Rezoning #20CZ14 Hackney PUD

April 27, 2021 Town Council Meeting



All property owners and neighborhood associations within 300 feet of this rezoning have been notified per UDO Sec. 2.2.11 *Public Notification*.

BACKGROUND INFORMATION:

Location: 0, 2500, & 2600 Olive Chapel Road **Applicant/Agent:** Brendie Vega, WithersRavenel **Owners:** Charles & Judy Hackney and Edwin Goodwin

PROJECT DESCRIPTION:

Acreage: ±73.64 acres

PINs: 0721492629, 0722406699, & 0722411102 **Current Zoning**: Rural Residential (RR) & R-80W

Proposed Zoning: Planned Unit Development-Conditional Zoning (PUD-CZ)

2045 Land Use Map Designation: Medium Density Residential

Town Limits: ETJ and Outside (annexation of portion in Wake County is required with rezoning)

Adjacent Zoning & Land Uses:

	Zoning	Land Use	
North:	Planned Unit Development-Conditional Zoning(PUD-CZ #17CZ21& #16CZ26)	Single Family Residential (Sweetwater and Linden subdivisions)	
Rural Residential (RR); Medium Density Residential-Conditional Zoning (MD-CZ #13CZ26 & #13CZ08); Wake Co. R-80W Rural Residential (RR); Planned Unit Development-Conditional Zoning (PUD-CZ		Olive Chapel Road; Single Family Residential (Riley's Pond and large lot single-family) Single Family Residential (Linden	
	#16CZ26)	subdivision and large lot single-family)	
West:	Planned Unit Development-Conditional Zoning (PUD-CZ #15CZ32) & Medium Density Residential-Conditional Zoning (MD-CZ #16CZ10)	Single Family Residential (Smith Farm and Haley Farm subdivisions)	

EXISTING CONDITIONS:

The site consist of three (3) parcels on the north side of Olive Chapel Road totaling ±73.64 acres. The site has one single family residence, several sheds/barns, and two existing ponds. Along the north boundary there is a 100' stream buffer from Reedy Branch.

NEIGHBORHOOD MEETING:

The applicant conducted a neighborhood meeting on October 29, 2020. The neighborhood meeting report is attached.

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WCPSS Coordination:

A Letter of Impact from Wake County Public School System (WCPSS) was received for this rezoning and is included in the staff report packet. WCPSS indicates that elementary and high schools within the current assignment area for this rezoning/development are anticipated to have insufficient capacity for future students; transportation to schools outside of the current assignment area should be anticipated. School expansion or construction within the next five years may address concerns at the high school level. Possible long-term solutions may include capping students out to schools with available seats (not very proximate), reassignments, or calendar changes.

2045 LAND USE MAP:

The 2045 Land Use Map designates the properties as Medium Density Residential. The proposed PUD is consistent with the Land Use Map designation.

PLANNED UNIT DEVELOPMENT PLAN:

The applicant is proposing a Planned Unit Development Plan with uses and development standards as follows:

Permitted Uses:

The Rezoned Lands may be used for, and only for, the uses listed immediately below. The permitted uses are subject to the limitations and regulations stated in the UDO and any additional limitations or regulations stated below. For convenience, some relevant sections of the UDO may be referenced; such references do not imply that other sections of the UDO do not apply.

Residential:

- Single Family
- Accessory Dwelling Unit
- Townhouse

Non-Residential:

- Utility, minor
- Greenway
- Park, active
- Park, passive

Proposed Design Controls:

Maximum Density: 3.5 units per acre*

*The residential density will be a maximum of 3.5 dwelling units per acre this is a change from 4 units per acre.

Maximum Building Height: 50 feet

Maximum Built-Upon Area: 70%



Setbacks

		Proposed PUD-CZ	MD zoning district
		minimum setbacks	minimum setbacks
		5' from façade	
	Front	20' from garage to	25'
Single family		back of sidewalk	
Single-family	Side	5'	6' min/16' total
	Rear	10'	20'
	Corner side	8'	15'
			HDSF zoning district
			minimum setbacks
		10' from façade	
	Front	20' from garage to back	15'
Townhouse,		of sidewalk	
front loaded	Side	5'	0' (8' between buildings)
	Rear	10'	15'
	Corner side	10'	15'
			HDSF zoning district
			minimum setbacks
	Front	10' from front facade	15'
Townhouse,	Side	5'	0' (8' between buildings)
alley loaded	Rear	5'	15'
	Corner side	10'	15'

Buffers

The proposed PUD meets or exceeds the buffers required by the UDO.

Perimeter Buffers:	UDO Required	Proposed
Northern property boundary	15' Type A	200' stream
		buffer**
Eastern property boundary	20' Type B	20' Type A
Western property boundary	15' Type A	20' Type A
Olive Chapel Road	30' Type B	30' Type E*

^{*}A 30' Type B buffer shall be provided if homes along Olive Chapel Road are not alley-loaded.

Architectural Standards

The proposed development offers the following architectural controls to ensure a consistency of character throughout the development, while allowing for enough variety to create interest and avoid monotony. Changes to the exterior materials, roof, windows, doors, process, trim, etc. are

^{**} In addition to the 100' riparian buffer to the north, an additional 100' buffer will be established. This additional 100' may include utilities, trails and other active or passive recreation.

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allowable with administrative approval at the staff level. Further details shall be provided at the time of Residential Master Subdivision Plan submittal. The following conditions shall apply:

- 1. Vinyl siding is not permitted; however, vinyl windows, decorative elements and trim are permitted.
- 2. The roofline cannot be a single mass; it must be broken up horizontally and vertically between every unit.
- 3. Garage doors must have windows, decorative details or carriage-style adornments on them.
- 4. The rear and side elevations of the units that can be seen from the right-of-way shall have trim around the windows.
- 5. The visible side of a townhome on a corner lot facing the public street shall contain at least 2 decorative elements such as, but not limited to, the following elements:
 - Windows
 - Bay window
 - Recessed window
 - Decorative window
 - Trim around the windows
 - Wrap-around porch or side porch
 - Two or more building materials
 - Column
 - Portico

- Balcony
- Dormer
- Decorative brick/stone
- Decorative trim
- Decorative shake
- Decorative air vents on gable
- Decorative gable
- Decorative cornice

6. The garage cannot protrude more than 1-foot from either the front façade or porch.

Resource Conservation Area

The Hackney PUD is south and west of NC 540 and is therefore required to provide 30% of the gross site as RCA. If the single-family portion of the PUD is mass graded, that portion of the project will be required to provide an additional 5% RCA. They propose to meet these requirements.

Tree Replanting

Existing deciduous trees greater than 18" in diameter (DBH), as identified in the tree survey, that are removed by site development shall be replaced by planting a 1.5" caliper native tree from the *Town of Apex Design and Development Manual* as a street tree or as other required landscaping. Excess required tree replacement will occur in common open space areas.

Clean Energy

Residential dwelling units will be provided with solar conduit to accommodate the future installation of solar panels.

Water Quality

- 1. Signs will be installed near SCMs in order to:
 - Reduce pet water near SCM drainage areas
 - Reduce fertilizer near SCM drainage areas
- 2. Installation of Pet Waste Stations in common areas will occur within the neighborhood

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Planting and Landscaping

- 1. Install Warm Season grasses (Bermuda, Zoysia, etc) in lawn areas to reduce the need for irrigation and chemicals.
- 2. Install required Street Trees, Buffer, and Re-Vegetation plantings that consist of a variety of native plant materials recognized by the New Hope Audubon Society or the NCSU manual for Landscaping for Wildlife with Native Plants as being bird and pollinator friendly; as allowed by the Town of Apex Deign & Development Manual or approved by Apex staff.
- 3. Specify pocket park plantings that are recognized by the NC Wildlife Federation as being Native Pollinator Plants as part of the Statewide Butterfly Highway initiative.
- 4. Include at least 4 hardwood tree varieties in the proposed plantings, as allowed by the Apex Design and Development Manual.

Environmental Resources

Parking

Parking and loading will comply with all applicable requirements of UDO Sec. 8.3 *Parking and Loading*. Per UDO Section 8.3.4 of the UDO, guest parking shall be designated within common areas and be distributed throughout the project. Striped on-street parking may be counted toward guest parking requirements. For townhouses, guest parking shall be distributed so that there is at least one parking space within 200' of each townhouse lot.

Public Facilities

The project's construction will consist of the extension of public facilities to serve the site. All public facilities and infrastructure shall comply with the Town of Apex Sewer and Water Master Plans and the Town of Apex Standards and Specifications. Public facilities include:

Water/Sanitary Sewer:

All lines will be designed according to Town of Apex Standards and Specifications.

Other Utilities:

Electric service shall be provided by the Town of Apex. Gas, telephone, and cable shall be provided by the builder as coordinated with the appropriate utility companies.

Stormwater Management

Two ponds exist on the parcels and drain to Reedy Branch Creek, eventually feeding into Jordan Lake. The proposed development plan will require stormwater management measures in accordance with Sections 6.1 and 7.5.7 in the Town of Apex Unified Development Ordinance. Stormwater captured on the site will be conveyed to proposed Stormwater Control Measures, which will be identified on plans during the major subdivision or site plan approval stage. Post-development peak runoff shall not exceed pre-development peak runoff for the 24-hour, 1-year and 10-year storm events in accordance with the Unified Development Ordinance. Treatment for the first 1-inch of runoff will be provided such that the removal of 85% Total Suspended Solids is achieved. All stormwater devices will meet the design requirements of NCDENR and the Town of Apex.

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APEX TRANSPORTATION PLAN/ACCESS and CIRCULATION:

The Site will require an internal public roadway network and parking spaces. The onsite transportation circulation system shall be consistent with the Town of Apex Transportation Plan and the Town of Apex Standard Specifications and Standard Details. The following conditions shall apply:

- 1. Hasse Avenue will be constructed between Olive Chapel Road and its current terminus north of the project.
- Olive Chapel Road will be widened to include construction of a 100-foot eastbound left-turn lane
 with appropriate deceleration length and taper and a 100-foot westbound right-turn lane with
 appropriate deceleration length and taper on Olive Chapel Road, subject to NCDOT review and
 approval.
- The Olive Chapel Road turn lane widening will be completed prior to platting Hasse Avenue access to Olive Chapel Road and the connection to Hasse Avenue north of the project will be completed prior to the last plat in the subdivision.
- 4. A 6-foot bike lane and 5-foot paved shoulder will be located on the north side of Olive Chapel Road per the Bicycle and Pedestrian System Plan Map.
- Alleys may be proposed to vary from Town standards in order to accommodate water and sewer utilities, provided they maintain the same or greater width of pavement and right of way, subject to staff review and approval at the time of subdivision and construction plans.
- 6. There will be no private driveways permitted along Olive Chapel Road.

Pedestrian Facilities

- 1. The development plan will incorporate sidewalk infrastructure along Olive Chapel Road as well as the internal street network.
- 2. A trail will serve as a connection from the western portion of the community to the Reedy Branch Greenway.
- 3. Sidewalks will be provided on both sides of all streets for single-family detached homes.
- 4. There will be a 10-foot side path provided along minor collector road as shown on the Bicycle and Pedestrian Systems Plan Map.
- 5. Prior platting the 75th lot in the neighborhood, the Developer will extend a 5' sidewalk approximately 860 feet along the north side of Olive Chapel to western limits of the Linden Subdivision. Developer will attempt to obtain the required right-of-way and/or easements for construction of this sidewalk from the adjacent property owners. If the required right-of-way and/or easements cannot be obtained by that time, a Fee-in-Lieu in the amount of 125% of the estimated cost of construction plus fair market value of the property to be acquired, shall be assessed. Any performance guarantee provided for this section of sidewalk shall be released upon acceptance of said fee-in-lieu by the Town.

Affordable Housing

If the Town of Apex has a fund or other mechanism in place to receive donations to construct, subsidize, or participate in the development of affordable housing units (the "Fund"), the developer will contribute \$215 per lot to this Fund prior to the first residential Certificate of Occupancy. In the event the Fund has not been established by the Town of Apex, the money will be conveyed to a local non-profit working on affordable housing initiatives. The developer will work with the Town of Apex to identify a mutually acceptable local non-profit organization to receive these funds.

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ENVIRONMENTAL ADVISORY BOARD:

This rezoning was submitted before the Environmental Advisory Board began holding pre-application meetings on rezonings.

PARKS, RECREATION, AND CULTURAL RESOURCES ADVISORY COMMISSION:

The Parks, Recreation, and Cultural Resources Advisory Commission reviewed this item at their December 9, 2020 meeting and unanimously recommended a fee-in-lieu of dedication with credit for construction of greenway which connects side path along Hasse Ave to the west connecting to the Reedy Branch Greenway in Smith Farm. The fee rate will be set at the time of Town Council approval and the credit of construction will be calculated prior to construction plan approval. Per UDO Article 14, the greenway must be completed and accepted prior to 25% of the building permits for the project being issued.

PLANNING BOARD RECOMMENDATION:

The Planning Board held a Public Hearing on March 8, 2021 and voted to recommend approval, with the conditions as offered by the applicant, by a vote of 6-0.

PLANNING STAFF RECOMMENDATION:

Planning staff recommends approval of rezoning #20CZ14 Hackney PUD with the conditions as proposed by the applicant.

ANALYSIS STATEMENT OF THE REASONABLENESS OF THE PROPOSED REZONING:

This Statement will address consistency with the Town's comprehensive and other applicable plans, reasonableness, and effect on public interest:

The 2045 Land Use Map designates the site as Medium Density Residential. The proposed PUD is consistent with that land use classification.

Approval of the rezoning is reasonable and in the public interest because the site will act a transition between higher and lower residential densities. The proposed rezoning also provides for increased stream buffers, higher planting standards, and a contribution to affordable housing.

The proposed rezoning is also reasonable and in the public interest because it will allow this property to develop in a way that is consistent with the surrounding areas and will build side path along the minor collector that will be constructed through the site to Olive Chapel Road.

PLANNED UNIT DEVELOPMENT DISTRICT AND CONDITIONAL ZONING STANDARDS: Standards

In return for greater flexibility in site design requirements, Planned Development (PD) Districts are expected to deliver exceptional quality community designs that preserve critical environmental resources; provide high quality community amenities; incorporate creative design in the layout of buildings, Resource Conservation Area and circulation; ensure compatibility with surrounding land uses and neighborhood character; provide high quality architecture; and provide greater efficiency in the layout and provision of roads, utilities, and other infrastructure. The Planned Development (PD) Districts shall not be used as a means of circumventing the Town's adopted land development regulations for routine developments.

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- 1) Planned Unit Development (PUD-CZ) District In approving a Planned Development (PD) Zoning District designation for a PUD-CZ, the Town Council shall find the PUD-CZ district designation and PD Plan for PUD-CZ demonstrates compliance with the following standards:
 - a) Development parameters
 - (i) The uses proposed to be developed in the PD Plan for PUD-CZ are those uses permitted in Sec. 4.2.2 *Use Table*.
 - (ii) The uses proposed in the PD Plan for PUD-CZ can be entirely residential, entirely non-residential, or a mix of residential and non-residential uses, provided a minimum percentage of non-residential land area is included in certain mixed use areas as specified on the 2030 Land Use Map. The location of uses proposed by the PUD-CZ must be shown in the PD Plan with a maximum density for each type of residential use and a maximum square footage for each type of non-residential use.
 - (iii) The dimensional standards in Sec. 5.1.3 *Table of Intensity and Dimensional Standards, Planned Development Districts* may be varied in the PD Plan for PUD-CZ. The PUD-CZ shall demonstrate compliance with all other dimensional standards of the UDO, North Carolina Building Code, and North Carolina Fire Code.
 - (iv) The development proposed in the PD Plan for PUD-CZ encourages cluster and compact development to the greatest extent possible that is interrelated and linked by pedestrian ways, bikeways and other transportation systems. At a minimum, the PD Plan must show sidewalk improvements as required by the Apex Transportation Plan and the *Town of Apex Standard Specifications and Standard Details*, and greenway improvements as required by the Town of Apex Parks, Recreation, Greenways, and Open Space Plan and the Apex Transportation Plan. In addition, sidewalks shall be provided on both sides of all streets for single-family detached homes.
 - v) The design of development in the PD Plan for PUD-CZ results in land use patterns that promote and expand opportunities for walkability, connectivity, public transportation, and an efficient compact network of streets. Cul-de-sacs shall be avoided unless the design of the subdivision and the existing or proposed street system in the surrounding area indicate that a through street is not essential in the location of the proposed cul-de-sac, or where sensitive environmental areas such as streams, floodplains, and wetlands would be substantially disturbed by making road connections.
 - (vi) The development proposed in the PD Plan for PUD-CZ is compatible with the character of surrounding land uses and maintains and enhances the value of surrounding properties.
 - (vii) The development proposed in the PD Plan for PUD-CZ has architectural and design standards that are exceptional and provide higher quality than routine developments. All residential uses proposed in a PD Plan for PUD-CZ shall provide architectural elevations representative of the residential structures to be built to ensure the Standards of this Section are met.

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- b) Off-street parking and loading. The PD Plan for PUD-CZ shall demonstrate compliance with the standards of Sec. 8.3 Off-Street Parking and Loading, except that variations from these standards may be permitted if a comprehensive parking and loading plan for the PUD-CZ is submitted as part of the PD Plan that is determined to be suitable for the PUD-CZ, and generally consistent with the intent and purpose of the off-street parking and loading standards.
- c) RCA. The PD Plan for PUD-CZ shall demonstrate compliance with Sec. 8.1.2 Resource Conservation Area, except that the percentage of RCA required under Sec. 8.1.2 may be reduced by the Town Council by no more than ten percent (10%) provided that the PD Plan for PUD-CZ includes one or more of the following:
 - (i) A non-residential component; or
 - (ii) An overall density of 7 residential units per acre or more; or
 - (iii) Environmental measures including but not limited to the following:
 - (a) The installation of a solar photovoltaic (PV) system on a certain number or percentage of single-family or townhouse lots or on a certain number or percentage of multifamily, mixed-use, or nonresidential buildings. All required solar installation shall be completed or under construction prior to 90% of the building permits being issued for the approved number of lots or buildings. For single-family or townhouse installations, the lots on which these homes are located shall be identified on the Master Subdivision Plat, which may be amended;
 - (b) The installation of a geothermal system for a certain number or percentage of units within the development; or
 - (c) Energy efficiency standards that exceed minimum Building Code requirements (i.e. SEER rating for HVAC).
- d) Landscaping. The PD Plan for PUD-CZ shall demonstrate compliance with the standards of Sec. 8.2 Landscaping, Buffering and Screening, except that variations from these standards may be permitted where it is demonstrated that the proposed landscaping sufficiently buffers uses from each other, ensures compatibility with land uses on surrounding properties, creates attractive streetscapes and parking areas and is consistent with the character of the area. In no case shall a buffer be less than one half of the width required by Sec. 8.2 or 10 feet in width, whichever is greater.
- e) Signs. Signage in the PD Plan for PUD-CZ shall demonstrate compliance with Sec. 8.7 Signs, except that the standards can be varied if a master signage plan is submitted for review and approval concurrent with the PD plan and is determined by the Town Council to be suitable for the PUD-CZ and generally consistent with the intent and purpose of the sign standards of the UDO. The master signage plan shall have design standards that are exceptional and provide for higher quality signs than those in routine developments and shall comply with Sec. 8.7.2 Prohibited Signs.

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- f) Public facilities. The improvements standards and guarantees applicable to the public facilities that will serve the site shall comply with Article 7: Subdivision and Article 14: Parks, Recreation, Greenways, and Open Space.
 - (i) The PD Plan for PUD-CZ demonstrates a safe and adequate on-site transportation circulation system. The on-site transportation circulation system shall be integrated with the off-site transportation circulation system of the Town. The PD Plan for PUD-CZ shall be consistent with the Apex Transportation Plan and the *Town of Apex Standard Specifications and Standard Details* and show required right-of-way widths and road sections. A Traffic Impact Analysis (TIA) shall be required per Sec. 13.19.
 - (ii) The PD Plan for PUD-CZ demonstrates a safe and adequate on-site system of potable water and wastewater lines that can accommodate the proposed development, and are efficiently integrated into off-site potable water and wastewater public improvement plans. The PD Plan shall include a proposed water and wastewater plan.
 - (iii) Adequate off-site facilities for potable water supply, sewage disposal, solid waste disposal, electrical supply, fire protection and roads shall be planned and programmed for the development proposed in the PD Plan for PUD-CZ, and the development is conveniently located in relation to schools and police protection services.
 - (iv) The PD Plan shall demonstrate compliance with the parks and recreation requirements of Sec. Article 14: *Parks, Recreation, Greenways, and Open Space* and Sec. 7.3.1 *Privately-owned Play Lawns* if there is a residential component in the PUD-CZ.
- g) Natural resource and environmental protection. The PD Plan for PUD-CZ demonstrates compliance with the current regulatory standards of this Ordinance related to natural resource and environmental protection in Sec. 6.1 Watershed Protection Overlay District, Sec. 6.2 Flood Damage Prevention Overlay District, and Sec. 8.1 Resource Conservation.
- h) Storm water management. The PD Plan shall demonstrate that the post-development rate of on-site storm water discharge from the entire site shall not exceed pre-development levels in accordance with Sec. 6.1.7 of the UDO.
- i) Phasing. The PD Plan for PUD-CZ shall include a phasing plan for the development. If development of the PUD-CZ is proposed to occur in more than one phase, then guarantees shall be provided that project improvements and amenities that are necessary and desirable for residents of the project, or that are of benefit to the Town, are constructed with the first phase of the project, or, if this is not possible, then as early in the project as is technically feasible.
- j) Consistency with 2045 Land Use Map. The PD Plan for PUD-CZ demonstrates consistency with the goals and policies established in the Town's 2030 Land Use.
- k) Complies with the UDO. The PD Plan for PUD-CZ demonstrates compliance with all other relevant portions of the UDO.

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Legislative Considerations

The Town Council shall find the PUD-CZ designation demonstrates compliance with the following standards. Sec. 2.3.3.F:

The applicant shall propose site-specific standards and conditions that take into account the following considerations, which are considerations that are relevant to the legislative determination of whether or not the proposed conditional zoning district rezoning request is in the public interest. These considerations do not exclude the legislative consideration of any other factor that is relevant to the public interest.

- 1) Consistency with 2030 Land Use Map. The proposed Conditional Zoning (CZ) District use's appropriateness for its proposed location and consistency with the purposes, goals, objectives, and policies of the 2030 Land Use Map.
- 2) Compatibility. The proposed Conditional Zoning (CZ) District use's appropriateness for its proposed location and compatibility with the character of surrounding land uses.
- 3) Zoning district supplemental standards. The proposed Conditional Zoning (CZ) District use's compliance with Sec. 4.4 Supplemental Standards, if applicable.
- 4) Design minimizes adverse impact. The design of the proposed Conditional Zoning (CZ) District use's minimization of adverse effects, including visual impact of the proposed use on adjacent lands; and avoidance of significant adverse impacts on surrounding lands regarding trash, traffic, service delivery, parking and loading, odors, noise, glare, and vibration and not create a nuisance.
- 5) Design minimizes environmental impact. The proposed Conditional Zoning District use's minimization of environmental impacts and protection from significant deterioration of water and air resources, wildlife habitat, scenic resources, and other natural resources.
- 6) Impact on public facilities. The proposed Conditional Zoning (CZ) District use's avoidance of having adverse impacts on public facilities and services, including roads, potable water and wastewater facilities, parks, schools, police, fire and EMS facilities.
- 7) Health, safety, and welfare. The proposed Conditional Zoning (CZ) District use's effect on the health, safety, or welfare of the residents of the Town or its ETJ.
- 8) Detrimental to adjacent properties. Whether the proposed Conditional Zoning (CZ) District use is substantially detrimental to adjacent properties.
- 9) Not constitute nuisance or hazard. Whether the proposed Conditional Zoning (CZ) District use constitutes a nuisance or hazard due to traffic impact or noise, or because of the number of persons who will be using the Conditional Zoning (CZ) District use.
- 10) Other relevant standards of this Ordinance. Whether the proposed Conditional Zoning (CZ) District use complies with all standards imposed on it by all other applicable provisions of this Ordinance for use, layout, and general development characteristics.



January 25, 2021

Baohong Wan, Phd, P.E. VHB Engineering NC Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 919-829-0328

Subject: Staff summary and comments for the Hackney Tract Subdivision TIA,

12/22/2020

Dr. Wan:

Please review the following summary of my comments and recommendations. You may schedule a meeting with me and your client to discuss at your convenience.

Study Area

The TIA studied access to the proposed subdivision development at the following intersection:

Access #1/Hasse Avenue Extension and Olive Chapel Road

The following four intersections were also studied in the TIA:

- Olive Chapel Road and Richardson Road
- Olive Chapel Road and Apex Barbecue Road
- Richardson Road and Hasse Avenue/Little Gem Lane
- US Highway 64 East at Richardson Road
- US Highway 64 West at U-turn east of Richardson Road

Trip Generation

The proposed development is expected to consist of up to 100 single-family homes and 133 multi-family homes. It's projected to generate approximately 33 new trips entering and 106 new trips exiting the site during the weekday A.M. peak hour and 112 new trips entering and 66 new trips exiting the site during the weekday P.M. peak hour. The development is projected to add an additional 2,005 daily trips onto the adjacent roadway network.

Background traffic

Background traffic consists of 3% annual background traffic growth compounded to build out year 2024, and the following approved developments:

- Saddlebrook 25% traffic (75% build out)
- Buckhorn Preserve 50% traffic (50% build out)
- Stillwater -15% traffic (85% build out)
- Westford 20% traffic (80% build out)
- Smith Farm Residential—25% traffic (75% build out)
- Linden 85% traffic (15% build out)
- Sweetwater residential 20% traffic (80% build out)
- Sweetwater commercial

Trip Distribution and Assignment

The trip distributions to and from the development site are as follows:

- 50% to/from the east via US Highway 64
- 10% to/from the west via US Highway 64
- 25% to/from the east via Olive Chapel Road
- 5% to/from the west via Olive Chapel Road
- 8% to/from the south via Richardson Road
- 2% to/from the south via Apex Barbecue Road

Traffic Capacity Analysis and Recommendations

Level of Service (LOS) is a grade of A through F assigned to an intersection, approach, or movement to describe how well or how poorly it operates. LOS A through D is considered acceptable for peak hour operation. LOS E or F describes potentially unacceptable operation and developers may be required to mitigate their anticipated traffic impact to improve LOS based on the Apex Unified Development Ordinance (UDO).

Tables 1 through 8 describe the levels of service (LOS) for the scenarios analyzed in the TIA. "NA" is shown when the scenario does not apply. The scenarios are as follows:

- Existing 2020 Existing year 2020 traffic.
- **No Build 2024** Projected year (2024) with background growth, approved development traffic from others, and committed transportation improvements by others where applicable.
- **Build 2024** Projected year (2024) with background traffic, background improvements, and site build-out including recommended improvements where applicable.

Access #1/Hasse Avenue Extension and Olive Chapel Road (Unsignalized)

Table 1. A.M. / P.M. Unsignalized Peak Hour Levels of Service Access #1/Hasse Avenue Extension and Olive Chapel Road		
Build 2024		
<u>Overall</u> <u>NA</u>		
Eastbound (Olive Chapel Road) A / A ²		
Westbound (Olive Chapel Road) NA		
Southbound (Access #1/Hasse Avenue Extension)	C/D ¹	

- 1. Level of service for stop-controlled minor street approaches.
- 2. Level of service for left turn movements on free-flowing approaches.

TIA recommendations:

• The TIA recommends construction of Future Access #1/Hasse Avenue to consist of one inbound lane and one outbound lane. The TIA also recommends construction of a dedicated left-turn lane on eastbound Olive Chapel Road with 100 feet of storage length and appropriate taper, and a dedicated right-turn lane on westbound Olive Chapel Road with 100 feet of storage length and appropriate taper.

Apex staff recommendations:

Apex staff concur with the recommendations. The stop-controlled southbound approach
is projected to operate at LOS D or better with delays of 16 and 25 seconds per vehicle
in the AM and PM peak hours. The turn lanes proposed on Olive Chapel Road are
projected to provide enough capacity to store queues into the development during both
peak hours.

Olive Chapel Road and Richardson Road

Table 2. A.M. / P.M. Peak Hour Levels of Service Olive Chapel Road and Richardson Road			
	Unsignalized	Signa	alized
Existing No Build Build 20 2024			
<u>Overall</u>	NA A/A A/A		<u>A / A</u>
Eastbound (Olive Chapel Road)	B/B ²	A/B	A/B
Westbound (Olive Chapel Road)	B/B^2	B/B	B/B
Northbound (Richardson Road)	B/B¹	B/B	B/B
Southbound (Richardson Road)	B/C¹	A/B	A/B

- 1. Level of service for stop-controlled minor street approaches.
- 2. Level of service for left turn movements on free-flowing approaches.

TIA recommendations:

The TIA recommends no improvements at this intersection.

Apex staff recommendations:

Apex staff concur with the recommendations in the TIA. When signalized, this
intersection is projected to operate at LOS A in both peak hours in the Build 2024
scenario. A traffic signal has been approved by NCDOT at this intersection, and is
committed by adjacent development for installation prior to the build out of this
development.

Olive Chapel Road and Apex Barbecue Road (Unsignalized)

Table 3. A.M. / P.M. Unsignalized Peak Hour Levels of Service Olive Chapel Road and Apex Barbecue Road				
Existing No Build Build 2024				
<u>Overall</u>	<u>NA</u>	<u>NA</u>	<u>NA</u>	
Eastbound (Olive Chapel Road)	NA	NA	NA	
Westbound (Olive Chapel Road)	A/A^2	A/B^2	A/B^2	
Northbound (Apex Barbecue Road)	B/C¹	C/F ¹	C/F ¹	

- 1. Level of service for stop-controlled minor street approaches.
- 2. Level of service for left turn movements on free-flowing approaches.

TIA recommendations:

The TIA recommends no improvements at this intersection.

Apex staff recommendations:

• Apex staff concur with the recommendations in the TIA. The stop-controlled northbound approach is projected to operate at LOS F in the PM peak hour with delays of 135 seconds per vehicle and 95th percentile queues of 250 feet. However the development is not anticipated to add more than 3% to the overall intersection traffic volume, therefore no improvements are recommended per the UDO. This intersection is identified for future realignment in the Town's Transportation Plan, but no funded project is identified at this time and both roadways are state-maintained.

Richardson Road and Hasse Avenue/Little Gem Lane (Unsignalized)

Table 4. A.M. / P.M. Unsignalized Peak Hour Levels of Service Richardson Road and Hasse Avenue/Little Gem Lane				
Existing No Build Build 2024				
Overall	<u>NA</u>	<u>NA</u>	<u>NA</u>	
Eastbound (Little Gem Lane)	A / B¹	C/C¹	C/D¹	
Westbound (Hasse Avenue)	A/A^1	C/C ¹	C/C ¹	
Northbound (Richardson Road)	A/A^2	A/A^2	A/A^2	
Southbound (Richardson Road)	A/A^2	A/A^2	A / B ²	

- 1. Level of service for stop-controlled minor street approaches.
- 2. Level of service for left turn movements on free-flowing approaches.

TIA recommendations:

The TIA recommends no improvements at this intersection.

Apex staff recommendations:

 Apex staff concur with the recommendations in the TIA. The minor street approaches are projected to operate at LOS D or better during both peak hours of operation, with 95th percentile queues not exceeding 50 feet on any approach.

US Highway 64 East at Richardson Road

Table 5. A.M. / P.M. Peak Hour Levels of Service US Highway 64 East at Richardson Road			
	Unsignalized	Signa	alized
Existing No Build Build 2020 2024			
<u>Overall</u>	<u>NA</u>	<u>C / D</u>	<u>C / D</u>
Eastbound (US Hwy 64)	NA	C/D	C/E
Westbound Left (US Hwy 64)	F/F^2	B/B	A/B
Northbound Right (Richardson Road)	C/C¹	C/D	C/D

- 1. Level of service for stop-controlled minor street approaches.
- 2. Level of service for left turn movements on free-flowing approaches.

TIA recommendations:

The TIA recommends no improvements at this intersection.

Apex staff recommendations:

• Apex staff concur with the recommendations in the TIA. When signalized with dual westbound left and dual northbound right turn lanes, this intersection is projected to operate at LOS C and D in the AM and PM peak hours of operation with average intersection delays of 22 seconds and 44.5 seconds per vehicle. The eastbound approach is projected to operate at LOS E in the PM peak hour. However, the development is not anticipated to add more than 4% to the overall intersection traffic volume, therefore no improvements are recommended per the UDO. A traffic signal has been approved by NCDOT at this intersection, and is committed by adjacent development along with the additional turn lanes for installation prior to the build out of this development.

US Highway 64 West at U-turn east of Richardson Road

Table 6. A.M. / P.M. Peak Hour Levels of Service US Highway 64 West at U-turn east of Richardson Road				
	Unsignalized Signalized			
Existing No Build Build 2024 Build 2024				
<u>Overall</u>	<u>NA</u>	<u>B / C</u>	<u>B/C</u>	
Eastbound U-turn (US Hwy 64)	B/C ²	C/E	C/E	
Westbound (US Hwy 64) NA A/C B/C				

- 1. Level of service for stop-controlled minor street approaches.
- 2. Level of service for left turn or U-turn movements on free-flowing approaches.

TIA recommendations:

The TIA recommends no improvements at this intersection.

Apex staff recommendations:

Apex staff concur with the recommendations in the TIA. When signalized with dual
eastbound U-turn lanes, this intersection is projected to operate at overall LOS C or
better in both peak hours in the Build 2024 scenario. A traffic signal has been approved
by NCDOT at this intersection, and is committed by adjacent development along with the
additional U-turn lane for installation prior to the build out of this development.

Please coordinate with the NCDOT District Engineer's Office concerning recommended improvements. Town staff will be available for meetings with NCDOT staff to discuss improvements on state maintained roadways as needed. All recommendations are subject to review by Town Council prior to approval.

Sincerely,

Serge Grebenschikov Traffic Engineer

919-372-7448



Planned Unit Development-Conditional Zoning District Petition 2045 Land Use Map Amendment Process Information



PD PLAN/PUD-CZ PETITION SUBMISSION: Applications are due by 12:00 pm on the first business day of each month. See the <u>PUD Plan Schedule</u> on the website for more details.

PD PLAN/PUD-CZ PETITION FEES:

PUD-CZ Request: \$1,500.00 + \$10 an acre

PD Plan Amendment not requiring full TRC Review: \$500.00

2045 Land Use Map Amendment: \$700.00

later than five (5) working days prior to the desired meeting day.

PRE-APPLICATION MEETING: A pre-application meeting with members of the Technical Review Committee is required to be scheduled prior to the submittal of a PD Plan for PUD-CZ. Pre-application meetings are typically scheduled on the 1st, 2nd and 5th Thursdays of the month.

To schedule a meeting, applicants must e-mail a pdf map, drawing, model, site or sketch plan to Planner Lauren Staudenmaier (lauren.staudenmaier@apexnc.org) no

PURPOSE OF A PUD-CZ (UDO Section 3.3.3(C)): The purpose of the PUD-CZ is to permit variations in order to allow flexibility for landowners to creatively plan for a site specific, higher quality overall development of their land in a way that is not possible through the strict application of the minimum standards of this Ordinance. This is done through the application of performance standards that: integrate and mix uses where a mix of uses is proposed, possess interconnectivity, reflect the small-town character of Apex, expand opportunities for public transportation, preserve of natural features, integrate resource conservation area into plan for development, and that public facilities are available.

NEIGHBORHOOD MEETING: Neighborhood meetings are required per UDO Section 2.2.7 prior to application submission. The applicant is required to notify property owners and any neighborhood association that represents citizens within that area within 300 feet of the subject property via first class mail a minimum of 10 days in advance of the neighborhood meeting. The applicant shall use their own return address on the envelopes as the meeting is a private meeting between the developer and the neighbors. The applicant shall submit the "Certified List of Property Owners" and "Neighborhood Meeting Packet" forms included in this application packet with their initial submittal. The Neighborhood Meeting Packet is located at the very end of this document.

ANNEXATION REQUIREMENTS: If a property or portion thereof subject to the PUD is outside the corporate limits and ETJ, an <u>annexation petition</u> is **REQUIRED** to be submitted on the same day as this application.

Electronic Submittal Requirements (submit in IDT): Click here to access IDT Plans Website

- PUD-CZ Application
- PD Plan Text (pdf & Word versions)
- Colored Rendering of Building Elevations 11"x17"
- Transportation Impact Analysis

Site Plan Set

- 24" x 36" size
- Scale not less than: 1" = 50' horizontal, 1" = 5' vertical
- Saved as pdf no scanned plans

Hard Copy Submittal Requirements: Submit to Planning Department

- PUD-CZ Petition Application
- Petition Fee
- One (1) hardcopy PD Plan Text
- Three (3) bound Site Plan Sets 24" x 36" size
- Colored Rendering of Building Elevations
- Legal Description (metes and bounds)
- Certified List of Property Owners within 300 feet of subject property
- Development Name Approval Application
- Town of Apex Utilities Offer & Agreement
- Agent Authorization Form
- WCPSS Residential Development Notice
- Neighborhood Meeting Packet
- If applicable: Annexation Petition, map, legal description and \$200.00 fee
- Two (2) bound copies of the Transportation Impact Analysis and 1 copy of the TIA & traffic analysis files

on disk or FTP site at first submittal (if applicable)

- One (1) set of envelopes addressed to Certified List of Property Owners within 300 feet of subject property and all the HOAs of those properties within 300' of the subject property. Planning staff may require an additional set of envelopes based on the timing of the Planning Board and Town Council meetings.
- Addresses must be from a current list obtained from the Wake County GIS Map Services. A buffer report service is offered for \$1 per page. Please contact them at 919-856-6360 or http://www.wakegov.com/tax/Pages/default.aspx
- Affixed with first class stamps & the following return address:

Town of Apex Planning Department P.O. Box 250 Apex, NC 27502

PETITION PROCESS INFORMATION

NEIGHBORHOOD MEETING: Neighborhood meetings are required per UDO Section 2.2.7 prior to application submission. The applicant is required to notify property owners and any neighborhood association that represents citizens within that area within 300 feet of the subject property via first class mail a minimum of 10 days in advance of the neighborhood meeting. The applicant shall use their own return address on the envelopes as the meeting is a private meeting between the developer and the neighbors. The applicant shall submit the "Certified List of Property Owners" and "Neighborhood Meeting Packet" forms included in this application packet with their initial submittal. The Neighborhood Meeting Packet is located at the very end of this document.

REVIEW FOR SUFFICIENCY: Incomplete plans will be returned to the applicant and sufficiently complete applications are forwarded to the planning staff for review.

REVIEW BY STAFF: Planning staff reviews the application to determine compliance with the Unified Development Ordinance (UDO). If the application is determined not to be compliant with the UDO, comments will be sent to the applicant. The applicant must address all staff comments before any public hearings are scheduled.

Public Hearing Notification: Notification of the public hearing will take place by three different methods. A written notice will be sent to nearby property owners not more than 25 days nor less than 14 days prior to the public hearings, as required by the UDO. The Planning Department will prepare these written notifications for all property owners of the land subject to the application and all property owners within 300 feet of the land subject to the application. A notice will be published on the Town of Apex website (www.apexnc.org) no less than 10 days, but not more than 25 days, prior to the public hearings, and a notice will be posted at the land subject to the application at least 14 days prior to the public hearings.

1st Public Hearing/Planning Board Meeting: The Planning Board will consider the application, relevant support materials, the Staff Report and public testimony given at the public hearing. After the public hearing the Planning Board will make a recommendation to the Town Council. The Planning Board may recommend approval, approval with conditions or disapproval. The application is then forwarded to the Town Council. The Planning Board meets at 4:30 p.m. in the Town Hall Council Chambers on the date indicated on the Rezoning Schedule.

2ND **PUBLIC HEARING/TOWN COUNCIL MEETING:** The Town Council will consider the application, relevant support materials, the Staff Report, the Planning Board recommendation and public testimony given at the public hearing. After the public hearing the Town Council will vote to approve, approve with conditions or disapprove the rezoning. The Town Council meets at 6:00 p.m. in the Town Council Chambers on the date indicated on the Rezoning Schedule.

Last Updated: January 10, 2020

PLANNED UNIT DEVELOPMENT APPLICATION

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to	Э
third parties.	

11-2-21 Application #: #20CZ14 Submittal Date: \$ 2,300 Fee Paid Check # credit card

PETITION TO AMEND THE OFFICIAL ZONING DISTRICT MAP

Hackney Tracts Project Name:

2600 Olive Chapel Road, 2500 Olive Chapel Road, & 0 Olive Chapel Road Address(es):

0721492629, 0722406699, & 0722411102 PIN(s)

> 73.64 ac. Acreage:

RR & R-80W **PUD-CZ** Current Zoning: Proposed Zoning:

Med. Density Residential Current 2045 LUM Designation:

Med. Density Residential Requested 2045 LUM Designation:

See next page for LUM amendment

If any portion of the project is shown as mixed use (3 or more stripes on the 2045 Land Use Map) provide the following:

0 ac. Area classified as mixed use: Acreage:

0 ac. Area proposed as non-residential development: Acreage:

0% Percent of mixed use area proposed as non-residential: Percent:

Applicant Information

WithersRavenel Name:

137 S. Wilmington Street, Suite 200 Address:

NC 27601 Raleigh City: State: Zip:

919.469.3340 bvega@withersravenel.com Phone: E-mail:

Owner Information

PIN: 0721492629

Owner: GOODWIN, EDWIN A Address: Judy Hackney. 2505 Olive

Chapel Rd., Apex, NC 27502

PIN: 0722406699

Owner: HACKNEY, CHARLES LEON HACKNEY, JUDY G

Address: 2505 Olive Chapel Rd.,

Apex, NC 27502

PIN: 0722411102

Owner: HACKNEY, JUDY G

Address: 2505 Olive Chapel Rd.,

Apex, NC 27502

Agent Information

Other contacts:

Brendie Vega, WithersRavenel Name:

137 S. Wilmington Street, Suite 200 Address:

Raleigh NC 27601 City: State: Zip:

919.469.3340 bvega@withersravenel.com Phone: E-mail:

Glenda Toppe

PLANNED UNIT DEVELOPMENT APPLICAT	TION
Application #: #20CZ14	Submittal Date: 11-2-21
2045 LAND USE MAP AMENDMENT (if a	pplicable)
The applicant does hereby respectfully req request, the following facts are shown:	uest the Town Council amend the 2045 Land Use Map. In support of this
The area sought to be amended on the 2 Not applicable. No proposed change in cla	
Current 2045 Land Use Classification:	Med. Density Residential
Proposed 2045 Land Use Classification:	Med. Density Residential
	e amendment to the 2045 Land Use Map? Discuss the existing use tion to the adjacent land use classifications.
Not applicable. No proposed change in cla	ssification.

Beginning at an Existing Iron Pipe located at the Southwest corner of Lot 1, "William E. Gerringer Subdivision", Recorded at Map Book 1982, Page 24, Wake County Registry. Said Existing Iron Pipe having North Carolina Geodetic Coordinates (NAD 83, 2011) N: 719,823.90', E: 2,025,316.49' Said point is also located on the Northern Margin of Olive Chapel Road, Thence, following the Northern Margin of Olive Chapel Road; South 70°32'42" West, 65.39 feet to a point, said point being the True Point of Beginning. Thence, following the Northern Margin of Olive Chapel Road, South 70°31'17" West, 649.92 feet to a point, Thence, Leaving Said Road, North 34°12'20" West, 445.67 feet to a point; Thence, North 00°58'41" West, 436.43 feet to a point; Thence, North 85°35'51" West, 339.02 feet to an Existing Iron Pipe; Thence, South 02°31'45" West, 382.15 feet to an Existing Iron Pipe; Thence, North 87°46'36" West, 443.92 feet to an Existing Iron Pipe; Thence, North 01°42'56" East, 1,191.60 feet to an Existing Iron Pipe; Thence, North 01°42'19" East, 635.94 feet to a point located in the centerline of a creek, Said point being located South 01°42'19" West, 8.02 feet from an Existing Iron Pipe found on the North bank of the creek; Thence, along the centerline of the creek the following seventy-eight (78) calls: North 62°12'20" East, 26.95 feet to a point; Thence, North 85°25'51" East, 12.16 feet to a point; Thence, South 89°25'18" East, 9.95 feet to a point; Thence, North 72°42'15" East, 16.28 feet to a point; Thence, North 35°12'38" East, 17.29 feet to a point; Thence, North 04°12'00" East, 12.96 feet to a point; Thence, North 21°34'14" West, 18.72 feet to a point; Thence, North 09°03'47" West, 8.16 feet to a point, Thence, North 41°28'27" East, 26.53 feet to a point, Thence, South 84°15'14" East, 11.15 feet to a point, Thence, South 44°43'11" East, 19.83 feet to a point, Thence, South 71°15'05" East, 13.95 feet to a point, Thence, South 74°11'34" East, 15.85 feet to a point, Thence, South 74°44'51" East, 12.72 feet to a point, Thence, South 83°49'13" East, 3.99 feet to a point, Thence, North 64°08'10" East, 16.34 feet to a point, Thence, North 47°07'30" East, 15.60 feet to a point, Thence, South 78°20'55" East, 15.26 feet to a point, Thence, South 56°02'16" East, 5.33 feet to a point, Thence, South 19°19'09" East, 6.90 feet to a point, Thence, South 56°44'29" East, 12.49 feet to a point, Thence, South 83°31'01" East, 16.05 feet to a point, Thence, North 59°49'27" East, 15.58 feet to a point, Thence, North 16°43'28" East, 6.92 feet to a point, Thence, North 01°57'42" West, 8.52 feet to a point, Thence, North 19°34'33" West, 8.53 feet to a point, Thence, North 22°27'53" West, 25.52 feet to a point, Thence, North 08°13'00" West, 17.60 feet to a point, Thence, North 13°08'01" West, 25.39 feet to a point, Thence, North 19°34'33" West, 12.83 feet to a point, Thence, North 00°51'00" East, 8.68 feet to a point, Thence, North 37°09'53" East, 11.70 feet to a point, Thence, North 49°22'35" East, 26.46 feet to a point, Thence, North 62°21'20" East, 30.37 feet to a point, Thence, North 67°46'29" East, 19.95 feet to a point, Thence, North 02°19'02" West, 8.02 feet to a point, Thence, North 48°37'20" West, 9.79 feet to a point, Thence, North 51°28'51" West, 14.82 feet to a point, Thence, North 10°18'42" West, 10.15 feet to a point, Thence, North 29°53'30" East, 7.06 feet to a point, Thence, North 67°41'49" East, 9.59 feet to a point, Thence, South 56°14'07" East, 5.77 feet to a point, Thence, South 63°24'14" East, 9.29 feet to a point, Thence, South 76°41'34" East, 9.25 feet to a point, Thence, North 77°10'45" East, 14.30 feet to a point, Thence, North 49°00'07" East, 13.34 feet to a point, Thence, North 10°50'19" West, 12.26 feet to a point, Thence, North 64°58'17" West, 15.90 feet to a point, Thence, North 31°59'29" West, 7.02 feet to a point, Thence, North 01°03'18" West, 7.87 feet to a point, Thence, North 17°34'16" East, 24.60 feet to a point, Thence, North 26°59'18" East, 8.17 feet to a point, Thence, South 81°51'44" East, 16.60 feet to a point, Thence, South 33°48'00" East, 15.96 feet to a point, Thence, South 49°25'00" East, 16.68 feet to a point, Thence, North 78°59'30" East, 12.42 feet to a point, Thence, North 50°28'53" East, 20.42 feet to a point, Thence, North 70°44'43" East, 46.11 feet to a point, Thence, South 89°01'57" East, 16.84 feet to a point, Thence, South 73°56'31" East, 11.76 feet to a point, Thence, North 66°33'30" East, 13.41 feet to a point, Thence, North 10°20'58" East, 8.36 feet to a point, Thence, North 17°44'49" West, 19.09 feet to a point, Thence, North 07°53'24" East, 12.39 feet to a point, Thence, North 59°58'19" East, 13.53 feet to a point, Thence, South 42°16'28" East, 13.69 feet to

a point, Thence, South 04°17'52" West, 12.70 feet to a point, Thence, South 10°35'03" West, 9.31 feet to a point, Thence, South 32°25'41" East, 5.70 feet to a point, Thence, South 46°46'35" East, 17.73 feet to a point, Thence, South 60°06'25" East, 16.74 feet to a point, Thence, North 86°29'56" East, 19.64 feet to a point, Thence, North 81°25'49" East, 16.54 feet to a point, Thence, South 80°06'27" East, 29.38 feet to a point, Thence, South 84°39'29" East, 22.26 feet to a point, Thence, North 58°33'23" East, 13.24 feet to a point, Thence, North 74°43'49" East, 8.91 feet to a point, Thence, leaving the centerline of said creek, South 20°58'05" East, 22.05 feet to a point, Thence, South 20°45'12" East, 790.03 feet to an Existing Iron Pipe, Thence, South 56°33'25" East, 611.03 feet to an Existing Iron Pipe, Thence, South 78°41'14" West, 615.50 feet to a point, Thence, South 11°18'46" East, 791.04 feet to a point, Thence, North 78°41'14" East, 566.96 feet to a point, Thence, South 09°38'52" East, 536.92 feet to a point, being the **True Point of Beginning**, and having an area of 51.280 Acres, more or less.

Together with the following area located within the public right of way of Olive Chapel Road

Beginning at an Existing Iron Pipe located at the Southwest corner of Lot 1, "William E. Gerringer Subdivision", Recorded at Map Book 1982, Page 24, Wake County Registry. Said Existing Iron Pipe having North Carolina Geodetic Coordinates (NAD 83, 2011) N: 719,823.90', E: 2,025,316.49' Said point is also located on the Northern Margin of Olive Chapel Road, Thence, following the Northern Margin of Olive Chapel Road; South 70°32'42" West, 65.39 feet to a point, said point being the **True Point of Beginning.** Thence, South 70°29'55" West, 636.77 feet to a point; Thence, North 34°12'20" West, 31.27 feet to a point; Thence, North 70°31'17" East, 649.92 feet to a point; Thence, South 09°38'52" East, 30.43 feet to a point; being the **True Point of Beginning**, and having an area of 0.445 Acres (19,375 sf), more or less.

Legal description for Tract 2 Hackney Property

Beginning at an Existing Iron Pipe located at the Southwest corner of Lot 1, "William E. Gerringer Subdivision", Recorded at Map Book 1982, Page 24, Wake County Registry. Said Existing Iron Pipe having North Carolina Geodetic Coordinates (NAD 83, 2011) N: 719,823.90', E: 2,025,316.49' Said point is also located on the Northern Margin of Olive Chapel Road, Thence, following the Northern Margin of Olive Chapel Road; South 70°32'42" West, 65.39 feet to a point; Thence, South 70°31'17" West, 649.92 feet to a point, said point being the **True Point of Beginning.**

Thence, following the Northern Margin of Olive Chapel Road, South 70°19'56" West, 682.58 feet to a New Iron Pipe, Thence, Leaving Said Right of Way, Thence, North 02°31'13" East, 5.41 feet to an Existing Iron Pipe; Thence, North 02°31'13" East, 674.17 feet to an Existing Iron Pipe; Thence, North 02°31'45" East, 382.15 feet to an Existing Iron Pipe; Thence, South 85°35'51" East, 339.02 feet to a point, Thence, South 00°58'41" East, 436.43 feet to a point; Thence, South 34°12'20" East, 445.67 feet to a point, being the **True Point of Beginning**, and having an area of 9.526 Acres, more or less.

Together with the following area located within the public right of way of Olive Chapel Road

Beginning at an Existing Iron Pipe located at the Southwest corner of Lot 1, "William E. Gerringer Subdivision", Recorded at Map Book 1982, Page 24, Wake County Registry. Said Existing Iron Pipe having North Carolina Geodetic Coordinates (NAD 83, 2011) N: 719,823.90', E: 2,025,316.49' Said point is also located on the Northern Margin of Olive Chapel Road, Thence, following the Northern Margin of Olive Chapel Road; South 70°32'42" West, 65.39 feet to a point; Thence, South 70°31'17" West, 649.92 feet to a point, said point being the **True Point of Beginning.**

Thence, South 34°12'20" East, 31.27 feet to a point; Thence, South 70°19'56" West, 702.77 feet to a point; Thence, North 02°31'13" East, 32.69 feet to a New Iron Pipe; Thence, North 70°19'56" East, 682.58 feet to a point; being the **True Point of Beginning**, and having an area of 0.481 Acres (20,967 sf), more or less.

Legal description for Tract 3 Hackney Property

Beginning at an Existing Iron Pipe located at the Southwest corner of Lot 1, "William E. Gerringer Subdivision", Recorded at Map Book 1982, Page 24, Wake County Registry. Said Existing Iron Pipe having North Carolina Geodetic Coordinates (NAD 83, 2011) N: 719,823.90', E: 2,025,316.49' Said point is also located on the Northern Margin of Olive Chapel Road, Thence, following the Northern Margin of Olive Chapel Road; South 70°32'42" West, 65.39 feet to a point; Thence, Leaving said Right of Way, North 09°38'52" West, 536.92 feet to a point; Thence, South 78°41'14" West, 566.96 feet to a point; Thence, North 11°18'46" West, 791.04 feet to a point; Thence, North 78°41'14" East, 615.50 feet to an Existing Iron Pipe; Thence, South 11°18'46" East, 500.58 feet to an Existing Iron Pipe; Thence, South 11°21'53" East, 392.29 feet to an Existing Iron Pipe; Thence, South 11°20'41" East, 425.59 feet to an Existing Iron Pipe, being the Point of Beginning, and having an area of 11.871 Acres, more or less.

Together with the following area located within the public right of way of Olive Chapel Road

Beginning at an Existing Iron Pipe located at the Southwest corner of Lot 1, "William E. Gerringer Subdivision", Recorded at Map Book 1982, Page 24, Wake County Registry. Said Existing Iron Pipe having North Carolina Geodetic Coordinates (NAD 83, 2011) N: 719,823.90', E: 2,025,316.49' Said point is also located on the Northern Margin of Olive Chapel Road; Thence, South 11°20'41" East, 30.29 feet to a point; Thence, South 70°32'42" West, 66.30 feet to a point; Thence, North 09°38'52" West, 30.43 feet to a point; Thence, North 70°32'42" East, 65.39 feet to an Existing Iron Pipe, being the Point of Beginning, and having an area of 0.045 acres (1,975 sf), more or less.

AGEN	T AUTHORIZAT	ION FORM				
Applic	ation #:	20CZ14	Submittal Date:	11-2-20		
Hackney, Judy G			is the owner* of the proper	is the owner* of the property for which the attached		
applica	tion is being su	bmitted:				
	Land Use Ar	nendment				
V	a	uthorization includes ex	nd Planned Development rezoning appl spress consent to zoning conditions that the application is approved.	ications, this t are agreed to by t	ne	
	Site Plan					
	Subdivision					
	Variance					
	Other:	=				
The pro	operty address	is: 0 Olive Chapel	Road (PIN 0722411102)			
The age	ent for this proj	ect is: WithersRavene				
	☐ I am the	owner of the property a	nd will be acting as my own agent			
Agent	Name:	Brendie Vega				
Addres		137 S. Wilmington Str	reet, Suite 200			
Teleph	one Number:	919.535.5212			_	
F-Mail	Address:	bvega@withersravene	el.com			
		Signature(s) of Owner Tudy	er(s)* 3. Aakkey 6, Hackney Type or print name	Oct 1	_7, 2076 Date	
			Type or print name		Date	

Attach additional sheets if there are additional owners.

*Owner of record as shown on the latest equalized assessment rolls of Wake County. An option to purchase does not constitute ownership. If ownership has been recently transferred, a copy of the deed must accompany this authorization.

Pursuant to Article 40 of Chapter 66 of the North Carolina General Statutes (the Uniform Electronic Transactions Act) this application and all documents related hereto containing an electronic or digitized signature are legally binding in the same manner as are hard copy documents executed by hand signature. The parties hereby consent to use electronic or digitized signatures in accordance with the Town's Electronic Signature Policy and intend to be bound by the application and any related documents. If electronic signatures are used the application shall be delivered in an electronic record capable of retention by the recipient at the time of receipt.

AGENT	AUTHORIZATI	on Form		
Application #: 20CZ14 Hackney, Charles Leon Hackney, Judy G		20CZ14	Submittal Date: 11-2-20	
		is the owner* of the prop	perty for which the attached	
applicat	ion is being su	bmitted:		
	Land Use An	nendment		
7	a	or Conditional Zoning and Pla uthorization includes expres gent which will apply if the a	anned Development rezoning a ss consent to zoning conditions application is approved.	pplications, this that are agreed to by the
	Site Plan			
V	Subdivision			×
	Variance			
	Other:			
The pro	perty address i	s: 2500 Olive Chapel F	Road (PIN 0722406699)	
The age	nt for this proj	ect is: WithersRavenel		
	☐ I am the o	owner of the property and w	vill be acting as my own agent	
Agent N	lame:	Brendie Vega		
Address	s:	137 S. Wilmington Street,	Suite 200	4
Telepho	one Number:	919.535.5212		
	Address:	bvega@withersravenel.co	om	
		Signature(s) of Owner(s)? Chaolas Value Judy C	Leon Hackney Type or print na y g, factae Type or print na Type or print na	8 Oct 27,2021

Attach additional sheets if there are additional owners.

*Owner of record as shown on the latest equalized assessment rolls of Wake County. An option to purchase does not constitute ownership. If ownership has been recently transferred, a copy of the deed must accompany this authorization.

Pursuant to Article 40 of Chapter 66 of the North Carolina General Statutes (the Uniform Electronic Transactions Act) this application and all documents related hereto containing an electronic or digitized signature are legally binding in the same manner as are hard copy documents executed by hand signature. The parties hereby consent to use electronic or digitized signatures in accordance with the Town's Electronic Signature Policy and intend to be bound by the application and any related documents. If electronic signatures are used the application shall be delivered in an electronic record capable of retention by the recipient at the time of receipt.

AGENT AUTHORIZATION FORM Application #: 20CZ14 Goodwin, Edwin A		Submittal Date:	11-2-20	
			is the owner* of the prope	rty for which the attached
applicat	tion is being sub	omitted:		
	Land Use Am	nendment		
Ø	au	r Conditional Zoning and Pla uthorization includes express gent which will apply if the a	nned Development rezoning app s consent to zoning conditions the application is approved.	olications, this nat are agreed to by the
	Site Plan			
V	Subdivision			
	Variance			
	Other:			
The pro	perty address is	s: 2600 Olive Chapel R	Road (PIN 0721492629)	
The age	ent for this proje	ect is: WithersRavenel		
	☐ I am the o	owner of the property and w	ill be acting as my own agent	
Agent N	Jame:	Brendie Vega		
Address		137 S. Wilmington Street,	Suite 200	
		919.535.5212		
	one Number:		m	
E-Mail	Address:	bvega@withersravenel.cor		
		Judy G. Hackne A. Goodwin Test	J. HACKMY Trates of the amentory Thypeor print nan Leon Hackey Sr	Edvin Date Date
		Winder	Juan Flockowy, Sie	Dat Z 1, Z

Attach additional sheets if there are additional owners.

*Owner of record as shown on the latest equalized assessment rolls of Wake County. An option to purchase does not constitute ownership. If ownership has been recently transferred, a copy of the deed must accompany this authorization.

Pursuant to Article 40 of Chapter 66 of the North Carolina General Statutes (the Uniform Electronic Transactions Act) this application and all documents related hereto containing an electronic or digitized signature are legally binding in the same manner as are hard copy documents executed by hand signature. The parties hereby consent to use electronic or digitized signatures in accordance with the Town's Electronic Signature Policy and intend to be bound by the application and any related documents. If electronic signatures are used the application shall be delivered in an electronic record capable of retention by the recipient at the time of receipt.

, ibb.	ication #:	20CZ14		Submittal Date:	11-2-20	
	ndersigned, Too or affirms as follow		cknen	(the "Affiant")	first being duly	sworn, hereby
		the authoriz	of age and ed agent	authorized to make this of all owners, of and legally described in	the propert	y located a
	incorporated here	ein (the "Property	/").			
	This Affidavit of O the Town of Apex		e for the pur	pose of filing an applicati	on for developme	ent approval with
	If Affiant is the ov	vner of the Prop	erty, Affiant	acquired ownership by	leed, dated	
	and recorded in t	ne Wake County	Register of D	Deeds Office on	, in Book	Page
1.		ency relationship		ner(s) of the Property, a e Affiant the authority to		
	in interest have k	een in sole and	undisturbed	ownership of the Propert I possession and use of t	he property duri	ing the period o
	Affiant's ownersh claim or action ha acting as an authonor is any claim Property.	ip or right to pos is been brought a orized agent for or action pendi	session nor against Affia owner(s)), w ng against /	roperty on	orofits. To Affiant r), or against own ght to possession	's knowledge, no er(s) (if Affiant i of the property
	Affiant's ownersh claim or action ha acting as an autho nor is any claim	ip or right to pos is been brought a orized agent for or action pendi	session nor against Affia owner(s)), w ng against /	roperty on	orofits. To Affiant r), or against own ght to possession	's knowledge, no er(s) (if Affiant i of the property
	Affiant's ownersh claim or action ha acting as an authonor is any claim Property.	ip or right to pos is been brought a orized agent for or action pendi	session nor against Affia owner(s)), w ng against /	roperty on	orofits. To Affiant'), or against own ght to possession ourt regarding po	's knowledge, no her(s) (if Affiant i h of the property ossession of the
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	Affiant's ownersh claim or action ha acting as an author is any claim Property. This the 27	ip or right to poses been brought a portized agent for or action pending day of Octob	session nor against Affia owner(s)), w ng against /	roperty on	orofits. To Affiant), or against own ght to possession burt regarding port clear indu Lean Ty Lean Heck	's knowledge, no ler(s) (if Affiant is of the property ossession of the seal of the least of the
the	Affiant's ownersh claim or action ha acting as an author is any claim Property. This the 27	ip or right to posits been brought a prized agent for or action pendiday of October NA	session nor against Affia owner(s)), w ng against /	roperty on	profits. To Affiant own ght to possession ourt regarding possession ourt regarding possession for the classical states and the classical states are considered as the classical states are classical states and the classical states are classical states and the classical states are classical states and the classical states are classical	's knowledge, no ler(s) (if Affiant in of the property ossession of the least ossession ossession of the least ossession o
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the aid Af	Affiant's ownersh claim or action ha acting as an authonor is any claim Property. This the 27 OF NORTH CAROLLEY OF WALE Undersigned, a language of the langu	ip or right to posits been brought a prized agent for or action pending day of October Public in Affiant, per	session nor against Affia owner(s)), wang against Affia owner(s), wang aga	roperty on	Sean Heck	s knowledge, no ler(s) (if Affiant is of the property ossession of the cossession of the least o
the aid Af	Affiant's ownersh claim or action ha acting as an authonor is any claim Property. This the 27 OF NORTH CAROLLING OF LOCAL CONTROLLING AND ACTION OF LOCAL CO	ip or right to posits been brought a prized agent for or action pending day of October Public in Affiant, per	session nor against Affia owner(s)), wang against Affia owner(s), wang aga	roperty on	Sean Heck	s knowledge, no ler(s) (if Affiant is of the property ossession of the cossession of the least o

TOWN OF APEX UTILITIES OFFER AND AGREEMENT

Application #:	20CZ14		44.2.20
Application #.		Submittal Date:	11-2-20
	73 P.O. Box	own of Apex Hunter Street 250 Apex, NC 27502 19-249-3400	
		LINA CUSTOMER SELECTION AGRE	EMENT
	Wake County PINs: 0721492629	9, 0722411102, 0722406699	
	0, 2500, 2600 Olive Chapel Roa	nd	
	(th	ne "Premises")	
you accept the Tov the Town. Hackney, Judy e Town of Apex (the	of Apex offers to provide you with elevn's offer, please fill in the blanks on t t al, the undersigned cu "Town") as the permanent electric su prary service if needed.	his form and sign and we will have ustomer ("Customer") hereby irrev	e an Agreement once signed by vocably chooses and selects the
	delivery, and use of electric power by and conditions of the Town's service r		
the requested serv	understands that the Town, based up rice. By signing this Agreement the un wider, for both permanent and tempo	dersigned signifies that he or she	has the authority to select the
	ional terms and conditions to this Agre utes the entire agreement of the parti		1. If no appendix is attached this
Acceptano	ce of this Agreement by the Town cons	stitutes a binding contract to purc	hase and sell electric power.
Please no supplier for the Pre	te that under North Carolina General S emises.	Statute §160A-332, you may be er	ntitled to choose another electric
	eptance of this Agreement, the Town on the second looks forward to working with the second looks forward looks for working with the second looks for which we will be second looks for which the second l	-	vill be pleased to provide electric
ACCEPTED:			
CUSTOMER: Ha	ckney, Judy, et. al	TOWN OF APEX	
Brendie		BY:	uthorized Agent
11/2/202	Authorized Agent		uthorized Agent
DATE:	- -	DATE:	

DEVELOPMENT NAME APPROVAL APPLICATION

Application #:	20CZ14	Submittal Date:	11-2-20
Fee for Initial Sub	omittal: No Charge	Fee for Name Chang	e after Approval: \$500*

Purpose

To provide a consistent and clearly stated procedure for the naming of subdivisions and/or developments and entrance roadways (in conjunction with *Town of Apex Address Policy*) so as to allow developers to define and associate the theme or aesthetics of their project(s) while maintaining the Town's commitment to preserving the quality of life and safety for all residents of Apex proper and extraterritorial jurisdiction.

