Rezoning #25CZ15 West Village PUD Amendment

May 27, 2025 Town Council Meeting



All property owners, tenants, and neighborhood associations within 300 feet of this rezoning have been notified per UDO Sec. 2.2.11 *Public Notification*.

BACKGROUND INFORMATION:

Location: 2517 Kelly Road

Owner: Fahey Family Farm, LLC

Applicant: Trilandco, LLC

Authorized Agent: Matthew Carpenter, Parker Poe

PROJECT DESCRIPTION:

Acreage: ±5.8591 **PIN:** 0731434504

Current Zoning: Planned Unit Development-Conditional Zoning (PUD-CZ #15CZ33)

Proposed Zoning: Planned Unit Development-Conditional Zoning (PUD-CZ)

2045 Land Use Map: Mixed Use: High Density Residential/Office Employment/Commercial Services

Town Limits: ETJ

Adjacent Zoning & Land Uses:

	Zoning	Land Use
North:	Planned Unit Development-Conditional Zoning (PUD-CZ #15CZ33)	Townhomes (West Village)
South:	Residential Agricultural (RA)	Single-family
East:	Residential Agricultural (RA)	NC 540 Highway
West:	Planned Unit Development-Conditional Zoning (PUD-CZ #15CZ33); Mixed Office-Residential-Retail-Conditional Zoning (MORR-CZ #19CZ23)	Kelly Road; Single-family; Office; Future West Village Commercial

EXISTING CONDITIONS:

The subject property is located on the east side of Kelly Road and west of NC 540 Highway. The parcel includes structures and existing vegetation. The property was originally rezoned to Planned Unit Development-Conditional Zoning on July 19, 2016 as part of the West Village development. The parcel is classified as "POD 1" and is approved for non-residential uses.

NEIGHBORHOOD MEETING:

The applicant conducted the first neighborhood meeting on January 30, 2025 and the second neighborhood meeting on April 15, 2025. The neighborhood meeting reports are attached.

2045 LAND USE MAP:

The 2045 Land Use Map classifies the property subject to this rezoning as Mixed Use: High Density Residential/Office Employment/Commercial Services. The proposed amendments to the PUD-CZ zoning are consistent with that classification.

APEX TRANSPORTATION PLAN

The Apex Transportation Plan identifies a roundabout at the location of the proposed site driveway on Kelly Road across from a future major collector street west of Kelly Road. Additionally, the Transportation Plan shows widening on Kelly Road from the intersection northward to a 4-lane, median divided

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thoroughfare with 6-foot bike lanes and 5-foot sidewalks, and southward as a 3-lane thoroughfare with widening with 6-foot bike lanes and 5-foot sidewalks. The proposed amendments to the PUD-CZ zoning are consistent with the Apex Transportation Plan.

WCPSS COORDINATION:

No increase in residential density is proposed as part of this rezoning and so an impact letter from WCPSS was not requested.

ENVIRONMENTAL ADVISORY BOARD RECOMMENDATIONS:

This rezoning was exempt from meeting with the Apex Environmental Advisory Board (EAB) per Unified Development Ordinance (UDO) Section 2.1.9.A.2.a. The rezoning amends zoning conditions which have no environmental impact on a site including, but not limited to, revisions to architectural standards, building height, setbacks, and uses.

BACKGROUND:

The current West Village PUD includes transportation and road improvement conditions linked to the non-residential parcels. Currently, the transportation infrastructure improvements in the non-residential district must be completed before issuance of the first certificate of occupancy for a non-residential building. As the PUD is currently written, the first parcel to develop a non-residential use(s) would need to construct all required road improvements and transportation infrastructure for the overall permitted 500,000 square feet of non-residential uses, with the exception of what is specifically tied to POD 3 (i.e. construction of the roundabout on Kelly Road and Public Street A connection from the Kelly Road roundabout to Street B roundabout).

The proposed PUD amendment would allow for POD 1 to develop and provide only those transportation improvements associated with this property with the remaining road improvements to occur with development in the remaining non-residential pods (PODS 2 and 3).

PLANNED UNIT DEVELOPMENT PLAN:

The applicant is proposing the following changes to Section 13: Public Facilities associated with POD 1 with this PUD amendment. There are no other changes proposed to the previously approved rezonings. Deletions are shown with strikethrough and additions are shown in bold.

Section 13: Public Facilities

The proposed PUD shall meet all Public Facilities requirements as set forth in UDO Section 2.3.4(F)(1)(f) and be designed according to sound engineering standards. Specifically, road and utility infrastructure shall be as follows:

General Roadway Infrastructure:

All proposed roadway infrastructure will be consistent with the Town of Apex UDO and Transportation Plan (updated in 2011), and the Traffic Impact Analysis approved by the Town of Apex and NCDOT. An internal road network will be provided in accordance with the Town's UDO. All road networks will promote connectivity wherever possible to adjacent neighborhoods, undeveloped property, nearby points of interest, and municipal destinations. Further, cul-de-sacs will be avoided except where environmental features make through streets unfeasible.

Roadway Phasing – Prior to time of the fifty-first certificate of occupancy associated with the residential located off of Old US HWY 1, the second point of access (southernmost portion of Street A),

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necessary portion of the southernmost roundabout and Street B shown shall be constructed. As a part of the non-residential development in Pod 3, the roundabout proposed on Kelly Road shall be constructed along with the portion of Street A tying back to the southernmost roundabout. Prior to time of the first certificate of occupancy associated with Pod 3 POD 3, as part of the development of POD 3, Street A will be complete the connection from Kelly Road and Old US HWY 1.

Off-Site Transportation Conditions:

The project will also provide the following off-site transportation conditions:

All recommendations on state-maintained roadways are subject to NCDOT review and approval as part of the driveway permits and encroachment agreements. Build 2018 refers to the first plat of residential development. or as otherwise determined by Apex Town Council during the review and approval of subdivision plans. Build 2020 Build 2030 refers to the first plat of commercial development or as otherwise determined by Apex Town Council during the review and approval of commercial stie plans for POD 2 or POD 3 and does not include the development of POD 1, also identified as Build 2028, which is subject to separate recommendations set forth below. Please note that prior analysis and the original basis for build-out recommendations was based on a Build 2020 analysis. Internal Protected Storage Length (IPS) refers to the required minimum distance from the intersection along the proposed driveway or street before any full movement commercial driveway access or public street intersection will be allowed.

Developer shall provide right-of-way dedication along Kelly Road and Old US 1 based on a 100-foot right-of-way. Where Old US 1 abuts railroad right-of-way the developer shall be responsible for dedicating public right-of-way 70 feet from roadway centerline along the project frontage or as otherwise required to accommodate a 100-foot road right-of-way exclusive of railroad right-of-way.

Street 'A' and Street 'B' (including Kelly Road at Site Drive #4)

- Street 'A' shall be constructed as a 3-lane 38-foot curb and gutter street with 5-foot sidewalk on both sides on 62-foot public right-of-way.
- Street 'B' shall be constructed as a 2-lane 39'-foot curb and gutter street with on-street parking and 6-foot sidewalk on both sides on 53-foot public right-of-way.
- Residential driveway access shall not be permitted along Streets 'A' and 'B'.
- Prior to platting the 51st residential unit in the Residential area located adjacent to Old US 1, developer shall construct and dedicate as public Street 'A' from Site Drive #5 to the roundabout at Street 'B', roundabout serving Street 'A' at Street 'B', and Street 'B' from Site Drive #6/Pleasant Plains Road to the roundabout at Street 'A'.
- Prior to the first certificate of occupancy within POD 3, developer shall construct and dedicate
 as public Street 'A' from the roundabout at Street 'B' to Kelly Road at Site Drive #4 and construct
 a roundabout on Kelly Road at Site Drive #4. The roundabout will serve a 4-lane divided roadway
 to the north and 2-lane roadwayto the south for Build 2020.
 - POD 1: Kelly Road Roundabout. Developer shall prepare a preliminary design and engineer's estimate for review and approval and dedicate right of way and construction easements for the future construction of a roundabout at the intersection of the site driveway, Kelly Road, and Street A as shown on the Town of Apex Thoroughfare and Collector Street Plan (the "Roundabout"). Prior to site plan final plat for POD 1, Developer shall pay a fee in lieu for eight percent (8%) of the estimated costs to design and construct the Roundabout.

Kelly Road at Olive Chapel Road

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- Developer shall construct a 200-foot eastbound right turn lane and a 300-foot additional westbound left turn lane on Olive Chapel Road (with southbound receiving through lane on Kelly Road) for Build 2020 Build 2030.
- POD 1: Developer shall provide a preliminary plan and engineer's estimate for review and approval and pay a fee in lieu in the amount of 8% of the total estimated cost of the aforementioned improvements prior to site plan final plat in POD 1.

Kelly Road at Apex Barbecue Road

- Developer shall construct a 400-foot eastbound left turn lane, 350-foot westbound left turn lane, 350-foot northbound left turn lane, 150-foot northbound right turn lane, 350-foot southbound left turn lane, and 200-foot southbound rightturn lane for Build 2020.
- Improvements have been completed by others satisfying the prior zoning requirements for a
 Build 2020 analysis including the following: construction of a 400-foot eastbound left turn
 lane, 350-foot westbound left turn lane, 350-foot northbound left turn lane, 150-foot
 northbound right turn lane, 350-foot southbound left turn lane, and 200-foot southbound
 right turn lane. No additional improvements are recommended for updated commercial
 build dates, Build 2028 (POD 1) and Build 2030 (PODS 2 & 3).

Kelly Road at Southwinds Run

Developer shall construct a 100-foot northbound left turn lane on Kelly Road, an additional (second) northbound through lane through the intersection to drop as a right turn at Site Drive #1, and begin an additional (second) southbound through lane immediately south of Southwinds Run for Building 2020 Build 2030.

Old US 1 at Kelly Road

- Developer shall construct a 100-foot westbound right turn lane on Old US 1 for Build 2018.
- Developer shall convert the intersection to right-in/right-out as well as construct an additional
 westbound through lane, beginning at the NC 540 Southbound off-ramp as a free-flow right
 turn exiting the ramp, along with a 200-foot westbound right turn lane on Old US 1 for Build
 2020-Build 2030.
- Prior to platting the 300th residential unit, the developer will complete a signal warrant analysis at the intersection of Old US 1 and Kelly Road to determine if a traffic signal is warranted at the intersection. If the signal is warranted and approved for installation by NCDOT, the developer will permit and install the traffic signal. However, if Street "A" through POD 3 is under construction prior toinstallation of the signal, then the requirement for the signal shall be waived and the Kelly Road / Old US 1 intersection shall be converted to a Right-In/Right-Out as required in the improvements for the commercial development.

NC 540 Ramps at Old US 1

 Developer shall provide a free-flow right turn lane for the NC 540 Southbound off-ramp and additional receiving through lane continuing west to drop as a right turn at Site Drive #6 on Old US 1 for <u>Build 2020-Build 2030</u>.

Old US 1 at Pleasant Plains Road & Site Drive #6

- Developer shall construct Site Drive #6 with a through-right lane and a 200-footleft turn lane for Build 2018.
- Developer shall construct a 200-foot eastbound left turn lane and 200-foot westbound left turn

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lane on Old US 1 for Build 2018.

- Developer shall install a traffic signal once warranted and approved by NCDOT and install communication with the NC 540 traffic signals for Build 2020.
- Developer shall construct an additional westbound through lane on Old US 1 to drop as a right turn lane at Site Drive #6 for Build 2020 Build 2030.
- Developer shall construct an additional eastbound through lane on Old US 1beginning 400 feet west of Site Drive #6 for Build 2020 Build 2030.

Kelly Road at Site Drive #1

- Developer shall construct Site Drive #1 as a right-in and right-out only providing 100 feet IPS and a 100-foot northbound right turn lane on Kelly Road for Build 2018.
- Developer shall construct an additional northbound through lane on Kelly Road to drop as a right turn lane at Site Drive #1 for Build 2020 Build 2030.

Kelly Road at Site Drive #2

- Developer shall construct Site Drive #2 as a right-in and right-out only providing 100 feet IPS for Build 2018.
- Developer shall construct an additional northbound through lane and an additional southbound through lane on Kelly Road at Site Drive #2 for Build 2020 Build 2030.

Kelly Road at Site Drive #3

- Developer shall construct Site Drive #3 with a 100-foot eastbound right turn lane and a 100-foot westbound right turn lane on the Site Drive #3 approaches adjacent to through-right lanes with 200 feet IPS for Build 2018.
- Developer shall construct 100-foot northbound and 100-foot southbound left turn lanes on Kelly Road for Build 2018.
- Developer shall construct an additional (second) southbound through lane and additional (second) northbound through lane on Kelly Road at Site Drive #3 for Build 2020 Build 2030.

