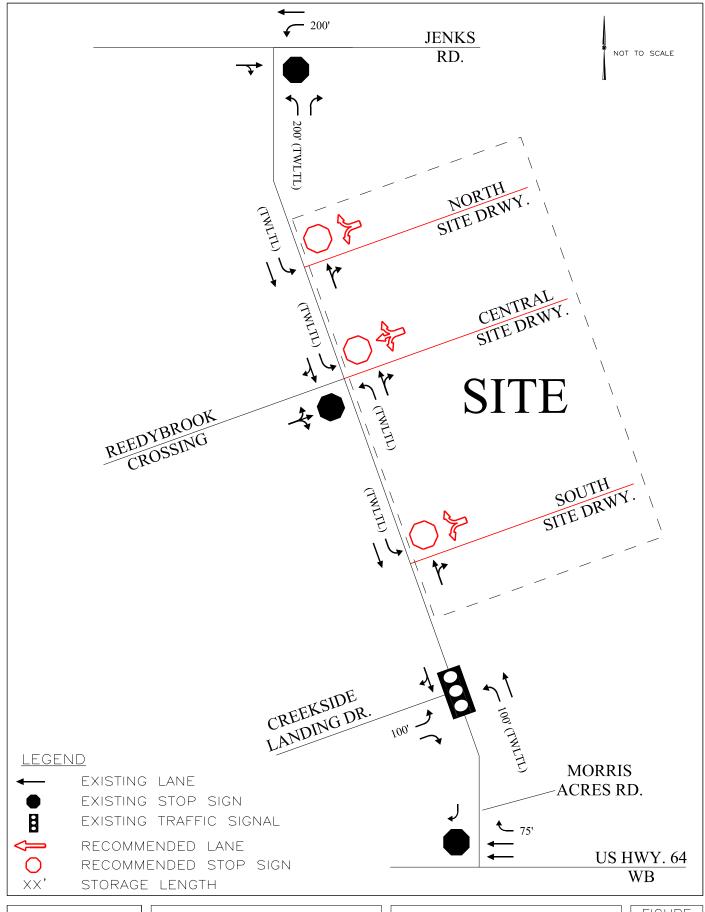


7.0 Recommendations

With the exception of southbound Morris A cres Road at US 64 Westbound, analyses indicate that all of the study intersections are expected to operate at an acceptable LOS at project build-out with only minor increases in delays and queues associated with the addition of site traffic. The intersection of US 64 Westbound at Morris A cres road is expected to operate with moderate delays on Morris A cres Road in the year 2022 with or without the proposed project in place.

No roadway improvements are recommended to be performed to accommodate projected site traffic volumes.

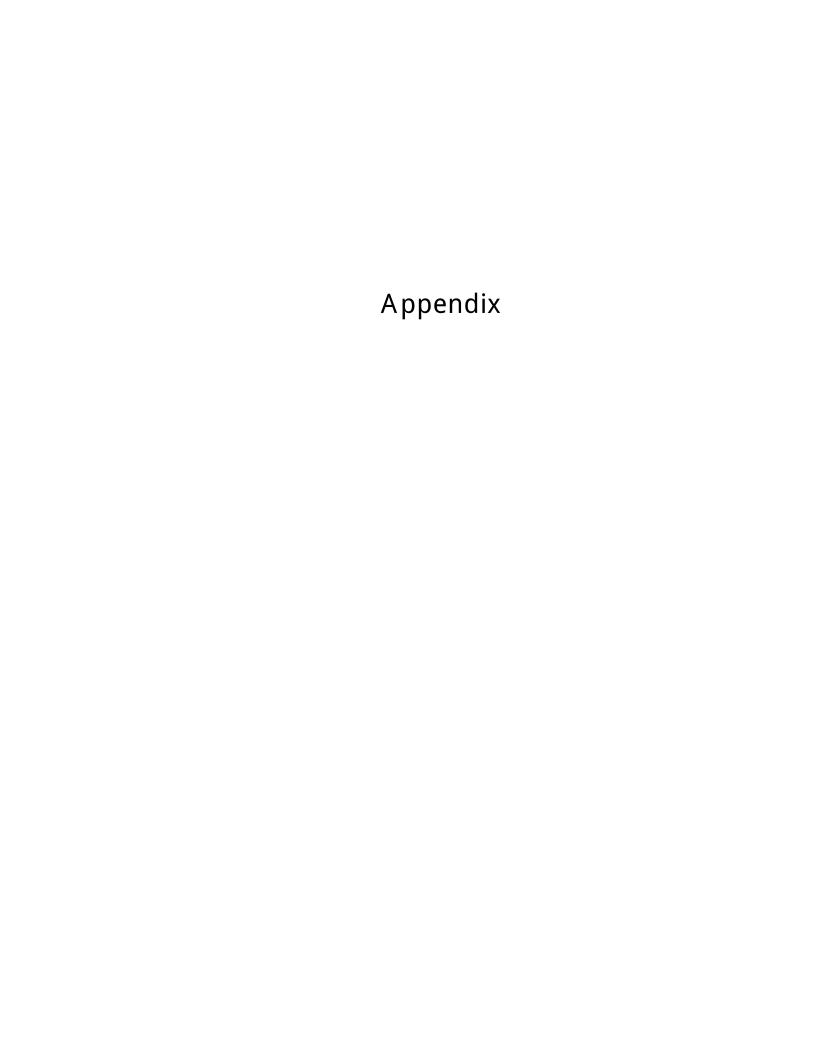
The build-out roadway laneage is shown on Figure 9.



Kimley»Horn

THE WAYFORTH AT APEX APEX, NC TRAFFIC CAPACITY ANALYSIS

BUILD-OUT ROADWAY LANEAGE FIGURE 9



Appendix A: Assumptions Memorandum

Preliminary Assumptions The Wayforth at Apex Apartments - Traffic Impact Analysis Apex, North Carolina

KHA will perform analyses for The Wayforth at Apex development, a proposed apartments project located on the east side of Morris Acres Road (formerly Green Level Church Road) between Jenks Road and Creekside Landing in Apex, North Carolina. The following assumptions will be used in the analysis of the site:

The study area will consist of the following intersections:

- é Jenks Road at Morris Acres Road
- é Morris Acres Road at Creekside Landing Drive
- é US 64 at Morris A cres Road
- é Morris Acres Road at Reedybrook Crossing/Central Site Driveway
- é Morris Acres Road at North Site Driveway
- é Morris Acres Road at South Site Driveway

The study scenarios will consist of:

- ¿ Existing (2018)
- ¿ Background (2022)
- ¿ Build-out (2022)

Based on discussions with the Town of Apex and the North Carolina Department of Transportation (NCDOT), the 540 Townes development located on the west side of Morris Acres Road (which is partially built-out and occupied) was identified for inclusion in this analysis as background traffic. Traffic for this development will be obtained from the Beaver Creek Residential Development TIA (Stantec, July 2015).

In addition to the approved development traffic, an annual growth rate of 3% will also be applied to the existing traffic volumes up to the year 2022.

Separate entering and existing directional distributions will be used for the site based on a review of surrounding land uses and the existing roadway network. The following overall distribution will be used for entering traffic:

- ¿ 25% from the east on US 64
- ¿ 25% from the east on Jenks Road
- ¿ 25% from the west on Jenks Road
- 25% from the south on Creekside Landing Drive

The following overall distribution will be used for entering traffic:

- ¿ 50% to the east on Jenks Road
- ¿ 25% to the west on US 64
- 2 15% to the south on Creekside Landing Drive
- ; 10% to the west on Jenks Road

The property is currently occupied by a few single-family homes, and as currently envisioned the development will consist of approximately 305 apartments. Trips will be generated using ITE Trip Generation 10th Edition rates. See attached trip generation table.

The Wayforth at Apex											
Table 1 - Trip Generation											
Land Use	Intensity		Daily			AM Peak Hour PM Peak Hour				ur	
Land USE			Total	In	Out	Total	In	Out	Total	In	Out
221 Multifamily Housing (Mid-Rise)	305	d.u.	1,662	831	831	102	27	75	129	79	50

10/26/18



Appendix B: Trip Generation

The Wayforth at Apex											
Table 1 - Trip Generation											
Land Use	Intensity		Daily			AM Peak Hour			PM Peak Hour		
Land OSE	linten		Total	In	Out	Total	In	Out	Total	In	Out
221 Multifamily Housing (Mid-Rise)											

1/21/19

Appendix C: Traffic Count Data

Morris Acres Road/and I enks Road AM and PM Peak Hour Traffic Count Count Performed: Tuesday, October 23, 2018

		0		1.6	enks Ro	ad	Morri	s Acres	Poad	1.6	nks Ro	ad	
	5.0	outhbou	nd		estbour			orthbou			astbour		Intersection
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Volume
7:00	0	0	0	2	15	0	28	0	13	0	53	9	120
7:15	0	0	0	2	21	0	26	0	13	0	38	9	109
7:30	0	0	0	2	37	0	24	0	13	0	54	9	139
7:45	0	0	0	4	31	0	37	0	13	0	55	3	143
8:00	0	0	0	4	23	0	29	0	9	0	54	9	128
8:15	0	0	0	6	22	0	20	0	6	0	46	13	113
8:30	0	0	0	3	27	0	18	0	15	0	45	11	119
8:45	0	0	0	4	28	0	15	0	17	0	53	12	129
16:00	0	0	0	11	39	0	36	0	16	0	47	24	173
16:15	0	0	0	16	47	0	38	0	12	0	39	24	176
16:30	0	0	0	9	45	0	48	0	16	0	38	14	170
16:45	0	0	0	11	59	0	35	0	17	0	41	21	184
17:00	0	0	0	10	65	0	41	0	15	0	47	23	201
17:15	0	0	0	15	52	0	44	0	20	0	45	31	207
17:30	0	0	0	15	67	0	43	0	11	0	63	28	227
17:45	0	0	0	17	66	0	47	0	17	0	61	17	225
Peak Hour	SBL	SBT	SBR	WBL	WBT	WBR	NBL	NBT	NBR	EBL	EBT	EBR	Volume
7:00 - 8:00	0	0	0	10	104	0	115	0	52	0	200	30	511
7:15 - 8:15	0	0	0	12	112	0	116	0	48	0	201	30	519
7:30 - 8:30	0	0	0	16	113	0	110	0	41	0	209	34	523
7:45 - 8:45	0	0	0	17	103	0	104	0	43	0	200	36	503
8:00 - 9:00	0	0	0	17	100	0	82	0	47	0	198	45	489
Peak Hour	SBL	SBT	SBR	WBL	WBT	WBR	NBL	NBT	NBR	EBL	EBT	EBR	Volume
16:00 - 17:00	0	0	0	47	190	0	157	0	61	0	165	83	703
16:15 - 17:15	0	0	0	46	216	0	162	0	60	0	165	82	731
16:30 - 17:30	0	0	0	45	221	0	168	0	68	0	171	89	762
16:45 - 17:45 17:00 - 18:00	0	0	0	51 57	243 250	0	163 175	0	63 63	0	196 216	103 99	819 860
17:00 - 18:00	U	U	U	5/	250	U	1/5	U	63	U	216	99	860
					Doal L	Jour Tra	ffic Volu	ımer					
Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Volume
7:30 - 8:30	110	0	41	0	0	0	0	209	34	16	113	0	523
17:00 - 18:00	175	0	63	0	0	0	0	216	99	57	250	0	860
17.00 - 10.00	1/3		1 05	<u> </u>				10	1 23	_ J/	230	J	000
				F	eak-Ho	ur Facto	r by Mo	vement					
Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	PHF
7:30 - 8:30	0.743	-	0.788	-	-	-	-	0.950	0.654	0.667	0.764	-	0.914
17:00 - 18:00		-	0.788		-	-	0.250	0.857			0.933	-	0.947
				1		ı			,			1	
				ı	Peak-Ho	ur Facto	or by Ap	proach					
Peak Hour		NB			SB		,	EB			WB		PHF
7:30 - 8:30		0.76			-			0.96			0.83		0.91
17:00 - 18:00		0.93			-		İ	0.87		İ	0.93		0.95
										•			
				Heav	v Vehicl	e Percei	ntage by	/ Movem	nent				
Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	%HV
7:30 - 8:30	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
		0%	0%	0%	0%		0%	0%		0%	0%		0%

	Heavy Vehicle Percentage by Approach											
Peak Hour NB SB EB WB %HV												
7:30 - 8:30	7:30 - 8:30											
17:00 - 18:00	0%	0%	0%	0%	0%							

0%

0%

0%

0%

0%

0%

0%

0%

17:00

18:00

0%

0%

0%

0%

0%

Morris Acres Road and Creekside Landing Drive/AM and PM Peak Hour Traffic Count Count Performed: Tuesday, October 23, 2018

		s Acres			0			s Acres			de Landi		Intersection
		uthbou			estbour			rthbou			astbour		Volume
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00	0	19	5	0	0	0	11	25	0	3	0	20	83
7:15	0	25	8	0	0	0	10	26	0	4	0	28	101
7:30	0	40	3	0	0	0	10	27	0	5	0	21	106
7:45	0	17	1	0	0	0	19	28	0	15	0	23	103
8:00	0	20	3	0	0	0	16	32	0	6	0	23	100
8:15	0	31	6	0	0	0	7	18	0	2	0	31	95 94
8:30 8:45	0	14 18	8	0	0	0	14 21	26 21	0	5 8	0	27 24	101
6.45	U	10	9	U	U	U	21	21	U	0	U	24	101
16:00	0	19	20	0	0	0	35	27	0	32	0	35	168
16:15	0	26	19	0	0	0	41	42	0	21	0	42	191
16:30	0	14	12	0	0	0	35	59	0	22	0	44	186
16:45	0	17	19	0	0	0	42	44	0	19	0	43	184
17:00	0	19	16	0	0	0	37	55	0	20	0	50	197
17:15	0	29	26	0	0	0	29	59	0	31	0	43	217
17:30	0	20	23	0	0	0	39	47	0	15	0	39	183
17:45	0	15	23	0	0	0	39	74	0	30	0	46	227
Peak Hour	SBL	SBT	SBR	WBL	WBT	WBR	NBL	NBT	NBR	EBL	EBT	EBR	Volume
7:00 - 8:00	0	101	17	0	0	0	50	106	0	27	0	92	393
7:15 - 8:15	0	102	15	0	0	0	55	113	0	30	0	95	410
7:30 - 8:30	0	108	13	0	0	0	52	105	0	28	0	98	404
7:45 - 8:45	0	82	18	0	0	0	56	104	0	28	0	104	392
8:00 - 9:00	U	83	26	U	0	U	58	97	U	21	U	105	390
Peak Hour	SBL	SBT	SBR	WBL	WBT	WBR	NBL	NBT	NBR	EBL	EBT	EBR	Volume
16:00 - 17:00	0	76	70	0	0	0	153	172	0	94	0	164	729
16:15 - 17:15	0	76	66	0	0	0	155	200	0	82	0	179	758
16:30 - 17:30	0	79	73	0	0	0	143	217	0	92	0	180	784
16:45 - 17:45	0	85	84	0	0	0	147	205	0	85	0	175	781
17:00 - 18:00	0	83	88	0	0	0	144	235	0	96	0	178	824
													•
							ffic Volu						
Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Volume
7:15 - 8:15	55	113	0	0	102	15	30	0	95	0	0	0	410
17:00 - 18:00	144	235	0	0	83	88	96	0	178	0	0	0	824
				-	look Ha	ır Eacta	r by Ma	10mant					
Peak Hour	NBL	NBT	NBR	SBL	eak-Hou SBT	ur Facto SBR	EBL	EBT	EBR	WBL	WBT	WBR	PHF
7:15 - 8:15	0.724	0.883	INDK	JDL -	0.638	0.469	0.500	-	0.848	VV D L	VVDI	WDR	0.967
17:00 - 18:00						0.846		_	0.890			_	0.907
.7.00 10.00	0.525	0,,,,,,,,			0.710	0.040	0.010		0.050		<u> </u>		0.507
				F	eak-Ho	ur Facto	r by Api	oroach					
Peak Hour		NB			SB		. ۲۰۰۰	EB			WB		PHF
7:15 - 8:15		0.88			0.68			0.82			-		0.97
17:00 - 18:00		0.84			0.78			0.90			-		0.91
					y Vehicl								
Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	%HV
7:15 - 8:15	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
17:00 - 18:00	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%

Heavy Vehicle Percentage by Approach

EΒ

0%

0%

SB

0%

0%

NB

0%

0%

Peak Hour

7:15 - 8:15

17:00 - 18:00

WB

0%

0%

%HV

0%

0%

/Morris Acres Road and /US 64 AM and PM Peak Hour Traffic Count Count Performed: Tuesday, October 23, 2018

	Morri	s Acres	Road		US 64			0			0		Intersection
	Sc	uthbou	nd	W	estbour'	nd	No	orthbou	nd	Е	astbour	nd	Volume
Start Time	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
7:00	0	0	19	0	300	31	0	0	0	0	0	0	350
7:15	0	0	34	0	320	34	0	0	0	0	0	0	388
7:30	0	0	35	0	399	37	0	0	0	0	0	0	471
7:45	0	0	26	0	450	43	0	0	0	0	0	0	519
8:00	0	0	26	0	428	32	0	0	0	0	0	0	486
8:15	0	0	32	0	416	28	0	0	0	0	0	0	476
8:30	0	0	23	0	370	40	0	0	0	0	0	0	433
8:45	0	0	24	0	372	41	0	0	0	0	0	0	437
16:00	0	0	33	0	394	55	0	0	0	0	0	0	482
16:15	0	0	35	0	417	61	0	0	0	0	0	0	513
16:30	0	0	37	0	415	80	0	0	0	0	0	0	532
16:45	0	0	28	0	416	78	0	0	0	0	0	0	522
17:00	0	0	32	0	456	76	0	0	0	0	0	0	564
17:15	0	0	29	0	459	71	0	0	0	0	0	0	559
17:30	0	0	34	0	450	74	0	0	0	0	0	0	558
17:45	0	0	31	0	418	87	0	0	0	0	0	0	536
Dook Hour	CDI	CDT	CDD	W/DI	WDT	WDD	NIDI	NDT	NDD	- FDI	LEDE	L E D D	\/ - l
Peak Hour	SBL	SBT	SBR	WBL	WBT	WBR	NBL	NBT	NBR	EBL	EBT	EBR	Volume
7:00 - 8:00	0	0	114	0	1,469	145	0	0	0	0	0	0	1,728
7:15 - 8:15 7:30 - 8:30	0	0	121	0	1,597	146	0	0	0	0	0	0	1,864
7:30 - 8:30 7:45 - 8:45	0	0	119 107	0	1,693 1,664	140 143	0	0	0	0	0	0	1,952 1,914
8:00 - 9:00	0	0	107	0	1,586	141	0	0	0	0	0	0	1,832
8.00 - 9.00	U		105	U	1,500	141	U	U	U	U		U	1,032
Peak Hour	SBL	SBT	SBR	WBL	WBT	WBR	NBL	NBT	NBR	EBL	EBT	EBR	Volume
16:00 - 17:00	0	0	133	0	1,642	274	0	0	0	0	0	0	2,049
16:15 - 17:15	0	0	132	0	1,704	295	0	0	0	0	0	0	2,131
16:30 - 17:30	0	0	126	0	1,746	305	0	0	0	0	0	0	2,177
16:45 - 17:45	0	0	123	0	1,781	299	0	0	0	0	0	0	2,203
17:00 - 18:00	0	0	126	0	1,783	308	0	0	0	0	0	0	2,217
		•								•	•	•	
					Peak-F	lour Tra	ffic Volu	ımes					
Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	Volume
7:30 - 8:30	0	0	0	0	0	119	0	0	0	0	1,693	140	1,952
17:00 - 18:00	0	0	0	0	0	126	0	0	0	0	1,783	308	2,217
					eak-Ho		r by Mo						
Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	PHF
7:30 - 8:30	-	-	-	-	-	0.850	-	-	-	-	0.941	0.814	0.940
17:00 - 18:00	-	-	-	-	-	0.926	-	-	-	-	0.971	0.885	0.983
					Peak-Ho	ur Facto	r by Ap			1			
Peak Hour		NB		SB				EΒ			WB		PHF
7:30 - 8:30		-			0.85					0.93		0.94	
17:00 - 18:00		-			0.93			-			0.98		0.98
						- D -	-4 '						
Dook User	ND:	L NIDT	NDD		y Vehicl					14/5:	LWST	L 14/22	0/11/
Peak Hour	NBL 0%	NBT 004	NBR 0%	SBL OW	SBT	SBR	EBL	EBT 0%	EBR	WBL 004	WBT	WBR	%HV

	Heavy Vehicle Percentage by Movement													
Peak Hour	NBL	NBT	NBR	SBL	SBT	SBR	EBL	EBT	EBR	WBL	WBT	WBR	%HV	
7:30 - 8:30	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	
17:00 - 18:00	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	

	Heavy Vehicle Percentage by Approach												
Peak Hour	ur NB SB EB WB %HV												
7:30 - 8:30	0%	0%	0%	0%	0%								
17:00 - 18:00	0%	0%	0%	0%	0%								

Appendix D: Approved Development Data

Beaver Creek Residential Development Final Traffic Impact Analysis

Green Level Church Road and US 64 Apex, NC



Prepared for: RST Development, LLC 6110 Executive Blvd, Suite 620 Rockville, Maryland 20852

Prepared by: Stantec Consulting Services Inc. 801 Jones Franklin Road Suite 300 Raleigh, NC 27606

1.0 INTRODUCTION

The proposed Beaver Creek Residential Development is located between US 64 and Green Level Church Road, just east of NC 540, in Apex, NC, as illustrated in Figure 1.

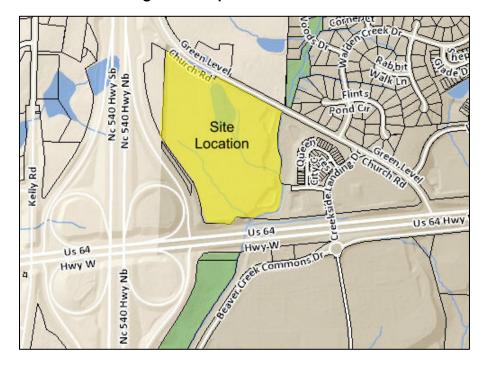


Figure 1: Proposed Site Location

The proposed 27.5 acre site is planned to have up to 300 apartments and 50 townhomes. The proposed site is anticipated to be built-out by the year 2020. A conceptual sketch illustrating the development property and the access locations used in this study is shown on the site plan in Figure 2.