Guidelines

- ✓ The subdivision/development name shall not duplicate, resemble, or present confusion with an existing subdivision/development within Apex corporate limits or extraterritorial jurisdiction except for the extension of an existing subdivision/development of similar or same name that shares a continuous roadway.
- ✓ The subdivision/development name shall not resemble an existing street name within Apex corporate limits or extraterritorial jurisdiction unless the roadway is a part of the subdivision/development or provides access to the main entrance.
- ✓ The entrance roadway of a proposed subdivision/development shall contain the name of the subdivision/development where this name does not conflict with the Town of Apex Road Name Approval Application and Town of Apex Address Policy guidelines.
- ✓ The name "Apex" shall be excluded from any new subdivision/development name.
- ✓ Descriptive words that are commonly used by existing developments will be scrutinized more seriously in order to limit confusion and encourage distinctiveness. A list of commonly used descriptive words in Apex's jurisdiction is found below.
- ✓ The proposed subdivision/development name must be requested, reviewed and approved during preliminary review by the Town.
- ✓ A \$500.00 fee will be assessed to the developer if a subdivision/development name change is requested after official submittal of the project to the Town.*

*The imposed fee offsets the cost of administrative changes required to alleviate any confusion for the applicant, Planning staff, other Town departments, decision-making bodies, concerned utility companies and other interested parties. There is no charge for the initial name submittal.

Existing Development Titles, Recurring

	Residential	Non-Residential
10 or more	Creek, Farm(s), Village(s),	Center/Centre
6 to 9	Crossing(s), Park, Ridge, Wood(s)	Commons, Park
3 to 5	Acres, Estates, Glen(s), Green*, Hills	Crossing(s), Plaza, Station, Village(s)

^{*}excludes names with Green Level

CERTIFIED LIST OF NEIGHBORING PROPERTY OWNERS

Application #:	Submittal Date:	11-2-20	

Provide a certified list of property owners subject to this application and all property owners within 300' of the subject property and HOA Contacts.

		Owner's Name		PIN
1.	See Attachment.			
2.				
3.				
4.				
5.				
6.				
7.				
8.				
9.				
10.				
11.				
12.				
14.				
15.				
,				listing of all property owners and
prop	erty owners within 300'	of the subject property	2	` (/
Date	11/2/2020	Ву:	Tend	u Vlgor
				U
COU	NTY OF WAKE STATE OF	NORTH CAROLINA		
Swor	n and subscribed before	e me, Jeri Chasta		lotary Public for the above State and
	ty, on this the		, 20 <u>20</u> .	
			Jesi Ch	astaw Pederson
SE	AL		Jeri Ci	Notary Public hastain Pederson
				Print Name
•	JERI CHASTAIN PE Notary Publi	C	My Commission Exp	oires: 03/10/2024
	Wake County, North My Commission E March 10, 20	xpires P		•

	Certified List of Neighboring Property Owners	
#	OWNER	PIN
1	GARWOOD, MARGARET GARWOOD, JOHN J	721396377
2	CENIS, NATHAN T CENIS, EMILY ANNE	721396585
	HURLEY, SONIA R HURLEY, ROBERT	721397152
	HORNEY, DIANNA HORNEY, JOEY MICHAEL	721397339
	SMITH FARM OF APEX HOMEOWNERS ASSOCIATION, INC.	721397362
6	RILEY, JAMES EDWARD JR DODSON, JILLIAN SMITH	721397491
	KOSHY, SIBY VARKEY KOSHY, THARU SARA	721397536
	ZHOU, QUAN LIU, SHUZHANG	721397599
	LABRU, VINEET UCHIL, SHRUTI KARUNAKAR	721398442
	KONAKATI, VIKRAM BHIMAVARAPU, PRATHYUSHA	721399015
	SAMPATH, PRABHU PRABHU, DEEPA	721399121
	MATTHEWS, JOHN HENRY III	721399127
	ARCADIA RIDGE HOMEOWNERS ASSOCIATION, INC.	721399233
	LACOSTE, FABRICE SANROMAN, STEPHANIE	721399404
	DE SOUSA, ALDO SILVIO CARNEIRO	721399466
-	LEDESMA, FELIPE ATENCIO, IBELISE MARIA	721399630
	MARKS, REBECCA R MARKS, JONATHAN A	721399646
	RUBIN, BEVERLY L	721482119
-	AUSTVOLD, SHAWN AUSTVOLD, JENNIFER	721491084
	RILEY'S POND HOMEOWNERS ASSOCIATION INC	721491103
-	CAVERO, CLAUDIA MARIANA BENAVIDES	721491270
	RILEY'S POND HOMEOWNERS ASSOCIATION INC	721491342
	LEWIS, ANGEL SPENCE LEWIS, COURTNEY DEVON	721492100
	RILEY'S POND HOMEOWNERS ASSOCIATION INC	721492366
-	WEBB, XAVIER JOHAN	721492372
	JOYCE, JOHN D JOYCE, ROSEMARY	721493109
	ALJADER, MAYSAM ALJADER, LORI	721493206
	LEARY, BRAD LEARY, BRENDA	721494283
	BAKER, SCOTT J BAKER, MARLO L H	721495137
	SMITH FARM OF APEX HOMEOWNERS ASSOCIATION, INC.	721396648
	RAJAN, SUNIL KUMAR OLIPARAMBIL PREMRAJ, RITHU	721396847
-	GANJI, BHAGYA LATHA RAMPA, IMMANUEL	721396870
	GARABEDIAN, MATTHEW KANG, EY JUNG	721396975
	BALAPURE, LAXMIKANT MALVI, VISHAKHA	721397746
	ESBJORN, ROBERT ESBJORN, AUDREY	721397948
	CHEN, DANDAN WANG, YANG	721398717
	MCCALL, NATHAN RF GIULIANI, TRACY J	721398917
	SIDDIQUI, ALI SIDDIQUI, TARANNUM	721399742
	PANDEY, ROSHAN RAJ	721399757
-	WILSON, BENJAMIN THOMAS THOMAS, JULIE ELIZABETH	721399853
	PANDEY, NAMIT JOSHI, TARA	721399859
	BRUMFIELD, RYAN MATTHEW BRUMFIELD, AMANDA PLOCH	721399954
-	MOCK, CHRISTOPHER RICHARD MOCK, ELENA BARRIO	721399969
-	BOLJESIC, JONATHON ELLIS BOLJESIC, VINCA PURI	721494337
45	MARTIN, JOANNE H	721494350

Certified List of Neighboring Property Ow	ners
# OWNER	PIN
46 RILEY'S POND HOMEOWNERS ASSOCIATION INC	721494411
47 DONALDSON, MARK R DONALDSON, HEATHER M	721495361
48 GOTUR, RAGHAVENDER THAMMISETTY, RADHIKA	721495379
49 KENT, THOMAS L. KENT, LEIGH R.	721496224
50 CHOI, KENNY JUNG, JIN	721496464
51 SINGH, SUNIL SINGH, PANCHALI	721497298
52 LEDESMA, ROBERTO LEDESMA, CARMEN	721497385
53 RILEY'S POND HOMEOWNERS ASSOCIATION INC	721497414
54 KUMAR VARMA, CHITRA DILEEP ADUKKATH, BISHAK	721497452
55 KNAPP, GEARY W KNAPP, SUSAN	721499346
56 LENNAR CAROLINAS, LLC	722219077
57 LENNAR CAROLINAS, LLC	722229350
58 LENNAR CAROLINAS, LLC	722303175
59 SMITH FARM OF APEX HOMEOWNERS ASSC INC	722303478
60 BAITER, STEVE MICHAEL BAITER, REAGAN	722303663
61 JOSEPH, BIKKU B VALIYAVEETIL, SAJIN J	722303770
62 BROCK, SIMON PAUL BROCK, ELISA SAYURI JISAK	722303779
63 NICOLAU, DANIEL NICOLAU, MARIA SIMONA	722303837
64 SANDBERG, GEOFFREY ERIK SANDBERG, RACHEL ANN	722304905
65 PITMAN, WESLEY SZYDLOWSKI, JESSICA	722305447
66 DYK, SHAUN M BIGELOW DYK, MELINDA M	722305656
67 SMITH FARM OF APEX HOMEOWNERS ASSOCIATION, INC.	722309093
68 PIKULIK, KENNETH CHARLES GUARD-PIKULIK, MEGAN THAYE	R 722313076
69 KLEIN, CAROLINE KLEIN, STEVEN	722327144
70 FEDERICO, MICHELLE EDERY, ARIEL	722327201
71 SWEETWATER PROPERTY OWNERS ASSOCIATION, INC	722327341
72 RUTIGLIANO, JOHN P RUTIGLIANO, KAREN E	722327354
73 PAYNE, DEAN ALAN PAYNE, LISA O'HARA	722327358
74 CLEARY, MICHAEL CLEARY, MAUREEN	722327452
75 FERGUSON, MICHAEL R FERGUSON, JESSICA J	722327455
76 FAIRHURST, JOSHUA FAIRHURST, AMANDA L	722327459
77 DUFFMAN, MARY WEBB	722329579
78 BENNETT, JESSICA SOPHIA BENNETT, RYAN CLARK	722416567
79 SMITH, SCOTT ROBERT SMITH, KIMBERLY DAWN	722416644
80 CRESCENT APEX LLC	722416751
81 CRESCENT APEX LLC	722416778
82 CRESCENT APEX LLC	722416847
83 VLADIMIROVA, ANNA V HOBBS, MERLIN E	722417467
84 DUDDUKURI, VENKATA SANDEEP KUMAR ALLU, SOWMYA	722417511
85 CRESCENT APEX LLC	722418579
86 CRESCENT APEX LLC	722418624
87 CRESCENT APEX LLC	722419526
88 CRESCENT APEX LLC	722419572
89 CRESCENT APEX LLC	722419696

Certified List of Neighboring Property Owners				
# OWNER				
	LAHRMAN, GREGORY E TRUSTEE GREGORY E. LAHRMAN REVOCABLE LIVING			
90	TRUST	722420650		
91	SWEETWATER PROPERTY OWNERS ASSOCIATION, INC	722421400		
92	FUNNA, KUCHI FUNNA, KUCHI M	722421612		
93	CRESCENT APEX LLC	722424038		
94	CRESCENT APEX LLC	722429361		
95	PILLA, ANTHONY MICHAEL PILLA, ANGELA	722510428		
96	ASPNES, DAVID E BALL, CYNTHIA J	721487120		
97	GOODWIN, EDWIN A	721492629		
98	FOSTER FARM LLC	721585231		
	MICHALSKI, TIMOTHY MICHALSKI, RHIANNON	721590573		
100	FOSTER FARM LLC	721592562		
101	FOSTER, FRANK A COPELAND, REBECCA	721595134		
102	HACKNEY, CHARLES LEON HACKNEY, JUDY G	722406699		
103	HACKNEY, JUDY G	722411102		
104	CRESCENT APEX LLC	722418369		
105	PALANIAPPAN, RAMANATHAN VIJAYAKUMAR, HARIPRABHA	722418413		
106	CRESCENT APEX LLC	722419315		
107	MULLEN, RICHARD ANDREW MULLEN, ELIZABETH CATHERINE	722419361		
108	PERKINS, ELIZABETH E	722503152		
109	BASS, MICHAEL E BASS, SHERRIE L	722503445		
110	BASS, MICHAEL E	722505167		
111	DUGGAN, KIM-MARIE DUGGAN, DOMINICK	722510237		
112	CRESCENT APEX LLC	722510474		
113	MONGONE, MERRIDITH MONGONE, FRANK	722511203		
114	CRESCENT APEX LLC	722511431		
115	CRESCENT APEX LLC	722512006		
116	TRUSTEES OF THE PINOT PARTNERS REVOCABLE LIVING TR	722512179		
117	OGNIBENE, DOMINICK OGNIBENE, MARIE ELENA	722512201		
118	CRESCENT APEX LLC	722513145		
119	CRESCENT APEX LLC	722513341		
120	CRESCENT APEX LLC	722514101		
121	CRESCENT APEX LLC	722528250		

Application #: 20CZ14	Submittal Date:	11-2-20						
Proposed Subdivision/Development Information								
Description of location: 2600, 2500, & 0 Olive Chapel Road								
Nearest intersecting roads: Olive Chapel Road / Kythira Drive								
Wake County PIN(s): 0721492629, 0722406699, & 072	22411102							
Township: Apex								
Contact Information (as appropriate)								
Contact person: Brendie Vega								
Phone number: 919.535.5212 Fax no	ımber:							
Address: 137 S. Wilmington Street, Suite 200								
E-mail address: bvega@withersravenel.com								
Owner:								
Phone number: Fax no								
Address:								
E-mail address:								
Proposed Subdivision/Development Name								
1 st Choice: TBD at time of Subdivision								
2 nd Choice (Optional):								
··(-p								
Town of Apex Staff Approval:								
Town of Apex Planning Department Staff Date								

DEVELOPMENT NAME APPROVAL APPLICATION

NOTICE OF ELECTRONIC NEIGHBORHOOD MEETING

	document is a public record under the North Carolina Pub	lic Records Act and may be pub	lished on the Town's website
	closed to third parties. 8/2020		
Dat			
Daar	Meighborn		
	 Neighbor: are invited to an electronic neighborhood meeting 	to review and discuss the d	evelonment proposal at
2600	0, 2500, and 0 Olive Chapel Road	0721492629, 072240	
	Address(es)	Р	IN(s)
and office held. Development www. distate hear. An E	e a way for the applicant to discuss the project and neighborhood organizations before the submittal organizations and discuss any containing submitted. If you are unable to attend, you may a contain the submitted to the submit	of an application to the Town neerns about the impacts of ay contact the applicant before the Town, it may be track port located on the Town eclarations, limits on in-perspecting may be scheduled as	n. This provides neighbors of the project before it is one or after the meeting is ted using the Interactive on of Apex website at son gatherings, and social and held prior to a public eck all that apply):
	plication Type		Approving Authority
X	Rezoning (including Planned Unit Development)		Town Council
	Major Site Plan		Town Council (QJPH*)
	Special Use Permit		Town Council (QJPH*)
			Technical Review

The following is a description of the proposal (also see attached map(s) and/or plan sheet(s)): This project involves the proposed rezoning of parcels zoned RR and R-80W to be zoned to PUD-CZ.

The proposed development is intended to be a residential development with a mix of housing products.

(A concept plan will be posted on the project website the day of the meeting.)

Residential Master Subdivision Plan (excludes exempt subdivisions)

Estimated submittal date: 11.02.2020

Electronic Meeting invitation/call in

MEETING INFORMATION:

Property Owner(s) name(s): Goodwin, Edwin A; Hackney, Charles Leon Hackney, Judy G; & Hackney, Judy G

Applicant(s): WithersRavenel

Contact information (email/phone): bvega@withersravenel.com / 919.535.5212

Manaking Walanisha, Industrial August Industrial

info:

Meeting Website: https://withersravenel.com/meeting/hackney-tracts-rezoning-neighborhood-meeting/Call-In Option: 1-415-655-0001

Committee (staff)

Last Updated: March 25, 2020

Event number: 171 659 8744

Date of meeting**: 10.29.2020

Time of meeting**: 5:00pm - 7:00pm

MEETING AGENDA TIMES:

Welcome: 5:00pm Project Presentation: 5:10pm Question & Answer: 6:30pm

^{*}Quasi-Judicial Public Hearing: The Town Council cannot discuss the project prior to the public hearing.

^{**}Meetings shall occur between 5:00 p.m.-9:00 p.m. on a Monday through Thursday (excluding Town recognized holidays). If you have questions about the general process for this application, please contact the Planning Department at 919-249-3426. You may also find information about the Apex Planning Department and on-going planning efforts at http://www.apexnc.org/180/Planning.

PROJECT CONTACT INFORMATION

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Development Contacts:							
Project Name: Hackney Tracts				Zoning: RR & R-80W (Prop. PUD-CZ)			
Location: 2600, 2500, & 0 Olive Chapel Road							
Property PIN(s): 0721492629, 0722406699, & 0722411102	Acreage	/Square	Feet:	79.79 ac. / 3,475,652 sq. ft.			
Property Owner: Goodwin, Edwin A; Hackney, Charles Leon Hackney, Judy G; & Hackney, Judy G							
Address: 2505 Olive Chapel Road							
City: Apex		State:	NC	Zip: 27502			
Phone: Email							
Developer: Glenda S. Toppe and Associat	es						
Address: 4139 Gardenlake Drive							
City: Raleigh	State:	NC		zip: 27612			
Phone: Fax:			Emai	il:			
Engineer: WithersRavenel							
Address: 137 S. Wilmington Street, Suit	e 200						
City: Raleigh		State:	NC	Zip: 27601			
Phone: 919.535.5212 Fax:			Emai	il: bvega@withersravenel.com			
Builder (if known):							
Address:							
City:		State:		Zip:			
Phone: Fax:			Emai	il:			

Please note that Town staff will not have complete information about a proposed development until the application is submitted for review. If you have a question about Town development standards and how they relate to the proposed development, please contact the appropriate staff person listed below.

Town of Apex Department Contacts					
Planning Department Main Number	(919) 249-3426				
(Provide development name or location to be routed to correct planner)	(919) 249-3426				
Parks, Recreation & Cultural Resources Department					
Angela Reincke, Parks Planner	(919) 249-7468				
Public Works - Transportation					
Russell Dalton, Senior Transportation Engineer	(919) 249-3358				
Water Resources Department					
Jessica Bolin, Senior Engineer (Stormwater, Sedimentation & Erosion Control)	(919) 249-3537				
Stan Fortier, Senior Engineer (Stormwater, Sedimentation & Erosion Control)	(919) 249-1166				
James Gregg, Utility Engineer (Water & Sewer)	(919) 249-3324				
Electric Utilities Division					
Rodney Smith, Electric Technical Services Manager	(919) 249-3342				

Last Updated: March 25, 2020

Providing Input to Town Council:

Each Town Council meeting agenda includes a Public Forum time when anyone is permitted to speak for three (3) minutes on any topic with the exception of items listed as Public Hearings for that meeting. The Town Council meets on the 1st and 3rd Tuesdays of each month at 6:00 p.m. (except for holidays, see schedule of meetings at http://www.apexnc.org/838/Agendas-Minutes). You may also contact Town Council by e-mail at AllCouncil@apexnc.org.

Private Agreements and Easement Negotiation:

The Town of Apex cannot enforce private agreements between developers and neighbors and is not a party to the easement and right-of-way negotiation that occurs between developers and neighboring property owners for easements or rights-of-way that are necessary to build the project.

It is recommended that all private agreements be made in writing and that if a property owner feels it necessary, they should obtain private legal counsel in order to protect their interests in both private agreements and during easement negotiations. The only conditions that the Town of Apex can enforce are those conditions that are made a part of the conditional zoning of the property by agreement of the developer and the Town.

As an example, if a developer offers to build a fence for a neighbor to mitigate some impact, the Town can only enforce the construction of the fence if the fence becomes a condition of the rezoning. This would occur by the developer offering the condition as part of their conditional zoning application package or at the Town Council public hearing on the conditional zoning and the Town accepting it as a condition. Private agreements regarding a fence being constructed will not be enforced by the Town.

To request that any agreement with a developer is made a part of the conditional zoning at the time of approval, you may ask at the Town Council public hearing if the agreement is included in the conditions. If it is not, you may request that the Town Council not approve the rezoning without the agreement being included in the conditions (note that it is up to Town Council whether to approve or deny the rezoning but they cannot impose conditions that the applicant does not agree to add). The developer's proposed conditions can be viewed any time after a rezoning is submitted on the Interactive Development Map at: http://apexnc.maps.arcgis.com/apps/OnePane/basicviewer/index.html?appid=fa9ba2017b784030b15ef4d a27d9e795

Documentation:

Neighbors to a requested new development and/or rezoning are strongly encouraged to fully document (such as through dated photographs) the condition of their property before any work is initiated for the new development. Stormwater controls installed on developed property are not designed to and will likely not remove 100% of the soil particles transported by stormwater runoff. As a result, creeks and ponds could become cloudy for a period of time after rain events.

Last Updated: March 25, 2020

COMMON CONSTRUCTION ISSUES & WHO TO CALL

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Noise & Hours of Construction: Non-Emergency Police

Noise from tree removal, grading, excavating, paving, and building structures is a routine part of the construction process. The Town generally limits construction hours from 7:00 a.m. to 8:30 p.m. so that there are quiet times even during the construction process. Note that construction outside of these hours is allowed with special permission from the Town when it makes more sense to have the construction occur at night, often to avoid traffic issues. In addition, the Town limits hours of blasting rock to Monday through Friday from 8:00 a.m. to 5:00 p.m. Report violations of construction hours and other noise complaints to the Non-Emergency Police phone number at 919-362-8661.

James Misciagno **Construction Traffic:**

Construction truck traffic will be heavy throughout the development process, including but not limited to removal of trees from site, loads of dirt coming in and/or out of the site, construction materials such as brick and wood brought to the site, asphalt and concrete trucks come in to pave, etc. The Town requires a construction entrance that is graveled to try to prevent as much dirt from leaving the site as possible. If dirt does get into the road, the Town can require they clean the street (see "Dirt in the Road" below).

Road Damage & Traffic Control: Water Resources – Infrastructure Inspections 919-362-8166

There can be issues with roadway damage, roadway improvements, and traffic control. Potholes, rutting, inadequate lanes/signing/striping, poor traffic control, blocked sidewalks/paths are all common issues that should be reported to Water Resources - Infrastructure Inspections at 919-249-3427. The Town will get NCDOT involved if needed.

Parking Violations:

Non-Emergency Police

Unless a neighbor gives permission, there should be no construction parking in neighbors' driveways or on their property. Note that parking in the right-of-way is allowed, but Town regulations prohibit parking within 15 feet of driveways so as not to block sight triangles. Trespassing and parking complaints should be reported to the Non-Emergency Police phone number at 919-362-8661.

Dirt in the Road:

James Misciagno

Sediment (dirt) and mud gets into the existing roads due to rain events and/or vehicle traffic. These incidents should be reported to James Misciagno. He will coordinate the cleaning of the roadways with the developer.

Dirt on Properties or in Streams:

James Misciagno Danny Smith

919-372-7470

Danny.Smith@ncdenr.gov

Sediment (dirt) can leave the site and get onto adjacent properties or into streams and stream buffers; it is typically transported off-site by rain events. These incidents should be reported to James Misciagno at 919-372-7470 so that he can coordinate the appropriate repairs with the developer. Impacts to the streams and stream buffers should also be reported to Danny Smith (danny.smith@ncdenr.gov) with the State.

James Misciagno

919-372-7470

During dry weather dust often becomes a problem blowing into existing neighborhoods or roadways. These incidents should be reported to James Misciagno at 919-372-7470 so that he can coordinate the use of water trucks onsite with the grading contractor to help control the dust.

James Misciagno

919-372-7470

Excessive garbage and construction debris can blow around on a site or even off of the site. These incidents should be reported to James Misciagno at 919-372-7470. He will coordinate the cleanup and trash collection with the developer/home builder.

Temporary Sediment Basins:

James Misciagno

919-372-7470

Temporary sediment basins during construction (prior to the conversion to the final stormwater pond) are often quite unattractive. Concerns should be reported to James Misciagno at 919-372-7470 so that he can coordinate the cleaning and/or mowing of the slopes and bottom of the pond with the developer.

Stormwater Control Measures:

Jessica Bolin

Post-construction concerns related to Stormwater Control Measures (typically a stormwater pond) such as conversion and long-term maintenance should be reported to Mike Deaton at 919-249-3413.

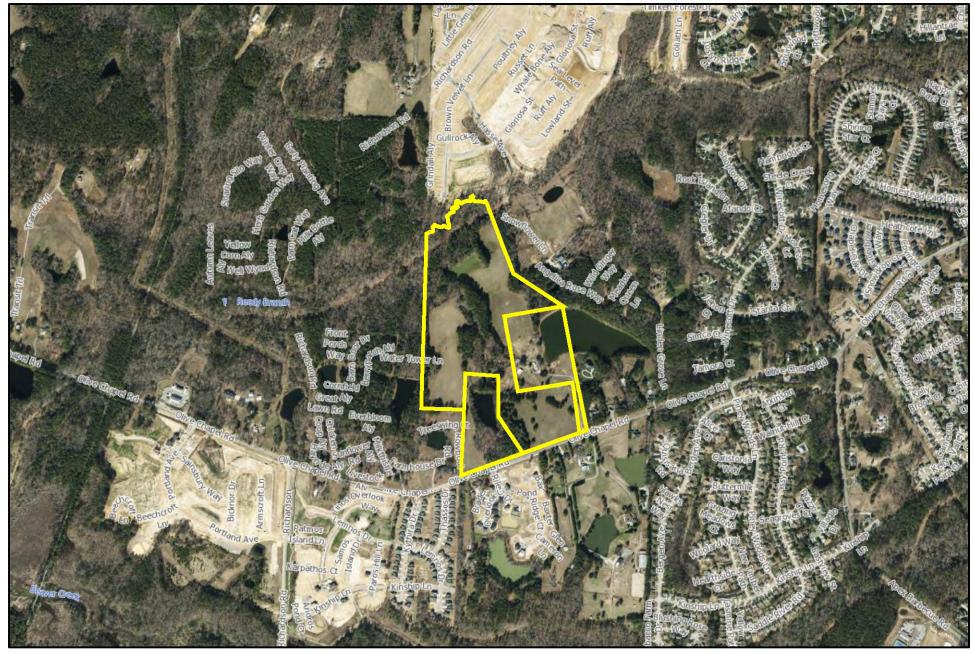
Electric Utility Installation:

Rodney Smith

919-249-3342

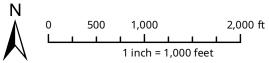
Last Updated: March 25, 2020

Concerns with electric utility installation can be addressed by the Apex Electric Utilities Department. Contact Rodney Smith at 919-249-3342.



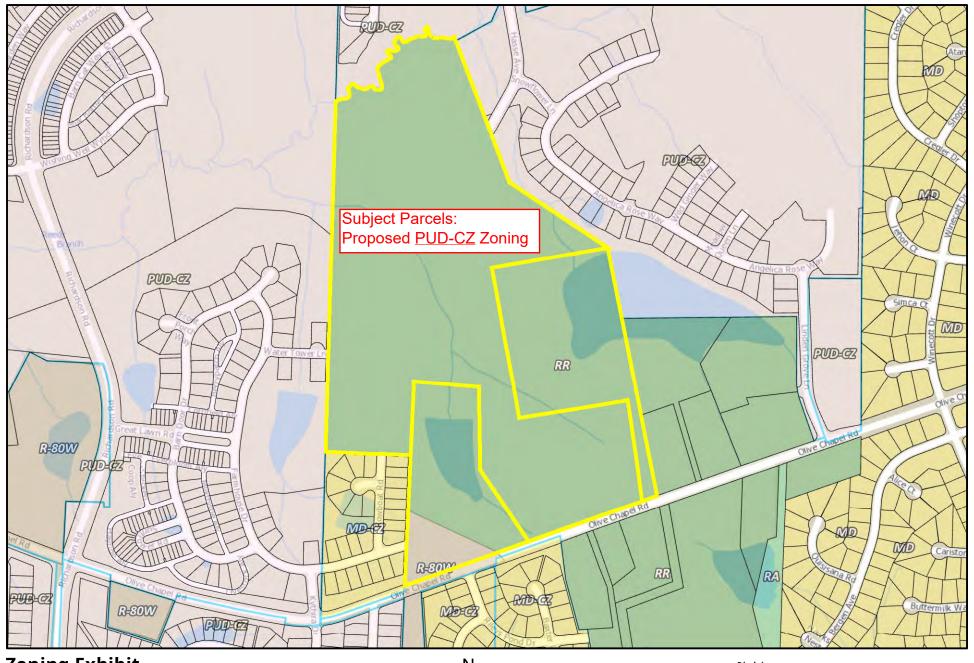
Vicinity Exhibit





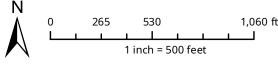
<u>Disclaim er</u>

iMaps makes every effort to produce and publish the most current and accurate information possible. However, the maps are produced for information purposes, and are **NOT** surveys. No warranties, expressed or implied ,are provided for the data therein, its use, or its interpretation.



Zoning Exhibit





<u>Disclaim er</u>

1,060 ft

iMaps makes every effort to produce and publish the most current and accurate information possible.

However, the maps are produced for information purposes, and are NOT surveys. No warranties, expressed or implied , are provided for the data therein, its use, or its interpretation.



Meeting Date: October 29th, 2020 Meeting Time: 5:00pm - 7:00pm

Attendance Count	First Name	Last Name	Affiliation	Email
1	Brendie	Vega	Project Team	
2	Nick	Antrilli	Project Team	
3	Bryant	Inge	Project Team	
4	Glenda	Торре	Project Team	
5	Daniel	Rauh	Project Team	
6	Jaime	Hackney	Neighbor	
7	Cynthia	Ball	Neighbor	
8	Jaime	Hackney	Neighbor	
9	Cynthia	Ball	Neighbor	
10	maysam	aljader	Neighbor	
11	Andrew	Suriano	Neighbor	
12	Randy	King	Neighbor	
13	Chris	Mock	Neighbor	
14	Melinda	Dyk	Neighbor	
15	David	Aspnes	Neighbor	
16	Ryan	Brumfield	Neighbor	
17	Thomas	Ball	Neighbor	
18	Kenny	Choi	Neighbor	
19	Beverly	Rubin	Neighbor	
20	Tracy	Giuliani	Neighbor	
21	Wes	Pitman	Neighbor	
22	Shruti	Uchil	Neighbor	
23	Maureen	Schmitt	Neighbor	
24	Fabrice	Lacoste	Neighbor	
25	Steven	McNally	Neighbor	
26	Felipe	Ledesma	Neighbor	
27	Pieter	de Ridder	Neighbor	



Hackney PUD Rezoning: Virtual Neighborhood Notification Meeting

October 29, 2020 5:00pm-7:00pm

Project Representatives:

- Brendie Vega
- Glenda Toppe
- Daniel Rauh
- Nick Antrilli
- Bryant Inge

Meeting Slides:

- Welcome
- Vicinity Map
- Jurisdiction Map
- Current Zoning
- Floodplains Map
- Future Land Use Map
- Future Transportation Maps
- Conceptual Layout
- Schedule of Project

Neighbor Questions:

Q: What does the MD-CZ zoning mean? Our house is within that so what does that mean for us?

A: Medium Density Conditional Zoning. Medium density residential zoning with specific conditions imparted on the land that are determined during the rezoning process. Your land is subject to the zoning conditions of the land.

Q: We live at 2800 Treeswing - what is happening with the piece of land behind us with the deer stand?

A: That is part of the rezoning parcels. Specific site features are not yet determined as we are early in the rezoning process.

Q: Is there any possibility the existing pond south of Hasse would be filled in?

A: It is too early to tell what features will be kept or modified on the site. There is no current intent to drain it.



Q: On the conceptual layout slide, are the lighter blue areas water retention ponds? If so, is it a city requirement that they would need to be fenced in for child safety?

A: There is no municipal requirement to fence these features.

Q: Will the 2 existing large ponds be accessible to the community?

A: It is too early to tell what the programming of the internal amenities will look like. Land will probably be turned over to the HOA.

Q: Where will the entrances be located on Olive Chapel Road? Will the road be widened?

A: The developer will be required to install ½ of the road widening as designated on the future transportation plan. The conceptual drawings show an early proposed entrance.

Q: When will a detailed road network and home layout be developed and available for review?

A: This will be up to the developers. If it happens soon, it would come out around the beginning of the year.

Q: (There were numerous questions concerning the internal connections within and throughout the site)

A: Using the Town of Apex Future Thoroughfare Map, we explained that the connections into and out of the site will be required to follow the transportation map. The conceptual layout demonstrated a conceptual internal roadway connection through the site.

Q: (There were multiple questions concerning when the project would begin.)

A: We expect the project to begin within 12 – 15 months, although this is dependent on many factors.

Q: I have a few questions. 1) is a builder planned yet. 2) We do not have a road ext sign on Water Tower Lane. We were told when we bought unless it was Lennar, other builders were not required to connect.

A: No builder planned yet. The developer generally will not have an impact on whether or not a road extension is provided. That is generally guided by the future transportation maps of the municipality.



Q: Will bordering communities have a say on the design of the community (location of Townhomes vs single family)?

A: Please reach out to us with your input and we will pass it along to the developer once one is identified.

Q: What are the construction hours in Apex.

A: 7:00am to 7:00pm during the normal work week. Weekends and holidays vary.

Q: What is going to happen to the trees in the lot?

A: There are no detailed plans for the site yet. There are certain environmental protection areas in place where trees will remain.

Q: There are some very old ok trees bordering hackney and Lennar preservation. Literally on the border. Greater than 50" diameter. Will these be preserved?

A: We are required to do a tree survey and protect trees above a certain caliper. Additionally, if the trees are located near the site border, then they should be protected.

Q: What elementary school would serve this community?

A: (Answered by another Neighbor) It's currently Olive Chapel (capped) followed by Salem (capped) and then Turner Creek.

Q: (There was a question concerning the western pond near Rowboat Road and future development.)

A: There is probably not going to be road or home development in that area, although there are no finalized plans at this time.

Q: Would perimeter buffers be maintained or would the development be opened up?

A: Buffers are required between neighborhoods.



Neighborhood Comments:

C: Don't fill the ponds.

C: Like to Like: Single-family should be designed adjacent to existing single-family homes.

C: Support townhomes along proposed main thoroughfare.

AFFIDAVIT OF COLJUCTING AN ELECTRONIC REIGHBORHOOD MEETING AND ISSUES/RESPONSES SUBMITTAL

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Brendie Vega
I,, do hereby declare as follows:
Print Name
 I have conducted an Electronic Neighborhood Meeting for the proposed Rezoning, Major Site Plan Residential Master Subdivision Plan, or Special Use Permit in accordance with UDO Sec. 2.2.7 Neighborhood Meeting.
 The meeting invitations were mailed to the Apex Planning Department, all property owners within 300 feet of the subject property and any neighborhood association that represents citizens in the area via first class mail a minimum of 10 days in advance of the Electronic Neighborhood Meeting.
3. The meeting was conducted via WebEx (indicate format or
meeting) on 10/29/2020 (date) from 5 pm (start time) to 7 pm (end time)
 I have included the mailing list, meeting invitation, attendance sheet issue/response summary, and zoning map/reduced plans with the application.
5. I have prepared these materials in good faith and to the best of my ability.
11/02/2020 By: By:
STATE OF NORTH CAROLINA COUNTY OF WAKE
Sworn and subscribed before me, <u>Jeri Chastain</u> , a Notary Public for the above State and
County, on this the
SEAL Gue Chastain Pederson Teri Chastain Pederson
JERI CHASTAIN PEDERSON Notary Public Print Name
Wake County, North Carolina My Commission Expires March 10, 2024 My Commission Expires: 03/10/2024

PD PLAN

Hackney Planned Unit Development

APEX, NORTH CAROLINA

APPLICANT

WithersRavenel
137 S Wilmington Street Suite 200
Raleigh, NC 27601

Date: March 24, 2021



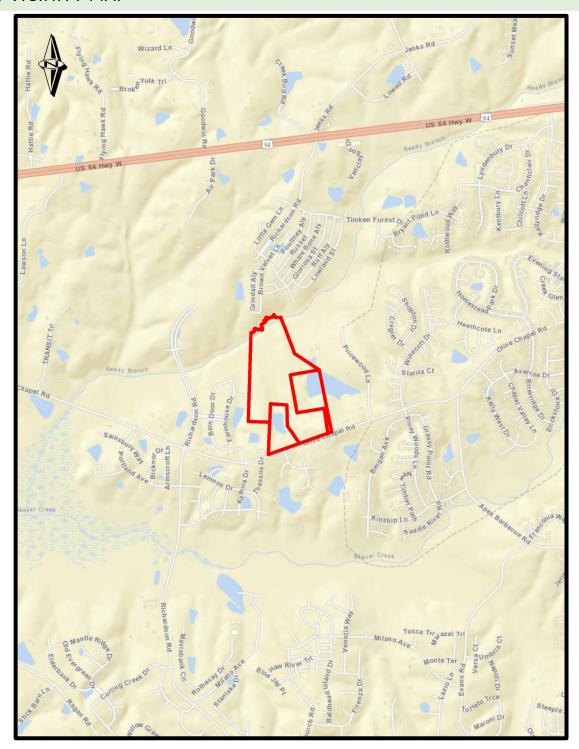
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1.0 VICINITY MAP



Project Parcels: Not to Scale

137 S Wilmington Street, Suite 200 | Raleigh, NC 27601
t: 919.469.3340 | f: 919.467.6008 | www.withersravenel.com | License No. C-0832
Asheville | Cary | Greensboro | Pittsboro | Raleigh | Wilmington



2.0 PROJECT DATA

Name of Project	Hackney Planned Unit Development		
PIN(s)	0721492629 0722406699 0722411102		
Preparer/Owner Information	Prepared by Owners	WithersRavenel 137 S. Wilmington Street, Suite 200 Raleigh, NC 27601 Phone: 919.469.3340 Fax: 919.467.6008 Email: Brendie Vega, AICP, CNU-A bvega@withersravenel.com Bryant Inge, PE binge@withersravenel.com Hackney, Charles Leon Hackney, Judy G Goodwin, Edwin A 2505 Olive Chapel Rd Apex, NC 27502-6788	
Current Zoning Designation	Rural Residential (RR) & Residential- 80W (R-80W)		
Proposed Zoning Designation	Planned Unit Development (PUD-CZ)		
Current 2045 Land Use Map Designation	Medium Density Residential		
Proposed 2045 Land Use Map Designation	No Proposed Change (Medium Density		
Area of Tracts (ac.)	10.01, 11.91, & 57.87 (73.64 ac. total)		



3.0 PROPOSED LIST OF USES

The Rezoned Lands may be used for, and only for, the uses listed immediately below. The permitted uses are subject to the limitations and regulations stated in the UDO and any additional limitations or regulations stated below. For convenience, some relevant sections of the UDO may be referenced; such references do not imply that other sections of the UDO do not apply.

Residential

- Single-Family
- Accessory Dwelling Unit
- Townhouse

Non-Residential

- Utility, Minor
- Greenway
- Park, Active
- Park, Passive

4.0 PURPOSE STATEMENT

The Hackney Planned Unit Development Conceptual Layout has been designed in order to help establish appropriately sized residential opportunities along Olive Chapel Road. Development is intended to reflect the neighboring residential communities in both density and product. This residential development philosophy is in line with the 2045 Apex Future Land Use Plan designation of "Medium Density Residential". The site will provide a mixture of amenities and strategic infrastructure connections for future residents to navigate the community.



5.0 PROPOSED DESIGN AND ARCHITECTURAL CONTROLS

Maximum Densities (du/Acre)	3.5 du/acre				
Maximum Height of Buildings	50 feet				
Setbacks: Single-Family	Front: 5' from façade 20' from garage to	Side: 5'	Rear: 10'		
	back of sidewalk	Corner Side: 8'			
Setbacks: Townhouse, Front loaded	Front: 10' from façade	Side: 5'	Rear: 10'		
	20' from garage to back of sidewalk	Building to Building: 10'			
Setbacks: Townhouse, Alley loaded	Front: 10' from façade	Side: 5'	Rear: 5'		
	,	Building to Building: 10'			
Amount and Percentage of Built Upon Area Allowed	70%				
Amount and Percentage of Proposed Built Upon Area (Max)	70%				

- 1. Vinyl siding is not permitted; however, vinyl windows, decorative elements and trim are permitted.
- 2. The roofline cannot be a single mass; it must be broken up horizontally and vertically between every unit.
- 3. Garage doors must have windows, decorative details or carriage-style adornments on them.
- 4. The rear and side elevations of the units that can be seen from the right-of-way shall have trim around the windows.



- 5. The visible side of a townhome on a corner lot facing the public street shall contain at least 2 decorative elements such as, but not limited to, the following elements:
- Windows
- Bay window
- Recessed window
- Decorative window
- Trim around the windows
- Wrap-around porch or side porch
- Two or more building materials
- Decorative brick/stone
- Decorative trim

- Decorative shake
- Decorative air vents on gables
- Decorative gable
- Decorative cornice
- Column
- Portico
- Balcony
- Dormer

6. The garage cannot protrude more than 1-foot from either the front façade or porch.

6.0 BUFFERS

Perimeter Buffers shall be designated as such:

North	100' Stream Buffer*
East	20' Type A Buffer
South	30' Type E Buffer**
West	20' Type A Buffer

^{*} In addition to the 100' riparian buffer on the north, an additional 100' buffer will be established. This additional 100' may include utilities, trails and other active or passive recreation.

^{**}A 30' Type B Buffer shall be provided if homes along Olive Chapel Road are not alley-loaded.



7.0 NATURAL RESOURCES AND ENVIRONMENT

Watershed

The Hackney Planned Unit Development is located within the Primary Watershed Protection Overlay District and is therefore subject to the requirements outlined in Section 6.1 of the Town of Apex Unified Development Ordinance.

Floodplain

The parcels that make up the Site do contain a small portion of FEMA designated 100-year floodplains near the site s northern termination according to FEMA FIRM Panel 3720072200J, effective 05/02/2006.

Resource Conservation

The Site is also subject to the Resource Conservation Area requirements outlined in the Town of Apex Unified Development Ordinance.

The PUD will meet the requirements of:

8.1.2.C.1 *Planned Developments*. The RCA for all planned developments shall be determined by the Town Council per Sec. 2.3.4.F.1.c and per Sec. 8.1.2.C.4, 5, 6, 7, or 10 as applicable.

8.1.2.C.4 Development located south and west of NC 540. All developments which do not meet the criteria of subsections 8.1.2.C.3 or 10 and which are located south and west of NC 540 shall provide buffers and RCA equal to or greater than 30% of the gross site acreage for single-family and townhome uses and 25% of the gross site acreage for multi-family, mixed-use, and non-residential uses.

Per UDO Section 7.2.5.B.8, if any mass grading is proposed in the single-family sections of the PUD, the following provision will apply to lot coverage area for single-family: An additional five percent (5%) Resource Conservation Area (RCA) shall be set aside. This requirement is added to the standard RCA percentage requirement found in Sec. 8.1.2.C Size of the RCA.



Tree Replanting

Existing deciduous trees greater than 18" in diameter (DBH), as identified in the tree survey, that are removed by site development shall be replaced by planting a 1.5" caliper native tree from the Town of Apex Design and Development Manual as a street tree or as other required landscaping. Excess required tree replacement will occur in common open space areas.

Clean Energy

Residential dwelling units will be provided with solar conduit to accommodate the future installation of solar panels.

If permitted by the electric company, the developer will install photo-voltaic solar panels on the pool house roof for connection to the electrical grid. The photo-voltaic system will be designed to produce 5 kilowatts. The system may be either owned or leased and will be turned over to the HOA.

Water Quality

Signs will be installed near SCMs in order to:

- 1. Reduce pet waste near SCM drainage areas.
- 2. Reduce fertilizer near SCM drainage areas.

Installation of Pet Waste Stations in common areas will occur within the neighborhood.

Planting and Landscaping

Install Warm Season grasses (Bermuda, Zoysia, etc) in lawn areas to reduce the need for irrigation and chemicals.

Install required Street Trees, Buffer and Re-Vegetation plantings that consist of a variety of native plant materials recognized by the New Hope Audubon Society or the NCSU manual for Landscaping for Wildlife with Native Plants as being bird and pollinator friendly; as allowed by the Town of Apex Design & Development Manual or approved by Apex Staff.



Specify pocket park plantings that are recognized by the NC Wildlife Federation as being Native Pollinator Plants as part of the Statewide Butterfly Highway initiative.

Include at least 4 native hardwood tree varieties in the proposed plantings, as allowed by the Apex Design and Development Manual.

Environmental Resources

The site will provide the following:

- 1. Purchase 20 bird houses from the New Hope Audubon Society (or other non-profit) and install in natural areas within the site.
- 2. Retain the 2 existing ponds if engineering studies confirm that the existing dams are structurally sound and meet regulatory requirements.

Historic Preservation

According to the North Carolina Historic Preservation Office s HPOWEB 2.0 Mapping application, there are no historic structures contained on the Site.

8.0 STORMWATER MANAGEMENT REQUIREMENTS

The parcels on which the development is proposed upon currently consist of a few existing structures, some cleared lands, and wooded lands. Two ponds exist on the parcels and drain to Reedy Branch Creek, eventually feeding into Jordan Lake. The proposed development plan will require stormwater management measures in accordance with Sections 6.1 and 7.5.7 in the Town of Apex Unified Development Ordinance. Stormwater captured on the site will be conveyed to proposed Stormwater Control Measures, which will be identified on plans during the major subdivision or site plan approval stage. Post-development peak runoff shall not exceed pre-development peak runoff for the 24-hour, 1-year and 10-year storm events in accordance with the Unified Development Ordinance. Treatment for the first 1-inch of runoff will be provided such that the removal of 85% Total Suspended Solids is achieved. All stormwater devices will meet the design requirements of NCDENR and the Town of Apex.



9.0 PARKS, RECREATION AND CULTURAL RESOURCES

The Apex Parks, Recreation and Cultural Resources Advisory Commission met on December 9, 2020 and unanimously recommended a fee-in-lieu of dedication with credit for construction of greenway which connects Sidepath along Hasse Ave to the west connecting to the Reedy Branch Greenway in Smith Farm. The fee rate will be set at the time of Town Council Review/ Approval and the credit for construction will be calculated prior to construction plan approval. Per the UDO Art 14, the greenway must be completed and accepted prior to 25% of the building permits for the project being issued.

10.0 PARKING AND LOADING

All parking provided on the Site will comply with the requirements outlined in Section 8.3 of the Town of Apex Unified Development Ordinance. Per 8.3.4(C) of the UDO, guest parking shall be designated within common areas and be distributed throughout residential projects. Striped on-street parking may be counted toward guest parking requirements. For Townhouse, guest parking shall be distributed so that there is at least one parking space within 200' of each townhouse lot.

11.0 SIGNAGE

All signage on the Site will comply with the requirements outlined in Section 8.7 of the Town of Apex Unified Development Ordinance.

12.0 PUBLIC FACILITIES REQUIREMENTS

All utilities shall meet the Town of Apex Master Utility Maps.

Sanitary Sewer Service

All on-site sanitary sewer lines will be extended to the property lines to allow future interconnectivity of properties. The design of the sanitary sewer will be according to the Town of Apex Engineering Standards and Specifications while accounting for downstream capacity and future upstream development. Sanitary Sewer easements will be established for public sewer outside of the Public R/W.



Gas

The Public Service Company of North Carolina (PSNC) will require a revenue analysis based on the proposed development in order to determine the applicable costs to the developer for installation of infrastructure.

Electric Service

The Site is in the service area of both the Town of Apex Electric Utilities and Progress Energy and the applicant will select the Town of Apex to serve as the electric provider.

Roadways

The Site will require an internal public roadway network and parking spaces. The onsite transportation circulation system shall be consistent with the Town of Apex Transportation Plan and the Town of Apex Standard Specifications and Standard Details and show required right-of-way widths and road sections.

Hasse Avenue will be constructed between Olive Chapel Road and its current terminus north of the project. Olive Chapel Road will be widened to include construction of a 100-foot eastbound left-turn lane with appropriate deceleration length and taper and a 100-foot westbound right-turn lane with appropriate deceleration length and taper subject to NCDOT review and approval. The Olive Chapel Road turn lane widening will be completed prior to platting Hasse Avenue access to Olive Chapel Road and the connection to Hasse Avenue north of the project will be completed prior to the last plat in the subdivision.

A 6-foot bike lane and 5-foot paved shoulder will be located on the north side of Olive Chapel Road per the bike/ped systems map.

Per the Long-Range Transportation Map, the following roadway sections apply to this development:

- Olive Chapel Road = 4-Lane with median, widening, 110' ROW, must provide 55' from centerline
- N/S = Future Major Collector, 60' ROW
- E/W = Future Local Connection, 50' ROW

There will be no private driveways permitted along Olive Chapel Road.



Alleys

Alleys may be proposed to vary from Town standards in order to accommodate water and sewer utilities, provided they maintain the same or greater width of pavement and right of way, subject to staff review and approval at the time of subdivision and construction plans.

Water Service

All on-site water lines will be designed according to Town of Apex Engineering Standards and Specifications.

Transit

According to the Apex 2045 Transportation Plan, there are no existing or proposed transit routes designated on or adjacent to the Site.

Pedestrian Facilities

The development plan will incorporate sidewalk infrastructure along Olive Chapel Road as well as the internal street network. A trail will serve as a connection from the western portion of the community to the Reedy Branch Greenway, thus in compliance with the future land use plan.

Sidewalks will be provided on both sides of all streets for single-family detached homes. There will be a 10-foot side path provided along minor collector roads as show on the bike/ped plan.

Prior to platting the 75th lot in the neighborhood, the Developer will extend a 5' sidewalk approximately 860 feet along the north side of Olive Chapel to the western limits of the Linden Subdivision. Developer will attempt to obtain the required right-of-way and/or easements for construction of this sidewalk from the adjacent property owners. If the required right-of-way and/or easements cannot be obtained by that time, a Fee-in-Lieu in the amount of 125% of the estimated cost of construction plus fair market value of the property to be acquired, shall be assessed. Any performance guarantee provided for this section of sidewalk shall be released upon acceptance of said fee-in-lieu by the Town.



13.0 PHASING PLAN

The Hackney Planned Unit Development will be constructed in phases according to economic considerations and infrastructure requirements.

Please note the following considerations for the phasing plan:

- 1. Access points are preliminary in nature and subject to Town of Apex and NCDOT review and approval.
- 2. Limits of land disturbance within each phase shall be determined at the master subdivision plan and site plan stages.
- 3. Public utilities shall be provided for each phase of development.

14.0 CONSISTENCY WITH 2045 LAND USE PLAN

The Apex 2045 Future Land Use Map depicts the future land use of the three parcels as Medium Density Residential. Medium Density Residential lands are described in the Land Use Plan as consisting of single-family homes, duplexes, and townhomes with densities between three (3) and seven (7) dwelling units per acre. It is intended to act as a transition between higher and lower residential densities. The maximum density proposed for the Hackney Planned Unit Development is four (4) dwelling units per acre.

The Hackney Planned Unit Development proposes medium density residential housing options appropriate to its proximity to the Olive Chapel Road thoroughfare and are consistent with uses found in the surrounding communities. The uses proposed for the site are directly in line with the uses stated in the 2045 Future Apex Land Use Plan thus the proposed rezoning is consistent with the Town's future plans for this area.

15.0 CONSISTENCY WITH UNIFIED DEVELOPMENT ORDINANCE

The proposed development is consistent with all applicable requirements of the Town of Apex Unified Development Ordinance.



16.0 ELEVATIONS

Elevations provided are representative of architecture, materials, and housing types. Final elevations submitted at Major Subdivision Plan will meet the requirements of the Architectural Controls in 5.0 of this PD Plan.

17.0 AFFORDABLE HOUSING

If the Town of Apex has a fund or other mechanism in place to receive donations to construct, subsidize, or participate in the development of affordable housing units (the "Fund"), the developer will contribute \$215 per lot to this Fund prior to the first residential Certificate of Occupancy. In the event the Fund has not been established by the Town of Apex, the money will be conveyed to a local non-profit working on affordable housing initiatives. The developer will work with the Town of Apex to identify a mutually acceptable local non-profit organization to receive these funds.

HACKNEY

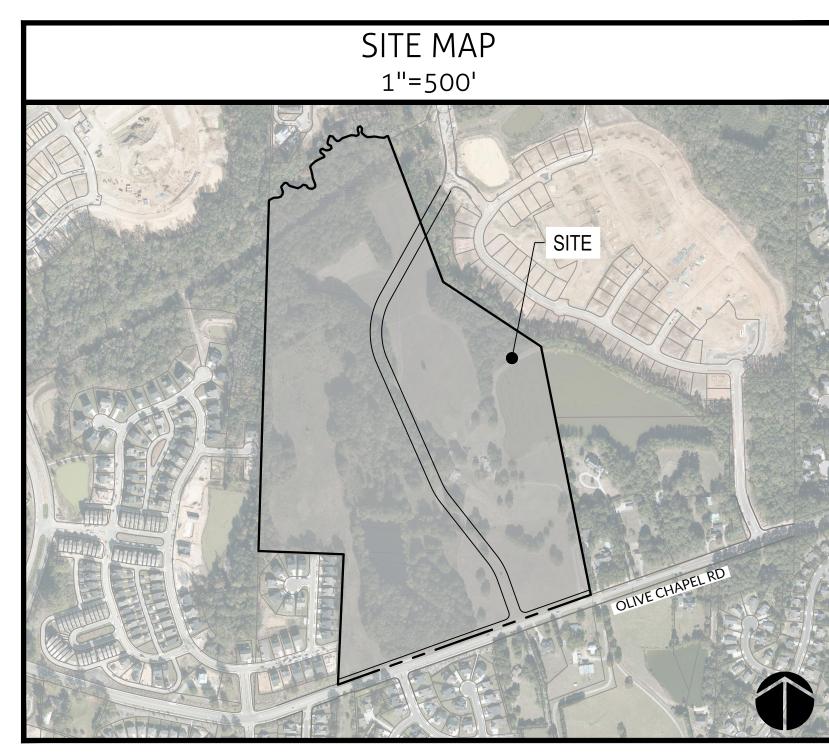
APEX, NORTH CAROLINA

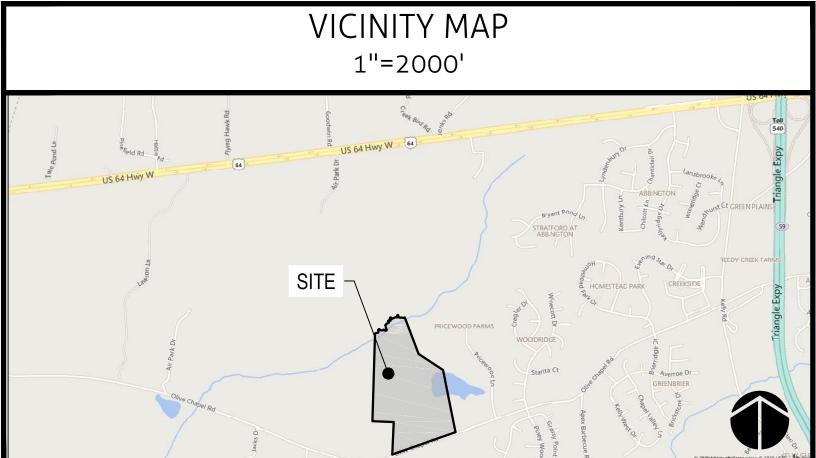
APRIL 20, 2021

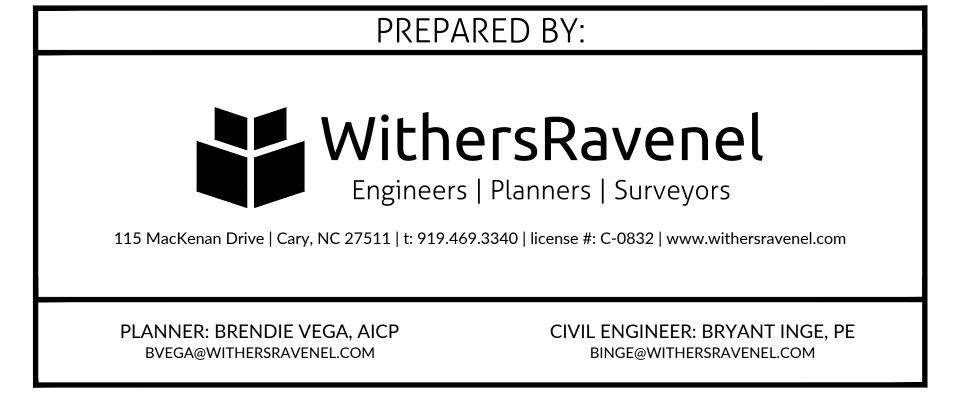
		SITE DATA	\				
2045 LAND USE PLAN DESIGNATION	CURRENT MEDIUM DENSITY RESIDENTIAL						
2040 LAND OOL I LAN DEGIGNATION	PROPOSED	PROPOSED NO CHANGE					
ZONING	CURRENT	CURRENT RURAL RESIDENTIAL (RR) (R-80W)					
2011110	PROPOSED	PROPOSED PLANNED UNIT DEVELOPMENT (PUD-CZ)					
	0722-41-1102	22-41-1102 51.725 ACRES					
AREA OF TRACTS IN PROPOSED PUD	0721-49-2629	10.007 ACRES	10.007 ACRES				
AREA OF TRACTO INTROF OSED FOR	0722-40-6699	11.916 ACRES	11.916 ACRES				
	TOTAL:	73.648 ACRES					
AREA DESIGNATED AS MIXED-USE ON 2045 LAND USE MAP	0 ACRES						
AREA OF MIXED-USE PROPERTY PROPOSED AS NON-RESIDENTIAL DEVELOPMENT	N/A	N/A					
PERCENT OF MIXED-USE PROPERTY PROPOSED AS NON-RESIDENTIAL DEVELOPMENT	N/A						
REQUESTED SEWER CAPACITY	TO BE DETERMINED						
MAXIMUM RESIDENTIAL DENSITY	3.5 DU/ACRE						
MAXIMUM BUILDING HEIGHT	50'-0"	50'-0"					
SETBACKS: SINGLE FAMILY	FRONT: 5 FT FROM FACADE 20 FT FROM GARAGE TO BACK OF SIDEWALK REAR: 10 FT SIDE: 5 FT CORNER SIDE: 8 FT				CORNER SIDE: 8 FT		
SETBACKS: TOWNHOUSE, FRONT LOADED	FRONT: 10 FT FROM FAC 20 FT FROM GAR	CADE RAGE TO BACK OF SIDEWALK	REAR: 10 FT	SIDE: 5 FT	BUILDING TO BUILDING: 10 FT		
SETBACKS: TOWNHOUSE, ALLEY LOADED	FRONT: 10 FT FROM FAC	ADE	REAR: 5 FT	SIDE: 5 FT	BUILDING TO BUILDING: 10 FT		
WATERSHED	JORDAN LAKE WATERSH	ED, PRIMARY WATERSHED PROTEC	TION OVERLAY				
HISTORIC STRUCTURES	N/A						
COMMUNITY AMENITIES	COMMUNITY GATHERING SPACE WITH BENCHES, TOT LOT						
	NORTH	100' RIPARIAN BUFFER *IN ADDITION TO THE 100' RIPARIAN BUFFER ON THE NORTH, AN ADDITIONAL 100' BUFFER WILL BE ESTABLISHED. THIS ADDITIONAL 100' MAY INCLUDE UTILITIES, TRAILS, AND OTHER ACTIVE OR PASSIVE RECREATION.					
SITE BUFFERS	EAST	20' TYPE A BUFFER					
	SOUTH	30' TYPE E BUFFER *A 30' TYPE B BUFFER SHALL BE PROVIDED IF HOMES ALONG OLIVE CHAPEL ROAD ARE NOT ALLEY LOADED.					
	WEST	20' TYPE A BUFFER					

ZONING CONDITIONS

- . THE APEX PARKS, RECREATION AND CULTURAL RESOURCES ADVISORY COMMISSION MET ON DECEMBER 9, 2020 AND UNANIMOUSLY RECOMMENDED A FEE-IN-LIEU OF DEDICATION WITH CREDIT FOR CONSTRUCTION OF GREENWAY WHICH CONNECTS SIDEPATH ALONG HASSE AVE TO THE WEST CONNECTING TO THE REEDY BRANCH GREENWAY IN SMITH FARM. THE FEE RATE WILL BE SET AT THE TIME OF TOWN COUNCIL REVIEW/APPROVAL AND THE CREDIT FOR CONSTRUCTION WILL BE CALCULATED PRIOR TO CONSTRUCTION PLAN APPROVAL PER THE UDO ART 14, THE GREENWAY MUST BE COMPLETED AND ACCEPTED PRIOR TO 25% OF THE BUILDING PERMITS FOR THE PROJECT BEING ISSUED.
- 2. HASSE AVENUE WILL BE CONSTRUCTED BETWEEN OLIVE CHAPEL ROAD AND ITS CURRENT TERMINUS NORTH OF THE PROJECT. OLIVE CHAPEL ROAD WILL BE WIDENED TO INCLUDE CONSTRUCTION OF A 100-FOOT EASTBOUND LEFT-TURN LANE WITH APPROPRIATE DECELERATION LENGTH AND TAPER AND A 100-FOOT WESTBOUND RIGHT-TURN LANE WITH APPROPRIATE DECELERATION LENGTH AND TAPER SUBJECT TO NCDOT REVIEW AND APPROVAL. THE OLIVE CHAPEL ROAD TURN LANE WIDENING WILL BE COMPLETED PRIOR TO PLATTING HASSE AVENUE ACCESS TO OLIVE CHAPEL ROAD AND THE CONNECTION TO HASSE AVENUE NORTH OF THE PROJECT WILL BE COMPLETED PRIOR TO THE LAST PLAT IN THE SUBDIVISION.
- 3. THERE WILL BE NO PRIVATE DRIVEWAYS ALONG OLIVE CHAPEL ROAD.
- 4. ALLEYS MAY BE PROPOSED TO VARY FROM TOWN STANDARDS IN ORDER TO ACCOMMODATE WATER AND SEWER UTILITIES, PROVIDED THEY MAINTAIN THE SAME OR GREATER WIDTH OF PAVEMENT AND RIGHT-OF-WAY, SUBJECT TO STAFF REVIEW AND APPROVAL AT THE TIME OF SUBDIVISION AND CONSTRUCTION PLANS.
- EXTEND A 5' SIDEWALK APPROXIMATELY 860 FEET ALONG THE NORTH SIDE OF OLIVE CHAPEL TO THE WESTERN LIMITS OF THE LINDEN SUBDIVISION. THE DEVELOPER WILL ATTEMPT TO OBTAIN THE REQUIRED RIGHT-OF-WAY AND/OR EASEMENTS FOR CONSTRUCTION OF THIS SIDEWALK FROM THE ADJACENT PROPERTY OWNERS. IF THE REQUIRED RIGHT-OF-WAY AND/OR EASEMENTS CANNOT BE OBTAINED BY THAT TIME, A FEE-IN-LIEU IN THE AMOUNT OF 125% OF THE ESTIMATED COST OF CONSTRUCTION PLUS FAIR MARKET VALUE OF THE PROPERTY TO BE ACQUIRED, SHALL BE ASSESSED. ANY PERFORMANCE GUARANTEE PROVIDED FOR THIS SECTION OF SIDEWALK SHALL BE RELEASED UPON ACCEPTANCE OF SAID FEE-IN-LIEU BY THE TOWN.