Old US 1 at Site Drive #5

- Developer shall construct convert Site Drive #5 as a full-movement intersection-from a right-in/right-out to a signalized full-movement intersection with 200-foot dual southbound left turn lanes and a 200-foot southbound right turn lane providing 300 feet IPS providing connectivity to both the residential and commercial phases for Build 2020-Build 2030.
- Developer shall construct an additional (second) westbound through lane and add a 200-foot westbound right turn lane on Old US 1 for Build 2020 Build 2030.
- Developer shall construct a 300-foot eastbound left turn lane and an additional (second)
 eastbound through lane on Old US 1 dropping 1000 feet east of the intersection for Build 2020
 Build 2030.

Construction of the preceding roadway infrastructure improvements shall not be required for the development of POD 1. Fee in lieu payments and additional right of way dedication where required for POD 1 are noted in the above sections where applicable. Development of POD 1 shall include the below roadway infrastructure improvements which shall be consistent with the Traffic Impact Analysis on file with the Town of Apex prepared by DRMP dated 1/30/2025 and the Town of Apex Transportation Plan. The road improvements shall be subject to Town of Apex and North Carolina

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Department of Transportation approval.

Kelly Road Widening. Developer shall dedicate right of way for the length of the property's
Kelly Road frontage, measured a minimum of 55 feet from the existing centerline of Kelly
Road, and widen and improve Kelly Road for the length of the property's Kelly Road frontage
based on an 84-foot back-to-back curb and gutter 4-lane divided roadway with 5-foot
sidewalks and 6-foot bike lanes in a 110-foot right of way.

• Kelly Road and Site Driveway. Developer shall construct:

- the Site Driveway with a stop-controlled approach, one ingress lane, and one egress lane; and
- a southbound left turn lane on Kelly Road with a minimum of 75 feet of storage.

• Electric Charging Stations:

Developer shall provide 2 charging stations, one within the residential and one within the non-residential for electric vehicles within the overall project. In addition to these committed stations, two additional charging stations will be installed as part of the overall project.

Water & Sewer Utilities:

All water and sanitary sewer service will be provided by the developer and conform to the Town of Apex Public Works and Utilities Department requirements. Preliminary location and tie in points are shown on sheet C-3 and C-4 of the PUD plans. The water extension shown along Kelly Road to Old US HWY 1, alternatively, could be located through Pod – 3 Commercial and southernmost residential accomplishing the intent of the routing shown on sheet C-3 and C-4. The ultimate routing will be dictated by timing of commercial development, roadway construction internal to the site and timing commitment related to the extension. However, this will be coordinated with the Town of Apex at time of site plan and construction documents.

Developer to provide \$75,000 in escrow for use by the Westwinds community for insurance associated with community wells, for use in drilling new wells or to extend public water into the community as a primary or secondary water source. Ultimately, the well-related use of the funds will be determined by the Westwinds community. Land owners will be required to annex into the Town ofApex prior to making any connections to public water services provided by the Town.

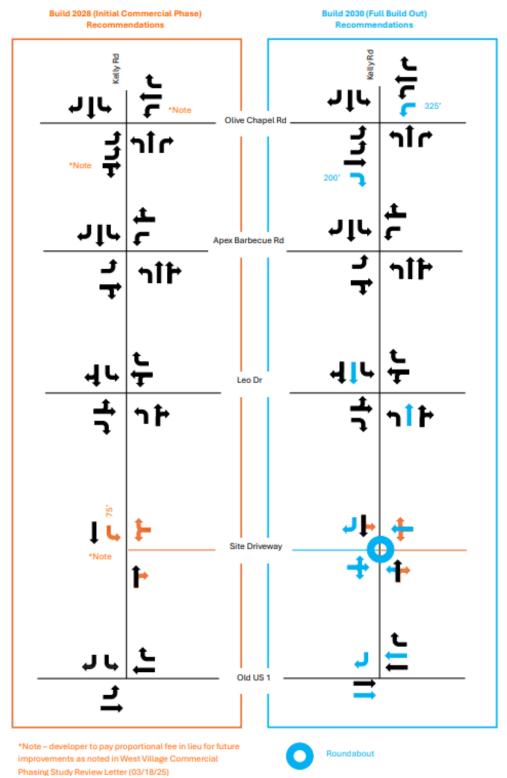
Other Utilities:

Electricity will be provided by Apex Electric. Phone, cable and gas will be provided by the developer and shall meet the Town of Apex standards as outlined in the UDO.



PROPOSED TRANSPORTATION IMPROVEMENTS:

The following figure illustrates the proposed Build 2028 (Initial Commercial Phase) Recommendations and Build 2030 (Full Build Out) recommendations:



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PLANNING STAFF RECOMMENDATION:

Planning staff recommends approval of Rezoning #25CZ05 West Village PUD Amendment as proposed by the applicant.

Transportation staff agree with the proposed transportation conditions proposed by the applicant and the TIA recommendations. Under Build 2028, the conditions of the site driveway are expected to operate acceptably as proposed.

PLANNING BOARD RECOMMENDATION:

The Planning Board held a public hearing on May 12, 2025 and unanimously recommended approval with the conditions offered by the applicant.

ANALYSIS STATEMENT OF THE REASONABLENESS OF THE PROPOSED REZONING:

This Statement will address consistency with the Town's comprehensive and other applicable plans, reasonableness, and effect on public interest:

The proposed amendments to the current PUD-CZ zoning are consistent with the 2045 Land Use Map which classifies the area to be rezoned as Mixed Use: High Density Residential/Office Employment/Commercial Services.

The proposed rezoning is reasonable and in the public interest as it will ensure compliance with the existing conditions of the previously approved PUDs while allowing the subject property (POD 1) to construct transportation improvements appropriate to the size of the parcel and the square footage of the non-residential use. The PUD amendment will allow the remaining road improvements from the previous rezoning to be constructed as the non-residential uses are developed in POD 2 and 3.

PLANNED UNIT DEVELOPMENT DISTRICT AND CONDITIONAL ZONING STANDARDS: Standards

In return for greater flexibility in site design requirements, Planned Development (PD) Districts are expected to deliver exceptional quality community designs that preserve critical environmental resources; provide high quality community amenities; incorporate creative design in the layout of buildings, Resource Conservation Area and circulation; ensure compatibility with surrounding land uses and neighborhood character; provide high quality architecture; and provide greater efficiency in the layout and provision of roads, utilities, and other infrastructure. The Planned Development (PD) Districts shall not be used as a means of circumventing the Town's adopted land development regulations for routine developments.

- Planned Unit Development (PUD-CZ) District
 In approving a Planned Development (PD) Zoning District designation for a PUD-CZ, the Town Council shall find the PUD-CZ district designation and PD Plan for PUD-CZ demonstrates compliance with the following standards:
 - a) Development parameters
 - (i) The uses proposed to be developed in the PD Plan for PUD-CZ are those uses permitted in Sec. 4.2.2 *Use Table*.
 - (ii) The uses proposed in the PD Plan for PUD-CZ can be entirely residential, entirely non-residential, or a mix of residential and non-residential uses, provided a minimum percentage of non-residential land area is included in certain mixed use areas as specified on the 2045 Land Use Map. The location of uses proposed by the PUD-CZ must be shown in the PD Plan with a maximum density for each type of residential use and a maximum square footage for each type of non-residential use.

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- (iii) The dimensional standards in Sec. 5.1.3 Table of Intensity and Dimensional Standards, Planned Development Districts may be varied in the PD Plan for PUD-CZ. The PUD-CZ shall demonstrate compliance with all other dimensional standards of the UDO, North Carolina Building Code, and North Carolina Fire Code.
- (iv) The development proposed in the PD Plan for PUD-CZ encourages cluster and compact development to the greatest extent possible that is interrelated and linked by pedestrian ways, bikeways and other transportation systems. At a minimum, the PD Plan must show sidewalk improvements as required by the Apex Transportation Plan and the *Town of Apex Standard Specifications and Standard Details*, and greenway improvements as required by the Town of Apex Parks, Recreation, Greenways, and Open Space Plan and the Apex Transportation Plan. In addition, sidewalks shall be provided on both sides of all streets for single-family detached homes.
- (v) The design of development in the PD Plan for PUD-CZ results in land use patterns that promote and expand opportunities for walkability, connectivity, public transportation, and an efficient compact network of streets. Cul-de-sacs shall be avoided unless the design of the subdivision and the existing or proposed street system in the surrounding area indicate that a through street is not essential in the location of the proposed cul-de-sac, or where sensitive environmental areas such as streams, floodplains, and wetlands would be substantially disturbed by making road connections.
- (vi) The development proposed in the PD Plan for PUD-CZ is compatible with the character of surrounding land uses and maintains and enhances the value of surrounding properties.
- (vii) The development proposed in the PD Plan for PUD-CZ has architectural and design standards that are exceptional and provide higher quality than routine developments. All residential uses proposed in a PD Plan for PUD-CZ shall provide architectural elevations representative of the residential structures to be built to ensure the Standards of this Section are met.
- b) Off-street parking and loading. The PD Plan for PUD-CZ shall demonstrate compliance with the standards of Sec. 8.3 Off-Street Parking and Loading, except that variations from these standards may be permitted if a comprehensive parking and loading plan for the PUD-CZ is submitted as part of the PD Plan that is determined to be suitable for the PUD-CZ, and generally consistent with the intent and purpose of the off-street parking and loading standards.
- c) RCA. The PD Plan for PUD-CZ shall demonstrate compliance with Sec. 8.1.2 Resource Conservation Area, except that the percentage of RCA required under Sec. 8.1.2 may be reduced by the Town Council by no more than 10% provided that the PD Plan for PUD-CZ includes one or more of the following:
 - (i) A non-residential component;
 - (ii) An overall density of 7 residential units per acre or more; or
 - (iii) Environmental measures including but not limited to the following:
 - a. The installation of a solar photovoltaic (PV) system on a certain number or percentage of single-family or townhouse lots or on a certain number or percentage of multifamily, mixed-use, or nonresidential buildings. All required solar installation shall be completed or under construction prior to 90% of the building permits being issued for the approved number of lots or buildings. For single-family or townhouse installations, the lots on which these homes are

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- located shall be identified on the Master Subdivision Plat, which may be amended;
- b. The installation of a geothermal system for a certain number or percentage of units within the development; or
- c. Energy efficiency standards that exceed minimum Building Code requirements (i.e. SEER rating for HVAC).
- d) Landscaping. The PD Plan for PUD-CZ shall demonstrate compliance with the standards of Sec. 8.2 Landscaping, Buffering and Screening, except that variations from these standards may be permitted where it is demonstrated that the proposed landscaping sufficiently buffers uses from each other, ensures compatibility with land uses on surrounding properties, creates attractive streetscapes and parking areas and is consistent with the character of the area. In no case shall a buffer be less than one half of the width required by Sec. 8.2 or 10 feet in width, whichever is greater.
- e) Signs. Signage in the PD Plan for PUD-CZ shall demonstrate compliance with Sec. 8.7 Signs, except that the standards can be varied if a master signage plan is submitted for review and approval concurrent with the PD plan and is determined by the Town Council to be suitable for the PUD-CZ and generally consistent with the intent and purpose of the sign standards of the UDO. The master signage plan shall have design standards that are exceptional and provide for higher quality signs than those in routine developments and shall comply with Sec. 8.7.2 Prohibited Signs.
- f) Public facilities. The improvements standards and guarantees applicable to the public facilities that will serve the site shall comply with Article 7: Subdivision and Article 14: Parks, Recreation, Greenways, and Open Space.
 - (i) The PD Plan for PUD-CZ demonstrates a safe and adequate on-site transportation circulation system. The on-site transportation circulation system shall be integrated with the off-site transportation circulation system of the Town. The PD Plan for PUD-CZ shall be consistent with the Apex Transportation Plan and the *Town of Apex Standard Specifications and Standard Details* and show required right-of-way widths and road sections. A Traffic Impact Analysis (TIA) shall be required per Sec. 13.19.
 - (ii) The PD Plan for PUD-CZ demonstrates a safe and adequate on-site system of potable water and wastewater lines that can accommodate the proposed development, and are efficiently integrated into off-site potable water and wastewater public improvement plans. The PD Plan shall include a proposed water and wastewater plan.
 - (iii) Adequate off-site facilities for potable water supply, sewage disposal, solid waste disposal, electrical supply, fire protection and roads shall be planned and programmed for the development proposed in the PD Plan for PUD-CZ, and the development is conveniently located in relation to schools and police protection services.
 - (iv) The PD Plan shall demonstrate compliance with the parks and recreation requirements of Sec. Article 14: *Parks, Recreation, Greenways, and Open Space* and Sec. 7.3.1 *Privately-owned Play Lawns* if there is a residential component in the PUD-CZ.
- g) Natural resource and environmental protection. The PD Plan for PUD-CZ demonstrates compliance with the current regulatory standards of this Ordinance related to natural resource and environmental protection in Sec. 6.1 Watershed Protection Overlay District, Sec. 6.2 Flood Damage Prevention Overlay District, and Sec. 8.1 Resource Conservation.
- h) Storm water management. The PD Plan shall demonstrate that the post-development rate of on-site storm water discharge from the entire site shall not exceed pre-development levels in accordance with Sec. 6.1.7 of the UDO.