The purpose of this report is to evaluate the proposed development in terms of projected traffic conditions, evaluate the ability of the adjacent roadways to accommodate the additional traffic volumes, and to recommend transportation improvements needed to mitigate congestion that may result from the additional site traffic. This report presents trip generation, trip distribution, traffic analyses, and recommendations for transportation improvements needed to meet anticipated traffic demands. This report examines the following scenarios for both the AM and PM peak hours:

- 2015 Existing
- 2020 No-Build
- 2020 Build



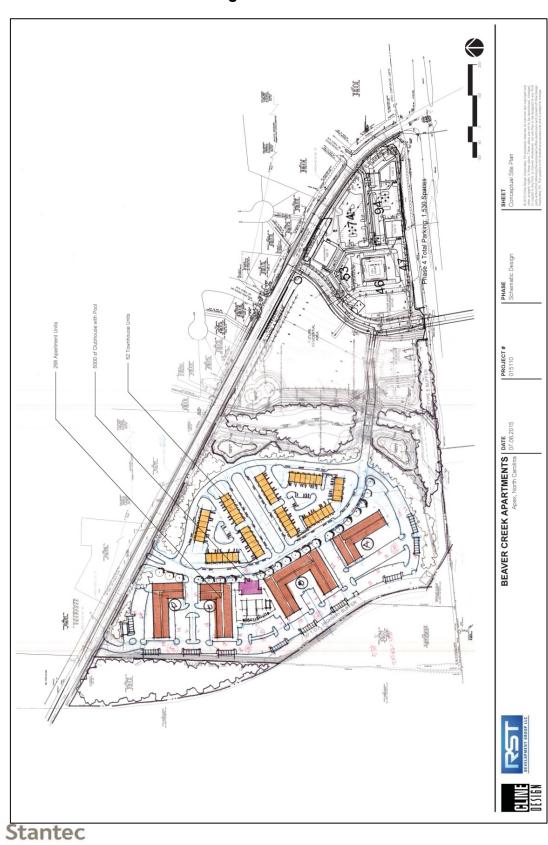


Figure 2: Site Plan

4.0 TRAFFIC GENERATION

The proposed Beaver Creek Residential Development will consist of up to 300 Apartments and 50 Condominiums. Table 2 below shows the number of anticipated trips entering and exiting the site during a typical week day and during the AM and PM peak hours.

Table 2: ITE Trip Generation Table

	ITE Trip Generation Beaver Creek Residential Development ITE Trip Generation													
1	ITE Site	,	••	Daily	,	AM Peak	(F	M Peak					
Land Use	Code	3	ize	Total	Total	Enter	Exit	Total	Enter	Exit				
Apartment	220	300	units	1942	151	30	121	183	119	64				
Residential Condominium/Townhouse	230	50	units	352	30	5	25	34	23	11				
Total New Trips	Total New Trips							217	142	75				

4.1 SITE TRIP DISTRIBUTION

In order to accurately determine the effect of the proposed development on the surrounding roadway network, an estimate of the expected distribution of traffic entering and exiting the site is needed. The following percentages were used in both the AM and PM peak hours.

- 5% to/from the west on US 64
- 20% to/from the east on US 64
- 25% to/from the north on W. Williams Street
- 15% to/from the south on W. Williams Street
- 10% to/from the north on Kelly Road
- 10% to/from the south on Kelly Road
- 15% to/from the north on NC 540

These percentages were developed using a combination of input from the Town of Apex, existing traffic volume counts, historic AADTs provided by NCDOT, and engineering judgment. Figure 8 shows the distributions described above as well as the turning movement percentages at each intersection.



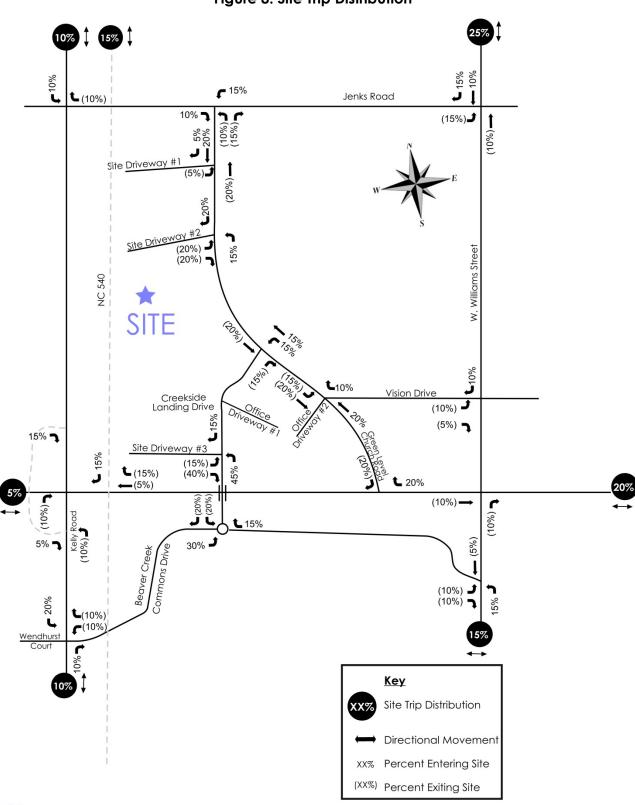


Figure 8: Site Trip Distribution



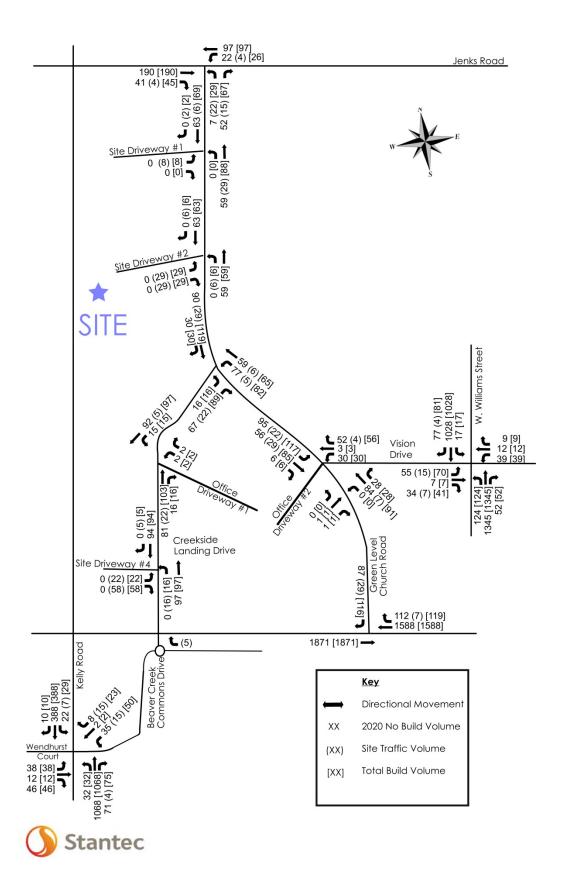


Figure 9: Future (2020) AM Build Out Traffic Volumes

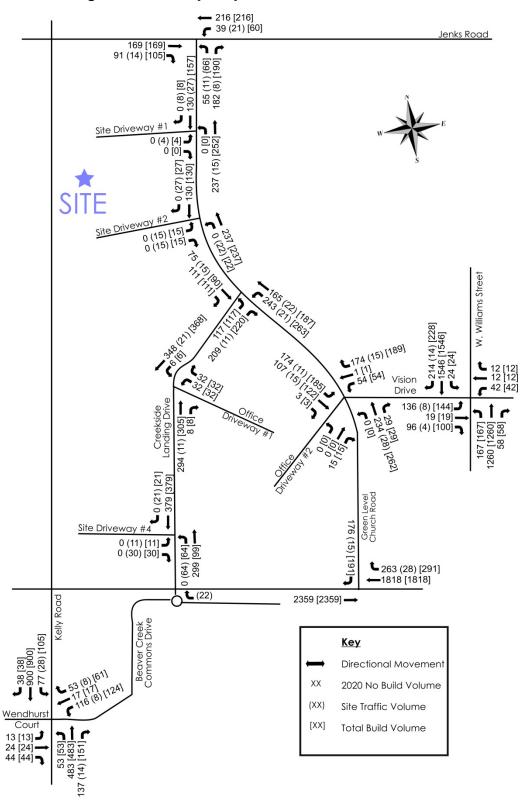


Figure 10: Future (2020) PM Build Out Traffic Volumes



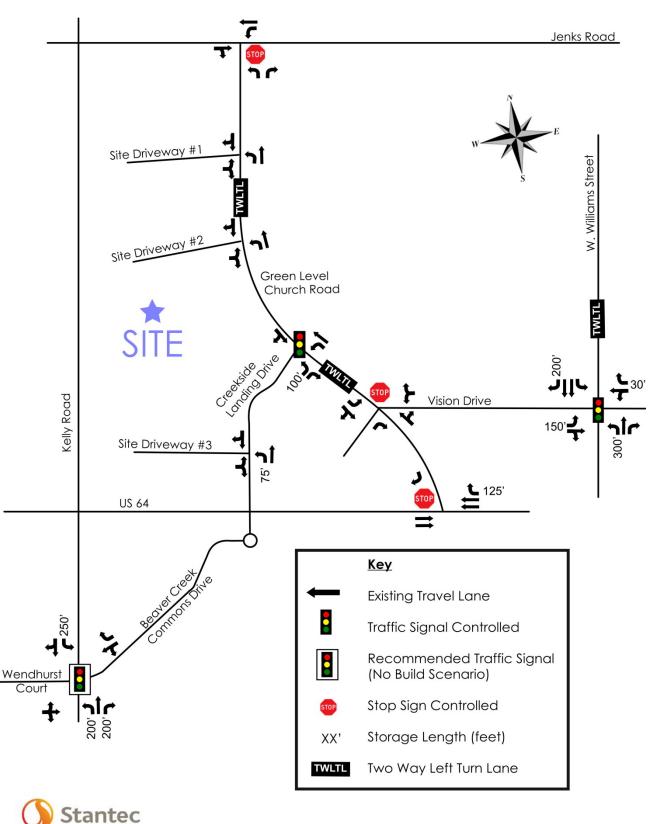


Figure 11: Recommended Geometry

Appendix E: Intersection Spreadsheets

 Project:
 The Wayforth at Apex

 Location:
 Apex, NC

 Ct. Date
 10/23/2018

 N/S Street:
 Morris Acres Road

E/W Street: Jenks Road

 AM In
 AM Out
 PM In
 PM Out

 Net New Trips:
 26
 74
 77
 50

Annual Growth Rate: 3.0% Existing Year: 2018
Growth Factor: 0.125509 Buildout Year: 2022

AM PEAK HOUR AM PHF = 0.91

				A	MPHF = 0	.91						
		Jenks Road			Jenks Road		l N	Iorris Acres Ro	ad	N	1orris A cres Ro	oad
		E astbound			W estbound			Northbound			Southbound	
Description	L eft	Through	Right	L eft	Through	Right	Left	Through	Right	Left	Through	Right
2018 Traffic Count	0	209	34	16	113	0	110	0	41	0	0	0
Count Balancing	0	0	0	0	0	0	0	0	0	0	0	0
2018 Existing Traffic	0	209	34	16	113	0	110	0	41	0	0	0
2016 EXISTING FRAITIC	0	209	34	16	113	U	110	U	41	U	U	U
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022 Background Growth	0	26	4	2	14	0	14	0	5	0	0	0
Committed Projects												
Beaver Creek Phase 4 Residential	0	0	0	0	0	0	2	0	2	0	0	0
T otal Committed T raffic	0	0	0	0	0	0	2	0	2	0	0	0
2022 Background Traffic	0	235	38	18	127	0	126	0	48	0	0	0
Project T raffic												
Percent Assignment Inbound	0%	0%	25%	25%	0%	0%	0%	0%	0%	0%	0%	0%
Inbound Project Traffic	0	0	6	7	0	0	0	0	0	0	0	0
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	10%	0%	50%	0%	0%	0%
Outbound Project Traffic	0	0	0	0	0	0	7	0	37	0	0	0
Total Project Traffic	0	0	6	7	0	0	7	0	37	0	0	0
2022 Buildout Total	0	235	44	25	127	0	133	0	85	0	0	0
Percent Impact (Approach)		2.1%			4.6%			20.2%			-	

Overall Percent Impact 8.8%

PM PEAK HOUR PM PHF = 0.95

											_	
		Jenks Road			Jenks Road		M	Iorris Acres Ro	ad	M	orris Acres Ro	
		E astbound		1	W estbound			Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
2018 Traffic Count	0	216	99	57	250	0	175	0	63	0	0	0
Count Balancing	0	0	0	0	0	0	0	0	0	0	0	0
2018 Existing Traffic	0	216	99	57	250	0	175	0	63	0	0	0
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022 Background Growth	0	27	12	7	31	0	22	0	8	0	0	0
C ommitted Projects												
Beaver Creek Phase 4 Residential	0	0	1	2	0	0	1	0	1	0	0	0
T otal C ommitted T raffic	0	0	1	2	0	0	1	0	1	0	0	0
2022 Background Traffic	0	243	112	66	281	0	198	0	72	0	0	0
Project Traffic												
Percent Assignment Inbound	0%	0%	25%	25%	0%	0%	0%	0%	0%	0%	0%	0%
Inbound Project Traffic	0	0	19	20	0	0	0	0	0	0	0	0
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	10%	0%	50%	0%	0%	0%
Outbound Project Traffic	0	0	0	0	0	0	5	0	25	0	0	0
Total Project Traffic	0	0	19	20	0	0	5	0	25	0	0	0
2022 Buildout Total	0	243	131	86	281	0	203	0	97	0	0	0
Percent Impact (A pproach)		5.1%			5.4%			10.0%			-	

Overall Percent Impact 6.6%

The Wayforth at Apex Apex, NC Balance with J enks at Morris Acres Morris Acres Road N/S Street:

E/W Street: Reedybrook Crossing/Central Site Driveway

Project:

Location:

Ct. Date

AM In AM Out PM In PM Out Net New Trips: [26 74 77 50

A nnual Growth R ate: 3.0% Growth Factor: 0.125509 E xisting Y ear: 2018
Buildout Y ear: 2022

AM PEAK HOUR AM PHF = 0.90

				^	MPHF = 0	.50						
	Re	edybrook Cross	sing	Cer	ntral Site Drive	way	M	lorris Acres Ro	ad	M	orris A cres Ro	ad
		E astbound	_		W estbound	-		Northbound			Southbound	
Description	Left	Through	Right	L eft	Through	Right	L eft	Through	Right	L eft	Through	Right
2018 Traffic Count						•		0	•			
	0	0	0	0	0	0	0	0	0	0	0	0
Count Balancing	26	0	26	0	0	0	5	125	0	0	45	5
2018 Existing Traffic	26	0	26	0	0	0	5	125	0	0	45	5
Growth Factor (0.03 per year)	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.126	0.000	0.000	0.126	0.000
2022 Background Growth	0	0	0	0	0	0	0	16	0	0	6	0
Committed Projects												
Beaver Creek Phase 4 Residential	3	0	3	0	0	0	1	0	0	0	0	1
Total Committed Traffic	3	0	3	0	0	0	1	0	0	0	0	1
2022 Background Traffic	29	0	29	0	0	0	6	141	0	0	51	6
Project Traffic												
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	5%	30%	30%	5%	0%
Inbound Project Traffic	0	0	0	0	0	0	0	1	8	8	1	0
Percent Assignment Outbound	0%	0%	0%	25%	0%	35%	0%	10%	0%	0%	5%	0%
Outbound Project Traffic	0	070	0	19	070	26	0	7	070	0	4	0
o account i roject i rame		Ü	·	'	Ü	20	"	,	Ū	ľ	7	Ü
Total Project Traffic	0	0	0	19	0	26	0	8	8	8	5	0
2022 Buildout Total	29	0	29	19	0	26	6	149	8	8	56	6
Percent Impact (A pproach)		0.0%		 	100.0%			9.8%			18.7%	

Overall Percent Impact 22.1%

PM PEAK HOUR PM PHF = 0.90

Control Circles Annual													
	Re	edybrook Cross	sing	Cer	ntral Site Drive	way	M	orris Acres Ro	ad	M	orris Acres Ro	ad	
		E astbound			W estbound			Northbound			Southbound		
Description	Left	Through	Right	L eft	Through	Right	Left	Through	Right	Left	Through	Right	
2018 Traffic Count	0	0	0	0	0	0	0	0	0	0	0	0	
Count Balancing	14	0	14	0	0	0	20	225	0	0	132	24	
2018 Existing Traffic	14	0	14	0	0	0	20	225	0	0	132	24	
Growth Factor (0.03 per year)	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.126	0.000	0.000	0.126	0.000	
2022 Background Growth	0	0	0	0	0	0	0	28	0	0	17	0	
Committed Projects													
Beaver Creek Phase 4 Residential	1	0	1	0	0	0	2	0	0	0	0	3	
T otal Committed T raffic	1	0	1	0	0	0	2	0	0	0	0	3	
2022 Background Traffic	15	0	15	0	0	0	22	253	0	0	149	27	
Project Traffic													
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	5%	30%	30%	5%	0%	
Inbound Project Traffic	0	0	0	0	0	0	0	4	23	23	4	0	
Percent Assignment Outbound	0%	0%	0%	25%	0%	35%	0%	10%	0%	0%	5%	0%	
Outbound Project Traffic	0	0	0	12	0	18	0	5	0	0	3	0	
Total Project Traffic	0	0	0	12	0	18	0	9	23	23	7	0	
2022 Buildout T otal	15	0	15	12	0	18	22	262	23	23	156	27	
Percent Impact (Approach)		0.0%			100.0%			10.4%			14.6%		

Overall Percent Impact

 Project:
 The Wayforth at Apex

 Location:
 Apex, NC

 Ct. Date
 10/23/2018

 N/S Street:
 Morris Acres Road

 E/W Street:
 Creekside Landing Drive

 AM In
 AM Out
 PM In
 PM Out

 Net New Trips:
 26
 74
 77
 50

Annual Growth Rate: 3.0% Existing Year: 2018
Growth Factor: 0.125509 Buildout Year: 2022

AM PEAK HOUR AM PHF = 0.97

				A	IVI PHF = U	.97						
	Cree	kside Landing <u>Eastbound</u>	Drive		-		N	Northbound	ad	N	Norris Acres Ro Southbound	ad
Danaviation			B. 1.		Westbound	D: 1.			51.1.	٠.,		51.1.
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
2018 Traffic Count	30	0	95	0	0	0	55	113	0	0	102	15
Count Balancing	0	0	0	0	0	0	0	0	0	0	0	0
2018 Existing Traffic	30	0	95	0	0	0	55	113	0	0	102	15
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022 Background Growth	4	0	12	0	0	0	7	14	0	0	13	2
Committed Projects Beaver Creek Phase 4 Residential	0	0	2	0	0	0	1	1	0	0	3	0
Total Committed Traffic	0	0	2	0	0	0	1	1	0	0	3	0
2022 Background Traffic	34	0	109	0	0	0	63	128	0	0	118	17
Project Traffic												
Percent Assignment Inbound	25%	0%	0%	0%	0%	0%	0%	25%	0%	0%	0%	0%
Inbound Project Traffic	7	0	0	0	0	0	0	7	0	0	0	0
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	25%	15%
Outbound Project Traffic	0	0	0	0	0	0	0	0	0	0	19	10
Total Project Traffic	7	0	0	0	0	0	0	7	0	0	19	10
2022 Buildout Total	41	0	109	0	0	0	63	135	0	0	137	27
Percent Impact (Approach)		4.7%			-			3.6%			17.7%	
Overall Percent Impact	8.4%					•			•	•		

Overall Percent Impact 8.4%

PM PEAK HOUR PM PHF = 0.91

	Croo	kside Landing I	Drivo				I 1/	lorris A cres Ro	ad	I NA	orris A cres Ro	ad I
	Cree		Jilve		-		"		du	l ivi		au
Binstant		E astbound			<u>W estbound</u>			Northbound			Southbound	
Description	Left	Through	Right	L eft	Through	Right	Left	Through	Right	Left	Through	Right
2018 Traffic Count	96	0	178	0	0	0	144	235	0	0	83	88
Count Balancing	0	0	0	0	0	0	0	0	0	0	0	0
2018 Existing Traffic	96	0	178	0	0	0	144	235	0	0	83	88
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022 Background Growth	12	0	22	0	0	0	18	29	0	0	10	11
C ommitted Projects												
Beaver Creek Phase 4 Residential	0	0	1	0	0	0	2	2	0	0	2	0
T otal Committed T raffic	0	0	1	0	0	0	2	2	0	0	2	0
2022 Background Traffic	108	0	201	0	0	0	164	266	0	0	95	99
Project Traffic												
Percent Assignment Inbound	25%	0%	0%	0%	0%	0%	0%	25%	0%	0%	0%	0%
Inbound Project Traffic	19	0	0	0	0	0	0	19	0	0	0	0
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	25%	15%
Outbound Project Traffic	0	0	0	0	0	0	0	0	0	0	13	7
Total Project Traffic	19	0	0	0	0	0	0	19	0	0	13	7
2022 Buildout Total	127	0	201	0	0	0	164	285	0	0	108	106
Percent Impact (Approach)		5.8%			-			4.2%			9.4%	

Overall Percent Impact 5.9%

The Wayforth at A pex A pex, NC 10/23/2018 N/S Street: Morris Acres Road E/W Street: US 64 WB

Project:

Location: Ct. Date

AM In AM Out PM In PM Out Net New Trips: [26 74 77 50

A nnual Growth Rate: 3.0% Growth Factor: 0.125509 Existing Y ear: 2018
Buildout Y ear: 2022