SHEET NUMBER SHEET TITLE

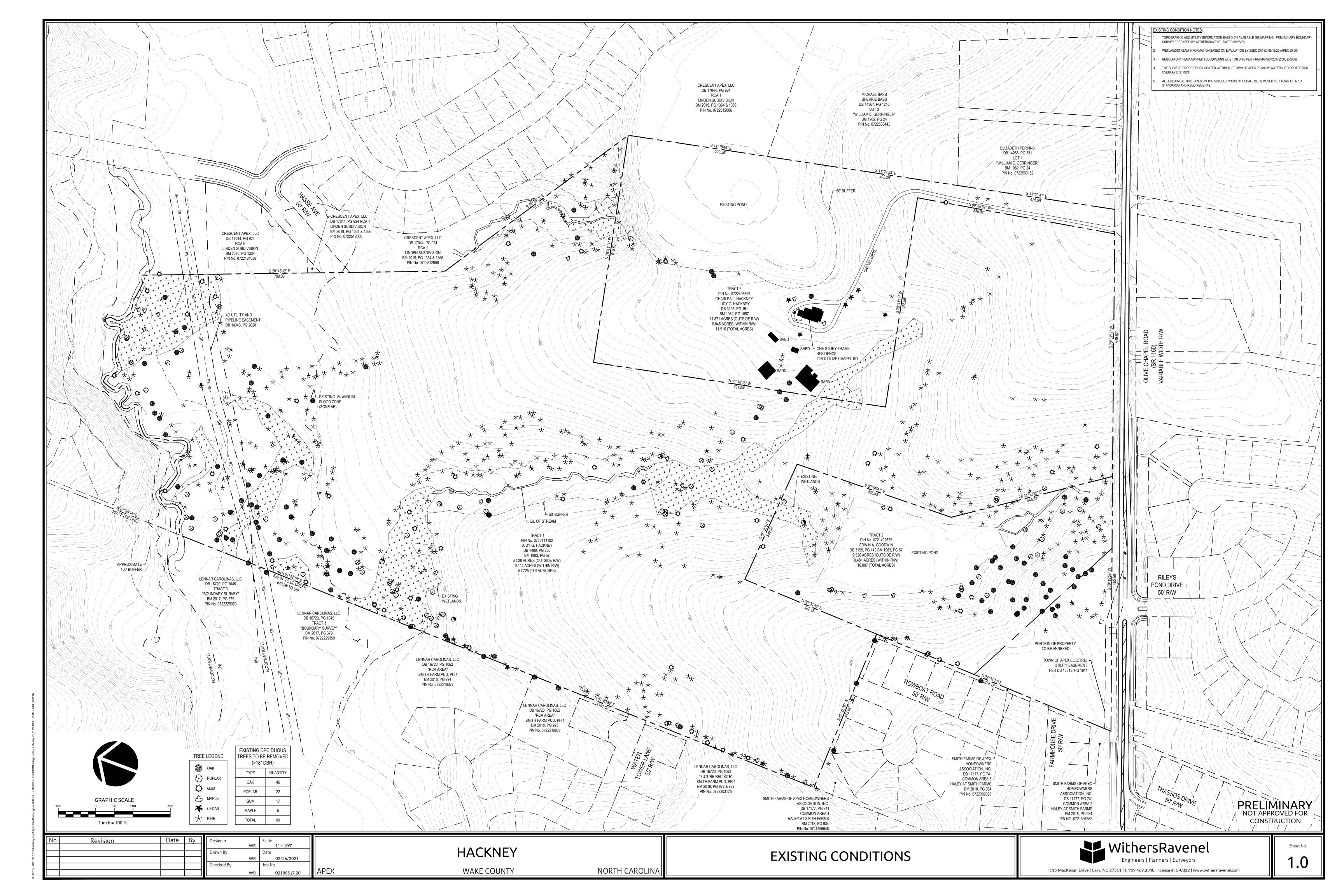
0.0 COVER

1.0 EXISTING CONDITIONS

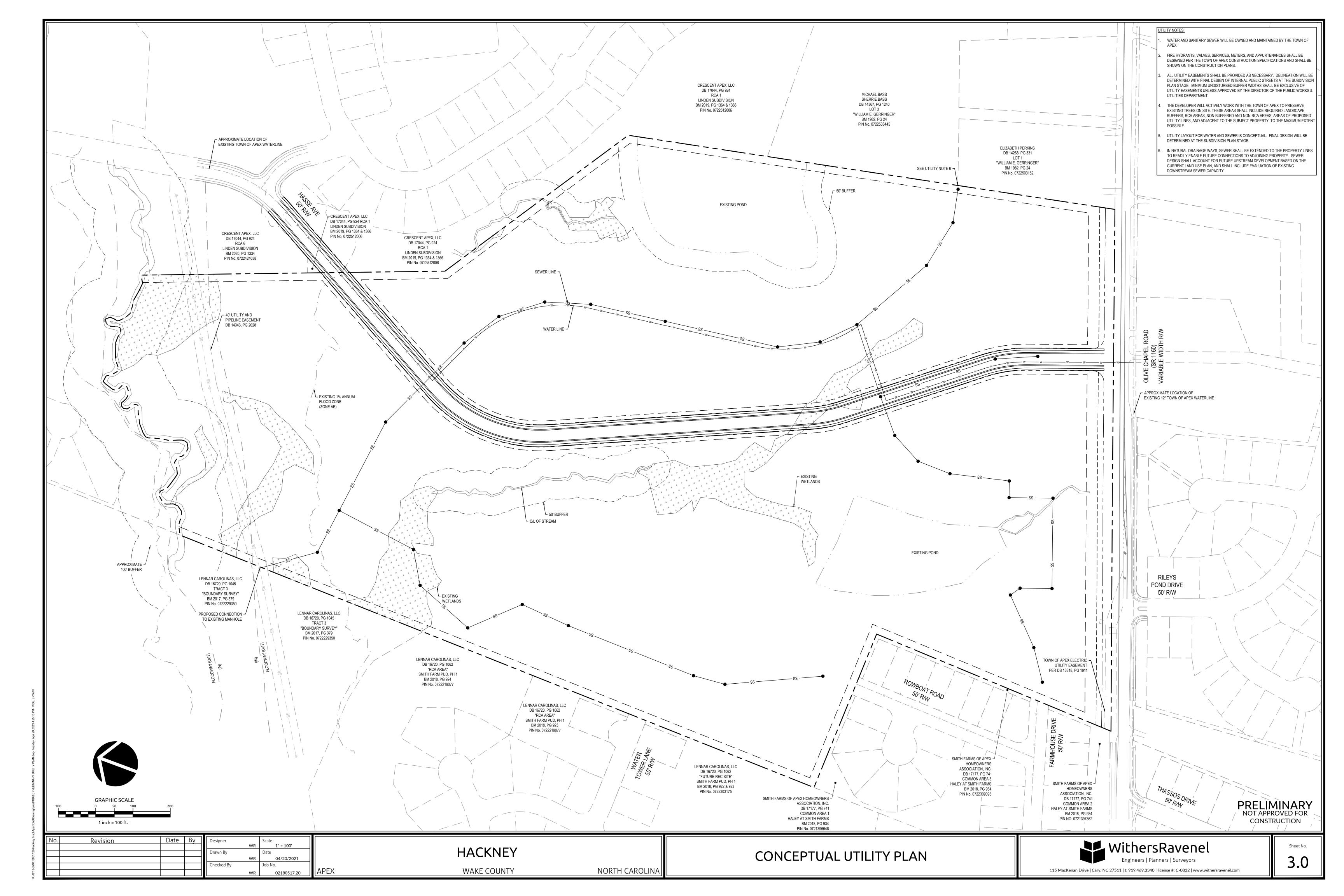
2.0 CONCEPTUAL LAYOUT PLAN

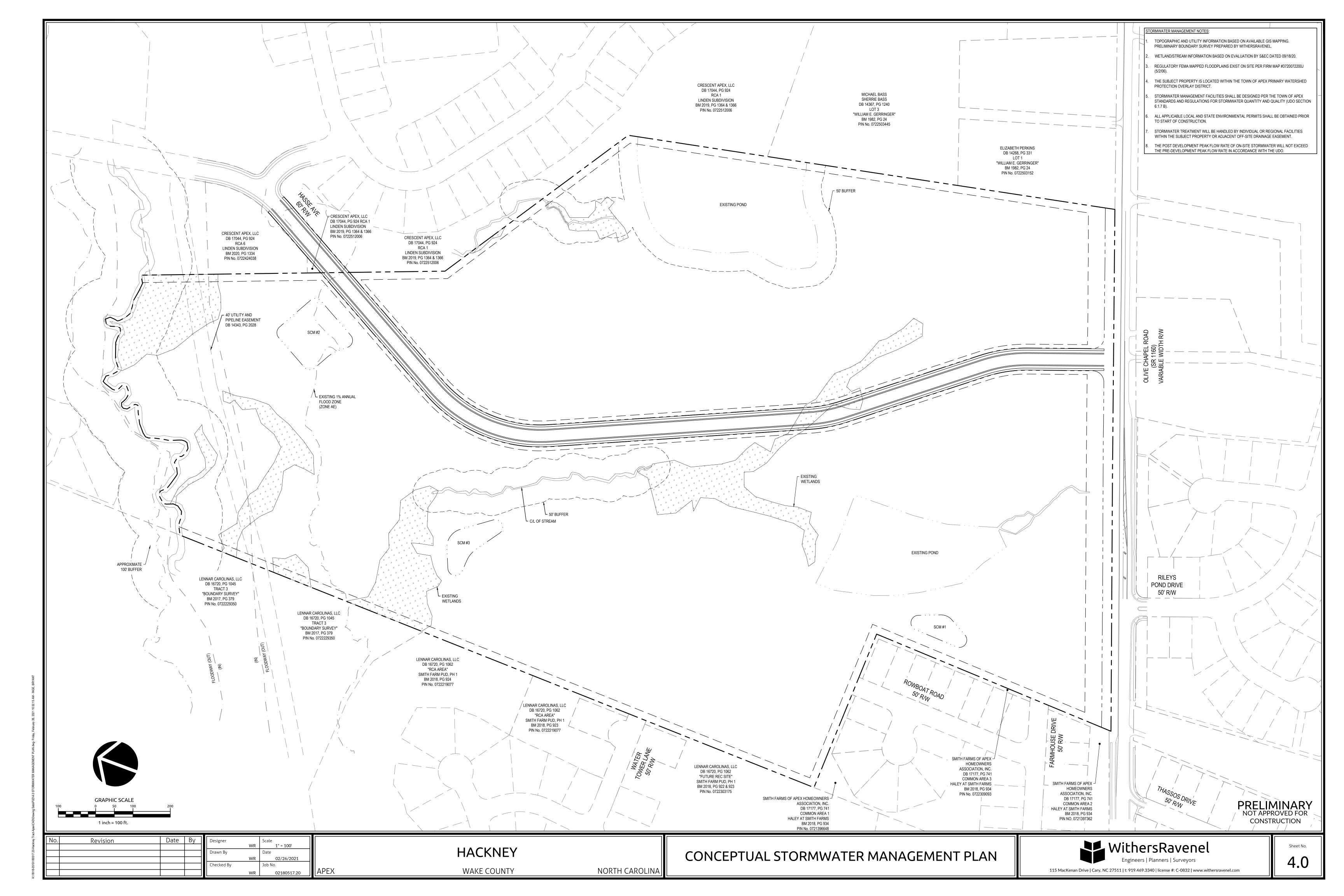
3.0 CONCEPTUAL UTILITY PLAN

4.0 CONCEPTUAL STORMWATER MANAGEMENT PLAN

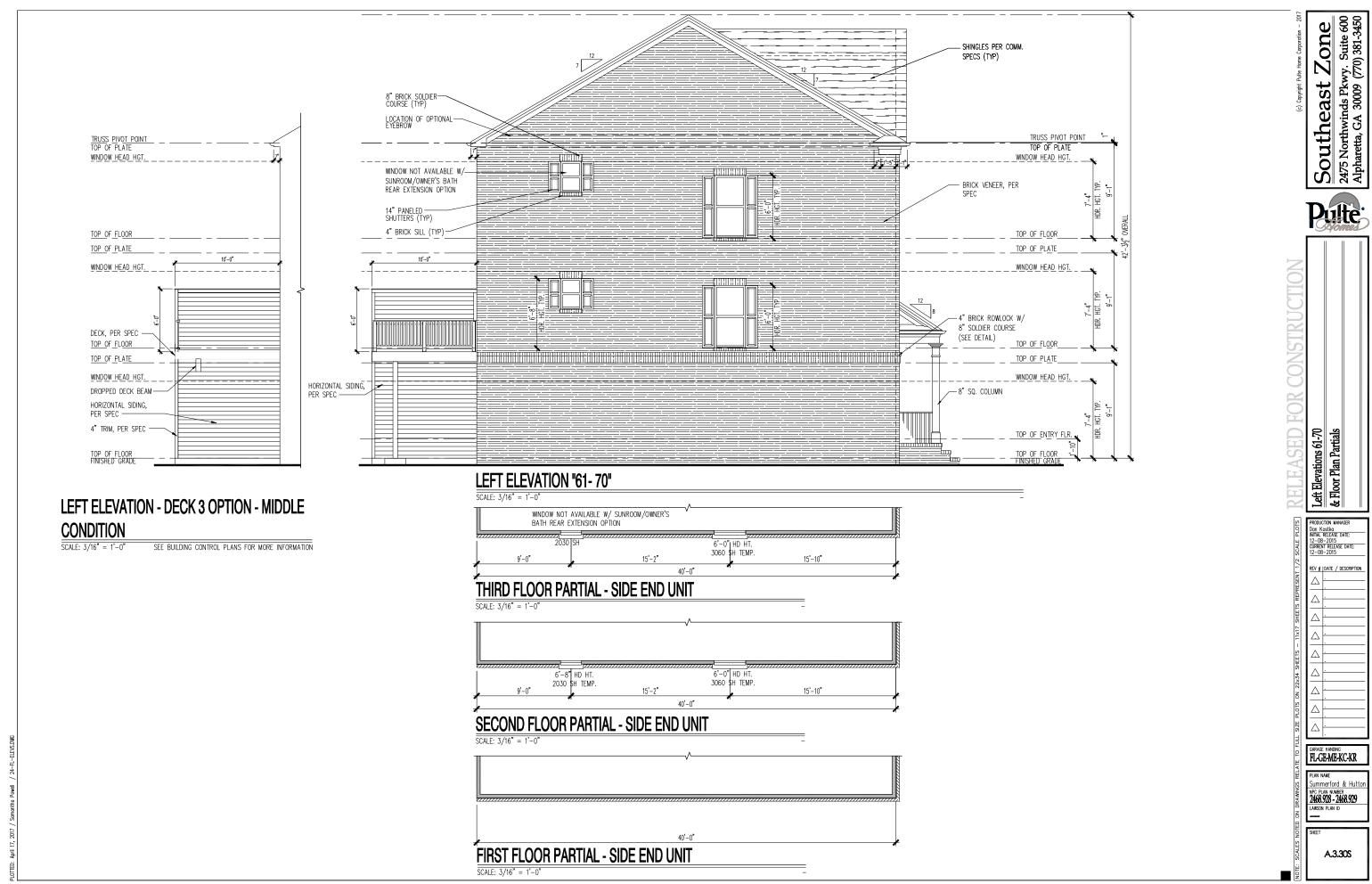








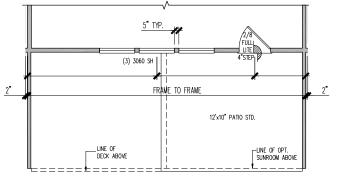






REAR ELEVATION

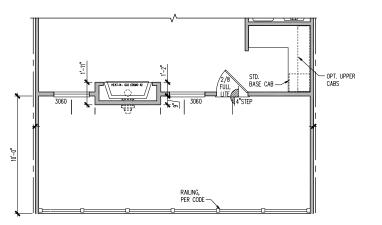
SCALE: 1/4" = 1'-0"



FIRST FLOOR PLAN
SCALE: 1/4" = 1'-0"

REAR ELEVATION - FIREPLACE OPTION w/ DECK 3

SCALE: 1/4" = 1'-0"



FIREPLACE OPTION AT GATHERING ROOM w/ DECK 3

E: 1/4" = 1'-0" SEE BASE PLANS FOR INFORMATION NOT SHOWN

Elevations are for illustrative purposes only; elevations submitted at MSP will be consistent with the architectural standards included in the PUD.

Southeast Zone 2475 Northwinds Pkwy. Suite 600 Alpharetta, GA 30009 (770) 381-3450

Pulte:

Real Elevations

Real Elevations

Real Elevations

Real Elevations

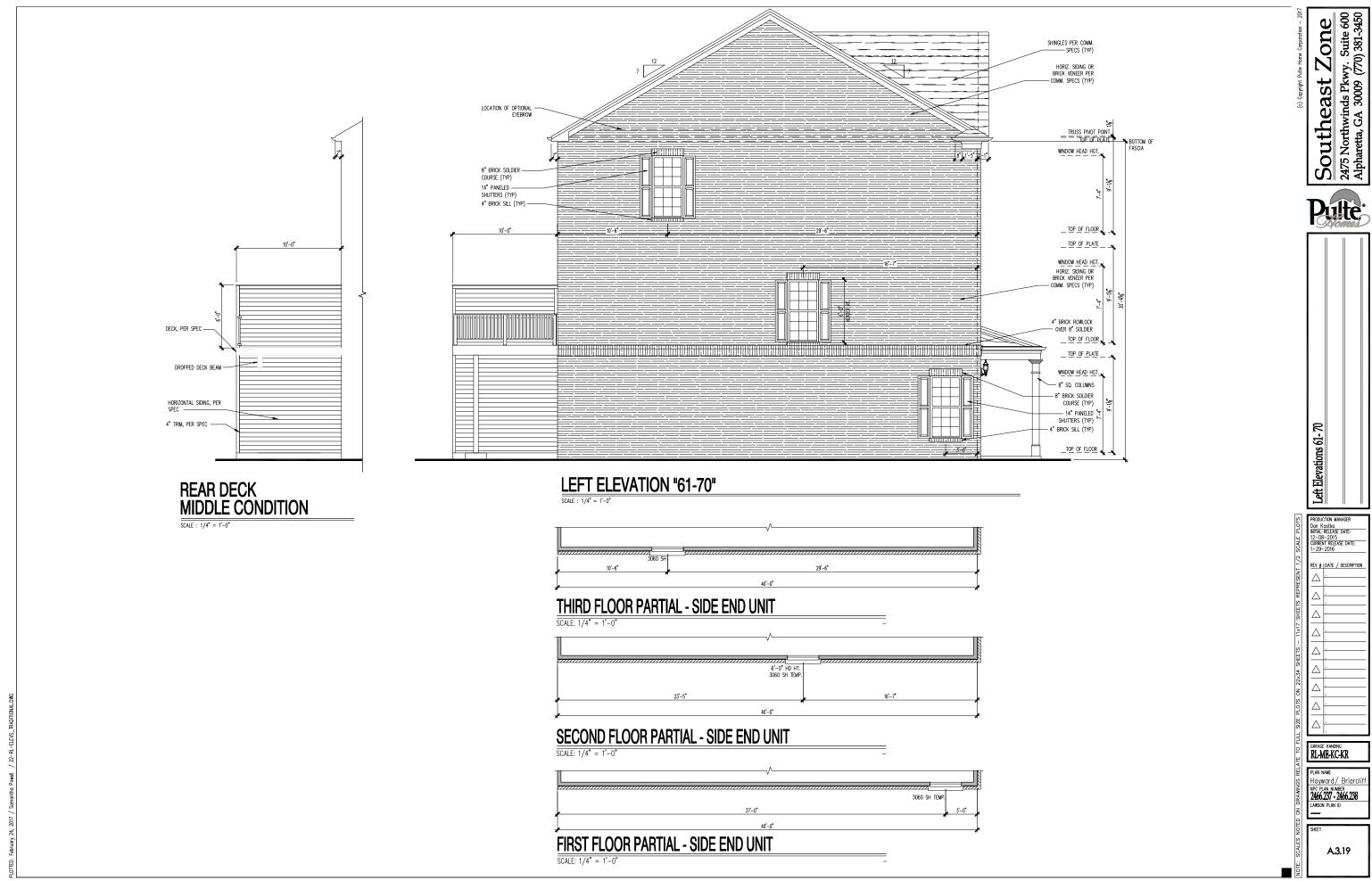
| PRODUCTION MANAGER | PRODUCT

GARAGE HANDING
FL-GE-ME-KC-KR

PLAN NAME
Summerford & Hutton
NPC PLAN NUMBER
2468,928 - 2468,929
LAWSON PLAN ID

A.3.60





ATTIC VENTILATION: (300 SQ FT ATTIC SPACE / 1 SQ FT VENTILATION)

W/ 40%-50% REQ. VENTS CREATER THAN OR EQUAL TO 3' ABOVE EAVE / CORNICE VENTS PER IRC R806.2 866 SQ FT UNDER ROOF ATTIC / 300 SQ FT / 1 SQ FT = 2.89 SQ FT VENTILATION 2.89 SQ FT x 50 % = 1.443SQ FT RIDGE, 2.89 SQ FT x 50 % = 1.443 SQ FT SOFFIT 2.89 SQ FT x 50 % = 1.44350 FT RIDGE, 2.89 SQ FT x 50 % = 1.44350 FT SOFFIT RIDGE VENT

1.443 SQ FT = 11.5 FEET OF RIDGE VENT

1.443 SQ FT = 4.2 BOX VENT(S)

1.50FFIT VENT

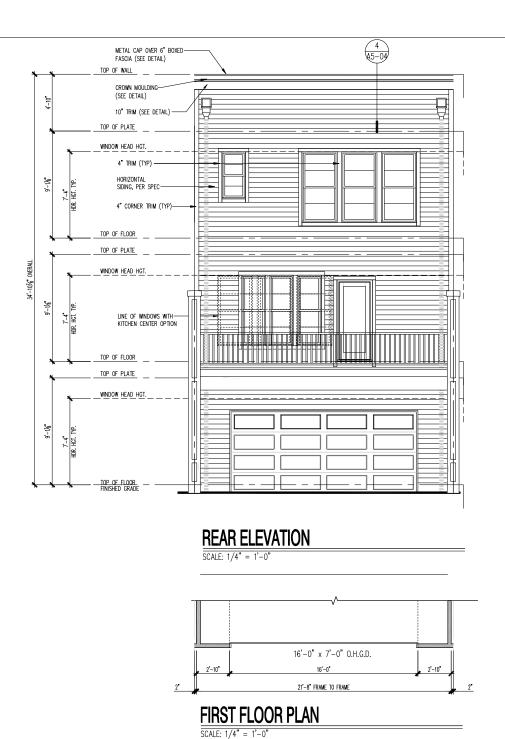
1.443 SQ FT = 23.1 FEET OF SOFFIT VENT

1.6082550 FT *CALCULATIONS REFLECT 50 % RIDGE AND 50 % SOFFIT VENTS ALLOWABLE PER SECTION IRC R806.2

RIDGE VENT 18 SQ IN = (.125 SQ FT) BOX VENT 50 SQ IN = (.3472 SQ FT) SOFFIT VENT 9 SQ IN = (.0625 SQ FT)

22'-4" FRAME TO FRAME ROOF AREA: "A" 947 SQ. FT. - 30" HIGH PARAPET WALL, PER SPEC SCUPPERS,
PER SPEC 22'-0" C/L TO C/L BV ВИ SLOPE 24"x32" ATTIC ACCESS (FLAT ROOF) -SCUPPERS, PER SPEC 21'-8" FRAME TO FRAME

ROOF PLAN



Rear Elevation 21-25 & Roof Plan PRODUCTION MANAGER
DON KOSTKO
INITIAL RELEASE DATE:
12-08-2015
CURRENT RELEASE DATE:
1-29-2016 REV # DATE / DESCRIPTION

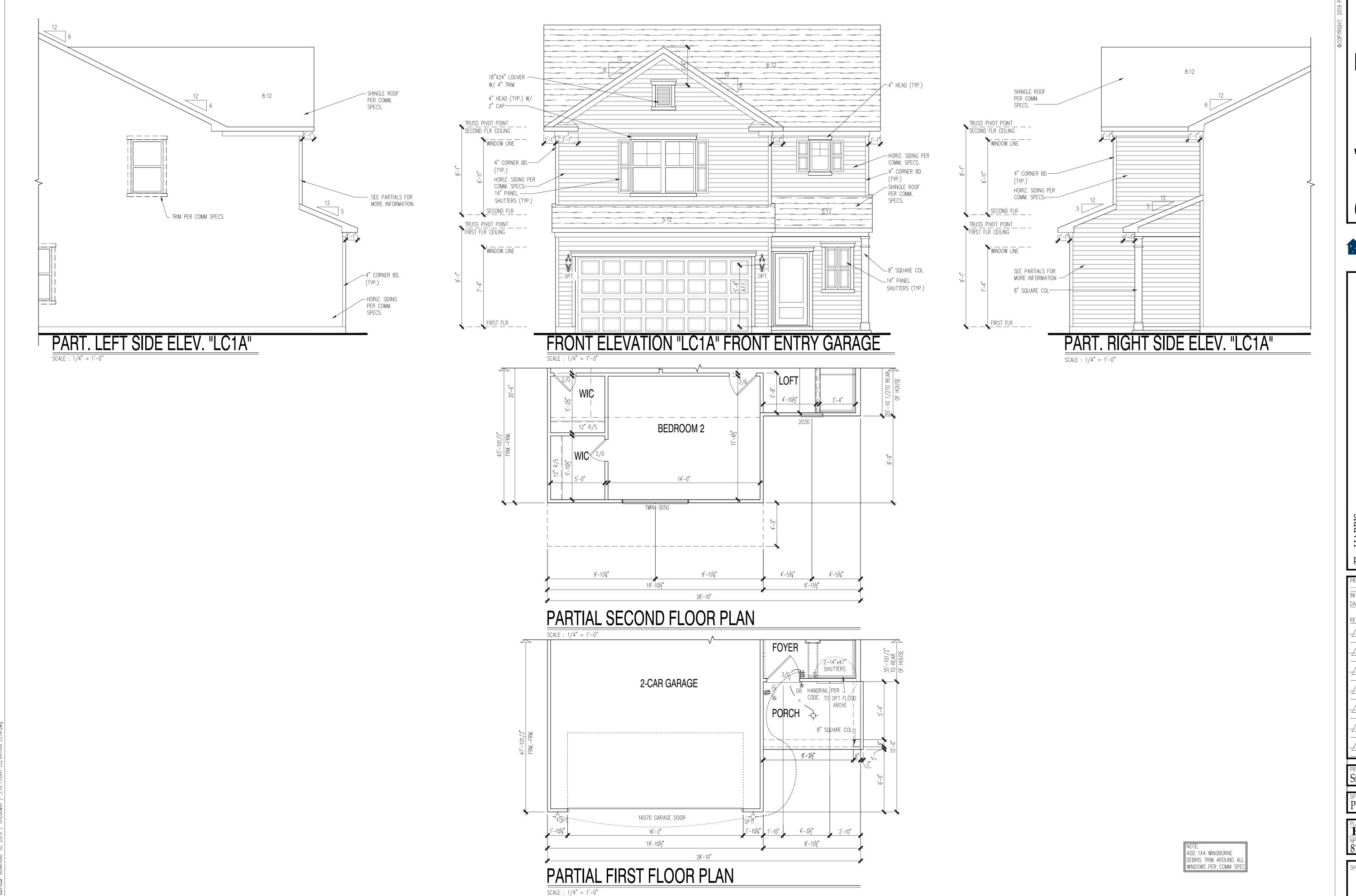
GARAGE HANDING
RL-ME-KC-KR

Hayward/ Briarcliff

A.3.20

Southeast Zone 2475 Northwinds Pkwy. Suite 600 Apharetta, GA 30009 (770) 381-3450

NPC PLAN NUMBER 2466.237 - 2466.238 LAWSON PLAN ID



Southwinds Pkwy, Suite Ipharetta, GA 30009 (770) 381-3

PulteGroup

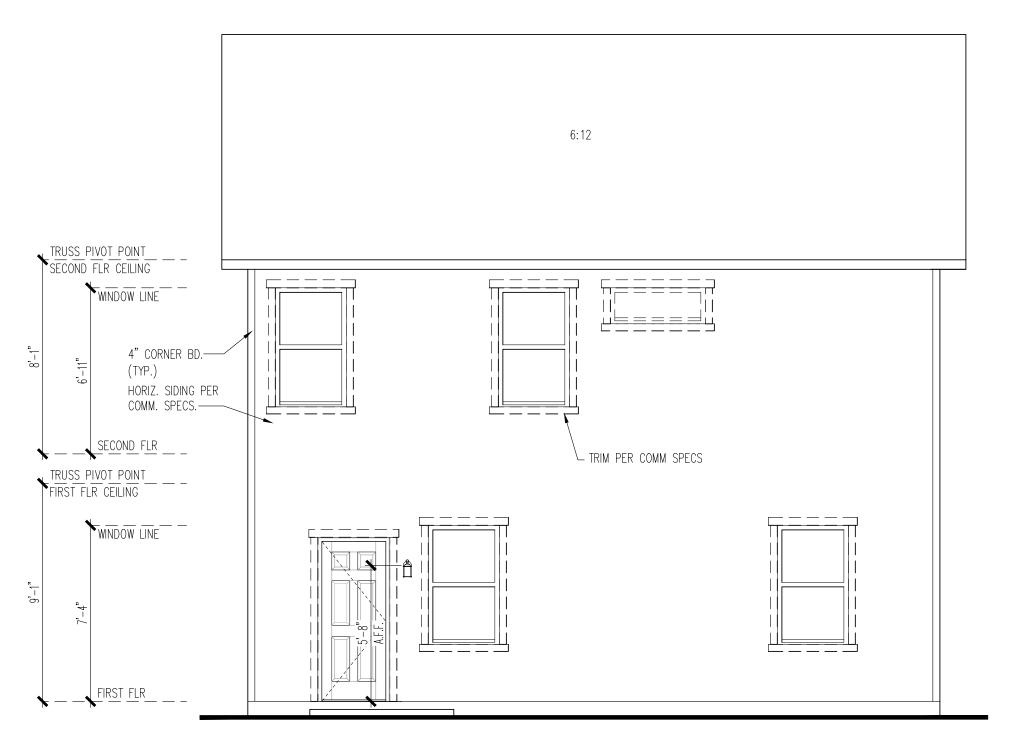
The HARRIS
FRONT ELEVATION "LC1A"
FRONT ENTRY GARAGE

PROJECT TYPE
Single Family

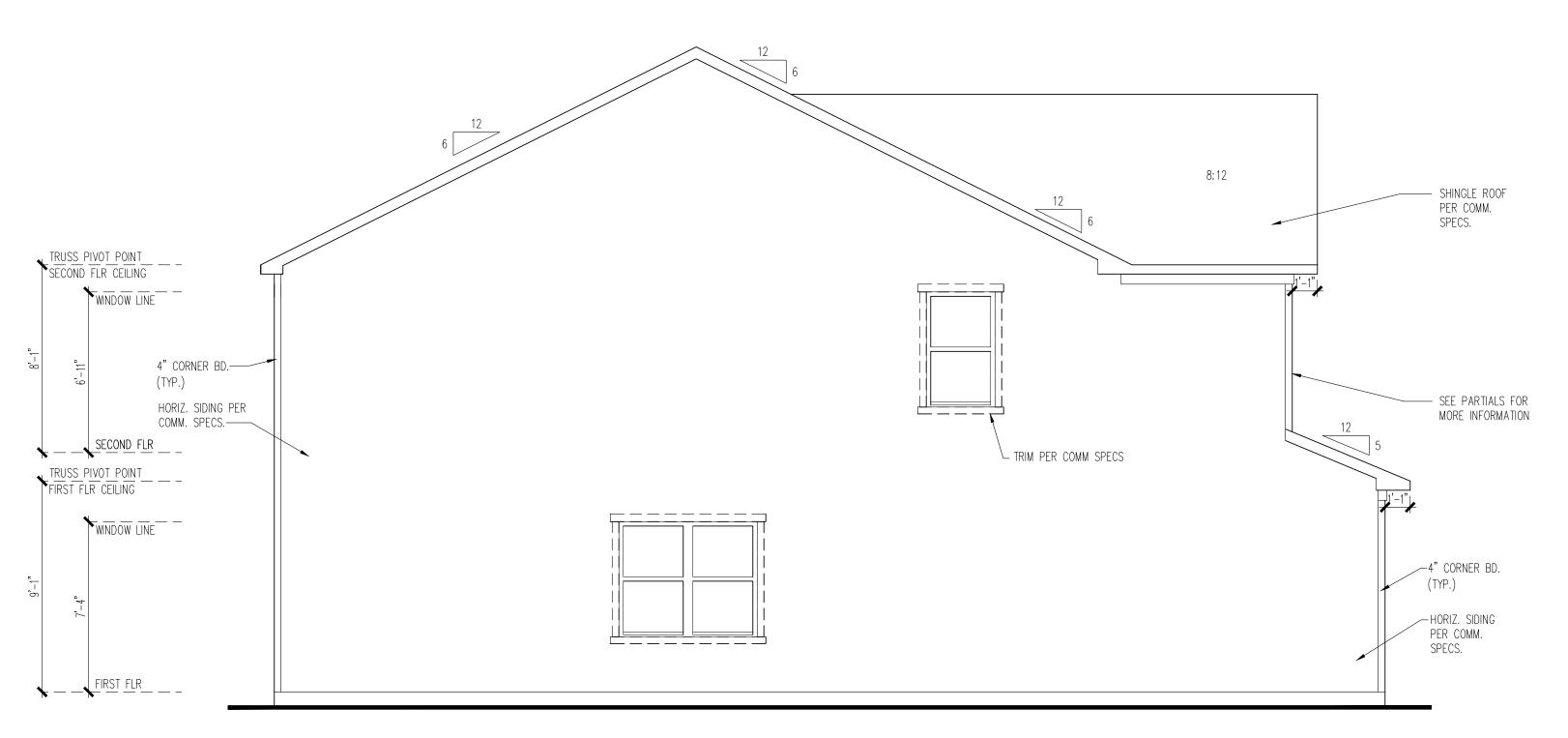
Pulte

Harris
NPC NUMBER
8126.200

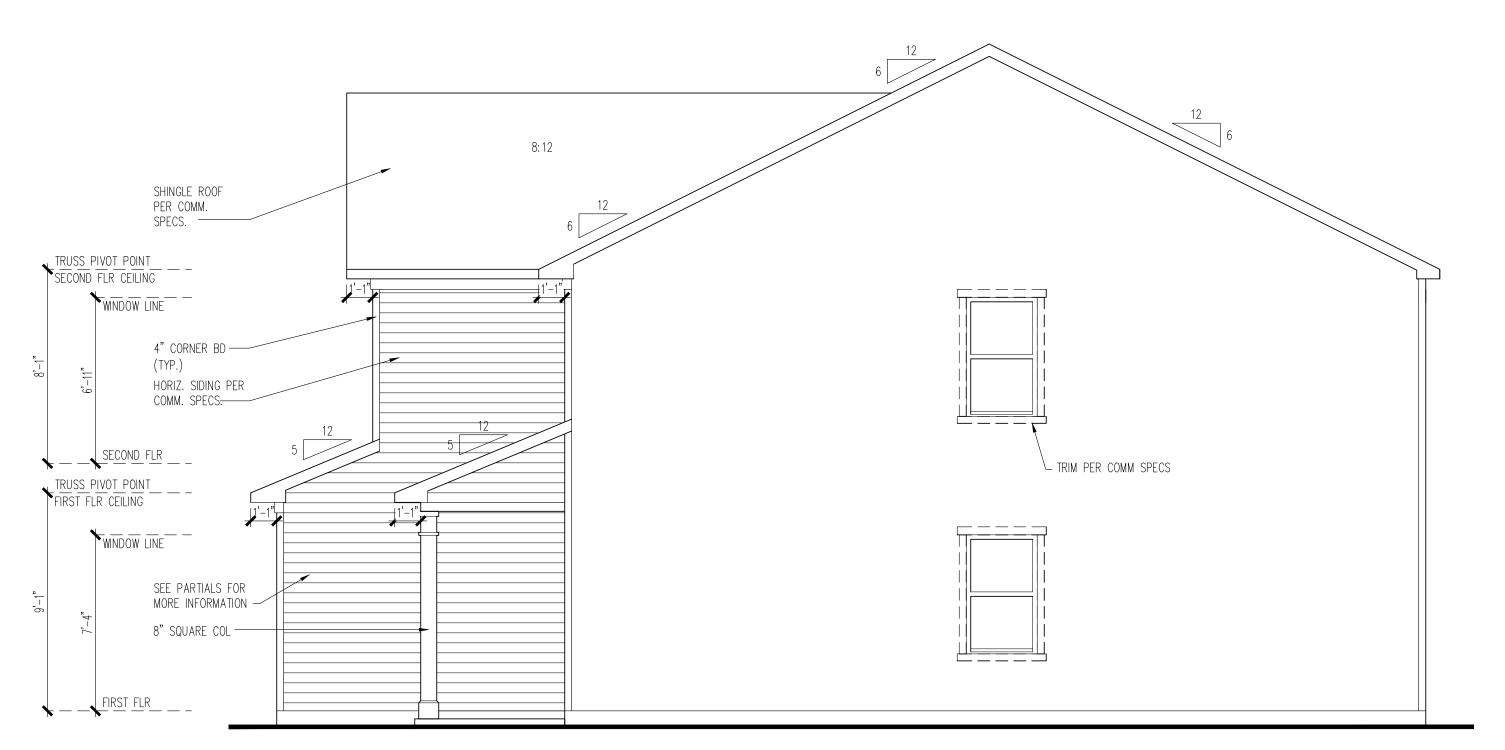
3.10



REAR ELEVATION "LC2A" FRONT ENTRY GARAGE SCALE : 1/4" = 1'-0"



LEFT SIDE ELEVATION "LC2A" FRONT ENTRY GARAGE



RIGHT SIDE ELEVATION "LC2A" FRONT ENTRY GARAGE

 $\frac{1}{\text{SCALE}} : 1/4" = 1'-0"$

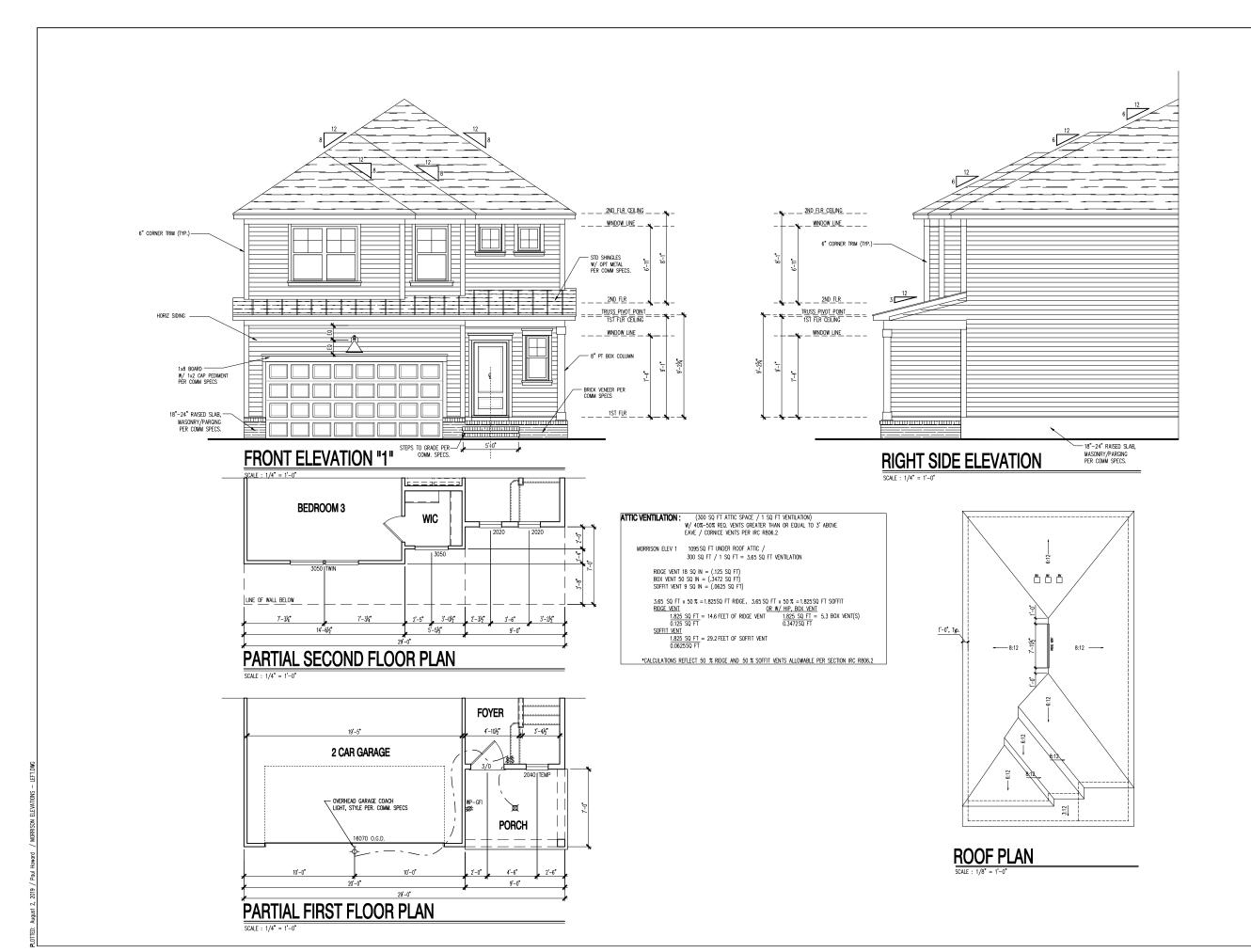
PulteGroup

The HARRIS
SIDE AND REAR ELEVATION "LC2A"
FRONT ENTRY GARAGE INITIAL RELEASE

Single Family

Harris 8126.200

3.SR.3



Southeast Area 2475 Northwinds Pkwy. Suite 525

Pulte

The MORRISON
FRONT ELEVATION "1"

PRODUCTION MANAGER
ACKET

ACKE

PROJECT TYPE SINGLE FAMILY

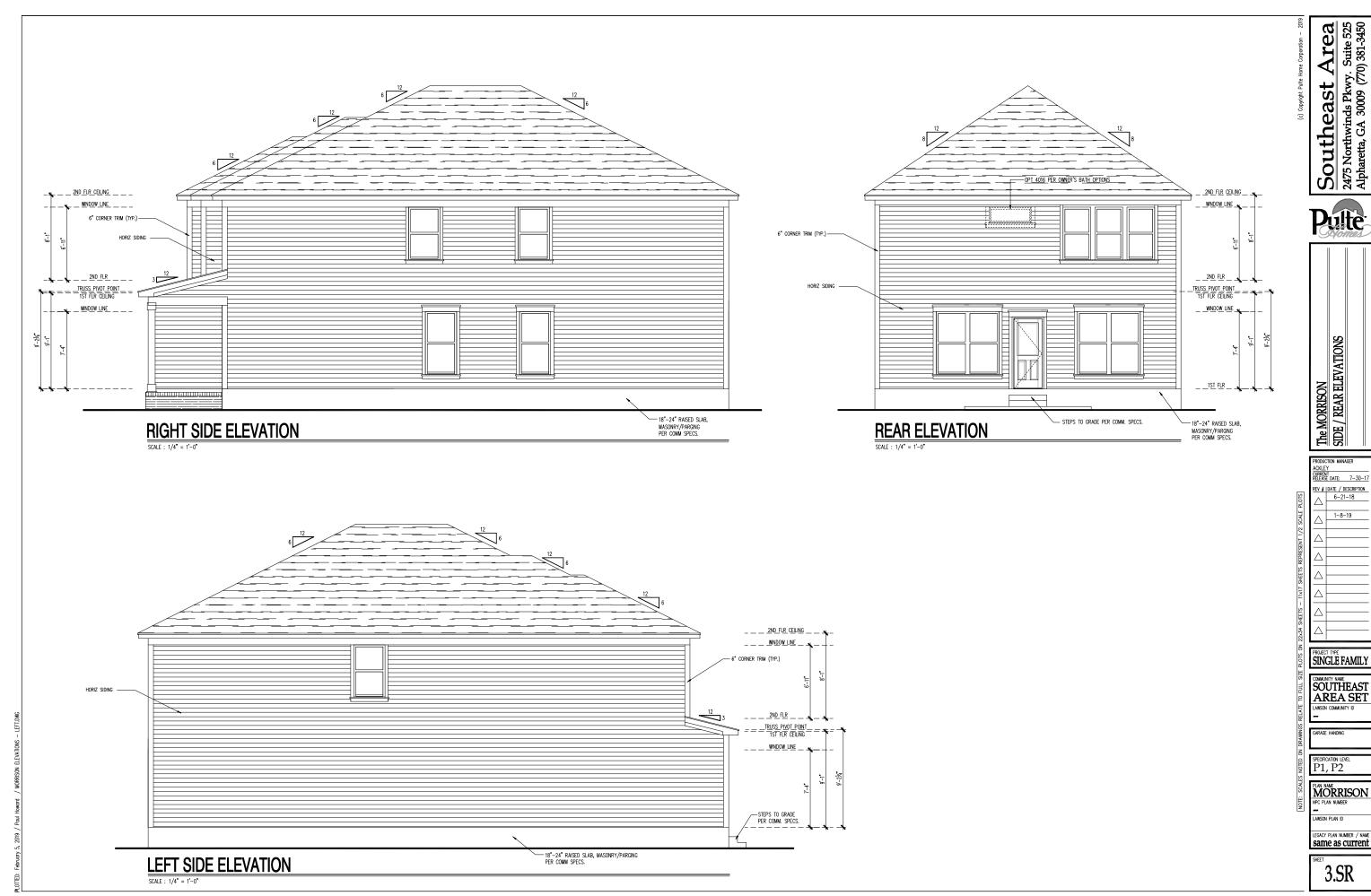
COMMUNITY NAME
SOUTHEAST
AREA SET
LAWSON COMMUNITY ID
--

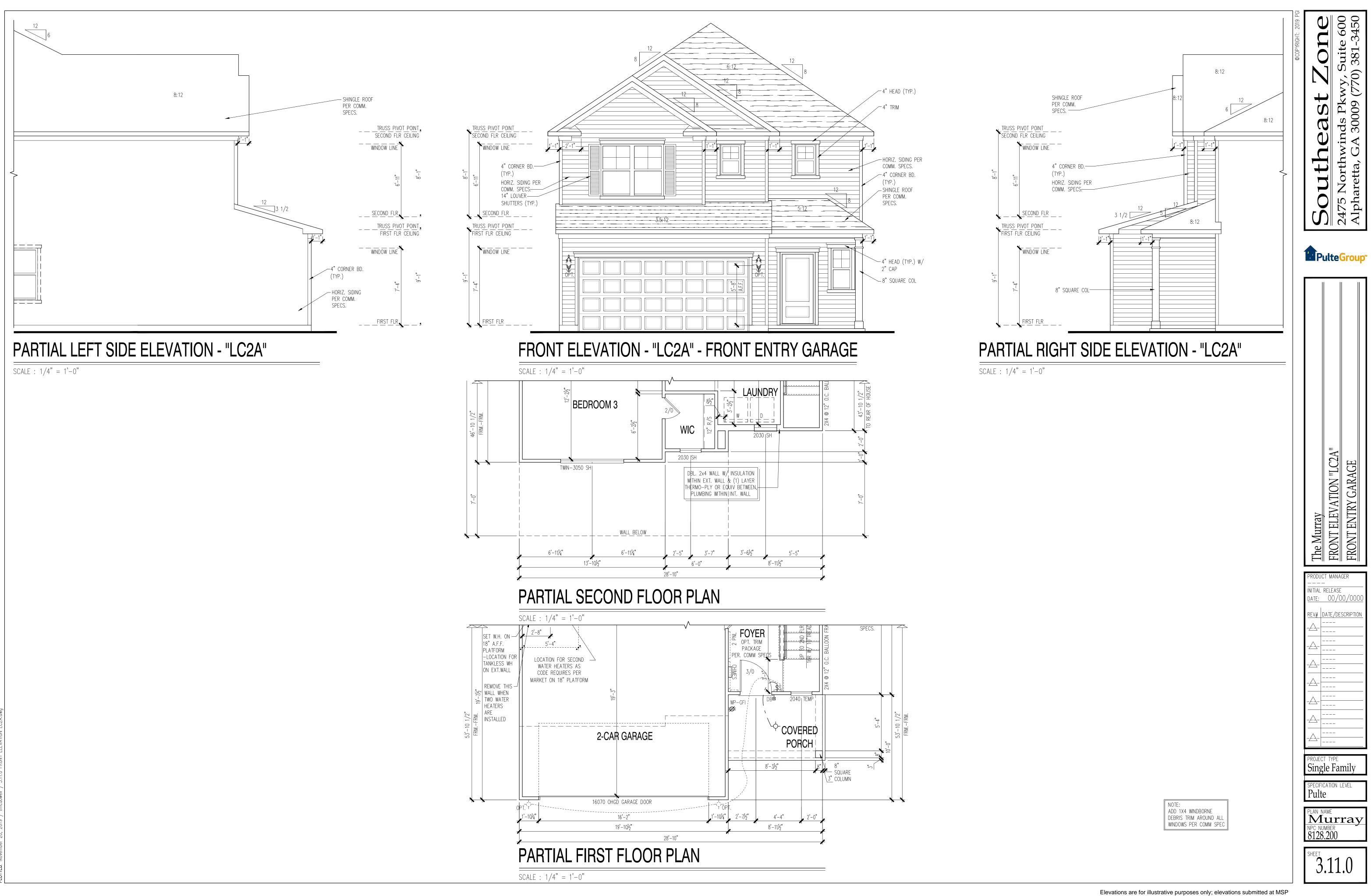
GARAGE HANDING

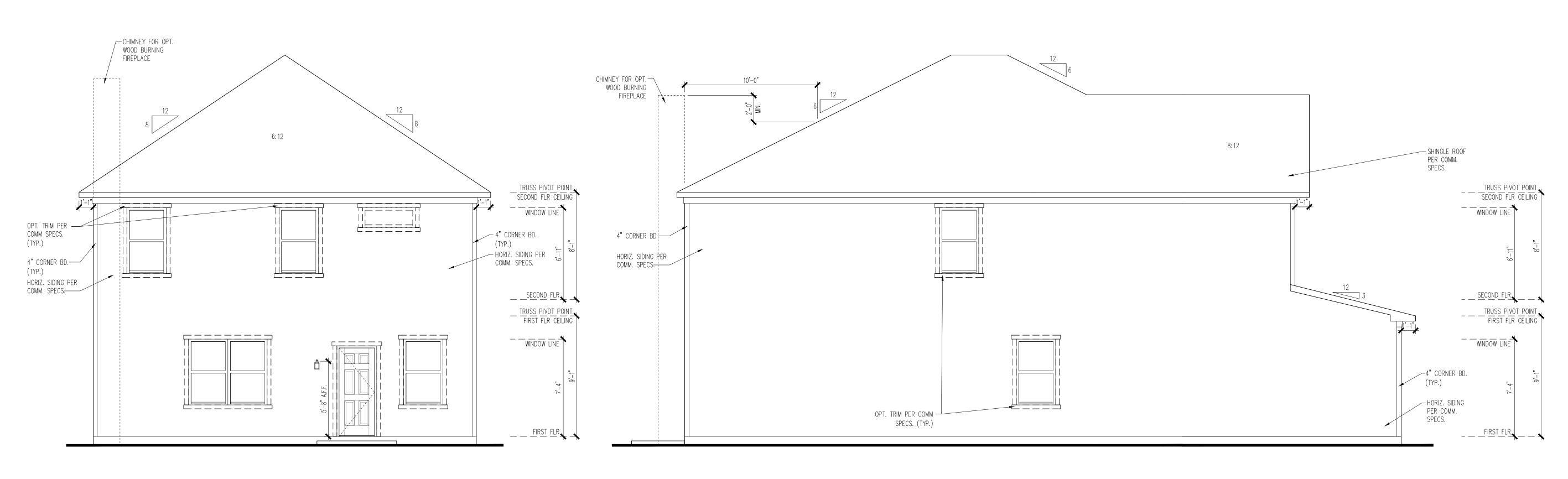
SPECIFICATION LEVEL P1, P2

PLAN NAME
MORRISON
NPC PLAN NUMBER
-LANSON PLAN ID
LEGACY PLAN NUMBER / NAME
Same as current

3.1





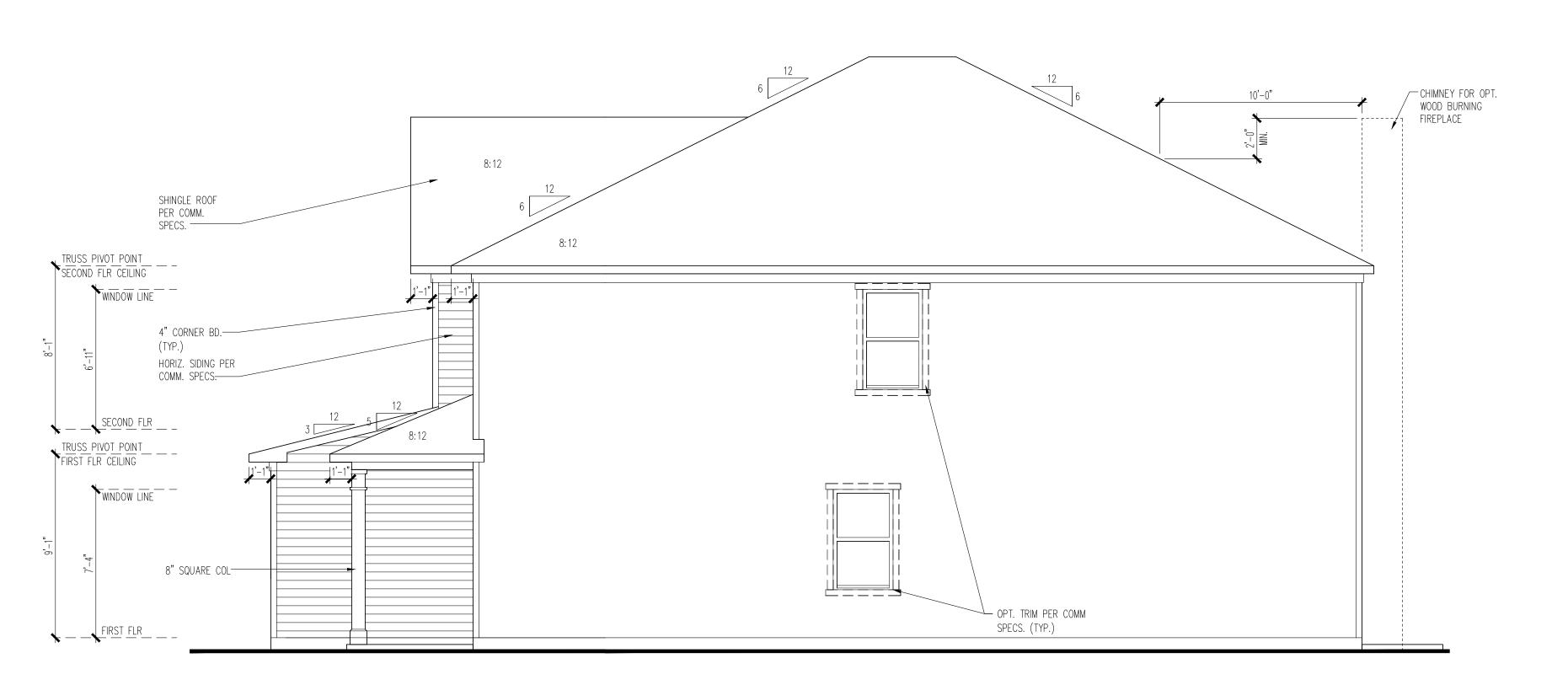


REAR ELEVATION - "LC1A" - FRONT ENTRY GARAGE

SCALE : 1/4" = 1'-0"

LEFT SIDE ELEVATION - "LC1A" - FRONT ENTRY GARAGE

 $\frac{}{}$ SCALE : 1/4" = 1'-0"



RIGHT SIDE ELEVATION - "LC1A" - FRONT ENTRY GARAGE

 $\overline{\text{SCALE} : 1/4" = 1'-0"}$

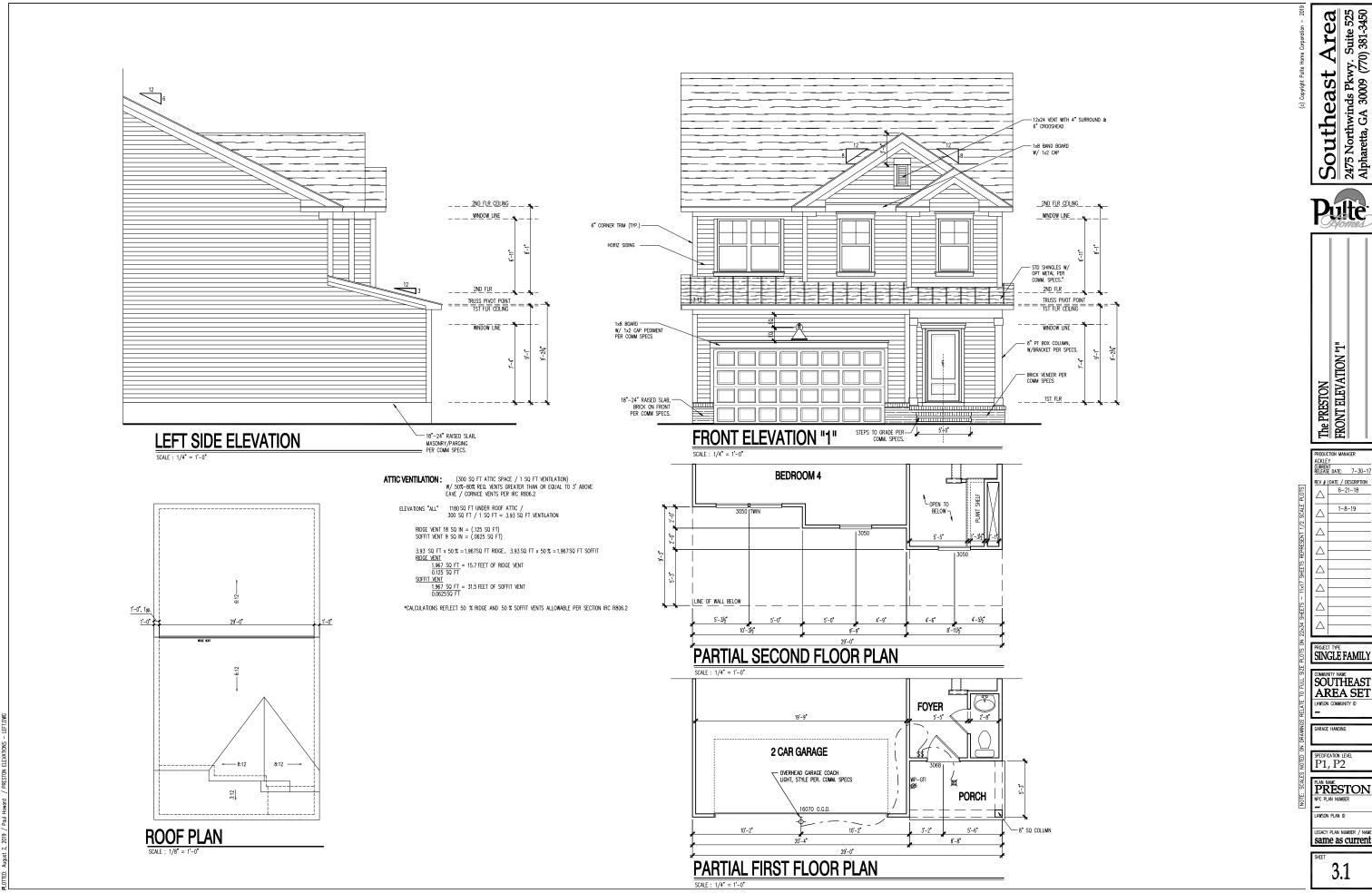
8128.200 will be consistent with the architectural standards included in the PUD.

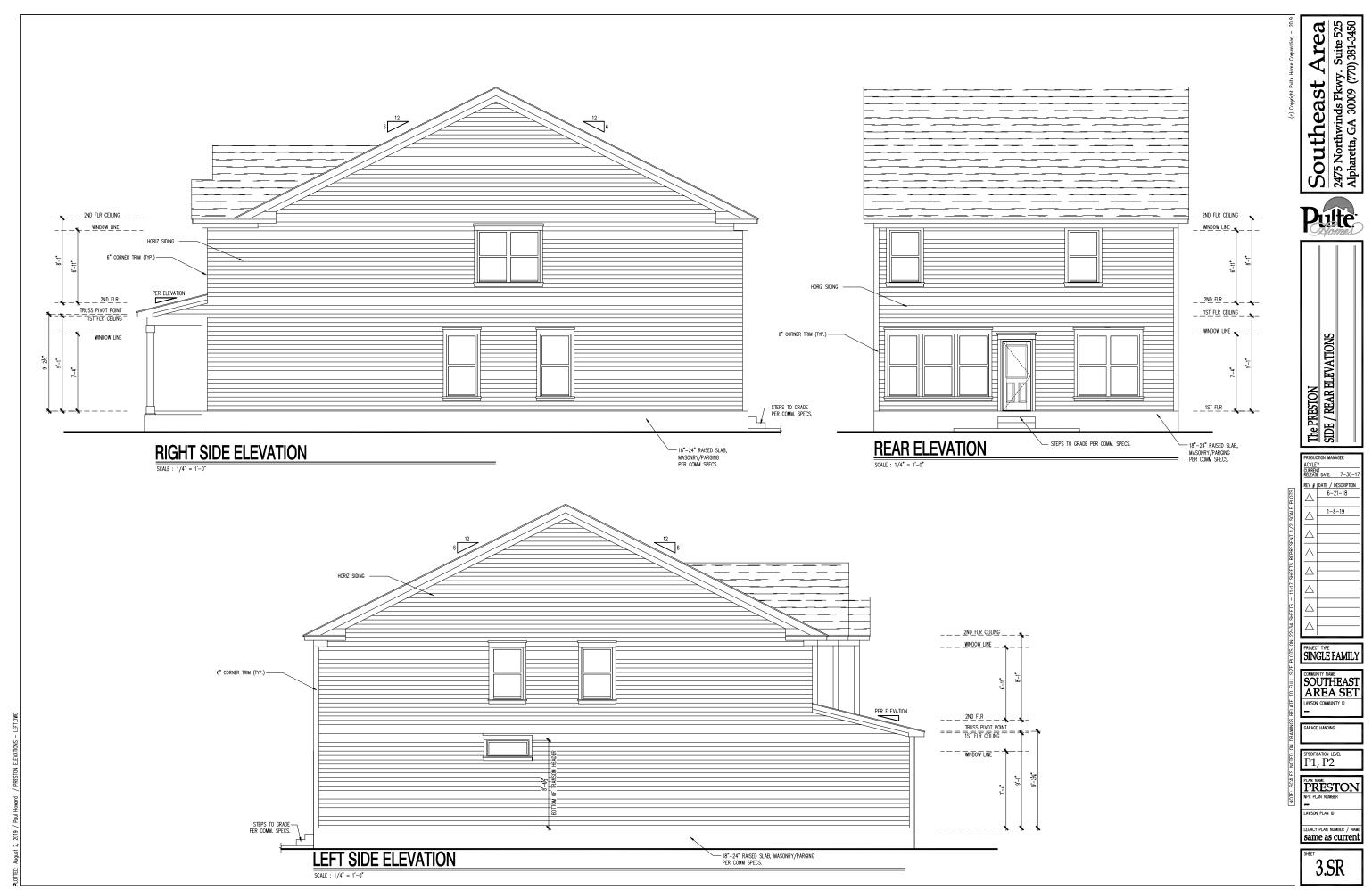
PulteGroup

INITIAL RELEASE

Single Family

Murray





Hackney Tract Subdivision

Apex, NC



PREPARED FOR

WithersRavenel c/o Nick Antrilli, PE 115 MacKenan Drive Cary, NC 27511

PREPARED BY



VHB Engineering NC, P.C. (C-3705)

Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606-5217 919.829.0328

December 22, 2020



Executive Summary

Project Background

There are plans to construct the proposed Hackney Tract Subdivision on the north side of Olive Chapel Road, east of the newly completed Richardson Road, in Apex, NC (Figure 1). The proposed Hackney Tract Subdivision is planned to consist of up to 100 single-family and 133 multi-family townhomes with full build-out expected in 2024. A traffic impact analysis is required by the Town of Apex and North Carolina Department of Transportation (NCDOT) to analyze the potential traffic impacts of the proposed the Hackney Tract Subdivision and to identify any necessary roadway improvements.