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- i) Phasing. The PD Plan for PUD-CZ shall include a phasing plan for the development. If development of the PUD-CZ is proposed to occur in more than one phase, then guarantees shall be provided that project improvements and amenities that are necessary and desirable for residents of the project, or that are of benefit to the Town, are constructed with the first phase of the project, or, if this is not possible, then as early in the project as is technically feasible.
- j) Consistency with 2045 Land Use Map. The PD Plan for PUD-CZ demonstrates consistency with the goals and policies established in the Town's 2045 Land Use.
- k) Complies with the UDO. The PD Plan for PUD-CZ demonstrates compliance with all other relevant portions of the UDO.

Legislative Considerations

The Town Council shall find the PUD-CZ designation demonstrates compliance with the following standards. Sec. 2.3.3.F:

The applicant shall propose site-specific standards and conditions that take into account the following considerations, which are considerations that are relevant to the legislative determination of whether or not the proposed conditional zoning district rezoning request is in the public interest. These considerations do not exclude the legislative consideration of any other factor that is relevant to the public interest.

- 1) Consistency with 2045 Land Use Map. The proposed Conditional Zoning (CZ) District use's appropriateness for its proposed location and consistency with the purposes, goals, objectives, and policies of the 2045 Land Use Map.
- 2) Compatibility. The proposed Conditional Zoning (CZ) District use's appropriateness for its proposed location and compatibility with the character of surrounding land uses.
- 3) Zoning district supplemental standards. The proposed Conditional Zoning (CZ) District use's compliance with Sec. 4.4 Supplemental Standards, if applicable.
- 4) Design minimizes adverse impact. The design of the proposed Conditional Zoning (CZ) District use's minimization of adverse effects, including visual impact of the proposed use on adjacent lands; and avoidance of significant adverse impacts on surrounding lands regarding trash, traffic, service delivery, parking and loading, odors, noise, glare, and vibration and not create a nuisance.
- 5) Design minimizes environmental impact. The proposed Conditional Zoning District use's minimization of environmental impacts and protection from significant deterioration of water and air resources, wildlife habitat, scenic resources, and other natural resources.
- 6) Impact on public facilities. The proposed Conditional Zoning (CZ) District use's avoidance of having adverse impacts on public facilities and services, including roads, potable water and wastewater facilities, parks, schools, police, fire and EMS facilities.
- 7) Health, safety, and welfare. The proposed Conditional Zoning (CZ) District use's effect on the health, safety, or welfare of the residents of the Town or its ETJ.
- 8) Detrimental to adjacent properties. Whether the proposed Conditional Zoning (CZ) District use is substantially detrimental to adjacent properties.

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- 9) Not constitute nuisance or hazard. Whether the proposed Conditional Zoning (CZ) District use constitutes a nuisance or hazard due to traffic impact or noise, or because of the number of persons who will be using the Conditional Zoning (CZ) District use.
- 10) Other relevant standards of this Ordinance. Whether the proposed Conditional Zoning (CZ) District use complies with all standards imposed on it by all other applicable provisions of this Ordinance for use, layout, and general development characteristics.



March 18, 2025

Rynal Stephenson, PE DRMP 5808 Faringdon PI., Suite 100 Raleigh, NC 27609

Subject: Staff summary and comments for West Village Commercial Phasing TIA,

01/30/2025

Mr. Stephenson:

Please review the following comments and recommendations on your traffic impact analysis (TIA). You may schedule a meeting with me and your client to discuss at your convenience.

Study Area

This phasing analysis studied access to the proposed development via one (1) site driveway:

Kelly Road and Proposed Site Driveway (Site Driveway #4 in the 2016 TIA)

Additionally, the following four (4) intersections were studied in the analysis:

- Kelly Road and Olive Chapel Road
- Kelly Road and Apex Barbeque Road
- Kelly Road and Leo Drive
- Kelly Road and Old US 1

Trip Generation

This phasing study analyzes the initial commercial phase of the West Village development as well as the Full Build Out of the development. The initial commercial phase, to be completed in 2028, is proposed to consist of:

- 12,130 s.f. daycare
- 14,500 s.f. of retail
- 14,500 s.f. of office space

Traffic generation for the initial commercial phase is 1,628 new daily trips with 113 inbound trips and 74 outbound trips in the A.M. peak hour and 97 inbound trips and 128 outbound trips in the P.M. peak hour. These trips are new trips assigned to the external roadway network.

The Full Build, to be completed in 2030, is proposed to consist of:

- 200,000 s.f. of office space
- 255,000 s.f. of retail and restaurant
- Bank with 8 drive-thru windows
- 20,000 s.f. daycare
- Gas Station with Convenience Store 16 pumps

Traffic generation at full build-out on the external roadway network is 23,761 new daily trips with 1,279 inbound trips and 888 outbound trips in the A.M. peak hour and 1,400 inbound trips and 1,590 outbound trips in the P.M. peak hour.

Trip Distribution, Assignment, and Growth

Retail and Office Trips:

- 10% north of Olive Chapel Road on Kelly Road
- 15% east and 5% west of Kelly Road on Olive Chapel Road
- 10% east and 10% west of Kelly Road on Apex Barbecue Road
- 10% north and 10% south on NC 540
- 15% east on Old US 1
- 15% west on Old US 1

Residential Phase (single-family and townhome) trips:

- 15% north of Olive Chapel Road on Kelly Road
- 10% east of Kelly Road on Olive Chapel Road
- 10% west of Kelly Road on Apex Barbecue Road
- 25% north and 20% south on NC 540
- 15% east on Old US 1
- 5% west on Old US 1

The TIA included traffic from four approved background developments in addition to 4% annual background growth.

- Depot 499
- Townes at Pleasant Park (fka Sears Property)
- Friendship Village
- Holland Road Assembly

Traffic Capacity Analysis and Recommendations

The following table shows the intersections that were included in the original West Village TIA. Some intersections were not analyzed in this phased analysis as improvements were unlikely to change based on traffic patterns. The table shows the improvements at each intersection that are identified as part of the zoning conditions for the West Village PUD but have not yet been constructed.

Intersection	Remaining Improvements per West Village PUD Zoning Conditions (not yet constructed)
Kelly Rd at Olive Chapel Rd	Construct 200' EB right-turn lane Construct an additional 300' WB left-turn lane (with SB receiving through lane on Kelly Rd)
Kelly Rd at Apex Barbecue Rd	None remaining
Kelly Rd at Southwinds Run	 Construct 100' NB left-turn lane Construct a second NB through lane through the intersection and drop as right-turn lane at Site Drive #1 (Eva Pearl Dr)
(not analyzed in phasing study)	Begin an additional SB through lane immediately south of Southwinds Run
Kelly Rd at Old US 1	 Convert the intersection to a right-in/right-out Construct an additional WB through lane, beginning at NC 540 SB off-ramp as a free-flow right turn existing the ramp Construct 200' WB right-turn lane
NC 540 NB Ramps at Old US 1	
(not analyzed in phasing study)	None remaining
NC 540 SB Ramps at Old US 1 (not analyzed in phasing	 Construct free-flow right-turn lane for the NC 540 SB off-ramp Construct additional receiving through lane continuing west to drop as right-turn lane Site Drive #6 (Pleasant Plans Rd)
study) Humie Olive Rd at Old US 1	
(not analyzed in phasing study)	None remaining
Kelly Rd at Eva Pearl Dr (fka Site Drive #1)	Construct an additional NB through lane to drop as right-turn lane at Site Drive #1 (Eva Pearl Dr)
(not analyzed in phasing study)	
Kelly Rd at Gaither St (fka Site Drive #2)	 Construct an additional NB through lane Construct an additional SB through lane
(not analyzed in phasing study)	
Kelly Rd at Leo Dr (fka Site Drive #3)	 Construct an additional NB through lane Construct an additional SB through lane

Table continued on next page.

Table continued from previous page.

Intersection	Remaining Improvements per West Village PUD Zoning Conditions (not yet constructed)
Kelly Rd at Site Driveway (fka Site Drive #4)	Construct and dedicate 'Road A' from the roundabout at Boyette St and Pleasant Plains Rd to Kelly Rd at Site Drive #4 Construct roundabout on Kelly Rd at Site Drive #4 (Site Driveway)
Old US 1 at Boyette St (fka Site Drive #5) (not analyzed in phasing study)	 Convert Boyette St to a full-movement intersection Construct dual 200' SB left-turn lanes Construct 200' SB right-turn lane Construct an additional WB through lane Construct 200' WB right-turn lane Construct 300' EB left-turn lane Construct additional EB through lane, dropping 1000' east of the intersection
Old US 1 at Pleasant Plains Rd (/fka Site Drive #6) (not analyzed in phasing study)	 Construct an additional WB through lane to drop as a right-turn lane at Site Drive #6 (Pleasant Plains Rd) Construct an additional EB through lane beginning 400' west of Site Drive #6 (Pleasant Plains Rd)

The initial commercial phase is projected to generate 1,628 daily trips and the total commercial portion of the West Village development is projected to generate 23,761 new daily trips. The initial commercial phase is approximately 7% of the total commercial development planned.

Level of Service (LOS) is a grade of A through F assigned to an intersection, approach, or movement to describe how well or how poorly it operates. LOS A through D is considered acceptable for peak hour operation. LOS E or F describes potentially unacceptable operation, and developers may be required to mitigate their anticipated traffic impact to improve LOS based on the Apex Unified Development Ordinance (UDO).

Tables 1 through 5 describe the levels of service (LOS) for the scenarios analyzed in the TIA. "*NA*" is shown when the scenario does not apply. "*Free*" indicates when an approach is in free-flow and there is no LOS metric for the approach. The scenarios are as follows:

- Existing 2024 Existing year 2024 traffic.
- No Build 2028 Projected year (2028) with background traffic growth and background development.
- **Build 2028** Projected year (2028) with background traffic, and initial commercial phase build-out, including recommended improvements where applicable.
- No Build 2030 Projected year (2030) with background traffic growth and background development.
- **Build 2030** Projected year (2030) with background traffic, and full build-out, including recommended improvements where applicable.

Kelly Road at Site Driveway (Site Drive #4 in original TIA)

Table 1. A.M./P.M. Peak Hour Levels of Service Kelly Road at Site Driveway						
	Build 2028 (Unsignalized)	Build 2030 (Roundabout)				
<u>Overall</u>	<u>NA</u>	<u>B/F</u>				
Site Drive #4 (Eastbound)	NA	B/F				
Site Driveway (Westbound)	B / C ¹	A/C				
Kelly Road (Northbound) Free B / D						
Kelly Road (Southbound)	A / A ²	A/B				

- 1. Level of service for stop-controlled minor street approaches.
- 2. Level of service for left turn movements on free-flowing approaches.

TIA recommendations:

Initial Commercial Phase (Build 2028)

- The site driveway will be constructed with one ingress lane and one egress lane, with stop control for the site driveway approach.
- Southbound left-turn lane with a minimum 75 feet full width storage plus appropriate taper.
- A northbound right-turn lane with 50 feet of full width storage is marginally warranted but not recommended due to future plans to construct a roundabout at this location.

Full Build (Build 2030) – Improvements per original West Village PUD Requirements

- Construction of a roundabout.
- Construct an additional northbound receiving lane and terminate the second southbound lane as an exclusive right-turn lane to make Kelly Road a four-lane median-divided roadway north of the intersection; 2 lane roadway remains south of intersection.
- Site Driveway 4 (EB and WB approaches) constructed with one ingress lane and one egress lane, each.

Apex staff recommendations:

Staff concurs with the operational results of the TIA. Under Build 2028 conditions the site driveway is expected to operate acceptably as proposed. It should be noted that the Apex Transportation Plan identifies a roundabout at this location on Kelly Road, to tie in with an eastbound major collector street. Additionally, the Transportation Plan shows widening of Kelly Road from this intersection north to a four lane, median divided roadway.

In addition to the southbound left-turn lane at the Site Driveway, staff recommends a proportional fee in lieu for the future roundabout at the Site Driveway which is required to be constructed during the development of POD 3, west of Kelly Road.