AM PEAK HOUR

				A	MPHF = 0	.94						
		US 64 WB			US 64 WB		M	orris A cres Ro	ad	l M	orris A cres Ro	oad
		E astbound		1	W estbound			Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
			_		4500	440		_			•	440
2018 Traffic Count	0	0	0	0	1693	140	0	0	0	0	0	119
Count Balancing	0	0	0	0	0	0	0	0	0	0	0	0
2018 Existing Traffic	0	0	0	0	1693	140	0	0	0	0	0	119
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022 Background Growth	0	0	0	0	212	18	0	0	0	0	0	15
Committed Projects												
Beaver Creek Phase 4 Residential	0	0	0	0	0	1	0	0	0	0	0	3
Total Committed Traffic	0	0	0	0	0	1	0	0	0	0	0	3
2022 Background Traffic	0	0	0	0	1905	159	0	0	0	0	0	137
Project Traffic												
Percent Assignment Inbound	0%	0%	0%	0%	0%	25%	0%	0%	0%	0%	0%	0%
Inbound Project Traffic	0	0	0	0	0	7	0	0	0	0	0	0
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	25%
Outbound Project Traffic	0	0	0	0	0	0	0	0	0	0	0	19
Total Project Traffic	0	0	0	0	0	7	0	0	0	0	0	19
2022 Buildout Total	0	0	0	0	1905	166	0	0	0	0	0	156
Percent Impact (A pproach)		-			0.3%			-			12.2%	

Overall Percent Impact 1.2%

PM PEAK HOUR PM PHF = 0.98

		US 64 WB			US 64 WB		l N	1orris Acres Ro	ad	M	orris A cres Ro	ad
		E astbound			W estbound			Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	L eft	Through	Right
2018 Traffic Count	0	0	0	0	1783	308	0	0	0	0	0	126
Count Balancing	0	0	0	0	0	0	0	0	0	0	0	0
2018 Existing Traffic	0	0	0	0	1783	308	0	0	0	0	0	126
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022 Background Growth	0	0	0	0	224	39	0	0	0	0	0	16
Committed Projects												
Beaver Creek Phase 4 Residential	0	0	0	0	0	3	0	0	0	0	0	2
Total Committed Traffic	0	0	0	0	0	3	0	0	0	0	0	2
2022 Background Traffic	0	0	0	0	2007	350	0	0	0	0	0	144
Project Traffic												
Percent Assignment Inbound	0%	0%	0%	0%	0%	25%	0%	0%	0%	0%	0%	0%
Inbound Project Traffic	0	0	0	0	0	19	0	0	0	0	0	0
Percent Assignment Outbound	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	25%
Outbound Project Traffic	0	0	0	0	0	0	0	0	0	0	0	13
Total Project Traffic	0	0	0	0	0	19	0	0	0	0	0	13
2022 Buildout Total	0	0	0	0	2007	369	0	0	0	0	0	157
Percent Impact (A pproach)		-			0.8%			-			8.3%	

Overall Percent Impact

The Wayforth at Apex Apex, NC Balance with J enks at Morris Acres Morris Acres Road N/S Street:

Project:

Location:

Ct. Date

E/W Street: North Site Driveway

AM In AM Out PM In PM Out Net New Trips: 26 74 50

A nnual Growth Rate: 3.0% Growth Factor: 0.125509 E xisting Y ear: 2018
Buildout Y ear: 2022

AM PEAK HOUR AM PHF = 0.90

				/ \	IVI FITE - U.	50							
	N ₀	orth Site Drivev	vay	No	orth Site Drivev	vay	l N	1orris Acres Ro	ad	Morris A cres Road			
	<u>E astbound</u>			<u>W estbound</u>				Northbound		<u>Southbound</u>			
Description	Left	Through	Right	L eft	Through	Right	Left	Through	Right	Left	Through	Right	
2018 Traffic Count	0	0	0	0	0	0	0	0	0	0	0	0	
Count Balancing	0	0	0	0	0	0	0	151	0	0	50	0	
2018 Existing Traffic	0	0	0	0	0	0	0	151	0	0	50	0	
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	
2022 Background Growth	0	0	0	0	0	0	0	19	0	0	6	0	
Committed Projects													
Beaver Creek Phase 4 Residential	0	0	0	0	0	0	0	4	0	0	1	0	
Total Committed Traffic	0	0	0	0	0	0	0	4	0	0	1	0	
2022 Background Traffic	0	0	0	0	0	0	0	174	0	0	57	0	
Project Traffic													
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	0%	5%	15%	35%	0%	
Inbound Project Traffic	0	0	0	0	0	0	0	0	1	4	9	0	
Percent Assignment Outbound	0%	0%	0%	5%	0%	15%	0%	45%	0%	0%	0%	0%	
Outbound Project Traffic	0	0	0	4	0	11	0	33	0	0	0	0	
Tatal Businest Tuesffin		^	•	1 .	0	44		22	4	,		•	
Total Project Traffic	0	0	0	4	0	11	0	33	1	4	9	0	
2022 Buildout Total	0	0	0	4	0	11	0	207	1	4	66	0	
Percent Impact (A pproach)		-			100.0%			16.4%			18.6%		

Overall Percent Impact 21.2%

PM PEAK HOUR PM PHF = 0.90

1													
	No.	orth Site Drivev	vay	No	orth Site Drivev	vay	l N	Iorris Acres Ro	ad	Morris A cres Road			
	<u>E astbound</u>				W estbound		<u>Northbound</u>			<u>Southbound</u>			
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
2018 Traffic Count	0	0	0	0	0	0	0	0	0	0	0	0	
Count Balancing	0	0	0	0	0	0	0	238	0	0	156	0	
2018 Existing Traffic	0	0	0	0	0	0	0	238	0	0	156	0	
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	
2022 Background Growth	0	0	0	0	0	0	0	30	0	0	20	0	
Committed Projects													
Beaver Creek Phase 4 Residential	0	0	0	0	0	0	0	2	0	0	4	0	
Total Committed Traffic	0	0	0	0	0	0	0	2	0	0	4	0	
2022 Background Traffic	0	0	0	0	0	0	0	270	0	0	180	0	
Project T raffic													
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	0%	5%	15%	35%	0%	
Inbound Project Traffic	0	0	0	0	0	0	0	0	4	12	27	0	
Percent Assignment Outbound	0%	0%	0%	5%	0%	15%	0%	45%	0%	0%	0%	0%	
Outbound Project Traffic	0	0	0	3	0	8	0	23	0	0	0	0	
Total Project Traffic	0	0	0	3	0	8	0	23	4	12	27	0	
2022 Buildout Total	0	0	0	3	0	8	0	293	4	12	207	0	
Percent Impact (A pproach)		-			100.0%			9.1%			17.8%		

Overall Percent Impact

Project: The Wayforth at Apex
Location: Apex, NC
Ct. Date Balance with J enks at Morris Acres
Morris Acres Road

E/W Street: South Site Driveway

 AM In
 AM Out
 PM In
 PM Out

 Net New Trips:
 26
 74
 77
 50

Annual Growth Rate: 3.0% Existing Year: 2018
Growth Factor: 0.125509 Buildout Year: 2022

AM PEAK HOUR AM PHF = 0.90

				A	MPHF = 0.	.90							
	Sc	outh Site Drivev	vay	So	uth Site Drivev	vay	M	orris Acres Ro	ad	Morris A cres Road			
	<u>E astbound</u>			<u>W estbound</u>				Northbound		<u>Southbound</u>			
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
2040 7 6 .		•					_						
2018 Traffic Count	0	0	0	0	0	0	0	0	0	0	0	0	
Count Balancing	0	0	0	0	0	0	0	130	0	0	71	0	
2018 Existing Traffic	0	0	0	0	0	0	0	130	0	0	71	0	
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	
2022 Background Growth	0	0	0	0	0	0	0	16	0	0	9	0	
Committed Projects													
Beaver Creek Phase 4 Residential	0	0	0	0	0	0	0	1	0	0	3	0	
T otal Committed Traffic	0	0	0	0	0	0	0	1	0	0	3	0	
				_			_			-			
2022 Background Traffic	0	0	0	0	0	0	0	147	0	0	83	0	
Project T raffic													
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	35%	15%	5%	0%	0%	
Inbound Project Traffic	0	0	0	0	0	0	0	9	4	1	0	0	
Percent Assignment Outbound	0%	0%	0%	10%	0%	10%	0%	0%	0%	0%	30%	0%	
Outbound Project Traffic	0	0	0	7	0	7	0	0	0	0	22	0	
T otal Project T raffic	0	0	0	7	0	7	0	9	4	1	22	0	
-													
2022 Buildout Total	0	0	0	7	0	7	0	156	4	1	105	0	
Percent Impact (Approach)		-			100.0%			8.1%			21.8%		

Overall Percent Impact 17.9%

PM PEAK HOUR PM PHF = 0.90

1													
_	Sc	outh Site Drivev	vay	So	uth Site Drivev	vay	l N	Iorris Acres Ro	ad	Morris A cres Road			
	<u>E astbound</u>				W estbound			Northbound		<u>Southbound</u>			
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right	
2018 Traffic Count	0	0	0	0	0	0	0	0	0	0	0	0	
Count Balancing	0	0	0	0	0	0	0	244	0	0	145	0	
2018 Existing Traffic	0	0	0	0	0	0	0	244	0	0	145	0	
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	
2022 Background Growth	0	0	0	0	0	0	0	31	0	0	18	0	
Committed Projects													
Beaver Creek Phase 4 Residential	0	0	0	0	0	0	0	2	0	0	1	0	
T otal C ommitted T raffic	0	0	0	0	0	0	0	2	0	0	1	0	
2022 Background Traffic	0	0	0	0	0	0	0	278	0	0	164	0	
Project T raffic													
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	35%	15%	5%	0%	0%	
Inbound Project Traffic	0	0	0	0	0	0	0	27	11	4	0	0	
Percent Assignment Outbound	0%	0%	0%	10%	0%	10%	0%	0%	0%	0%	30%	0%	
Outbound Project Traffic	0	0	0	4	0	5	0	0	0	0	15	0	
Total Project Traffic	0	0	0	4	0	5	0	27	11	4	15	0	
2022 Buildout Total	0	0	0	4	0	5	0	305	11	4	179	0	
Percent Impact (A pproach)		-			100.0%			12.0%			10.4%		

Overall Percent Impact 13.0%

Appendix F: Synchro Output: Existing (2018)

	→	•	•	←	4	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ»		*	†	¥	7
Traffic Volume (vph)	209	34	16	113	110	41
Future Volume (vph)	209	34	16	113	110	41
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	200		200	0
Storage Lanes		0	1		1	1
Taper Length (ft)			100		100	
Satd. Flow (prot)	1827	0	1770	1863	1770	1583
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	1827	0	1770	1863	1770	1583
Link Speed (mph)	45			45	45	
Link Distance (ft)	645			534	1006	
Travel Time (s)	9.8			8.1	15.2	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Shared Lane Traffic (%)						
Lane Group Flow (vph)	267	0	18	124	121	45
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize						
Intersection Capacity Utili	zation 26.1%			IC	CU Level	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	3.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	LDIX	VVDL	<u>₩</u>	NDL Š	TADK.
Traffic Vol, veh/h	209	34	16	T	110	41
Future Vol, veh/h	209	34	16	113	110	41
•	0	0	0	0	0	0
Conflicting Peds, #/hr						
Sign Control RT Channelized	Free -	Free	Free	Free	Stop	Stop
		None		None	200	None
Storage Length	-	-	200	-	200	0
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	230	37	18	124	121	45
Major/Minor Ma	ajor1	N	Major2	ı	Minor1	
Conflicting Flow All	0	0	267	0	407	248
Stage 1			201		248	240
	-	-	-	-	159	-
Stage 2	-	-	4.40	-		
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	
Pot Cap-1 Maneuver	-	-	1297	-	600	791
Stage 1	-	-	-	-	793	-
Stage 2	-	-	-	-	870	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1297	-	592	791
Mov Cap-2 Maneuver	-	-	-	-	645	-
Stage 1	-	-	-	-	793	-
Stage 2	-	-	-	-	858	-
Ŭ						
A L			MD		ND	
Approach	EB		WB		NB	
HCM Control Delay, s	0		1		11.3	
HCM LOS					В	
Minor Lane/Major Mvmt	N	NBLn1N	VBLn2	EBT	EBR	WBL
Capacity (veh/h)		645	791	-		1297
HCM Lane V/C Ratio			0.057	_		0.014
HCM Long LOS		11.9	9.8	-	-	7.8
HCM Lane LOS		В	A	-	-	A
HCM 95th %tile Q(veh)		0.7	0.2	-	-	0

2: Morris Acres Road & Reedybrook Crossing

	۶	\rightarrow	1	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	M		7	^	f)	
Traffic Volume (vph)	26	26	5	125	45	5
Future Volume (vph)	26	26	5	125	45	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			0
Storage Lanes	1	0	1			0
Taper Length (ft)	25		100			
Satd. Flow (prot)	1694	0	1770	1863	1837	0
Flt Permitted	0.976		0.950			
Satd. Flow (perm)	1694	0	1770	1863	1837	0
Link Speed (mph)	25			45	45	
Link Distance (ft)	294			470	758	
Travel Time (s)	8.0			7.1	11.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)						
Lane Group Flow (vph)	58	0	6	139	56	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	d					

Intersection Capacity Utilization 16.6% Analysis Period (min) 15

ICU Level of Service A

Intersection						
Int Delay, s/veh	2.3					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	**	00	ዃ	405	∱	_
Traffic Vol, veh/h	26	26	5	125	45	5
Future Vol, veh/h	26	26	5	125	45	5
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	29	29	6	139	50	6
Major/Minor	Minor2		Major1	٨	/lajor2	
		53				0
Conflicting Flow All	203		56	0	-	0
Stage 1	53	-	-	-	-	-
Stage 2	150	-	- 4.40	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-		-	-	-
Follow-up Hdwy	3.518	3.318	2.218	-	-	-
Pot Cap-1 Maneuver	786	1014	1549	-	-	-
Stage 1	970	-	-	-	-	-
Stage 2	878	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	783	1014	1549	-	-	-
Mov Cap-2 Maneuver	771	-	-	-	-	-
Stage 1	970	-	-	-	-	_
Stage 2	875	-	-	-	-	-
, and the second						
			ND		0.0	
Approach	EB		NB		SB	
HCM Control Delay, s	9.4		0.3		0	
HCM LOS	Α					
Minor Lane/Major Mvn	nt	NBL	NRT	EBLn1	SBT	SBR
		1549	-		-	-
Capacity (veh/h) HCM Lane V/C Ratio		0.004		0.066	_	<u>-</u>
	١	7.3	-	9.4	-	
HCM Long LOS)		_			
HCM Lane LOS	.\	A	-	A	-	-
HCM 95th %tile Q(veh	1)	0	-	0.2	-	-

	•	•	4	†	ļ	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ች	7	ሻ	↑	7	
Traffic Volume (vph)	30	95	55	113	102	15
Future Volume (vph)	30	95	55	113	102	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Grade (%)	2%	1000	1300	-5%	5%	1000
Storage Length (ft)	100	0	100	0 /0	0 /0	0
Storage Lanes	1	1	100			0
Taper Length (ft)	55	1	100			0
Satd. Flow (prot)	1752	1567	1814	1909	1785	0
Flt Permitted	0.950	1007	0.510	1000	1700	U
Satd. Flow (perm)	1752	1567	974	1909	1785	0
Right Turn on Red	17.02	Yes	314	1303	1700	Yes
Satd. Flow (RTOR)		98			11	163
Link Speed (mph)	25	30		45	45	
,	396			548	1004	
Link Distance (ft)	10.8			8.3	15.2	
Travel Time (s) Peak Hour Factor	0.97	0.97	0.97	0.97		0.97
	0.97	0.97	0.97	0.97	0.97	0.97
Shared Lane Traffic (%)	24	00	E7	110	100	^
Lane Group Flow (vph)	31 Drot	98	57	116	120 NA	0
Turn Type Protected Phases	Prot	pm+ov	pm+pt	NA		
	8	1 8	ı	6	2	
Permitted Phases	0		6	c	2	
Detector Phase	8	1	1	6	2	
Switch Phase	7.0	7.0	7.0	40.0	40.0	
Minimum Initial (s)	7.0	7.0	7.0	12.0	12.0	
Minimum Split (s)	22.7	12.2	12.2	24.0	23.1	
Total Split (s)	25.0	15.0	15.0	90.0	75.0	
Total Split (%)	21.7%	13.0%	13.0%	78.3%	65.2%	
Yellow Time (s)	3.7	3.5	3.5	5.0	4.1	
All-Red Time (s)	1.0	1.7	1.7	1.0	1.0	
Lost Time Adjust (s)	0.3	-0.2	-0.2	-1.0	-0.1	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Lead/Lag		Lead	Lead		Lag	
Lead-Lag Optimize?		Yes	Yes		Yes	
Recall Mode	None	None	None	Min	Min	
Act Effct Green (s)	6.8	9.2	25.8	30.0	16.8	
Actuated g/C Ratio	0.21	0.28	0.79	0.92	0.51	
v/c Ratio	0.09	0.19	0.06	0.07	0.13	
Control Delay	12.6	3.2	2.1	1.7	7.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	12.6	3.2	2.1	1.7	7.8	
LOS	В	Α	Α	Α	Α	
Approach Delay	5.4			1.8	7.8	
Approach LOS	Α			Α	Α	
Queue Length 50th (ft)	4	0	0	0	9	
Queue Length 95th (ft)	22	16	13	22	46	
Internal Link Dist (ft)	316			468	924	
Turn Bay Length (ft)	100		100			
Base Capacity (vph)	1086	637	1027	1909	1785	

	۶	•	•	†	↓	✓		
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR		
Starvation Cap Reductn	0	0	0	0	0			
Spillback Cap Reductn	0	0	0	0	0			
Storage Cap Reductn	0	0	0	0	0			
Reduced v/c Ratio	0.03	0.15	0.06	0.06	0.07			
Intersection Summary								
Area Type:	ther							
Cycle Length: 115								
Actuated Cycle Length: 32.7								
Natural Cycle: 60								
Control Type: Actuated-Unco	ordinated							
Maximum v/c Ratio: 0.19								
Intersection Signal Delay: 4.6	6			Int	tersection	LOS: A		
Intersection Capacity Utilizati	on 24.2%			IC	U Level o	of Service A		
Analysis Period (min) 15								
Description: 05-2280								
Splits and Phases: 3: Morr	is Acres F	load & Ci	eekside l	_anding D)rive		_	
\$ Ø1								
Ø6								
90 s							25 s	

	•	→	←	•	\	1	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations			^	7		7	
Traffic Volume (vph)	0	0	1693	140	0	119	
Future Volume (vph)	0	0	1693	140	0	119	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	0			75	0	0	
Storage Lanes	0			1	0	1	
Taper Length (ft)	25				25		
Satd. Flow (prot)	0	0	3539	1583	0	1611	
Flt Permitted							
Satd. Flow (perm)	0	0	3539	1583	0	1611	
Link Speed (mph)		55	55		45		
Link Distance (ft)		890	661		383		
Travel Time (s)		11.0	8.2		5.8		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	0	1801	149	0	127	
Sign Control		Free	Free		Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	d						
Intersection Capacity Utiliz	zation 60.8%			IC	U Level	of Service	э В
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	1.7					
		EST	MAIST	WED	051	000
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations				- 7		7
Traffic Vol, veh/h	0		1693	140	0	119
Future Vol, veh/h	0	0	1693	140	0	119
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	75	-	0
Veh in Median Storage,	,# -	-	0	-	0	-
Grade, %	_	0	0	_	0	_
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	0	1801	149	0	127
IVIVIIIL I IOW	U	U	1001	143	U	121
Major/Minor		N	Major2	N	/linor2	
Conflicting Flow All			-	0	-	901
Stage 1			_	_	_	_
Stage 2			_	_	_	_
Critical Hdwy			_	_	_	6.94
Critical Hdwy Stg 1			_	_	_	0.5-
Critical Hdwy Stg 2						_
			-	-		3.32
Follow-up Hdwy			-	-	-	
Pot Cap-1 Maneuver			-	-	0	281
Stage 1			-	-	0	-
Stage 2			-	-	0	-
Platoon blocked, %			-	-		
Mov Cap-1 Maneuver			-	-	-	281
Mov Cap-2 Maneuver			-	-	-	-
Stage 1			_	-	_	_
Stage 2			_	_	_	_
otago _						
Approach			WB		SB	
HCM Control Delay, s			0		27.9	
HCM LOS					D	
MC L /M - ' M		MOT	WDD	ODL . 4		
Minor Lane/Major Mvm	Į .	WBT	WBR			
Capacity (veh/h)		-	-	_0.		
HCM Lane V/C Ratio		-	-	0.451		
HCM Control Delay (s)		-	-	27.9		
HCM Lane LOS		-	-	D		
HCM 95th %tile Q(veh)		-	-	2.2		

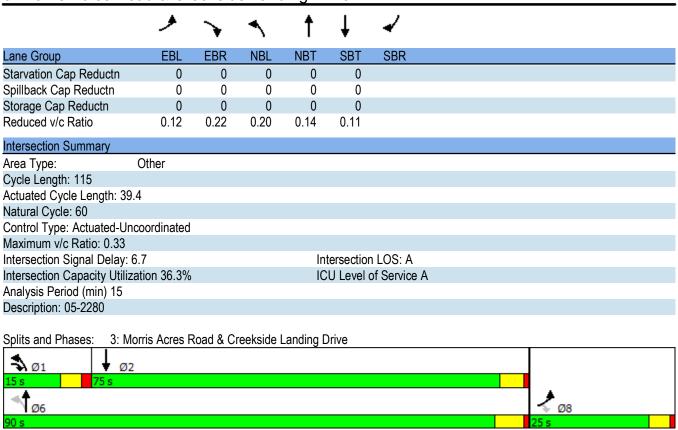
	-	\rightarrow	•	←	1	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		*	†	ħ	7
Traffic Volume (vph)	216	99	57	250	175	63
Future Volume (vph)	216	99	57	250	175	63
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	200		200	0
Storage Lanes		0	1		1	1
Taper Length (ft)			100		100	
Satd. Flow (prot)	1785	0	1770	1863	1770	1583
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	1785	0	1770	1863	1770	1583
Link Speed (mph)	45			45	45	
Link Distance (ft)	645			534	1006	
Travel Time (s)	9.8			8.1	15.2	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)						
Lane Group Flow (vph)	331	0	60	263	184	66
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized	t					
Intersection Capacity Utiliz	ation 40.4%			IC	CU Level	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	4.6					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	<u>₽</u>	LDI	VVDL T	VVB1	NDL	TADK.
Traffic Vol, veh/h	216	99	57	250	175	63
Future Vol, veh/h	216	99	57	250	175	63
Conflicting Peds, #/hr	0	0	0	230	0	03
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	Stop -	None
		NOTIE	200	NONE -	200	0
Storage Length	- 4 0		200			
Veh in Median Storage,		-		0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	227	104	60	263	184	66
Major/Minor M	ajor1	ı	Major2	ı	Minor1	
Conflicting Flow All	0	0	332	0	662	279
Stage 1	-	-	-	-	279	-
Stage 2	_	_	_	_	383	_
Critical Hdwy	_	_	4.12	_	6.42	6.22
•	_	-	4.12	-	5.42	0.22
Critical Hdwy Stg 1	-	-	_	-		
Critical Hdwy Stg 2	-	-	2 240	-	5.42	2 240
Follow-up Hdwy	-	-	2.218	-	3.518	
Pot Cap-1 Maneuver	-	_	1227	-	427	760
Stage 1	-	-	-	-	768	-
Stage 2	-	-	-	-	689	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1227	-	406	760
Mov Cap-2 Maneuver	-	-	-	-	505	-
Stage 1	-	-	-	-	768	-
Stage 2	-	-	-	-	655	-
Approach	EB		WB		NB	
					14.6	
HCM Control Delay, s	0		1.5			
HCM LOS					В	
Minor Lane/Major Mvmt	1	NBLn11	VBLn2	EBT	EBR	WBL
Capacity (veh/h)		505	760	_		1227
HCM Lane V/C Ratio		0.365		_		0.049
HCM Control Delay (s)		16.2	10.2	_	_	8.1
HCM Lane LOS		C	В	_	_	Α
HCM 95th %tile Q(veh)		1.7	0.3	_	_	0.2
HOW Sour Wille Q(ven)		1.1	0.3	-	-	0.2