As shown on the conceptual site plan (Figure 2), the development will be accessed through one full movement access along Olive Chapel Road:

 Access #1: full movement access on Olive Chapel Road, approximately 2,500 feet east of Richardson Road

In addition, access will be provided via Hasse Avenue extension to the north to Richardson Road, and cross-connections will be provided via local street extensions to the west to Smith Farm.

Based on the agreement with the Town of Apex and NCDOT (Appendix A), the following existing and future intersections were included in the study and analyzed under the AM and PM peak hour conditions:

- SR 1160 (Olive Chapel Road) and SR 1145 (Richardson Road) (unsignalized/future signalized)
- SR 1160 (Olive Chapel Road) and SR 1162 (Apex Barbecue Road) (unsignalized)
- Richardson Road and Hasse Avenue (unsignalized)
- US Highway 64 East at Richardson Road (unsignalized/future signalized)
- US Highway 64 West at U-turn east of Richardson Road (unsignalized/future signalized)
- SR 1160 (Olive Chapel Road) and Future Access #1/Hasse Avenue Extension (full movement access)

The analysis for the Hackney Tract Subdivision was performed under three (3) scenarios: Existing (2020), No-Build (2024) and Build (2024) conditions. The Existing



(2020) scenario includes AM and PM peak hour analysis based on turning movement count data collected in November 2020. The No-Build (2024) scenario includes existing traffic, a three percent (3%) annual growth rate, and site trips generated by seven planned developments within or adjacent to the study area. The Build (2024) scenario includes No-Build (2024) volumes with the addition of site trips generated by the proposed Hackney Tract Subdivision.

Existing (2020) Conditions

Existing analyses were conducted based on current roadway geometrics and intersection turning movement counts.

As reported in the Summary Level of Service (LOS) table on page v, all of the stop- and yield-controlled approaches in the study area are operating at acceptable levels of service (i.e., LOS D or better) during both the AM and PM peak hours under the Existing (2020) conditions, with an exception that the southbound approach of Richardson Road (westbound left-turn of US 64) at US 64 Eastbound operates at LOS F during both peak hours.

No-Build (2024) Conditions

Based on the requirements by the Town of Apex and NCDOT, an annual growth rate of three percent (3%) was applied to the existing traffic to account for ambient growth between the base year (2020) and the future analysis year (2024). In addition, site trips generated by seven (7) planned developments in the study area were aggregated and included in the No-Build (2024) volumes. It should be noted that although significant traffic increases were expected with the inclusion of background developments, an undiscounted annual growth rate was applied to offset the impacts on traffic data collected in 2020 with COVID-19 pandemic restrictions in place.

As for transportation improvements, mitigation requirements associated with Sweetwater are expected to include two new signals and additional turn lanes along US 64 at the Richardson Road and U-turn east of Richardson Road intersections; in addition, a new signal is expected to be installed by Smith Farm at the Olive Chapel Road and Richardson Road intersection once it is warranted.

Based on the No-Build (2024) analysis, the study area is projected to experience traffic and delay increases, but the impacts will be substantially mitigated by the background transportation improvements. As a result, all of the signalized intersections and stop-controlled approaches in the study area are projected to operate at acceptable levels of service except that the stop-controlled northbound approach of Apex Barbecue Road at Olive Chapel Road is projected to decline to operate at LOS F in the PM peak hour.



Trip Generation and Assignment

Trip generation was conducted based on the most appropriate corresponding trip generation codes included in the *ITE Trip Generation Manual*, 10th Edition and the suggested method of calculation in the NCDOT's "Rate vs. Equation" Spreadsheet. To provide a conservative analysis, no transit, walking, or bicycling reductions will be applied.

Land Use	T 111	T.T. **	Unit $ADT = AM$ Enter	AM Peak Hour		PM Peak Hour			
Code	Land Use	Unit		Enter	Exit	Total	Enter	Exit	Total
210	Single-Family Detached Housing	100 du	1,040	19	57	76	64	38	102
220	Multi-Family Housing (Low-Rise)	133 du	965	14	49	63	48	28	76
Development Total		2,005	33	106	139	112	66	178	

In total, the proposed Hackney Tract Subdivision is projected to generate 2,005 daily trips with 139 trips (33 entering, 106 exiting) occurring in the AM peak hour and 178 trips (112 entering, 66 exiting) occurring the PM peak hour. The resulting site trips were distributed in accordance with the existing traffic patterns and anticipated land uses.

Build (2024) Conditions

The Build (2024) conditions account for both the No-Build (2024) traffic and site traffic generated by the proposed Hackney Tract Subdivision.

As shown in the Summary LOS table on page v, the stop-controlled northbound approach of Apex Barbeque Road at Olive Chapel Road is projected to continue to operate at failing levels of services in the PM peak hour with delay increases. The rest of the intersections included in the study area are projected to continue operating at acceptable levels of service during both peak hours. The planned stop-controlled Future Access #1 is projected to operate at LOS C in the AM peak hour and LOS D in the PM peak hour.

Roadway Improvement Recommendations

As indicated in the traffic operations analyses, the proposed Hackney Tract Subdivision is projected to have minimum impacts on traffic operations of the surrounding roadway network and intersections. Nevertheless, the following roadway improvements are recommended to improve traffic operations and safety:



SR 1160 (Olive Chapel Road) and Future Access #1/Hasse Avenue Extension (unsignalized, full movement)

Future Access #1 is projected to operate at acceptable levels of service during the AM and PM peak hour with a two-lane cross-section. Although traffic volumes are not projected to automatically warrant turn lanes on Olive Chapel Road, dedicated turn lanes should be provided with the required frontage widening to meet the Town of Apex Comprehensive Transportation Plan standards. Therefore, the following site access configuration and transportation improvements are recommended at this intersection:

- Construct Future Access #1 to consist of one inbound lane and one outbound lane.
- Provide a dedicated left-turn lane on eastbound Olive Chapel Road with 100 feet of storage length and appropriate taper.
- Provide a dedicated right-turn lane on westbound Olive Chapel Road with 100 feet of storage length and appropriate taper.

SR 1160 (Olive Chapel Road) and SR 1162 (Apex Barbecue Road) (unsignalized)

Traffic analysis indicated that the northbound approach of Apex Barbecue Road is projected to operate at LOS F in the PM peak hour under the No-Build and Build conditions. The intersection is not anticipated to meet warrants for installing a new traffic signal, while options for adding new turn lanes are limited due to the skewed angle of intersection on a curve of Olive Chapel Road and potential right-of-way/drainage restrictions. As shown on the Apex Comprehensive Transportation Plan, this intersection is identified for future intersection realignment. Since site trips are anticipated to contribute less than 4% traffic increases in the AM and 3% in the PM at this intersection (increases of only 1 VPH in the AM peak hour and 2 VPH in the PM peak on the stop-controlled approach), improvement should not be required by this development based on the Town of Apex UDO. Nevertheless, alternative traffic control method (such as AWSC), if warranted by crash analysis, may be considered before this intersection is realigned in the future based on the Town of Apex CTP.

The rest of study area intersections are expected to operate acceptably. Therefore, no mitigation is required.



Summary Level of Service Table

Intersection and Approach	Control	Existing	g (2020)	No-Build (2024)		Build (2024)	
		AM	PM	AM	PM	AM	PM
Richardson Rd and Olive Chapel Rd		-	-	A (9.7)	B (11.8)	A (9.8)	B (12.0)
Eastbound	TWSC/			A-9.7	B-10.7	A-9.8	B-10.9
Westbound	Signal			B-10.3	B-12.0	B-10.5	B-12.2
Northbound		B-11.9	B-14.1	B-10.2	B-12.7	B-10.3	B-13.0
Southbound		B-11.7	C-15.5	A-8.4	B-11.5	A-8.5	B-11.6
Apex Barbecue Rd and Olive Chapel Rd	TWSC	-	-	-	-	-	-
Northbound		B-11.8	C-19.5	C-16.8	F-92.5	C-17.9	F-134.5
Richardson Rd and Little Gem Ln/Hasse Ave	TWSC	-	-	-	-	-	-
Eastbound	TWSC	A-9.8	B-10.2	C-16.5	C-21.8	C-19.1	D-32.0
Westbound		A-9.7	A-9.9	C-15.1	C-19.1	C-17.0	C-21.9
Richardson Rd/WB Left- Over and US 64	TWICE /	TWSC/ Signal C-20.7	D (42.0)	C (22.0)	D (44.5)		
Eastbound	, , , , , , , , , , , , , , , , , , ,			C-20.7	D-51.3	C-23.7	E-56.0
Northbound	5181141	C-23.5	C-23.3	C-28.2	D-47.4	C-28.5	D-50.1
Southbound		F-66.0	F-216.7	B-10.9	B-19.6	A-9.8	B-19.5
U-Turn East of Richardson Rd and US 64	TWSC/	-	-	B (11.8)	C (27.6)	B (12.5)	C (30.9)
Westbound	Signal			A-9.6	C-20.5	B-10.5	C-24.1
Northbound		B-14.2	C-18.2	C-27.8	E-59.9	C-26.5	E-62.1
Olive Chapel Rd & Hasse Ave/Future Access #1	TWSC	-	-	-	-	-	-
Southbound						C-16.1	D-25.0

 $\label{eq:legender} \mbox{LEGEND: } X \mbox{ (XX) = Overall intersection LOS (intersection delay in sec/veh);}$

X - XX = approach LOS - approach delay in sec/veh

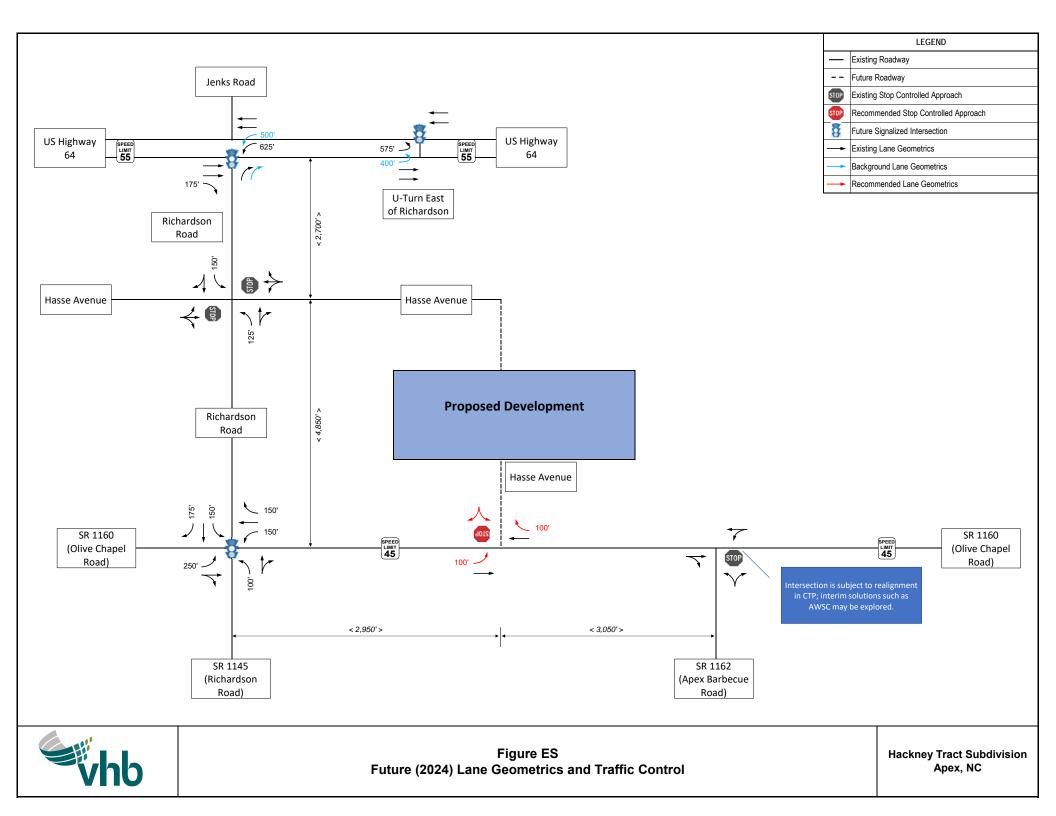




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1

Introduction

There are plans to construct the proposed Hackney Tract Subdivision on the north side of Olive Chapel Road, east of the newly completed Richardson Road, in Apex, NC (Figure 1). The proposed Hackney Tract Subdivision is planned to consist of up to 100 single-family and 133 multi-family townhomes with full build-out expected in 2024.

As shown on the conceptual site plan (Figure 2), the development will be accessed through one full movement access along Olive Chapel Road:

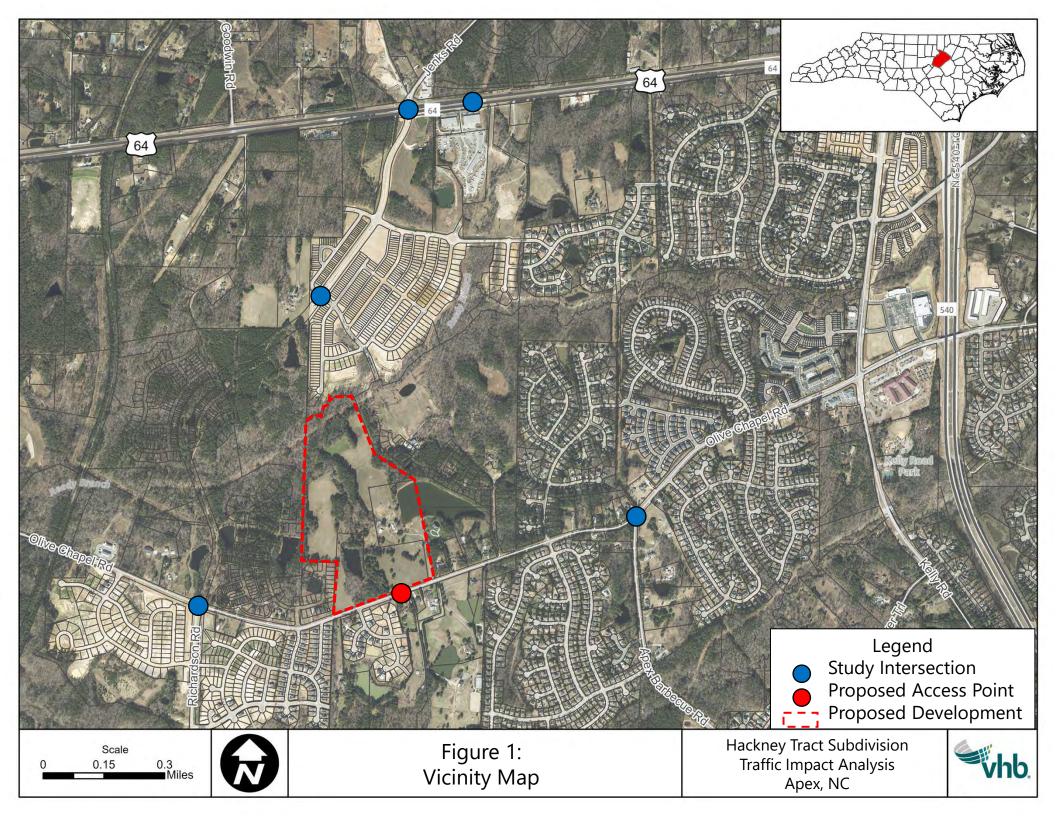
 Access #1: full movement access on Olive Chapel Road, approximately 2,500 feet east of Richardson Road

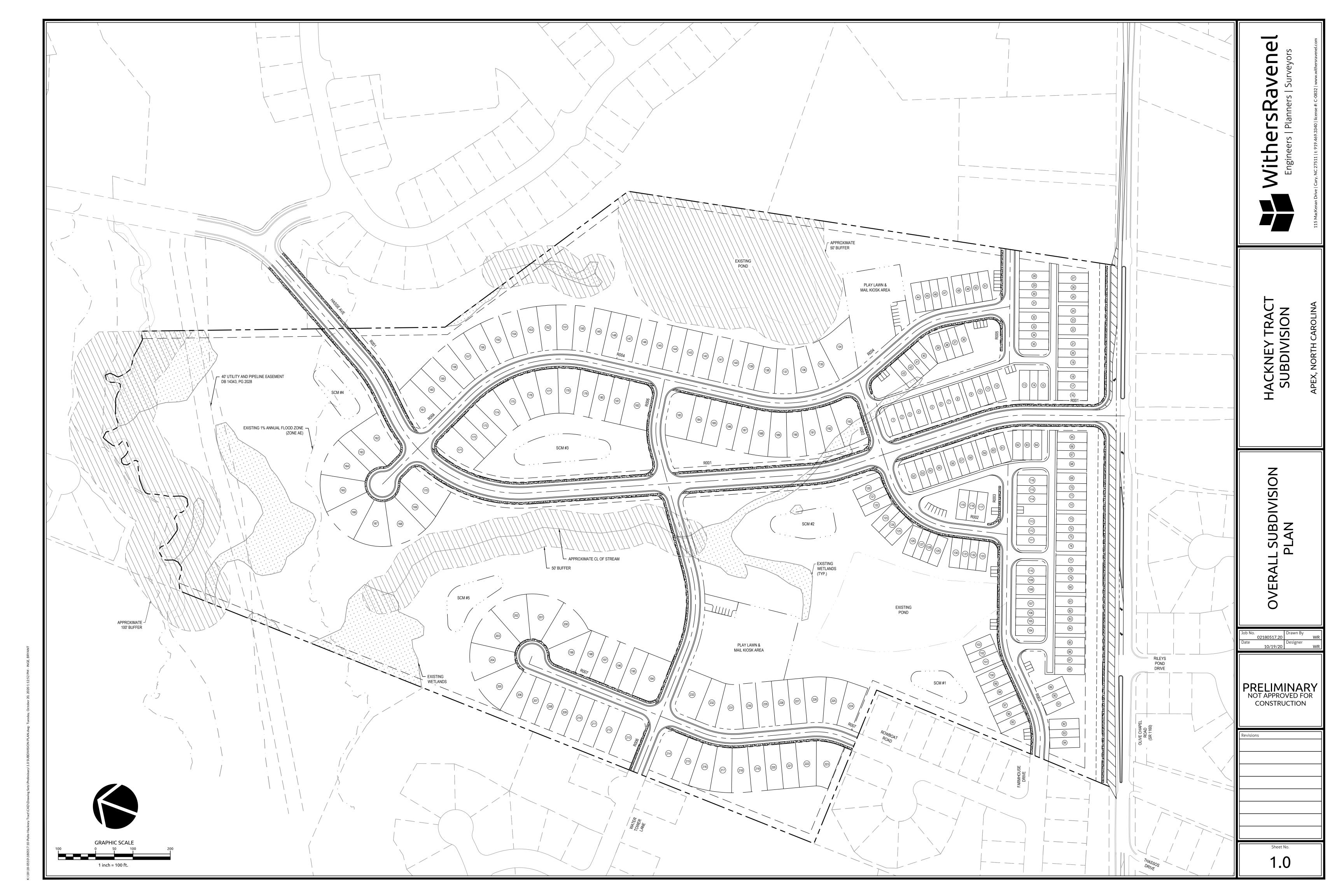
In addition, access will be provided via Hasse Avenue extension to the north to Richardson Road, and cross-connections will be provided via local street extensions to the west to Smith Farm.

Based on the agreement with the Town of Apex and NCDOT (Appendix A), the following existing and future intersections were included in the study and analyzed under the AM and PM peak hour conditions:

- SR 1160 (Olive Chapel Road) and SR 1145 (Richardson Road) (unsignalized/future signalized)
- SR 1160 (Olive Chapel Road) and SR 1162 (Apex Barbecue Road) (unsignalized)
- Richardson Road and Hasse Avenue (unsignalized)
- US Highway 64 East at Richardson Road (unsignalized/future signalized)
- US Highway 64 West at U-turn east of Richardson Road (unsignalized/future signalized)
- SR 1160 (Olive Chapel Road) and Future Access #1/Hasse Avenue Extension (full movement access)

VHB Engineering NC, P.C. (VHB) is contracted with the development team to analyze the potential traffic impacts of the proposed development and to identify any necessary roadway improvements. This Traffic Impact Analysis (TIA) summarizes trip generation, distribution, traffic assignment, and traffic analyses for the proposed development. The Memorandum of Understanding, which summarizes the assumptions for the study is included in Appendix A.





2

Existing (2020) Conditions

Existing Roadway Conditions

This section describes the existing roadways in the vicinity of the proposed development. Annual Average Daily Traffic (AADT) data for the surrounding network of roadway were obtained from the North Carolina Department of Transportation (NCDOT). The most recent AADT counts from the NCDOT are for 2019 on the study area roadways.

Olive Chapel Road (SR 1160)

- Olive Chapel Road is a two-lane undivided road within the study area. The posted speed limit along this roadway is 45 miles per hour (mph).
- As shown on the Town of Apex Thoroughfare and Collector Street Plan (last amended October 2020), Olive Chapel Road is planned to be widened to a 4lane thoroughfare with median across the study area.
- The 2019 NCDOT AADT along Olive Chapel Road was 2,500 vehicles per day (vpd) between New Hill Olive Chapel Road and Kelly Road.

Richardson Road (SR 1145)

- Richardson Road is a new two-lane median divided roadway connecting US
 Highway 64 and Olive Chapel Road within the study area. The posted speed
 limit along this roadway is 45 mph.
- As shown on the Town of Apex Thoroughfare and Collector Street Plan, Richardson Road is planned to be widened to a 4-lane thoroughfare with median across the study area.
- The 2019 AADT along Richard Road was 810 vpd south of Olive Chapel Road. No AADT information is available for Richardson Road between Olive Chapel Road and US 64.

Apex Barbecue Road (SR 1162)

- Apex Barbecue Road is a two-lane undivided roadway south of the project site within the study area. The posted speed limit along this roadway is 45 mph.
- As shown on the Town of Apex Thoroughfare and Collector Street Plan, Apex Barbeque Road is planned to be widened to a 3-lane thoroughfare with intersection realignment planned at Olive Chapel Road.
- The 2019 NCDOT AADT along Apex Barbeque Road was 4,500 vpd between Kelly Road and Olive Chapel Road.



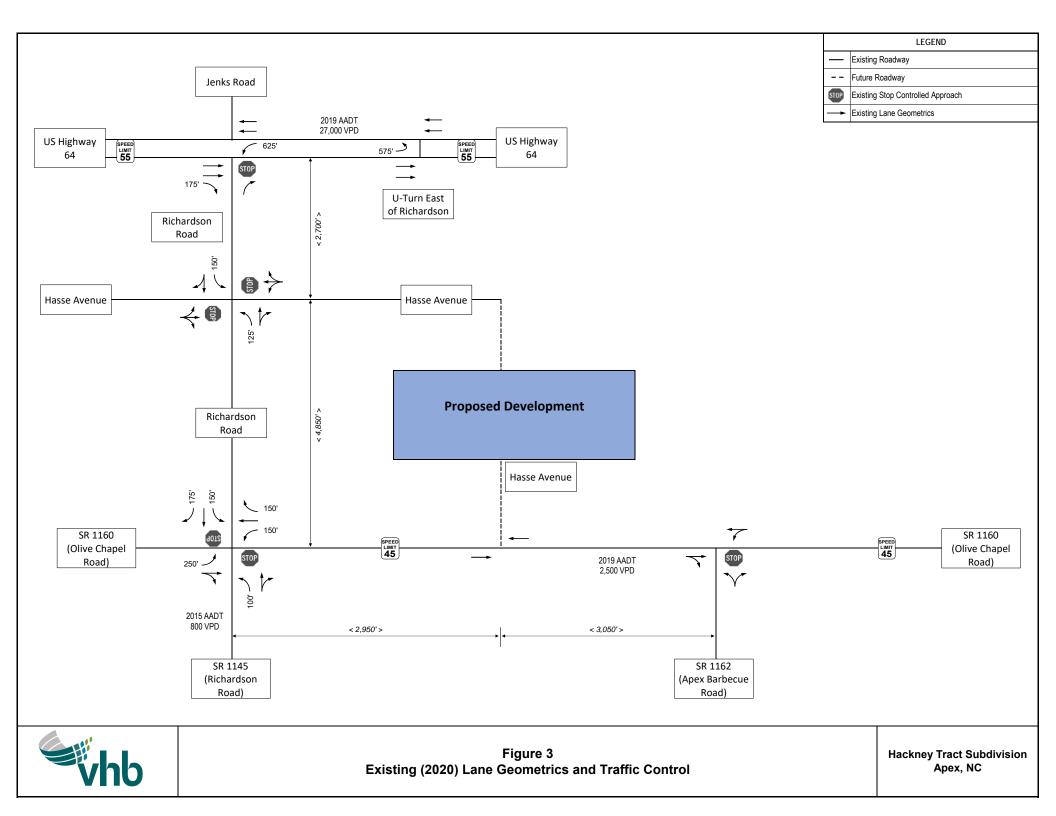
US Highway 64 (US 64)

- US Highway 64 is a four-lane median divided highway with partial control of access within the study area. The posted speed limit along US 64 is 55 mph.
- As shown on the Town of Apex Thoroughfare and Collector Street Plan, US
 Highway 64 is planned to be a freeway facility with full control of access, and
 a future interchange is planned along US 64 with Richardson Road/Jenks
 Road.
- The 2019 NCDOT AADT along US 64 was 27,000 vpd between New Hill Road and Kelly Road.

Hasse Avenue

- Hasse Avenue is a two-lane undivided local road within the study area. No posted speed limit was observed along Hasse Avenue.
- As shown on the Town of Apex Thoroughfare and Collector Street Plan, Hasse Avenue is planned to be extended to a two-lane major collector street between Richardson Road and Olive Chapel Road.
- No AADT information is available for Hasse Avenue within the study area.

Figure 3 provides a schematic diagram of the roadways near the proposed development including the existing intersection geometrics.





Existing Turning Movement Data

VHB Engineering NC, P.C. collected the intersection turning movement counts analyzed in this TIA in November 2020. Traffic data were collected during typical AM (7:00 – 9:00 AM) and PM (4:00 – 6:00 PM) peak periods while schools were partially open due to the COVID-19 pandemic restrictions. Table 1 summarizes the schedule used to obtain the turning movement data. A detailed summary of the traffic counts can be found in Appendix B.

Table 1: Weekday Peak Hour Turning Movement Count Schedule

Intersection	Time Period	Data Collection Date
Olive Chapel Road and Richardson Road	7:00 AM – 9:00 AM 4:00 PM – 6:00 PM	Thursday November 5, 2020
Richardson Road and Hasse Avenue/Little Gem Lane	7:00 AM – 9:00 AM 4:00 PM – 6:00 PM	Thursday November 5, 2020
US Highway 64 and Richardson Road	7:00 AM – 9:00 AM 4:00 PM – 6:00 PM	Thursday November 5, 2020
US Highway 64 and U-Turn East of Richardson Road	7:00 AM – 9:00 AM 4:00 PM – 6:00 PM	Thursday November 5, 2020
Olive Chapel Road and Apex Barbecue Road	7:00 AM – 9:00 AM 4:00 PM – 6:00 PM	Thursday November 5, 2020

The existing peak hour turning movement volumes are shown in Figure 4.

Level of Service Criteria

Peak hour level of service (LOS) measures the adequacy of the intersection geometrics and traffic controls of a particular intersection or approach for the given turning volumes. Levels of service range from A through F, based on the average control delay experienced by vehicles traveling through the intersection during the peak hour. Control delay represents the portion of total delay attributed to traffic control devices (e.g., signals or stop signs). Table 2 provides a general description of various levels of service categories and delay ranges.



Table 2: Level of Service Standard for Intersections

Level of Service	Signalized Intersection	Unsignalized Intersection
A	<= 10 sec.	<= 10 sec.
В	10-20 sec.	10-15 sec.
С	20-35 sec.	15-25 sec.
D	35-55 sec.	25-35 sec.
E	55-80 sec.	35-50 sec.
F	> 80 sec.	> 50 sec.

The engineering profession generally accepts LOS D as an acceptable operating condition for signalized intersections. Based on the Policy on Street and Driveway Access to North Carolina Highways (NCDOT Driveway Manual) and the Town of Apex Unified Development Ordinance (UDO), geometric and/or traffic control improvements should be identified at signalized intersections to prevent the traffic generated by the proposed development from causing any intersection or roadway approach to fall below LOS D. For intersections projected to operate worse than LOS D under the background conditions, improvements should be identified to minimize the increase in average overall intersection delay when site traffic accounts for at least 10% of the projected total peak hour traffic at the intersections.

At unsignalized intersections, stop-controlled minor street approaches may exceed LOS D provided the addition of development traffic is not anticipated to warrant a traffic signal upon build-out and the resulting congestion does not block traffic movements at adjacent intersections. Guidelines provided by NCDOT shall be used in the evaluation of the need for and length of exclusive right and/or left turn lanes to support development traffic; for any and all turning movements where the development is anticipated to add at least 10% to the existing peak hour traffic volume, improvements may be required to mitigate the impact of development traffic on turn lane storage requirements.

Level of Service Analysis

Intersection levels of service analyses were performed for the typical weekday AM and PM peak hours using *Synchro/SimTraffic Professional Version 10.* A summary of the findings for the Existing (2020) scenario LOS analysis can be found in Table 3 and the full *Synchro/HCS* output can be found in Appendix D.

As reported in Table 3, all of the stop- and yield-controlled approaches in the study area are operating at acceptable levels of service (i.e., LOS D or better) during both the AM and PM peak hours under the Existing (2020) conditions, with an exception that the southbound approach of Richardson Road (westbound left-turn of US 64) at US 64 Eastbound operates at LOS F during both peak hours.

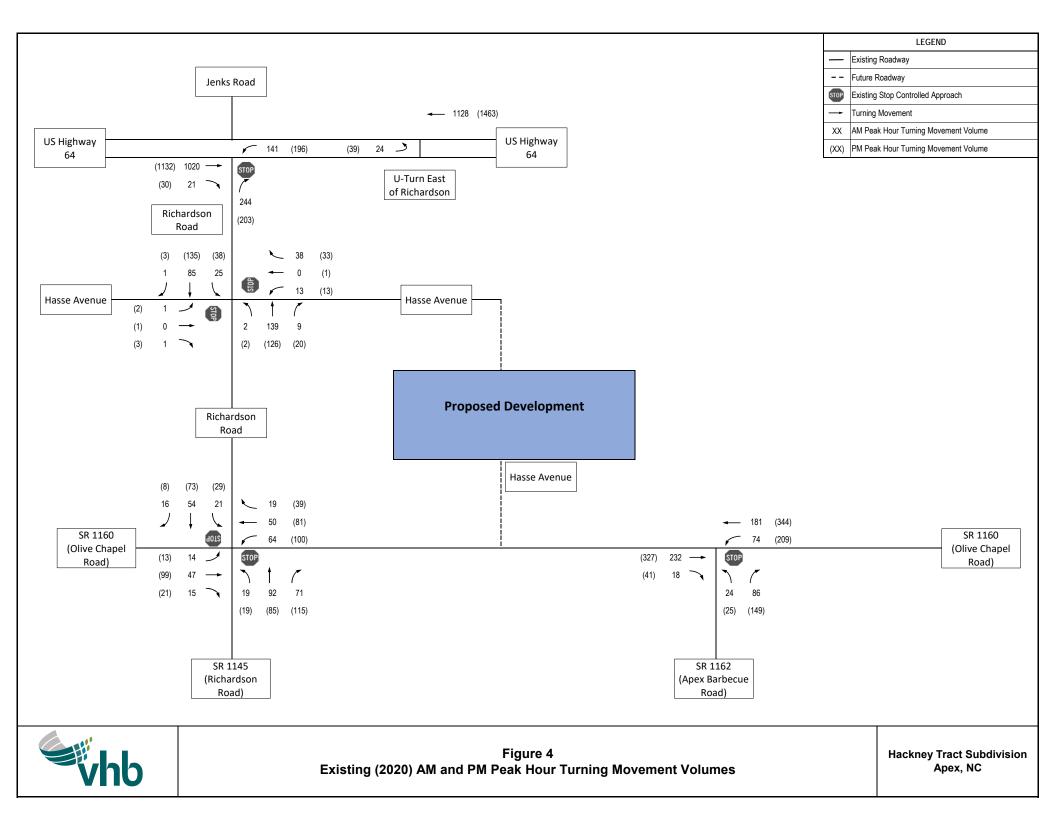


Table 3: Existing (2020) LOS Results

Intersection and Approach	Control	Existing (2020)		
		AM	PM	
Richardson Rd and Olive Chapel Rd	TWSC	-	-	
Northbound	1 W 5 C	B-11.9	B-14.1	
Southbound		B-11.7	C-15.5	
Apex Barbecue Rd and Olive Chapel Rd	TWSC	-	-	
Northbound		B-11.8	C-19.5	
Richardson Rd and Little Gem Ln/Hasse Ave	TWSC	-	-	
Eastbound		A-9.8	B-10.2	
Westbound		A-9.7	A-9.9	
Richardson Rd/WB Left-Over and US 64	TWSC	-	-	
Northbound		C-23.5	C-23.3	
Southbound		F-66.0	F-216.7	
U-Turn East of Richardson Rd and US 64	TWSC	-	-	
Northbound		B-14.2	C-18.2	

LEGEND: **X (XX)** = Overall intersection LOS (intersection delay in sec/veh);

X - XX = approach LOS - approach delay in sec/veh





3

No-Build (2024) Conditions

Background Growth and Development

Based on discussions with the Town of Apex and NCDOT, an annual growth rate of three percent (3%) was applied to the existing traffic to account for the growth between the base year (2020) and the future analysis year (2024). In addition, site trips from seven (7) adjacent developments that are expected to occur before the analysis year were incorporated into the analysis.

Saddlebrook (Lawrence Assemblage/Richardson West) – Located in the southwest corner of the Olive Chapel Road and Richardson Road intersection, this residential development is proposed to consist of 104 single-family homes and be constructed by 2017. A traffic analysis report was prepared by Ramey Kemp & Associates and submitted to the Town on November 3, 2014. As detailed in the report, the development is projected to generate 1,090 daily site trips, with 83 trips (21 entering, 62 exiting) occurring in the AM peak hour and 109 trips (69 entering, 40 exiting) occurring in the PM peak hour. These trips were distributed to the study area based on the assumed distribution patterns in the report. Field visits indicate that construction has begun but has not been completed; therefore, a percentage of traffic associated with the development was included in the No-Build (2024) analysis.

Sweetwater – Located on US 64 in Wake County, this mixed-use development is projected to consist of 375 single-family homes, 60 condominiums, 50,000 sf of office, 200,000 sf of retail, 7,000 sf of high-turnover restaurant, 3,000 sf of fast food with drive through window, and a drive-in bank with 4 lanes and be constructed by 2019. A TIA was prepared by Ramey Kemp & Associates and submitted on December 18, 2014. As detailed in the report the development is projected to generate 18,360 daily site trips, with 914 trips (457 entering, 457 exiting) occurring in the AM peak hour and 1,736 trips (865 entering, 871 exiting) occurring in the PM peak hour. These trips were distributed to the study area based on the assumed distribution patterns in the report. Field visits indicate that approximately 80% of the residential and 0% of the mixed-use phases have been constructed; therefore, a percentage of traffic associated with the development was included in the No-Build (2024) analysis.

Buckhorn Preserve (Goodwin-MacNair) – Located on the east side of Richardson Road, just north of M. Zion Church Road, this residential development is projected to consist of 347 single-family homes and be constructed by 2020. A TIA was prepared by VHB



and submitted to the Town on June 26, 2015, with an addendum submitted on August 3, 2015. As detailed in the report, the development is projected to generate 3,299 daily site trips, with 253 trips (63 entering, 190 exiting) occurring in the AM peak hour and 322 trips (203 entering, 119 exiting) occurring in the PM peak hour. These trips were distributed to the study area based on the assumed distribution patterns in the report. Field visits indicate that the development has not been fully constructed; therefore, a percentage of the traffic associated with the development was included in the No-Build (2024) analysis.

Stillwater (Womble) - Located between Ragan Road and Richardson Road north of Humie Olive Road, this residential development is projected to consist of 303 single-family homes and be constructed by 2018. A TIA was prepared by Stantec and submitted to the Town on February 27, 2014. As detailed in the report, the development is projected to generate 2,912 daily site trips, with 221 trips (55 entering, 166 exiting) occurring in the AM peak hour and 285 trips (180 entering, 105 exiting) occurring in the PM peak hour. These trips were distributed to the study area based on the assumed distribution patterns in the report. Field visits indicated that the development has not been fully constructed; therefore, a percentage of the traffic associated with the development was included in the No-Build (2024) analysis.

Westford - Located on the north side of US 64 and east of Jenks Road, this residential development is projected to consist of 300 apartment units, 225 townhomes, and 90 single-family homes and be constructed by 2019. A TIA was prepared by Kimley-Horn and submitted to the Town on December 7, 2016. As detailed in the report, the development is projected to generate 4,188 daily site trips, with 323 trips (65 entering, 258 exiting) occurring in the AM peak hour and 396 trips (257 entering, 139 exiting) occurring in the PM peak hour. These trips were distributed to the study area based on the assumed distribution patterns in the report. Field visits indicated that the development has not been fully constructed; therefore, a percentage of the traffic associated with the development was included in the No-Build (2024) analysis.

Smith Farm – Located north of Olive Chapel Road, west of Kelly Road, and south of US 64, this mixed-use development is projected to consist of 430 single-family homes, 170 townhomes, 150 apartments, 100,000 sf of office, 150,000 sf of retail, 10,000 sf of pharmacy, 16,000 sf of high-turnover sit-down restaurant, 9,000 sf of fast-food restaurant, 12,000 sf of drive-in bank, and a gas station with 8 fueling positions and be constructed by 2021. A TIA was prepared by Ramey Kemp & Associates and submitted to the Town on November 24, 2015. As detailed in the report, the development is projected to generate 27,930 daily site trips, with 1,709 trips (847 entering, 862 exiting) occurring in the AM peak hour and 2,545 trips (1,301 entering, 1,244 exiting) occurring in the PM peak hour. These trips were distributed to the study area based on the assumed distribution patterns in the report. Field visits indicated that the development has not been fully constructed; therefore, a percentage of the traffic associated with the development was included in the No-Build (2024) analysis.



Linden (Pricewood Assemblage) – Located in the northwest quadrant of the intersection of Olive Chapel Road and Pricewood Lane, this residential development is projected to consist of 211 single-family homes and be constructed by 2022. A TIA was prepared by Ramey Kemp & Associates and submitted to the Town on August 31, 2016. As detailed in the report, the development is projected to generate 2,010 daily site trips, with 158 trips (40 entering, 118 exiting) occurring in the AM peak hour and 211 trips (133 entering, 78 exiting) occurring in the PM peak hour. These trips were distributed to the study area based on the assumed distribution patterns in the report. Field visits indicated that the development has not been fully constructed; therefore, a percentage of the traffic associated with the development was included in the No-Build (2024) analysis.

As for transportation improvements, mitigation requirements associated with Sweetwater are expected to include two new signals and additional turn lanes along US 64 at the Richardson Road and U-Turn East of Richardson Road intersections, and Smith Farm is committed to installing a new signal at the Olive Chapel Road and Richardson Road intersection once it is warranted.

Note that although significant traffic increases are expected due to the inclusion of background developments, an undiscounted annual traffic growth rate of three percent (3%) was applied to offset the impacts on traffic data collected under the Existing (2020) conditions with COVID-19 pandemic restrictions in place. The No-Build (2024) AM and PM peak hour volumes are shown in Figure 5.

Level of Service Analysis

Intersection levels of service analyses were performed for the typical weekday AM and PM peak hours using *Synchro/SimTraffic Professional Version 10*. A summary of the findings for the No-Build (2024) scenario LOS analysis can be found in Table 4. The full *Synchro/HCS* output for the No-Build scenario can be found in Appendix D.

As reported in Table 4, the study area is projected to experience traffic and delay increases, but the impacts will be substantially mitigated by the background transportation improvements. As a result, all of the signalized intersections and stop-controlled approaches in the study area are projected to operate at acceptable levels of service except that the stop-controlled northbound approach of Apex Barbecue Road at Olive Chapel Road is projected to decline to operate at LOS F in the PM peak hour.

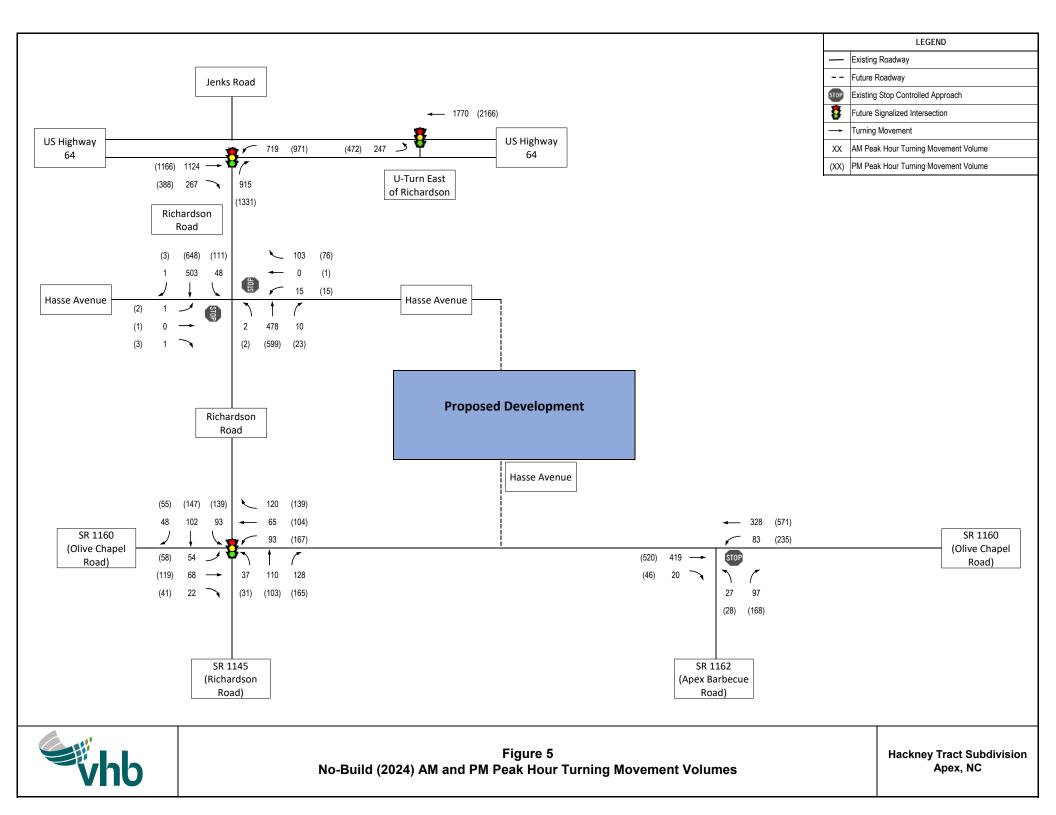


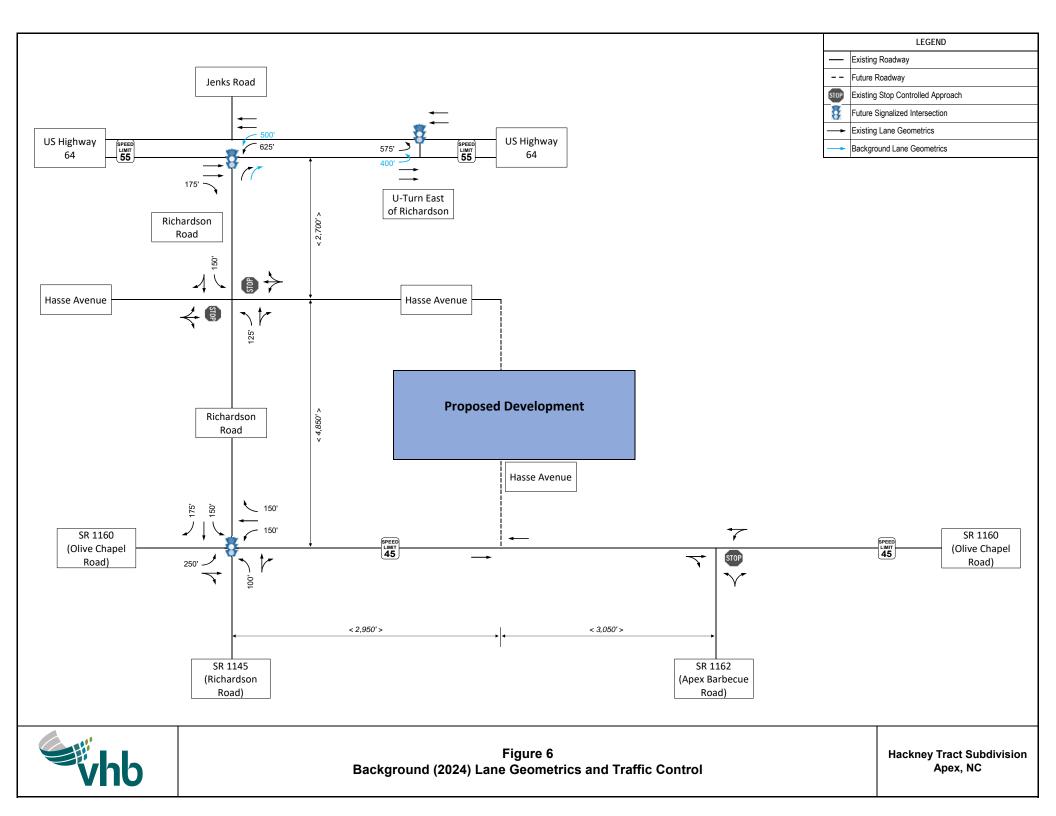
Table 4: No-Build (2024) LOS Results

Intersection and Approach	Control	No-Build (2024)		
moreosten and Approach	30114101	AM	PM	
Richardson Rd and Olive Chapel Rd		A (9.7)	B (11.8)	
Eastbound	Signal	A-9.7	B-10.7	
Westbound	oignai	B-10.3	B-12.0	
Northbound		B-10.2	B-12.7	
Southbound		A-8.4	B-11.5	
Apex Barbecue Rd and Olive Chapel Rd	TWSC	-	-	
Northbound		C-16.8	F-92.5	
Richardson Rd and Little Gem Ln/Hasse Ave	TWSC	-	-	
Eastbound		C-16.5	C-21.8	
Westbound		C-15.1	C-19.1	
Richardson Rd/WB Left-Over and US 64		C (20.7)	D (42.0)	
Eastbound	Signal	C-20.7	D-51.3	
Northbound		C-28.2	D-47.4	
Southbound		B-10.9	B-19.6	
U-Turn East of Richardson Rd and US		В	С	
64	Signal	(11.8)	(27.6)	
Westbound	Signal	A-9.6	C-20.5	
Northbound		C-27.8	E-59.9	

LEGEND: **X (XX)** = Overall intersection LOS (intersection delay in sec/veh);

X - XX = approach LOS - approach delay in sec/veh







4

Build (2024) Conditions

There are plans to construct the proposed Hackney Tract Subdivision on the north side of Olive Chapel Road, east of the newly completed Richardson Road, in Apex, NC (Figure 1). The proposed Hackney Tract Subdivision is planned to consist of up to 100 single-family and 133 multi-family townhomes with full build-out expected in 2024.

Trip Generation

Trip generation was conducted based on the most appropriate corresponding trip generation codes included in the *ITE Trip Generation Manual, 10th Edition* and the suggested method of calculation in the NCDOT's "Rate vs. Equation" Spreadsheet. To provide a conservative analysis, no transit, walking, or bicycling reductions will be applied.

Table 5 summarizes the estimated trip generation for the proposed Hackney Tract Subdivision for weekday AM and PM peak hours.

Table 5: Trip Generation Rates

Land Use	T J T T	IIie	ADT	AM	Peak I	Iour	PM	Peak H	Iour
Code	Land Use	Unit	ADT	Enter	Exit	Total	Enter	Exit	Total
210	Single-Family Detached Housing	100 du	1,040	19	57	76	64	38	102
220	Multi-Family Housing (Low-Rise)	133 du	965	14	49	63	48	28	76
	Development Total		2,005	33	106	139	112	66	178

In total, the proposed Hackney Tract Subdivision is projected to generate 2,005 daily trips with 139 trips (33 entering, 106 exiting) occurring in the AM peak hour and 178 trips (112 entering, 66 exiting) occurring the PM peak hour.

Traffic Distribution and Assignment

As shown on the conceptual site plan (Figure 2), the development will be accessed through one full movement access along Olive Chapel Road:



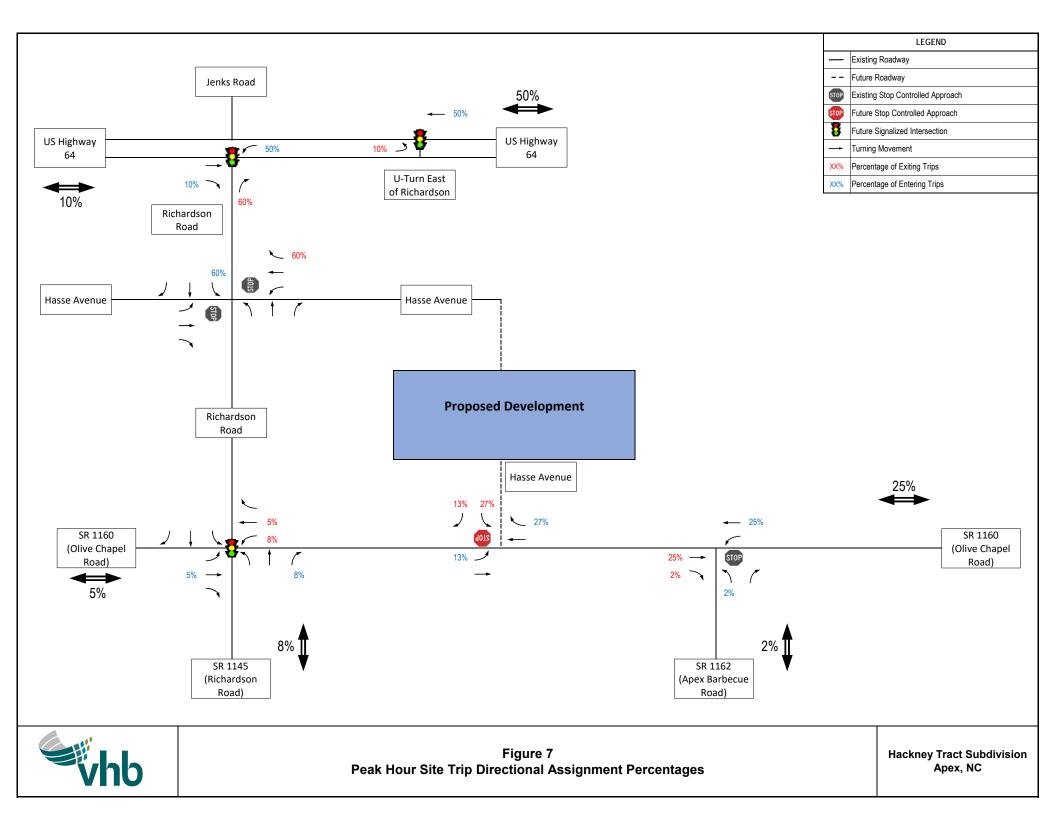
 Access #1: full movement access on Olive Chapel Road, approximately 2,500 feet east of Richardson Road

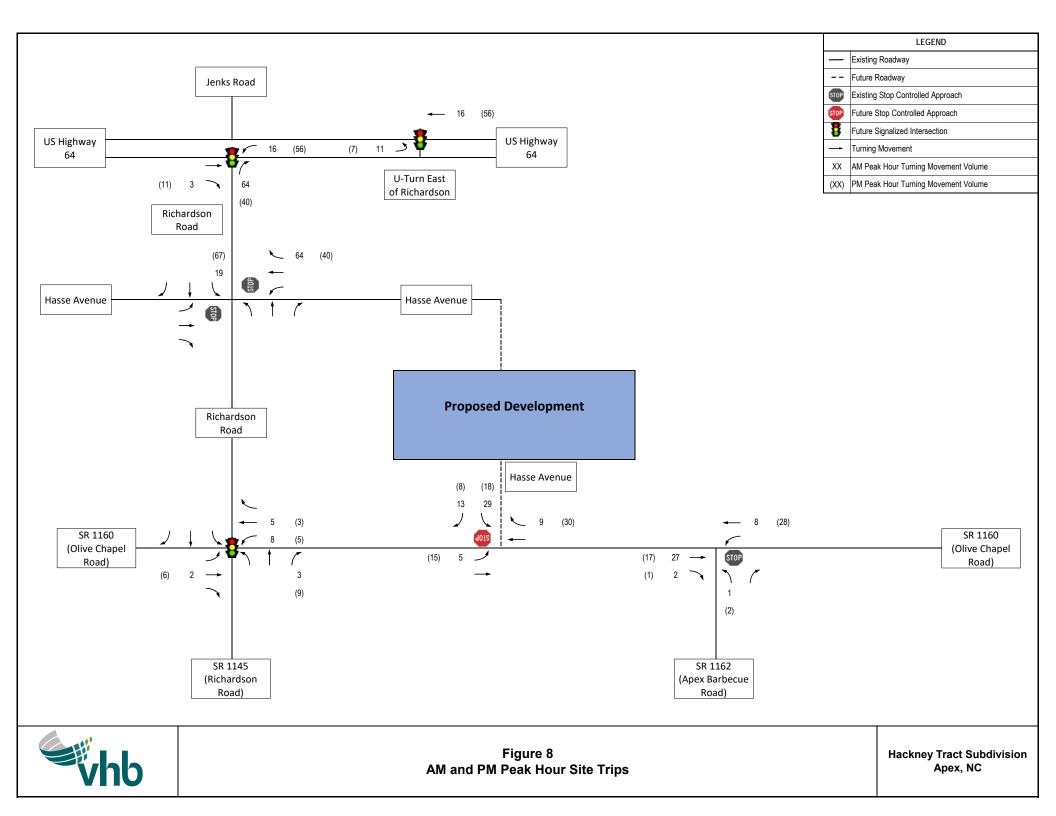
In addition, access will be provided via Hasse Avenue extension to the north to Richardson Road, and cross-connections will be provided via local street extensions to the west to Smith Farm. Potential traffic reductions due to cross-connections are not accounted for in this TIA to be conservative.

Based on agreements with the Town of Apex and NCDOT through the Memorandum of Understanding (Appendix A), the directional distribution percentages are as follows:

- from/to the east via US Highway 64 50%
- from/to the west via US Highway 64 10%
- from/to the east via Olive Chapel Road 25%
- from/to the west via Olive Chapel Road 5%
- from/to the south via Richardson Road 8%
- from/to the south via Apex Barbecue Road 2%

A graphic illustration of the proposed peak hour directional distribution percentages is shown in Figure 7, with the resulting site trips shown in Figure 8.







Level of Service Analysis

The Build (2024) analysis scenario includes the No-Build (2024) traffic as well as site-generated trips from the proposed development. Figure 9 depicts the turning movement volumes used in the Build (2024) scenario analysis.

Intersection levels of service analyses were performed for the typical weekday AM and PM peak hours using *Synchro/SimTraffic Professional Version 10*. Table 6 summarizes the LOS results for the Build (2024) scenario and Appendix D contains the full *Synchro/HCS* reports of the analysis.

As reported in Table 6, the stop-controlled northbound approach of Apex Barbeque Road at Olive Chapel Road is projected to continue to operate at failing levels of services in the PM peak hour with delay increases. The rest of the intersections included in the study area are projected to continue operating at acceptable levels of service during both peak hours. The planned stop-controlled Future Access #1 is projected to operate at LOS C in the AM peak hour and LOS D in the PM peak hour.

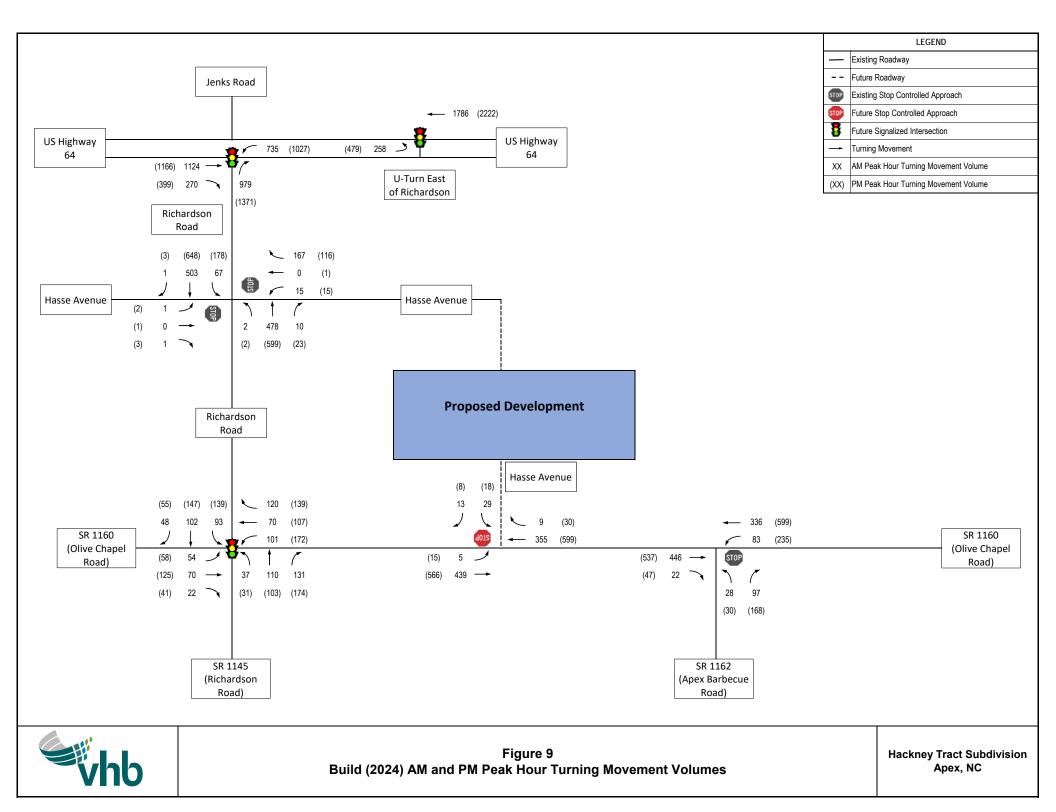


Table 6: Build (2024) LOS Results

Intersection and Approach	Control	Build	(2024)
intersection and Approach	Control	AM	PM
Bishandara Bd and Olive Changl Bd		Α	В
Richardson Rd and Olive Chapel Rd		(9.8)	(12.0)
Eastbound	Signal	A-9.8	B-10.9
Westbound	Signai	B-10.5	B-12.2
Northbound		B-10.3	B-13.0
Southbound		A-8.5	B-11.6
Apex Barbecue Rd and Olive Chapel Rd	TWSC	-	-
Northbound	1	C-17.9	F-134.5
Richardson Rd and Little Gem Ln/Hasse			
Ave	TWSC	-	-
Eastbound	1 W 3 C	C-19.1	D-32.0
Westbound		C-17.0	C-21.9
Richardson Rd/WB Left-Over and US 64		С	D
Richardson Rd/WB Left-Over and 03 64		(22.0)	(44.5)
Eastbound	Signal	C-23.7	E-56.0
Northbound		C-28.5	D-50.1
Southbound		A-9.8	B-19.5
U-Turn East of Richardson Rd and US 64		В	С
0-Turn East of Richardson Rd and 03 64	Signal	(12.5)	(30.9)
Westbound	Signai	B-10.5	C-24.1
Northbound		C-26.5	E-62.1
Olive Chapel Rd & Hasse Ave/Future		_	
Access #1	TWSC		-
Southbound		C-16.1	D-25.0

 $\label{eq:legender} \mbox{LEGEND: } X \mbox{ (XX) = Overall intersection LOS (intersection delay in sec/veh);}$

X - XX = approach LOS - approach delay in sec/veh





Findings and Conclusions

As indicated in the traffic operations analyses, the proposed Hackney Tract Subdivision is projected to have minimum impacts on traffic operations of the surrounding roadway network and intersections. Nevertheless, the following roadway improvements are recommended to improve traffic operations and safety:

SR 1160 (Olive Chapel Road) and Future Access #1/Hasse Avenue Extension (unsignalized, full movement)

Future Access #1 is projected to operate at acceptable levels of service during the AM and PM peak hour with a two-lane cross-section. Although traffic volumes are not projected to automatically warrant turn lanes on Olive Chapel Road, dedicated turn lanes should be provided with the required frontage widening to meet the Town of Apex Comprehensive Transportation Plan standards. Therefore, the following site access configuration and transportation improvements are recommended at this intersection:

- Construct Future Access #1 to consist of one inbound lane and one outbound lane.
- Provide a dedicated left-turn lane on eastbound Olive Chapel Road with 100 feet of storage length and appropriate taper.
- Provide a dedicated right-turn lane on westbound Olive Chapel Road with 100 feet of storage length and appropriate taper.

SR 1160 (Olive Chapel Road) and SR 1162 (Apex Barbecue Road) (unsignalized)

Traffic analysis indicated that the northbound approach of Apex Barbecue Road is projected to operate at LOS F in the PM peak hour under the No-Build and Build conditions. The intersection is not anticipated to meet warrants for installing a new traffic signal, while options for adding new turn lanes are limited due to the skewed angle of intersection on a curve of Olive Chapel Road and potential right-of-way/drainage restrictions. As shown on the Apex Comprehensive Transportation Plan, this intersection is identified for future intersection realignment. Since site trips are anticipated to contribute less than 4% traffic increases in the AM and 3% in the PM at this intersection (increases of only 1 VPH in the AM peak hour and 2 VPH in the PM peak on the stop-controlled approach), improvement should not be required by this development based on the Town of Apex UDO. Nevertheless, alternative traffic control method (such as AWSC), if warranted by crash analysis, may be considered



before this intersection is realigned in the future based on the Town of Apex CTP.