Kelly Road at Leo Drive (Site Drive #3 in original TIA)

Table 2. A.M./P.M. Peak Hour Unsignalized Levels of Service Kelly Road and Leo Drive						
Existing No Build Build No Build Build 2024 2028 2028 2030 203						
<u>Overall</u>	<u>NA</u>	<u>NA</u>	<u>NA</u>	<u>NA</u>	<u>NA</u>	
Leo Drive (Eastbound)	B / B ¹	B/C ¹	B / C ¹	B / C ¹	E/F¹	
Leo Drive (Westbound)	B / B ¹	B / B ¹	B / C ¹	B / C ¹	D/F¹	
Kelly Road (Northbound)	A / A ²	A / B ²				
Kelly Road (Southbound)	A / A ²	A / B ²				

- 1. Level of service for stop-controlled minor street approaches.
- 2. Level of service for left turn movements on free-flowing approaches.

TIA recommendations:

Initial Commercial Phase (Build 2028)

• No improvements are recommended for the developer under Build 2028 conditions.

Full Build (Build 2030) – Improvements per original West Village PUD Requirements

 Construct additional northbound and southbound through lanes to make Kelly Road a four-lane median-divided roadway.

Apex staff recommendations:

Staff concurs with the TIA. The site trips for the first commercial phase of the West Village development will only add to the northbound and southbound through movements at this intersection. Under Build 2030 conditions, the eastbound and westbound approaches are anticipated to operate at LOS D or worse during both the AM and PM peak hours. However, the queues are between one and three vehicles long on these approaches. The poor level of service is mainly due to the increase in volumes along Kelly Road, reducing gaps available for minor approach traffic.

Kelly Road at Olive Chapel Road

Table 3. A.M./P.M. Peak Hour Signalized Levels of Service Kelly Road at Olive Chapel Road						
	Existing 2024	No Build 2028	Build 2028	No Build 2030	Build 2030	
Overall	<u>C/D</u>	<u>E/E</u>	<u>E/E</u>	<u>E/F</u>	<u>E/F</u>	
Olive Chapel Road (Eastbound)	D/E	D/F	D/F	D/F	E/F	
Olive Chapel Road (Westbound)	C/C	C/C	C/C	C/C	D/E	
Kelly Road D/D F/F F/F D/					D/F	
Kelly Road (Southbound)	C/C	C/D	D/D	D/F	E/E	

TIA recommendations:

Initial Commercial Phase (Build 2028)

No improvements are recommended for the developer under Build 2028 conditions.

Full Build (Build 2030) - Improvements per original West Village PUD Requirements

- Construct an additional westbound left-turn lane
- Construct an additional southbound receiving lane south of intersection on Kelly Road
- Construct an exclusive eastbound right-turn lane
- Signal phasing and timing adjustments to accommodate new lanes

Apex staff recommendations:

Staff concurs with the TIA findings based on the UDO thresholds for this site by itself, but notes that the site is still a portion of the build-out of the commercial phase of the PUD which has a greater impact as a whole and would still be committed to providing future improvements as noted. Staff recommends a proportionate share of a fee in lieu toward the future improvements for Build 2030 be committed by the Build 2028 phase.

The site trips for the first commercial phase of the West Village development account for less than 3% of the overall traffic at this intersection; therefore, no improvements are recommended for construction in Build 2028. With the improvements for Full Build in place, the intersection is expected to still operate at an overall LOS E during the AM peak hour and LOS F during the PM peak hour, but with less delay than under No Build 2030 conditions.

Kelly Road at Apex Barbecue Road

Table 4. A.M./P.M. Peak Hour Signalized Levels of Service Kelly Road at Apex Barbecue Road						
Existing No Build Build No Build Build 2024 2028 2028 2030 2030						
<u>Overall</u>	<u>C/C</u>	<u>C/C</u>	<u>C/D</u>	<u>D/E</u>	<u>F/F</u>	
Apex Barbecue Road (Eastbound)	B/B	B/B	B/B	C/C	C/C	
Apex Barbecue Road (Westbound)	C/C	C/C	C/C	C/C	C/C	
Kelly Road (Northbound)	C/C	D/C	D/D	D/D	F/F	
Kelly Road (Southbound)	C/C	C/D	C/D	D/F	F/F	

TIA recommendations:

Initial Commercial Phase (Build 2028)

No improvements are recommended for the developer under Build 2028 conditions.

Full Build (Build 2030) – Improvements per original West Village PUD Requirements

 Improvements for this intersection identified in the original TIA for full build conditions have been constructed.

Apex staff recommendations:

Staff concurs with the TIA. The signal is expected to operate at LOS D or better under Build 2028 conditions. The site trips for the first commercial phase of the West Village development account for approximately 6% of the overall traffic at this intersection. However, it should be noted that Build 2030 is extending beyond the build year for the original TIA assumptions that resulted in those recommendations, and the newly projected build out shows overall LOS F and Kelly Road approaches at LOS F. Therefore, staff recommend a reevaluation of recommendations to accommodate Build 2030 when that future site plan is proposed.

Old US 1 at Kelly Road

Table 5. A.M./P.M. Peak Hour Unsignalized Levels of Service Old US 1 at Kelly Road							
Existing 2024 No Build 2028 No Build 2030 Right-in/Right-out)							
<u>Overall</u>	<u>NA</u>	<u>NA</u>	<u>NA</u>	<u>NA</u>	<u>NA</u>		
Old US 1 (Eastbound)	A / B ²	B / B ²	B / C ²	B / C ²	NA		
Old US 1 (Westbound)	Free	Free	Free	Free	Free		
Kelly Road (Southbound)	F/F ¹	F/F ¹	F/F ¹	F/F¹	D / F ¹		

- 2. Level of service for stop-controlled minor street approaches.
- 2. Level of service for left turn movements on free-flowing approaches.

TIA recommendations:

Initial Commercial Phase (Build 2028)

- No improvements are recommended for the developer under Build 2028 conditions.
 - The TIA acknowledges a signal was reviewed, but NCDOT commented they are unlikely to approve one due to proximity to NC 540.

Full Build (Build 2030) – Improvements per original West Village PUD Requirements

 Under Build 2030 conditions this intersection will be converted to a right-in/right-out when a new connection is made between Kelly Road and Old US 1 to the west via a separate portion of the overall West Village development.

Apex staff recommendations:

Staff concurs with the TIA given NCDOT was not in favor of signalization due to proximity to NC 540 or constructing access restrictions at this time. Under Build 2028 conditions, the Kelly Road approach is anticipated to continue to operate at LOS F during both the AM and PM peak hours. The southbound left-turn queue increases by approximately 4 vehicles during the AM peak hour and 6 vehicles during the PM peak hour. Site traffic for the initial commercial phase is expected to be 7% or less of the existing 2024 traffic at this intersection; therefore, no improvements are required.

Under Build 2030 conditions, this intersection will be converted to a right-in/right-out with the new internal street connection between the roundabout on Kelly Road and the one that was previously constructed in West Village according to the zoning conditions. This connection will provide access from Kelly Road at the planned roundabout (Site Drive #4/Site Driveway) to a full movement, signalized intersection at Old US 1.

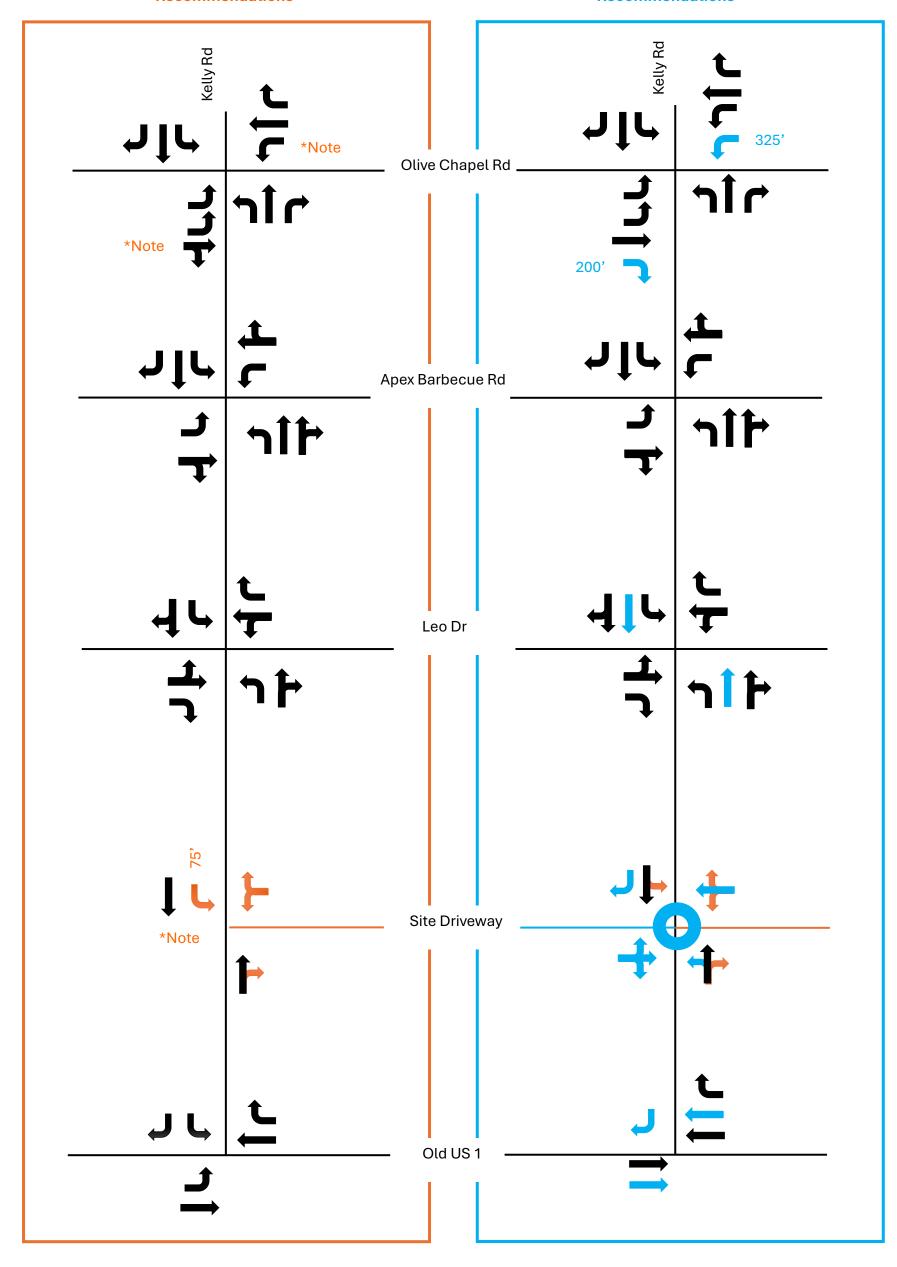
Please coordinate with the NCDOT District Engineer's Office concerning any recommendations on NCDOT facilities. Town staff will be available for meetings to discuss recommendations as needed.

Sincerely,

Jessica McClure, PE

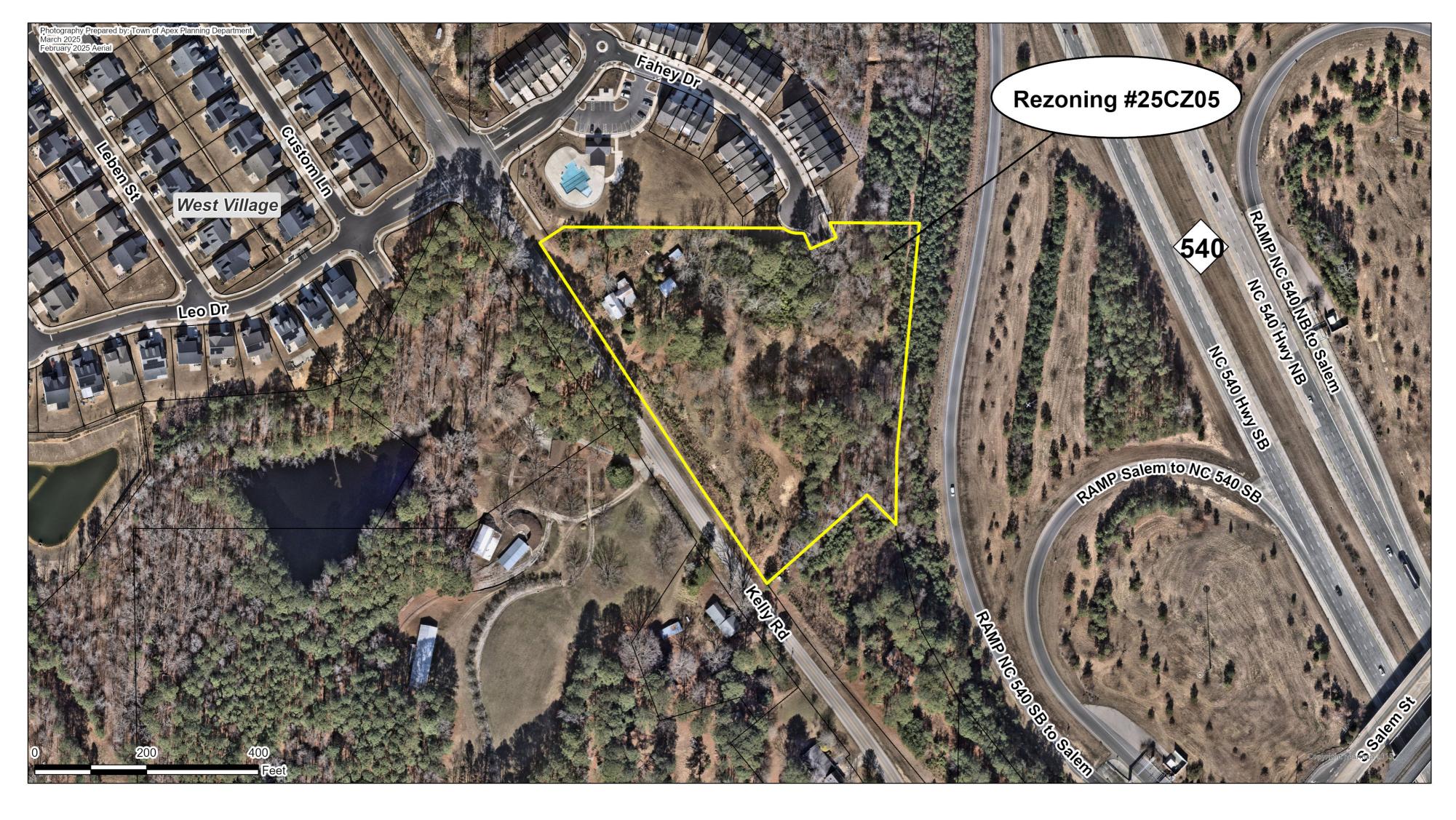
Jose Men

919-372-7448



^{*}Note – developer to pay proportional fee in lieu for future improvements as noted in West Village Commercial Phasing Study Review Letter (03/18/25)