	٠	*	1	†	ļ	1
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		Ţ	†	f)	
Traffic Volume (vph)	14	14	20	225	132	24
Future Volume (vph)	14	14	20	225	132	24
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			0
Storage Lanes	1	0	1			0
Taper Length (ft)	25		100			
Satd. Flow (prot)	1694	0	1770	1863	1824	0
Flt Permitted	0.976		0.950			
Satd. Flow (perm)	1694	0	1770	1863	1824	0
Link Speed (mph)	25			45	45	
Link Distance (ft)	294			470	758	
Travel Time (s)	8.0			7.1	11.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)						
Lane Group Flow (vph)	32	0	22	250	174	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize						
Intersection Capacity Utiliz	zation 25.1%			IC	U Level of	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	1					
iiii Delay, S/VeII	1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	- 14		- 1		₽	
Traffic Vol, veh/h	14	14	20	225	132	24
Future Vol, veh/h	14	14	20	225	132	24
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	_	None	_	None	_	None
Storage Length	0	-	100	-	_	-
Veh in Median Storage		_	-	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	90	90	90	90	90	90
	2	2	2	2	2	2
Heavy Vehicles, %						
Mvmt Flow	16	16	22	250	147	27
Major/Minor	Minor2	1	Major1	١	/lajor2	
Conflicting Flow All	454	160	173	0		0
Stage 1	160	-	-	-	_	-
Stage 2	294	_		_	_	_
Critical Hdwy	6.42	6.22	4.12	_	_	_
	5.42		4.12			
Critical Hdwy Stg 1		-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518		2.218	-	-	-
Pot Cap-1 Maneuver	564	885	1404	-	-	-
Stage 1	869	-	-	-	-	-
Stage 2	756	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	555	885	1404	-	-	-
Mov Cap-2 Maneuver	614	-	-	-	-	-
Stage 1	869	_	_	_	_	_
Stage 2	744	_	_	_	_	_
Olago Z	/					
Approach	EB		NB		SB	
HCM Control Delay, s	10.2		0.6		0	
HCM LOS	В					
NA' 1 /NA - ' NA	- 1	NDI	NDT	EDL .4	ODT	ODD
Minor Lane/Major Mvn	nt	NBL	NRT	EBLn1	SBT	SBR
Capacity (veh/h)		1404	-	725	-	-
HCM Lane V/C Ratio		0.016	-	0.043	-	-
HCM Control Delay (s)	7.6	-	10.2	-	-
HCM Lane LOS		Α	-	В	-	-
HCM 95th %tile Q(veh	1)	0	-	0.1	-	-
	,					

	•	•	4	†	↓	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	*	7	ሻ	<u> </u>	7	UBIN
Traffic Volume (vph)	96	178	144	235	83	88
Future Volume (vph)	96	178	144	235	83	88
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Grade (%)	2%	1300	1300	-5%	5%	1300
Storage Length (ft)	100	0	100	3 70	J /0	0
Storage Lanes	1	1	100			0
Taper Length (ft)	55	1	100			U
Satd. Flow (prot)	1752	1567	1814	1909	1689	0
Flt Permitted	0.950	1001	0.448	1303	1000	U
Satd. Flow (perm)	1752	1567	855	1909	1689	0
Right Turn on Red	1732	Yes	000	1303	1009	Yes
•		196			85	165
Satd. Flow (RTOR)	25	190		45	45	
Link Speed (mph)						
Link Distance (ft)	396			548	1004	
Travel Time (s)	10.8	0.04	0.04	8.3	15.2	0.04
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Shared Lane Traffic (%)	105	100	450	050	400	^
Lane Group Flow (vph)	105	196	158	258	188	0
Turn Type	Prot	pm+ov	pm+pt	NA	NA	
Protected Phases	8	1	1	6	2	
Permitted Phases		8	6			
Detector Phase	8	1	1	6	2	
Switch Phase	= -					
Minimum Initial (s)	7.0	7.0	7.0	12.0	12.0	
Minimum Split (s)	22.7	12.2	12.2	24.0	23.1	
Total Split (s)	25.0	15.0	15.0	90.0	75.0	
Total Split (%)	21.7%	13.0%	13.0%	78.3%	65.2%	
Yellow Time (s)	3.7	3.5	3.5	5.0	4.1	
All-Red Time (s)	1.0	1.7	1.7	1.0	1.0	
Lost Time Adjust (s)	0.3	-0.2	-0.2	-1.0	-0.1	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Lead/Lag		Lead	Lead		Lag	
Lead-Lag Optimize?		Yes	Yes		Yes	
Recall Mode	None	None	None	Min	Min	
Act Effct Green (s)	7.3	16.8	25.1	26.4	12.3	
Actuated g/C Ratio	0.19	0.43	0.64	0.67	0.31	
v/c Ratio	0.33	0.25	0.22	0.20	0.32	
Control Delay	18.5	2.2	4.8	4.6	9.1	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	18.5	2.2	4.8	4.6	9.1	
LOS	В	A	A	A	A	
Approach Delay	7.9			4.7	9.1	
Approach LOS	Α.			A	A	
Queue Length 50th (ft)	22	0	13	22	17	
Queue Length 95th (ft)	55	21	33	51	58	
Internal Link Dist (ft)	316	21	- 00	468	924	
Turn Bay Length (ft)	100		100	+00	324	
		070		1000	1600	
Base Capacity (vph)	907	872	792	1909	1689	



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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations			^	7		7	
Traffic Volume (vph)	0	0	1783	308	0	126	
Future Volume (vph)	0	0	1783	308	0	126	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	0			75	0	0	
Storage Lanes	0			1	0	1	
Taper Length (ft)	25				25		
Satd. Flow (prot)	0	0	3539	1583	0	1611	
Flt Permitted							
Satd. Flow (perm)	0	0	3539	1583	0	1611	
Link Speed (mph)		55	55		45		
Link Distance (ft)		890	661		383		
Travel Time (s)		11.0	8.2		5.8		
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	0	1819	314	0	129	
Sign Control		Free	Free		Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize							
Intersection Capacity Utili	zation 63.8%			IC	U Level of	of Service	e B
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	1.6					
		EST	MOT	14/00	051	000
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			^	7		- 7
Traffic Vol, veh/h	0		1783	308	0	126
Future Vol, veh/h	0	0	1783	308	0	126
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	75	-	0
Veh in Median Storage,	# -	-	0	-	0	-
Grade, %	_	0	0	-	0	_
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	0	1819	314	0	129
IVIVIIIL I IOW	U	U	1013	017	U	125
Major/Minor		N	Major2	N	/linor2	
Conflicting Flow All			-	0	-	910
Stage 1			-	-	-	-
Stage 2			-	_	_	_
Critical Hdwy			_	_	_	6.94
Critical Hdwy Stg 1			_	_	_	-
Critical Hdwy Stg 2						_
			_	_	_	3.32
Follow-up Hdwy			_	-		277
Pot Cap-1 Maneuver			-	-	0	
Stage 1			-	-	0	-
Stage 2			-	-	0	-
Platoon blocked, %			-	-		
Mov Cap-1 Maneuver			-	-	-	277
Mov Cap-2 Maneuver			-	-	-	-
Stage 1			-	-	-	-
Stage 2			-	-	-	-
			\4/D		0.5	
Approach			WB		SB	
HCM Control Delay, s			0		28.8	
HCM LOS					D	
Minor Lang/Major Mymt		WDT	WBR :	CDI n1		
Minor Lane/Major Mvmt		WBT				
Capacity (veh/h)		-	-			
HCM Lane V/C Ratio		-		0.464		
HCM Control Delay (s)		-	-	-0.0		
HCM Lane LOS		-	-	D		
HCM 95th %tile Q(veh)		-	-	2.3		

Appendix G: Synchro Output: Background (2022)

	→	\rightarrow	•	←	4	/
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)		7	†	7	7
Traffic Volume (vph)	235	38	18	127	126	48
Future Volume (vph)	235	38	18	127	126	48
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	200		200	0
Storage Lanes		0	1		1	1
Taper Length (ft)			100		100	
Satd. Flow (prot)	1827	0	1770	1863	1770	1583
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	1827	0	1770	1863	1770	1583
Link Speed (mph)	45			45	45	
Link Distance (ft)	645			534	1006	
Travel Time (s)	9.8			8.1	15.2	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Shared Lane Traffic (%)						
Lane Group Flow (vph)	300	0	20	140	138	53
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 28.6%			IC	CU Level	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	3.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	ĵ.		ች	↑	*	7
Traffic Vol, veh/h	235	38	18	127	126	48
Future Vol, veh/h	235	38	18	127	126	48
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	200	-	200	0
Veh in Median Storage,	# 0	_	-	0	0	-
Grade, %	0	_	_	0	0	_
Peak Hour Factor	91	91	91	91	91	91
	2	2	2	2	2	2
Heavy Vehicles, %		42		140	138	53
Mvmt Flow	258	42	20	140	130	53
Major/Minor M	lajor1	<u> </u>	Major2	<u> </u>	Minor1	
Conflicting Flow All	0	0	300	0	458	279
Stage 1	-	-	-	-	279	-
Stage 2	-	-	-	-	179	_
Critical Hdwy	_	_	4.12	-	6.42	6.22
Critical Hdwy Stg 1	_	-	-	_	5.42	-
Critical Hdwy Stg 2	_	_	_	_	5.42	_
Follow-up Hdwy	_	_	2.218	_	3.518	3 318
Pot Cap-1 Maneuver	_	_	1261	_	561	760
Stage 1	_	_	-	_	768	-
Stage 2	_	_	_	-	852	_
Platoon blocked, %	_	_		_	002	
Mov Cap-1 Maneuver	_	_	1261	_	552	760
Mov Cap-1 Maneuver	_	_	1201	_	616	- 100
	-	_	_	_	768	_
Stage 1		-	-			
Stage 2	-	-	-	-	838	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		1		11.8	
HCM LOS					В	
					_	
		IDI (IDI C			14/51
Minor Lane/Major Mvmt	<u> </u>	NBLn11		EBT	EBR	WBL
Capacity (veh/h)		616	760	-		1261
HCM Lane V/C Ratio		0.225		-	-	0.016
HCM Control Delay (s)		12.5	10.1	-	-	7.9
HCM Lane LOS		В	В	-	-	Α
HCM 95th %tile Q(veh)		0.9	0.2	-	-	0

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W		ħ	†	ĵ.	
Traffic Volume (vph)	29	29	6	141	51	6
Future Volume (vph)	29	29	6	141	51	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			0
Storage Lanes	1	0	1			0
Taper Length (ft)	25		100			
Satd. Flow (prot)	1694	0	1770	1863	1835	0
Flt Permitted	0.976		0.950			
Satd. Flow (perm)	1694	0	1770	1863	1835	0
Link Speed (mph)	25			45	45	
Link Distance (ft)	294			470	758	
Travel Time (s)	8.0			7.1	11.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)						
Lane Group Flow (vph)	64	0	7	157	64	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utili	ization 17.5%			IC	U Level	of Service A

Intersection Capacity Utilization 17.5%

Analysis Period (min) 15

ICU Level of Service A

Intersection						
Int Delay, s/veh	2.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y		- ነ		₽	
Traffic Vol, veh/h	29	29	6	141	51	6
Future Vol, veh/h	29	29	6	141	51	6
Conflicting Peds, #/hr		0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	-	-
Veh in Median Storage	e,# 0	-	-	0	0	-
Grade, %	0	-	_	0	0	_
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	32	32	7	157	57	7
WWW. LIOW	02	02	•	107	O1	
Major/Minor	Minor2		Major1	١	/lajor2	
Conflicting Flow All	230	60	63	0	-	0
Stage 1	60	-	-	-	-	-
Stage 2	170	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	-	_
Critical Hdwy Stg 1	5.42	-		_	_	-
Critical Hdwy Stg 2	5.42	_	_	_	_	_
Follow-up Hdwy	3.518	3.318	2 218	_	_	_
Pot Cap-1 Maneuver	758	1005	1540	_	_	_
Stage 1	963	1000	1070		_	_
	860	-	-	-	_	-
Stage 2	000	-				
Platoon blocked, %	755	4005	4540	-	-	-
Mov Cap-1 Maneuver		1005	1540	-	-	-
Mov Cap-2 Maneuver	751	-	-	-	-	-
Stage 1	963	-	-	-	-	-
Stage 2	856	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s			0.3		0	
HCM LOS	9.5 A		0.5		U	
HCWI LOS	A					
Minor Lane/Major Mvr	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1540	_	860	_	_
HCM Lane V/C Ratio		0.004		0.075	_	_
HCM Control Delay (s)	7.3	_	9.5	_	
HCM Lane LOS	7	7.5 A	_	9.5 A	_	_
	2)	0		0.2		-
HCM 95th %tile Q(veh	1)	U	-	0.2	-	-

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ሻ	7	ሻ	<u>↑</u>	7	
Traffic Volume (vph)	34	109	63	128	118	17
Future Volume (vph)	34	109	63	128	118	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Grade (%)	2%	1500	1000	-5%	5%	1000
Storage Length (ft)	100	0	100	-5 /0	3 /0	0
Storage Lanes	100	1	100			0
Taper Length (ft)	55		100			U
Satd. Flow (prot)	1752	1567	1814	1909	1785	0
Flt Permitted	0.950	1307	0.497	1909	1700	U
		1507		1000	1705	^
Satd. Flow (perm)	1752	1567	949	1909	1785	0
Right Turn on Red		Yes			40	Yes
Satd. Flow (RTOR)		112			12	
Link Speed (mph)	25			45	45	
Link Distance (ft)	396			548	1004	
Travel Time (s)	10.8			8.3	15.2	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Shared Lane Traffic (%)						
Lane Group Flow (vph)	35	112	65	132	140	0
Turn Type	Prot	pm+ov	pm+pt	NA	NA	
Protected Phases	8	1	1	6	2	
Permitted Phases		8	6			
Detector Phase	8	1	1	6	2	
Switch Phase	J	<u> </u>	· · · · · ·	U		
Minimum Initial (s)	7.0	7.0	7.0	12.0	12.0	
Minimum Split (s)	22.7	12.2	12.2	24.0	23.1	
	25.0	15.0	15.0	90.0	75.0	
Total Split (s)						
Total Split (%)	21.7%	13.0%	13.0%	78.3%	65.2%	
Yellow Time (s)	3.7	3.5	3.5	5.0	4.1	
All-Red Time (s)	1.0	1.7	1.7	1.0	1.0	
Lost Time Adjust (s)	0.3	-0.2	-0.2	-1.0	-0.1	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Lead/Lag		Lead	Lead		Lag	
Lead-Lag Optimize?		Yes	Yes		Yes	
Recall Mode	None	None	None	Min	Min	
Act Effct Green (s)	6.8	9.2	25.5	29.8	16.4	
Actuated g/C Ratio	0.21	0.28	0.79	0.92	0.51	
v/c Ratio	0.10	0.21	0.07	0.08	0.15	
Control Delay	12.6	3.2	2.1	1.7	8.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	12.6	3.2	2.1	1.7	8.0	
LOS	12.0 B	3.2 A	Α.1	Α	0.0 A	
Approach Delay	5.4	A	A	1.8	8.0	
Approach LOS	A	^	^	A	Α	
Queue Length 50th (ft)	4	0	0	0	11	
Queue Length 95th (ft)	24	17	14	24	52	
Internal Link Dist (ft)	316			468	924	
Turn Bay Length (ft)	100		100			
Base Capacity (vph)	1100	652	1017	1909	1785	

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Starvation Cap Reductn	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	0.03	0.17	0.06	0.07	0.08		
Intersection Summary							
Area Type:	Other						
Cycle Length: 115							
Actuated Cycle Length: 32	.4						
Natural Cycle: 60							
Control Type: Actuated-Un	coordinated						
Maximum v/c Ratio: 0.21							
Intersection Signal Delay:					tersection		
Intersection Capacity Utiliz	ation 34.2%			IC	U Level o	of Service A	
Analysis Period (min) 15							
Description: 05-2280							
	orris Acres F	Road & C	reekside l	_anding [Orive		
\$ Ø1	2						
15 s 75 s							
↑ ø6							₹ ø8
90 s							25 s

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			^	7		7
Traffic Volume (vph)	0	0	1905	159	0	137
Future Volume (vph)	0	0	1905	159	0	137
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0			75	0	0
Storage Lanes	0			1	0	1
Taper Length (ft)	25				25	
Satd. Flow (prot)	0	0	3539	1583	0	1611
Flt Permitted						
Satd. Flow (perm)	0	0	3539	1583	0	1611
Link Speed (mph)		55	55		45	
Link Distance (ft)		890	661		383	
Travel Time (s)		11.0	8.2		5.8	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	2027	169	0	146
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize						
Intersection Capacity Utiliz	zation 67.8%			IC	U Level of	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	2.6					
<u> </u>		FOT	MAIST	14/00	051	000
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			^	7		7
Traffic Vol, veh/h	0		1905	159	0	137
Future Vol, veh/h	0	0	1905	159	0	137
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	75	-	0
Veh in Median Storage,	# -	-	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	2027	169	0	146
WWIIICTIOW	U	U	LULI	100	U	140
Major/Minor		ľ	Major2	Λ	/linor2	
Conflicting Flow All			-	0	-	1013
Stage 1			-	-	-	-
Stage 2			-	-	-	-
Critical Hdwy			-	-	-	6.94
Critical Hdwy Stg 1			_	_	_	_
Critical Hdwy Stg 2			_	_	_	_
Follow-up Hdwy			_	_	_	3.32
Pot Cap-1 Maneuver			_	_	0	237
Stage 1			_	_	0	-
Stage 2			_		0	_
Platoon blocked, %			_	-	U	_
			_	-		227
Mov Cap-1 Maneuver			-	_	-	237
Mov Cap-2 Maneuver			-	-	-	-
Stage 1			-	-	-	-
Stage 2			-	-	-	-
Approach			WB		SB	
HCM Control Delay, s			0		41.8	
HCM LOS			U		±1.0	
TIOWI LOG						
Minor Lane/Major Mvmt		WBT	WBR :	SBLn1		
Capacity (veh/h)		_	-	237		
HCM Lane V/C Ratio		_	_	0.615		
HCM Control Delay (s)		_	_			
HCM Lane LOS		_	_	F		
HCM 95th %tile Q(veh)		_	_	3.6		
How som while Q(ven)		-	-	3.0		

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	f)		¥	†	, N	7	
Traffic Volume (vph)	243	112	66	281	198	72	
Future Volume (vph)	243	112	66	281	198	72	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)		0	200		200	0	
Storage Lanes		0	1		1	1	
Taper Length (ft)			100		100		
Satd. Flow (prot)	1783	0	1770	1863	1770	1583	
Flt Permitted			0.950		0.950		
Satd. Flow (perm)	1783	0	1770	1863	1770	1583	
Link Speed (mph)	45			45	45		
Link Distance (ft)	645			534	1006		
Travel Time (s)	9.8			8.1	15.2		
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	374	0	69	296	208	76	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type:	Other						
Control Type: Unsignalize	ed						
Intersection Capacity Utili				IC	CU Level	of Service	e A
Analysis Period (min) 15							

Intersection						
Int Delay, s/veh	5.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$		ች	↑	*	7
Traffic Vol, veh/h	243	112	66	281	198	72
Future Vol, veh/h	243	112	66	281	198	72
Conflicting Peds, #/h		0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	200	-	200	0
Veh in Median Storag	ae,# 0	-	-	0	0	-
Grade, %	0	_	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	256	118	69	296	208	76
N 4 = 1 = 1/N 41 = 2 = 1	Na:1		Ma:0	N	1:1	
Major/Minor	Major1		Major2		Minor1	045
Conflicting Flow All	0	0	374	0	750	315
Stage 1	-	-	-	-	315	-
Stage 2	-	-	-	-	435	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	
Pot Cap-1 Maneuver	-	-	1184	-	379	725
Stage 1	-	-	-	-	740	-
Stage 2	-	-	-	-	653	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuve		-	1184	-	357	725
Mov Cap-2 Maneuve	r -	-	-	-	467	-
Stage 1	-	-	-	-	740	-
Stage 2	-	-	-	-	615	-
Approach	EB		WB		NB	
			1.6		16.6	
HCM Control Delay, s HCM LOS	S 0		1.0			
HCIVI LOS					С	
Minor Lane/Major Mv	mt I	NBLn11	VBLn2	EBT	EBR	WBL
Capacity (veh/h)		467	725	-	-	1184
HCM Lane V/C Ratio		0.446	0.105	-	-	0.059
HCM Control Delay (s)	18.8	10.5	-	-	8.2
HCM Lane LOS		С	В	-	-	Α
HCM 95th %tile Q(ve	h)	2.3	0.3	-	-	0.2