The rest of study area intersections are expected to operate acceptably. Therefore, no mitigation is required. A summary of the findings for the analysis scenarios is shown in Table 7, and the resulting future lane configurations and traffic controls in 2024 are shown in Figure 10.

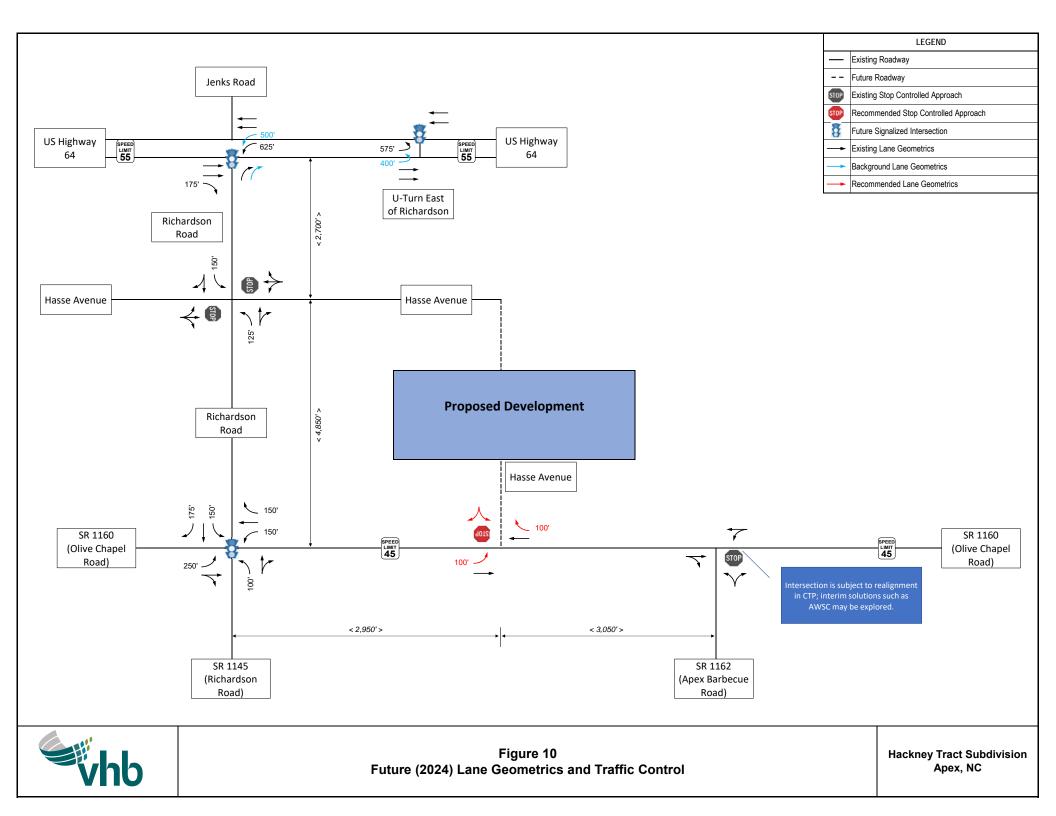


Table 7: Summary LOS Table

Intersection and Approach	Control	Existing	g (2020)	No-Build	d (2024)	Build ((2024)
		AM	PM	AM	PM	AM	РМ
Richardson Rd and Olive Chapel Rd		-	-	A (9.7)	B (11.8)	A (9.8)	B (12.0)
Eastbound	TWSC/			A-9.7	B-10.7	A-9.8	B-10.9
Westbound	Signal			B-10.3	B-12.0	B-10.5	B-12.2
Northbound		B-11.9	B-14.1	B-10.2	B-12.7	B-10.3	B-13.0
Southbound		B-11.7	C-15.5	A-8.4	B-11.5	A-8.5	B-11.6
Apex Barbecue Rd and Olive Chapel Rd	TWSC	-	-	-	-	-	-
Northbound		B-11.8	C-19.5	C-16.8	F-92.5	C-17.9	F-134.5
Richardson Rd and Little Gem Ln/Hasse Ave	TWSC	-	-	-	-	-	-
Eastbound	TWSC	A-9.8	B-10.2	C-16.5	C-21.8	C-19.1	D-32.0
Westbound		A-9.7	A-9.9	C-15.1	C-19.1	C-17.0	C-21.9
Richardson Rd/WB Left- Over and US 64	TWICC /	-	-	C (20.7)	D (42.0)	C (22.0)	D (44.5)
Eastbound	TWSC/ Signal			C-20.7	D-51.3	C-23.7	E-56.0
Northbound	0181141	C-23.5	C-23.3	C-28.2	D-47.4	C-28.5	D-50.1
Southbound		F-66.0	F-216.7	B-10.9	B-19.6	A-9.8	B-19.5
U-Turn East of Richardson Rd and US 64	TWSC/	-	-	B (11.8)	C (27.6)	B (12.5)	C (30.9)
Westbound	Signal			A-9.6	C-20.5	B-10.5	C-24.1
Northbound		B-14.2	C-18.2	C-27.8	E-59.9	C-26.5	E-62.1
Olive Chapel Rd & Hasse Ave/Future Access #1	TWSC	-	-	-	-	-	-
Southbound						C-16.1	D-25.0

 $\label{eq:legender} \mbox{LEGEND: } \textbf{X (XX)} = \mbox{Overall intersection LOS (intersection delay in sec/veh);}$

X - XX = approach LOS - approach delay in sec/veh





APPENDICES



APPENDIX A:

Memorandum of Understanding



To: Russell H. Dalton, PE
Public Works & Transportation
Town of Apex
73 Hunter Street
Apex, NC 27502

Date: November 12, 2020

Memorandum

Project #: 38504.25

From: Baohong Wan, PhD, PE Senior Project Manager Re: Hackney Tract Subdivision TIA

Memorandum of Understanding

This memorandum summarizes the assumptions for a Traffic Impact Analysis (TIA) prepared for the proposed Hackney Tract Subdivision on Olive Chapel Road, west of the newly completed Richardson Road, in Apex, NC. Based on the preliminary plan (attached), the development is to consist of a mix of single-family and multi-family townhome uses:

- 100 single family homes
- 133 townhomes

Access to the development is to be provided primarily through a collector street (Hasse Avenue Extension) planned across the property. In addition, cross-connections will be provided via several street extensions to Smith Farm.

Study Area

Based on our previous correspondence, the following existing and future study area intersections will be included for analysis under the AM and PM peak hour conditions:

- SR 1160 (Olive Chapel Road) and SR 1145 (Richardson Road) (unsignalized/future signalized)
- SR 1160 (Olive Chapel Road) and SR 1162 (Apex Barbecue Road) (unsignalized)
- Richardson Road and Hasse Avenue (unsignalized)
- US Highway 64 East at Richardson Road (unsignalized/future signalized)
- US Highway 64 West at U-turn east of Richardson Road (unsignalized/future signalized)
- SR 1160 (Olive Chapel Road) and Future Access #1/Hasse Avenue Extension (full movement access)

The signalized intersection of SR 1160 (Olive Chapel Road) and SR 1163 (Kelly Road) was initially considered, but it was excluded from the study area due to its distance from the project site and the fact that this intersection has recently been upgraded with new turn lanes and crosswalks, and traffic is expected to decrease at this intersection due to the newly completed Richardson Road connection.

Data Collection

As discussed with the Town of Apex and NCDOT, collecting new traffic data was preferred to reflect new traffic patterns with the recently completed Richard Road between Olive Chapel Road and US 64. Turning movement data at the study intersections were collected by VHB during the AM (7:00 AM - 9:00 AM) and PM (4:00 PM - 6:00 PM) peak periods in November 2020. Traffic counts were collected while area schools were partially open with the

From: Baohong Wan, PhD, PE Senior Project Manager

Ref: 38504.25 November 12, 2020

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COVID-19 restrictions. The Existing (2020) AM and PM peak hour turning movement volumes are shown in the Figure MOU-1.

Analysis Scenarios

In accordance with the Town of Apex's Unified Development Ordinance (UDO), a build-out year of 2024 will be analyzed. Therefore, weekday AM and PM peak hour analysis for the proposed development will be performed for four (4) scenarios:

- Existing (2020) Conditions
- Background (2024) Conditions
- Build (2024) Scenario
- Build (2024) Scenario with Improvements

Background Projects and Growth

As concurred by the Town of Apex, an annual growth rate of three percent (3%) will be applied to the existing year (2020) traffic to project future conditions (2024). In addition, the following approved developments are identified as within the study area, and will be included the future year traffic analysis:

- Saddlebrook (Lawrence Assemblage/Richardson West), TIA by RKA, November 2014, 75% completed
- Sweetwater, TIA by RKA December 2014, 80% completed for residential portion, 0% for mixed use
- Buckhorn Preserve (Goodwin-MacNair), TIA by VHB, June 2015, 50% completed
- Stillwater (Womble), TIA by Stantec, February 2014, 85% completed
- Westford, TIA by KHA, December 2016, 80% completed
- Smith Farm, TIA by RKA, November 2015, 75% completed for residential portion, 0% for mixed-use
- Linden (Pricewood Assemblage) TIA by RKA, August 2016, 15% completed

Note that although a significant number of trips are expected due to the approved developments, a 3% annual traffic growth rate will still be used to offset lower-than-normal traffic counts collected under the Existing (2020) conditions. Transportation improvements due to approved developments (particularly Sweetwater and Smith Farm) will be included in the future year analysis based on the transportation zoning conditions.

Trip Generation

Trip Generation will be conducted based on the most appropriate corresponding trip generation codes included in the *ITE Trip Generation Manual, 10th Edition.* Trip generation calculations will be based on the suggested method in the NCDOT's "Rate vs. Equation" spreadsheet. To provide a conservative analysis, no transit, walking, or bicycling reductions will be applied.

As shown in the preliminary trip generation results (attached), the proposed development is projected to generate 2,005 trips on a typical weekday with 139 trips occurring during the AM peak hour and 178 trips in the PM peak hour.

From: Baohong Wan, PhD, PE Senior Project Manager

Ref: 38504.25 November 12, 2020

Page 3



Land				AN	I Peak H	our	PM	I Peak H	our
Use Code ¹	Land Use	Unit	ADT	Enter	Exit	Total	Enter	Exit	Total
210	Single-Family Detached Housing	100 du	1,040	19	57	76	64	38	102
220	Multi-Family Housing (Low-Rise)	133 du	965	14	49	63	48	28	76
	Development Total		2,005	33	106	139	112	66	178

Notes:

- 1. Land Use Code and trip generation rates are based on ITE Trip Generation, 10th Edition
- 2. Trips are determined based on the suggested method in the NCDOT Rate Vs Equation Spreadsheet.

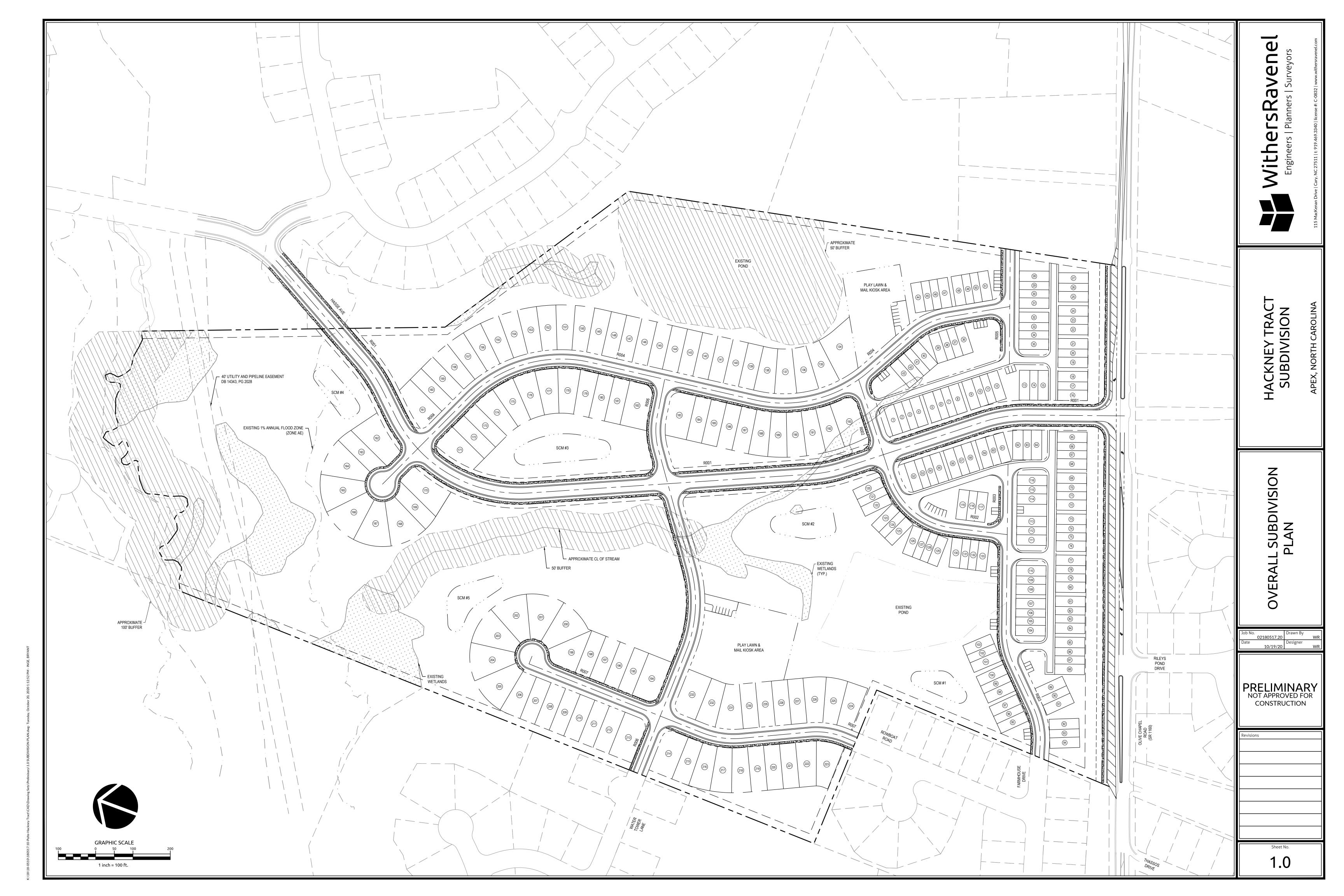
Trip Distribution and Assignment

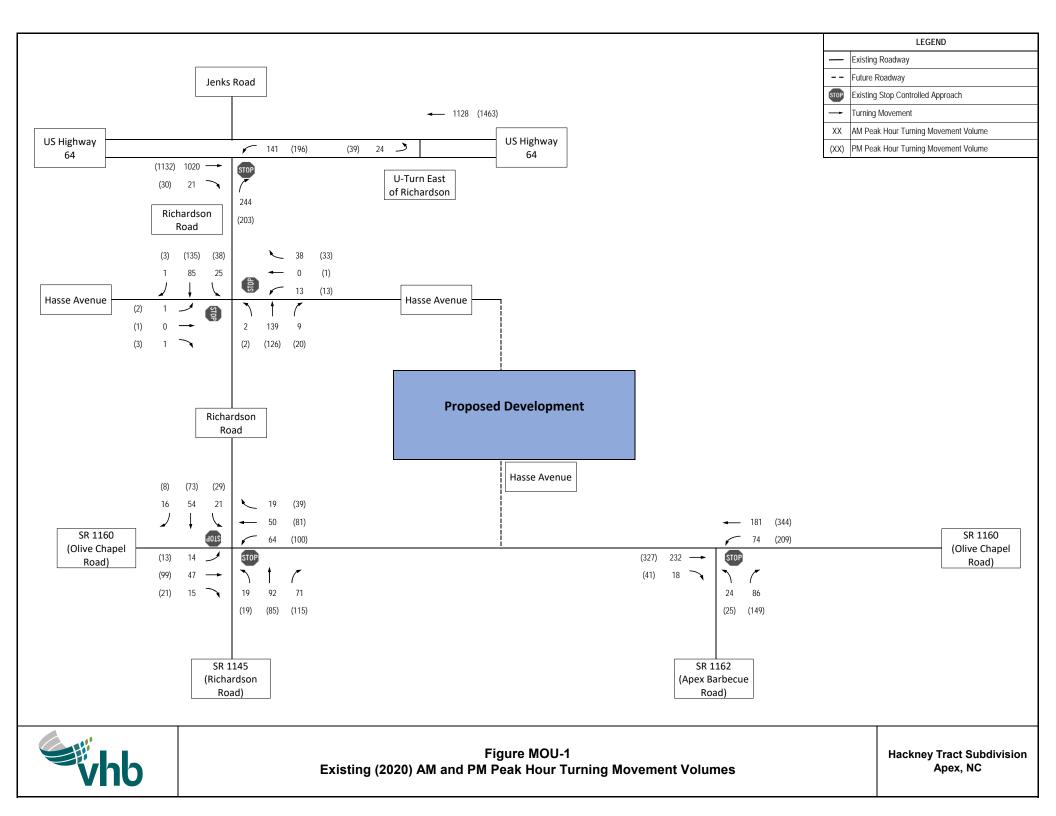
The site trips will be distributed in accordance with the existing traffic patterns and planned land uses in the vicinity of the study area. Based on the traffic data, the site trips will be distributed as follows:

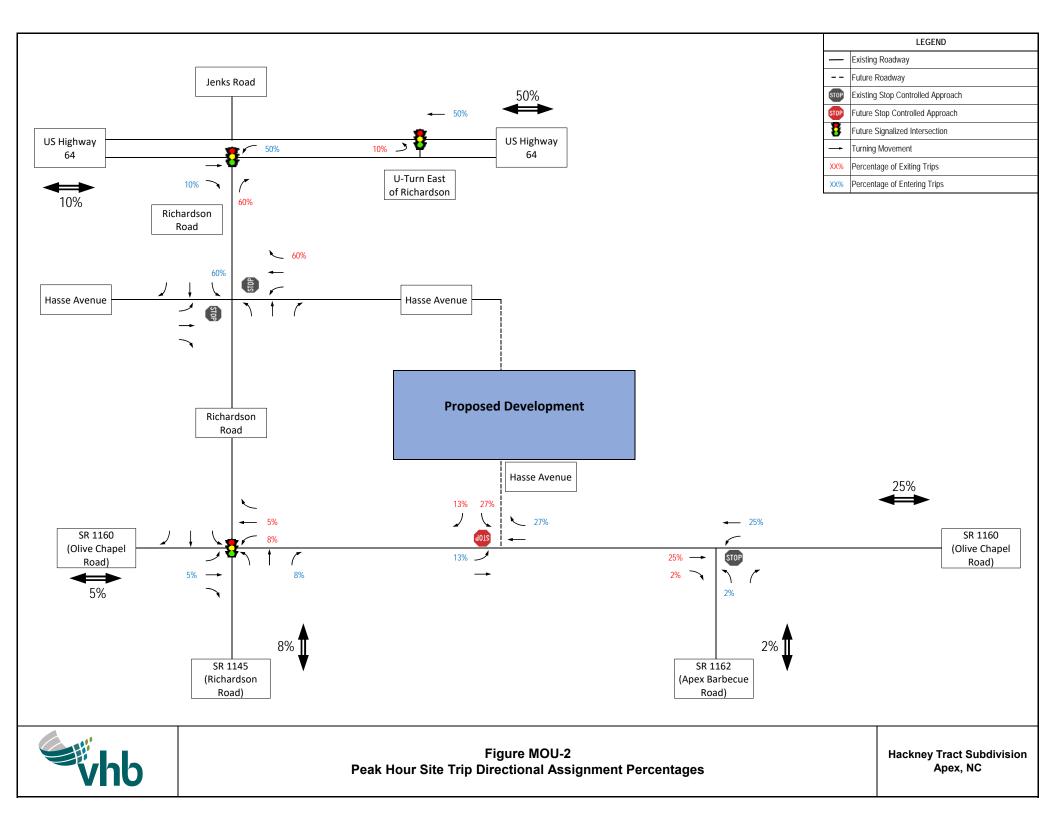
- from/to the east via US Highway 64 50%
- from/to the west via US Highway 64 10%
- from/to the east via Olive Chapel Road 25%
- from/to the west via Olive Chapel Road 5%
- from/to the south via Richardson Road 8%
- from/to the south via Apex Barbecue Road 2%

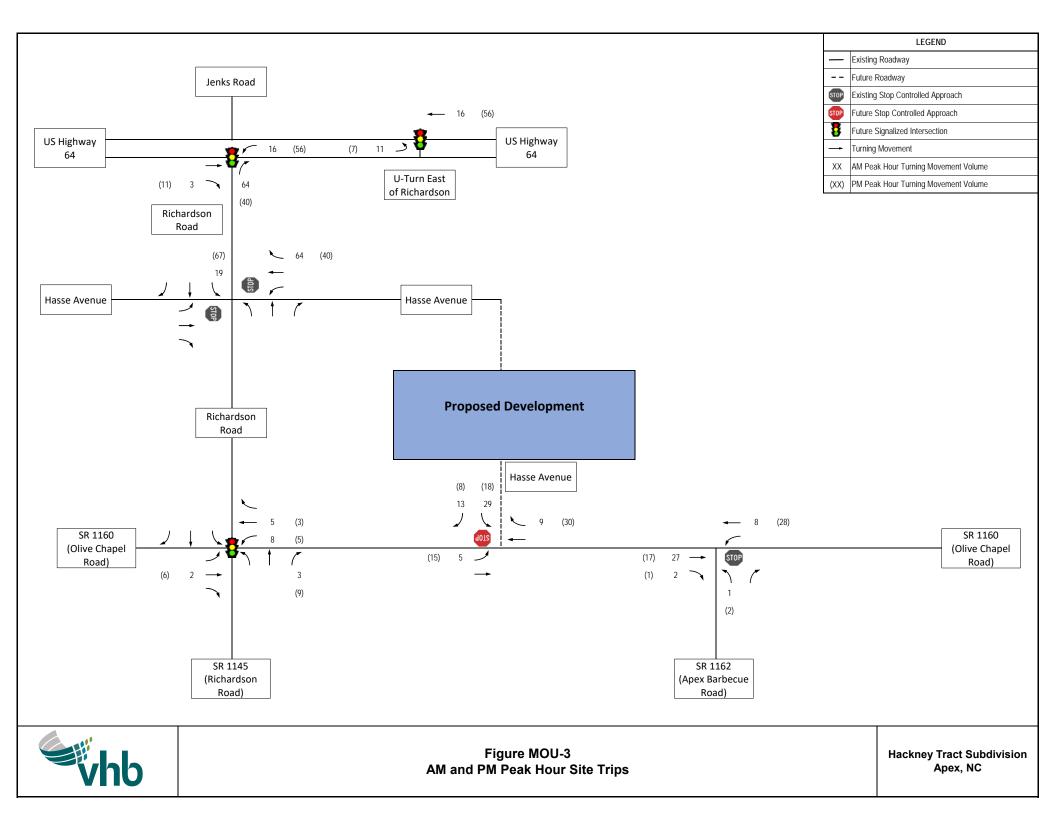
A graphic illustration of the proposed peak hour directional distribution percentages is shown in the attached Figure MOU-2, and the resulting AM and PM peak hour trips at each study intersection are shown in Figure MOU-3.

CC: Amy N. Neidringhaus, PE, NCDOT Highway Division 5 District 1











APPENDIX B:

Turning Movement Counts

Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: OliveChapel@ApexBarbecue

Site Code :

Start Date : 11/5/2020

Page No : 1

Groups Printed- Passenger Vehicles - Single Unit - TTST - Bicycles on Crosswalk - Pedestrians

		Drive		13 F HIII		ve Cha					ecue R	ycies o			pel Ro				
		South			OII	Westb		au	Ape	North		oau	Oil	Eastb		au			
Start Time	Left	Thru	Right	Peds	Left		Right	Peds	Left	Thru		Peds	Left	Thru		Peds	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	0	0	9	18	0	0	2	0	21	1	0	35	3	0	Exciu. Total	88	89
07:15 AM	0	0	0	0	9	20	0	0	4	0	19	1	0	44	5	0	1	101	102
07:30 AM	0	0	0	0	13	24	0	0	4	0	29	3	0	46	5	0	3	121	124
07:45 AM	0	0	0	0	21	45	0	0	6	0	30	4	0	57	10	0	4	169	173
Total	0	0	0	0	52	107	0	0	16	0	99	9	0	182	23	0	9	479	488
	-	-	-	-			-	_		_			-			-			
08:00 AM	0	0	0	0	13	43	0	0	7	0	19	5	0	52	2	0	5	136	141
08:15 AM	0	0	0	0	14	51	0	0	7	0	23	0	0	58	5	0	0	158	158
08:30 AM	0	0	0	0	16	36	0	0	6	0	27	2	0	54	4	0	2	143	145
08:45 AM	0	0	0	0	31	51	0	0	4	0	17	0	0	68	7	0	0	178	178
Total	0	0	0	0	74	181	0	0	24	0	86	7	0	232	18	0	7	615	622
*** BREAK ***																			
04:00 DM	0	1	0	0	1 47	70	0	0		0	27	1	0	/ 1	0	0	l 1	225	22/
04:00 PM 04:15 PM	0	0	0	0	47 39	72 87	0 1	0	6 12	0	27 30	1 0	0	64 52	8 10	0	1 0	225 231	226
	0	0	0	0	43		0	-	4	0		- 1	0	52 74		-	1		231
04:30 PM 04:45 PM	0	0	0	0	32	72 76	0	0	7	0	32 30	6 5	0	74 70	6 9	0	6 5	231 224	237 229
Total	0	1	0	0	161	307	1	0	29	0	119	12	0	260	33	0	12	911	923
TUIdI	U	1	U	U	101	307		U	29	U	119	12	U	200	აა	U	12	911	923
05:00 PM	0	0	1	0	49	94	0	0	8	0	32	4	0	83	8	0	4	275	279
05:15 PM	0	0	0	0	59	81	0	0	9	0	42	8	0	91	8	0	8	290	298
05:30 PM	0	0	0	0	50	85	0	0	6	0	32	9	0	83	8	0	9	264	273
05:45 PM	0	0	0	0	51	84	0	0	2	0	43	4	0	70	17	0	4	267	271
Total	0	0	1	0	209	344	0	0	25	0	149	25	0	327	41	0	25	1096	1121
0 17.11	0	4		0	101	000	4	•	0.4	0	450	F0	0	1001	445	0	l 50	0101	0454
Grand Total	0	1	1	0	496	939	1	0	94	0	453	53	0	1001	115	0	53	3101	3154
Apprch %	0	50 0	50		34.5 16	65.4 30.3	0.1		17.2 3	0	82.8		0	89.7 32.3	10.3 3.7		1.7	98.3	
Total % Passenger Vehicles	0	1	<u>0</u> 1		476	916	<u>0</u> 1		93	0	14.6 446		0	981	115		0	96.3	3030
9	0	100	100	0	96	916 97.6	100	0	98.9	0	98.5	0	0	981	100	0	0	0	96.1
% Passenger Vehicles Single Unit	0	0	0		20	20	0	0	70.7	0	70.5	- 0	0	19	0	- 0	0	0	67
% Single Unit	0	0	0	0	4	2.1	0	0	1.1	0	1.5	0	0	1.9	0	0	0	0	2.1
TTST	0	0	0		0	3	0	- 0	0	0	0		0	1.7	0	- 0	0	0	4
% TTST	0	0	0	0	0	0.3	0	0	0	0	0	0	0	0.1	0	0	0	0	0.1
Bicycles on Crosswalk	0	0	0		0	0	0		0	0	0		0	0	0		0	0	4
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	7.5	0	0	0	0	0	0	0.1
Pedestrians	0	0	0		0	0	0		0	0	0		0	0	0		0	0	49
% Pedestrians	0	0	0	0	0	0	0	0	0	0	0	92.5	0	0	0	0	0	0	1.6

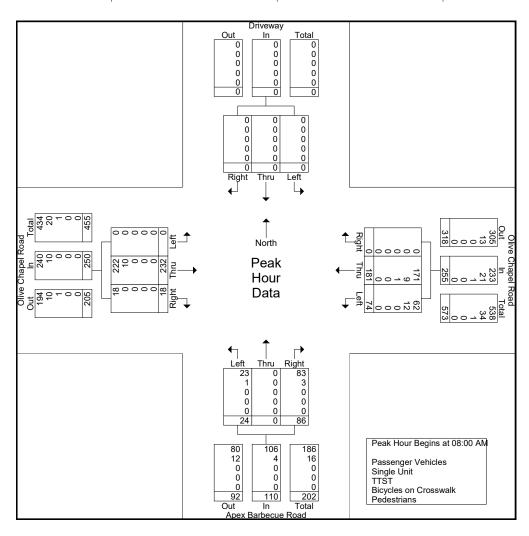
Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: OliveChapel@ApexBarbecue

Site Code :

Start Date : 11/5/2020

		Driv	eway		0	live Ch	apel Ro	ad	Ap	ex Bark	ecue R	oad	0	live Cha	apel Ro	ad	
			bound				bound		7.4		bound				oound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis From	07:00 A	M to 11	:45 AM - F	eak 1 of	1											
Peak Hour for Entire	e Intersecti	on Begins	s at 08:00	AM													
08:00 AM	0	0	0	0	13	43	0	56	7	0	19	26	0	52	2	54	136
08:15 AM	0	0	0	0	14	51	0	65	7	0	23	30	0	58	5	63	158
08:30 AM	0	0	0	0	16	36	0	52	6	0	27	33	0	54	4	58	143
08:45 AM	0	0	0	0	31	51	0	82	4	0	17	21	0	68	7	75	178
Total Volume	0	0	0	0	74	181	0	255	24	0	86	110	0	232	18	250	615
% App. Total	0	0	0		29	71	0		21.8	0	78.2		0	92.8	7.2		
PHF	.000	.000	.000	.000	.597	.887	.000	.777	.857	.000	.796	.833	.000	.853	.643	.833	.864
Passenger Vehicles	0	0	0	0	62	171	0	233	23	0	83	106	0	222	18	240	579
% Passenger Vehicles	0	0	0	0	83.8	94.5	0	91.4	95.8	0	96.5	96.4	0	95.7	100	96.0	94.1
Single Unit	0	0	0	0	12	9	0	21	1	0	3	4	0	10	0	10	35
% Single Unit	0	0	0	0	16.2	5.0	0	8.2	4.2	0	3.5	3.6	0	4.3	0	4.0	5.7
TTST	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
% TTST	0	0	0	0	0	0.6	0	0.4	0	0	0	0	0	0	0	0	0.2
Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



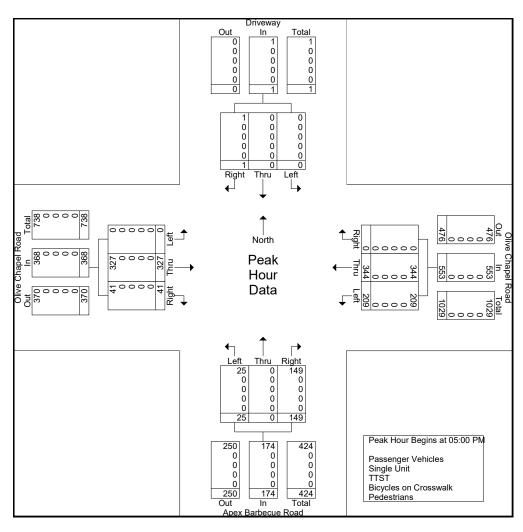
Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: OliveChapel@ApexBarbecue

Site Code :

Start Date : 11/5/2020

		Driv	eway		0	live Ch	apel Ro	ad	Ap	ex Barb	ecue R	oad	0	live Ch	apel Ro	ad	
		South	bound				bound		-	North	bound				oound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis From	12:00 F	PM to 05	:45 PM - P	eak 1 of	1											
Peak Hour for Entire	e Intersect	ion Begin:	s at 05:00	PM													
05:00 PM	0	0	1	1	49	94	0	143	8	0	32	40	0	83	8	91	275
05:15 PM	0	0	0	0	59	81	0	140	9	0	42	51	0	91	8	99	290
05:30 PM	0	0	0	0	50	85	0	135	6	0	32	38	0	83	8	91	264
05:45 PM	0	0	0	0	51	84	0	135	2	0	43	45	0	70	17	87	267
Total Volume	0	0	1	1	209	344	0	553	25	0	149	174	0	327	41	368	1096
% App. Total	0	0	100		37.8	62.2	0		14.4	0	85.6		0	88.9	11.1		
PHF	.000	.000	.250	.250	.886	.915	.000	.967	.694	.000	.866	.853	.000	.898	.603	.929	.945
Passenger Vehicles	0	0	1	1	209	344	0	553	25	0	149	174	0	327	41	368	1096
% Passenger Vehicles	0	0	100	100	100	100	0	100	100	0	100	100	0	100	100	100	100
Single Unit	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Single Unit	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TTST	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% TTST	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: OliveChapel@Richardson

Site Code :

Start Date : 11/5/2020

Page No : 1

Groups Printed- Passenger Vehicles - Single Unit - TTST - Bicycles on Crosswalk - Pedestrians

	ъ.				ea- Pas														
	RI		on Roa	ıa	Oii	ve Cha		ad	RI	chards		aa	Oli		pel Ro	ad			
Ot - ut Time -	1 - 64	South		D. J.	1 - 64	Westb		D. J.	1 - 64	Northk		D- d-	1 - 64	Eastb		D. J.			
Start Time	Left	Thru	Right		Left		Right	Peds	Left		Right	Peds	Left		Right		Exclu. Total	Inclu. Total	Int. Total
07:00 AM	3	14	1	0	9	8	ı	1	6	14	5	0	I A	8	4	0	0		75 05
07:15 AM	2	8	7	0	4	14	3 7	0	6	23	11	0	4	9	4	0	0	95	95
07:30 AM	0	15	2	0	8	9	,	1	6	24	10	2	8	23	3	1	0	119	119
07:45 AM	5	14	4	0	13	16	4	0	5	15	11	0	0	11	2	0	0	100	100
Total	10	51	14	0	34	47	15	2	23	76	37	2	13	51	13	1	0	389	389
MA 00:80	3	11	3	0	14	17	5	1	4	23	14	4	3	8	2	0	0	112	112
08:15 AM	3	18	4	0	15	9	5	1	4	30	17	2	2	14	3	0	0	127	127
08:30 AM	4	13	3	2	15	14	6	0	6	16	18	0	5	12	4	0	2	116	118
08:45 AM	11	12	6	1	20	10	3	0	5	23	22	0	4	13	6	0	1	135	136
Total	21	54	16	3	64	50	19	2	19	92	71	6	14	47	15	0	3	490	493
*** BREAK ***																			
04:00 PM	6	19	2	0	27	18	7	0	6	14	13	1	4	26	1	0	0	144	144
04:15 PM	4	13	7	0	22	30	9	4	3	16	13	0	1	18	2	3	0	145	145
04:30 PM	12	23	0	2	16	20	8	2	7	12	23	0	5	21	3	0	2	152	154
04:45 PM	5	23	9	1	21	18	12	0	3	15	18	0	1	21	3	0	1	149	150
Total	27	78	18	3	86	86	36	6	19	57	67	1	11	86	9	3	3	590	593
05:00 PM	6	23	2	1	28	23	7	0	4	21	39	1	3	18	4	0	1	179	180
05:15 PM	8	17	2	0	29	30	9	1	4	26	29	0	3	21	3	0	0	182	182
05:30 PM	4	18	4	1	19	12	16	0	9	23	26	0	2	28	6	0	1	167	168
05:45 PM	11	15	0	1	24	16	7	0	2	15	21	0	5	32	8	0	1	156	157
Total	29	73	8	3	100	81	39	1	19	85	115	1	13	99	21	0	3	684	687
Grand Total	87	256	56	9	284	264	109	11	80	310	290	10	51	283	58	4	9	2153	2162
Apprch %	21.8	64.2	14		42.5	39.5	16.3	1.6	11.6	44.9	42	1.4	12.9	71.5	14.6	1			
Total %	4	11.9	2.6		13.2	12.3	5.1	0.5	3.7	14.4	13.5	0.5	2.4	13.1	2.7	0.2	0.4	99.6	
Passenger Vehicles	86	247	45		276	257	107	0	79	298	283	0	47	280	56	0	0	0	2061
% Passenger Vehicles	98.9	96.5	80.4	0	97.2	97.3	98.2	0	98.8	96.1	97.6	0	92.2	98.9	96.6	0	0	0	95.3
Single Unit	1	7	9		8	6	2	0	0	8	7	0	4	3	1	0	0	0	56
% Single Unit	1.1	2.7	16.1	0	2.8	2.3	1.8	0	0	2.6	2.4	0	7.8	1.1	1.7	0	0	0	2.6
TTST	0	2	2		0	1	0	0	1	4	0	0	0	0	1	0	0	0	11
% TTST	0	0.8	3.6	0	0	0.4	0	0	1.2	1.3	0	0	0	0	1.7	0	0	0	0.5
Bicycles on Crosswalk	0	0	0		0	0	0	0	0	0	0	2	0	0	0	0	0	0	2
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	20	0	0	0	0	0	0	0.1
Pedestrians	0	0	0		0	0	0	11	0	0	0	8	0	0	0	4	0	0	32
% Pedestrians	0	0	0	100	0	0	0	100	0	0	0	80	0	0	0	100	0	0	1.5

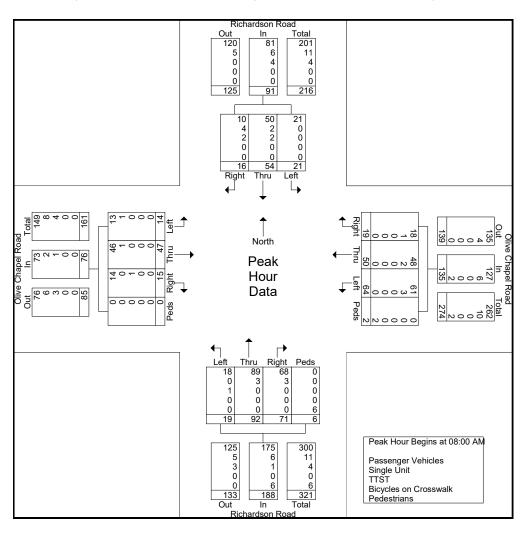
Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: OliveChapel@Richardson

Site Code :

Start Date : 11/5/2020

	Ri	chards	son Ro	ad		Olive	Chape	l Road			Richa	ardson	Road			Olive	Chape	l Road		
		South	bound			W	estbou	ınd			No	rthbou	ınd			E	astbou	nd		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Ana	alysis Fr	om 07:	00 AM	to 11:45	AM - Pe	eak 1 o	f 1													
Peak Hour for En	tire Inters	section E	Begins at	08:00 AM																
08:00 AM	3	11	3	17	14	17	5	1	37	4	23	14	4	45	3	8	2	0	13	112
08:15 AM	3	18	4	25	15	9	5	1	30	4	30	17	2	53	2	14	3	0	19	127
08:30 AM	4	13	3	20	15	14	6	0	35	6	16	18	0	40	5	12	4	0	21	116
08:45 AM	11	12	6	29	20	10	3	0	33	5	23	22	0	50	4	13	6	0	23	135
Total Volume	21	54	16	91	64	50	19	2	135	19	92	71	6	188	14	47	15	0	76	490
% App. Total	23.1	59.3	17.6		47.4	37	14.1	1.5		10.1	48.9	37.8	3.2		18.4	61.8	19.7	0		
PHF	.477	.750	.667	.784	.800	.735	.792	.500	.912	.792	.767	.807	.375	.887	.700	.839	.625	.000	.826	.907
Passenger Vehicles	21	50	10	81	61	48	18	0	127	18	89	68	0	175	13	46	14	0	73	456
% Passenger Vehicles	_	_			_	_		_		_	_	_	_				_	_	_	
Single Unit	0	2	4	6	3	2	1	0	6	0	3	3	0	6	1	1	0	0	2	20
% Single Unit	0	3.7	25.0	6.6	4.7	4.0	5.3	0	4.4	0	3.3	4.2	0	3.2	7.1	2.1	0	0	2.6	4.1
TTST	0	2	2	4	0	0	0	0	0	1	0	0	0	1	0	0	1	0	1	6
% TTST	0	3.7	12.5	4.4	0	0	0	0	0	5.3	0	0	0	0.5	0	0	6.7	0	1.3	1.2
Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	2	2	0	0	0	6	6	0	0	0	0	0	8
% Pedestrians	0	0	0	0	0	0	0	100	1.5	0	0	0	100	3.2	0	0	0	0	0	1.6



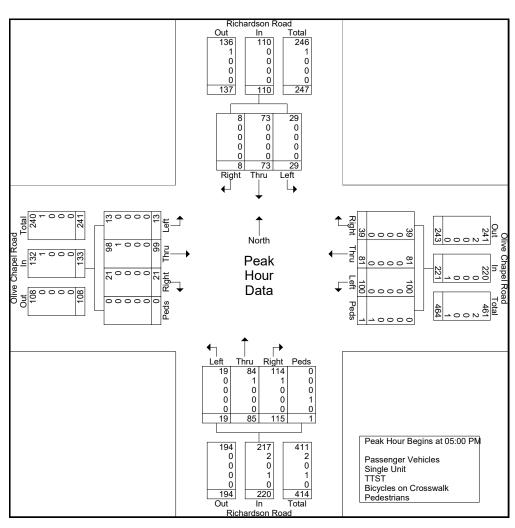
Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: OliveChapel@Richardson

Site Code :

Start Date : 11/5/2020

	Ri	chard	son Ro	ad		Olive	Chape	l Road			Rich	ardson	Road			Olive	Chape	l Road		ĺ
			bound				estbo					orthbo					astbou			
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Ana						eak 1 o	f 1													
Peak Hour for En	tire Inters	section E	Begins at	05:00 PM																
05:00 PM	6	23	2	31	28	23	7	0	58	4	21	39	1	65	3	18	4	0	25	179
05:15 PM	8	17	2	27	29	30	9	1	69	4	26	29	0	59	3	21	3	0	27	182
05:30 PM	4	18	4	26	19	12	16	0	47	9	23	26	0	58	2	28	6	0	36	167
05:45 PM	11	15	0	26	24	16	7	0	47	2	15	21	0	38	5	32	8	0	45	156
Total Volume	29	73	8	110	100	81	39	1	221	19	85	115	1	220	13	99	21	0	133	684
% App. Total	26.4	66.4	7.3		45.2	36.7	17.6	0.5		8.6	38.6	52.3	0.5		9.8	74.4	15.8	0		
PHF	.659	.793	.500	.887	.862	.675	.609	.250	.801	.528	.817	.737	.250	.846	.650	.773	.656	.000	.739	.940
Passenger Vehicles	29	73	8	110	100	81	39	0	220	19	84	114	0	217	13	98	21	0	132	679
% Passenger Vehicles																				
Single Unit	0	0	0	0	0	0	0	0	0	0	1	1	0	2	0	1	0	0	1	3
% Single Unit	0	0	0	0	0	0	0	0	0	0	1.2	0.9	0	0.9	0	1.0	0	0	0.8	0.4
TTST	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% TTST	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	0	1
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	100	0.5	0	0	0	0	0	0.1
Pedestrians	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	1
% Pedestrians	0	0	0	0	0	0	0	100	0.5	0	0	0	0	0	0	0	0	0	0	0.1



Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: Richardson@Hasse

Site Code :

Start Date : 11/5/2020

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Groups Printed- Passenger Vehicles - Single Unit - TTST - Bicycles on Crosswalk - Pedestrians

Richardson Road Southbound Westbound Westbound Richardson Road Northbound Eastbound Eastbound
Start Time Left Thru Right Peds Exclu. Total Inct. Total Inct. Total O7:00 AM 3
07:00 AM 3 16 0 0 2 0 6 2 0 22 0
07:30 AM 2 17 0 0 1 0 14 0 2 36 3 1 3 1 0 1 2 79 8 07:45 AM 5 23 0 0 3 0 13 1 0 26 2 0 1 0 1 1 2 74 7 Total 16 72 0 2 7 0 41 4 2 120 6 1 4 1 1 2 9 270 27 08:00 AM 9 17 0 0 2 0 11 0 1 35 3 0 0 0 0 1 1 78 7 08:05 AM 4 27 1 0 4 0 9 1 0 29 4 0 0 0 0 0 1 2 78
07:45 AM 5 23 0 0 3 0 13 1 0 26 2 0 1 0 1 1 2 74 7 Total 16 72 0 2 7 0 41 4 2 120 6 1 4 1 1 2 9 270 27 08:00 AM 9 17 0 0 2 0 11 0 1 35 3 0 0 0 0 1 1 78 7 08:15 AM 4 27 1 0 4 0 9 1 0 29 4 0 0 0 0 1 2 78 8
Total 16 72 0 2 7 0 41 4 2 120 6 1 4 1 1 2 9 270 27 08:00 AM 9 17 0 0 2 0 11 0 1 35 3 0 0 0 0 1 1 78 7 08:15 AM 4 27 1 0 4 0 9 1 0 29 4 0 0 0 0 1 2 78 8
08:00 AM 9 17 0 0 2 0 11 0 1 35 3 0 0 0 0 1 1 78 7 08:15 AM 4 27 1 0 4 0 9 1 0 29 4 0 0 0 0 1 2 78 8
08:15 AM 4 27 1 0 4 0 9 1 0 29 4 0 0 0 0 1 2 78 8
08:30 AM 6 22 0 0 3 0 9 1 0 36 1 0 1 0 0 1 2 78 8
08:45 AM 6 19 0 0 4 0 9 0 1 39 1 1 0 0 1 1 2 80 8
Total 25 85 1 0 13 0 38 2 2 139 9 1 1 0 1 4 7 314 32
*** BREAK ***
04:00 PM 2 24 0 3 0 1 8 1 0 17 5 0 1 0 0 1 5 58 6
04:15 PM 7 19 3 1 7 0 6 4 0 28 2 1 1 0 0 0 6 73 7
04:30 PM 5 22 0 2 1 0 8 2 0 26 3 1 0 0 0 2 7 65 7
04:45 PM 11 36 0 2 3 0 10 0 0 21 6 0 1 1 0 0 2 89 9
Total 25 101 3 8 11 1 32 7 0 92 16 2 3 1 0 3 20 285 30
05:00 PM 10 33 0 2 2 0 7 0 0 33 6 0 1 0 1 0 2 93 9
05:15 PM 8 24 0 2 5 1 7 2 0 40 3 1 0 0 2 1 6 90 9
05:30 PM 9 42 3 0 3 0 9 0 2 32 5 0 0 0 0 1 1 105 10
05:45 PM 7 21 0 0 2 0 3 0 2 27 5 0 0 0 1 0 0 68 6
Total 34 120 3 4 12 1 26 2 4 132 19 1 1 0 4 2 9 356 36
Grand Total 100 378 7 14 43 2 137 15 8 483 50 5 9 2 6 11 45 1225 127
Apprch % 20.6 77.9 1.4 23.6 1.1 75.3 1.5 89.3 9.2 52.9 11.8 35.3
Total % 8.2 30.9 0.6 3.5 0.2 11.2 0.7 39.4 4.1 0.7 0.2 0.5 3.5 96.5
Passenger Vehicles 98 365 6 41 2 136 8 467 49 8 0 6 0 0 118
% Passenger Vehicles 98 96.6 85.7 0 95.3 100 99.3 0 100 96.7 98 0 88.9 0 100 0 0 93.
Single Unit 2 6 1 2 0 1 0 10 1 1 1 0 0 0 0 2 % Single Unit 2 1.6 14.3 0 4.7 0 0.7 0 0 2.1 2 0 11.1 50 0 0 0 0
% Single Unit 2 1.6 14.3 0 4.7 0 0.7 0 0 2.1 2 0 11.1 50 0 0 0 0 TTST 0 7 0 0 0 0 6 0 0 1 0 0 0 1
%TTST 0 1.9 0 0 0 0 0 0 0 1.2 0 0 0 50 0 0 0 1.
No 1131 0 1.9 0 0 0 0 0 0 1.2 0 0 0 0 0 0 1.2 0 0 0 0 0 0 0 0 0
% Bicycles on Crosswalk 0 0 0 7.1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Pedestrians 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
% Pedestrians 0 0 0 92.9 0 0 0 100 0 0 80 0 0 0 100 0 0 3.

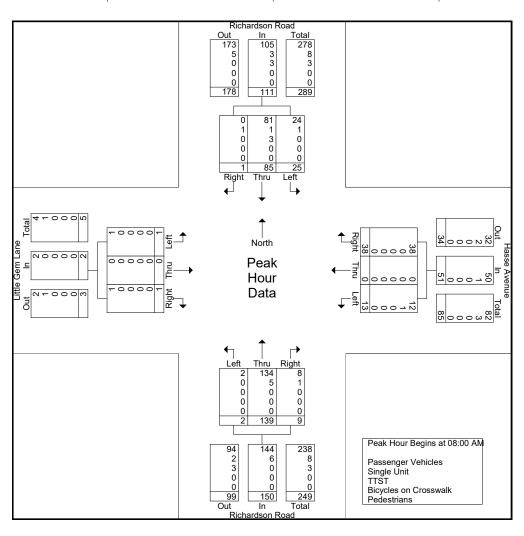
Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: Richardson@Hasse

Site Code :

Start Date : 11/5/2020

	R	ichards	son Roa	ad		Hasse	Avenue	.	R	Richard	son Ro	ad		l ittle G	em Lan	e	
			bound	-			bound		•		bound				bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis From	07:00 A	M to 12	:30 PM - P	eak 1 of	1											
Peak Hour for Entire	e Intersecti	on Begins	s at 08:00	AM													
08:00 AM	9	17	0	26	2	0	11	13	1	35	3	39	0	0	0	0	78
08:15 AM	4	27	1	32	4	0	9	13	0	29	4	33	0	0	0	0	78
08:30 AM	6	22	0	28	3	0	9	12	0	36	1	37	1	0	0	1	78
08:45 AM	6	19	0	25	4	0	9	13	1	39	1	41	0	0	1	1	80
Total Volume	25	85	1	111	13	0	38	51	2	139	9	150	1	0	1	2	314
% App. Total	22.5	76.6	0.9		25.5	0	74.5		1.3	92.7	6		50	0	50		
PHF	.694	.787	.250	.867	.813	.000	.864	.981	.500	.891	.563	.915	.250	.000	.250	.500	.981
Passenger Vehicles	24	81	0	105	12	0	38	50	2	134	8	144	1	0	1	2	301
% Passenger Vehicles	96.0	95.3	0	94.6	92.3	0	100	98.0	100	96.4	88.9	96.0	100	0	100	100	95.9
Single Unit	1	1	1	3	1	0	0	1	0	5	1	6	0	0	0	0	10
% Single Unit	4.0	1.2	100	2.7	7.7	0	0	2.0	0	3.6	11.1	4.0	0	0	0	0	3.2
TTST	0	3	0	3	0	0	0	0	0	0	0	0	0	0	0	0	3
% TTST	0	3.5	0	2.7	0	0	0	0	0	0	0	0	0	0	0	0	1.0
Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



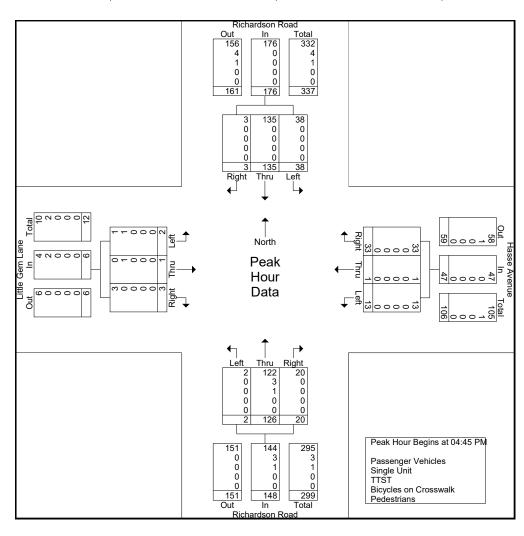
Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: Richardson@Hasse

Site Code :

Start Date : 11/5/2020

	R	Richard	son Roa	ad		Hasse	Avenue	•	R	Richard	son Ro	ad					
		South	bound			West	bound			North	bound			Eastl	bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy					eak 1 of	1											
Peak Hour for Entire	e Intersecti	on Begin	s at 04:45	PM													
04:45 PM	11	36	0	47	3	0	10	13	0	21	6	27	1	1	0	2	89
05:00 PM	10	33	0	43	2	0	7	9	0	33	6	39	1	0	1	2	93
05:15 PM	8	24	0	32	5	1	7	13	0	40	3	43	0	0	2	2	90
05:30 PM	9	42	3	54	3	0	9	12	2	32	5	39	0	0	0	0	105
Total Volume	38	135	3	176	13	1	33	47	2	126	20	148	2	1	3	6	377
% App. Total	21.6	76.7	1.7		27.7	2.1	70.2		1.4	85.1	13.5		33.3	16.7	50		
PHF	.864	.804	.250	.815	.650	.250	.825	.904	.250	.788	.833	.860	.500	.250	.375	.750	.898
Passenger Vehicles	38	135	3	176	13	1	33	47	2	122	20	144	1	0	3	4	371
% Passenger Vehicles	100	100	100	100	100	100	100	100	100	96.8	100	97.3	50.0	0	100	66.7	98.4
Single Unit	0	0	0	0	0	0	0	0	0	3	0	3	1	1	0	2	5
% Single Unit	0	0	0	0	0	0	0	0	0	2.4	0	2.0	50.0	100	0	33.3	1.3
TTST	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	1
% TTST	0	0	0	0	0	0	0	0	0	0.8	0	0.7	0	0	0	0	0.3
Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



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File Name: US64@Richardson

Site Code :

Start Date : 11/5/2020

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Groups Printed- Passenger Vehicles - Single Unit - TTST - Bicycles on Crosswalk - Pedestrians

				S Print	ea-Pas			es - Sii				ycies o	n Cross			trians	İ		
		Jenks				US			RIC	hardso		iue		US					
		South				West				North				Eastb					
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru	Right		Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	19	0	22	174	18	0	0	0	31	0	17	228	1	0	0	510	510
07:15 AM	0	0	29	0	38	222	10	0	0	0	55	0	31	239	4	0	0	628	628
07:30 AM	0	0	13	0	22	244	16	0	0	0	67	0	19	275	5	0	0	661	661
07:45 AM	0	0	21	0	41	257	17	0	0	0	55	0	15	257	6	0	0	669	669
Total	0	0	82	0	123	897	61	0	0	0	208	0	82	999	16	0	0	2468	2468
08:00 AM	0	0	25	0	40	210	15	0	0	0	67	0	12	249	6	0	0	624	624
08:15 AM	0	0	25	0	39	230	25	0	0	0	43	0	17	226	6	0	0	611	611
08:30 AM	0	0	22	0	31	242	21	0	0	0	55	0	14	209	4	0	0	598	598
08:45 AM	0	0	18	0	24	185	17	0	0	0	59	0	17	192	5	0	0	517	517
Total	0	0	90	0	134	867	78	0	0	0	224	0	60	876	21	0	0	2350	2350
*** BREAK ***																			
04:00 PM	0	0	29	0	29	280	11	0	0	0	39	0	14	223	6	0	0	631	631
04:15 PM	0	0	30	0	41	295	11	0	0	0	43	0	21	226	6	0	0	673	673
04:30 PM	0	0	26	0	35	271	11	0	0	0	53	0	14	268	4	0	0	682	682
04:45 PM	0	0	25	0	59	255	21	0	0	0	45	0	16	238	6	0	0	665	665
Total	0	0	110	0	164	1101	54	0	0	0	180	0	65	955	22	0	0	2651	2651
05:00 PM	0	0	41	0	52	313	17	0	0	0	51	0	17	291	6	0	0	788	788
05:15 PM	0	0	42	0	42	330	24	0	0	0	50	0	16	297	7	0	0	808	808
05:30 PM	0	0	40	0	60	281	16	0	0	0	60	0	24	262	12	0	0	755	755
05:45 PM	0	0	37	0	42	265	13	0	0	0	42	0	24 17	282	5	0	0	703	703
Total	0	0	160	0	196	1189	70	0	0	0	203	0	74	1132	30	0	0	3054	3054
TOLAT	U	U	100	U	190	1109	70	0	U	U	203	0	74	1132	30	U	0	3054	3034
Grand Total	0	0	442	0	617	4054	263	0	0	0	815	0	281	3962	89	0	0	10523	10523
Apprch %	0	0	100		12.5	82.2	5.3		0	0	100		6.5	91.5	2.1				
Total %	0	0	4.2		5.9	38.5	2.5		0	0	7.7		2.7	37.7	0.8		0	100	
Passenger Vehicles	0	0	417		599	3775	234		0	0	788		265	3716	82		0	0	9876
% Passenger Vehicles	0	0	94.3	0	97.1	93.1	89	0	0	0	96.7	0	94.3	93.8	92.1	0	0	0	93.9
Single Unit	0	0	22		13	163	23		0	0	23		11	105	7		0	0	367
% Single Unit	0	0	5	0	2.1	4	8.7	0	0	0	2.8	0	3.9	2.7	7.9	0	0	0	3.5
TTST	0	0	3		5	116	6		0	0	4		5	141	0		0	0	280
% TTST	0	0	0.7	0	0.8	2.9	2.3	0	0	0	0.5	0	1.8	3.6	0	0	0	0	2.7
Bicycles on Crosswalk	0	0	0		0	0	0		0	0	0		0	0	0		0	0	0
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	-	0	0	0		0	0	0		0	0	0		0	0	0
% Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

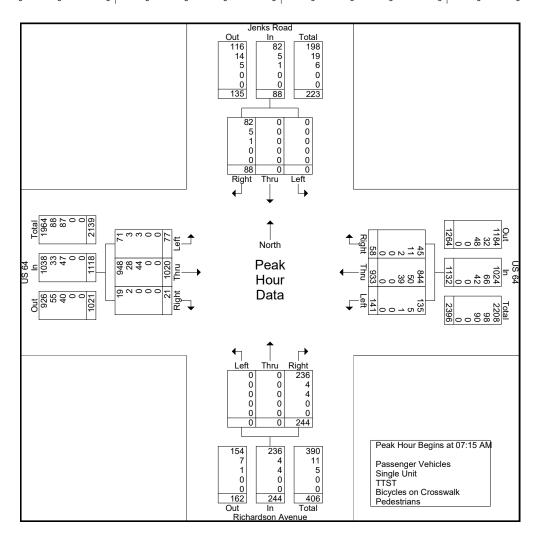
Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: US64@Richardson

Site Code :

Start Date : 11/5/2020

		Jenks	Road			US	64		Ri	chards	on Aver						
		South	bound			West	bound			North	bound			East	bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis From	07:00 A	M to 11	:45 AM - P	eak 1 of	1											_
Peak Hour for Entire	e Intersecti	on Begins	s at 07:15	AM													
07:15 AM	0	0	29	29	38	222	10	270	0	0	55	55	31	239	4	274	628
07:30 AM	0	0	13	13	22	244	16	282	0	0	67	67	19	275	5	299	661
07:45 AM	0	0	21	21	41	257	17	315	0	0	55	55	15	257	6	278	669
08:00 AM	0	0	25	25	40	210	15	265	0	0	67	67	12	249	6	267	624
Total Volume	0	0	88	88	141	933	58	1132	0	0	244	244	77	1020	21	1118	2582
% App. Total	0	0	100		12.5	82.4	5.1		0	0	100		6.9	91.2	1.9		
PHF	.000	.000	.759	.759	.860	.908	.853	.898	.000	.000	.910	.910	.621	.927	.875	.935	.965
Passenger Vehicles	0	0	82	82	135	844	45	1024	0	0	236	236	71	948	19	1038	2380
% Passenger Vehicles	0	0	93.2	93.2	95.7	90.5	77.6	90.5	0	0	96.7	96.7	92.2	92.9	90.5	92.8	92.2
Single Unit	0	0	5	5	5	50	11	66	0	0	4	4	3	28	2	33	108
% Single Unit	0	0	5.7	5.7	3.5	5.4	19.0	5.8	0	0	1.6	1.6	3.9	2.7	9.5	3.0	4.2
TTST	0	0	1	1	1	39	2	42	0	0	4	4	3	44	0	47	94
% TTST	0	0	1.1	1.1	0.7	4.2	3.4	3.7	0	0	1.6	1.6	3.9	4.3	0	4.2	3.6
Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



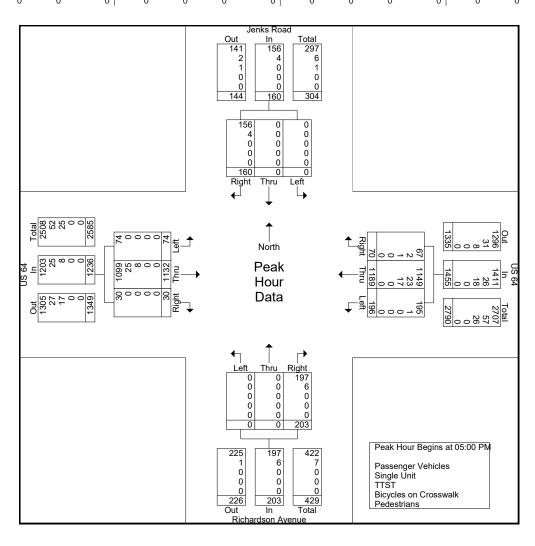
Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: US64@Richardson

Site Code :

Start Date : 11/5/2020

		Jenks	Road			US	6 64		Ri	chards	on Ave	nue		US	S 64		
		South	bound			West	bound			North	bound			East	bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analy	sis From	12:00 F	PM to 05	:45 PM - P	eak 1 of	1											
Peak Hour for Entire	e Intersection	on Begins	s at 05:00	PM													
05:00 PM	0	0	41	41	52	313	17	382	0	0	51	51	17	291	6	314	788
05:15 PM	0	0	42	42	42	330	24	396	0	0	50	50	16	297	7	320	808
05:30 PM	0	0	40	40	60	281	16	357	0	0	60	60	24	262	12	298	755
05:45 PM	0	0	37	37	42	265	13	320	0	0	42	42	17	282	5	304	703
Total Volume	0	0	160	160	196	1189	70	1455	0	0	203	203	74	1132	30	1236	3054
% App. Total	0	0	100		13.5	81.7	4.8		0	0	100		6	91.6	2.4		
PHF	.000	.000	.952	.952	.817	.901	.729	.919	.000	.000	.846	.846	.771	.953	.625	.966	.945
Passenger Vehicles	0	0	156	156	195	1149	67	1411	0	0	197	197	74	1099	30	1203	2967
% Passenger Vehicles	0	0	97.5	97.5	99.5	96.6	95.7	97.0	0	0	97.0	97.0	100	97.1	100	97.3	97.2
Single Unit	0	0	4	4	1	23	2	26	0	0	6	6	0	25	0	25	61
% Single Unit	0	0	2.5	2.5	0.5	1.9	2.9	1.8	0	0	3.0	3.0	0	2.2	0	2.0	2.0
TTST	0	0	0	0	0	17	1	18	0	0	0	0	0	8	0	8	26
% TTST	0	0	0	0	0	1.4	1.4	1.2	0	0	0	0	0	0.7	0	0.6	0.9
Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: US64@U-turn E Richardson

Site Code :

Start Date : 11/5/2020

Page No : 1

Groups Printed- Passenger Vehicles - Single Unit - TTST - Bicycles on Crosswalk - Pedestrians

		Νο Δηι	oroach		cu-i us	US		C3 - OII		No Ap		y cics o	11 01058	US		uiuiis			
		South				Westb				North				Eastb					
Start Time	Left	Thru	Right	Peds	Left	Thru	Right	Peds	Left	Thru		Peds	U-Turn	Thru	Right	Peds	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	1	0	0	218	0	0	0	0	0	0	2	247	0	0	0	468	468
07:15 AM	0	0	0	0	0	268	0	0	0	0	0	0	5	302	0	0	0	575	575
07:30 AM	0	0	3	0	0	278	0	0	0	0	0	0	8	315	0	0	0	604	604
07:45 AM	0	0	0	0	0	309	0	0	0	0	0	0	4	272	0	0	0	585	585
Total	0	0	4	0	0	1073	0	0	0	0	0	0	19	1136	0	0	0	2232	2232
1													_			_ 1			
08:00 AM	0	0	0	0	0	273	0	0	0	0	0	0	7	331	0	0	0	611	611
08:15 AM	0	0	0	0	0	283	0	0	0	0	0	0	8	264	0	0	0	555	555
08:30 AM	0	0	0	0	0	305	0	0	0	0	0	0	6	261	0	0	0	572	572
08:45 AM	0	0	0	0	0	203	0	0	0	0	0	0	7	239	0	0	0	449	449
Total	0	0	0	0	0	1064	0	0	0	0	0	0	28	1095	0	0	0	2187	2187
*** BREAK ***																			
04:00 PM	0	0	0	0	0	324	0	0	0	0	0	0	6	263	0	0	0	593	593
04:15 PM	0	0	0	0	0	349	0	0	0	0	0	0	5	266	0	0	0	620	620
04:30 PM	0	0	0	0	0	303	0	0	0	0	0	0	9	311	0	0	0	623	623
04:45 PM	0	0	0	0	0	330	0	0	0	0	0	0	7	277	0	0	0	614	614
Total	0	0	0	0	0	1306	0	0	0	0	0	0	27	1117	0	0	0	2450	2450
								·											
05:00 PM	0	0	3	0	0	376	0	0	0	0	0	0	10	341	0	0	0	730	730
05:15 PM	0	0	1	0	0	394	0	0	0	0	0	0	10	334	0	0	0	739	739
05:30 PM	0	0	2	0	0	363	0	0	0	0	0	0	12	294	0	0	0	671	671
05:45 PM	0	0	2	0	0	301	0	0	0	0	0	0	9	294	0	0	0	606	606
Total	0	0	8	0	0	1434	0	0	0	0	0	0	41	1263	0	0	0	2746	2746
Grand Total	0	0	12	0	0	4877	0	0	0	0	0	0	115	4611	0	0	0	9615	9615
Apprch %	0	0	100	U	0	100	0	0	0	0	0	0	2.4	97.6	0	0	U	7013	7013
Total %	0	0	0.1		0	50.7	0	0	0	0	0	0	1.2	48	0	0	0	100	
Passenger Vehicles	0	0	8		0	4597	0	0	0	0	0	0	103	4370	0	0	0	0	9078
% Passenger Vehicles	0	0	66.7	0	0	94.3	0	0	0	0	0	0	89.6	94.8	0	0	0	0	94.4
Single Unit	0	0	3		0	157	0	0	0	0	0	0	11	125	0	0	0	0	296
% Single Unit	0	0	25	0	0	3.2	0	0	0	0	0	0	9.6	2.7	0	0	0	0	3.1
TTST	0	0	1		0	123	0	0	0	0	0	0	1	116	0	0	0	0	241
% TTST	0	0	8.3	0	0	2.5	0	0	0	0	0	0	0.9	2.5	0	0	0	0	2.5
Bicycles on Crosswalk	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	-	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

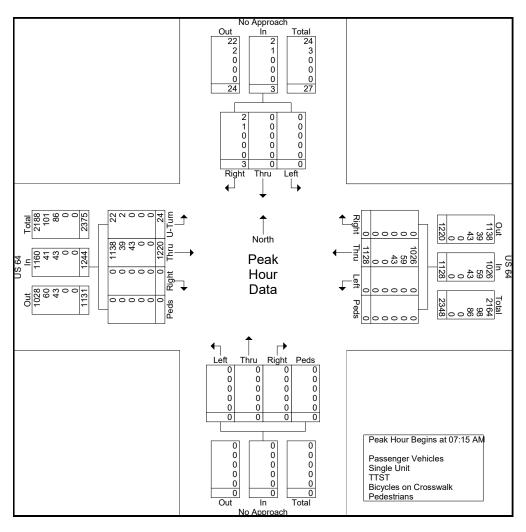
Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

File Name: US64@U-turn E Richardson

Site Code :

Start Date : 11/5/2020

		No An	proach	1			US 64				No	Appro	ach				US 64			1
			bound			W	estbou					orthbou				Е	astbou			
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Ana	alysis Fi	rom 07:	00 AM	to 11:45	AM - P	eak 1 o	f 1													
Peak Hour for En	tire Inters	section E	Begins at	07:15 AM																
07:15 AM	0	0	0	0	0	268	0	0	268	0	0	0	0	0	5	302	0	0	307	575
07:30 AM	0	0	3	3	0	278	0	0	278	0	0	0	0	0	8	315	0	0	323	604
07:45 AM	0	0	0	0	0	309	0	0	309	0	0	0	0	0	4	272	0	0	276	585
08:00 AM	0	0	0	0	0	273	0	0	273	0	0	0	0	0	7	331	0	0	338	611
Total Volume	0	0	3	3	0	1128	0	0	1128	0	0	0	0	0	24	1220	0	0	1244	2375
% App. Total	0	0	100		0	100	0	0		0	0	0	0		1.9	98.1	0	0		1
PHF	.000	.000	.250	.250	.000	.913	.000	.000	.913	.000	.000	.000	.000	.000	.750	.921	.000	.000	.920	.972
Passenger Vehicles	0	0	2	2	0	1026	0	0	1026	0	0	0	0	0	22	1138	0	0	1160	2188
% Passenger Vehicles																				
Single Unit	0	0	1	1	0	59	0	0	59	0	0	0	0	0	2	39	0	0	41	101
% Single Unit	0	0	33.3	33.3	0	5.2	0	0	5.2	0	0	0	0	0	8.3	3.2	0	0	3.3	4.3
TTST	0	0	0	0	0	43	0	0	43	0	0	0	0	0	0	43	0	0	43	86
% TTST	0	0	0	0	0	3.8	0	0	3.8	0	0	0	0	0	0	3.5	0	0	3.5	3.6
Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



VHB Engineering NC, P.C.