PLANNED UNIT DEVELOPMENT APPLICATION This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties. 4-25-2025 25CZ05 Application #: Submittal Date: \$ Fee Paid Check # PETITION TO AMEND THE OFFICIAL ZONING DISTRICT MAP West Village PUD Amendment Project Name: 2517 Kelly Road (West Village PUD POD 1) Address(es): 0731434504 PIN(s) 5.8591 Acreage: **PUD-CZ** PUD-CZ w/ amended conditions Current Zoning: **Proposed Zoning:** Community Mixed Use (CMU); High Density Residential/Office Employment/Commercial Services Current 2045 LUM Designation: Is the proposed rezoning consistent with the 2045 LUM Classification(s)? No \square Yes If any portion of the project is shown as mixed use (3 or more stripes on the 2045 Land Use Map) provide the following: 5.8591 Acreage: Area classified as mixed use: 5.8591 Area proposed as non-residential development: Acreage: 100% Percent of mixed use area proposed as non-residential: Percent: **Applicant Information** Trilandco, LLC, a North Carolina limited liability company Name: 4400 Triland Way Address: NC 27518 Cary Zip: City: State: c/o Matthew J. Carpenter; 919-835-4032 E-mail: rhamad@trilandproperty.com Phone: **Owner Information** Fahey Family Farm, LLC, a North Carolina limited liability company Name: 1115 Capitata Xing Address: NC Apex 27502 Citv: State: Zip: N/A N/A Phone: E-mail: **Agent Information** Matthew J. Carpenter Name: 301 Fayetteville Street, Suite 1400 Address: NC Raleigh 27601 City: State: Zip: MatthewCarpenter@parkerpoe.com 919-835-4032 E-mail: Phone:

Other contacts:

Jeff Roach; Peak Engineering; jroach@peakengineering.com; 919-439-0100

Rynal Stephenson; DRMP; rstephenson@drmp.com

PLANNED UNIT DEVELOPMENT APPLICATION

Application #: 25CZ05 Submittal Date: 4-25-2025

PLANNED UNIT DEVELOPMENT DISTRICT STANDARDS:

In return for greater flexibility in site design requirements, Planned Development (PD) Districts are expected to deliver exceptional quality community designs that preserve critical environmental resources; provide high quality community amenities; incorporate creative design in the layout of buildings, Resource Conservation Area and circulation; ensure compatibility with surrounding land uses and neighborhood character; provide high quality architecture; and provide greater efficiency in the layout and provision of roads, utilities, and other infrastructure. The Planned Development (PD) Districts shall not be used as a means of circumventing the Town's adopted land development regulations for routine developments. The PD text and plan should demonstrate how the standards of Sec. 2.3.4.F are met be the proposed rezoning.

LEGISLATIVE CONSIDERATIONS - CONDITIONAL ZONING

The applicant shall propose site-specific standards and conditions that take into account the following considerations, which are considerations that are relevant to the legislative determination of whether or not the proposed conditional zoning district rezoning request is in the public interest. These considerations do not exclude the legislative consideration of any other factor that is relevant to the public interest. Use additional pages as needed.

1) Consistency with 2045 Land Use Map. The proposed Conditional Zoning (CZ) District use's appropriateness for its proposed location and consistency with the purposes, goals, objectives, and policies of the 2045 Land Use Map.

The proposed PUD Amendment is consistent with the property's Community Mixed-Use LUM designation which calls for High Density Residential, Office Employment, and Commercial Services. The existing West Village PUD permits a variety of residential, office, and commercial uses on the property and this PUD Amendment does not propose changes to permitted uses. Rather, it proposes revisions to the phasing of transportation infrastructure improvements to facilitate the development of non-residential uses on the property as envisioned by the LUM and West Village PUD.

2) Compatibility. The proposed Conditional Zoning (CZ) District use's appropriateness for its proposed location and compatibility with the character of surrounding land uses.

The PUD Amendment does not propose any changes to permitted uses on the property which are set forth in the previously approved West Village PUD and consistent with the LUM. The proposed small-scale non-residential development will act as a transition between the 460,000 sf of non-residential entitlement west of Kelly Road and existing residential neighborhoods to the north.

3) Zoning district supplemental standards. The proposed Conditional Zoning (CZ) District use's compliance with Sec 4.4 *Supplemental Standards*, if applicable.

The proposed development will comply with all applicable Supplemental Standards in UDO Section 4.4.

PETITION PROCESS INFORMATION

4) Design minimizes adverse impact. The design of the proposed Conditional Zoning (CZ) District use's minimization of adverse effects, including visual impact of the proposed use on adjacent lands; and avoidance of significant adverse impacts on surrounding lands regarding trash, traffic, service delivery, parking and loading, odors, noise, glare, and vibration and not create a nuisance.

The PUD will follow design guidelines included in the original PUD and all UDO standards for trash, traffic, service delivery, parking and loading, odors, noise, glare, and vibration. Traffic impacts will be mitigated by transportation infrastructure improvements detailed in the revised Public Facilities section of the PUD.

5) Design minimizes environmental impact. The proposed Conditional Zoning District use's minimization of environmental impacts and protection from significant deterioration of water and air resources, wildlife habitat, scenic resources, and other natural resources.

The amended PUD will not have adverse impacts on natural resources. The site does not have streams or other environmentally sensitive areas and the amendments do not propose any changes to existing environmental conditions in the West Village PUD.

6) *Impact on public facilities.* The proposed Conditional Zoning (CZ) District use's avoidance of having adverse impacts on public facilities and services, including roads, potable water and wastewater facilities, parks, schools, police, fire and EMS facilities.

The amended PUD will not have adverse impacts on public facilities and services. The site is in the Town's ETJ, will be annexed prior to construction, and will connect to Town water and sewer services. The PUD does not permit residential uses on the property, so there will be minimal impact on parks and schools. Police, fire, and EMS facilities in the area are sufficient and the site will provide safe and efficient access for emergency service vehicles. The PUD includes significant road improvements and this PUD Amendment phases those improvements to correlate with planned non-residential phases.

7) Health, safety, and welfare. The proposed Conditional Zoning (CZ) District use's effect on the health, safety, or welfare of the residents of the Town or its ETJ.

The PUD amendment will improve the health, safety, and welfare of residents of the Town by facilitating the development of neighborhood serving non-residential uses on the property and completing necessary infrastructure improvements.

8) Detrimental to adjacent properties. Whether the proposed Conditional Zoning (CZ) District use is substantially detrimental to adjacent properties.

The proposed uses will not be substantially detrimental to adjacent properties. As discussed above, the site is designated for non-residential uses on the LUM and entitled for non-residential uses by the West Village PUD. The project will meet all UDO standards for noise and lighting to ensure compatibility with the adjacent residential neighborhood.

Last Updated: June 27, 2024

PETITION PROCESS INFORMATION

9) Not constitute nuisance or hazard. Whether the proposed Conditional Zoning (CZ) District use constitutes a nuisance or hazard due to traffic impact or noise, or because of the number of persons who will be using the Conditional Zoning (CZ) District use.

The proposed non-residential uses will not constitute a nuisance or hazard. The site is located in a planned commercial corridor adjacent to the 540/S Salem Street interchange.

10) Other relevant standards of this Ordinance. Whether the proposed Conditional Zoning (CZ) District use complies with all standards imposed on it by all other applicable provisions of this Ordinance for use, layout, and general development characteristics.

The amended PUD complies with all standards imposed on it by all other applicable provisions of the Ordinance.

DEVELOPMENT NAME APPROVAL APPLICATION

Application #:	25CZ05	Submittal Date:	4-25-2025	
Fee for Initial Sub	omittal: No Charge	Fee for Name Chang	e after Approval: \$500*	

Purpose

To provide a consistent and clearly stated procedure for the naming of subdivisions and/or developments and entrance roadways (in conjunction with *Town of Apex Address Policy*) so as to allow developers to define and associate the theme or aesthetics of their project(s) while maintaining the Town's commitment to preserving the quality of life and safety for all residents of Apex proper and extraterritorial jurisdiction.

Guidelines

- ✓ The subdivision/development name shall not duplicate, resemble, or present confusion with an existing subdivision/development within Apex corporate limits or extraterritorial jurisdiction except for the extension of an existing subdivision/development of similar or same name that shares a continuous roadway.
- ✓ The subdivision/development name shall not resemble an existing street name within Apex corporate limits or extraterritorial jurisdiction unless the roadway is a part of the subdivision/development or provides access to the main entrance.
- ✓ The entrance roadway of a proposed subdivision/development shall contain the name of the subdivision/development where this name does not conflict with the Town of Apex Road Name Approval Application and Town of Apex Address Policy guidelines.
- √ The name "Apex" shall be excluded from any new subdivision/development name.
- ✓ Descriptive words that are commonly used by existing developments will be scrutinized more seriously in order to limit confusion and encourage distinctiveness. A list of commonly used descriptive words in Apex's jurisdiction is found below.
- ✓ The proposed subdivision/development name must be requested, reviewed and approved during preliminary review by the Town.
- ✓ A \$500.00 fee will be assessed to the developer if a subdivision/development name change is requested after official submittal of the project to the Town.*

*The imposed fee offsets the cost of administrative changes required to alleviate any confusion for the applicant, Planning staff, other Town departments, decision-making bodies, concerned utility companies and other interested parties. There is no charge for the initial name submittal.