Analysis Period (min) 15

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		ħ	†	f)	
Traffic Volume (vph)	15	15	22	253	149	27
Future Volume (vph)	15	15	22	253	149	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	100			0
Storage Lanes	1	0	1			0
Taper Length (ft)	25		100			
Satd. Flow (prot)	1694	0	1770	1863	1824	0
Flt Permitted	0.976		0.950			
Satd. Flow (perm)	1694	0	1770	1863	1824	0
Link Speed (mph)	25			45	45	
Link Distance (ft)	294			470	758	
Travel Time (s)	8.0			7.1	11.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)						
Lane Group Flow (vph)	34	0	24	281	196	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 26.1%			IC	U Level o	of Service A

Intersection						
Int Delay, s/veh	1					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		ሻ	↑	\$	
Traffic Vol, veh/h	15	15	22	253	149	27
Future Vol, veh/h	15	15	22	253	149	27
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	100	-	_	-
Veh in Median Storage		_	-	0	0	_
Grade, %	, + 0	_	_	0	0	-
Peak Hour Factor	90	90	90	90	90	90
	2	2	2	2	2	2
Heavy Vehicles, %						
Mvmt Flow	17	17	24	281	166	30
Major/Minor I	Minor2	ľ	Major1	١	/lajor2	
Conflicting Flow All	511	181	196	0	-	0
Stage 1	181	-	-	-	-	-
Stage 2	330	-	-	-	-	-
Critical Hdwy	6.42	6.22	4.12	-	_	_
Critical Hdwy Stg 1	5.42	_	_	_	-	_
Critical Hdwy Stg 2	5.42	_	_	_	_	_
Follow-up Hdwy		3.318	2 218	_	_	_
Pot Cap-1 Maneuver	523	862	1377	_	_	_
Stage 1	850	- 002	-	_	_	_
Stage 2	728	_	_	_	_	_
Platoon blocked, %	120	_	_	_	_	_
	514	862	1377	-	-	-
Mov Cap-1 Maneuver			13//	-	-	-
Mov Cap-2 Maneuver	584	-	-	-	-	-
Stage 1	850	-	-	-	-	-
Stage 2	715	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	10.4		0.6		0	
HCM LOS	В		0.0			
TIOM LOO						
Minor Lane/Major Mvm	<u>nt</u>	NBL	NBT I	EBLn1	SBT	SBR
Capacity (veh/h)		1377	-	000	-	-
HCM Lane V/C Ratio		0.018	-	0.048	-	-
HCM Control Delay (s)		7.7	-	10.4	-	-
HCM Lane LOS		Α	-	В	-	-
HCM 95th %tile Q(veh)	0.1	-	0.2	-	-

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ሻ	7	ሻ	<u> </u>	7	UDIK
Traffic Volume (vph)	108	201	164	266	95	99
Future Volume (vph)	108	201	164	266	95	99
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Grade (%)	2%	1000	1300	-5%	5%	1000
Storage Length (ft)	100	0	100	0 /0	0 /0	0
Storage Lanes	1	1	1			0
Taper Length (ft)	55	1	100			0
Satd. Flow (prot)	1752	1567	1814	1909	1691	0
Flt Permitted	0.950		0.438			
Satd. Flow (perm)	1752	1567	836	1909	1691	0
Right Turn on Red	1102	Yes	300	1303	1001	Yes
Satd. Flow (RTOR)		221			84	163
Link Speed (mph)	25	221		45	45	
Link Distance (ft)	396			548	1004	
, ,	10.8			8.3	15.2	
Travel Time (s) Peak Hour Factor		0.04	0.01			0.01
	0.91	0.91	0.91	0.91	0.91	0.91
Shared Lane Traffic (%)	440	004	400	202	040	^
Lane Group Flow (vph)	119	221	180	292	213	0
Turn Type Protected Phases	Prot	pm+ov	pm+pt	NA	NA	
	8	1	1	6	2	
Permitted Phases	0	8	6	^	_	
Detector Phase	8	1	1	6	2	
Switch Phase	7.0	7.0	7.0	40.0	40.0	
Minimum Initial (s)	7.0	7.0	7.0	12.0	12.0	
Minimum Split (s)	22.7	12.2	12.2	24.0	23.1	
Total Split (s)	25.0	15.0	15.0	90.0	75.0	
Total Split (%)	21.7%	13.0%	13.0%	78.3%	65.2%	
Yellow Time (s)	3.7	3.5	3.5	5.0	4.1	
All-Red Time (s)	1.0	1.7	1.7	1.0	1.0	
Lost Time Adjust (s)	0.3	-0.2	-0.2	-1.0	-0.1	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Lead/Lag		Lead	Lead		Lag	
Lead-Lag Optimize?		Yes	Yes		Yes	
Recall Mode	None	None	None	Min	Min	
Act Effct Green (s)	7.5	17.3	25.4	26.7	12.4	
Actuated g/C Ratio	0.19	0.43	0.64	0.67	0.31	
v/c Ratio	0.36	0.27	0.25	0.23	0.37	
Control Delay	19.1	2.2	5.1	4.8	10.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	19.1	2.2	5.1	4.8	10.3	
LOS	В	Α	Α	Α	В	
Approach Delay	8.1			4.9	10.3	
Approach LOS	Α			Α	В	
Queue Length 50th (ft)	25	0	15	26	22	
Queue Length 95th (ft)	62	22	38	60	69	
Internal Link Dist (ft)	316			468	924	
Turn Bay Length (ft)	100		100		,_,	
Base Capacity (vph)	898	884	782	1909	1691	
	300		. 02	1000	1001	

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR		
Starvation Cap Reductn	0	0	0	0	0			
Spillback Cap Reductn	0	0	0	0	0			
Storage Cap Reductn	0	0	0	0	0			
Reduced v/c Ratio	0.13	0.25	0.23	0.15	0.13			
Intersection Summary								
Area Type:	Other							
Cycle Length: 115								
Actuated Cycle Length: 39.	9							
Natural Cycle: 60								
Control Type: Actuated-Und	coordinated							
Maximum v/c Ratio: 0.37								
Intersection Signal Delay: 7					tersection			
Intersection Capacity Utiliza	ation 38.6%			IC	U Level o	of Service A		
Analysis Period (min) 15								
Description: 05-2280								
Splits and Phases: 3: Mo	orris Acres F	Road & C	reekside l	Landing [Orive			
\$ Ø1								
15 s 75 s								
↑ ø6							≯ Ø8	
90 s							25 s	

	۶	→	←	•	>	4
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			^	7		7
Traffic Volume (vph)	0	0	2007	350	0	144
Future Volume (vph)	0	0	2007	350	0	144
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0			75	0	0
Storage Lanes	0			1	0	1
Taper Length (ft)	25				25	
Satd. Flow (prot)	0	0	3539	1583	0	1611
Flt Permitted						
Satd. Flow (perm)	0	0	3539	1583	0	1611
Link Speed (mph)		55	55		45	
Link Distance (ft)		890	661		383	
Travel Time (s)		11.0	8.2		5.8	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	2048	357	0	147
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization	ation 71.1%			IC	U Level of	of Service C
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	2.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			^	- 7		7
Traffic Vol, veh/h	0	0	2007	350	0	144
Future Vol, veh/h	0	0	2007	350	0	144
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	Stop
Storage Length	-	-	-	75	-	0
Veh in Median Storage,	, # -	-	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	2048	357	0	147
		_				
Major/Minor		N	Major2		/linor2	
Conflicting Flow All			-	0	-	1024
Stage 1			-	-	-	-
Stage 2			-	-	-	-
Critical Hdwy			-	-	-	6.94
Critical Hdwy Stg 1			-	-	-	-
Critical Hdwy Stg 2			-	-	-	-
Follow-up Hdwy			-	-	-	3.32
Pot Cap-1 Maneuver			-	-	0	233
Stage 1			-	-	0	-
Stage 2			-	-	0	-
Platoon blocked, %			_	-		
Mov Cap-1 Maneuver			_	_	-	233
Mov Cap-2 Maneuver			_	_	_	-
Stage 1			_	_	_	_
Stage 2			_	_	_	_
Olaye Z			-	_		
Approach			WB		SB	
HCM Control Delay, s			0		43.6	
HCM LOS					Ε	
Minar Lana/Maiar Muna	L	WDT	WDD	CDL =1		
Minor Lane/Major Mvm		WBT	WBR :			
Capacity (veh/h)		-	-	233		
HCM Lane V/C Ratio		-	-	0.631		
HCM Control Delay (s)		-	-			
HCM Lane LOS		-	-	E		
HCM 95th %tile Q(veh)		-	-	3.8		

Appendix H: Synchro Output: Build-out (2022)

	→	7	1	•	1	-
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1		7	^	*	7
Traffic Volume (vph)	235	44	25	127	133	85
Future Volume (vph)	235	44	25	127	133	85
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	200		200	0
Storage Lanes		0	1		1	1
Taper Length (ft)			100		100	
Satd. Flow (prot)	1824	0	1770	1863	1770	1583
Flt Permitted			0.950		0.950	
Satd, Flow (perm)	1824	0	1770	1863	1770	1583
Link Speed (mph)	45			45	45	
Link Distance (ft)	645			534	1006	
Travel Time (s)	9.8			8.1	15.2	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Shared Lane Traffic (%)						
Lane Group Flow (vph)	306	0	27	140	146	93
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize						
Intersection Capacity Utili	zation 34.8%			IC	:U Level c	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	4.3					
	EDT	EDD	\ <i>\</i> /\DI	\\/DT	VIDI	NIDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			^		7
Traffic Vol, veh/h	235	44	25	127	133	85
Future Vol, veh/h	235	44	25	127	133	85
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	200	-	200	0
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	_	_	0	0	_
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	258	48	27	140	146	93
Major/Minor N	/lajor1	- 1	Vlajor2		Minor1	
Conflicting Flow All	0	0	307	0	477	282
					282	
Stage 1	-	-	-	-		-
Stage 2	-	-	-	-	195	-
Critical Hdwy	-	-	4.12	-	· · · -	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Holwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1254	-	547	757
Stage 1	-	-	-	_	766	-
Stage 2	_	_	_	_	838	_
Platoon blocked, %	_	_		_	050	
Mov Cap-1 Maneuver	_		1254	_	535	757
		_			605	-
Mov Cap-2 Maneuver	-	-	-	-		
Stage 1	-	-	-	-	766	-
Stage 2	-	-	-	-	820	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.3			
	U		1.5		11.9	
HCMLOS					В	
Minor Lane/Major Mvmt	- 1	NBLn1 i	NRI n2	EBT	EBR	WBL
Capacity (veh/h)		605	757	-		1254
HCM Cartral Dalay (a)		0.242		-	-	0.022
HCM Control Delay (s)		12.8	10.4	-	-	7.9
HCM Lane LOS		В	В	-	-	Α
HCM 95th %tile Q(veh)		0.9	0.4	-	-	0.1

	۶	-	*	1	•	*	1	†	1	1	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		1	1		7	1	
Traffic Volume (vph)	29	4	29	19	4	26	6	149	8	8	56	6
Future Volume (vph)	29	4	29	19	4	26	6	149	8	8	56	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	25			25			100			100		
Satd. Flow (prot)	0	1703	0	0	1694	0	1770	1848	0	1770	1835	0
Flt Permitted		0.977			0.981		0.950			0.950		
Satd. Flow (perm)	0	1703	0	0	1694	0	1770	1848	0	1770	1835	0
Link Speed (mph)		25			25			45			45	
Link Distance (ft)		294			267			470			758	
Travel Time (s)		8.0			7.3			7.1			11.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	68	0	0	54	0	7	175	0	9	69	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

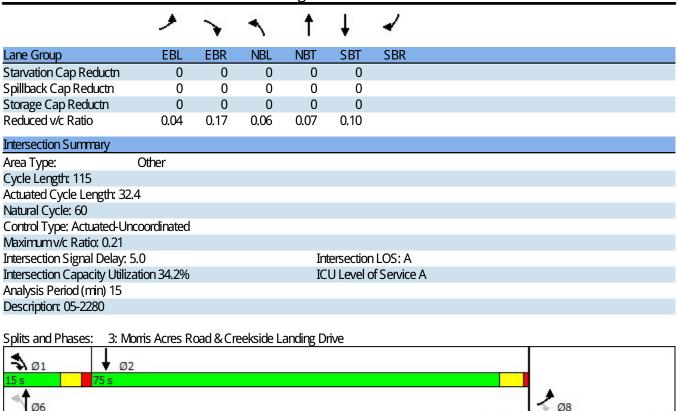
Area Type: Other Control Type: Unsignalized

Intersection Capacity Utilization 20.0% ICU Level of Service A

Analysis Period (min) 15

Intersection												
Int Delay, s/veh	3.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		*	₽		*	₽	
Traffic Vol, veh/h	29	4	29	19	4	26	6	149	8	8	56	6
Future Vol, veh/h	29	4	29	19	4	26	6	149	8	8	56	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	<u> </u>	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage	2,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	32	4	32	21	4	29	7	166	9	9	62	7
Major/Minor	Minor2			Minor1			Major1		1	Vlajor2		
Conflicting Flow All	283	271	66	285	270	170	69	0	0	174	0	0
Stage 1	83	83	-	183	183	-	-	-	-	-	-	-
Stage 2	200	188	-	102	87	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	_	_	_	_	_	_
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	669	636	998	667	636	874	1532	-	-	1403	-	-
Stage 1	925	826	-	819	748	-	-	-	-	-	-	-
Stage 2	802	745	-	904	823	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	638	629	998	637	629	874	1532	-	-	1403	-	-
Mov Cap-2 Maneuver	638	629	-	637	629	-	-	-	-	-	-	-
Stage 1	921	821	-	815	745	-	-	-	-	-	-	-
Stage 2	767	742	-	864	818	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	10.2			10.2			0.3			0.9		
HCMLOS	В			В								
Minor Lane/Major Mvn	nt	NBL	NBT	NBR	EBLn1\	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1532	_	-	767		1403	-	-			
HCM Lane V/C Ratio		0.004	_	_		0.073		_	_			
HCM Control Delay (s)		7.4	_	_	10.2	10.2	7.6	-	-			
HCM Lane LOS		Α	-	-	В	В	A	-	-			
HCM 95th %tile Q(veh))	0	-	-	0.3	0.2	0	-	-			

	۶	*	1	†	↓	4
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	*	7	ሻ	^	1→	
Traffic Volume (vph)	41	109	63	135	137	27
Future Volume (vph)	41	109	63	135	137	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Grade (%)	2%			-5%	5%	
Storage Length (ft)	100	0	100			0
Storage Lanes	1	1	1			0
Taper Length (ft)	55		100			
Satd. Flow (prot)	1752	1567	1814	1909	1776	0
Flt Permitted	0.950		0.484			
Satd. Flow (perm)	1752	1567	924	1909	1776	0
Right Tum on Red		Yes			_	Yes
Satd. Flow(RTOR)		112			16	. 20
Link Speed (mph)	25			45	45	
Link Distance (ft)	396			548	1004	
Travel Time (s)	10.8			8.3	15.2	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97
Shared Lane Traffic (%)	0.57	0.57	0.57	0.57	0,97	0.57
Lane Group Flow (vph)	42	112	65	139	169	0
Tum Type	Prot	pm+ov		NA	NA	U
Protected Phases	8	рп г оv 1	pm+pt 1	1 V A	2	
Permitted Phases	ð	8	6	Ö	۷	
Detector Phase	8		0 1	6	2	
	ð	1	ı	О	۷	
Switch Phase	7.0	70	7.0	12.0	12.0	
Minimum Initial (s)	7.0	7.0	7.0	12.0	12.0	
Minimum Split (s)	22.7	12.2	12.2	24.0	23.1	
Total Split (s)	25.0	15.0	15.0	90.0	75.0	
Total Split (%)	21.7%	13.0%	13.0%	78.3%	65.2%	
Yellow Time (s)	3.7	3.5	3.5	5.0	4.1	
All-Red Time (s)	1.0	1.7	1.7	1.0	1.0	
Lost Time Adjust (s)	0.3	-0.2	-0.2	-1.0	-0.1	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Lead/Lag		Lead	Lead		Lag	
Lead-Lag Optimize?		Yes	Yes		Yes	
Recall Mode	None	None	None	Min	Min	
Act Effct Green (s)	6.8	9.2	25.5	29.8	16.4	
Actuated g/C Ratio	0.21	0.28	0.79	0.92	0.51	
v/c Ratio	0.11	0.21	0.07	0.08	0.19	
Control Delay	12.7	3.2	2.1	1.7	8.0	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	12.7	3.2	2.1	1.7	8.0	
LOS	В	Α	A	Α	A	
Approach Delay	5.8			1.8	8.0	
Approach LOS	Α.			A	A	
Queue Length 50th (ft)	5	0	0	0	13	
Queue Length 95th (ft)	27	17	14	25	61	
Internal Link Dist (ft)	316	17	17	468	924	
Turn Bay Length (ft)	100		100	400	324	
		(E2		1000	1776	
Base Capacity (vph)	1100	652	1005	1909	1776	



	•	-	•	1	-	1
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			^	7		7
Traffic Volume (vph)	0	0	1905	166	0	156
Future Volume (vph)	0	0	1905	166	0	156
Ideal Flow(vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0			75	0	0
Storage Lanes	0			1	0	1
Taper Length (ft)	25				25	
Satd. Flow (prot)	0	0	3539	1583	0	1611
Flt Permitted						
Satd. Flow (perm)	0	0	3539	1583	0	1611
Link Speed (mph)		55	55		45	
Link Distance (ft)		890	661		383	
Travel Time (s)		11.0	8.2		5.8	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	2027	177	0	166
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 69.0%			IC	U Level c	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	3.5					
	EBL	EDT	\\/DT	W/DD	SBL	CDD
	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	_	•	^	155	•	150
Traffic Vol, veh/h	0	0	1905	166	0	156
Future Vol, veh/h	0	0	1905	166	0	156
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	75	-	0
Veh in Median Storage, #	# -	-	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	0	2027	177	0	166
IVIVITETIOVV	U	U	2027	1//	U	100
Major/Minor		1	Vajor2	N	Viinor2	
Conflicting Flow All			_	0	-	1013
Stage 1			_	_	_	-
Stage 2			_	_	_	_
Critical Holwy			_		_	6.94
			_	-	_	
Critical Hdwy Stg 1			-	-	-	-
Critical Hdwy Stg 2			-	-	-	-
Follow-up Hdwy			-	-	-	3.32
Pot Cap-1 Maneuver			-	-	0	237
Stage 1			-	-	0	-
Stage 2			-	-	0	-
Platoon blocked, %			-	-		
Mov Cap-1 Maneuver			_	_	_	237
Mov Cap-2 Maneuver			_	_	_	
Stage 1			_	_	_	_
J			_			-
Stage 2			-	-	-	-
Approach			WB		SB	
HCM Control Delay, s			0		49.4	
HCMLOS			J		-5	
TICIVILOS						
Minor Lane/Major Mvmt		WBT	WBR S	SBLn1		
Capacity (veh/h)		_	_	237		
HCM Lane V/C Ratio				0.7		
HCM Control Delay (s)				49.4		
nciviculiudi Delav (S)		-	_	43.4		
HCM Lane LOS HCM 95th %tile Q(veh)		-	-	E 4.6		

	•	•	†	1	-	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	NA.		-↑		7	^
Traffic Volume (vph)	4	11	207	4	4	66
Future Volume (vph)	4	11	207	4	4	66
Ideal Flow(vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	25				100	
Satd. Flow (prot)	1655	0	1859	0	1770	1863
Flt Permitted	0.988				0.950	
Satd. Flow (perm)	1655	0	1859	0	1770	1863
Link Speed (mph)	25		45			45
Link Distance (ft)	317		758			1006
Travel Time (s)	8.6		11.5			15.2
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)						
Lane Group Flow (vph)	16	0	234	0	4	73
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utiliz	ation 21.1%			IC	:U Level o	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	1		1		7	^
Traffic Vol, veh/h	4	11	207	4	4	66
Future Vol, veh/h	4	11	207	4	4	66
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storag		-	0	-	-	0
Grade, %	0	_	0	_	_	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	12	230	4	4	73
IVIVITETIOVV	7	12	230	7	7	75
Major/Minor	Minor1	N	Vajor1	1	Vajor2	
Conflicting Flow All	314	232	0	0	234	0
Stage 1	232	-	-	-	-	-
Stage 2	82	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Howy Stg 1	5.42	-	_	_	-	_
Critical Howy Stg 2	5.42	_	_	-	_	_
Follow-up Hdwy	3.518	3 318	_	_	2.218	_
Pot Cap-1 Maneuver	679	807	_	_	1333	_
Stage 1	807	-	_	_	-	_
Stage 2	941	_	_	_	_	_
Platoon blocked, %	741					
	677	807	-	_	1333	-
Mov Cap-1 Maneuver			-	-		-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	807	-	-	-	-	-
Stage 2	938	-	-	-	-	-
					SB	
Approach	WB		NB			
Approach	WB 9.8		NB 0		0.4	
HCM Control Delay, s	9.8		NB 0		0.4	
					0.4	
HCM Control Delay, s	9.8				0.4	
HCM Control Delay, s	9.8 A	NBT	0	WBLn1	0.4 SBL	SBT
HCM Control Delay, s HCM LOS	9.8 A	NBT -	0		SBL	SBT_
HCM Control Delay, s HCM LOS Minor Lane/Major Mvr	9.8 A		0 NBRV		SBL 1333	
HCM Control Delay, s HCM LOS Minor Lane/Wajor Mr Capacity (veh/h) HCM Lane V/C Ratio	9.8 A mt	-	0 NBRV	774 0.022	SBL 1333 0.003	-
HCM Control Delay, s HCM LOS Minor Lane/Wajor Mr Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s	9.8 A mt	- -	NBRV - -	774 0.022 9.8	SBL 1333 0.003 7.7	-
HCM Control Delay, s HCM LOS Minor Lane/Wajor Mr Capacity (veh/h) HCM Lane V/C Ratio	9.8 A mt	- - -	NBRV - -	774 0.022	SBL 1333 0.003	- - -