Venture I 940 Main Campus Drive, Suite 500 Raleigh, NC 27606 p: 919.829.0328 f: 919.833.0034

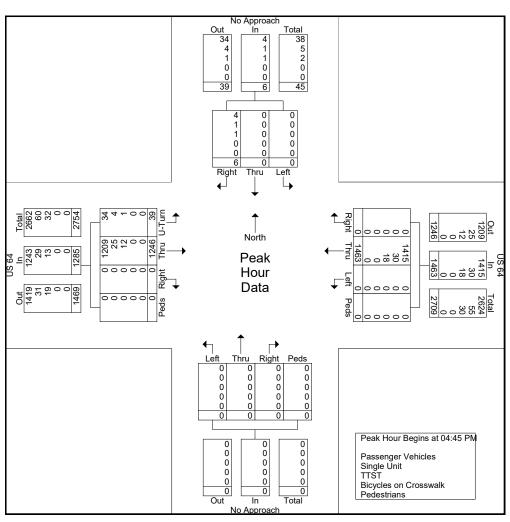
File Name: US64@U-turn E Richardson

Site Code :

Start Date : 11/5/2020

Page No : 3

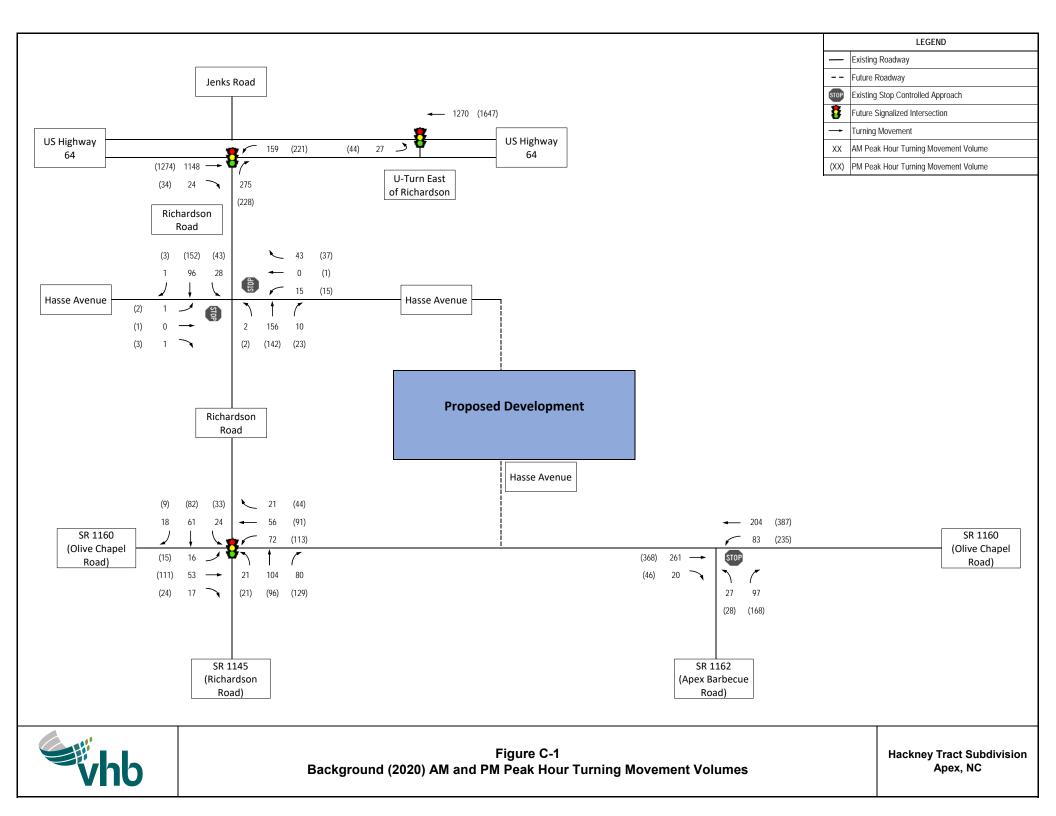
		Νο Δη	proach	,			US 64	l .			No	Appro	ach				US 64			
			bound			W	estbo					orthboi				Е	astbou			
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	Peds	App. Total	Left	Thru	Right	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	Int. Total
Peak Hour Ana	alysis Fi	rom 12:	00 PM	to 05:45	PM - P	eak 1 o	f 1			•										
Peak Hour for En	tire Inters	section E	Begins at	04:45 PM																
04:45 PM	0	0	0	0	0	330	0	0	330	0	0	0	0	0	7	277	0	0	284	614
05:00 PM	0	0	3	3	0	376	0	0	376	0	0	0	0	0	10	341	0	0	351	730
05:15 PM	0	0	1	1	0	394	0	0	394	0	0	0	0	0	10	334	0	0	344	739
05:30 PM	0	0	2	2	0	363	0	0	363	0	0	0	0	0	12	294	0	0	306	671
Total Volume	0	0	6	6	0	1463	0	0	1463	0	0	0	0	0	39	1246	0	0	1285	2754
% App. Total	0	0	100		0	100	0	0		0	0	0	0		3	97	0	0		
PHF	.000	.000	.500	.500	.000	.928	.000	.000	.928	.000	.000	.000	.000	.000	.813	.913	.000	.000	.915	.932
Passenger Vehicles	0	0	4	4	0	1415	0	0	1415	0	0	0	0	0	34	1209	0	0	1243	2662
% Passenger Vehicles																				
Single Unit	0	0	1	1	0	30	0	0	30	0	0	0	0	0	4	25	0	0	29	60
% Single Unit	0	0	16.7	16.7	0	2.1	0	0	2.1	0	0	0	0	0	10.3	2.0	0	0	2.3	2.2
TTST	0	0	1	1	0	18	0	0	18	0	0	0	0	0	1	12	0	0	13	32
% TTST	0	0	16.7	16.7	0	1.2	0	0	1.2	0	0	0	0	0	2.6	1.0	0	0	1.0	1.2
Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bicycles on Crosswalk	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

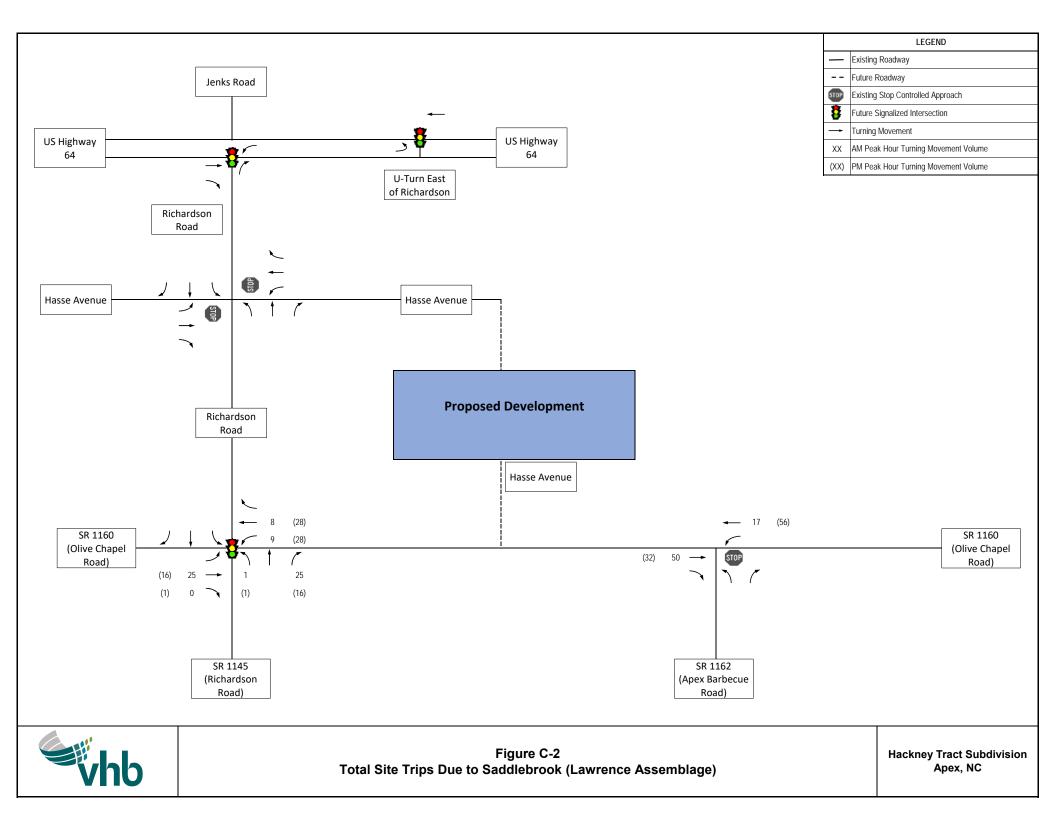


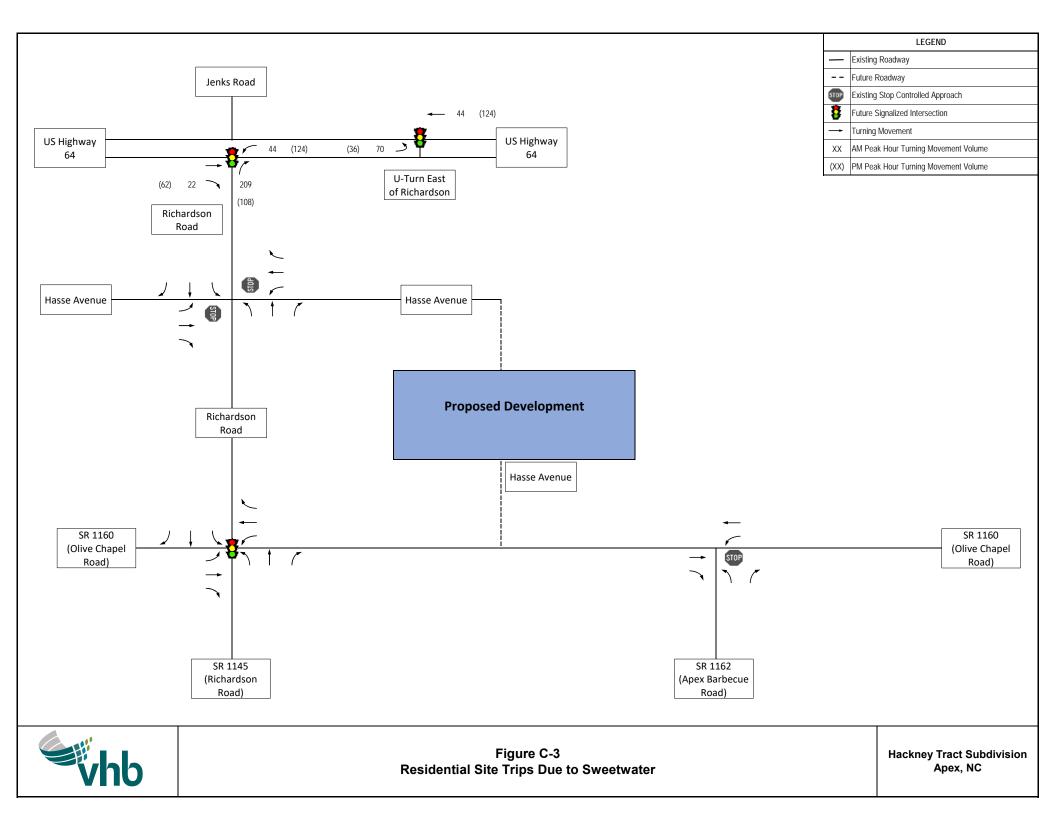


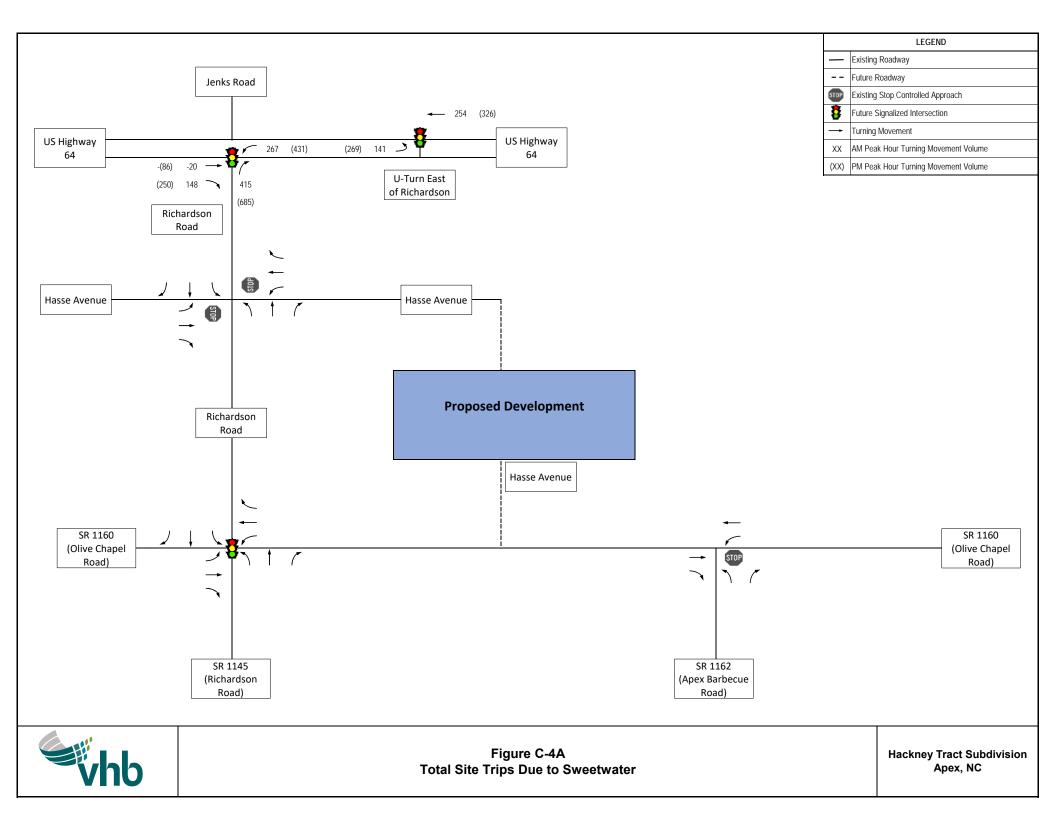
APPENDIX C:

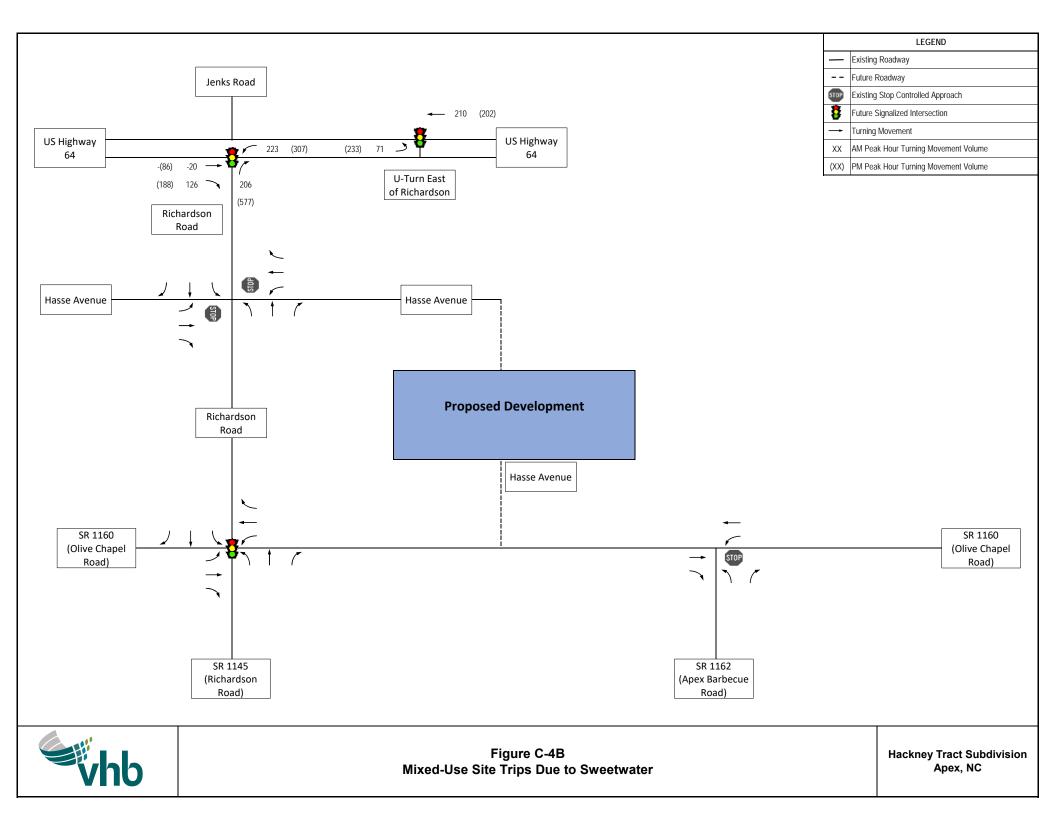
Background Projects

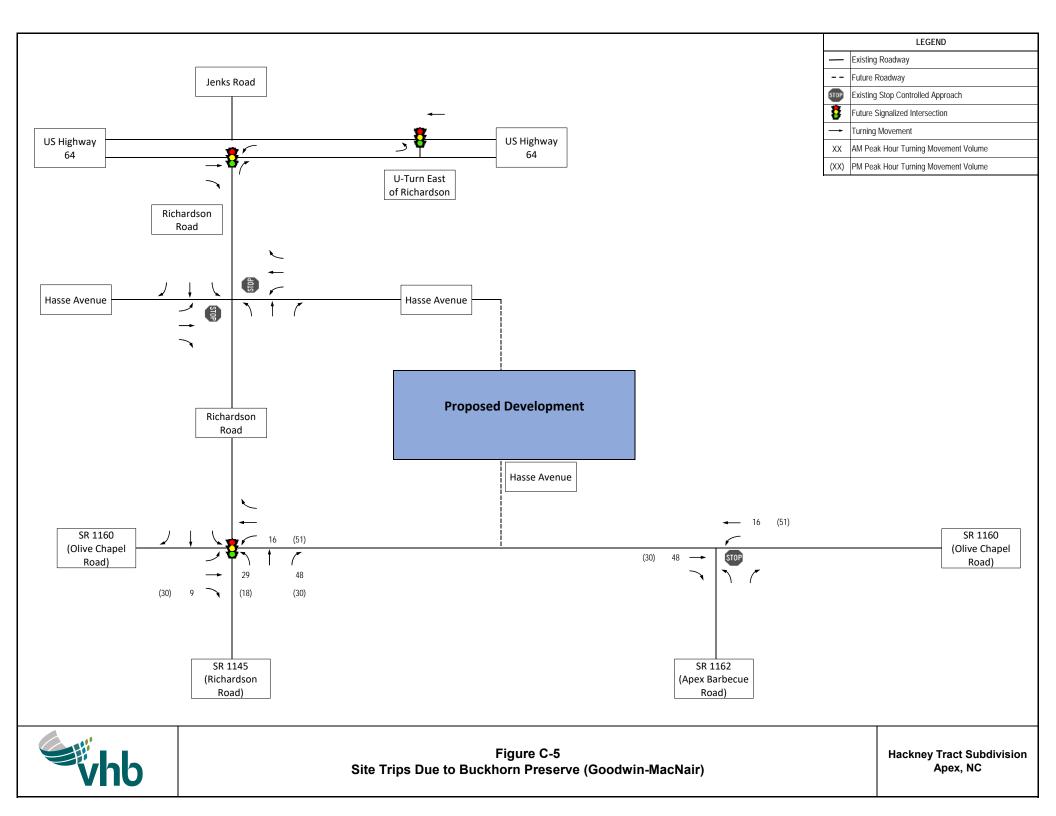


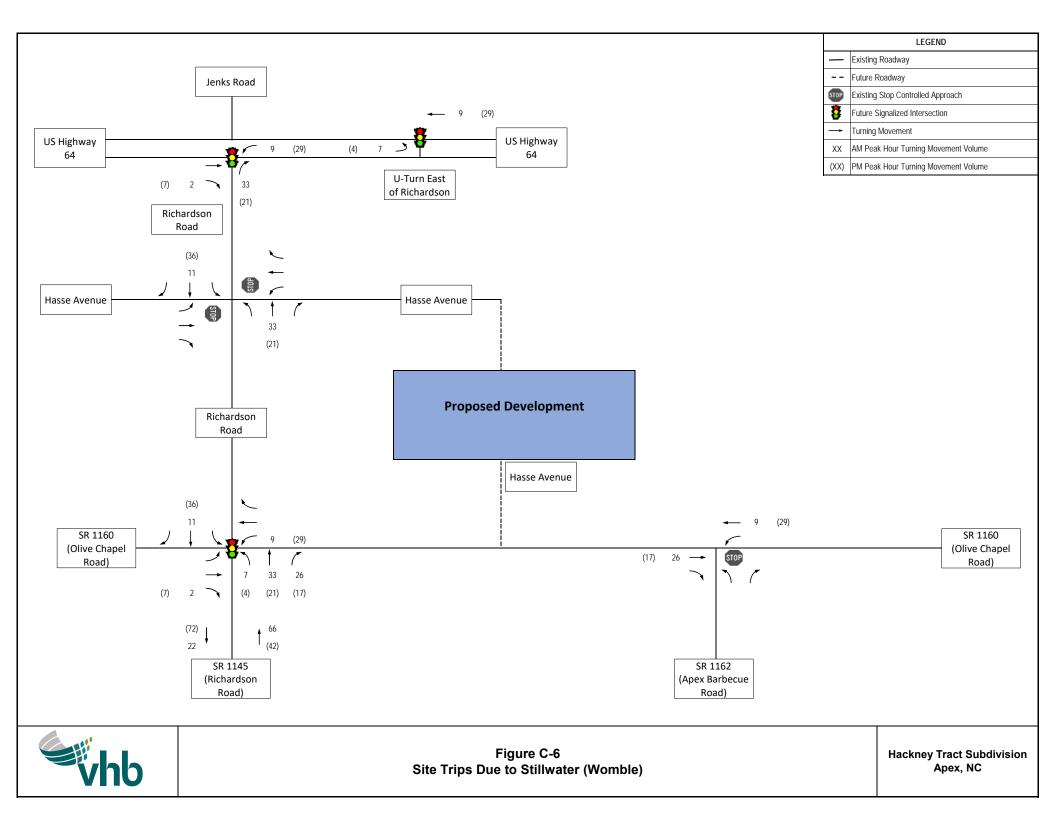


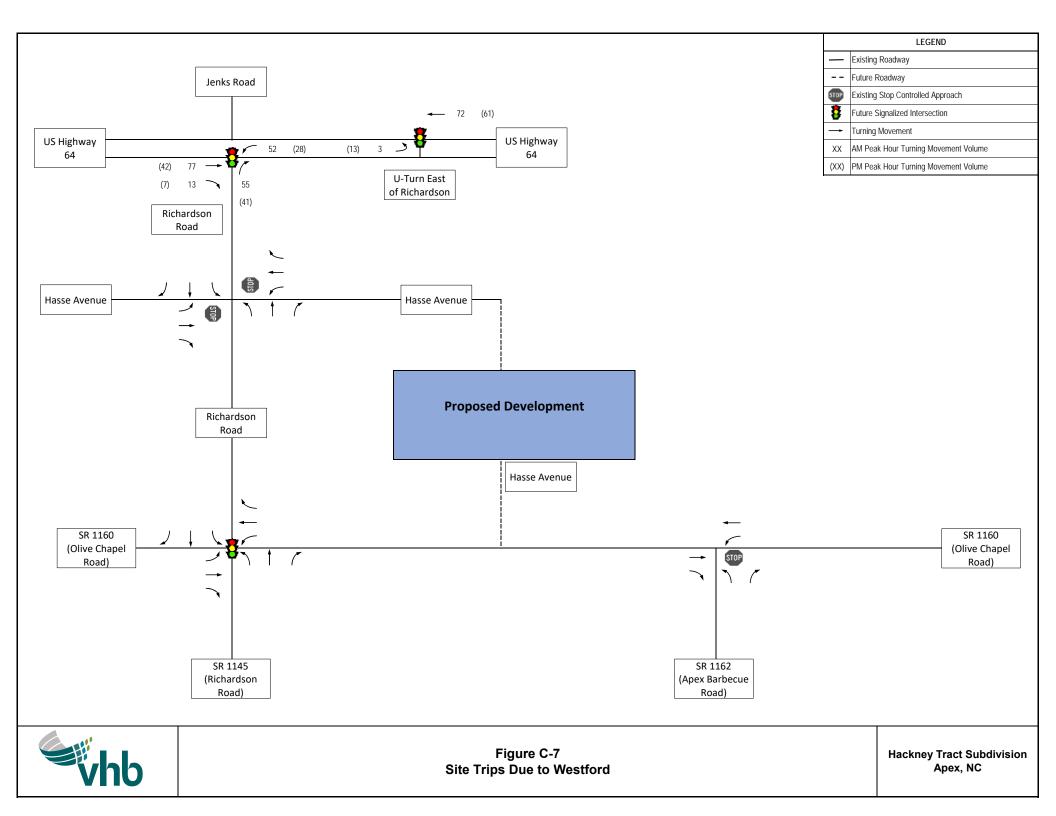


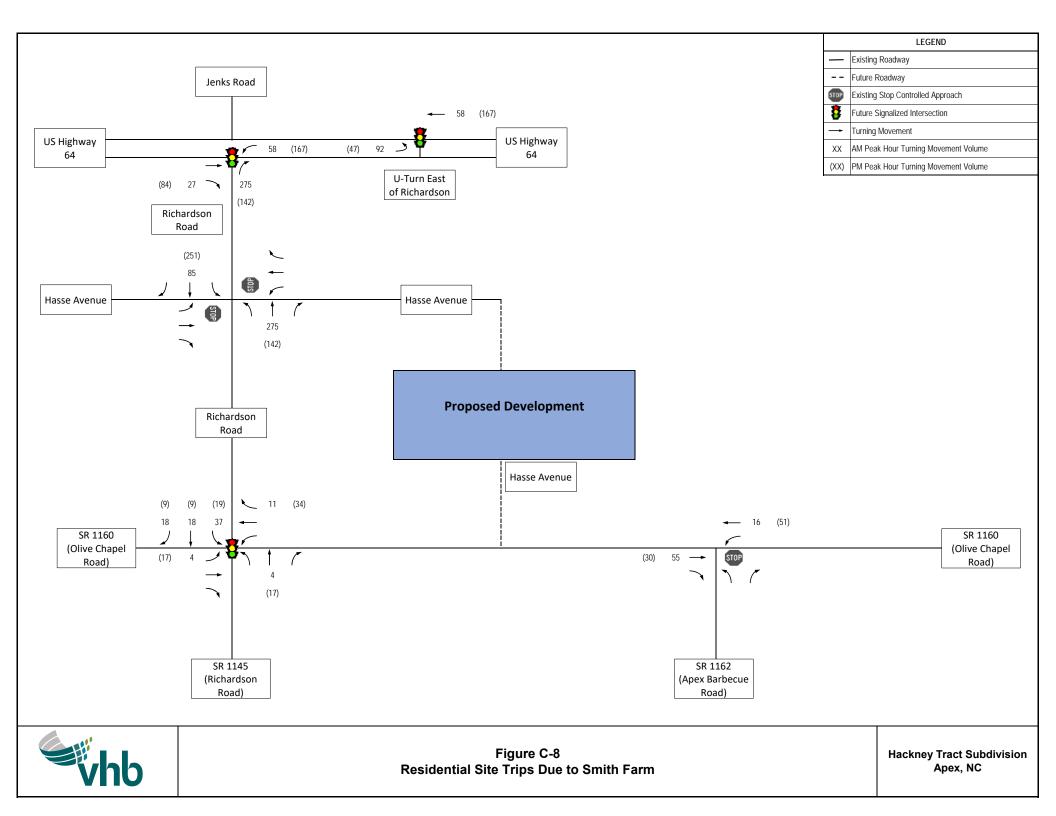


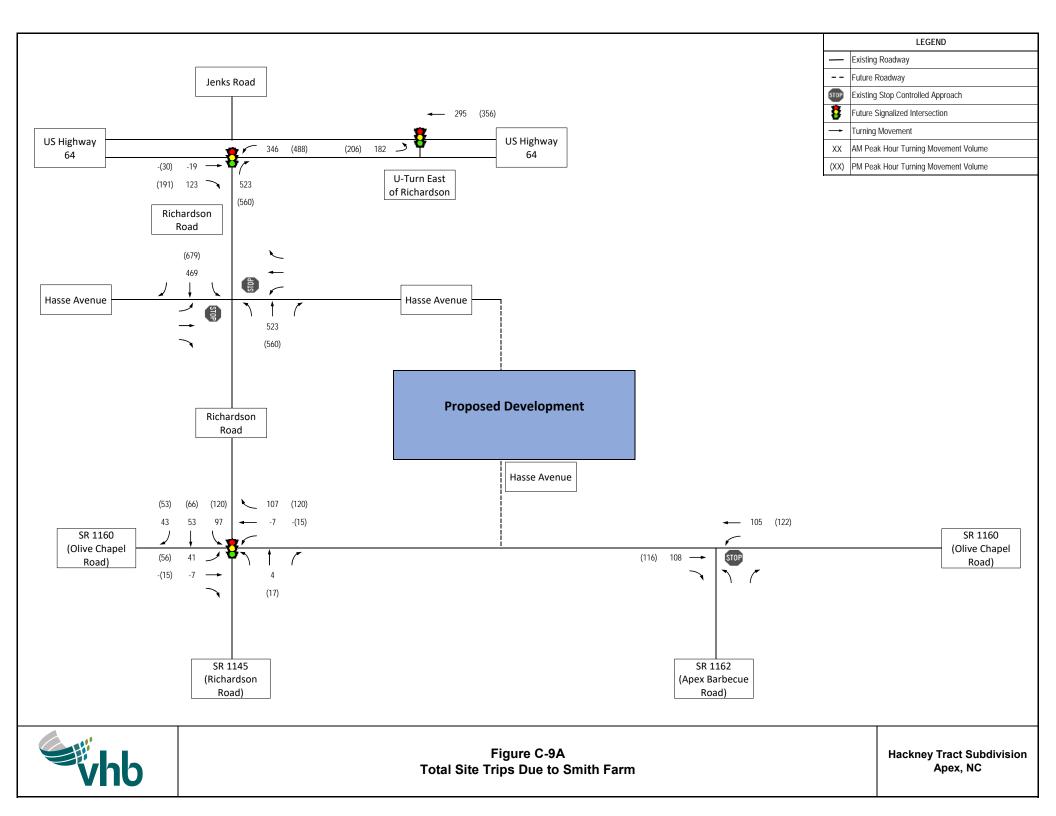


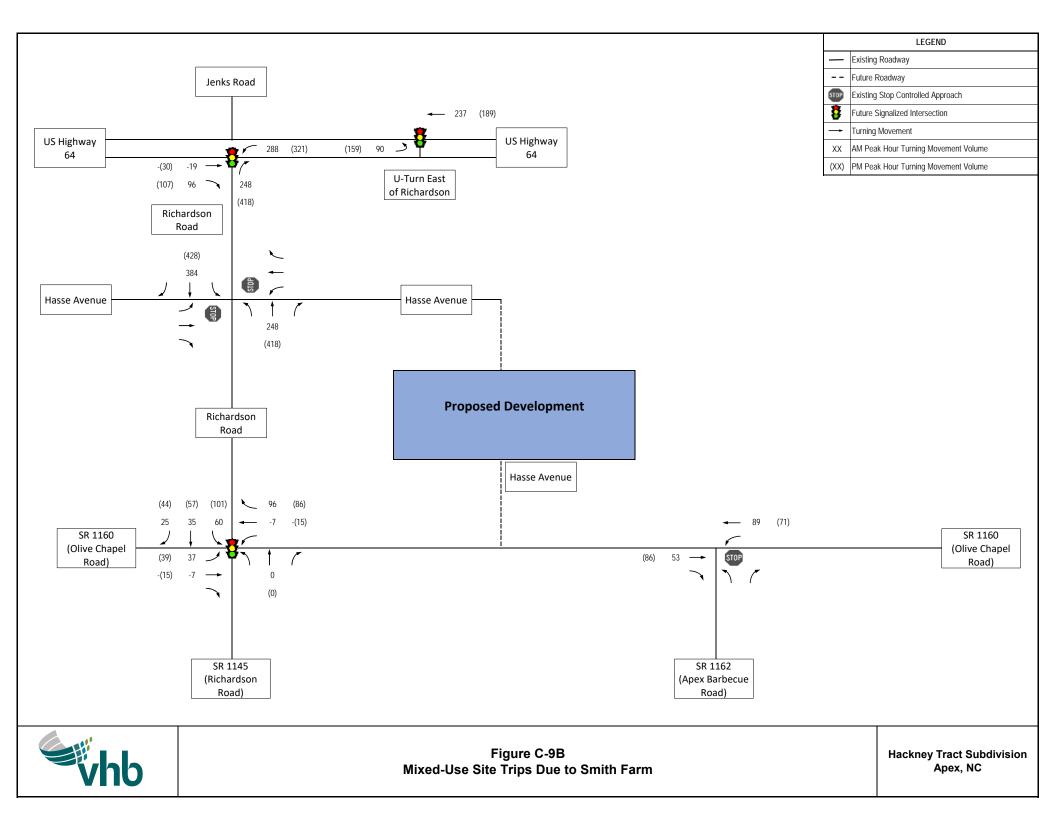


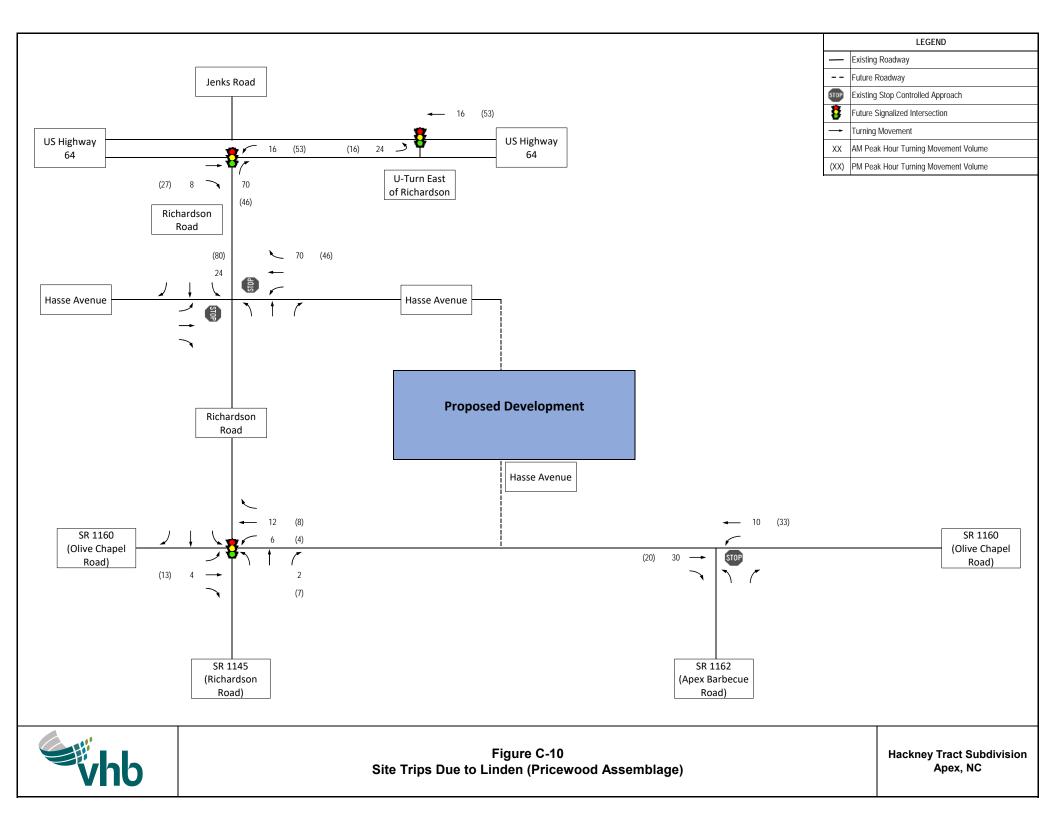


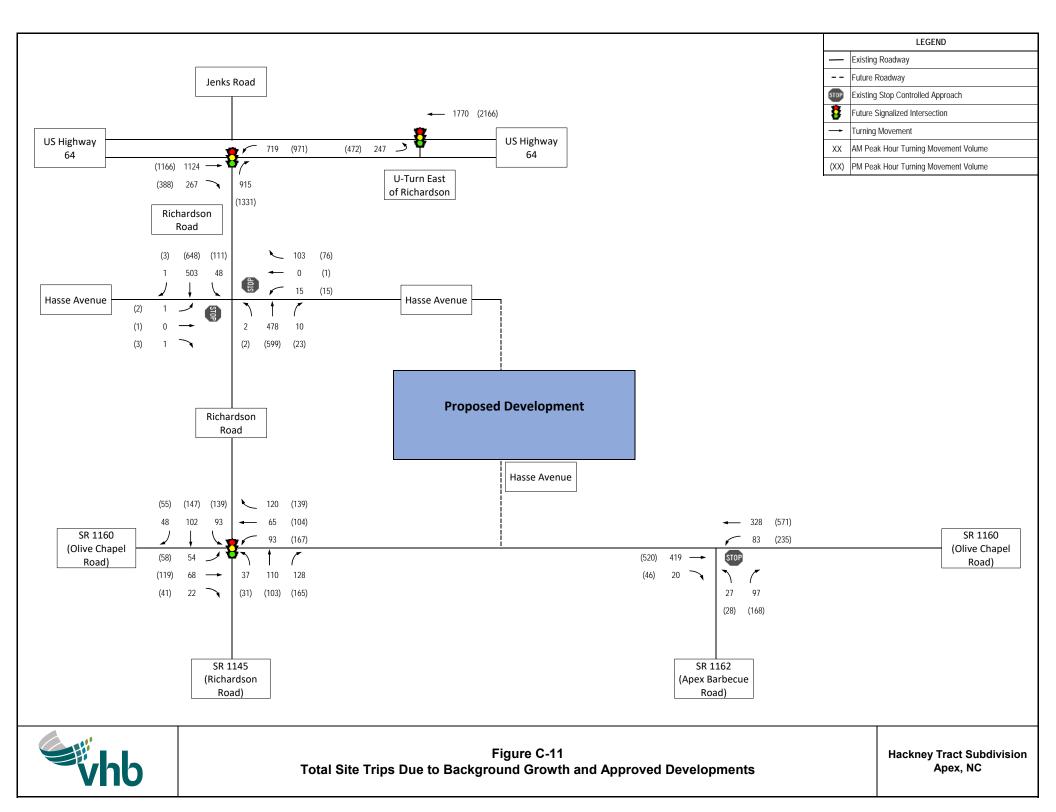


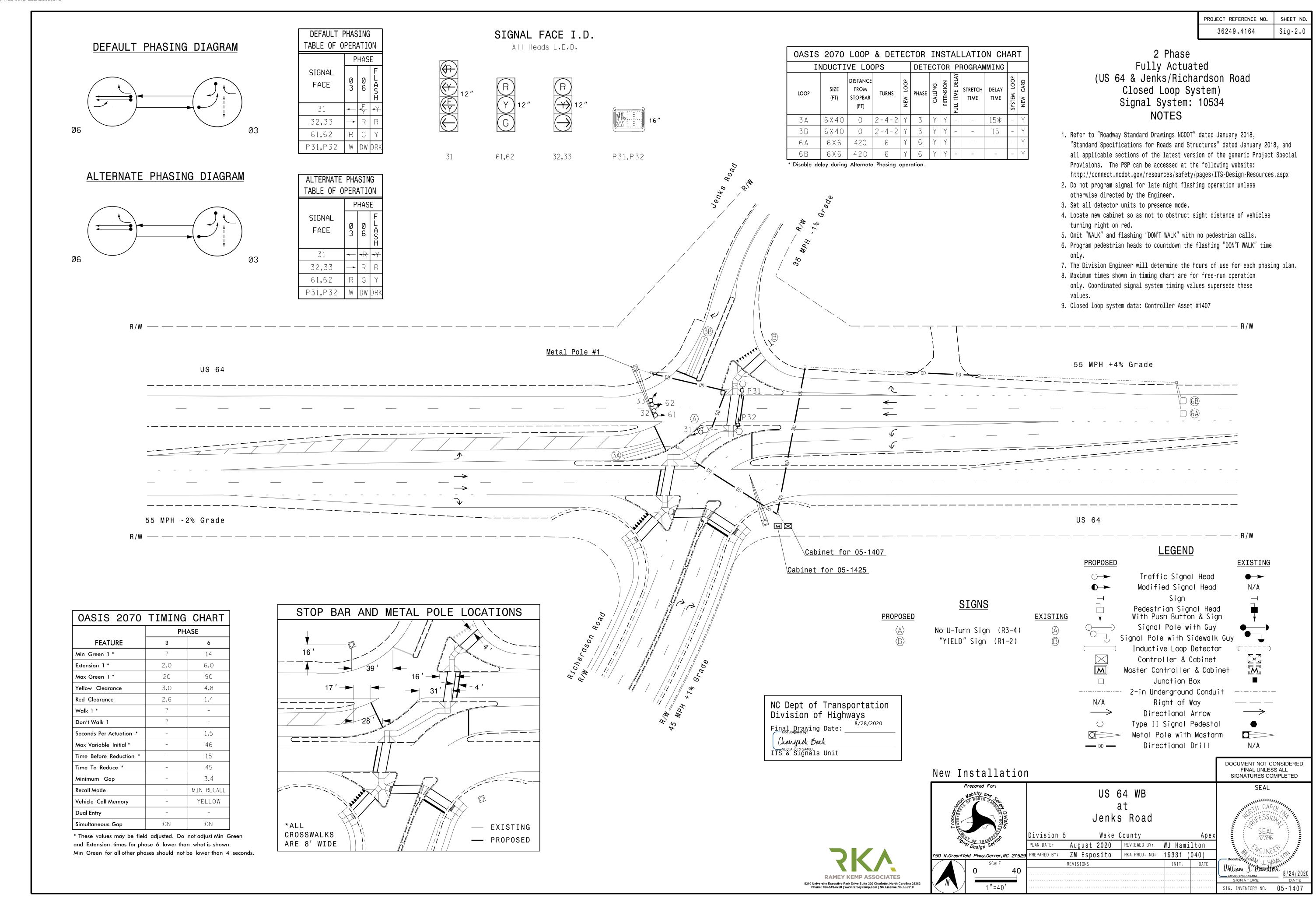


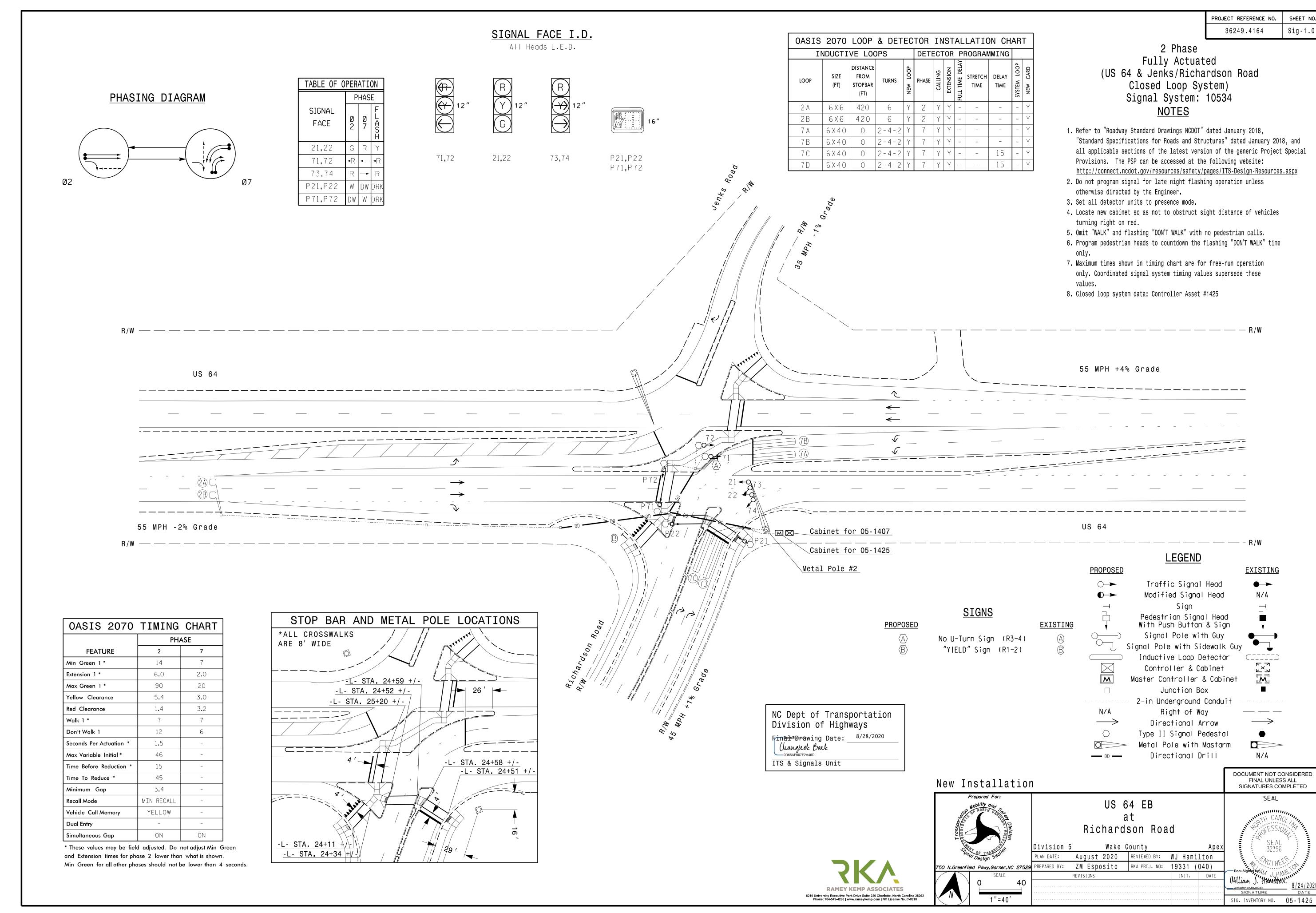








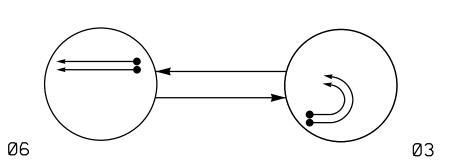


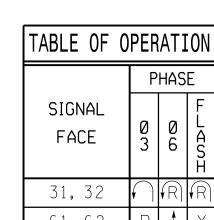


PROJECT REFERENCE NO.

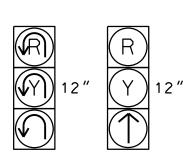
36249.4164

PHASING DIAGRAM









31, 32 61,62

OASIS	2070	LOOP	& DET	EC	TOR	IN	ST	AL	LATIC	ON CH	AR	т
1I	NDUCTI	VE LOC	PS		DET	ECT	OR	PI	ROGRAN	MMING		
LOOP	SIZE (FT)	DISTANCE FROM STOPBAR (FT)	TURNS	NEW LOOP	PHASE	CALLING	EXTENSION	FULL TIME DELAY	STRETCH TIME	DELAY TIME	SYSTEM LOOP	NEW CARD
3 A	6 X 4 O	0	2-4-2	Υ	3	Υ	Υ	-	-	-	-	Υ
3 B	6 X 4 O	0	2-4-2	Υ	3	Υ	Υ	-	-	-	-	Υ
6 A	6 X 6	420	6	Υ	6	Υ	Y	_	_	_	_	Y
6 B	6 X 6	420	6	Υ	6	Y	Y	_	_	_	_	Y

2 Phase Fully Actuated (US 64 & Jenks/Richardson Road Closed Loop System) Signal System: 10534

NOTES

- 1. Refer to "Roadway Standard Drawings NCDOT" dated January 2018, "Standard Specifications for Roads and Structures" dated January 2018, and all applicable sections of the latest version of the generic Project Special Provisions. The PSP can be accessed at the following website: http://connect.ncdot.gov/resources/safety/pages/ITS-Design-Resources.aspx
- 2. Do not program signal for late night flashing operation unless otherwise directed by the Engineer.
- 3. Set all detector units to presence mode.
- 4. Locate new cabinet so as not to obstruct sight distance of vehicles turning right on red
- 5. Maximum times shown in timing chart are for free-run operation only. Coordinated signal system timing values supersede thes values.

LEGEND

Traffic Signal Head Modified Signal Head

Sign Pedestrian Signal Head With Push Button & Sign Signal Pole with Guy Signal Pole with Sidewalk Guy Inductive Loop Detector Controller & Cabinet

Junction Box

Right of Way

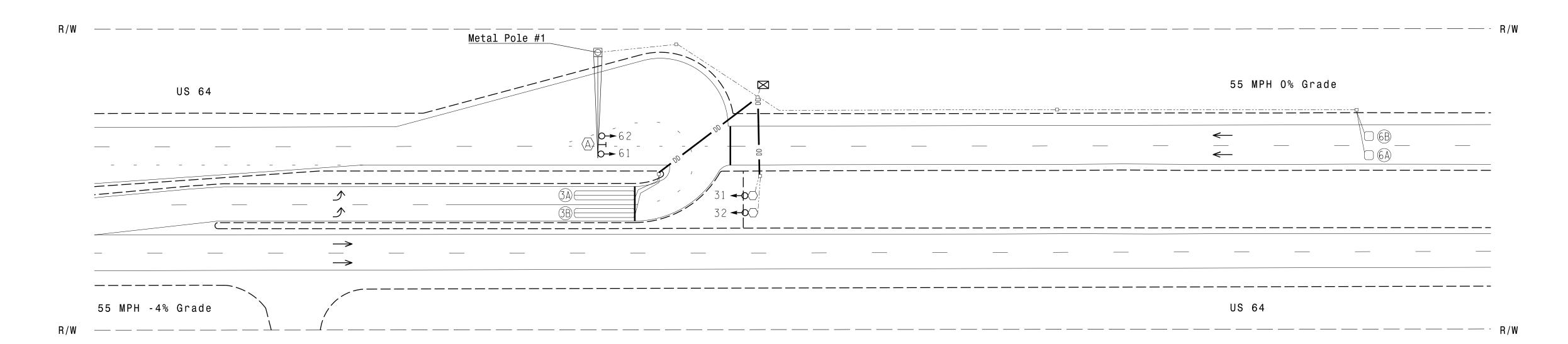
No Left Turn Sign (R3-2)

Type II Signal Pedestal

Directional Arrow Directional Drill

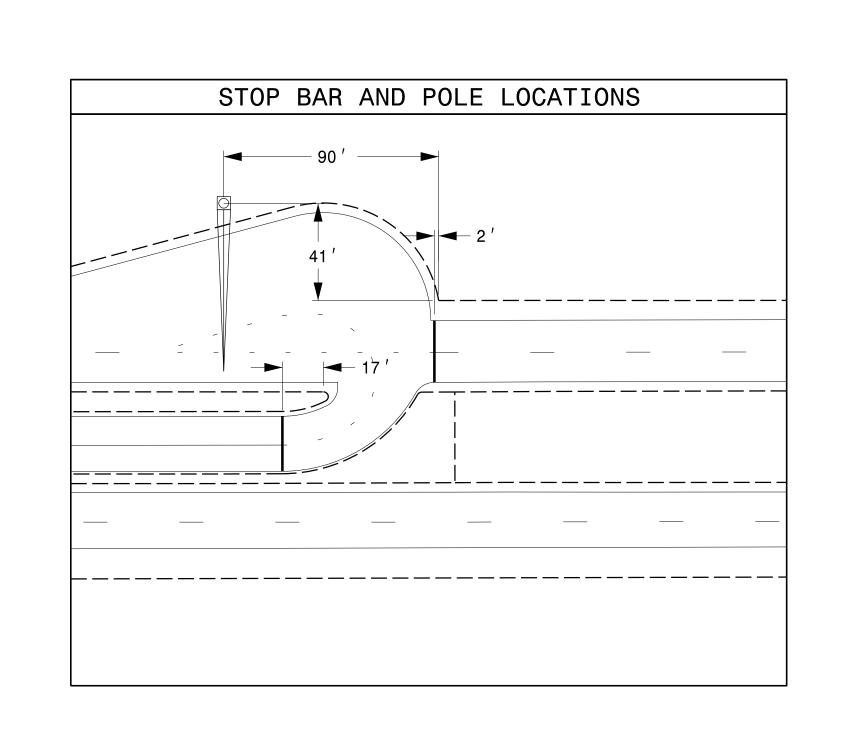
----- 2-in Underground Conduit

6. Closed loop system data: Controller Asset #1426



OASIS 2070	TIMING	CHART
	PH.	ASE
FEATURE	3	6
Min Green 1 *	7	14
Extension 1 *	2.0	6.0
Max Green 1 *	20	90
Yellow Clearance	3.0	5.2
Red Clearance	3.3	1.0
Walk 1 *	-	-
Don't Walk 1	-	-
Seconds Per Actuation *	-	1.5
Max Variable Initial *	-	46
Time Before Reduction *	-	15
Time To Reduce *	-	45
Minimum Gap	-	3.4
Recall Mode	-	MIN RECALL
Vehicle Call Memory	-	YELLOW
Dual Entry	-	-
Simultaneous Gap	ON	ON

* These values may be field adjusted. Do not adjust Min Green and Extension times for phase 6 lower than what is shown. Min Green for all other phases should not be lower than 4 seconds.