Existing Development Titles, Recurring

	Residential	Non-Residential
10 or more	Creek, Farm(s), Village(s),	Center/Centre
6 to 9	Crossing(s), Park, Ridge, Wood(s)	Commons, Park
3 to 5	Acres, Estates, Glen(s), Green*, Hills	Crossing(s), Plaza, Station, Village(s)

^{*}excludes names with Green Level

Application #:	25CZ05	Sub	mittal Date:	4-25-2025				
Proposed Subdivi	Proposed Subdivision/Development Information							
Description of loca	ation: 2517 Kelly Road							
Nearest intersecti	ng roads: Kelly Road/S Salem	Street						
Wake County PIN((s): 0731434504							
Township: White								
Contact Informati	ion (as appropriate)							
Contact person:	Rehab Hamad; Trilandco, LLC							
Phone number: 2	c/o Matthew J. Carpenter; 919-835-403	Fax number:	N/A					
Address: 4400 Tr	iland Way, Cary, NC 27518							
E-mail address:	hamad@trilandproperty.com							
Owner: Fahey F	Family Farm, LLC							
Phone number:	N/A	Fax number:	N/A					
Address: 1115 Ca	apitata Xing, Apex, NC 27502							
E-mail address:	I/A							
_								
Proposed Subdivi	sion/Development Name							
1 st Choice: TBD								
2 nd Choice <i>(Optior</i>	nal): TBD							
Town of Apex Sta	ff Approval:							
Town of Apex Plar	nning Department Staff			Date				

DEVELOPMENT NAME APPROVAL APPLICATION

STREET NAME APPROVAL APPLICATION

Application #:	25CZ05	Submittal Date:	4-25-2025
Wake County Ap	proval Date:		

Guidelines:

- No names duplicating or sounding similar to existing road names
- Avoid difficult to pronounce names
- No individuals' names
- · Avoid proper names of a business, e.g. Hannaford Drive
- Limit names to 14 characters in length
- No directionals, e.g. North, South, East, West
- No punctuation marks, e.g. periods, hyphens, apostrophes, etc.
- Avoid using double suffixes, e.g. Deer Path Lane
- All names must have an acceptable suffix, e.g. Street, Court, Lane, Path, etc.
- Use only suffixes which are Town of Apex approved
- Town of Apex has the right to deny any street name that is determined to be inappropriate

Information:				
Description of location: 2517 Kelly Road				
Nearest intersecting roads: Kelly Road/S Salem Street				
Wake County PIN(s): 0731434504				
Township: White Oak				
Contact information (as appropriate)				
Contact person: Rehab Hamad; Trilandco, LLC				
c/o Matthew J. Carpenter; 919-835-403 Phone number: 2	Fax number: N/A			
Address: 4400 Triland Way, Cary, NC 27518				
E-mail address: rhamad@trilandproperty.com				
Owner: Fahey Family Farm, LLC				
Phone number: N/A	Fax number: N/A			
Address: 1115 Capaitata Xing, Apex, NC 27502				
E-mail address: N/A				

STREET NAME APPROVAL APPLICATION 25CZ05 4-25-2025 Application #: Submittal Date: # of roads to be named: NONE Please submit twice as many road names as needed, with preferred names listed first. Proposed road names should be written exactly as one would want them to appear. Town of Apex Planning Department staff will send all approved street names to the Wake County GIS Department for county approval. Please allow several weeks for approval. Upon approval Wake County GIS – Street Addressing will inform you of the approved street names. Example: Road Name Suffix Hunter Street 11 2 ______ 12 _____ 3 13 4 ______ 14 _____ 5 15 6 ______ 16 _____ 7 ______ 17 _____ 8 18 19 20 ____ TOWN OF APEX STAFF APPROVAL

Town of Apex Staff Approval		Date	2		
WAKE COUNTY STAFF APPROVAL: GIS certifies that na Please disregard all other names.	ames indicated by checkmark	Ø	are approved.		
Comments:					
Wake County GIS Staff Approval		Da	ite		
			ι	Last Updated: June 13, 20	16

TOWN OF APEX UTILITIES OFFER AND AGREEMENT

4-25-2025 25CZ05 Application #: Submittal Date: **Town of Apex** 73 Hunter Street P.O. Box 250 Apex, NC 27502 919-249-3400 WAKE COUNTY, NORTH CAROLINA CUSTOMER SELECTION AGREEMENT 2517 Kelly Road (the "Premises") The Town of Apex offers to provide you with electric utilities on the terms described in this Offer & Agreement. If you accept the Town's offer, please fill in the blanks on this form and sign and we will have an Agreement once signed by the Town. Trilandco, LLC __, the undersigned customer ("Customer") hereby irrevocably chooses and selects the Town of Apex (the "Town") as the permanent electric supplier for the Premises. Permanent service to the Premises will be preceded by temporary service if needed. The sale, delivery, and use of electric power by Customer at the Premises shall be subject to, and in accordance with, all the terms and conditions of the Town's service regulations, policies, procedures and the Code of Ordinances of the Town. Customer understands that the Town, based upon this Agreement, will take action and expend funds to provide the requested service. By signing this Agreement the undersigned signifies that he or she has the authority to select the electric service provider, for both permanent and temporary power, for the Premises identified above. Any additional terms and conditions to this Agreement are attached as Appendix 1. If no appendix is attached this Agreement constitutes the entire agreement of the parties. Acceptance of this Agreement by the Town constitutes a binding contract to purchase and sell electric power. Please note that under North Carolina General Statute §160A-332, you may be entitled to choose another electric supplier for the Premises. Upon acceptance of this Agreement, the Town of Apex Electric Utilities Division will be pleased to provide electric service to the Premises and looks forward to working with you and the owner(s). ACCEPTED: Trilandco, LLC **CUSTOMER: TOWN OF APEX** BY: BY: **Authorized Agent** DATE: DATE:

AGENT AUTHORIZA	TION FORM				
Application #:	25CZ05	Submittal Date: _	4-25-2025		
FAHEY FAMILY FAR	M, LLC	is the owner* of the pro	operty for which the attached		
application is being s	submitted:				
	For Conditional Zoning and Planne authorization includes express co Agent which will apply if the appl	nsent to zoning condition	applications, this s that are agreed to by the		
Site Plan					
Subdivision	n				
Variance					
Other:					
The property address is: 2517 Kelly Road, Apex, NC 27502; PIN 0731434504					
The agent for this project is: Matthew J. Carpenter and Rehab Hamad					
☐ I am the	☐ I am the owner of the property and will be acting as my own agent				
Agent Name:	Matthew J. Carpenter and Rel	hab Hamad			
Address:	301 Fayetteville Street, Suite	1400, Raleigh, NC 27601			
Telephone Number:	919-835-4032				
E-Mail Address:	MatthewCarpenter@parkerpo	e.com			
	Signature(s) of Owner(s)* FAHEY FAMILY FARM, LLC, a North Carolina limited liab	pility company			
	By: Patrick > 7	taken			

Attach additional sheets if there are additional owners.

*Owner of record as shown on the latest equalized assessment rolls of Wake County. An option to purchase does not constitute ownership. If ownership has been recently transferred, a copy of the deed must accompany this authorization.

Name: PATRICK S. FAMEY Title: MANAGER

Pursuant to Article 40 of Chapter 66 of the North Carolina General Statutes (the Uniform Electronic Transactions Act) this application and all documents related hereto containing an electronic or digitized signature are legally binding in the same manner as are hard copy documents executed by hand signature. The parties hereby consent to use electronic or digitized signatures in accordance with the Town's Electronic Signature Policy and intend to be bound by the application and any related documents. If electronic signatures are used the application shall be delivered in an electronic record capable of retention by the recipient at the time of receipt.

AFF	IDAVIT OF OW	/NERSHIP				- X	
App	olication #:	25CZ05	Subm	ittal Date:	4-25-2025		
	undersigned, _ rs or affirms as		TAHEY (the	"Affiant")	first being duly	sworn, hereby	
1.	owner, or 2517 Kelly Roa	rer eighteen (18) years o is the authorized d, Apex, NC 27502; PIN 0731434 d herein (the "Property")	agent of all ov ⁵⁰⁴ and legally de	vners, of		located at	
2.	This Affidavi	t of Ownership is made fo Apex.	or the purpose of filing a	n applicatio	n for developmen	approval with	
3.	If Affiant is the owner of the Property, Affiant acquired ownership by deed, dated 3/6/2018 and recorded in the Wake County Register of Deeds Office on 3/6/2018 in Book 17062 Page 1716						
4.	indicating th	the authorized agent one agency relationship grather the owner(s).					
If Affiant is the owner of the Property, from the time Affiant was deeded the Property 3/6/2018, Affiant has claimed sole ownership of the Property. Affiant or Affiant's preside in interest have been in sole and undisturbed possession and use of the property during the ownership. Since taking possession of the Property on 3/6/2018, no one has defiant's ownership or right to possession nor demanded any rents or profits. To Affiant's know claim or action has been brought against Affiant (if Affiant is the owner), or against owner(s) (if acting as an authorized agent for owner(s)), which questions title or right to possession of the nor is any claim or action pending against Affiant or owner(s) in court regarding possess Property. This the						s predecessors the period of as questioned knowledge, no (s) (if Affiant is f the property, session of the	
STAT	E OF NORTH C	AROLINA NEBRASKE Valas		itle MA	TRICK S.F. HUAGER HEY FAMIL		
that	Valairy 1	Tol was	, personally known to	o me or l	9		
•		due and voluntary execu	tion of the foregoing Af	fidavit.	77		
	Pari P	OTARY - State of Nebraska NDREA YORK nm. Exp. October 19, 2025	Notary Publi State of Nor My Commiss	th Carolina	1-110/00	35	

[NOTARY SEAL]

AFFIDAVIT OF OWNERSHIP: EXHIBIT A — LEGAL DESCRIPTION

25CZ05 Submittal Date: 4-25-2025 Application #: Insert legal description below. BEING all of Lot 2, containing 6.2501 acres, more or less, as shown on that map entitled "MINOR SUBDIVISION PLAT PROPERTY OF SM RLAEIGH, LLC", by Dan Gregory, Professional Land Surveyor of Bass, Nixon & Kennedy, Inc., dated December 5, 2017, and recorded in Book of Maps 2018, Page 175, in the office of the Register of Deeds, Wake County, North Carolina.

Legal Description 2517 Kelly Road PIN 0731434504

BEGINNING AT A POINT LOCATED ON THE EASTERN RIGHT-OF-WAY LINE OF KELLY ROAD, BEING THE SOUTHWESTERN PROPERTY CORNER OF WEST VILLAGE NORTH OPEN SPACE AS RECORDED IN BOOK OF MAPS 2023, PAGE 1512, WAKE COUNTY REGISTRY, AND HAVING NC GRID (NAD '83/2011) COORDINATES OF NORTH 713,765.72 FEET, EAST 2,034,027.38 FEET; THENCE ALONG AND WITH SAID SOUTHERN PROPERTY LINE NORTH 56°46'21" EAST A DISTANCE OF 31.80 FEET TO AN EXISTING IRON PIPE; THENCE SOUTH 89°42'52" EAST A DISTANCE OF 392.72 FEET TO A POINT LOCATED ON THE WESTERN RIGHT-OF-WAY LINE OF FAHEY DRIVE AS SHOWN ON BOOK OF MAPS 2022, PAGE 2228, WAKE COUNTY REGISTRY; THENCE ALONG AND WITH SAID RIGHT-OF-WAY LINE WITH A CURVE TO THE LEFT AN ARC DISTANCE OF 30.93 FEET, SAID CURVE HAVING A RADIUS OF 38.50 FEET, A CHORD DIRECTION OF SOUTH 72°27'20" EAST, AND A CHORD DISTANCE OF 30.11 FEET TO A POINT; THENCE WITH A CURVE TO THE RIGHT AN ARC DISTANCE OF 8.08 FEET, SAID CURVE HAVING A RADIUS OF 19.50 FEET, A CHORD DIRECTION OF SOUTH 83°36'25" EAST, AND A CHORD DISTANCE OF 8.02 FEET TO A POINT; THENCE SOUTH 23°33'07" EAST A DISTANCE OF 29.53 FEET TO A POINT; THENCE CROSSING SAID RIGHT-OF-WAY NORTH 66°26'53" EAST A DISTANCE OF 50.00 FEET TO A POINT LOCATED ON THE EASTERN RIGHT-OF-WAY LINE OF FAHEY DRIVE: THENCE ALONG AND WITH SAID RIGHT-OF-WAY LINE NORTH 23°33'07" WEST A DISTANCE OF 29.07 FEET TO A POINT LOCATED ON THE SOUTHWESTERN PROPERTY CORNER OF COMMON OPEN SPACE 2 AS RECORDED IN BOOK OF MAPS 2022, PAGE 2228, WAKE COUNTY REGISTRY: THENCE ALONG AND WITH SAID SOUTHERN PROPERTY LINE SOUTH 89°42'52" EAST A DISTANCE OF 160.42 FEET TO AN EXISTING IRON PIPE LOCATED ON THE WESTERN RIGHT-OF-WAY LINE OF NC HIGHWAY 540; THENCE ALONG AND WITH SAID RIGHT-OF-WAY LINE SOUTH 05°40'24" WEST A DISTANCE OF 113.38 FEET TO AN EXISTING CONCRETE MONUMENT; THENCE SOUTH 05°37'10" WEST A DISTANCE OF 298.89 FEET TO AN EXISTING CONCRETE MONUMENT; THENCE SOUTH 00°44'07" WEST A DISTANCE OF 128.58 FEET TO AN EXISTING IRON PIPE LOCATED ON THE NORTHEASTERN PROPERTY CORNER OF LANDS NOW OR FORMERLY OF FRIENDSHIP COWORKING LLC AS RECORDED IN DEED BOOK 18673, PAGE 206, WAKE COUNTY REGISTRY; THENCE ALONG AND WITH SAID NORTHERN PROPERTY LINE NORTH 44°10'51" WEST A DISTANCE OF 73.95 FEET TO AN EXISTING IRON PIPE; THENCE SOUTH 48°27'04" WEST A DISTANCE OF 219.36 FEET TO A POINT LOCATED ON THE EASTERN RIGHT-OF-WAY LINE OF KELLY ROAD; THENCE ALONG AND WITH SAID RIGHT-OF-WAY LINE NORTH 34°34'58" WEST A DISTANCE OF 205.03 FEET TO A POINT; THENCE NORTH 33°28'03" WEST A DISTANCE OF 228.20 FEET TO A POINT; THENCE NORTH 33°13'39" WEST A DISTANCE OF 296.33 FEET TO A THE POINT OF BEGINNING, CONTAINING 5.8591 ACRES.