	•		†	-	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	M		13		7	^
Traffic Volume (vph)	7	7	156	4	4	105
Future Volume (vph)	7	7	156	4	4	105
Ideal Flow(vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	25				100	
Satd. Flow (prot)	1694	0	1857	0	1770	1863
Flt Permitted	0.976				0.950	
Satd. Flow (perm)	1694	0	1857	0	1770	1863
Link Speed (mph)	25		45			45
Link Distance (ft)	369		1004			470
Travel Time (s)	10.1		15.2			7.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)						
Lane Group Flow (vph)	16	0	177	0	4	117
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed .					
Intersection Capacity Utili	zation 18.5%			IC	:U Level o	of Service /
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	VVDIX	1>	NDIX	7	†
Traffic Vol, veh/h	7	7	156	4	4	105
Future Vol, veh/h	7	7	156	4	4	105
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	8	8	173	4	4	117
			.,5		•	
Major/Minor	Minor1	N	Vajor1	l	Vlajor2	
Conflicting Flow All	302	176	0	0	178	0
Stage 1	176	-	-	-	-	-
Stage 2	126	_	_	_	_	_
Critical Howy	6.42	6.22	-	_	4.12	_
Critical Howy Stg 1	5.42	-	_	_	1, 12	_
Critical Holwy Stg 2	5.42	_	-			
	3.518		_	_	2,218	-
Follow-up Hdwy			-	_		-
Pot Cap-1 Maneuver	690	867	-	-	1398	-
Stage 1	855	-	-	-	-	-
Stage 2	900	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	688	867	-	-	1398	-
Mov Cap-2 Maneuver	713	-	-	-	-	-
Stage 1	855	_	_	_	_	_
Stage 2	897	_	_	_	_	_
Junge 2	057					
Approach	WB		NB		SB	
HCM Control Delay, s	9.7		0		0.3	
HCMLOS	Α					
TICIVILOS	,,					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	782	1398	-
HCM Lane V/C Ratio		-	-		0.003	-
HCM Control Delay (s)		_	_	9.7	7.6	-
HCM Lane LOS		_	_	Α	Α	_
HCM 95th %tile Q(veh	1	_	_	0.1	0	_
וופטויאטוני עניפטויאווי	7	_	_	0, 1	U	_

	→	7	1	•	1	-
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1		*	†	*	7
Traffic Volume (vph)	243	131	86	281	203	97
Future Volume (vph)	243	131	86	281	203	97
Ideal Flow(vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)		0	200		200	0
Storage Lanes		0	1		1	1
Taper Length (ft)			100		100	
Satd. Flow (prot)	1775	0	1770	1863	1770	1583
Flt Permitted			0.950		0.950	
Satd. Flow (perm)	1775	0	1770	1863	1770	1583
Link Speed (mph)	45			45	45	
Link Distance (ft)	645			534	1006	
Travel Time (s)	9.8			8.1	15.2	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)						
Lane Group Flow (vph)	394	0	91	296	214	102
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utili	zation 46.8%			IC	CU Level o	of Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	5.8					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽		7	•	7	7
Traffic Vol, veh/h	243	131	86	281	203	97
Future Vol, veh/h	243	131	86	281	203	97
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	200	-	200	0
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	256	138	91	296	214	102
	ajor1		Vajor2		Minor1	
Conflicting Flow All	0	0	394	0	802	325
Stage 1	-	-	-	-	325	-
Stage 2	-	-	-	-	477	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	_	_	5.42	_
Follow-up Hdwy	_	_	2.218	_	3.518	3.318
Pot Cap-1 Maneuver	_	-	1165	-	353	716
Stage 1	_	_	_	_	732	_
Stage 2	_	-	_	_	624	-
Platoon blocked, %	_	_		_	5	
Mov Cap-1 Maneuver	_	_	1165	_	325	716
Mov Cap-2 Maneuver	_	_	- 1105	_	438	-
Stage 1			-	_	732	_
<u> </u>	_	-	_	-	575	-
Stage 2	-	-	-	-	3/3	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		2		17.6	
HCMLOS	-				C	
						14=
Minor Lane/Wajor Mvmt	1	NBLn1 I		EBT	EBR	WBL
Capacity (veh/h)		438	716	-		1165
HCM Lane V/C Ratio		0.488		-	-	0.078
HCM Control Delay (s)		20.8	10.9	-	-	8.3
HCM Lane LOS		C	В	-	-	Α
HCM 95th %tile Q(veh)		2.6	0.5	-	-	0.3
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		1	1		*	1	
Traffic Volume (vph)	15	4	15	12	4	18	22	262	23	23	156	27
Future Volume (vph)	15	4	15	12	4	18	22	262	23	23	156	27
Ideal Flow(vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	25			25			100			100		
Satd. Flow (prot)	0	1712	0	0	1697	0	1770	1840	0	1770	1822	0
Flt Permitted		0.978			0.983		0.950			0.950		
Satd. Flow (perm)	0	1712	0	0	1697	0	1770	1840	0	1770	1822	0
Link Speed (mph)		25			25			45			45	
Link Distance (ft)		294			267			470			758	
Travel Time (s)		8.0			7.3			7.1			11.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	38	0	0	37	0	24	317	0	26	203	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary

Area Type: Other Control Type: Unsignalized

Intersection Capacity Utilization 29.1%

Analysis Period (min) 15

ICU Level of Service A

Intersection												
Int Delay, s/veh	2.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		Y	1		Y	13	
Traffic Vol, veh/h	15	4	15	12	4	18	22	262	23	23	156	27
Future Vol, veh/h	15	4	15	12	4	18	22	262	23	23	156	27
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage	2,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	17	4	17	13	4	20	24	291	26	26	173	30
Major/Minor	Minor2			Vinor1			Vajor1		1	Vlajor2		
Conflicting Flow All	604	605	188	603	607	304	203	0	0	317	0	0
Stage 1	239	239	-	353	353	JU4 -	-	-	-	J17	-	-
Stage 2	365	366	_	250	254	_	_	_	_	_	_	_
Critical Holwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	_	_	4.12	_	_
Critical Howy Stg 1	6.12	5.52	-	6.12	5.52	-	- 1, 12	_	_	- 1.12	_	_
Critical Howy Stg 2	6.12	5.52	_	6.12	5.52	_	_	_	_	_	_	_
Follow-up Hdwy			3.318		4.018	3,318	2.218	_	_	2.218	_	_
Pot Cap-1 Maneuver	410	412	854	411	411	736	1369	-	-	1243	-	_
Stage 1	764	708	-	664	631	-	_	_	_		_	_
Stage 2	654	623	-	754	697	_	_	_	_	_	_	_
Platoon blocked, %								_	_		_	-
Mov Cap-1 Maneuver	384	396	854	388	395	736	1369	-	-	1243	-	-
Mov Cap-2 Maneuver	384	396	-	388	395	_	_	_	_	-	-	_
Stage 1	751	693	-	652	620	-	-	-	-	-	-	-
Stage 2	621	612	-	719	682	-	-	-	-	-	-	-
S												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.6			12.5			0.6			0.9		
HCMLOS	В			В			3.0			3,5		
Minor Lane/Major Mvn	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1369		-	510	519	1243	_	_			
HCM Lane V/C Ratio		0.018	-			0.073		_	_			
HCM Control Delay (s)		7.7	-	_	12.6	12.5	8	-	-			
HCM Lane LOS		Α	_	_	В	В	A	_	_			
HCM 95th %tile Q(veh)	0.1	-	_	0.2	0.2	0.1	-	-			
	,	J. 1			J	J	J. .					

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ሻ	T T	NDL 1		7	JUN
Traffic Volume (vph)	127	201	164	285	108	106
Future Volume (vph)	127	201	164	285	108	106
Ideal Flow(vphpl)	1900	1900	1900	1900	1900	1900
Grade (%)	2%	1500	1500	-5%	5%	1500
Storage Length (ft)	100	0	100	370	370	0
Storage Lanes	100	1	100			0
Taper Length (ft)	55	i i	100			U
Satd. Flow(prot)	1752	1567	1814	1909	1694	0
Flt Permitted	0.950	1307	0.432	1000	10,7-	U
Satd. Flow(perm)	1752	1567	825	1909	1694	0
Right Turn on Red	1/32	Yes	رعن	1303	1054	Yes
Satd. Flow(RTOR)		221			78	163
Link Speed (mph)	25	221		45	76 45	
Link Distance (ft)	396			548	1004	
	10.8			8.3	15.2	
Travel Time (s) Peak Hour Factor		0.01	0.91			0.91
	0.91	0.91	0.91	0.91	0.91	0.91
Shared Lane Traffic (%)	1.40	224	100	242	225	^
Lane Group Flow (vph)	140	221	180	313	235	0
Turn Type	Prot	pm+ov	pm+pt	NA	NA	
Protected Phases	8	1	1	6	2	
Permitted Phases	_	8	6		_	
Detector Phase	8	1	1	6	2	
Switch Phase				40.0	40.5	
Minimum Initial (s)	7.0	7.0	7.0	12.0	12.0	
Minimum Split (s)	22.7	12.2	12.2	24.0	23.1	
Total Split (s)	25.0	15.0	15.0	90.0	75.0	
Total Split (%)	21.7%	13.0%	13.0%	78.3%	65.2%	
Yellow Time (s)	3.7	3.5	3.5	5.0	4.1	
All-Red Time (s)	1.0	1.7	1.7	1.0	1.0	
Lost Time Adjust (s)	0.3	-0.2	-0.2	-1.0	-0.1	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	
Lead/Lag		Lead	Lead		Lag	
Lead-Lag Optimize?		Yes	Yes		Yes	
Recall Mode	None	None	None	Min	Min	
Act Effct Green (s)	7.9	17.8	25.8	27.2	12.6	
Actuated g/C Ratio	0.19	0.44	0.63	0.67	0.31	
v/c Ratio	0.41	0.27	0.25	0.25	0.41	
Control Delay	20.1	2.2	5.3	5.1	11.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	
Total Delay	20.1	2.2	5.3	5.1	11.6	
LOS	C	Α	A	A	В	
Approach Delay	9.1			5.1	11.6	
Approach LOS	Α			Α.	В	
Queue Length 50th (ft)	30	0	16	30	28	
Queue Length 95th (ft)	73	23	40	68	83	
Internal Link Dist (ft)	316	ے ۔	70	468	924	
Tum Bay Length (ft)	100		100	400	324	
		000		1000	1604	
Base Capacity (vph)	884	885	771	1909	1694	

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Starvation Cap Reductn	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	
Reduced v/c Ratio	0.16	0.25	0.23	0.16	0.14	
Intersection Summary						
Area Type:	Other					
Cycle Length: 115						
Actuated Cycle Length: 40.	7					
Natural Cycle: 60						
Control Type: Actuated-Und	coordinated					
Maximum v/c Ratio: 0.41						
Intersection Signal Delay: 7					tersection	
Intersection Capacity Utiliza	ation 40.8%			IC	U Level c	of Service A
Analysis Period (min) 15						
Description: 05-2280						
		10.5				
Splits and Phases: 3: Mo	orris Acres R	oad & Cr	eekside L	anding Di	nve	
\$ Ø1						
15 s 75 s						
106						

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Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations			^	7		7
Traffic Volume (vph)	0	0	2007	369	0	157
Future Volume (vph)	0	0	2007	369	0	157
Ideal Flow(vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0			75	0	0
Storage Lanes	0			1	0	1
Taper Length (ft)	25				25	
Satd. Flow (prot)	0	0	3539	1583	0	1611
Flt Permitted						
Satd. Flow (perm)	0	0	3539	1583	0	1611
Link Speed (mph)		55	55		45	
Link Distance (ft)		890	661		383	
Travel Time (s)		11.0	8.2		5.8	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	2048	377	0	160
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 71.9%			IC	U Level c	of Service
Analysis Period (min) 15						

-						
Intersection						
Int Delay, s/veh	3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
	EDL	EDI			SDL	
Lane Configurations	•	_	44	7	•	7
Traffic Vol, veh/h	0	0	2007	369	0	157
Future Vol, veh/h	0	0	2007	369	0	157
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	Stop
Storage Length	-	-	-	75	-	0
Veh in Median Storage,	# -	-	0	-	0	-
Grade, %	_	0	0	-	0	_
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	2048	377	0	160
Major/Minor		1	Vajor2	N	/linor2	
Conflicting Flow All			_	0	-	1024
Stage 1			-	-	-	-
Stage 2			_	-	_	-
Critical Hdwy			-	-	-	6.94
Critical Hdwy Stg 1			-	-	-	-
Critical Hdwy Stg 2			-	-	-	-
Follow-up Hdwy			-	-	-	3.32
Pot Cap-1 Maneuver			-	-	0	233
Stage 1			_	_	0	
Stage 2			_	_	0	_
Platoon blocked, %					U	
			-	-		าาา
Mov Cap-1 Maneuver			-	-	-	233
Mov Cap-2 Maneuver			-	-	-	-
Stage 1			-	-	-	-
Stage 2			-	-	-	-
_						
Amurandh			\		CD	
Approach			WB		SB	
HCM Control Delay, s			0		48.8	
HCMLOS					Ε	
Minor Lang Maior NA		\\/DT	WDD (CDL ~1		
Minor Lane/Major Mvm	L	WBT				
Capacity (veh/h)		-	-	233		
HCM Lane V/C Ratio		-	-	0.688		
HCM Control Delay (s)		-	-	48.8		
HCM Lane LOS		-	-	Е		
HCM 95th %tile Q(veh)		_	_	4.4		
				10 1		

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	M		ĵ.		7	^
Traffic Volume (vph)	4	8	293	4	12	207
Future Volume (vph)	4	8	293	4	12	207
Ideal Flow(vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	25				100	
Satd. Flow (prot)	1664	0	1859	0	1770	1863
Flt Permitted	0.985				0.950	
Satd, Flow (perm)	1664	0	1859	0	1770	1863
Link Speed (mph)	25		45			45
Link Distance (ft)	317		758			1006
Travel Time (s)	8.6		11.5			15.2
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)						
Lane Group Flow (vph)	13	0	330	0	13	230
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 25.7%			IC	:U Level o	of Service
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	A		1		7	•
Traffic Vol, veh/h	4	8	293	4	12	207
Future Vol, veh/h	4	8	293	4	12	207
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	4	9	326	4	13	230
THE POST	•		320	•		
Major/Minor	Minor1	N	Vajor1	1	Vajor2	
Conflicting Flow All	585	328	0	0	330	0
Stage 1	328	-	-	-	-	-
Stage 2	257	-	-	-	-	-
Critical Howy	6.42	6.22	-	-	4.12	_
Critical Howy Stg 1	5.42	-	_	_	-	_
Critical Howy Stg 2	5.42	_	_	_	_	_
Follow-up Hdwy	3.518	3 318	_	_	2.218	_
Pot Cap-1 Maneuver	473	713	_	_	1229	_
Stage 1	730	-	_		1223	_
	786	_	-	_	_	-
Stage 2	700	-	-	-	_	-
Platoon blocked, %	460	740	-	-	4220	-
Mov Cap-1 Maneuver	468	713	-	-	1229	-
Mov Cap-2 Maneuver	557	-	-	-	-	-
Stage 1	730	-	-	-	-	-
Stage 2	778	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.6		0		0.4	
HCMLOS	В					
Minor Lane/Major Mvn	nt	NBT	NBRV	WBLn1	SBL	SBT
Capacity (veh/h)		-	-		1229	-
HCM Lane V/C Ratio		_	-		0.011	_
HCM Control Delay (s)		_	_		8	_
HCM Lane LOS						
	١	-	-	B	Α	-
HCM 95th %tile Q(veh)	-	-	0.1	0	-

	•		†	-	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	M		13		7	^
Traffic Volume (vph)	4	5	305	11	4	179
Future Volume (vph)	4	5	305	11	4	179
Ideal Flow(vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	25				100	
Satd. Flow (prot)	1678	0	1853	0	1770	1863
Flt Permitted	0.980				0.950	
Satd. Flow (perm)	1678	0	1853	0	1770	1863
Link Speed (mph)	25		45			45
Link Distance (ft)	369		1004			470
Travel Time (s)	10.1		15.2			7.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)						
Lane Group Flow (vph)	10	0	351	0	4	199
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed .					
Intersection Capacity Utiliz	zation 26.7%			IC	:U Level o	of Service A
Analysis Period (min) 15						

-						
Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	VVDL	VVDR		NDR		
Lane Configurations Traffic Vol, veh/h	T	5	1 → 305	11	ነ	† 179
•						
Future Vol, veh/h	4	5	305	11	4	179
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None		None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	4	6	339	12	4	199
	Minor1		√ajor1		Vajor2	
Conflicting Flow All	553	345	0	0	351	0
Stage 1	345	-	-	-	-	-
Stage 2	208	-	-	-	-	-
Critical Howy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	_	_	_	_
Critical Howy Stg 2	5.42	_	_	_	_	_
Follow-up Hdwy	3.518		_	_	2.218	_
Pot Cap-1 Maneuver	494	698	_	_	1208	_
•				_	1206	
Stage 1	717	-	-	-	-	-
Stage 2	827	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	492	698	-	-	1208	-
Mov Cap-2 Maneuver	572	-	-	-	-	-
Stage 1	717	-	-	-	-	-
Stage 2	824	-	-	-	-	-
0						
	1.0					
Approach	WB		NB		SB	
HCM Control Delay, s	10.8		0		0.2	
HCMLOS	В					
Minor Lane/Major Mvn	nt	NBT	NRRV	VBLn1	SBL	SBT
		INDI				
Capacity (veh/h)		-	-		1208	-
HCM Lane V/C Ratio		-	-	0.016		-
HCM Control Delay (s)		-	-	10.8	8	-
HCM Lane LOS		-	-	В	Α	-
HCM 95th %tile Q(veh)	-	-	0	0	-

Appendix I: Signal Plans

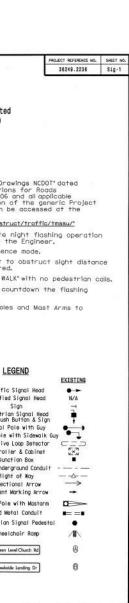


TABLE OF OPERATION PHASE 2070L LOOP & DETECTOR INSTALLATION SIGNAL INDUCTIVE LOOPS DETECTOR PROGRAMMING FACE FROM STOPBAR (FT) LOOP 21, 22 61 £A. 6X40 2-4-2 0 62 GGRY 81 IB 6X40 2-4-2 0 Y I Y Y -82 5 300 Y 2 Y Y -5 300 Y 6 Y Y -2A 6X6 DW W DW DRK P21, P22 6A 6X6 P8I, P82 DW DW W DRK 8A 6X40 2-4-2 0 Y 8 Y Y -SIGNAL FACE I.D.

O Denotes L.E.D. Metal Pole and Stop Bar Locations Sta. 9+79± 29'± Rt. Ö ast Arm B — Sta. 9+28 24, 22 Sta. 9+99 Metal Pole * Sta. 9+24± 37'± Lt.

2070L TIMING CHART										
		PH	ASE							
FEATURE	1	2	6	8						
Min Green 1 *	7	12	12	7						
Extension 1 *	2.0	6.0	6.0	2.0						
Max Green 1 *	15	90	90	25						
Yellow Clearance	3.5	4.1	5.0	3.7						
Red Clearance	1.7	1.0	1.0	1.0						
Walk 1 *	-	7	-	7						
Don't Welk 1	*	10	-	8						
Seconds Per Actuation *	55	2,5	2.5							
Max Variable Initial*	-	34	34							
Time Before Reduction *	-	15	15	-						
Time To Reduce *	-	30	30	-						
Minimum Gop	-	3.0	3.0	-						
Recall Mode	-	MIN RECALL	MIN RECALL	- 8						
Vehicle Call Memory		YELLOW	YELLOW	-						
Dual Entry	-		*							
Simultaneous Gap	ON	ON	ON	ON						

PHASING DIAGRAM

01+6

UNDETECTED MOVEMENT (OVERLAP)

PHASING DIAGRAM DETECTION LEGEND

DETECTED MOVEMENT

UNSIGNALIZED MOVEMENT

PEDESTRIAN MOVEMENT

3 Phase Fully Actuated (Isolated)

NOTES

I. Refer to "Roadway Standard Drawings NCDOT" dated July 2006, "Standard Specifications for Roads and Structures" dated July 2006 and all applicable sections of the latest version of the generic Project Special Provisions. The PSP can be accessed at the following website:

'http://www.ncdot.org/doh/preconstruct/traffic/tmssu/"

- Do not program signal for late night flashing operation unless otherwise directed by the Engineer.
- 3. Set all detector units to presence mode.
- Locate new cabinet so as not to obstruct sight distance of vehicles turning right on red.
- 5. Omit "WALK" and flashing "DON'T WALK" with no pedestrian calls.
- 6. Program pedestrian heads to countdown the flashing "Don't Walk" time only.
- 7. Contractor to furnish MetalPoles and Mast Arms to be black powder coated.

	LLULIND	
PROPOSED		EXISTING
0-	Traffic Signal Head	
0-	Modified Signal Head	N/A
_	Sign	-4
₽	Pedestrian Signal Head With Push Button & Sign	•
0	Signal Pole with Guy	•)
03 8	signal Pole with Sidewalk Guy	• 1
	Inductive Loop Detector	$\subset = \supset$
\bowtie	Controller & Cobinet	5×3
	Junction Box	
	2-in Underground Conduit	
N/A	Right of Way	
\longrightarrow	Directional Arrow	\rightarrow
-	Pavement Marking Arrow	-
(c)	Metal Pole with Mastarm	
D= =0	Rigid Metal Conduit	===
0	Pedestrian Signal Pedestal	•
N/A	Wheelchair Ramp	/ II \
(A)	Green Level Church Rd	(A)
(B)	Creekside Landing Dr	₿

New Installation SR 1600 (Green Level Church Rd.) Creekside Landing Drive Wake County

PLAN DATE: Sept 2006 REVIEWS BY: D.J. Darity
PREFUNCO ST: H.W. SUFTI REA PROD. NO.: 04111 (041)



RAMEY KEMP & ASSOCIATES, INC. TRANSPORTATION ENGINEERS

^{*} These values may be field adjusted. Do not adjust Min Green and Extension times for phases 2 and 6 lower than what is shown. Min Green for all other phases should not be lower than 4 seconds.



KIMLEY-HORN AND ASSOCIATES, INC

NC License #F-0102

MEMORANDUM

Mr. Sean Brennan, P.E., NCDOT

To:

Mr. Russell Dalton, P.E., Town of Apex

From:

Kevin Dean, P.E.