NC Dept of Transportation Division of Highways Final Drawing Date: Changseok Back ITS & Signals Unit

New Installation

1"=40'

US 64 WB U-Turn East of Richardson Road

<u>PROPOSED</u>

N/A

Division 5 Wake County August 2020 | REVIEWED BY: WJ Hamilton 750 N.Greenfield Pkwy.Garner.NC 27529 PREPARED BY: ZM ESPOSITO RKA PROJ. NO: 19331 (040) REVISIONS INIT. DATE

DOCUMENT NOT CONSIDERED

FINAL UNLESS ALL SIGNATURES COMPLETED

EXISTING

N/A

N/A

5808 Faringdon Place Raleigh, North Carolina 27609 Phone: 919-872-5115 | www.rameykemp.com | NC License No. C-0910



APPENDIX D:

Intersection Capacity Analysis

Intersection												
Int Delay, s/veh	7.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	\$		ሻ	<u> </u>	7	ሻ	7>		<u> </u>	<u> </u>	7
Traffic Vol, veh/h	14	47	15	64	50	19	19	92	71	21	54	16
Future Vol, veh/h	14	47	15	64	50	19	19	92	71	21	54	16
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	250	-	-	150	-	150	100	-	-	150	-	175
Veh in Median Storage	,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	16	52	17	71	56	21	21	102	79	23	60	18
Major/Minor N	/lajor1		<u> </u>	Major2			Minor1			Minor2		
Conflicting Flow All	77	0	0	69	0	0	341	312	61	381	299	56
Stage 1	-	-	-	-	-	-	93	93	-	198	198	-
Stage 2	-	-	-	-	-	-	248	219	-	183	101	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1522	-	-	1532	-	-	613	603	1004	577	613	1011
Stage 1	-	-	-	-	-	-	914	818	-	804	737	-
Stage 2	-	-	-	-	-	-	756	722	-	819	811	-
Platoon blocked, % Mov Cap-1 Maneuver	1522	-	-	1532	-	-	531	569	1004	439	579	1011
Mov Cap-2 Maneuver	1022	-	-	1002	-	-	531	569	1004	439	579	1011
Stage 1	_	_	-	_	-	-	904	809	-	795	703	-
Stage 2	_	_	_	_	_	_	648	689	_	652	802	_
Stage 2							0-10	307		552	302	
Approach	EB			WB			NB			SB		
HCM Control Delay, s	1.4			3.6			11.9			11.7		
HCM LOS	1.4			3.0			В			В		
TOW LOS							U			U		
Minor Long/Maior M		JDI 1 !	VIDL := 2	EDI	EDT	EDD	WDI	MDT	MDD	CDL1	CDI 2	CDL 2
Minor Lane/Major Mvm	t P	VBLn1 I		EBL	EBT	EBR	WBL	WBT			SBLn2	
Capacity (veh/h)		531	701	1522	-		1532	-	-	439		1011
HCM Control Dolay (c)			0.258	0.01	-	-	0.046	-			0.104	
HCM Control Delay (s) HCM Lane LOS		12.1 B	11.9	7.4	-	-	7.5 A	-	-	13.7 B	11.9 B	8.6
HCM 95th %tile Q(veh)		0.1	B 1	A 0	-	-	0.1	-	-	0.2	0.3	A 0.1
HOW FOUT MURE Q(VEH)		U. I		U	-		U. I		-	0.2	0.3	0.1

Intersection						
Int Delay, s/veh	3.1					
		EDD	WDI	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	\$	10	74	4	Y	07
Traffic Vol, veh/h	232	18	74	181	24	86
Future Vol, veh/h	232	18	74	181	24	86
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	258	20	82	201	27	96
N. (a. i. a. v. / N. (i. a.	-!1		11-10		/l!1	
	ajor1		Major2		Minor1	0.40
Conflicting Flow All	0	0	278	0	633	268
Stage 1	-	-	-	-	268	-
Stage 2	-	-	-	-	365	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1285	-	444	771
Stage 1	-	-	-	-	777	-
Stage 2	-	-	-	-	702	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1285	-	412	771
Mov Cap-2 Maneuver	-	-	-	-	412	-
Stage 1	-	_	-	-	777	_
Stage 2	_	_	_	_	651	_
o tago 2						
Approach	EB		WB		NB	
HCM Control Delay, s	0		2.3		11.8	
HCM LOS					В	
Minor Lane/Major Mvmt	ı	NBLn1	EBT	EBR	WBL	WBT
	<u>'</u>			LDIN		VVDI
Capacity (veh/h)		648	-	-	1285	-
HCM Lane V/C Ratio		0.189	-	-	0.064	-
HCM Control Delay (s)		11.8	-	-	8	0
HCM Lane LOS		В	-	-	Α	Α
HCM 95th %tile Q(veh)		0.7	-	-	0.2	-
,						

Intersection												
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		ሻ	1≽		ች	ĵ.	
Traffic Vol., veh/h	1	0	1	13	0	38	2	139	9	25	85	1
Future Vol, veh/h	1	0	1	13	0	38	2	139	9	25	85	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	125	-	-	150	-	-
Veh in Median Storage	e, # -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	0	1	14	0	42	2	154	10	28	94	1
Major/Minor I	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	335	319	95	314	314	159	95	0	0	164	0	0
Stage 1	151	151	-	163	163	-	-	-	-	-	-	-
Stage 2	184	168	_	151	151	_	-	_	_	_	_	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	_	_	4.12	_	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	619	598	962	639	601	886	1499	-	-	1414	-	-
Stage 1	851	772	-	839	763	-	-	-	-	-	-	-
Stage 2	818	759	-	851	772	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	580	585	962	628	588	886	1499	-	-	1414	-	-
Mov Cap-2 Maneuver	625	606	-	667	615	-	-	-	-	-	-	-
Stage 1	850	757	-	838	762	-	-	-	-	-	-	-
Stage 2	778	758	-	833	757	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	9.8			9.7			0.1			1.7		
HCM LOS	A			Α								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1499			758	818	1414					
HCM Lane V/C Ratio		0.001	_	_	0.003		0.02	_	_			
HCM Control Delay (s)		7.4	_	_	9.8	9.7	7.6	-	_			
HCM Lane LOS		A	_	_	Α.	A	A	_	_			
HCM 95th %tile Q(veh))	0	_	_	0	0.2	0.1	-	_			
/ 5 / 5 6 6		- 5			J	5.2	3.1					

Intersection												
Int Delay, s/veh	10.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		^	7						1		र्स	
Traffic Vol., veh/h	0	1020	21	0	0	0	0	0	244	0	141	0
Future Vol, veh/h	0	1020	21	0	0	0	0	0	244	0	141	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	Yield	-	-	None	-	-	None	-	-	None
Storage Length	-	-	175	-	-	-	-	-	0	-	-	-
Veh in Median Storage,	# -	0	-	-	16983	-	-	0	-	-	0	-
Grade, %	-	-2	-	-	0	-	-	1	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	1133	23	0	0	0	0	0	271	0	157	0
Major/Minor M	ajor1						/linor1		<u> </u>	/linor2		
Conflicting Flow All	-	0	0				-	-	567	567	1133	-
Stage 1	-	-	-				-	-	-	0	0	-
Stage 2	-	-	-				-	-	-	567	1133	-
Critical Hdwy	-	-	-				-	-	7.04	7.54	6.54	-
Critical Hdwy Stg 1	-	-	-				-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-				-	-	-	6.54	5.54	-
Follow-up Hdwy	-	-	-				-	-	3.32	3.52	4.02	-
Pot Cap-1 Maneuver	0	-	-				0	0	459	406	202	0
Stage 1	0	-	-				0	0	-	-	-	0
Stage 2	0	-	-				0	0	-	476	276	0
Platoon blocked, %		-	-									
Mov Cap-1 Maneuver	-	-	-				-	-	459	166	202	-
Mov Cap-2 Maneuver	-	-	-				-	-	-	166	202	-
Stage 1	-	-	-				-	-	-	-	-	-
Stage 2	-	-	-				-	-	-	195	276	-
Approach	EB						NB			SB		
HCM Control Delay, s	0						23.5			66		
HCM LOS							С			F		
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR S	SBLn1							
Capacity (veh/h)		459	-	-	202							
HCM Lane V/C Ratio		0.591	-	-	0.776							
HCM Control Delay (s)		23.5	-	-	66							
HCM Lane LOS		С	-	-	F							
HCM 95th %tile Q(veh)		3.7	-	-	5.3							

Intersection						
Int Delay, s/veh	0.3					
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations				^	- ሻ	
Traffic Vol, veh/h	0	0		1128	24	0
Future Vol, veh/h	0	0	0	1128	24	0
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	0	1253	27	0
N A			4 1 0		l' 1	
Major/Minor		I\	/lajor2		/linor1	
Conflicting Flow All			-	-	627	-
Stage 1			-	-	0	-
Stage 2			-	-	627	-
Critical Hdwy			-	-	6.84	-
Critical Hdwy Stg 1			-	-	-	-
Critical Hdwy Stg 2			-	-	5.84	-
Follow-up Hdwy			-	-	3.52	-
Pot Cap-1 Maneuver			0	-	416	0
Stage 1			0	-	-	0
Stage 2			0	-	495	0
Platoon blocked, %				-		
Mov Cap-1 Maneuver			_	-	416	-
Mov Cap-2 Maneuver			_	_	416	-
Stage 1			_	_	-	_
Stage 2			_	_	495	_
Jiago Z					773	
Approach			WB		NB	
HCM Control Delay, s			0		14.2	
HCM LOS					В	
Minor Lang/Major Muset	N	IDI n1	WDT			
Minor Lane/Major Mvmt	ľ	VBLn1	WBT			
Capacity (veh/h)		416	-			
HCM Lane V/C Ratio		0.064	-			
HCM Control Delay (s)		14.2	-			
HCM Lane LOS		В	-			
HCM 95th %tile Q(veh)		0.2	-			

Intersection												
Int Delay, s/veh	8.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ነ	Þ				7	ነ	ĵ»		1		7
Traffic Vol, veh/h	13	99	21	100	81	39	19	85	115	29	73	8
Future Vol, veh/h	13	99	21	100	81	39	19	85	115	29	73	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	250	-	-	150	-	150	100	-	-	150	-	175
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	14	110	23	111	90	43	21	94	128	32	81	9
Major/Minor	Major1			Major2		1	Minor1			Minor2		
Conflicting Flow All	133	0	0	133	0	0	529	505	122	573	473	90
Stage 1	100	-	-	-	-	-	150	150	122	312	312	-
Stage 2			_	_	_	_	379	355	_	261	161	_
Critical Hdwy	4.12	_	_	4.12	_	_	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	_	_	-	_	_	6.12	5.52	-	6.12	5.52	- 0.22
Critical Hdwy Stg 2	-	_	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	_	_	2.218	_	_	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1452	-	-	1452	-	-	460	470	929	430	490	968
Stage 1	-	_	_		_	_	853	773	-	699	658	-
Stage 2	-	_	-	-	-	-	643	630	-	744	765	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1452	-	-	1452	-	-	368	430	929	289	448	968
Mov Cap-2 Maneuver	-	-	-	-	-	-	368	430	-	289	448	-
Stage 1	-	-	-	-	-	-	844	765	-	692	608	-
Stage 2	-	-	-	-	-	-	510	582	-	557	757	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.7			3.5			14.1			15.5		
HCM LOS	0.7			3.5			14.1 B			15.5 C		
TICIVI LUJ							ט			C		
Minor Lane/Major Mvm	nt I	NBLn1 I	\IDI p2	EBL	EBT	EBR	WBL	WBT	WDD	CDI n1	SBLn2	CDI n2
	π							VVD1				
Capacity (veh/h)		368	622	1452	-		1452	-	-	20,	448	968
HCM Control Dolay (c)		0.057	0.357	0.01	-	-	0.077	-		0.111	0.181	0.009
HCM Long LOS		15.4	14	7.5	-	-	7.7	-	-		14.8	8.8
HCM OF the 90 tille O(yeah	1	C	B 1 4	A	-	-	A	-	-	C	B	A
HCM 95th %tile Q(veh)	0.2	1.6	0	-	-	0.2	-	-	0.4	0.7	0

Int Delay, s/veh	Intersection						
Movement		/1 Q					
Lane Configurations							
Traffic Vol, veh/h 327 41 209 344 25 149 Future Vol, veh/h 327 41 209 344 25 149 Conflicting Peds, #/hr 0 0 0 0 0 0 0 Sign Control Free Free Free Free Free Free Stop Stop RT Channelized - None - 0 0 - - 0 0 - - - 0 0 0 0 -			EBR	WBL			NBR
Future Vol, veh/h 327 41 209 344 25 149 Conflicting Peds, #/hr 0 - None - None None </td <td></td> <td>₽</td> <td></td> <td></td> <td>ની</td> <td>W</td> <td></td>		₽			ની	W	
Conflicting Peds, #/hr 0 0 0 0 0 0 0 Sign Control Free Free Free Free Free Free Stop Stop Stop Stop Rtop None <	Traffic Vol, veh/h	327	41	209	344	25	149
Sign Control Free RTC Pree RTC Free RTC Pree RTC Free RTC Pree RTC Free RTC Pree RTC Stop RT Channelized RTC Pree RTC PR	Future Vol, veh/h	327	41	209	344	25	149
RT Channelized - None - None - None Storage Length	Conflicting Peds, #/hr	0	0	0	0	0	0
RT Channelized - None - None - None Storage Length		Free	Free	Free	Free	Stop	Stop
Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 90 90 90 90 90 90 Heavy Vehicles, % 2 3 386 166 4		-	None	-	None		
Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Peak Hour Factor 90 90 90 90 90 90 Heavy Vehicles, % 2 3 386 166 4	Storage Length	-		-		0	-
Grade, % 0 - - 0 0 - Peak Hour Factor 90 166 66		# 0	-	_	0		-
Peak Hour Factor 90				_			_
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2							
Mymt Flow 363 46 232 382 28 166 Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 409 0 1232 386 Stage 1 - - - 386 - Stage 2 - - - 846 - Critical Hdwy - - 4.12 - 6.42 6.22 Critical Hdwy Stg 1 - - - 5.42 - - Critical Hdwy Stg 2 - - - 5.42 - - Follow-up Hdwy - - 2.218 - 3.518 3.318 Pot Cap-1 Maneuver - 1150 - 196 662 Stage 1 - - - - 421 - Platoon blocked, % - - - - 4421 - Mov Cap-2 Maneuver - - 1150							
Major/Minor Major1 Major2 Minor1 Conflicting Flow All 0 0 409 0 1232 386 Stage 1 - - - 386 - Stage 2 - - - 846 - Critical Hdwy - 4.12 - 6.42 6.22 Critical Hdwy Stg 1 - - - 5.42 - Follow-up Hdwy - - 2.218 - 3.518 3.318 Pot Cap-1 Maneuver - 1150 - 196 662 Stage 1 - - - 687 - Stage 2 - - - 421 - Mov Cap-1 Maneuver - 1150 - 146 662 Mov Cap-2 Maneuver - - 1150 - 146 - Stage 1 - - - 687 - - Stage 2 - </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Conflicting Flow All 0 0 409 0 1232 386 Stage 1 - - - 386 - Stage 2 - - - - 846 - Critical Hdwy - - 4.12 - 6.42 6.22 Critical Hdwy Stg 1 - - - 5.42 - Critical Hdwy Stg 2 - - - 5.42 - Follow-up Hdwy - - 2.218 - 3.518 3.318 Pot Cap-1 Maneuver - 1150 - 196 662 Stage 1 - - - 687 - Stage 2 - - - 146 662 Mov Cap-2 Maneuver - - 146 - - Stage 1 - - - 687 - - - 146 - Stage 2 - - -	IVIVIIIL FIOW	303	40	232	302	20	100
Conflicting Flow All 0 0 409 0 1232 386 Stage 1 - - - 386 - Stage 2 - - - - 846 - Critical Hdwy - - 4.12 - 6.42 6.22 Critical Hdwy Stg 1 - - - 5.42 - Critical Hdwy Stg 2 - - - 5.42 - Follow-up Hdwy - - 2.218 - 3.518 3.318 Pot Cap-1 Maneuver - 1150 - 196 662 Stage 1 - - - 687 - Stage 2 - - - 146 662 Mov Cap-2 Maneuver - - 146 - - Stage 1 - - - 687 - - - 146 - Stage 2 - - -							
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Stage 1 - - - 386 - Stage 2 - - - 846 - Critical Hdwy - - 4.12 - 6.42 6.22 Critical Hdwy Stg 1 - - - 5.42 - Critical Hdwy Stg 2 - - - 5.42 - Follow-up Hdwy - - 2.218 - 3.518 3.318 Pot Cap-1 Maneuver - 1150 - 196 662 Stage 1 - - - 687 - Stage 2 - - - 146 662 Mov Cap-1 Maneuver - - 1150 - 146 662 Mov Cap-2 Maneuver - - - 687 - Stage 1 - - - 687 - Stage 2 - - - 313 - Approach EB WB NB HCM Control Delay, s 0 3.4 19.5<			0	409	0	1232	386
Stage 2 - - - 846 - Critical Hdwy - 4.12 - 6.42 6.22 Critical Hdwy Stg 1 - - - 5.42 - Critical Hdwy Stg 2 - - - 5.42 - Follow-up Hdwy - - 2.218 - 3.518 3.318 Pot Cap-1 Maneuver - 1150 - 196 662 Stage 1 - - - 687 - Stage 2 - - - 421 - Platoon blocked, % - - - - 421 - Mov Cap-1 Maneuver - 1150 - 146 662 Mov Cap-2 Maneuver - - 146 - - Stage 1 - - - 687 - Stage 2 - - - 313 - Approach EB WB NB HCM Control Delay, s 0 3.4 19.5 <			-				
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Critical Hdwy Stg 2 - - - 5.42 - Follow-up Hdwy - - 2.218 - 3.518 3.318 Pot Cap-1 Maneuver - - 1150 - 196 662 Stage 1 - - - - 421 - Platoon blocked, % -							
Follow-up Hdwy - 2.218 - 3.518 3.318 Pot Cap-1 Maneuver - 1150 - 196 662 Stage 1 687 - 687 Stage 2 421 - Platoon blocked, % 1150 - 146 662 Mov Cap-1 Maneuver - 1150 - 146 662 Mov Cap-2 Maneuver 1150 - 146 - Stage 1 687 - Stage 2 313 - Approach EB WB NB HCM Control Delay, s 0 3.4 19.5 HCM LOS C Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 439 - 1150 - HCM Lane V/C Ratio 0.44 - 0.202 - HCM Control Delay (s) 19.5 - 8.9 0 HCM Lane LOS C - A A							
Pot Cap-1 Maneuver - - 1150 - 196 662 Stage 1 - - - 687 - Stage 2 - - - 421 - Platoon blocked, % -			-				
Stage 1 - - - 687 - Stage 2 - - - 421 - Platoon blocked, % - - - - - Mov Cap-1 Maneuver - - 1150 - 146 662 Mov Cap-2 Maneuver - - - 146 - Stage 1 - - - 687 - Stage 2 - - - 313 - Approach EB WB NB HCM Control Delay, s 0 3.4 19.5 HCM LOS C - 1150 - Minor Lane/Major Mvmt NBLn1 EBR WBL WBT Capacity (veh/h) 439 - - 1150 - HCM Lane V/C Ratio 0.44 - - 0.202 - HCM Control Delay (s) 19.5 - - 8.9 0 HCM Lane LOS C - - A A							
Stage 2 - - - 421 - Platoon blocked, % - - - - - Mov Cap-1 Maneuver - - 1150 - 146 662 Mov Cap-2 Maneuver - - - - 146 - Stage 1 - - - 687 - Stage 2 - - - - 313 - Approach EB WB NB HCM Control Delay, s 0 3.4 19.5 - HCM LOS C - - 1150 - Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 439 - - 1150 - HCM Lane V/C Ratio 0.44 - - 0.202 - HCM Lane LOS C - - A A	•						
Platoon blocked, % - - - Mov Cap-1 Maneuver - - 1150 - 146 662 Mov Cap-2 Maneuver - - - - 146 - Stage 1 - - - - 687 - Stage 2 - - - - 313 - Approach EB WB NB NB HCM Control Delay, s 0 3.4 19.5 - HCM LOS C C - - 1150 - Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 439 - - 1150 - HCM Lane V/C Ratio 0.44 - - 0.202 - HCM Lane LOS C - - A A		-	-	-			
Mov Cap-1 Maneuver - - 1150 - 146 662 Mov Cap-2 Maneuver - - - - 146 - Stage 1 - - - - 687 - Stage 2 - - - - 313 - Approach EB WB NB HCM Control Delay, s 0 3.4 19.5 - HCM LOS C C - <td></td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>421</td> <td>-</td>		-	-	-	-	421	-
Mov Cap-2 Maneuver - - - 146 - Stage 1 - - - 687 - Stage 2 - - - 313 - Approach EB WB NB HCM Control Delay, s 0 3.4 19.5 HCM LOS C C Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 439 - 1150 - 1150 - HCM Lane V/C Ratio 0.44 - 0.202 - HCM Control Delay (s) HCM Lane LOS C - A A	The state of the s	-	-		-		
Stage 1 - - - 687 - Stage 2 - - - 313 - Approach EB WB NB HCM Control Delay, s 0 3.4 19.5 HCM LOS C C Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 439 - 1150 - HCM Lane V/C Ratio 0.44 - 0.202 - HCM Control Delay (s) 19.5 - 8.9 0 HCM Lane LOS C - A A	Mov Cap-1 Maneuver	-	-	1150	-	146	662
Stage 2 - - - 313 - Approach EB WB NB HCM Control Delay, s 0 3.4 19.5 HCM LOS C C Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 439 - 1150 - HCM Lane V/C Ratio 0.44 - 0.202 - HCM Control Delay (s) 19.5 - 8.9 0 HCM Lane LOS C - A A	Mov Cap-2 Maneuver	-	-	-	-	146	-
Approach EB WB NB HCM Control Delay, s 0 3.4 19.5 HCM LOS C Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 439 - - 1150 - HCM Lane V/C Ratio 0.44 - - 0.202 - HCM Control Delay (s) 19.5 - - 8.9 0 HCM Lane LOS C - - A A	Stage 1	-	-	-	-	687	-
Approach EB WB NB HCM Control Delay, s 0 3.4 19.5 HCM LOS C Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 439 - - 1150 - HCM Lane V/C Ratio 0.44 - - 0.202 - HCM Control Delay (s) 19.5 - - 8.9 0 HCM Lane LOS C - - A A	O .	-	-	-	-	313	-
HCM Control Delay, s	J. J.						
HCM Control Delay, s	Λ 1	- F-D		MD		ND	
Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 439 - - 1150 - HCM Lane V/C Ratio 0.44 - - 0.202 - HCM Control Delay (s) 19.5 - - 8.9 0 HCM Lane LOS C - - A A							
Minor Lane/Major Mvmt NBLn1 EBT EBR WBL WBT Capacity (veh/h) 439 - - 1150 - HCM Lane V/C Ratio 0.44 - - 0.202 - HCM Control Delay (s) 19.5 - - 8.9 0 HCM Lane LOS C - - A A		0		3.4			
Capacity (veh/h) 439 - - 1150 - HCM Lane V/C Ratio 0.44 - - 0.202 - HCM Control Delay (s) 19.5 - - 8.9 0 HCM Lane LOS C - A A	HCM LOS					С	
Capacity (veh/h) 439 - - 1150 - HCM Lane V/C Ratio 0.44 - - 0.202 - HCM Control Delay (s) 19.5 - - 8.9 0 HCM Lane LOS C - A A							
Capacity (veh/h) 439 - - 1150 - HCM Lane V/C Ratio 0.44 - - 0.202 - HCM Control Delay (s) 19.5 - - 8.9 0 HCM Lane LOS C - A A	Minor Lano/Major Mymt	N	JRI n1	FRT	FRD	\M/RI	\MRT
HCM Lane V/C Ratio 0.44 - - 0.202 - HCM Control Delay (s) 19.5 - - 8.9 0 HCM Lane LOS C - - A A					LDK		WDI
HCM Control Delay (s) 19.5 - 8.9 0 HCM Lane LOS C - A A					-		-
HCM Lane LOS C A A				-	-		
				-	-		
$\Box CM \cap C$ th $O(tilo \cap C(tob))$ 2.2				-	-		Α
	HCM 95th %tile Q(veh)		2.2	-	-	8.0	-

Intersection												
Int Delay, s/veh	2.2											
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement	EBL		EBK	WBL		WBK			NBK			SBR
Lane Configurations	2	↔ 1	3	13	4	33	`	1 26	20	ነ	135	3
Traffic Vol, veh/h Future Vol, veh/h	2	1	3	13	1 1	33	2	126	20	38	135	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	310p	Jiop	None	- Jiop	- Jiop	None	-	-	None	-	-	None
Storage Length		_	TNOTIC	_	_	-	125	_	- INOTIC	150	_	-
Veh in Median Storage		1		_	1	_	120	0		-	0	_
Grade, %	-	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	2	1	3	14	1	37	2	140	22	42	150	3
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	410	402	152	393	392	151	153	0	0	162	0	0
Stage 1	236	236	132	155	155	-	-	-	-	- 102	-	-
Stage 2	174	166	_	238	237	_	_	_	_	_	_	_
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	_	_	4.12	_	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52		-	_	_	-	_	-
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52	_	-	-	_	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	552	537	894	566	544	895	1428	-	-	1417	-	-
Stage 1	767	710	-	847	769	-	-	-	-	-	-	-
Stage 2	828	761	-	765	709	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	516	520	894	550	527	895	1428	-	-	1417	-	-
Mov Cap-2 Maneuver	581	558	-	606	568	-	-	-	-	-	-	-
Stage 1	766	689	-	846	768	-	-	-	-	-	-	-
Stage 2	792	760	-	738	688	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	10.2			9.9			0.1			1.6		
HCM LOS	В			Α								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1428	-	-	698	782	1417	-	-			
HCM Lane V/C Ratio		0.002	-	-	0.01	0.067	0.03	-	-			
HCM Control Delay (s)		7.5	-	-	10.2	9.9	7.6	-	-			
HCM Lane LOS		A	-	-	В	Α	A	-	-			
HCM 95th %tile Q(veh)	0	-	-	0	0.2	0.1	-	-			

Intersection															
Int Delay, s/veh	30.2														
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR			
Lane Configurations	LDL	† †	T T	VVDL	וטייי	WDIX	NDL	וטוו	TVDIX	JUL	<u>3₽1</u>	JUK			
Traffic Vol, veh/h	0	1132	30	0	0	0	0	0	203	0	196	0			
Future Vol, veh/h	0	1132	30	0	0	0	0	0	203	0	196	0			
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0			
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop			
RT Channelized	-	-	Yield	-	-	None	- Jiup	Jiop -	None	310p -	310p	None			
Storage Length	_		175		_	TNOTIC	_	_	0	_	_	-			
Veh in Median Storage		0	-		16983	_	_	0	-	_	0	_			
Grade, %	-, π -	-2	-	_	0	-	_	1	_	-	0	_			
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90			
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2			
Mvmt Flow	0	1258	33	0	0	0	0	0	226	0	218	0			
IVIVIIIL I IOW	U	1230	33	U	U	U	U	U	220	U	210	U			
	Major1					N	/linor1			Minor2					
Conflicting Flow All	-	0	0				-	-	629	629	1258	-			
Stage 1	-	-	-				-	-	-	0	0	-			
Stage 2		-	-				-	-	-	629	1258	-			
Critical Hdwy	-	-	-				-	-	7.04	7.54	6.54	-			
Critical Hdwy Stg 1	-	-	-				-	-	-	-	-	-			
Critical Hdwy Stg 2	-	-	-				-	-	-	6.54	5.54	-			
Follow-up Hdwy	-	-	-				-	-	3.32	3.52	4.02	-			
Pot Cap-1 Maneuver	0	-	-				0	0	418	367	~ 170	0			
Stage 1	0	-	-				0	0	-	-	-	0			
Stage 2	0	-	-				0	0	-	437	241	0			
Platoon blocked, %		-	-												
Mov Cap-1 Maneuver	-	-	-				-	-	418		~ 170	-			
Mov Cap-2 Maneuver	-	-	-				-	-	-	169	~ 170	-			
Stage 1	-	-	-				-	-	-	-	-	-			
Stage 2	-	-	-				-	-	-	201	241	-			
Approach	EB						NB			SB					
HCM Control Delay, s	0						23.3			216.7					
HCM LOS							C			F					
TION LOO										'					
	IDI. 1			201											
Minor Lane/Major Mvm	VBLn1	EBT	EBR S	SBLn1											
Capacity (veh/h)	418	-	-	., 0											
HCM Lane V/C Ratio	0.54	-		1.281											
HCM Control Delay (s)	23.3	-	-	216.7											
HCM Lane LOS C			-	-	F										
HCM 95th %tile Q(veh)		3.1	-	-	12.5										
Notes															
~: Volume exceeds car	nacity	\$ De	lav exc	eeds 30	00s	+: Com	nutation	Not D	efined	*· ΔII	maiory	/olume i	in platoon		
. Volume exceeds cap	Judity	ψ. DC	hay chu	iccus 3	003	T. CUIT	patatioi	ו ויטניט	cilicu	d *: All major volume in platoon					

Intersection						
Int Delay, s/veh	0.5					
		LDD	WDL	WDT	NDI	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	0	0	0	14/2	ነ	0
Traffic Vol, veh/h	0	0		1463	39	0
Future Vol, veh/h	0	0	0	1463	39	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	0	1626	43	0
Major/Minor		N	Major2	N	/linor1	
Conflicting Flow All			-		813	_
Stage 1			_	_	0	_
Stage 2			_	_	813	_
Critical Hdwy			_	_	6.84	_
Critical Hdwy Stg 1			_	_	-	_
Critical Hdwy Stg 2			_	_	5.84	_
Follow-up Hdwy			_	_	3.52	_
Pot Cap-1 Maneuver			0	_	316	0
Stage 1			0	_	-	0
Stage 2			0	_	396	0
Platoon blocked, %			U		370	U
Mov Cap-1 Maneuver				-	316	_
Mov Cap-1 Maneuver			-	_	316	-
			-	-		
Stage 1			-	-	207	-
Stage 2			-	-	396	-
Approach			WB		NB	
HCM Control Delay, s			0		18.2	
HCM LOS					С	
NA'		UDL 4	MOT			
Minor Lane/Major Mvmt	. [VBLn1	WBT			
Capacity (veh/h)		316	-			
HCM Lane V/C Ratio		0.137	-			
HCM Control Delay (s)		18.2	-			
HCM Lane LOS		С	-			
HCM 95th %tile Q(veh)		0.5	-			

	•	-	•	•	←	•	4	†	~	/	ţ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*	^}		ሻ	†	7	ሻ	ĵ.		*	†	7
Traffic Volume (vph)	54	68	22	93	65	120	37	110	128	93	102	48
Future Volume (vph)	54	68	22	93	65	120	37	110	128	93	102	48
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250		0	150		150	100		0	150		175
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.964				0.850		0.919				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1796	0	1770	1863	1583	1770	1712	0	1770	1863	1583
Flt Permitted	0.710			0.692			0.684			0.596		
Satd. Flow (perm)	1323	1796	0	1289	1863	1583	1274	1712	0	1110	1863	1583
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1889			1311			1771			2925	
Travel Time (s)		28.6			19.9			26.8			44.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	60	76	24	103	72	133	41	122	142	103	113	53
Shared Lane Traffic (%)												
Lane Group Flow (vph)	60	100	0	103	72	133	41	264	0	103	113	53
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	Perm
Protected Phases		2			6			4			8	
Permitted Phases	2			6		6	4			8		8
Detector Phase	2	2		6	6	6	4	4		8	8	8
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0	7.0	7.0	7.0		7.0	7.0	7.0
Minimum Split (s)	14.0	14.0		14.0	14.0	14.0	14.0	14.0		14.0	14.0	14.0
Total Split (s)	26.0	26.0		26.0	26.0	26.0	34.0	34.0		34.0	34.0	34.0
Total Split (%)	43.3%	43.3%		43.3%	43.3%	43.3%	56.7%	56.7%		56.7%	56.7%	56.7%
Maximum Green (s)	19.0	19.0		19.0	19.0	19.0	27.0	27.0		27.0	27.0	27.0
Yellow Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	Min	Min		Min	Min	Min	None	None		None	None	None
Act Effct Green (s)	12.1	12.1		12.1	12.1	12.1	12.7	12.7		12.7	12.7	12.7
Actuated g/C Ratio	0.35	0.35		0.35	0.35	0.35	0.36	0.36		0.36	0.36	0.36
v/c Ratio	0.13	0.16		0.23	0.11	0.24	0.09	0.43		0.26	0.17	0.09
Control Delay	9.8	9.7		10.8	9.3	10.6	7.7	10.6		9.4	7.9	7.5
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	9.8	9.7		10.8	9.3	10.6	7.7	10.6		9.4	7.9	7.5
LOS	Α	Α		В	Α	В	Α	В		Α	Α	Α
Approach Delay		9.7			10.3			10.2			8.4	
Approach LOS		Α			В			В			Α	

1: Richardson Rd & Olive Chapel Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	7	12		13	8	16	4	31		11	12	5
Queue Length 95th (ft)	27	39		42	30	51	18	82		38	37	21
Internal Link Dist (ft)		1809			1231			1691			2845	
Turn Bay Length (ft)	250			150		150	100			150		175
Base Capacity (vph)	805	1093		785	1134	964	1071	1440		933	1567	1331
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.07	0.09		0.13	0.06	0.14	0.04	0.18		0.11	0.07	0.04

Intersection Summary

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 35 Natural Cycle: 40

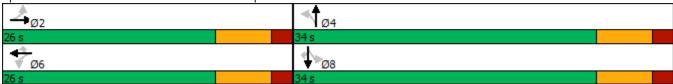
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.43

Intersection Signal Delay: 9.7 Intersection LOS: A Intersection Capacity Utilization 43.8% ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: Richardson Rd & Olive Chapel Rd



Intersection						
Int Delay, s/veh	2.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	EB1	LDK	WDL	WBI 4	INDL	NDK
Traffic Vol, veh/h	419	20	83	심 328	'T' 27	97
Future Vol, veh/h	419	20	83	328	27	97
	419	0	0	328		
Conflicting Peds, #/hr		Free	Free	Free	O Ctop	O Ctop
Sign Control RT Channelized	Free			None	Stop	Stop
Storage Length	-	None	-		-	None
	- # 0		-	-	0	
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	466	22	92	364	30	108
Major/Minor N	1ajor1	ľ	Major2		Minor1	
Conflicting Flow All	0	0	488	0	1025	477
Stage 1	-	-	-	-	477	-
Stage 2	-	_	_	-	548	_
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	_	_		_	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	_	_	2.218	_		3.318
Pot Cap-1 Maneuver	_	-	1075	-	260	588
Stage 1	_	_	- 1070	_	624	-
Stage 2	_	_	_	_	579	_
Platoon blocked, %				_	317	
Mov Cap-1 Maneuver			1075	_	232	588
Mov Cap-1 Maneuver	-		1075	-	232	500
		-	-		624	
Stage 1	-	-		-		-
Stage 2	-	-	-	-	517	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.7		16.8	
HCM LOS					С	
Ndinon Long /Ndolog Nd		UDI 1	EDT	EDD	MDI	MDT
Minor Lane/Major Mvmt	. [VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		441	-		1075	-
HCM Lane V/C Ratio		0.312	-		0.086	-
HCM Control Delay (s)		16.8	-	-	8.7	0
HCM Lane LOS		С	-	-	Α	Α
HCM 95th %tile Q(veh)		1.3	-	-	0.3	-

Intersection												
Int Delay, s/veh	1.9											
	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Movement Lang Configurations	EBL		EBR	WBL		WBR			NDK			SBR
Lane Configurations	1	- ♣	1	1 Γ	♣	100	`	}	10	ነ	^	1
Traffic Vol, veh/h	1 1	0	1	15	0	103	2	478 478	10 10	48 48	503 503	1
Future Vol, veh/h	0	0	1 0	15	0	103	2	4/8	0	40	0	1 0
Conflicting Peds, #/hr Sign Control		O Stop		Stop				Free	Free	Free	Free	Free
RT Channelized	Stop	Stop	Stop None	Siop -	Stop	Stop None	Free -	riee -	None	riee -		None
Storage Length	-	-	None	-	-	None -	125	-	None -	150	-	None
Veh in Median Storage		1	-	-	1	-	123	0	-	100	0	-
Grade, %	Ξ, π -	0	_	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	0	1	17	0	114	2	531	11	53	559	1
IVIVIII I IOVV		U		17	U	117	2	551	- 11	55	337	
Major/Minor	Minara			Minera			Moler1			Mais		
	Minor2	4040		Minor1	400=		Major1			Major2		
Conflicting Flow All	1264	1212	560	1207	1207	537	560	0	0	542	0	0
Stage 1	666	666	-	541	541	-	-	-	-	-	-	-
Stage 2	598	546	- ())	666	666	- / 00	4.10	-	-	- 4.10	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	2 210	6.12	5.52	2 210	2 210	-	-	2 210	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318 544	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	146 449	182 457	528	160 525	183 521	344	1011	-	-	1027	-	-
Stage 1 Stage 2	449	518	-	449	457	-	-	-	-	-	-	-
Platoon blocked, %	409	210	-	449	437	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	111	172	528	153	173	544	1011	-	-	1027	-	-
Mov Cap-1 Maneuver	223	283	520	282	293	344	1011	_	_	1027	_	_
Stage 1	448	433	-	524	520	-	-	-	-		-	<u>-</u>
Stage 2	385	517		425	433							
Jiage Z	303	317		723	700							
A				ME			ND			0.0		
Approach	EB			WB			NB			SB		
HCM Control Delay, s	16.5			15.1			0			8.0		
HCM LOS	С			С								
Minor Lane/Major Mvn	nt	NBL	NBT	NBR	EBLn1V	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1011	-	-	314	487	1027	-	-			
HCM Lane V/C Ratio		0.002	-	-	0.007		0.052	-	-			
HCM Control Delay (s))	8.6	-	-	16.5	15.1	8.7	-	-			
HCM Lane LOS		Α	-	-	С	С	Α	-	-			
HCM 95th %tile Q(veh)	0	-	-	0	1.1	0.2	-	-			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		† †	7						77		414	
Traffic Volume (vph)	0	1124	267	0	0	0	0	0	915	0	719	0
Future Volume (vph)	0	1124	267	0	0	0	0	0	915	0	719	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		-2%			0%			1%			0%	
Storage Length (ft)	0		175	0		0	0		0	0		0
Storage Lanes	0		1	0		0	0		2	0		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.88	0.95	0.95	1.00
Frt			0.850						0.850			
Flt Protected												
Satd. Flow (prot)	0	3575	1599	0	0	0	0	0	2773	0	3539	0
Flt Permitted												-
Satd. Flow (perm)	0	3575	1599	0	0	0	0	0	2773	0	3539	0
Right Turn on Red	-		No			No	-		No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)		55			55			45			35	
Link Distance (ft)		3066			489			978			454	
Travel Time (s)		38.0			6.1			14.8			8.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	1249	297	0	0	0	0	0	1017	0	799	0
Shared Lane Traffic (%)		1217	277						1017		.,,	
Lane Group Flow (vph)	0	1249	297	0	0	0	0	0	1017	0	799	0
Turn Type		NA	Perm						Perm		NA	
Protected Phases		2	1 01111						1 01111		8	
Permitted Phases		_	2						8	8		
Detector Phase		2	2						8	8	8	
Switch Phase		_										
Minimum Initial (s)		14.0	14.0						7.0	7.0	7.0	
Minimum Split (s)		20.8	20.8						13.2	13.2	13.2	
Total Split (s)		30.0	30.0						30.0	30.0	30.0	
Total Split (%)		50.0%	50.0%						50.0%	50.0%	50.0%	
Maximum Green (s)		23.2	23.2						23.8	23.8	23.8	
Yellow Time (s)		5.4	5.4						3.0	3.0	3.0	
All-Red Time (s)		1.4	1.4						3.2	3.2	3.2	
Lost Time Adjust (s)		-1.8	-1.8						-1.2	0.2	-1.2	
Total Lost Time (s)		5.0	5.0						5.0		5.0	
Lead/Lag		5.0	3.0						5.0		5.0	
Lead-Lag Optimize?												
Vehicle Extension (s)		6.0	6.0						2.0	2.0	2.0	
Minimum Gap (s)		3.4	3.4						0.2	0.2	0.2	
Time Before Reduce (s)		15.0	15.0						0.2	0.2	0.2	
Time To Reduce (s)		45.0	45.0						0.0	0.0	0.0	
Recall Mode		C-Min	C-Min						None	None	None	
Act Effct Green (s)		25.2	25.2						24.8	None	24.8	
Actuated g/C Ratio		0.42	0.42						0.41		0.41	
		0.42							0.41			
v/c Ratio			0.44								0.55	
Control Delay		22.1	15.1						28.2		10.9	
Queue Delay		0.0	0.0						0.0		0.0	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay		22.1	15.1						28.2		10.9	
LOS		С	В						С		В	
Approach Delay		20.7						28.2			10.9	
Approach LOS		С						С			В	
Queue Length 50th (ft)		203	74						183		106	
Queue Length 95th (ft)		#296	132						#311		m120	
Internal Link Dist (ft)		2986			409			898			374	
Turn Bay Length (ft)			175									
Base Capacity (vph)		1502	672						1155		1474	
Starvation Cap Reductn		0	0						0		0	
Spillback Cap Reductn		0	0						0		0	
Storage Cap Reductn		0	0						0		0	
Reduced v/c Ratio		0.83	0.44						0.88		0.54	
Intersection Summary												
<i>J</i> 1	Other											
Cycle Length: 60												
Actuated Cycle Length: 60												
Offset: 0 (0%), Referenced to	o phase 2:E	EBT, Star	t of Gree	n								
Natural Cycle: 55												
Control Type: Actuated-Coor	dinated											
Maximum v/c Ratio: 0.89												
Intersection Signal Delay: 20					itersection							
Intersection Capacity Utilizat	ion 99.9%			IC	CU Level of	of Service	F					
Analysis Period (min) 15												
# 95th percentile volume e.			eue may	be longe	r.							
Queue shown is maximur												
m Volume for 95th percent	ile queue is	s metered	d by upstr	eam sigr	nal.							
Splits and Phases: 4: Rich	ardson Rd	& US 64	EB									
▼ Ø2 (R)												
30 s					٠.							
					- ₽₩	78						

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations				^	ሻሻ	
Traffic Volume (vph)	0	0	0	1770	247	0
Future Volume (vph)	0	0	0	1770	247	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	0.97	1.00
Frt	1.00	1.00	1.00	0.75	0.71	1.00
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	3539	3433	0
Flt Permitted	U	U	U	3337	0.950	U
Satd. Flow (perm)	0	0	0	3539	3433	0
Right Turn on Red	U	No	U	3337	No	No
		NO			INO	NO
Satd. Flow (RTOR) Link Speed (mph)	55			55	25	
	459			2512	426	
Link Distance (ft)						
Travel Time (s)	5.7	0.00	0.00	31.1	11.6	0.00
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	0	0	1967	274	0
Shared Lane Traffic (%)	_					
Lane Group Flow (vph)	0	0	0	1967	274	0
Turn Type				NA	Prot	
Protected Phases				6	8	
Permitted Phases						
Detector Phase				6	8	
Switch Phase						
Minimum Initial (s)				14.0	7.0	
Minimum Split (s)				20.2	13.3	
Total Split (s)				46.7	13.3	
Total Split (%)				77.8%	22.2%	
Maximum Green (s)				40.5	7.0	
Yellow Time (s)				5.2	3.0	
All-Red Time (s)				1.0	3.3	
Lost Time Adjust (s)				-1.2	-1.3	
Total Lost Time (s)				5.0	5.0	
Lead/Lag				5.0	5.0	
Lead-Lag Optimize?						
Vehicle Extension (s)				6.0	2.0	
				3.4	0.2	
Minimum Gap (s)						
Time Before Reduce (s)				15.0	0.0	
Time To Reduce (s)				45.0	0.0	
Recall Mode				C-Min	None	
Act Effct Green (s)				41.7	8.3	
Actuated g/C Ratio				0.70	0.14	
v/c Ratio				0.80	0.58	
Control Delay				9.6	27.8	
Queue Delay				0.0	0.0	
Total Delay				9.6	27.8	
LOS				Α	С	
Approach Delay				9.6	27.8	
Approach LOS				Α	С	
- Prince						

Internal Link Dist (ft) 379 2432 346 Turn Bay Length (ft) 2459 474 Base Capacity (vph) 2459 474 Starvation Cap Reductn 0 0 Spillback Cap Reductn 0 0 Storage Cap Reductn 0 0 Reduced v/c Ratio 0.80 0.58 Intersection Summary		→	•	•	←	4	<i>></i>		
Queue Length 95th (ft) 288 m60 Internal Link Dist (ft) 379 2432 346 Turn Bay Length (ft) Base Capacity (vph) 2459 474 Starvation Cap Reductn 0 0 0 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.80 0.58 Intersection Summary Area Type: Other Cycle Length: 60 Actuated Cycle Length: 60 Offset: 0 (0%), Referenced to phase 6:WBT, Start of Green Natural Cycle: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB	Lane Group	EBT	EBR	WBL	WBT	NBL	NBR		
Internal Link Dist (ft) 379 2432 346 Turn Bay Length (ft) Base Capacity (vph) 2459 474 Starvation Cap Reductn 0 0 0 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.80 0.58 Intersection Summary Area Type: Other Cycle Length: 60 Actuated Cycle Length: 60 Actuated Cycle Length: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal.	Queue Length 50th (ft)				199	50			
Turn Bay Length (ft) Base Capacity (vph) 2459 474 Starvation Cap Reductn 0 0 0 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.80 0.58 Intersection Summary Area Type: Other Cycle Length: 60 Actuated Cycle Length: 60 Offset: 0 (0%), Referenced to phase 6:WBT, Start of Green Natural Cycle: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB	Queue Length 95th (ft)				288	m60			
Base Capacity (vph) Starvation Cap Reductn O Spillback Cap Reductn O Storage Cap Reductn O Reduced v/c Ratio O Reduced v/c Ratio O O O Reduced v/c Ratio O O O O O Cycle Length: 60 O O O O O O O O O O O O O		379			2432	346			
Starvation Cap Reductn 0 0 0 Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.80 0.58 Intersection Summary Area Type: Other Cycle Length: 60 Actuated Cycle Length: 60 Offset: 0 (0%), Referenced to phase 6:WBT, Start of Green Natural Cycle: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB									
Spillback Cap Reductn 0 0 0 Storage Cap Reductn 0 0 0 Reduced v/c Ratio 0.80 0.58 Intersection Summary Area Type: Other Cycle Length: 60 Actuated Cycle Length: 60 Offset: 0 (0%), Referenced to phase 6:WBT, Start of Green Natural Cycle: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB	Base Capacity (vph)				2459	474			
Storage Cap Reducth Reduced v/c Ratio 0 0 Reduced v/c Ratio 0.80 0.58 Intersection Summary Area Type: Other Cycle Length: 60 Actuated Cycle Length: 60 Offset: 0 (0%), Referenced to phase 6:WBT, Start of Green Natural Cycle: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal.					0	0			
Reduced v/c Ratio 0.80 0.58 Intersection Summary Area Type: Other Cycle Length: 60 Actuated Cycle Length: 60 Offset: 0 (0%), Referenced to phase 6:WBT, Start of Green Natural Cycle: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB					0	0			
Intersection Summary Area Type: Other Cycle Length: 60 Actuated Cycle Length: 60 Offset: 0 (0%), Referenced to phase 6:WBT, Start of Green Natural Cycle: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB									
Area Type: Other Cycle Length: 60 Actuated Cycle Length: 60 Offset: 0 (0%), Referenced to phase 6:WBT, Start of Green Natural Cycle: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB	Reduced v/c Ratio				0.80	0.58			
Cycle Length: 60 Actuated Cycle Length: 60 Offset: 0 (0%), Referenced to phase 6:WBT, Start of Green Natural Cycle: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB	Intersection Summary								
Actuated Cycle Length: 60 Offset: 0 (0%), Referenced to phase 6:WBT, Start of Green Natural Cycle: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB	Area Type:	Other							
Offset: 0 (0%), Referenced to phase 6:WBT, Start of Green Natural Cycle: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB	Cycle Length: 60								
Natural Cycle: 60 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB									
Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB		to phase 6:\	NBT, Sta	rt of Gree	en				
Maximum v/c Ratio: 0.80 Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB									
Intersection Signal Delay: 11.8 Intersection LOS: B Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB		rdinated							
Intersection Capacity Utilization 64.3% ICU Level of Service C Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB									
Analysis Period (min) 15 m Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB									
M Volume for 95th percentile queue is metered by upstream signal. Splits and Phases: 5: U-Turn East & US 64 WB		tion 64.3%			IC	:U Level o	f Service C		
Splits and Phases: 5: U-Turn East & US 64 WB									
_	m Volume for 95th percen	tile queue is	s metered	l by upsti	ream sign	al.			
_	Snlits and Phases: 5.11-T	iırn Fast &	IIS 64 W	R					
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	f _a		ች	†	7	*	f.		*	+	7
Traffic Volume (vph)	58	119	41	167	104	139	31	103	165	139	147	55
Future Volume (vph)	58	119	41	167	104	139	31	103	165	139	147	55
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250		0	150		150	100		0	150		175
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.961				0.850		0.908				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1790	0	1770	1863	1583	1770	1691	0	1770	1863	1583
Flt Permitted	0.682			0.645			0.654			0.579		
Satd. Flow (perm)	1270	1790	0	1201	1863	1583	1218	1691	0	1079	1863	1583
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1889			1311			1771			2925	
Travel Time (s)		28.6			19.9			26.8			44.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	64	132	46	186	116	154	34	114	183	154	163	61
Shared Lane Traffic (%)	01	102	10	100	110	101	01		100	101	100	01
Lane Group Flow (vph)	64	178	0	186	116	154	34	297	0	154	163	61
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	Perm
Protected Phases		2			6			4			8	
Permitted Phases	2			6		6	4			8	-	8
Detector Phase	2	2		6	6	6	4	4		8	8	8
Switch Phase												
Minimum Initial (s)	7.0	7.0		7.0	7.0	7.0	7.0	7.0		7.0	7.0	7.0
Minimum Split (s)	14.0	14.0		14.0	14.0	14.0	14.0	14.0		14.0	14.0	14.0
Total Split (s)	29.0	29.0		29.0	29.0	29.0	31.0	31.0		31.0	31.0	31.0
Total Split (%)	48.3%	48.3%		48.3%	48.3%	48.3%	51.7%	51.7%		51.7%	51.7%	51.7%
Maximum Green (s)	22.0	22.0		22.0	22.0	22.0	24.0	24.0		24.0	24.0	24.0
Yellow Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	Min	Min		Min	Min	Min	None	None		None	None	None
Act Effct Green (s)	14.2	14.2		14.2	14.2	14.2	14.6	14.6		14.6	14.6	14.6
Actuated g/C Ratio	0.36	0.36		0.36	0.36	0.36	0.37	0.37		0.37	0.37	0.37
v/c Ratio	0.14	0.28		0.43	0.17	0.27	0.08	0.47		0.39	0.24	0.10
Control Delay	10.2	10.9		13.9	10.0	11.1	9.6	13.1		13.4	10.4	9.6
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	10.2	10.9		13.9	10.0	11.1	9.6	13.1		13.4	10.4	9.6
LOS	В	В		В	В	В	Α.	В		В	В	A
Approach Delay		10.7			12.0		, ,	12.7			11.5	, (
Approach LOS		В			В			В			В	

1: Richardson Rd & Olive Chapel Rd

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	8	24		27	15	21	4	43		21	21	8
Queue Length 95th (ft)	33	73		86	50	66	21	124		73	67	31
Internal Link Dist (ft)		1809			1231			1691			2845	
Turn Bay Length (ft)	250			150		150	100			150		175
Base Capacity (vph)	817	1152		773	1199	1018	849	1179		752	1299	1103
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.08	0.15		0.24	0.10	0.15	0.04	0.25		0.20	0.13	0.06

Intersection Summary

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 39.4

Natural Cycle: 40

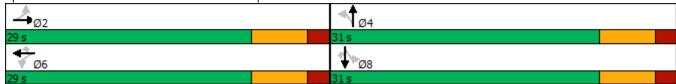
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.47

Intersection Signal Delay: 11.8 Intersection LOS: B
Intersection Capacity Utilization 57.9% ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Richardson Rd & Olive Chapel Rd



Intersection						
Int Delay, s/veh	13.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	7>	LDI	VVDL	4	¥	NDIX
Traffic Vol, veh/h	520	46	235	571	28	168
Future Vol, veh/h	520	46	235	571	28	168
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None			310p	None
	-	NOTIC -	-	None -	0	None -
Storage Length			-	0	0	
Veh in Median Storage,		-	-			-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	578	51	261	634	31	187
Major/Minor Major/Minor	ajor1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	629	0	1760	604
Stage 1	-	_	-	-	604	-
Stage 2	_	_	_	_	1156	_
Critical Hdwy			4.12		6.42	6.22
Critical Hdwy Stg 1	_	_	4.12	_	5.42	0.22
Critical Hdwy Stg 2	-	-	_		5.42	
		-	2.218	-		3.318
Follow-up Hdwy	-	-	953	-	93	
Pot Cap-1 Maneuver	-	-	953	-		498
Stage 1	-	-	-	-	546	-
Stage 2	-	-	-	-	300	-
Platoon blocked, %	-	-	050	-	- 1	100
Mov Cap-1 Maneuver	-	-	953	-	54	498
Mov Cap-2 Maneuver	-	-	-	-	54	-
Stage 1	-	-	-	-	546	-
Stage 2	-	-	-	-	173	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		3		92.5	
	U		3			
HCM LOS					F	
Minor Lane/Major Mvmt	1	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		229		_		
HCM Lane V/C Ratio		0.951	_		0.274	_
HCM Control Delay (s)		92.5	_	_		0
HCM Lane LOS		72.5 F	_	_	В	A
HCM 95th %tile Q(veh)		8.4	-	_	1.1	-
115W 75W 75W 75W Q(VCH)		0.7				

Intersection												
Int Delay, s/veh	2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		<u>ነ</u>	₽		7	f)	
Traffic Vol, veh/h	2	1	3	15	1	76	2	599	23	111	648	3
Future Vol, veh/h	2	1	3	15	1	76	2	599	23	111	648	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	125	-	-	150	-	-
Veh in Median Storage	e,# -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	2	1	3	17	1	84	2	666	26	123	720	3
Major/Minor	Minor2		ı	Minor1			Major1			Major2		
Conflicting Flow All	1694	1664	722	1653	1652	679	723	0	0	692	0	0
	968	968		683	683	0/9	123	U	U	092		U
Stage 1		696	-	970	969	-		-		-	-	-
Stage 2	726	6.52					112	-	-	4.12	-	-
Critical Hdwy Stg 1	7.12		6.22	7.12	6.52	6.22	4.12	-	-		-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	2 210	6.12	5.52	2 210	2 210	-	-	2 210	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	73	97	427	78	98	452	879	-	-	903	_	-
Stage 1	305	332	-	439	449	-	-	-	-	-	-	-
Stage 2	416	443	-	304	332	-	-	-	-	-	-	-
Platoon blocked, %		0.4	407		0.4	450	070	-	-	000	-	-
Mov Cap-1 Maneuver	53	84	427	69	84	452	879	-	-	903	-	-
Mov Cap-2 Maneuver	138	176	-	177	195	-	-	-	-	-	-	-
Stage 1	304	287	-	438	448	-	-	-	-	-	-	-
Stage 2	337	442	-	260	287	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	21.8			19.1			0			1.4		
HCM LOS	С			С								
Minor Lane/Major Mvm	nt	NBL	NBT	NRR	EBLn1\	WRI n1	SBL	SBT	SBR			
Capacity (veh/h)		879		-	221	357	903					
HCM Lane V/C Ratio		0.003	-	-		0.286		-	-			
HCM Control Delay (s)	\	9.1	-	-		19.1	9.6	-	-			
HCM Lane LOS		9.1 A			21.0 C	19.1 C	9.0 A	-				
	1		-	-		1.2			-			
HCM 95th %tile Q(veh)	0	-	-	0.1	1.2	0.5	-	-			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		^	7						77		4₽	
Traffic Volume (vph)	0	1166	388	0	0	0	0	0	1331	0	971	0
Future Volume (vph)	0	1166	388	0	0	0	0	0	1331	0	971	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		-2%			0%			1%			0%	
Storage Length (ft)	0		175	0		0	0		0	0		0
Storage Lanes	0		1	0		0	0		2	0		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.88	0.95	0.95	1.00
Frt			0.850						0.850			
Flt Protected												
Satd. Flow (prot)	0	3575	1599	0	0	0	0	0	2773	0	3539	0
Flt Permitted												
Satd. Flow (perm)	0	3575	1599	0	0	0	0	0	2773	0	3539	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)		55			55			45			35	
Link Distance (ft)		3066			489			978			454	
Travel Time (s)		38.0			6.1			14.8			8.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	1296	431	0	0	0	0	0	1479	0	1079	0
Shared Lane Traffic (%)	-											
Lane Group Flow (vph)	0	1296	431	0	0	0	0	0	1479	0	1079	0
Turn Type		NA	Perm						Perm		NA	
Protected Phases		2									8	
Permitted Phases			2						8	8		
Detector Phase		2	2						8	8	8	
Switch Phase												
Minimum Initial (s)		14.0	14.0						7.0	7.0	7.0	
Minimum Split (s)		20.8	20.8						13.2	13.2	13.2	
Total Split (s)		50.0	50.0						70.0	70.0	70.0	
Total Split (%)		41.7%	41.7%						58.3%	58.3%	58.3%	
Maximum Green (s)		43.2	43.2						63.8	63.8	63.8	
Yellow Time (s)		5.4	5.4						3.0	3.0	3.0	
All-Red Time (s)		1.4	1.4						3.2	3.2	3.2	
Lost Time Adjust (s)		-1.8	-1.8						-1.2	<u> </u>	-1.2	
Total Lost Time (s)		5.0	5.0						5.0		5.0	
Lead/Lag		0.0	0.0						0.0		0.0	
Lead-Lag Optimize?												
Vehicle Extension (s)		6.0	6.0						2.0	2.0	2.0	
Minimum Gap (s)		3.4	3.4						0.2	0.2	0.2	
Time Before Reduce (s)		15.0	15.0						0.0	0.0	0.0	
Time To Reduce (s)		45.0	45.0						0.0	0.0	0.0	
Recall Mode		C-Min	C-Min						None	None	None	
Act Effct Green (s)		45.0	45.0						65.0	140110	65.0	
Actuated g/C Ratio		0.38	0.38						0.54		0.54	
v/c Ratio		0.97	0.72						0.98		0.56	
Control Delay		55.0	40.2						47.4		19.6	
Queue Delay		0.0	0.0						0.0		0.0	
Zucuc Delay		0.0	0.0						0.0		0.0	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay		55.0	40.2						47.4		19.6	
LOS		Ε	D						D		В	
Approach Delay		51.3						47.4			19.6	
Approach LOS		D						D			В	
Queue Length 50th (ft)		512	283						613		278	
Queue Length 95th (ft)		#668	408						#817		341	
Internal Link Dist (ft)		2986			409			898			374	
Turn Bay Length (ft)			175									
Base Capacity (vph)		1340	599						1502		1916	
Starvation Cap Reductn		0	0						0		0	
Spillback Cap Reductn		0	0						0		0	
Storage Cap Reductn		0	0						0		0	
Reduced v/c Ratio		0.97	0.72						0.98		0.56	
Intersection Summary												
<i>3</i> I	ther											
Cycle Length: 120												
Actuated Cycle Length: 120												
Offset: 0 (0%), Referenced to	phase 2:E	BT, Star	t of Gree	n								
Natural Cycle: 100												
Control Type: Actuated-Coord	linated											
Maximum v/c Ratio: 0.98												
Intersection Signal Delay: 42.					itersection							
Intersection Capacity Utilization	on 129.6%			IC	CU Level o	of Service	Н					
Analysis Period (min) 15												
# 95th percentile volume ex			eue may	be longer	r.							
Queue shown is maximum	after two	cycles.										
Splits and Phases: 4: Richa	ardson Rd	& US 64	EB									
▼ Ø2 (R)												
50 s				ı								
				l I								
				▼ Ø8	3							

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations				^	ሻሻ	
Traffic Volume (vph)	0	0	0	2166	472	0
Future Volume (vph)	0	0	0	2166	472	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	0.97	1.00
Frt	1.00	1.00	1.00	0.75	0.77	1.00
Flt Protected					0.950	
	0	0	Λ	2520	3433	0
Satd. Flow (prot)	U	U	0	3539		U
Flt Permitted	٥	0	0	2520	0.950	0
Satd. Flow (perm)	0	0	0	3539	3433	0
Right Turn on Red		No			No	No
Satd. Flow (RTOR)						
Link Speed (mph)	55			55	25	
Link Distance (ft)	459			2512	426	
Travel Time (s)	5.7			31.1	11.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	0	0	2407	524	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2407	524	0
Turn Type				NA	Prot	
Protected Phases				6	8	
Permitted Phases						
Detector Phase				6	8	
Switch Phase				U	U	
Minimum Initial (s)				14.0	7.0	
Minimum Split (s)				20.2	13.3	
Total Split (s)				70.0	20.0	
Total Split (%)				77.8%	22.2%	
Maximum Green (s)				63.8	13.7	
Yellow Time (s)				5.2	3.0	
All-Red Time (s)				1.0	3.3	
Lost Time Adjust (s)				-1.2	-1.3	
Total Lost Time (s)				5.0	5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)				6.0	2.0	
Minimum Gap (s)				3.4	0.2	
Time Before Reduce (s)				15.0	0.0	
Time To Reduce (s)				45.0	0.0	
Recall Mode				C-Min	None	
Act Effct Green (s)				65.0	15.0	
Actuated g/C Ratio				0.72	0.17	
v/c Ratio				0.72	0.17	
				20.5	59.9	
Control Delay						
Queue Delay				0.0	0.0	
Total Delay				20.5	59.9	
LOS				С	Е	
Approach Delay				20.5	59.9	
Approach LOS				С	Е	

	-	\rightarrow	•	←	4	<i>></i>		
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR		
Queue Length 50th (ft)				523	152			
Queue Length 95th (ft)				#847	#246			
Internal Link Dist (ft)	379			2432	346			
Turn Bay Length (ft)								
Base Capacity (vph)				2555	572			
Starvation Cap Reductn				0	0			
Spillback Cap Reductn				0	0			
Storage Cap Reductn				0	0			
Reduced v/c Ratio				0.94	0.92			
Intersection Summary								
Area Type: (Other							
Cycle Length: 90								
Actuated Cycle Length: 90								
Offset: 0 (0%), Referenced to	o phase 6:\	NBT, Sta	rt of Gree	en				
Natural Cycle: 90								
Control Type: Actuated-Cool	rdinated							
Maximum v/c Ratio: 0.94								
Intersection Signal Delay: 27					tersection			
Intersection Capacity Utilizat	ion 81.7%			IC	CU Level o	f Service D		
Analysis Period (min) 15								
# 95th percentile volume e		<i>J</i> 1	eue may	be longer	r.			
Queue shown is maximu	m after two	cycles.						
o			_					
Splits and Phases: 5: U-T	urn East &	US 64 W	В					
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Ø6 (R)							 √ Ø8	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	1>		*	†	7	ሻ	f _è		ች	+	7
Traffic Volume (vph)	54	70	22	101	70	120	37	110	131	93	102	48
Future Volume (vph)	54	70	22	101	70	120	37	110	131	93	102	48
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250	.,	0	150	1,00	150	100	.,,,,	0	150	.,,,	175
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (ft)	100			100		•	100			100		•
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1100	0.965		1100	1100	0.850		0.918	1100	1100		0.850
Flt Protected	0.950	0.700		0.950		0.000	0.950	0.7.0		0.950		0.000
Satd. Flow (prot)	1770	1798	0	1770	1863	1583	1770	1710	0	1770	1863	1583
Flt Permitted	0.706	1770		0.691	1000	1000	0.684	1710		0.594	1000	1000
Satd. Flow (perm)	1315	1798	0	1287	1863	1583	1274	1710	0	1106	1863	1583
Right Turn on Red	1010	1770	No	1207	1000	No	1271	1710	No	1100	1000	No
Satd. Flow (RTOR)			110			110			NO			140
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1889			1311			1771			2925	
Travel Time (s)		28.6			19.9			26.8			44.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	60	78	24	112	78	133	41	122	146	103	113	53
Shared Lane Traffic (%)	00	70	24	112	70	133	41	122	140	103	113	ეა
` ,	60	102	0	112	78	133	41	268	0	103	113	53
Lane Group Flow (vph)	Perm	NA	U	Perm	NA	Perm	Perm	NA	0	Perm	NA	Perm
Turn Type Protected Phases	Pellii	2		Pellii	NA 6	Pelili	Pellii	4		Pellii	NA 8	Pellii
Permitted Phases	2	Z		6	Ü	6	4	4		8	0	8
Detector Phase	2	2		6	6	6	4	4		8	8	8
Switch Phase	2	Z		Ü	Ü	0	4	4		0	0	0
Minimum Initial (s)	7.0	7.0		7.0	7.0	7.0	7.0	7.0		7.0	7.0	7.0
	14.0	14.0		14.0	14.0	14.0	14.0	14.0		14.0	14.0	14.0
Minimum Split (s)	26.0	26.0		26.0	26.0	26.0	34.0	34.0		34.0	34.0	34.0
Total Split (s) Total Split (%)	43.3%	43.3%		43.3%	43.3%	43.3%	56.7%	56.7%		56.7%	56.7%	56.7%
	19.0	19.0		19.0	19.0	19.0	27.0	27.0		27.0	27.0	27.0
Maximum Green (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Yellow Time (s)	2.0	2.0		2.0	2.0	2.0	2.0			2.0	2.0	5.0
All-Red Time (s)								2.0				2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Lead/Lag												
Lead-Lag Optimize?	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	Min	Min		Min	Min	Min	None	None		None	None	None
Act Effct Green (s)	12.2	12.2		12.2	12.2	12.2	12.8	12.8		12.8	12.8	12.8
Actuated g/C Ratio	0.35	0.35		0.35	0.35	0.35	0.36	0.36		0.36	0.36	0.36
v/c Ratio	0.13	0.16		0.25	0.12	0.24	0.09	0.43		0.26	0.17	0.09
Control Delay	9.9	9.8		11.1	9.5	10.6	7.8	10.7		9.5	8.0	7.6
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	9.9	9.8		11.1	9.5	10.6	7.8	10.7		9.5	8.0	7.6
LOS	А	А		В	A	В	А	В		А	A	Α
Approach Delay		9.8			10.5			10.3			8.5	
Approach LOS		А			В			В			Α	