Wake County Residential Development Notification

Developer Company Information				
Company Name	N/A - No residential development proposed			
Company Phone Number				
Developer Representative Name				
Developer Representative Phone Number				
Developer Representative Email				

New Residential Subdivi	sion <i>Information</i>
Date of Application for Subdivision	
City, Town or Wake County Jurisdiction	
Name of Subdivision	
Address of Subdivision (if unknown enter nearest cross streets)	
REID(s)	
PIN(s)	

Please complete each section of this form and submit with your application.

Please complete each section of this form and submit with your application.

Please send any questions about this form to: studentassignment-gis-group@wcpss.net.

Projected Date	es Information
Subdivision Completion Date	
Subdivision Projected First Occupancy Date	

						Lot by L	ot Deve	lopment <i>l</i>	nformati	on							
Unit Type	Total # of Units	Senior Living	Studio	1 Bedroom	2 Bedroom	3 Bedroom	4 Bedroom	Squar Raı	e Foot nge	Price	Range	,	Anticipate	d Compl	etion Uni	ts & Dat	es
								Min	Max	Low	High	Year	# Units	Year	# Units	Year	# Units
Single Family																	
Townhomes																	
Condos																	
Apartments																	
Other																	

This or di Jan	sclosed to third parties. uary 16, 2025	North Carolina Public Records Act and may	be published on the Town's website				
Da	te						
You	r Neighbor: are invited to a neighborhood mee 17 Kelly Road	ting to review and discuss the develop	ment proposal at				
	Address(es)		PIN(s)				
for neigopp substitute main Develope http:	the applicant to discuss the programmer to discuss the programmer to discuss the programmer the ortunity to raise questions and discussified. If you are unable to attend, papplicant. Notified neighbors may refer to a polication has been elopment.	eighborhood Meeting procedures. This ject and review the proposed plans a submittal of an application to the To uss any concerns about the impacts of please refer to the Project Contact Information that the applicant provide updated submitted to the Town, it may be evelopment Report located on the ons for Rezoning must hold a second earing date.	meeting is intended to be a way with adjacent neighbors and the project before it is officially mation page for ways to contact ates and send plans via email or tracked using the Interactive Town of Apex website at the with the interactive at the contact and the interactive at the contact at the interactive at the i				
		ecause this project includes (check all the	I				
	pplication Type		Approving Authority				
	Rezoning (including Planned Unit D Major Site Plan	Development)	Town Council Technical Review Committee (staff)				
Ø		y care facility", "Government service", urant, drive-through", or "Convenience	Technical Review Committee (staff)				
	Special Use Permit		Board of Adjustment (QJPH*)				
	Residential Master Subdivision Plan		Technical Review Committee (staff)				
*Qı	uasi-Judicial Public Hearing: The Board	d of Adjustment cannot discuss the proje	ect prior to the public hearing.				
The	following is a description of the pro	oposal (also see attached map(s) and/o	r plan sheet(s)):				
site		oning to amend conditions of the non-residential uses, including, b space.					
Est	imated submittal date: February	3, 2025					
M	EETING INFORMATION:						
		Property Owner(s) name(s): Fahey Family Farm, LLC					
		· · · · · · · · · · · · · · · · · · ·					
Αį	oplicant(s):	Trilandco, LLC c/o Matthew Carpenter	240) 225 4222				
A _l	oplicant(s): ontact information (email/phone):	Trilandco, LLC c/o Matthew Carpenter matthewcarpenter@parkerpoe.com; (9)					
Ap Co M	oplicant(s):	Trilandco, LLC c/o Matthew Carpenter					

holidays). If you have questions about the general process for this application, please contact the Planning Department at 919-249-3426. You may also find information about the Apex Planning Department and on-going planning efforts at

http://www.apexnc.org/180.

Welcome: 5:30 PM Project Presentation: 5:30 PM Question & Answer: 6:00PM **Meetings shall occur between 5:00 p.m.-9:00 p.m. on a Monday through Thursday (excluding Town recognized



To: Neighboring Property Owners and Tenants

From: Matthew Carpenter Date: January 16, 2025

Re: Notice of Virtual Neighborhood Meeting

You are invited to attend a virtual neighborhood meeting on January 30, 2025 at 5:30 PM to discuss upcoming rezoning and site plan applications for an approximately 6.19-acre property located at 2517 Kelly Road (PIN 0731434504). The rezoning will amend the West Village PUD zoning conditions that currently apply to the property and rezone the property from Planned Unit Development Conditional Zoning (PUD-CZ) to PUD-CZ with revised conditions. The site plan application will allow for the development of non-residential uses on the property, including, but not limited to, a day care facility, retail space, and office space.

During the meeting, the applicant will describe the nature of the rezoning request and field any questions from the public. Enclosed are: (1) a vicinity map outlining the location of the property; (2) a zoning map of the area; (3) a copy of the PUD Plan, (4) a draft site plan, (5) a Transportation Plan exhibit, (6) a project contact information sheet; and (7) a common construction issues & who to call information sheet.

The meeting will be held virtually. You can participate online via Zoom or by telephone. To participate in the Zoom online meeting:

Visit: https://zoom.us./join

Enter the following meeting ID: 825 8184 0818

Enter the following password: 895920

To participate by telephone:

Dial: 1 929 205 6099 Enter the following meeting ID: 825 8184 0818

Enter the Participant ID: #

Enter the Meeting password: 895920

If you have any questions about this rezoning, please contact me at (919) 835-4032 or via email at matthewcarpenter@parkerpoe.com.

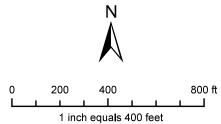
Sincerely,

Matthew Carpenter



2517 Kelly Road

Vicinity Map



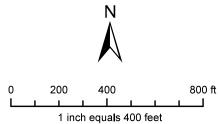
<u>Disclaimer</u> iMaps makes every effort to produce and publish the most current and accurate information possible. However, the maps are produced for information purposes, and are NOT surveys. No warranties, expressed or implied , are provided for the data therein, its use, or its interpretation.



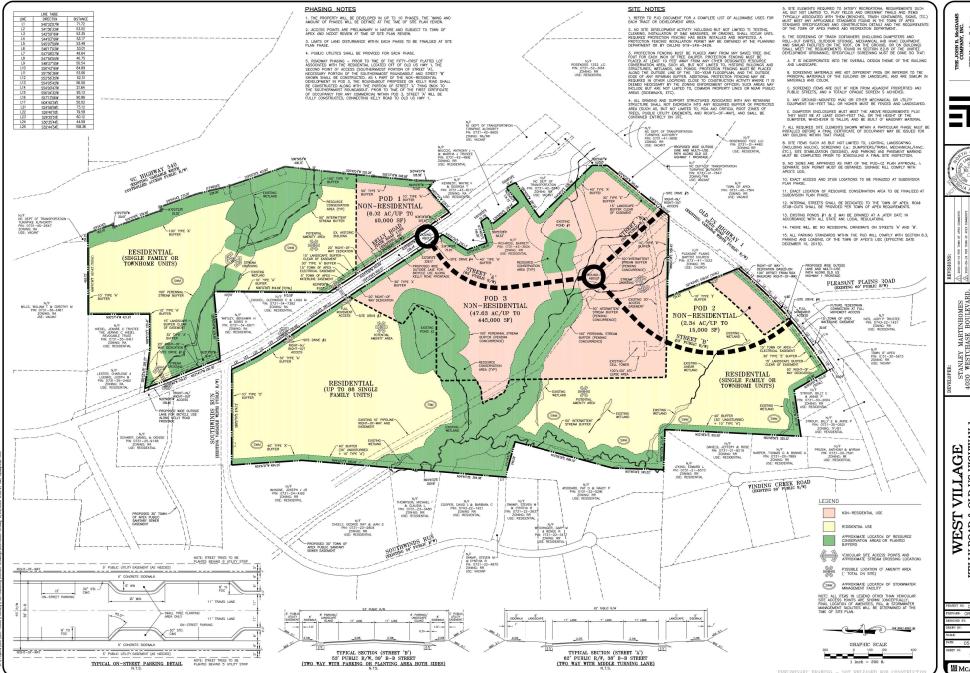
2517 Kelly Road

Zoning Map

Current Zoning: PUD-CZ



<u>Disclaimer</u> iMaps makes every effort to produce and publish the most current and accurate information possible. However, the maps are produced for information purposes, and are NOT surveys. No warranties, expressed or implied , are provided for the data therein, its use, or its interpretation.