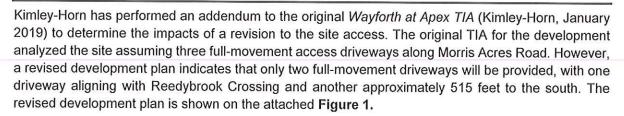
Kimley-Horn and Associates, Inc.

Date:

April 30, 2019

Subject:

The Wayforth at Apex - Traffic Analysis Addendum - Revised Site Access



As this change in access will not have impacts on any off-site intersections, it was confirmed with the Town of Apex that this addendum should only include analysis for the site driveway intersections along Morris Acres Road. It should be noted that as the previously-analyzed "North Site Driveway" is no longer proposed, the "Central Site Driveway" described in the previous analysis has been renamed to "North Site Driveway" for this addendum analysis.

This report presents trip generation, distribution, traffic analyses, and recommendations for transportation improvements required to meet anticipated traffic demands at the proposed site driveways.

Background Volume Development

AM and PM peak hour background traffic volumes were obtained from the original TIA and were not modified as part of this analysis.

Trip Generation

Trip generation data was obtained from the original TIA and was not modified as part of this analysis.

Trip Distribution and Assignment

No changes were made to the overall distribution of site traffic assumed in the original TIA. However, to account for this revised access scenario, site traffic assignment percentages were modified for this addendum. The revised site traffic assignment is shown on the attached **Figure 2**.

The attached **Figures 3** and **4** show the AM and PM peak hour site traffic and total build-out volumes at the study intersections, and volume development is detailed on the attached intersection spreadsheets.



Capacity Analysis

Consistent with the original TIA, capacity analyses were performed using Synchro Version 9.2 software. Synchro intersection level-of-service (LOS) reports are attached. The LOS for the study intersections are summarized in Table 1.

	Table 2 ervice Summary	
Condition	AM Peak Hour LOS (Delay)	PM Peak Hour LOS (Delay)
Morris Acres Road at Reedybrook Cr	ossing/North Site Driv	eway (Unsignalized)
Existing (2018) Traffic	EB – A (9.4) NBL – A (7.3)	EB – B (10.2) NBL – A (7.6)
Background (2022) Traffic	EB – A (9.5) NBL – B (7.3)	EB – B (10.4) NBL – A (7.7)
Build-out (2022) Traffic	EB – B (10.2) WB – B (10.3) NBL – A (7.4) SBL – A (7.6)	EB – B (12.8) WB – B (12.6) NBL – A (7.7) SBL – A (8.0)
Morris Acres Road at Sou	th Site Driveway (Unsi	gnalized)
Build-out (2022) Traffic	WB – A (9.7) SBL – A (7.6)	WB – B (10.8) SBL – A (8.0)

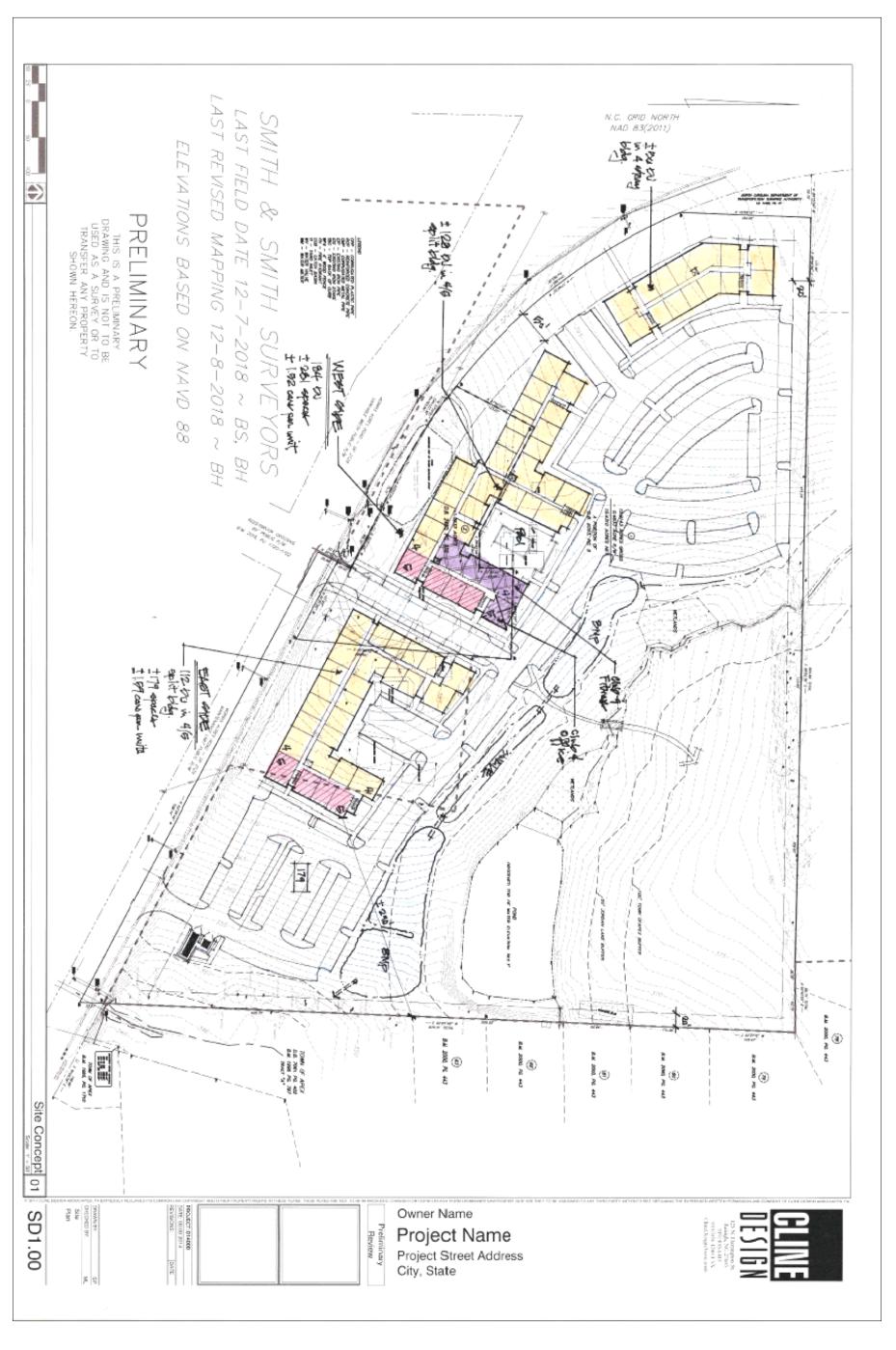
Analysis indicates that the site driveway intersections are expected to operate at an acceptable level-of-service at project build-out, and no queueing issues are expected at these intersections.

Recommendations

Based on the findings of this addendum analysis, and consistent with the original TIA, no roadway improvements are recommended to be performed to accommodate projected site traffic volumes.

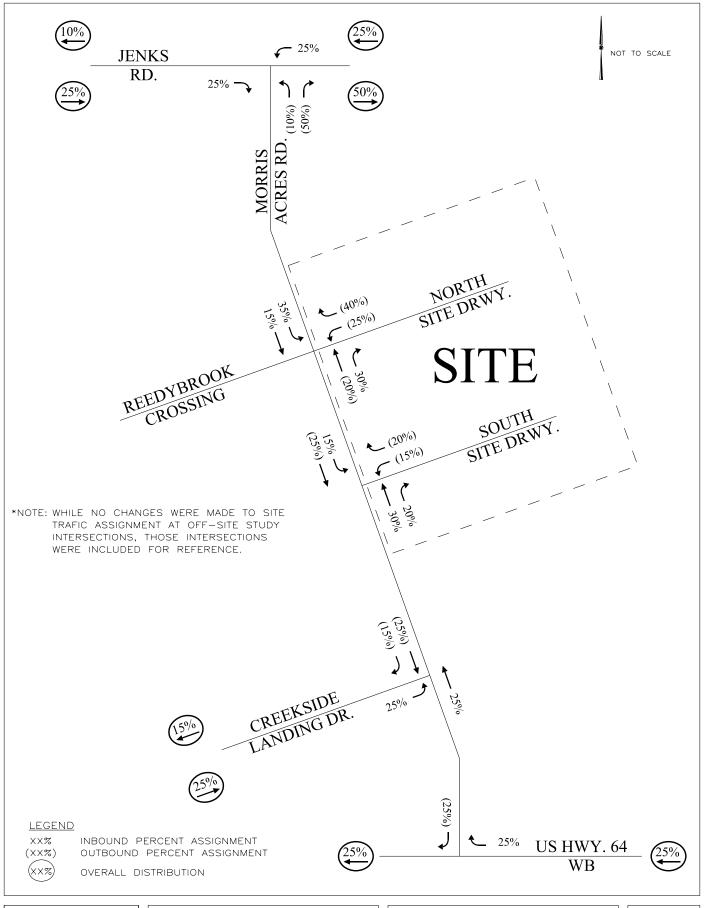
The build-out roadway laneage is shown on Figure 5.

Should you have any questions or comments, please do not hesitate to contact me at (919) 678-4185 or kevin.dean@kimley-horn.com.

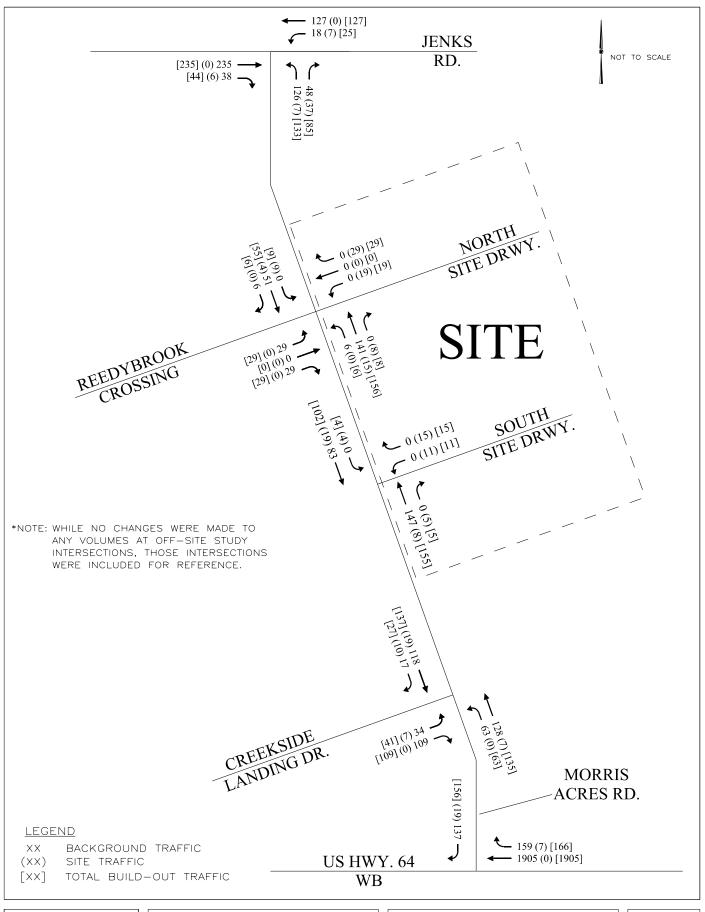


THE WAYFORTH AT APEX
APEX, NC
TRAFFIC CAPACITY ANALYSIS

ADDENDUM ANALYSIS — REVISED DEVELOPMENT PLAN

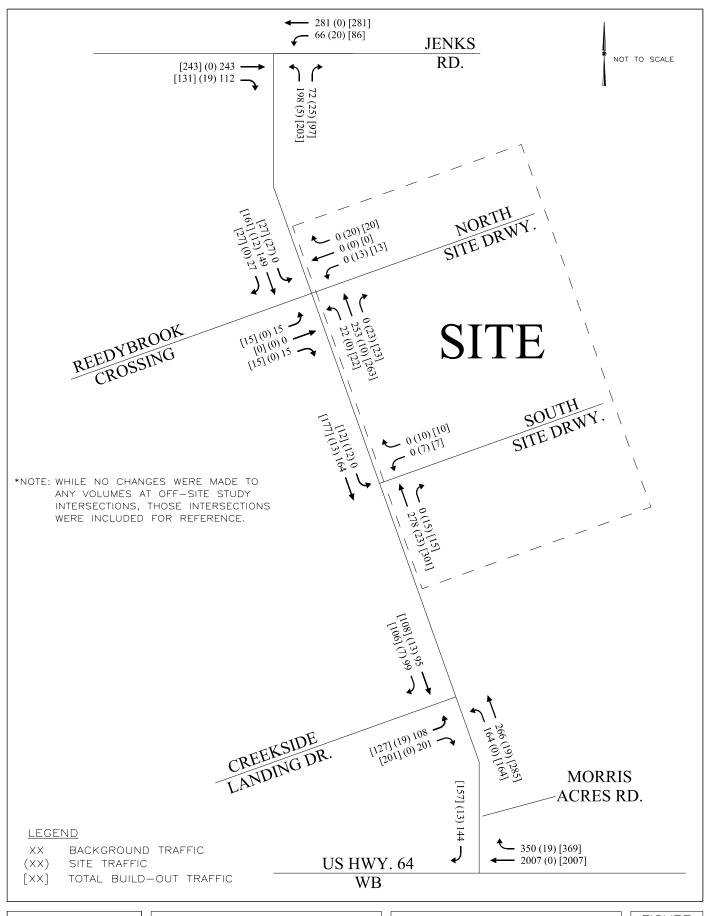


THE WAYFORTH AT APEX APEX, NC TRAFFIC CAPACITY ANALYSIS ADDENDUM ANALYSIS — SITE TRAFFIC DISTRIBUTION AND PERCENT ASSIGNMENT



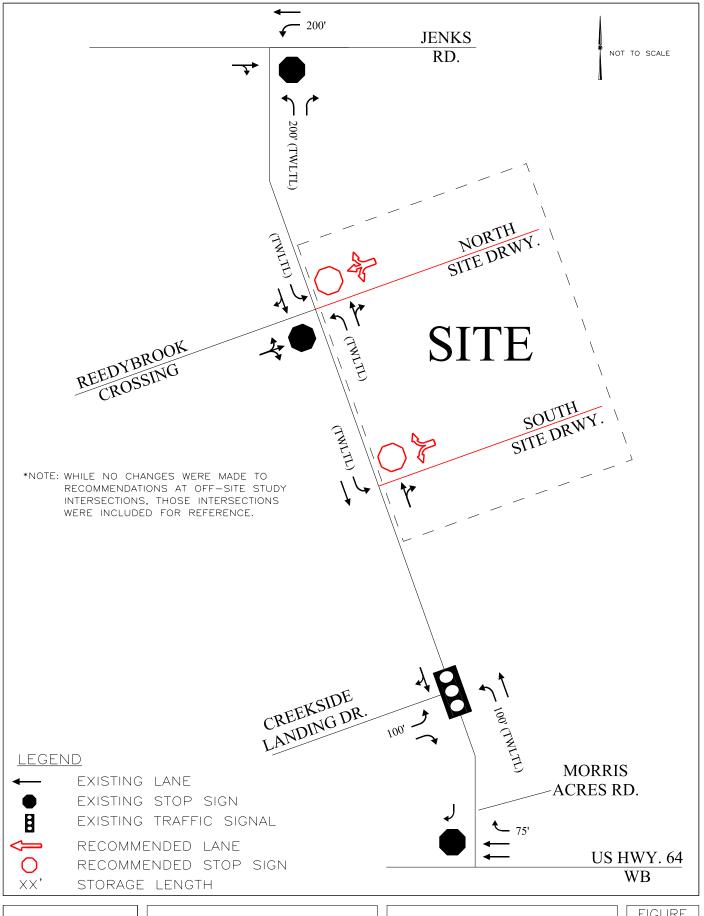
THE WAYFORTH AT APEX
APEX, NC
TRAFFIC CAPACITY ANALYSIS

ADDENDUM ANALYSIS — PROJECTED (2022) BUILD-OUT AM PEAK HOUR TRAFFIC VOLUMES



THE WAYFORTH AT APEX APEX, NC TRAFFIC CAPACITY ANALYSIS

ADDENDUM ANALYSIS -PROJECTED (2022) BUILD-OUT PM PEAK HOUR TRAFFIC VOLUMES



THE WAYFORTH AT APEX APEX, NC TRAFFIC CAPACITY ANALYSIS

ADDENDUM ANALYSIS — BUILD—OUT ROADWAY LANEAGE

INTERSECTION ANALYSIS SHEET

Project: The Wayforth at Apex
Location: Apex, NC
Scenario: Addendum - 2 Site Driveways
Ct. Date
N/S Street: Morris Acres Road
E/W Street: Reedybrook Crossing/North Site Driveway

 AM In
 AM Out
 PM In
 PM Out

 Net New Trips:
 26
 74
 77
 50

Annual Growth Rate: 3.0% Existing Year: 2018
Growth Factor: 0.125509 Buildout Year: 2022

AM PEAK HOUR AM PHF = 0.90

-				/A.	vi r nr – u.	70						
	Re	edybrook Cros	sing	No	rth Site Drivey	vay	M	lorris Acres Ro	ad	M	lorris Acres Ro	ad
		Eastbound			Westbound			Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
2018 Traffic Count	0	0	0	0	0	0	0	0	0	0	0	0
Count Balancing	26	0	26	0	0	0	5	125	0	0	45	5
2018 Existing Traffic	26	0	26	0	0	0	5	125	0	0	45	5
Growth Factor (0.03 per year)	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.126	0.000	0.000	0.126	0.000
2022 Background Growth	0	0	0	0	0	0	0	16	0	0	6	0
Committed Projects												
Beaver Creek Phase 4 Residential	3	0	3	0	0	0	1	0	0	0	0	1
Total Committed Traffic	3	0	3	0	0	0	1	0	0	0	0	1
2022 Background Traffic	29	0	29	0	0	0	6	141	0	0	51	6
Project Traffic												
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	0%	30%	35%	15%	0%
Inbound Project Traffic	0	0	0	0	0	0	0	0	8	9	4	0
Percent Assignment Outbound	0%	0%	0%	25%	0%	40%	0%	20%	0%	0%	0%	0%
Outbound Project Traffic	0	0	0	19	0	29	0	15	0	0	0	0
Total Project Traffic	0	0	0	19	0	29	0	15	8	9	4	0
2022 Buildout Total	29	0	29	19	0	29	6	156	8	9	55	6
Percent Impact (Approach)		0.0%	-		100.0%			13.5%			18.7%	

Overall Percent Impact 24.3%

PM PEAK HOUR PM PHF = 0.90

					WI I III - U.	70						
	Re	edybrook Cros	sing	No	orth Site Drivey	vay	M	Iorris Acres Ro	ad	N	Iorris Acres Ro	oad
		Eastbound			Westbound			Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
2018 Traffic Count	0	0	0	0	0	0	0	0	0	0	0	0
Count Balancing	14	0	14	0	0	0	20	225	0	0	132	24
2018 Existing Traffic	14	0	14	0	0	0	20	225	0	0	132	24
Growth Factor (0.03 per year)	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.126	0.000	0.000	0.126	0.000
2022 Background Growth	0	0	0	0	0	0	0	28	0	0	17	0
Committed Projects												
Beaver Creek Phase 4 Residential	1	0	1	0	0	0	2	0	0	0	0	3
Total Committed Traffic	1	0	1	0	0	0	2	0	0	0	0	3
2022 Background Traffic	15	0	15	0	0	0	22	253	0	0	149	27
Project Traffic												
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	0%	30%	35%	15%	0%
Inbound Project Traffic	0	0	0	0	0	0	0	0	23	27	12	0
Percent Assignment Outbound	0%	0%	0%	25%	0%	40%	0%	20%	0%	0%	0%	0%
Outbound Project Traffic	0	0	0	13	0	20	0	10	0	0	0	0
Total Project Traffic	0	0	0	13	0	20	0	10	23	27	12	0
2022 Buildout Total	15	0	15	13	0	20	22	263	23	27	161	27
Percent Impact (Approach)		0.0%			100.0%			10.7%			18.2%	

Overall Percent Impact 18.0%

INTERSECTION ANALYSIS SHEET

Project: The Wayforth at Apex
Location: Apex, NC
Ct. Date
N/S Street: Morris Acres Road
E/W Street: South Site Driveway

	AM In	AM Out	PM In	PM Out	
Net New Trips:	26	74	77	50	

Annual Growth Rate: 3.0% Existing Year: 2018
Growth Factor: 0.125509 Buildout Year: 2022

AM PEAK HOUR AM PHF = 0.90

_					VI I III — 0.	.,0						
	So	uth Site Drive	way	So	uth Site Drivey	vay	M	lorris Acres Ro	ad	M	lorris Acres Ro	ad
		Eastbound			Westbound			Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
2018 Traffic Count	0	0	0	0	0	0	0	0	0	0	0	0
Count Balancing	0	0	0	0	0	0	0	130	0	0	71	0
2018 Existing Traffic	0	0	0	0	0	0	0	130	0	0	71	0
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022 Background Growth	0	0	0	0	0	0	0	16	0	0	9	0
Committed Projects												
Beaver Creek Phase 4 Residential	0	0	0	0	0	0	0	1	0	0	3	0
Total Committed Traffic	0	0	0	0	0	0	0	1	0	0	3	0
2022 Background Traffic	0	0	0	0	0	0	0	147	0	0	83	0
Project Traffic												
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	30%	20%	15%	0%	0%
Inbound Project Traffic	0	0	0	0	0	0	0	8	5	4	0	0
Percent Assignment Outbound	0%	0%	0%	15%	0%	20%	0%	0%	0%	0%	25%	0%
Outbound Project Traffic	0	0	0	11	0	15	0	0	0	0	19	0
Total Project Traffic	0	0	0	11	0	15	0	8	5	4	19	0
2022 Buildout Total	0	0	0	11	0	15	0	155	5	4	102	0
Percent Impact (Approach)	·	-			100.0%		1	8.1%			21.8%	