1: Richardson Rd & Olive Chapel Rd

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EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
7	12		14	9	17	4	31		11	12	5
28	40		46	32	52	18	85		39	38	22
	1809			1231			1691			2845	
250			150		150	100			150		175
796	1088		779	1128	958	1065	1430		924	1558	1323
0	0		0	0	0	0	0		0	0	0
0	0		0	0	0	0	0		0	0	0
0	0		0	0	0	0	0		0	0	0
0.08	0.09		0.14	0.07	0.14	0.04	0.19		0.11	0.07	0.04
	7 28 250 796 0 0	7 12 28 40 1809 250 796 1088 0 0 0 0	7 12 28 40 1809 250 796 1088 0 0 0 0 0 0	7 12 14 28 40 46 1809 250 150 796 1088 779 0 0 0 0 0 0 0	7 12 14 9 28 40 46 32 1809 1231 250 150 796 1088 779 1128 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7 12 14 9 17 28 40 46 32 52 1809 1231 250 150 150 796 1088 779 1128 958 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7 12 14 9 17 4 28 40 46 32 52 18 1809 1231 150 150 100 796 1088 779 1128 958 1065 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7 12 14 9 17 4 31 28 40 46 32 52 18 85 1809 1231 1691 250 150 150 100 796 1088 779 1128 958 1065 1430 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7 12 14 9 17 4 31 28 40 46 32 52 18 85 1809 1231 1691 250 150 150 100 796 1088 779 1128 958 1065 1430 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7 12 14 9 17 4 31 11 28 40 46 32 52 18 85 39 1809 1231 1691 150 <td>7 12 14 9 17 4 31 11 12 28 40 46 32 52 18 85 39 38 1809 1231 1691 2845 250 150 150 100 150 796 1088 779 1128 958 1065 1430 924 1558 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0</td>	7 12 14 9 17 4 31 11 12 28 40 46 32 52 18 85 39 38 1809 1231 1691 2845 250 150 150 100 150 796 1088 779 1128 958 1065 1430 924 1558 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

Intersection Summary

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 35.2

Natural Cycle: 40

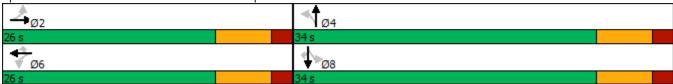
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.43

Intersection Signal Delay: 9.8 Intersection LOS: A Intersection Capacity Utilization 44.4% ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 1: Richardson Rd & Olive Chapel Rd



Intersection						
Int Delay, s/veh	2.9					
		EDD.	MDL	MOT	ND	NDD
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	f)	00	00	4	Y	07
Traffic Vol, veh/h	446	22	83	336	28	97
Future Vol, veh/h	446	22	83	336	28	97
Conflicting Peds, #/hr	0	0	0	0	0	0
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	496	24	92	373	31	108
Major/Minor M	ajor1	N	Major2		Minor1	
Conflicting Flow All	0	0	520	0	1065	508
			320		508	
Stage 1	-	-	_	-	557	-
Stage 2		-	110			
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	2 210	-	5.42	2 210
Follow-up Hdwy	-		2.218	-		3.318
Pot Cap-1 Maneuver	-	-	1046	-	246	565
Stage 1	-	-	-	-	604	-
Stage 2	-	-	-	-	574	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1046	-	219	565
Mov Cap-2 Maneuver	-	-	-	-	219	-
Stage 1	-	-	-	-	604	-
Stage 2	-	-	-	-	510	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		1.7		17.9	
HCM LOS					С	
Minor Lane/Major Mvmt	ľ	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		417	_		1046	_
HCM Lane V/C Ratio		0.333	_		0.088	_
HCM Control Delay (s)		17.9	-	-	8.8	0
HCM Lane LOS		C	_	_	A	A
HCM 95th %tile Q(veh)		1.4	_	_	0.3	-
110W 75W 76W Q(VCH)		1.7			0.5	

Intersection												
Int Delay, s/veh	3											
		CDT	EDD	WDI	WDT	WDD	NDI	NDT	NDD	CDI	CDT	CDD
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		1	ĵ⇒		ሻ	Þ	
Traffic Vol, veh/h	1	0	1	15	0	167	2	478	10	67	503	1
Future Vol, veh/h	1	0	1	15	0	167	2	478	10	67	503	1
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	125	-	-	150	-	-
Veh in Median Storage	e,# -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	0	1	17	0	186	2	531	11	74	559	1
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	1342	1254	560	1249	1249	537	560	0	0	542	0	0
Stage 1	708	708	-	541	541	-	-	-	-	-	-	-
Stage 2	634	546	_	708	708	_	_	_	_	_	_	_
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	_	_	4.12	_	_
Critical Hdwy Stg 1	6.12	5.52	- 0.22	6.12	5.52	0.22	7.12	_	_	7.12	_	_
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52	_	_	_		_	_	_
Follow-up Hdwy	3.518	4.018		3.518	4.018	3.318	2 212	_	_	2.218	_	_
Pot Cap-1 Maneuver	129	172	528	150	173	544	1011	_		1027	_	
Stage 1	426	438	520	525	521	344	1011	-	-	1027	_	_
Stage 2	467	518		426	438	-	-	-	-	-	-	-
Platoon blocked, %	407	310	-	420	430	-		-	-	-	-	-
Mov Cap-1 Maneuver	80	159	528	141	160	544	1011	-	-	1027	_	-
Mov Cap-1 Maneuver	170	266	520	267	279	344	1011	-	-	1027	-	-
Stage 1	425	406	-	524	520	-	-	-	-	<u>-</u>	-	-
Stage 2	307	517	_	394	406						_	
Siayt 2	307	JII	-	J74	400	-	_	-	_	-	_	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	19.1			17			0			1		
HCM LOS	С			С								
Minor Lane/Major Mvn	nt	NBL	NBT	NBR	EBLn1\	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1011	_	_	257	501	1027	_	_			
HCM Lane V/C Ratio		0.002	_	_		0.404		_	_			
HCM Control Delay (s))	8.6	_		19.1	17	8.8	-				
HCM Lane LOS		A	_	_	C	C	Α	_	-			
HCM 95th %tile Q(veh)	0	_	_	0	1.9	0.2	-	_			
1.5W 7001 70010 Q(VCI)	7	U			U	1.7	0.2					

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		^	7						77		414	
Traffic Volume (vph)	0	1124	270	0	0	0	0	0	979	0	735	0
Future Volume (vph)	0	1124	270	0	0	0	0	0	979	0	735	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		-2%			0%			1%			0%	
Storage Length (ft)	0		175	0		0	0		0	0		0
Storage Lanes	0		1	0		0	0		2	0		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.88	0.95	0.95	1.00
Frt			0.850						0.850			
Flt Protected												
Satd. Flow (prot)	0	3575	1599	0	0	0	0	0	2773	0	3539	0
Flt Permitted												
Satd. Flow (perm)	0	3575	1599	0	0	0	0	0	2773	0	3539	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)		55			55			45			35	
Link Distance (ft)		3066			489			978			454	
Travel Time (s)		38.0			6.1			14.8			8.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	1249	300	0	0	0	0	0	1088	0	817	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1249	300	0	0	0	0	0	1088	0	817	0
Turn Type		NA	Perm						Perm		NA	
Protected Phases		2									8	
Permitted Phases			2						8	8		
Detector Phase		2	2						8	8	8	
Switch Phase												
Minimum Initial (s)		14.0	14.0						7.0	7.0	7.0	
Minimum Split (s)		20.8	20.8						13.2	13.2	13.2	
Total Split (s)		29.0	29.0						31.0	31.0	31.0	
Total Split (%)		48.3%	48.3%						51.7%	51.7%	51.7%	
Maximum Green (s)		22.2	22.2						24.8	24.8	24.8	
Yellow Time (s)		5.4	5.4						3.0	3.0	3.0	
All-Red Time (s)		1.4	1.4						3.2	3.2	3.2	
Lost Time Adjust (s)		-1.8	-1.8						-1.3	<u> </u>	-1.2	
Total Lost Time (s)		5.0	5.0						4.9		5.0	
Lead/Lag		0.0	0.0								0.0	
Lead-Lag Optimize?												
Vehicle Extension (s)		6.0	6.0						2.0	2.0	2.0	
Minimum Gap (s)		3.4	3.4						0.2	0.2	0.2	
Time Before Reduce (s)		15.0	15.0						0.0	0.0	0.0	
Time To Reduce (s)		45.0	45.0						0.0	0.0	0.0	
Recall Mode		C-Min	C-Min						None	None	None	
Act Effct Green (s)		24.0	24.0						26.1	110110	26.0	
Actuated g/C Ratio		0.40	0.40						0.44		0.43	
v/c Ratio		0.40	0.47						0.90		0.53	
Control Delay		25.5	16.3						28.5		9.8	
Queue Delay		0.0	0.0						0.0		0.0	
= Cuodo Dolay		0.0	0.0						0.0		0.0	

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Lane Group	EBL EB	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	25.	16.3						28.5		9.8	
LOS	(В						С		Α	
Approach Delay	23.						28.5			9.8	
Approach LOS	(С			Α	
Queue Length 50th (ft)	21							197		99	
Queue Length 95th (ft)	#33							#333		m110	
Internal Link Dist (ft)	298			409			898			374	
Turn Bay Length (ft)		175									
Base Capacity (vph)	1430	639						1206		1533	
Starvation Cap Reductn		0						0		0	
Spillback Cap Reductn		0						0		0	
Storage Cap Reductn		0						0		0	
Reduced v/c Ratio	0.8	0.47						0.90		0.53	
Intersection Summary											
<i>J</i> I	ther										
Cycle Length: 60											
Actuated Cycle Length: 60											
Offset: 0 (0%), Referenced to	phase 2:EBT, S	tart of Gre	en								
Natural Cycle: 60											
Control Type: Actuated-Coord	linated										
Maximum v/c Ratio: 0.90											
Intersection Signal Delay: 22.				ntersectio							
Intersection Capacity Utilization	on 103.0%			CU Level	of Service	e G					
Analysis Period (min) 15											
<pre># 95th percentile volume ex</pre>			y be longe	er.							
Queue shown is maximum											
m Volume for 95th percentil	e queue is mete	red by ups	stream sig	nal.							
Splits and Phases: 4: Richa	ardson Rd & US	61 FR									
Spills and Friases. 4. Kiene	ilusoii iku a US	04 LD		1							
● Ø2 (R)				╛							
29 s											
				1 08							

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Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	LUI	LUIT	VVDL	<u> </u>	T T	NUIL
Traffic Volume (vph)	0	0	0	1786	258	0
Future Volume (vph)	0	0	0	1786	258	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	0.97	1.00
Frt	1.00	1.00	1.00	0.75	0.71	1.00
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	3539	3433	0
Flt Permitted		- 0		0007	0.950	0
Satd. Flow (perm)	0	0	0	3539	3433	0
Right Turn on Red		No		3337	No	No
Satd. Flow (RTOR)		INU			INU	INU
Link Speed (mph)	55			55	25	
	459			2512	426	
Link Distance (ft)						
Travel Time (s)	5.7	0.00	0.00	31.1	11.6	0.00
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	0	0	1984	287	0
Shared Lane Traffic (%)	_	_	_	1004	007	_
Lane Group Flow (vph)	0	0	0	1984	287	0
Turn Type				NA	Prot	
Protected Phases				6	8	
Permitted Phases					_	
Detector Phase				6	8	
Switch Phase						
Minimum Initial (s)				14.0	7.0	
Minimum Split (s)				21.2	14.0	
Total Split (s)				46.0	14.0	
Total Split (%)				76.7%	23.3%	
Maximum Green (s)				39.8	7.7	
Yellow Time (s)				5.2	3.0	
All-Red Time (s)				1.0	3.3	
Lost Time Adjust (s)				-1.2	-1.3	
Total Lost Time (s)				5.0	5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)				6.0	2.0	
Minimum Gap (s)				3.4	0.2	
Time Before Reduce (s)				15.0	0.0	
Time To Reduce (s)				45.0	0.0	
Recall Mode				C-Min	None	
Act Effct Green (s)				41.1	8.9	
Actuated g/C Ratio				0.68	0.15	
v/c Ratio				0.82	0.13	
Control Delay				10.5	26.5	
Queue Delay				0.0	0.0	
Total Delay				10.5	26.5	
LOS				10.5 B	20.5 C	
Approach LOS				10.5	26.5	
Approach LOS				В	С	

	→	•	•	←	4	<i>></i>		
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR		
Queue Length 50th (ft)				215	51			
Queue Length 95th (ft)				312	m59			
Internal Link Dist (ft)	379			2432	346			
Turn Bay Length (ft)								
Base Capacity (vph)				2426	514			
Starvation Cap Reductn				0	0			
Spillback Cap Reductn				0	0			
Storage Cap Reductn				0	0			
Reduced v/c Ratio				0.82	0.56			
Intersection Summary								
Area Type: Of	ther							
Cycle Length: 60								
Actuated Cycle Length: 60								
Offset: 0 (0%), Referenced to	phase 6:\	WBT, Sta	rt of Gree	en				
Natural Cycle: 60								
Control Type: Actuated-Coord	linated							
Maximum v/c Ratio: 0.82								
Intersection Signal Delay: 12.					tersection			
Intersection Capacity Utilization	on 65.1%			IC	CU Level o	f Service C		
Analysis Period (min) 15								
m Volume for 95th percentile	e queue is	s metered	d by upstr	ream sign	al.			
Cality and Dhases. Full Tu	rn East &	LIC 44 W	'D					
Splits and Phases: 5: U-Tui	III East &	US 64 W	В					
←							▲	
Ø6 (R)						- I	\ Ø8	

Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
				WBR		SBK
Lane Configurations Traffic Vol, veh/h	ጟ	120	255		\	10
Future Vol, veh/h	5 5	439 439	355 355	9	29 29	13 13
	0	439	333	0	0	0
Conflicting Peds, #/hr Sign Control	Free	Free	Free	Free	Stop	
RT Channelized	riee -	None		None	Stop	Stop None
	100	None -	-	100		None
Storage Length Veh in Median Storage					0	-
	:,# -	0	0	-	0	-
Grade, %	-	0		-	0	
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	6	488	394	10	32	14
Major/Minor N	Major1	N	Major2	1	Minor2	
Conflicting Flow All	404	0	-	0	894	394
Stage 1	-	-	-	-	394	-
Stage 2	-	-	-	-	500	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1155	-	-	-	312	655
Stage 1	-	-	-	-	681	-
Stage 2	-	-	-	-	609	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1155	-	_	-	310	655
Mov Cap-2 Maneuver	_	_	-	-	310	-
Stage 1	-	_	-	-	678	_
Stage 2	_	_	_	_	609	_
Olago 2					007	
	- F-D		\4/D		0.0	
Approach	EB		WB		SB	
HCM Control Delay, s	0.1		0		16.1	
HCM LOS					С	
Minor Lane/Major Mvm	ıt	EBL	EBT	WBT	WBR:	SBI n1
Capacity (veh/h)		1155				370
HCM Lane V/C Ratio		0.005	_	_	_	0.126
HCM Control Delay (s)		8.1		_	-	16.1
HCM Lane LOS		Α	_	_	_	C
HCM 95th %tile Q(veh)		0				0.4
How four four Q(Veri)		U				0.4

	•	→	•	•	+	•	•	†	~	/		-√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	f)		*	†	7	ሻ	f)		ች	†	7
Traffic Volume (vph)	58	125	41	172	107	139	31	103	174	139	147	55
Future Volume (vph)	58	125	41	172	107	139	31	103	174	139	147	55
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	250	.,,,,	0	150	.,,,	150	100	.,,,,	0	150	.,,,	175
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (ft)	100			100		•	100			100		•
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.963	1100			0.850		0.906	1100	.,,,		0.850
Flt Protected	0.950	0.700		0.950		0.000	0.950	0.700		0.950		0.000
Satd. Flow (prot)	1770	1794	0	1770	1863	1583	1770	1688	0	1770	1863	1583
Flt Permitted	0.681	1771		0.641	1000	1000	0.654	1000		0.573	1000	1000
Satd. Flow (perm)	1269	1794	0	1194	1863	1583	1218	1688	0	1067	1863	1583
Right Turn on Red	1207	1771	No	1171	1000	No	1210	1000	No	1007	1000	No
Satd. Flow (RTOR)			NO			110			140			NO
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1889			1311			1771			2925	
Travel Time (s)		28.6			19.9			26.8			44.3	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	64	139	46	191	119	154	34	114	193	154	163	61
Shared Lane Traffic (%)	04	137	40	171	117	134	34	114	175	134	103	UI
Lane Group Flow (vph)	64	185	0	191	119	154	34	307	0	154	163	61
Turn Type	Perm	NA	U	Perm	NA	Perm	Perm	NA	U	Perm	NA	Perm
Protected Phases	1 Cilli	2		1 Cilli	6	1 Cilli	1 Cilli	4		1 Cilli	8	T CITI
Permitted Phases	2	2		6	U	6	4	7		8	U	8
Detector Phase	2	2		6	6	6	4	4		8	8	8
Switch Phase	_	_		J		J	•	•		J		J
Minimum Initial (s)	7.0	7.0		7.0	7.0	7.0	7.0	7.0		7.0	7.0	7.0
Minimum Split (s)	14.0	14.0		14.0	14.0	14.0	14.0	14.0		14.0	14.0	14.0
Total Split (s)	29.0	29.0		29.0	29.0	29.0	31.0	31.0		31.0	31.0	31.0
Total Split (%)	48.3%	48.3%		48.3%	48.3%	48.3%	51.7%	51.7%		51.7%	51.7%	51.7%
Maximum Green (s)	22.0	22.0		22.0	22.0	22.0	24.0	24.0		24.0	24.0	24.0
Yellow Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	Min	Min		Min	Min	Min	None	None		None	None	None
Act Effct Green (s)	14.5	14.5		14.5	14.5	14.5	15.0	15.0		15.0	15.0	15.0
Actuated g/C Ratio	0.36	0.36		0.36	0.36	0.36	0.37	0.37		0.37	0.37	0.37
v/c Ratio	0.14	0.29		0.44	0.18	0.27	0.07	0.49		0.39	0.23	0.10
Control Delay	10.3	11.1		14.2	10.2	11.2	9.8	13.4		13.6	10.5	9.7
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	10.3	11.1		14.2	10.2	11.2	9.8	13.4		13.6	10.5	9.7
LOS	В	В		В	В	В	Α.	В		В	В	A
Approach Delay		10.9			12.2		,,	13.0			11.6	, ,
Approach LOS		В			В			В			В	
Marra 200												

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Queue Length 50th (ft)	9	26		29	16	22	4	45		22	21	8
Queue Length 95th (ft)	33	77		89	52	67	21	131		75	69	31
Internal Link Dist (ft)		1809			1231			1691			2845	
Turn Bay Length (ft)	250			150		150	100			150		175
Base Capacity (vph)	804	1137		757	1181	1004	836	1159		733	1280	1088
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.08	0.16		0.25	0.10	0.15	0.04	0.26		0.21	0.13	0.06

Intersection Summary

Area Type: Other

Cycle Length: 60

Actuated Cycle Length: 40.2

Natural Cycle: 40

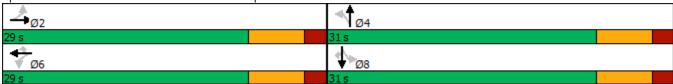
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.49

Intersection Signal Delay: 12.0 Intersection LOS: B
Intersection Capacity Utilization 59.1% ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: Richardson Rd & Olive Chapel Rd



Intersection Int Delay, s/veh						
ini Deiay, s/ven	10					
	18					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	(ર્ન	W	
Traffic Vol, veh/h	537	47	235	599	30	168
Future Vol, veh/h	537	47	235	599	30	168
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	_	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	597	52	261	666	33	187
WWW. LOW	371	52	201	000	33	107
Major/Minor M	1ajor1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	649	0	1811	623
Stage 1	-	-	-	-	623	-
Stage 2	-	-	-	-	1188	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	_	-	_	5.42	-
Follow-up Hdwy	-	_	2.218	_	3.518	3.318
Pot Cap-1 Maneuver	_	_	937	_	86	486
Stage 1	_	_	-	_	535	-
Stage 2	_	_		_	289	_
Platoon blocked, %	_	_		_	207	
i latoon blockcu, 70			007			
Mov Can-1 Manquivor			Q Z I		10	126
Mov Cap-1 Maneuver	-	-	937	-	48	486
Mov Cap-2 Maneuver	-	-	937	-	48	-
Mov Cap-2 Maneuver Stage 1	-	-	-	-	48 535	-
Mov Cap-2 Maneuver	- - -	- - -			48	-
Mov Cap-2 Maneuver Stage 1	-	-	-	-	48 535	-
Mov Cap-2 Maneuver Stage 1	-	-	-	-	48 535	-
Mov Cap-2 Maneuver Stage 1 Stage 2 Approach	- - EB	-	- - - WB	-	48 535 161 NB	-
Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s	-	-	- - -	-	48 535 161 NB 134.5	-
Mov Cap-2 Maneuver Stage 1 Stage 2 Approach	- - EB	-	- - - WB	-	48 535 161 NB	-
Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS	EB 0	-	WB 2.9		48 535 161 NB 134.5	-
Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt	EB 0	- - - NBLn1	- - - WB	-	48 535 161 NB 134.5 F	-
Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h)	EB 0	- - - NBLn1 204	WB 2.9	EBR	48 535 161 NB 134.5 F WBL	-
Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	EB 0	NBLn1 204 1.078	- - - WB 2.9	EBR	48 535 161 NB 134.5 F WBL 937 0.279	WBT
Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	EB 0	- - - NBLn1 204	- - - WB 2.9	EBR	48 535 161 NB 134.5 F WBL	WBT
Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio	EB 0	NBLn1 204 1.078	- - - WB 2.9 EBT	- - - EBR	48 535 161 NB 134.5 F WBL 937 0.279	WBT

Intersection												
Intersection Int Delay, s/veh	3.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4		- ሻ	₽			- ₽	
Traffic Vol, veh/h	2	1	3	15	1	116	2	599	23	178	648	3
Future Vol, veh/h	2	1	3	15	1	116	2	599	23	178	648	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	125	-	-	150	-	-
Veh in Median Storage	≘,# -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	2	1	3	17	1	129	2	666	26	198	720	3
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	1866	1814	722	1803	1802	679	723	0	0	692	0	0
Stage 1	1118	1118	-	683	683	-		-	-	-	-	-
Stage 2	748	696	_	1120	1119	_	-	_	_	_	_	_
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-		_	-	-	_	_
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52	_	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	_	2.218	-	-
Pot Cap-1 Maneuver	56	78	427	62	80	452	879	-	-	903	-	-
Stage 1	251	282	-	439	449	-	-	-	-	-	-	-
Stage 2	404	443	-	251	282	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	33	61	427	51	62	452	879	-	-	903	-	-
Mov Cap-2 Maneuver	71	133	-	140	158	-	-	-	-	-	-	-
Stage 1	250	220	-	438	448	-	-	-	-	-	-	-
Stage 2	287	442	-	193	220	-	-	-	-	-	-	-
January 1												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	32			21.9			0			2.2		
HCM LOS	52 D			21.9 C			U			2.2		
TIGIVI EUS	U			U								
Minor Lane/Major Mvn	nt	NBL	NBT	NBR	EBLn1V		SBL	SBT	SBR			
Capacity (veh/h)		879	-	-	140	357	903	-	-			
HCM Lane V/C Ratio		0.003	-	-	0.048		0.219	-	-			
HCM Control Delay (s)		9.1	-	-	32	21.9	10.1	-	-			
HCM Lane LOS		Α	-	-	D	С	В	-	-			
HCM 95th %tile Q(veh)	0	-	-	0.1	1.9	0.8	-	-			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		† †	7						77		4₽	
Traffic Volume (vph)	0	1166	399	0	0	0	0	0	1371	0	1027	0
Future Volume (vph)	0	1166	399	0	0	0	0	0	1371	0	1027	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		-2%			0%			1%			0%	
Storage Length (ft)	0		175	0		0	0		0	0		0
Storage Lanes	0		1	0		0	0		2	0		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.88	0.95	0.95	1.00
Frt			0.850						0.850			
Flt Protected												
Satd. Flow (prot)	0	3575	1599	0	0	0	0	0	2773	0	3539	0
Flt Permitted												
Satd. Flow (perm)	0	3575	1599	0	0	0	0	0	2773	0	3539	0
Right Turn on Red			No			No			No	No		No
Satd. Flow (RTOR)												
Link Speed (mph)		55			55			45			35	
Link Distance (ft)		3066			489			978			454	
Travel Time (s)		38.0			6.1			14.8			8.8	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	1296	443	0	0	0	0	0	1523	0	1141	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	1296	443	0	0	0	0	0	1523	0	1141	0
Turn Type		NA	Perm						Perm		NA	
Protected Phases		2									8	
Permitted Phases			2						8	8		
Detector Phase		2	2						8	8	8	
Switch Phase												
Minimum Initial (s)		14.0	14.0						7.0	7.0	7.0	
Minimum Split (s)		20.8	20.8						13.2	13.2	13.2	
Total Split (s)		49.0	49.0						71.0	71.0	71.0	
Total Split (%)		40.8%	40.8%						59.2%	59.2%	59.2%	
Maximum Green (s)		42.2	42.2						64.8	64.8	64.8	
Yellow Time (s)		5.4	5.4						3.0	3.0	3.0	
All-Red Time (s)		1.4	1.4						3.2	3.2	3.2	
Lost Time Adjust (s)		-1.8	-1.8						-1.2	<u> </u>	-1.2	
Total Lost Time (s)		5.0	5.0						5.0		5.0	
Lead/Lag		0.0	0.0						0.0		0.0	
Lead-Lag Optimize?												
Vehicle Extension (s)		6.0	6.0						2.0	2.0	2.0	
Minimum Gap (s)		3.4	3.4						0.2	0.2	0.2	
Time Before Reduce (s)		15.0	15.0						0.0	0.0	0.0	
Time To Reduce (s)		45.0	45.0						0.0	0.0	0.0	
Recall Mode		C-Min	C-Min						None	None	None	
Act Effct Green (s)		44.0	44.0						66.0	140110	66.0	
Actuated g/C Ratio		0.37	0.37						0.55		0.55	
v/c Ratio		0.99	0.76						1.00		0.59	
Control Delay		60.5	43.0						50.1		19.5	
Queue Delay		0.0	0.0						0.0		0.0	
Zucuc Delay		0.0	0.0						0.0		0.0	

Lane Group Fotal Delay LOS Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) Internal Link Dist (ft) Furn Bay Length (ft)	EBL EBT 60.5 E 56.0 E 520 #681 2986	EBR 43.0 D	WBL	WBT	WBR	NBL	NBT 50.1	NBR 50.1 D	SBL	SBT 19.5 B	SBF
LOS Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) nternal Link Dist (ft)	E 56.0 E 520 #681	D 298								В	
Approach Delay Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) nternal Link Dist (ft)	56.0 E 520 #681	298						D			
Approach LOS Queue Length 50th (ft) Queue Length 95th (ft) nternal Link Dist (ft)	E 520 #681										
Queue Length 50th (ft) Queue Length 95th (ft) nternal Link Dist (ft)	520 #681									19.5	
Queue Length 95th (ft) nternal Link Dist (ft)	#681						D			В	
nternal Link Dist (ft)		430						640		295	
	2986							#847		361	
Furn Bay Length (ft)				409			898			374	
		175									
Base Capacity (vph)	1310	586						1525		1946	
Starvation Cap Reductn	0	0						0		0	
Spillback Cap Reductn	0	0						0		0	
Storage Cap Reductn	0	0						0		0	
Reduced v/c Ratio	0.99	0.76						1.00		0.59	
ntersection Summary											
Area Type: Ot	her										
Cycle Length: 120											
Actuated Cycle Length: 120											
Offset: 0 (0%), Referenced to	phase 2:EBT, Star	t of Gree	n								
Natural Cycle: 100											
Control Type: Actuated-Coord	inated										
Maximum v/c Ratio: 1.00											
ntersection Signal Delay: 44.5				tersectior							
ntersection Capacity Utilizatio	n 134.1%		IC	:U Level o	of Service	Н					
Analysis Period (min) 15											
# 95th percentile volume exc		eue may	be longer	.							
Queue shown is maximum	after two cycles.										
Splits and Phases: 4: Richa	rdson Rd & US 64	EB									
1 🐨 Ø2 (R) 49 s											
12.3			k.								

	-	•	•	←	1	~
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations				^	ሻሻ	
Traffic Volume (vph)	0	0	0	2222	479	0
Future Volume (vph)	0	0	0	2222	479	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	0.95	0.97	1.00
Frt	1.00	1.00	1.00	0.75	0.77	1.00
Flt Protected					0.950	
Satd. Flow (prot)	0	0	0	3539	3433	0
Flt Permitted	U	U	U	3337	0.950	U
Satd. Flow (perm)	0	0	0	3539	3433	0
	U		U	3339		
Right Turn on Red		No			No	No
Satd. Flow (RTOR)	FF				25	
Link Speed (mph)	55			55	25	
Link Distance (ft)	459			2512	426	
Travel Time (s)	5.7			31.1	11.6	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	0	0	0	2469	532	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	0	0	2469	532	0
Turn Type				NA	Prot	
Protected Phases				6	8	
Permitted Phases						
Detector Phase				6	8	
Switch Phase						
Minimum Initial (s)				14.0	7.0	
Minimum Split (s)				20.2	13.3	
Total Split (s)				70.0	20.0	
Total Split (%)				77.8%	22.2%	
Maximum Green (s)				63.8	13.7	
				5.2	3.0	
Yellow Time (s)						
All-Red Time (s)				1.0	3.3	
Lost Time Adjust (s)				-1.2	-1.3	
Total Lost Time (s)				5.0	5.0	
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)				6.0	2.0	
Minimum Gap (s)				3.4	0.2	
Time Before Reduce (s)				15.0	0.0	
Time To Reduce (s)				45.0	0.0	
Recall Mode				C-Min	None	
Act Effct Green (s)				65.0	15.0	
Actuated g/C Ratio				0.72	0.17	
v/c Ratio				0.97	0.93	
Control Delay				24.1	62.1	
Queue Delay				0.0	0.0	
Total Delay				24.1	62.1	
LOS				24.1 C	02.1 E	
				24.1	62.1	
Approach Delay						
Approach LOS				С	E	

	→	\rightarrow	•	•	•	<i>></i>	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Queue Length 50th (ft)				567	155		
Queue Length 95th (ft)				#886	#252		
Internal Link Dist (ft)	379			2432	346		
Turn Bay Length (ft)							
Base Capacity (vph)				2555	572		
Starvation Cap Reductn				0	0		
Spillback Cap Reductn				0	0		
Storage Cap Reductn				0	0		
Reduced v/c Ratio				0.97	0.93		
Intersection Summary							
Area Type: O	ther						
Cycle Length: 90							
Actuated Cycle Length: 90							
Offset: 0 (0%), Referenced to	phase 6:\	NBT, Sta	rt of Gree	en			
Natural Cycle: 90							
Control Type: Actuated-Coord	linated						
Maximum v/c Ratio: 0.97							
Intersection Signal Delay: 30.					itersection		
Intersection Capacity Utilization	on 83.4%			IC	CU Level o	f Service E	
Analysis Period (min) 15							
# 95th percentile volume ex			eue may	be longe	r.		
Queue shown is maximum	after two	cycles.					
Splits and Phases: 5: U-Tu	rn East &	11S 64 W	R				
Spins and mases. S. U-Tu	III LUSI X	03 04 W	D .				I
←							 ◆
Ø6 (R)							\ Ø8

Intersection						
Int Delay, s/veh	0.6					
		EDT	WDT	WDD	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	<u>ነ</u>	†	†	70	\	0
Traffic Vol, veh/h	15	566	599	30	18	8
Future Vol, veh/h	15	566	599	30	18	8
Conflicting Peds, #/hr	0	0	0	_ 0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	100	0	-
Veh in Median Storage	:,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	17	629	666	33	20	9
Major/Minor N	Major1	N	Major2		Minor2	
Conflicting Flow All	699	0	viajorz	0	1329	666
			-			
Stage 1	-	-	-	-	666	-
Stage 2	- 4.10	-	-	-	663	- ())
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-		3.318
Pot Cap-1 Maneuver	898	-	-	-	171	459
Stage 1	-	-	-	-	511	-
Stage 2	-	-	-	-	512	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	898	-	-	-	168	459
Mov Cap-2 Maneuver	-	-	-	-	168	-
Stage 1	-	-	-	-	501	-
Stage 2	-	-	-	-	512	-
Approach	EB		WB		SB	
Approach	LD		0		25	
LICM Control Dolovi o	0.2		()			
HCM Control Delay, s	0.2		U			
HCM Control Delay, s HCM LOS	0.2				D	
	0.2		Ū		D	
HCM LOS		EBL	EBT	WBT	WBR S	SBLn1
HCM LOS Minor Lane/Major Mvm				WBT -		
HCM LOS		898			WBR :	209
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	t	898 0.019		-	WBR :	209 0.138
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	t	898 0.019 9.1	EBT - -	-	WBR :	209 0.138 25
Minor Lane/Major Mvm Capacity (veh/h) HCM Lane V/C Ratio	t	898 0.019	EBT - -	- - -	WBR S	209 0.138



Rezoning Case: 20CZ14 Hackney PUD

Planning Board Meeting Date: March 8, 2021



Report Requirements:

Per NCGS §160D-604(b), all proposed amendments to the zoning ordinance or zoning map shall be submitted to the Planning Board for review and comment. If no written report is received from the Planning Board within 30 days of referral of the amendment to the Planning Board, the Town Council may act on the amendment without the Planning Board report. The Town Council is not bound by the recommendations, if any, of the Planning Board.

Per NCGS §160D-604(d), the Planning Board shall advise and comment on whether the proposed action is consistent with all applicable officially adopted plans, and provide a written recommendation to the Town Council that addresses plan consistency and other matters as deemed appropriate by the Planning Board, but a comment by the Planning Board that a proposed amendment is inconsistent with the officially adopted plans shall not preclude consideration or approval of the proposed amendment by the Town Council.

approval of the proposed amendment by the Town Council.							
PROJECT DESCRIPTION Acreage: PIN(s):	N: ±79.79 acres 0721492629, 0722406699, & 0722411102						
Current Zoning:	Rural Resident	ial (RR) & R-80W					
Proposed Zoning:	Planned Unit Development-Conditional Zoning (PUD-CZ)						
2045 Land Use Map:	Medium Density Residential						
Town Limits:	ETJ and Outsid	e (annexation of por	rtion in Wake County is required with rezoning)				
Applicable Officially The Board must state of the state o	whether the prole	oject is consistent or	r inconsistent with the following officially adopted plans, them. Reason:				
Apex Transporta Consistent	ition Plan	☐ Inconsistent	Reason:				
Parks, Recreatio Consistent	n, Open Space,	and Greenways Plan Inconsistent	n Reason:				

Rezoning Case: 20CZ14 Hackney PUD

Planning Board Meeting Date: March 8, 2021



Legislative Considerations:

The applicant shall propose site-specific standards and conditions that take into account the following considerations, which are considerations that are relevant to the legislative determination of whether or not the proposed conditional zoning district rezoning request is in the public interest. These considerations do not exclude the legislative consideration of any other factor that is relevant to the public interest.

1.	its proposed location and coplan.	onsistency with the purposes	ditional Zoning (CZ) District use's appropriateness for , goals, objectives, and policies of the 2045 Land Us		
	✓ Consistent	Inconsistent	Reason:		
2.		ed Conditional Zoning (CZ) Dis character of surrounding land Inconsistent	strict use's appropriateness for its proposed location d uses. Reason:		
3.	Zoning district supplement Sec. 4.4 Supplemental Star ✓ Consistent		onditional Zoning (CZ) District use's compliance with		
4.	minimization of adverse avoidance of significant a	effects, including visual imp	e proposed Conditional Zoning (CZ) District use's act of the proposed use on adjacent lands; and ing lands regarding trash, traffic, service delivery, nd not create a nuisance. Reason:		
5.	environmental impacts ar habitat, scenic resources,	•	d Conditional Zoning District use's minimization of t deterioration of water and air resources, wildlife		
	✓ Consistent	Inconsistent	Reason:		

Rezoning Case: 20CZ14 Hackney PUD

Planning Board Meeting Date: March 8, 2021



6. Impact on public facilities. The proposed Conditional Zoning (CZ) District use's avoidance of harmonic impacts on public facilities and services, including roads, potable water and wastewater facilities.						
	✓ Consistent	☐ Inconsistent	Reason:			
7.	Health, safety, and welfare. The or welfare of the residents of the Consistent		ing (CZ) District use's effect on the health, safety, Reason:			
8.	Detrimental to adjacent pro substantially detrimental to ac Consistent	•	oposed Conditional Zoning (CZ) District use is			
9.		fic impact or noise, or becau	Conditional Zoning (CZ) District use constitutes a se of the number of persons who will be using the Reason:			
		_				
10.	-	posed on it by all other appli	ne proposed Conditional Zoning (CZ) District use cable provisions of this Ordinance for use, layout, Reason:			
			·			

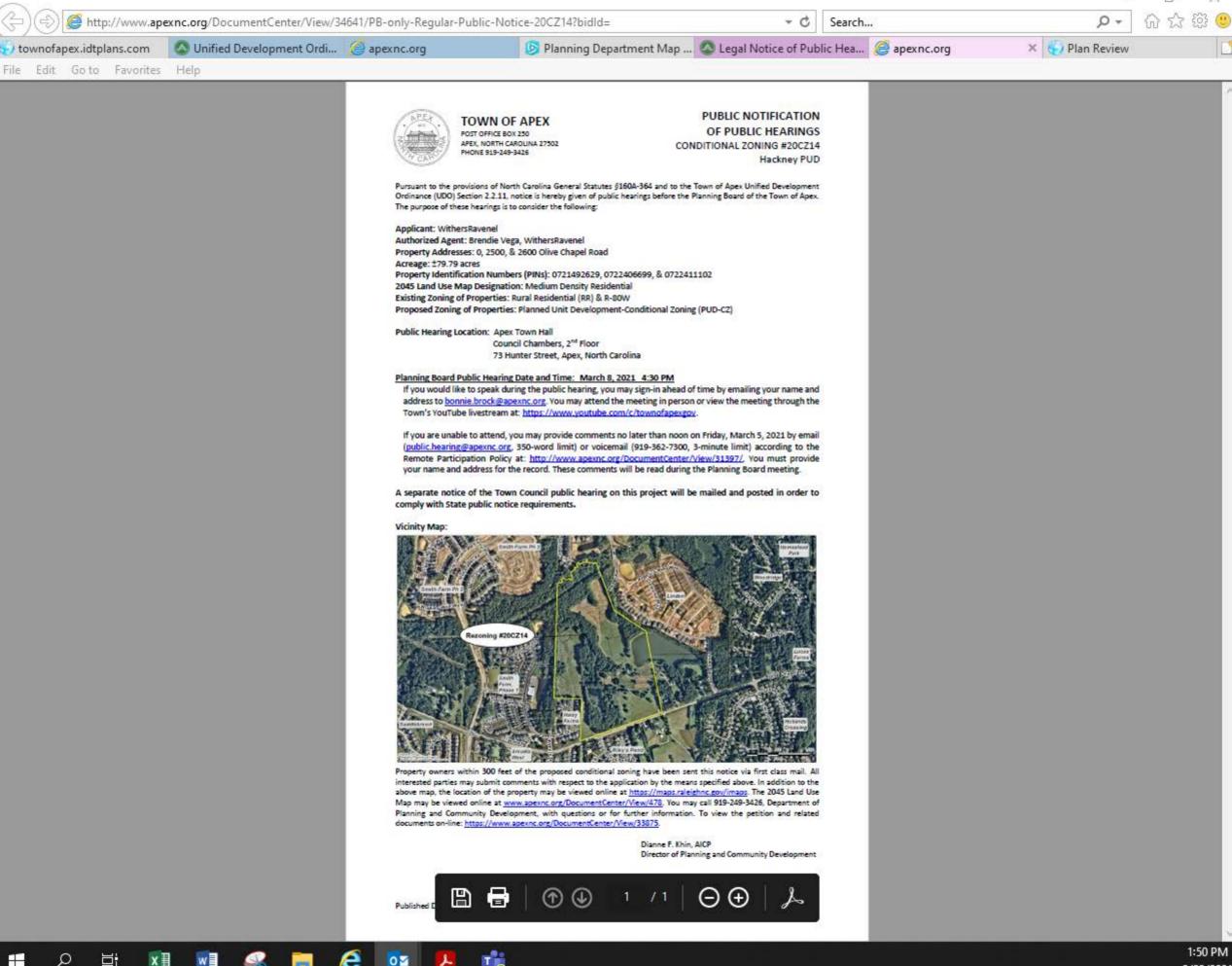
Rezoning Case: 20CZ14 Hackney PUD

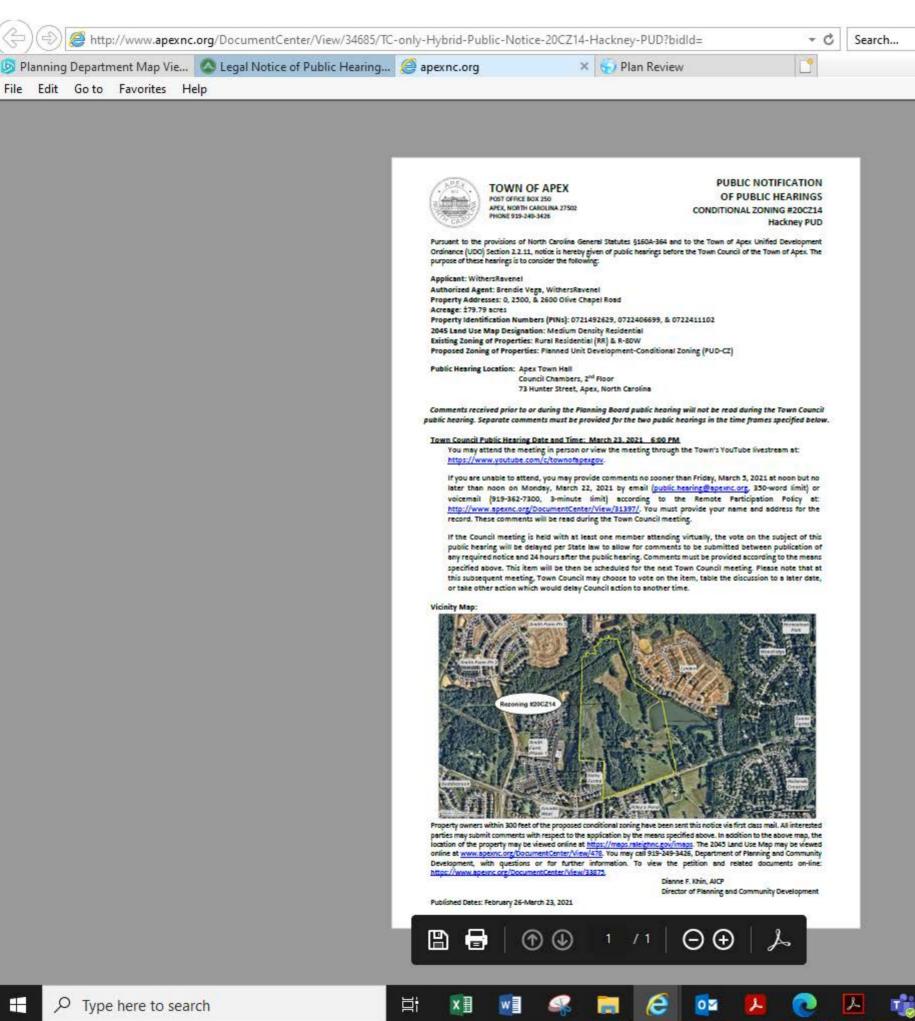
Planning Board Meeting Date: March 8, 2021



Planning Board Recommendation:

	Motion:	To recommend app	roval as presented.
ı	introduced by Planning Board member:	Keith Braswell	
			lly adopted plans and the applicable legislative
√		s noted above, so the	Il applicable officially adopted plans and/or the following conditions are recommended to be
Cond	litions proposed by the applicant.		
	Denial: the project is not consistent legislative considerations as noted about		officially adopted plans and/or the applicable
		With 6 Planning	Board Member(s) voting "aye"
			Board Member(s) voting "no"
		with <u> </u>	Board Member(s) voting "no"
	Reasons for dissenting votes:		
This	report reflects the recommendation of	the Planning Board, th	is the 8th day of March 2021.
Atte	st:		
Mic	chael Marks Digitally signed by Michael Date: 2021.03.09 14:50:1	el Marks 5 -05'00'	Dianne Khin Digitally signed by Dianne Khin Date: 2021.03.08 17:59:54 -05'00'
Mich	nael Marks, Planning Board Chair		Dianne Khin, Director of Planning and







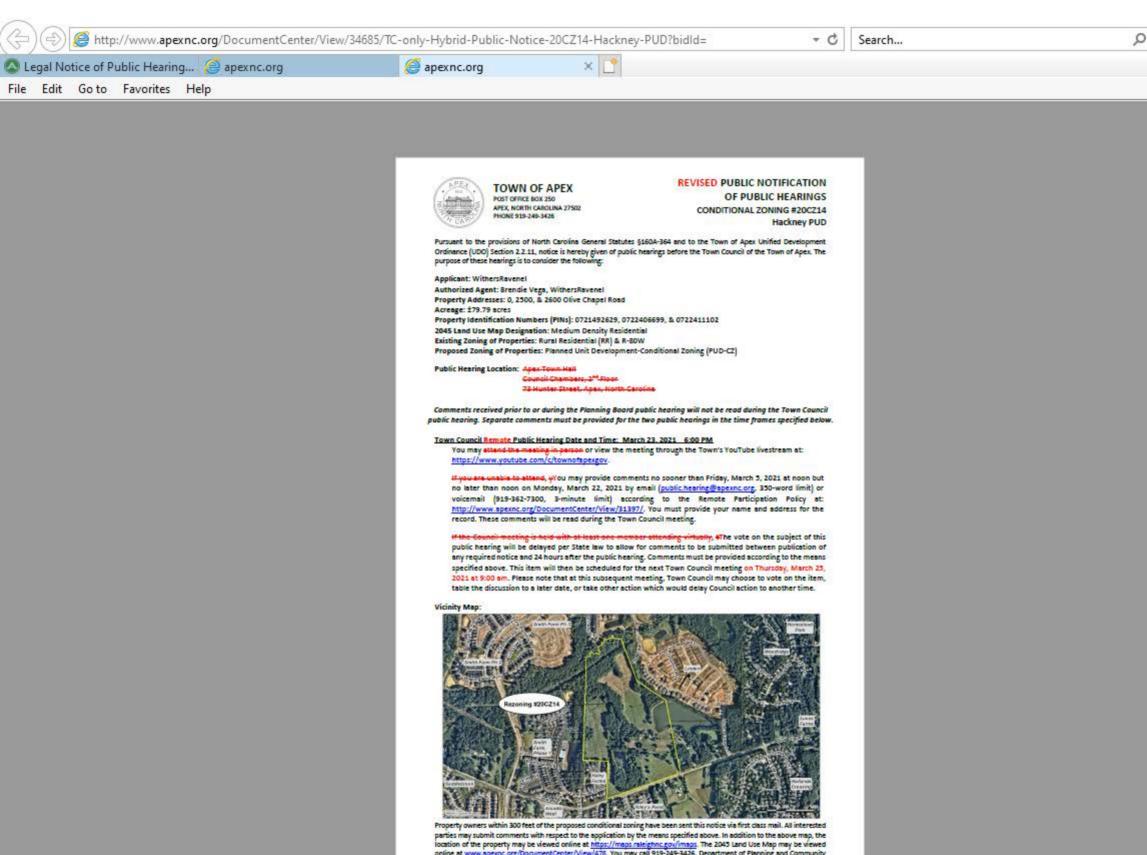












online at www.apexnc.org/DocumentCenter/View/478. You may call 919-249-3426, Department of Planning and Community Development, with questions or for further information. To view the petition and related documents on-line: https://www.apexnc.org/DocumentCenter/view/33873.

Director of Planning and Community Development

Published Dates: Sebruary 26 March 16-March 23, 2021













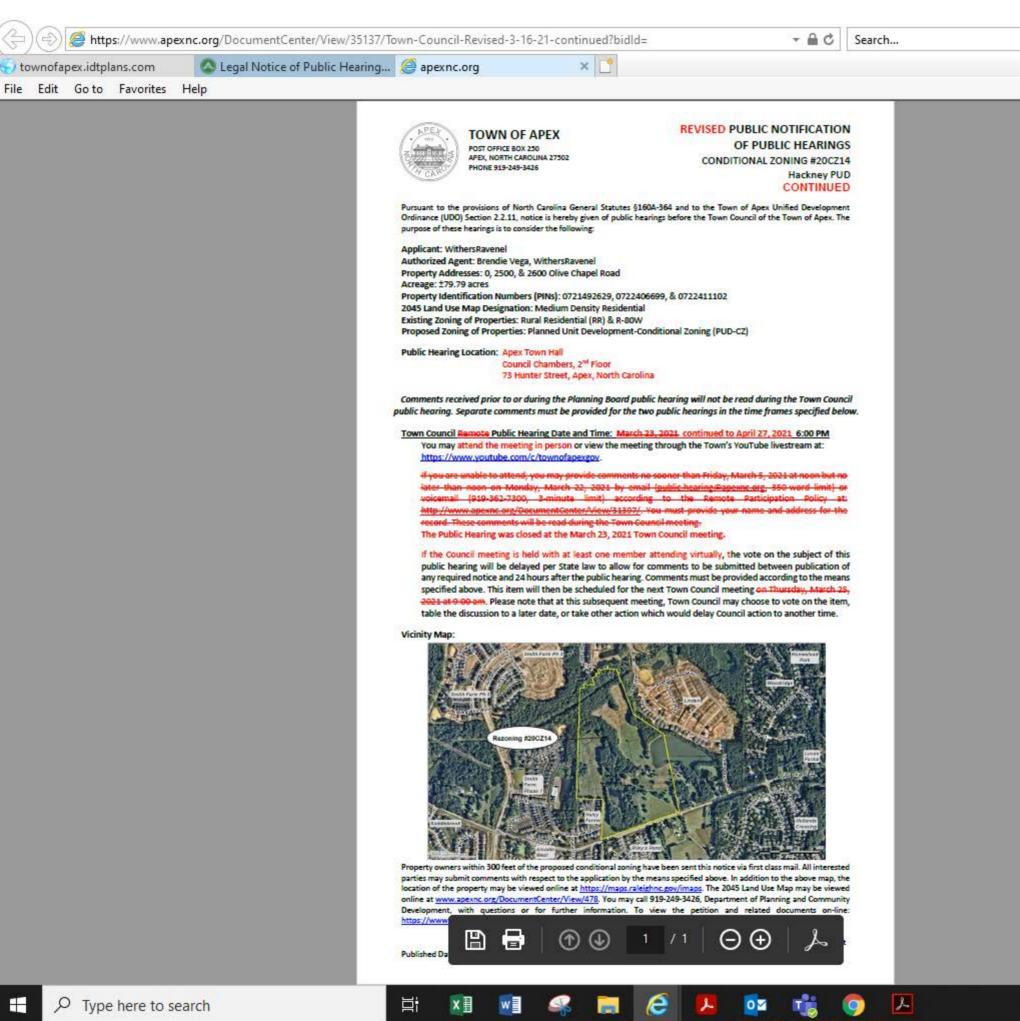












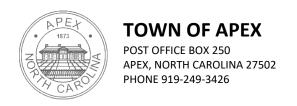












PUBLIC NOTIFICATION OF PUBLIC HEARINGS

CONDITIONAL ZONING #20CZ14
Hackney PUD

Pursuant to the provisions of North Carolina General Statutes §160A-364 and to the Town of Apex Unified Development Ordinance (UDO) Section 2.2.11, notice is hereby given of public hearings before the Town Council of the Town of Apex. The purpose of these hearings is to consider the following:

Applicant: WithersRavenel

Authorized Agent: Brendie Vega, WithersRavenel **Property Addresses:** 0, 2500, & 2600 Olive Chapel Road

Acreage: ±79.79 acres

Property Identification Numbers (PINs): 0721492629, 0722406699, & 0722411102

2045 Land Use Map Designation: Medium Density Residential **Existing Zoning of Properties:** Rural Residential (RR) & R-80W

Proposed Zoning of Properties: Planned Unit Development-Conditional Zoning (PUD-CZ)

Public Hearing Location: Apex Town Hall

Council Chambers, 2nd Floor

73 Hunter Street, Apex, North Carolina

Comments received prior to or during the Planning Board public hearing will not be read during the Town Council public hearing. Separate comments must be provided for the two public hearings in the time frames specified below.

Town Council Public Hearing Date and Time: March 23, 2021 6:00 PM

You may attend the meeting in person or view the meeting through the Town's YouTube livestream at: https://www.youtube.com/c/townofapexgov.

If you are unable to attend, you may provide comments no sooner than Friday, March 5, 2021 at noon but no later than noon on Monday, March 22, 2021 by email (public.hearing@apexnc.org, 350-word limit) or voicemail (919-362-7300, 3-minute limit) according to the Remote Participation Policy at: http://www.apexnc.org/DocumentCenter/View/31397/. You must provide your name and address for the record. These comments will be read during the Town Council meeting.

If the Council meeting is held with at least one member attending virtually, the vote on the subject of this public hearing will be delayed per State law to allow for comments to be submitted between publication of any required notice and 24 hours after the public hearing. Comments must be provided according to the means specified above. This item will be then be scheduled for the next Town Council meeting. Please note that at this subsequent meeting, Town Council may choose to vote on the item, table the discussion to a later date, or take other action which would delay Council action to another time.

Vicinity Map:



Property owners within 300 feet of the proposed conditional zoning have been sent this notice via first class mail. All interested parties may submit comments with respect to the application by the means specified above. In addition to the above map, the location of the property may be viewed online at https://maps.raleighnc.gov/imaps. The 2045 Land Use Map may be viewed online at https://www.apexnc.org/DocumentCenter/View/478. You may call 919-249-3426, Department of Planning and Community Development, with questions or for further information. To view the petition and related documents on-line: https://www.apexnc.org/DocumentCenter/View/33875.

Dianne F. Khin, AICP
Director of Planning and Community Development

Published Dates: February 26-March 23, 2021



PUBLIC NOTIFICATION OF PUBLIC HEARINGS

CONDITIONAL ZONING #20CZ14
Hackney PUD

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Public Hearing Location: Apex Town Hall

Council Chambers, 2nd Floor

73 Hunter Street, Apex, North Carolina

Planning Board Public Hearing Date and Time: March 8, 2021 4:30 PM

If you would like to speak during the public hearing, you may sign-in ahead of time by emailing your name and address to bonnie.brock@apexnc.org. You may attend the meeting in person or view the meeting through the Town's YouTube livestream at: https://www.youtube.com/c/townofapexgov.

If you are unable to attend, you may provide comments no later than noon on Friday, March 5, 2021 by email (public.hearing@apexnc.org, 350-word limit) or voicemail (919-362-7300, 3-minute limit) according to the Remote Participation Policy at: http://www.apexnc.org/DocumentCenter/View/31397/. You must provide your name and address for the record. These comments will be read during the Planning Board meeting.

A separate notice of the Town Council public hearing on this project will be mailed and posted in order to comply with State public notice requirements.

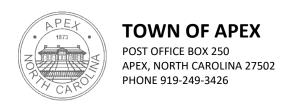
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Dianne F. Khin, AICP
Director of Planning and Community Development

Published Dates: February 23-March 8, 2021



REVISED PUBLIC NOTIFICATION OF PUBLIC HEARINGS

CONDITIONAL ZONING #20CZ14
Hackney PUD

Pursuant to the provisions of North Carolina General Statutes §160A-364 and to the Town of Apex Unified Development Ordinance (UDO) Section 2.2.11, notice is hereby given of public hearings before the Town Council of the Town of Apex. The purpose of these hearings is to consider the following:

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Public Hearing Location: Apex Town Hall

Council Chambers, 2nd Floor

73 Hunter Street, Apex, North Carolina

Comments received prior to or during the Planning Board public hearing will not be read during the Town Council public hearing. Separate comments must be provided for the two public hearings in the time frames specified below.

Town Council Remote Public Hearing Date and Time: March 23, 2021 6:00 PM

You may attend the meeting in person or view the meeting through the Town's YouTube livestream at: https://www.youtube.com/c/townofapexgov.

If you are unable to attend, yYou may provide comments no sooner than Friday, March 5, 2021 at noon but no later than noon on Monday, March 22, 2021 by email (public.hearing@apexnc.org, 350-word limit) or voicemail (919-362-7300, 3-minute limit) according to the Remote Participation Policy at: http://www.apexnc.org/DocumentCenter/View/31397/. You must provide your name and address for the record. These comments will be read during the Town Council meeting.

If the Council meeting is held with at least one member attending virtually, the vote on the subject of this public hearing will be delayed per State law to allow for comments to be submitted between publication of any required notice and 24 hours after the public hearing. Comments must be provided according to the means specified above. This item will then be scheduled for the next Town Council meeting on Thursday, March 25, 2021 at 9:00 am. Please note that at this subsequent meeting, Town Council may choose to vote on the item, table the discussion to a later date, or take other action which would delay Council action to another time.

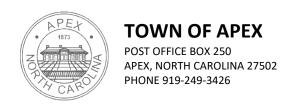
Vicinity Map:



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Dianne F. Khin, AICP
Director of Planning and Community Development

Published Dates: February 26 March 16-March 23, 2021



REVISED PUBLIC NOTIFICATION OF PUBLIC HEARINGS

CONDITIONAL ZONING #20CZ14

Hackney PUD

CONTINUED

Pursuant to the provisions of North Carolina General Statutes §160A-364 and to the Town of Apex Unified Development Ordinance (UDO) Section 2.2.11, notice is hereby given of public hearings before the Town Council of the Town of Apex. The purpose of these hearings is to consider the following:

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Public Hearing Location: Apex Town Hall

Council Chambers, 2nd Floor

73 Hunter Street, Apex, North Carolina

Comments received prior to or during the Planning Board public hearing will not be read during the Town Council public hearing. Separate comments must be provided for the two public hearings in the time frames specified below.

Town Council Remote Public Hearing Date and Time: March 23, 2021 continued to April 27, 2021 6:00 PM

You may attend the meeting in person or view the meeting through the Town's YouTube livestream at: https://www.youtube.com/c/townofapexgov.

If you are unable to attend, you may provide comments no sooner than Friday, March 5, 2021 at noon but no later than noon on Monday, March 22, 2021 by email (<u>public.hearing@apexnc.org</u>, 350 word limit) or voicemail (919-362-7300, 3 minute limit) according to the Remote Participation Policy at: http://www.apexnc.org/DocumentCenter/View/31397/. You must provide your name and address for the record. These comments will be read during the Town Council meeting.

The Public Hearing was closed at the March 23, 2021 Town Council meeting.

If the Council meeting is held with at least one member attending virtually, the vote on the subject of this public hearing will be delayed per State law to allow for comments to be submitted between publication of any required notice and 24 hours after the public hearing. Comments must be provided according to the means specified above. This item will then be scheduled for the next Town Council meeting on Thursday, March 25, 2021 at 9:00 am. Please note that at this subsequent meeting, Town Council may choose to vote on the item, table the discussion to a later date, or take other action which would delay Council action to another time.

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Dianne F. Khin, AICP
Director of Planning and Community Development

Published Dates: February 26 March 16 March 26 - March 23 April 27, 2021





TOWN OF APEX

POST OFFICE BOX 250 APEX, NORTH CAROLINA 27502 PHONE 919-249-3426

AFFIDAVIT CERTIFYING Public Notification – Written (Mailed) Notice

Section 2.2.11

Town of Apex Unified Development Ordinance

Project Name:

Conditional Zoning #20CZ14

Hackney PUD

Project Location:

0, 2500, & 2600 Olive Chapel Road

Applicant or Authorized Agent:

Brendie Vega, WithersRavenel

Firm:

WithersRavenel

This is to certify that I, as Director of Planning and Community Development, mailed or caused to have mailed by first class postage for the above mentioned project on February 26, 2021, a notice containing the time and place, location, nature and scope of the application, where additional information may be obtained, and the opportunity for interested parties to be heard, to the property owners within 300' of the land subject to notification. I further certify that I relied on information provided to me by the above-mentioned person as to accuracy and mailing addresses of property owners within 300' of the land subject to notification.

2-26-2021 Date

STATE OF NORTH CAROLINA **COUNTY OF WAKE**

Sworn and subscribed before me,

Jeri Chastain Pederson, a Notary Public for the above

State and County, this the

26 day of <u>February</u>, 2021.

JERI CHASTAIN PEDERSON Notary Public Wake County, North Carolina My Commission Expires SEAL March 10, 2024

Jere Chastan Pederson Notary Public

My Commission Expires: $\frac{3}{10}$ / $\frac{2024}{100}$



TOWN OF APEX

POST OFFICE BOX 250 APEX, NORTH CAROLINA 27502 PHONE 919-249-3426

AFFIDAVIT CERTIFYING Public Notification – Written (Mailed) Notice

Section 2.2.11

Town of Apex Unified Development Ordinance

Project Name:

Conditional Zoning #20CZ14

Hackney PUD

Project Location:

0, 2500, & 2600 Olive Chapel Road

Applicant or Authorized Agent:

Brendie Vega, WithersRavenel

Firm:

WithersRavenel

This is to certify that I, as Director of Planning and Community Development, mailed or caused to have mailed by first class postage for the above mentioned project on February 23, 2021, a notice containing the time and place, location, nature and scope of the application, where additional information may be obtained, and the opportunity for interested parties to be heard, to the property owners within 300' of the land subject to notification. I further certify that I relied on information provided to me by the above-mentioned person as to accuracy and mailing addresses of property owners within 300' of the land subject to notification.

2-23-2021 Date

STATE OF NORTH CAROLINA **COUNTY OF WAKE**

Sworn and subscribed before me,

Jeri Chastain Pederson, a Notary Public for the above

State and County, this the

23 day of February , 202 1 .

JERI CHASTAIN PEDERSON Notary Public Wake County, North Carolina My Commission Expires March 10, 2024

Jeu Chastain Poclesson
Notary Public

My Commission Expires: 3 / 10 / 2024



Student Assignment Glenn Carrozza 5625 Dillard Drive Cary, NC 27518

February 17, 2021

tel: (919) 431-7333 fax: (919) 694-7753

Dianne Khin, AICP
Director, Department of Planning and Community Development
Town of Apex
Dianne.Khin@apexnc.org

Dear Dianne,

The Wake County Public School System (WCPSS) Office of School Assignment received information about a proposed rezoning/development within the Town of Apex planning area. We are providing this letter to share information about WCPSS's capacity related to the proposal. The following information about the proposed rezoning/development was provided through the Wake County Residential Development Notification database:

- Date of application: November 2, 2020
- Name of development: 20CZ14 Hackney Tracts PUD
- Address of rezoning/development: 0, 2500, & 2600 Olive Chapel Rd
- Total number of proposed residential units: 319
- Type(s) of residential units proposed: Single-family; townhouse; townhouse, detached; accessory apartment

Based on the information received at the time of application, the Office of School Assignment is providing the following assessment of possible impacts to the Wake County Public School System:

		· · · · · · · · · · · · · · · · · · ·	· unc co	and rubile beneal by.	sterri.			
	Schools at <u>all</u> grade levels within the current assignment area for the proposed rezoning/development are anticipated to have <u>sufficient</u> capacity for future students.							
X	Schools at the following grade levels within the current assignment area for the proposed rezoning/development are anticipated to have insufficient capacity for future students; transportation to schools outside of the current assignment area should be anticipated:							
C	XI	Elementary		Middle	X	High		
The foll	he following mitigation of capacity concerns due to school construction or expansion is anticipated:							
	Not app	plicable – existing school	capacit	ty is anticipated to be	sufficient.			
	School	expansion or constructio	n withii	n the next five years is	not anticip	pated to address concerns.		
又	School	expansion or constructio	n withii	n the next five years m	ay address	concerns at these grade levels:		
		Elementary		Middle	X	High		
Thank you for sharing this information with the Town of Apex Planning Board and Town Council as they consider the proposed rezoning/development.								

Sincerely,

Glenn Carrozza

Glenn Carrozza