Overall Percent Impact 21.3%

PM PEAK HOUR PM PHF = 0.90

				P	M PHF = 0.	.90						
	Sc	outh Site Drive	way	So	uth Site Drive	way	M	orris Acres Ro	ad	M	lorris Acres Re	oad
		Eastbound			Westbound			Northbound			Southbound	
Description	Left	Through	Right	Left	Through	Right	Left	Through	Right	Left	Through	Right
2018 Traffic Count	0	0	0	0	0	0	0	0	0	0	0	0
Count Balancing	0	0	0	0	0	0	0	244	0	0	145	0
2018 Existing Traffic	0	0	0	0	0	0	0	244	0	0	145	0
Growth Factor (0.03 per year)	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126	0.126
2022 Background Growth	0	0	0	0	0	0	0	31	0	0	18	0
Committed Projects												
Beaver Creek Phase 4 Residential	0	0	0	0	0	0	0	2	0	0	1	0
Total Committed Traffic	0	0	0	0	0	0	0	2	0	0	1	0
2022 Background Traffic	0	0	0	0	0	0	0	278	0	0	164	0
Project Traffic												
Percent Assignment Inbound	0%	0%	0%	0%	0%	0%	0%	30%	20%	15%	0%	0%
Inbound Project Traffic	0	0	0	0	0	0	0	23	15	12	0	0
Percent Assignment Outbound	0%	0%	0%	15%	0%	20%	0%	0%	0%	0%	25%	0%
Outbound Project Traffic	0	0	0	7	0	10	0	0	0	0	13	0
Total Project Traffic	0	0	0	7	0	10	0	23	15	12	13	0
2022 Buildout Total	0	0	0	7	0	10	0	301	15	12	177	0
Percent Impact (Approach)		-			100.0%			12.0%			13.2%	

Overall Percent Impact 15.3%

	•	→	•	•	•	•	4	†	_	>	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	f)		7	ĵ»	
Traffic Volume (vph)	29	4	29	19	4	29	6	156	8	9	55	6
Future Volume (vph)	29	4	29	19	4	29	6	156	8	9	55	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	25			25			100			100		
Satd. Flow (prot)	0	1703	0	0	1690	0	1770	1850	0	1770	1835	0
Flt Permitted		0.977			0.982		0.950			0.950		
Satd. Flow (perm)	0	1703	0	0	1690	0	1770	1850	0	1770	1835	0
Link Speed (mph)		25			25			45			45	
Link Distance (ft)		294			267			470			758	
Travel Time (s)		8.0			7.3			7.1			11.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	68	0	0	57	0	7	182	0	10	68	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type: Unsignalized												
Intersection Capacity Utilization	n 20.6%			IC	U Level of	Service A						
Analysis Period (min) 15												

Intersection												
Int Delay, s/veh	3.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	1>		*	ĵ.	
Traffic Vol. veh/h	29	4	29	19	4	29	6	156	8	9	55	6
Future Vol, veh/h	29	4	29	19	4	29	6	156	8	9	55	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	_	_	None	-	-	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage, #	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	_	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mymt Flow	32	4	32	21	4	32	7	173	9	10	61	7
					•							
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	293	280	64	294	279	178	68	0	0	182	0	0
	293 84	280	04	191	191	1/8	50	-	-	182	-	-
Stage 1 Stage 2	209	196	-	103	191	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
	6.12	5.52	0.22	6.12	5.52	0.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12						-	-	-	-	-	-
Critical Hdwy Stg 2 Follow-up Hdwy	3.518	5.52 4.018	3.318	6.12 3.518	5.52 4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	659	628	1000	658	629	865	1533	-	-	1393		-
Stage 1	924	825	1000	811	742	- 005	1000	-	-	1090	-	-
Stage 1 Stage 2	793	739	-	903	822	-	-	-	-	-	-	-
Platoon blocked, %	193	139	-	903	022	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	625	621	1000	628	622	865	1533	-	-	1393	-	-
Mov Cap-1 Maneuver	625	621	1000	628	622	000	1333	-	-	1030	-	-
Stage 1	920	819	-	807	739	-	-	-	-	-	-	-
Stage 1 Stage 2	755	736	-	863	816	-	-	-	-	-	-	-
Slaye 2	755	730	-	003	010	-	-	-	-	-	-	-
				14/5								
Approach	EB			WB			NB			SB		
HCM Control Delay, s	10.2			10.3			0.3			1		
HCM LOS	В			В								
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1		SBL	SBT	SBR			
Capacity (veh/h)		1533	-	-	758	741	1393	-	-			
HCM Lane V/C Ratio		0.004	-	-	0.091	0.078	0.007	-	-			
HCM Control Delay (s)		7.4	-	-	10.2	10.3	7.6	-	-			
HCM Lane LOS		Α	-	-	В	В	Α	-	-			
HCM 95th % tile Q(veh)		0	-	-	0.3	0.3	0	-	-			

	•	•	†	1	>	Ţ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	•	1>		7	†
Traffic Volume (vph)	11	15	155	5	4	102
Future Volume (vph)	11	15	155	5	4	102
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	25				100	
Satd. Flow (prot)	1681	0	1853	0	1770	1863
Flt Permitted	0.980				0.950	
Satd. Flow (perm)	1681	0	1853	0	1770	1863
Link Speed (mph)	25		45			45
Link Distance (ft)	369		1004			470
Travel Time (s)	10.1		15.2			7.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)						
Lane Group Flow (vph)	29	0	178	0	4	113
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilizati	on 18.5%			IC	U Level of	Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WDL	WDIX	1ND1	NDIX	SDL 1	<u>361</u>
Traffic Vol. veh/h	'T' 11	15	155	5	1	102
Future Vol, veh/h	11	15	155	5	4	102
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None		None	riee -	None
	0	None -	-	None -	100	None -
Storage Length	-				100	
Veh in Median Storage, #		-	0	-		0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	12	17	172	6	4	113
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	297	175	0	0	178	0
Stage 1	175	1/3	-	-	1/0	-
Stage 2	122	-			-	
	6.42	6.22	-	-		-
Critical Hdwy			-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	694	868	-	-	1398	-
Stage 1	855	-	-	-	-	-
Stage 2	903	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	692	868	-	-	1398	-
Mov Cap-2 Maneuver	715	-	-	-	-	-
Stage 1	855	-	-	-	-	-
Stage 2	900	-	-	-	-	-
Ŭ						
A	MA		ND		0.0	
Approach	WB		NB		SB	
HCM Control Delay, s	9.7		0		0.3	
HCM LOS	Α					
Minor Lane/Major Mvmt		NBT	NRP	WBLn1	SBL	SBT
		INDI	NDK		1398	<u> </u>
Capacity (veh/h)				796		
HCM Lane V/C Ratio		-	-		0.003	-
HCM Control Delay (s)		-	-	9.7	7.6	-
HCM Lane LOS		-	-	A	A	-
HCM 95th % tile Q(veh)		-	-	0.1	0	-

	•	→	*	•	—	•	•	†	~	\	+	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		7	f)		, J	ĵ»	
Traffic Volume (vph)	15	4	15	13	4	20	22	263	23	27	161	27
Future Volume (vph)	15	4	15	13	4	20	22	263	23	27	161	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	100		0	100		0
Storage Lanes	0		0	0		0	1		0	1		0
Taper Length (ft)	25			25			100			100		
Satd. Flow (prot)	0	1712	0	0	1696	0	1770	1840	0	1770	1822	0
Flt Permitted		0.978			0.983		0.950			0.950		
Satd. Flow (perm)	0	1712	0	0	1696	0	1770	1840	0	1770	1822	0
Link Speed (mph)		25			25			45			45	
Link Distance (ft)		294			267			470			758	
Travel Time (s)		8.0			7.3			7.1			11.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	38	0	0	40	0	24	318	0	30	209	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type: Unsignalized												
Intersection Capacity Utilization	on 31.9%			IC	U Level of	Service A						
Analysis Period (min) 15												

Intersection												
Int Delay, s/veh	2.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		*	1		*	1>	
Traffic Vol. veh/h	15	4	15	13	4	20	22	263	23	27	161	27
Future Vol, veh/h	15	4	15	13	4	20	22	263	23	27	161	27
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	_	None
Storage Length	-	-	-	-	-	-	100	-	-	100	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	17	4	17	14	4	22	24	292	26	30	179	30
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	621	621	194	618	623	305	209	0	0	318	0	0
Stage 1	254	254	194	354	354	303	209	-	-	310	-	-
Stage 1	367	367	-	264	269	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	<u>-</u>	-
Critical Hdwy Stg 1	6.12	5.52	0.22	6.12	5.52	0.22	4.12	-		4.12	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	_					
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218			2.218	-	
Pot Cap-1 Maneuver	400	403	847	402	402	735	1362			1242	<u>-</u>	_
Stage 1	750	697	-	663	630	-	1002	_	_	1272	_	_
Stage 2	653	622	_	741	687	_	_	_	_	_	_	_
Platoon blocked, %	- 000	ULL		, , , ,	001			_			_	_
Mov Cap-1 Maneuver	372	386	847	378	385	735	1362	_	-	1242	-	_
Mov Cap-2 Maneuver	372	386	-	378	385	-	-	-		-	-	_
Stage 1	737	680	-	651	619	-	-	_	-	-	-	_
Stage 2	618	611	_	704	670	_	_	-		_	_	_
J J. L	0.0	3 11			3.0							
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.8			12.6			0.5			1		
HCM LOS	12.0 B			12.0 B			0.0			-		
TOM EGO				U								
Minor Lane/Major Mvmt		NBL	NBT	NRD	EBLn1	WRI n1	SBL	SBT	SBR			
Capacity (veh/h)		1362	IND I	NDK	497	514	1242	- 301	JDK -			
HCM Lane V/C Ratio		0.018	-	-	0.076	0.08	0.024	-	-			
HCM Control Delay (s)		7.7	-	-	12.8	12.6	0.024	-	-			
HCM Lane LOS		7.7 A	-	-	12.8 B	12.0 B	o A	-	-			
HCM 95th % tile Q(veh)		0.1	-	-	0.2	0.3	0.1	-	-			
HOW SOUT /6 tile Q(VeII)		0.1	-	-	0.2	0.3	0.1	-	-			

	•	•	†	~	\	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		ĵ _e		7	†
Traffic Volume (vph)	7	10	301	15	12	177
Future Volume (vph)	7	10	301	15	12	177
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		0	100	
Storage Lanes	1	0		0	1	
Taper Length (ft)	25				100	
Satd. Flow (prot)	1681	0	1850	0	1770	1863
Flt Permitted	0.979				0.950	
Satd. Flow (perm)	1681	0	1850	0	1770	1863
Link Speed (mph)	25		45			45
Link Distance (ft)	369		1004			470
Travel Time (s)	10.1		15.2			7.1
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Shared Lane Traffic (%)						
Lane Group Flow (vph)	19	0	351	0	13	197
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalized						
Intersection Capacity Utilization	on 26.8%			IC	U Level of	Service A
Analysis Period (min) 15						

Intersection						
Int Delay, s/veh	0.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	11011	1	, ,DIT	ሻ	<u> </u>
Traffic Vol, veh/h	7	10	301	15	12	177
Future Vol, veh/h	7	10	301	15	12	177
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized		None	Free -	None	Free -	None
	- 0					
Storage Length		-	-	-	100	-
Veh in Median Storage, #		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	8	11	334	17	13	197
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	566	343	0	0	351	0
Stage 1	343	J4J -	-	-	331	-
Stage 2	223	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	0.22	-	-	4.12	-
			-	-		-
Critical Hdwy Stg 2	5.42	-	-	-	- 0.040	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	486	700	-	-	1208	-
Stage 1	719	-	-	-	-	-
Stage 2	814	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	481	700	-	-	1208	-
Mov Cap-2 Maneuver	564	-	-	-	-	-
Stage 1	719	-	-	-	-	-
Stage 2	805	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	10.8		0		0.5	
HCM LOS	10.0 B		U		0.5	
HCM LOS	В					
Minor Lane/Major Mvmt		NBT	NBR \	WBLn1	SBL	SBT
Capacity (veh/h)		-	-	637	1208	-
HCM Lane V/C Ratio		-	-	0.03	0.011	-
HCM Control Delay (s)		-	-	10.8	8	-
HCM Lane LOS		-	-	В	A	-
HCM 95th % tile Q(veh)		_	-	0.1	0	-
2(1311)						

Rezoning #19CZ02 Morris Acres PUD

November 12, 2019 Planning Board Meeting



Report Requirements:

Per NCGS 160A-387, all proposed amendments to the zoning ordinance or zoning map shall have a written report provided from the Planning Board to the Town Council within 30 days of referral of the amendment to the Planning Board, or the Town Council may proceed in its consideration of the amendment without the Planning Board report. Furthermore, in no case is the Town Council bound by the recommendations, if any, of the Planning Board.

Per NCGS 160A-383, the Planning Board shall advise and comment on whether the proposed zoning amendment is consistent with all applicable officially adopted plans, and provide a written recommendation to the Town Council that addresses plan consistency and other matters as deemed appropriate by the Planning Board, but a comment by the Planning Board that a proposed amendment is inconsistent with the officially adopted plans shall not preclude consideration or approval of the proposed amendment by the Town Council.

PR	OJECT DESCRIPTION:									
Acr	reage:	± 17.4	376 acres							
PIN	ls:	07322	89587, 0732382	530, 0732382709						
Cui	rrent Zoning:	Rural	Residential (RR)							
Pro	pposed Zoning:	Planne	ed Unit Developn	nent-Conditional Zoning (PUD-CZ)						
204	15 Land Use Map:	Mediu	ım Density Reside	ential						
Tov	wn Limits:	0732382709 is in the ETJ; PINs 0732289587 & 0732382530 are in Town limits								
App	licable Officially Adopted F	Plans:								
	Board must state whether the plicable. Applicable plans hav			nconsistent with the following officially adopted plans, nem.						
M	2045 Land Use Map									
,	Consistent		Inconsistent	Reason:						
	/ Consistent	_								
Ħ	Apex Transportation Plan									
	Consistent		Inconsistent	Reason:						
	- consistent	_	meonsistem							
	•									
M	Parks, Recreation, Open S	pace, a	nd Greenways F	Plan						
	Consistent		Inconsistent	Reason:						
				2 5						
				<u>'</u>						

Rezoning #19CZ02 Morris Acres PUD

November 12, 2019 Planning Board Meeting



Legislative Considerations:

The applicant shall propose site-specific standards and conditions that take into account the following considerations, which are considerations that are relevant to the legislative determination of whether or not the proposed conditional zoning district rezoning request is in the public interest. These considerations do not exclude the legislative consideration of any other factor that is relevant to the public interest.

1.		y with the purposes, go	onal Zoning (CZ) District use's appropriateness for als, objectives, and policies of the 2045 Land Use Reason:
		,	*
2.	and compatibility with the characte		t use's appropriateness for its proposed location ses. Reason:
3.	Zoning district supplemental stand with Sec. 4.4 Supplemental Standar ☑ Consistent □		nditional Zoning (CZ) District use's compliance Reason:
4.	minimization of adverse effects, i	ncluding visual impact npacts on surrounding	roposed Conditional Zoning (CZ) District use's of the proposed use on adjacent lands; and lands regarding trash, traffic, service delivery, not create a nuisance. Reason:
			· · · · · · · · · · · · · · · · · · ·
5.		ction from significant	Conditional Zoning District use's minimization of deterioration of water and air resources, wildlife Reason:

Rezoning #19CZ02 Morris Acres PUD

November 12, 2019 Planning Board Meeting



6.		nd se	ervices, including roads	ing (CZ) District use's avoidance of having adverse s, potable water and wastewater facilities, parks, Reason:
	Consistent		meonsistem	Reason:
7.	welfare of the residents of the	Tow	n or its ETJ.	ing (CZ) District use's effect on the health, safety, or
			Inconsistent	Reason:
8.	Detrimental to adjacent proper detrimental to adjacent proper Consistent			Conditional Zoning (CZ) District use is substantially
9.		ic in	npact or noise, or becau	d Conditional Zoning (CZ) District use constitutes a use of the number of persons who will be using the Reason:
10.		oose	d on it by all other app	he proposed Conditional Zoning (CZ) District use licable provisions of this Ordinance for use, layout, Reason:
		H		

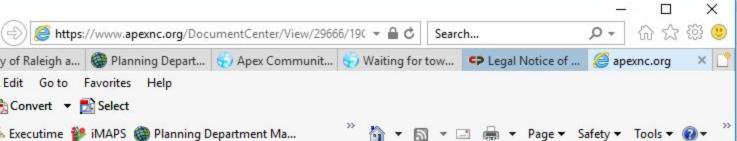
Rezoning #19CZ02 Morris Acres PUD

November 12, 2019 Planning Board Meeting



Planning Board Recommendation:

	Motion: To vecommend approval as proposed. Introduced by Planning Board member: Seconded by Planning Board member: Beth Godfrey
	Approval: the project is consistent with all applicable officially adopted plans and the applicable legislative considerations listed above.
Þ	Approval with conditions: the project is not consistent with all applicable officially adopted plans and/or the applicable legislative considerations as noted above, so the following conditions are recommended to be included in the project in order to make it fully consistent: As proposed by applicant.
	Denial: the project is not consistent with all applicable officially adopted plans.
	With <u></u> Planning Board Member(s) voting "aye"
	With O Planning Board Member(s) voting "no"
	Reasons for dissenting votes:
This	report reflects the recommendation of the Planning Board, this the 12 th day of November 2019.
Atte	st: Ago Bills, Planning Board Chair Dianne Khin, Planning Director





TOWN OF APEX

POST OFFICE BOX 250 APEX, NORTH CAROLINA 27502 PHONE 919-249-3426

PUBLIC NOTIFICATION OF PUBLIC HEARINGS

CONDITIONAL ZONING #19CZ02 0, 7208, 7208-B Morris Acres Road Morris Acres PUD

Pursuant to the provisions of North Carolina General Statutes Section 160A-364 and to the Town of Apex Unified Development Ordinance (UDO) Section 2.2.11, notice is hereby given of public hearings before the Planning Board and Town Council of the Town of Apex. The purpose of these hearings is to consider the following:

Applicant: Kaplan Residential

Authorized Agent: Jason Barron, Morningstar Law Group Property Addresses: 0, 7208, & 7208B Morris Acres Road

Acreage: ± 17.4376

Property Identification Numbers (PINs): 0732289587, 0732382530, & 0732382709

Existing 2045 Land Use Map Designation: Medium Density Residential

Existing Zoning of Property: Rural Residential (RR)

Proposed Zoning of Property: Planned Unit Development-Conditional Zoning (PUD-CZ)

Public Hearing Location: Apex Town Hall

73 Hunter Street, Apex, North Carolina

Council Chambers, 2nd Floor

Planning Board Public Hearing Date and Time: November 12, 2019 4:30 P.M. Town Council Public Hearing Date and Time: November 19, 2019 7:00 P.M.

Vicinity Map:



Property owners within 300 feet of the proposed conditional zoning have been sent this notice via first class mail. All interested parties may appear at the public hearing and be heard with respect to the application. Maps showing the location for the above site(s) to be considered in addition to a copy of the 2045 Land Use Map can be inspected at the Apex Town Hall or call 919-249-3426, Department of Planning and Community Development for further information. To view the petition and related documents on-line: https://www.apexnc.org/DocumentCenter/View/26285.

Dianne F. Khin, AICP Planning Director

Published Dates: November 1 - November 19, 2019



TOWN OF APEX

POST OFFICE BOX 250 APEX, NORTH CAROLINA 27502 PHONE 919-249-3426

PUBLIC NOTIFICATION OF PUBLIC HEARINGS

CONDITIONAL ZONING #19CZ02 0, 7208, 7208-B Morris Acres Road Morris Acres PUD

Pursuant to the provisions of North Carolina General Statutes Section 160A-364 and to the Town of Apex Unified Development Ordinance (UDO) Section 2.2.11, notice is hereby given of public hearings before the Planning Board and Town Council of the Town of Apex. The purpose of these hearings is to consider the following:

Applicant: Kaplan Residential

Authorized Agent: Jason Barron, Morningstar Law Group **Property Addresses:** 0, 7208, & 7208B Morris Acres Road

Acreage: ± 17.4376

Property Identification Numbers (PINs): 0732289587, 0732382530, & 0732382709

Existing 2045 Land Use Map Designation: Medium Density Residential

Existing Zoning of Property: Rural Residential (RR)

Proposed Zoning of Property: Planned Unit Development-Conditional Zoning (PUD-CZ)

Public Hearing Location: Apex Town Hall

73 Hunter Street, Apex, North Carolina

Council Chambers, 2nd Floor

Planning Board Public Hearing Date and Time: November 12, 2019 4:30 P.M. Town Council Public Hearing Date and Time: November 19, 2019 7:00 P.M.

Vicinity Map:



Property owners within 300 feet of the proposed conditional zoning have been sent this notice via first class mail. All interested parties may appear at the public hearing and be heard with respect to the application. Maps showing the location for the above site(s) to be considered in addition to a copy of the 2045 Land Use Map can be inspected at the Apex Town Hall or call 919-249-3426, Department of Planning and Community Development for further information. To view the petition and related documents on-line: https://www.apexnc.org/DocumentCenter/View/26285.





TOWN OF APEX

POST OFFICE BOX 250 APEX, NORTH CAROLINA 27502 PHONE 919-249-3426

AFFIDAVIT CERTIFYING Public Notification – Written (Mailed) Notice

Section 2.2.11

Town of Apex Unified Development Ordinance

Project Name:

Rezoning 19CZ02

Project Location:

0, 7208, & 7208B Morris Acres Road

Applicant or Authorized Agent:

Jason Barron

Firm:

Morningstar Law Group

This is to certify that I, as Planning Director, mailed or caused to have mailed by first class postage for the above mentioned project **November 1, 2019**, a notice containing the time and place, location, nature and scope of the application, where additional information may be obtained, and the opportunity for interested parties to be heard, to the property owners within 300' of the land subject to notification. I further certify that I relied on information provided to me by the above-mentioned person as to accuracy and mailing addresses of property owners within 300' of the land subject to notification.

Date

Planning Director

STATE OF NORTH CAROLINA COUNTY OF WAKE

Sworn and subscribed before me, <u>feri Chastain Padessn</u>, a Notary Public for the above State and County, this the <u>0/</u> day of <u>November</u>, 201 <u>9</u>.

JERI CHASTAIN PEDERSON
Notary Public
Wake County, North Carolina
My Commission Expires
March 10, 2024

Jew Chastain Federson Notary Public

My Commission Expires: 03 10 12024