CADAMS

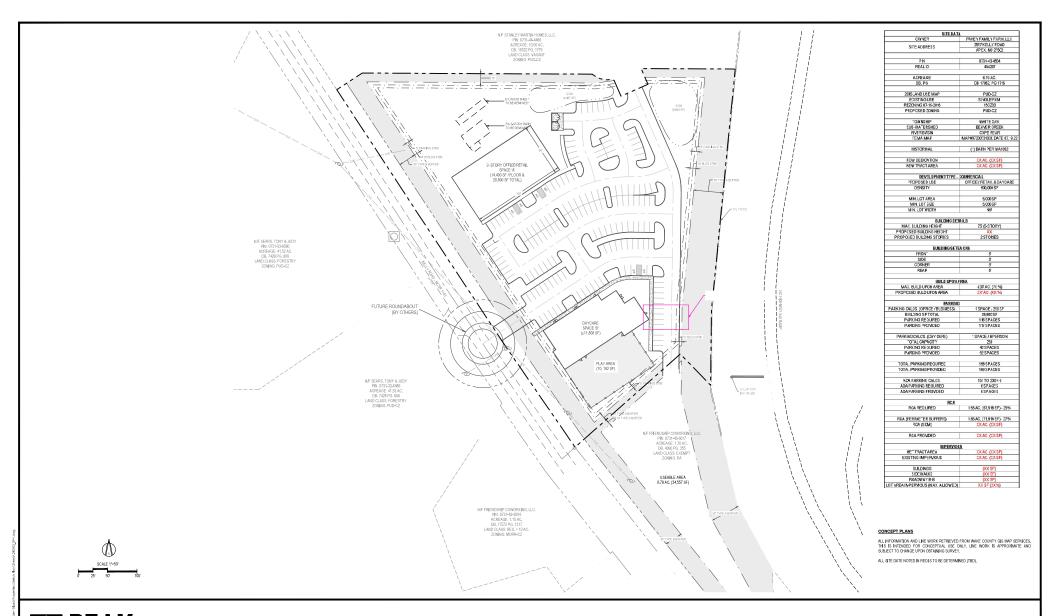
STANLEY MARTINHOMES 4020 WESTCHASE BOULEVARD, SUITE 190 RALEIGH, NC 27607

VILLAGE

2 OLD US HIGHWAY 1 ROAD APEX.

C-2

MCADAMS





KELLY ROAD COMMERCIAL

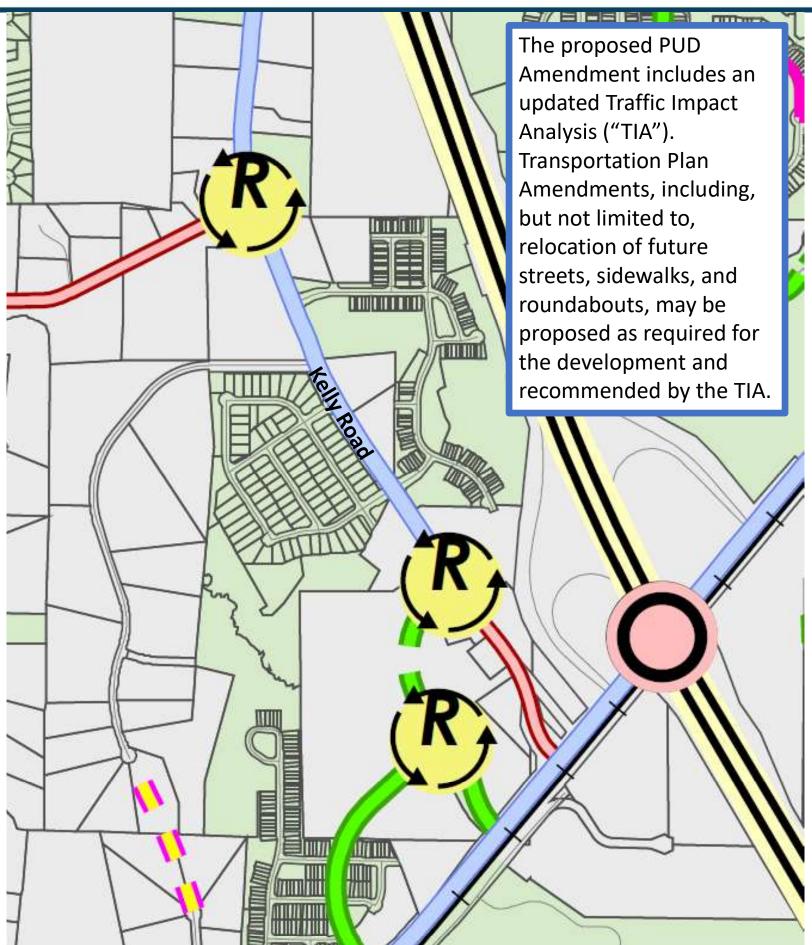
2517 KELLY ROAD APEX, NC WAKE COUNTY

ORNORI DE:	CHECKED IN	PROJECTNO.
MD		240403
MI		SHEET HED.
01.	09.25	SP-1



Thoroughfare and Collector Street Plan





PROJECT CONTACT INFORMATION

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Development Contacts:				
Project Name: West Village PUD A	Amendment		Z	oning: PUD-CZ
Location: 2517 Kelly Road				
Property PIN(s): <u>0731434504</u>	Acreage	e/Square	e Feet: 6.	19 ac
Property Owner: Fahey Family Fam	ı, LLC			
Address: 1115 Capitata Crossing				
City: Apex		State:	NC	zip: 27502-9011
Phone:	Email:			
Developer: Trilandco, LLC c/o Matth	ew Carpenter			
Address: 301 Fayetteville Street, S	Suite 1400			
City: Raleigh	State:	NC		Zip: 27601
Phone: (919) 835-4032 Fax	N/A		Email:	matthewcarpenter@parkerpoe.com
Engineer: Peak Engineering & Designment	ın, PLLC, attn	: Jeff Ro	ach	
Address: 1125 Apex Peakway				
City: Apex		_ State:	NC	zip: 27502
Phone: (919) 439-0100 Fax	: <u>N/A</u>		Email:	jroach@peakengineering.com
Builder (if known): Trilandco, LLC c	o Matthew C	arpenter		
Address: 301 Fayetteville Street, S	Suite 1400			
City: Raleigh		State:	NC	Zip: 27601
Phone: (919) 835-4032 Fax	. <u>N/A</u>		Email:	matthewcarpenter@parkerpoe.com

Please note that Town staff will not have complete information about a proposed development until the application is submitted for review. If you have a question about Town development standards and how they relate to the proposed development, please contact the appropriate staff person listed below.

Town of Apex Department Contacts	
Planning Department Main Number	
(Provide development name or location to be routed to correct planner)	(919) 249-3426
Parks, Recreation & Cultural Resources Department	
Angela Reincke, Parks Planning Project Manager	(919) 372-7468
Transportation & Infrastructure Development	
Russell Dalton, Traffic Engineering Manager	(919) 249-3358
Water Resources Department	
Jessica Bolin, Environmental Engineering Manager (Stormwater, Sedimentation &	(919) 249-3537
Erosion Control)	
Matt Reker, Utility Engineer/FOG Program Manager (Water & Sewer)	(919) 946-4394
Electric Utilities Division	
Rodney Smith, Electric Technical Services Manager	(919) 249-3342

COMMON CONSTRUCTION ISSUES & WHO TO CALL

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Noise & Hours of Construction:

Non-Emergency Police

Noise from tree removal, grading, excavating, paving, and building structures is a routine part of the construction process. The Town generally limits construction hours from 7:00 am to 8:30 pm so that there are quiet times even during the construction process. Note that construction outside of these hours is allowed with special permission from the Town when it makes more sense to have the construction occur at night, often to avoid traffic issues. In addition, the Town limits hours of blasting rock to Monday-Friday from 8:00 am to 5:00 pm. Report violations of construction hours and other noise complaints to the Non-Emergency Police phone number at 919-362-8661.

Construction Traffic:

Infrastructure Inspections

919-249-3386

Construction truck traffic will be heavy throughout the development process, including but not limited to removal of trees from site, loads of dirt coming in and/or out of the site, construction materials such as brick and wood brought to the site, asphalt and concrete trucks come in to pave, etc. The Town requires a construction entrance that is graveled to try to prevent as much dirt from leaving the site as possible. If dirt does get into the road, the Town can require they clean the street (see "Dirt in the Road" below).

Road Damage & Traffic Control:

Infrastructure Inspections

There can be issues with roadway damage, roadway improvements, and traffic control. Potholes, rutting, inadequate lanes/signing/striping, poor traffic control, blocked sidewalks/paths are all common issues that should be reported to Transportation & Infrastructure Development – Infrastructure Inspections at 919-249-1109. The Town will get NCDOT involved if needed.

Parking Violations:

Non-Emergency Police

Unless a neighbor gives permission, there should be no construction parking in neighbors' driveways or on their property. Note that parking in the right-of-way is allowed, but Town regulations prohibit parking within 15 feet of driveways so as not to block sight triangles. Trespassing and parking complaints should be reported to the Non-Emergency Police phone number at 919-362-8661.

Dirt in the Road:

Water Resources

Sediment (dirt) and mud gets into the existing roads due to rain events and/or vehicle traffic. These incidents should be reported by visiting the Report a Concern page at https://www.apexnc.org/1173/ or by calling the number listed. Staff will coordinate the cleaning of the roadways with the developer.

Dirt on Properties or in Streams:

Water Resources

919-362-8166

Sediment (dirt) can leave the site and get onto adjacent properties or into streams and stream buffers; it is typically transported off-site by rain events. These incidents should be reported by visiting the Report a Concern page at https://www.apexnc.org/1173/ or by calling the number listed so that staff can coordinate the appropriate repairs with the developer.

Dust:

Water Resources

919-362-8166

During dry weather dust often becomes a problem blowing into existing neighborhoods or roadways. These incidents should be reported by visiting the Report a Concern page at https://www.apexnc.org/1173/ or by calling the number listed so that staff can coordinate the use of water trucks onsite with the grading contractor to help control the dust.

Trash:

Water Resources

919-362-8166

Excessive garbage and construction debris can blow around on a site or even off of the site. These incidents should be reported by visiting the Report a Concern page at https://www.apexnc.org/1173/ or by calling the number listed. Staff will coordinate the cleanup and trash collection with the developer/home builder.

Temporary Sediment Basins:

Water Resources

Temporary sediment basins during construction (prior to the conversion to the final stormwater pond) are often quite unattractive. Concerns should be reported by visiting the Report a Concern page at https://www.apexnc.org/1173/ or by calling the number listed so that he can coordinate the cleaning and/or mowing of the slopes and bottom of the pond with the developer.

Stormwater Control Measures: Water Resources

Post-construction concerns related to Stormwater Control Measures (typically a stormwater pond) such as conversion and long-term maintenance should be reported by visiting the Report a Concern page at https://www.apexnc.org/1173/ or by calling the number listed.

Electric Utility Installation:

Rodney Smith

919-249-3342

Concerns with electric utility installation can be addressed by the Apex Electric Utilities Department. Contact Rodney Smith at 919-249-3342.

NEIGHBORHOOD MEETING SIGN-IN SHEET

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Meeting Address: virtual via Zoom	
Date of meeting: January 30, 2025	Time of meeting: 5:30 PM
Property Owner(s) name(s): Fahey Family Farm, LLC	
Applicant(s): Trilandco, LLC c/o Matthew J. Carpe	

Please <u>print</u> your name below, state your address and/or affiliation with a neighborhood group, and provide your phone number and email address. Providing your name below does not represent support or opposition to the project; it is for documentation purposes only. For virtual meetings, applicants must include all known participants and request the information below.

	NAME/ORGANIZATION	ADDRESS	PHONE #	EMAIL	SEND PLANS & UPDATES
1.	No neighbors attended the meeting				
2.					
3.					
4.					
5.					
6.					
7.					
8.					
9.					
10.					
11.					
12.					
13.					
14.					

Use additional sheets, if necessary.

SUMMARY OF DISCUSSION FROM THE NEIGHBORHOOD MEETING

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Property Owner(s) name(s): Faney Family Farm, LLC
Applicant(s): Trilandco, LLC c/o Matthew J. Carpenter
Contact information (email/phone): MatthewCarpenter@parkerpoe.com; 919-835-4032
Meeting Address: virtual via Zoom
Date of meeting: January 30, 2025 Time of meeting: 5:30 PM
Please summarize the questions/comments and your responses from the Neighborhood Meeting or emails/phone calls received in the spaces below (attach additional sheets, if necessary). Please state if/how the project has been modified in response to any concerns. The response should not be "Noted" or "No Response". There has to be documentation of what consideration the neighbor's concern was given and justification for why no change was deemed warranted. Question/Concern #1:
No neighbors attended the meeting
Applicant's Response:
Question/Concern #2:
Applicant's Response:
Question/Concern #3:
Applicant's Response:
Question/Concern #4:
Applicant's Response:

AFFIDAVIT OF CONDUCTING A NEIGHBORHOOD MEETING, SIGN-IN SHEET AND ISSUES/RESPONSES SUBMITTAL

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

l,/	Matthew J. Carpenter, do hereby declare as follows:
	Print Name
1.	I have conducted a Neighborhood Meeting for the proposed Rezoning, Major Site Plan, Minor Site Plan, Residential Master Subdivision Plan, or Special Use Permit in accordance with UDO Sec. 2.2.7.B Neighborhood Meeting.
2.	The meeting invitations were mailed to the Apex Planning Department, all property owners and tenants abutting and within 300 feet of the subject property and any neighborhood association that represents citizens in the notification area via first class mail a minimum of 14 days in advance of the Neighborhood Meeting.
3.	The meeting was conducted at <u>virtually via Zoom</u> (location/address)
	The meeting was conducted at <u>virtually via Zoom</u> (location/address) on <u>1/30/2025</u> (date) from <u>5:30 PM</u> (start time) to <u>6:30 PM</u> (end time).
4.	I have included the mailing list, meeting invitation, sign-in sheet, issue/response summary, and zoning map/reduced plans with the application.
5.	I have prepared these materials in good faith and to the best of my ability.
_//	30/2025 By:
	OF NORTH CAROLINA Y OF WAKE
Sworn a	and subscribed before me. MatthewCarpenter a Notary Public for the above State and
County,	on this the 30 day of January, 2025.
	SEAL Maria Stally
	Notary Public Mana Staurs Print Name
	My Commission Expires: April 202 (a

OWNER	MAILING ADDRESS	
ARYA, ANJALI NAIR, AJAY	3577 RISE DR	MORRISVILLE NC 27560-5924
BELLAM, ANUDEEP VUNNAM, SAI KAMALA	1971 FAHEY DR	APEX NC 27502-7093
BHAVSAR, DEWANG S BHAVSAR, DIPTI PATEL	2010 LEO DR	APEX NC 27502-7033 APEX NC 27502-4361
BRAHMANDAY, GOVINDA RAGHURAM PALURU, HIMABINDU	829 NIJINSKI WAY	CARY NC 27519-6740
COHEN, STEVEN A SLUSS-COHEN, ILDAURA TANISHA	608 PENINSULA FOREST PL	CARY NC 27519-01505
COUNCIL, GLENWOOD C COUNCIL, LINDA M	2411 KELLY RD	APEX NC 27502-9589
FAHEY FAMILY FARM LLC	1115 CAPITATA XING	APEX NC 27502-9309 APEX NC 27502-9011
FRIENDSHIP COWORKING LLC	2121 MCKENZIE RIDGE LN	APEX NC 27502-9011 APEX NC 27502-6629
GUPTA, MANIK GUPTA, RAJASAVI	842 PEPPER TREE LN	SANTA CLARA CA 95051-5227
GUPTA, MANIK GUPTA, RAJASAVI GUPTA, SANDEEP RAMBACHAN RAI, SWETA	1952 FAHEY DR	APEX NC 27502-7093
HARGROVE, CATHERINE HEART HARGROVE, CHRISTOPHER EARL	2000 LEO DR	APEX NC 27502-1095 APEX NC 27502-4361
HENDERSON, SAMUEL JUSTIN WALTERS, TYLER SCOTT	1953 FAHEY DR	APEX NC 27502-4361 APEX NC 27502-7093
· · · · · · · · · · · · · · · · · · ·		
KARUPPASWAMY, RAMKUMAR KARUPPUSWAMY, RADHIKKAA	1950 FAHEY DR	APEX NC 27502-7093
LI, JUAN	1954 FAHEY DR	APEX NC 27502-7093
LU, YI-TIEN WANG, NAN-YI	1977 FAHEY DR	APEX NC 27502-7093
MALATHI, MOGAN RAJ NADARAJAN BABU, PRIYADARSHINI	1970 FAHEY DR	APEX NC 27502-7093
MASADI, AKHILA BIKKINENI, KONDAL RAO	1969 FAHEY DR	APEX NC 27502-7093
MIUCCIO, ANTHONY J TRUSTEE MIUCCIO, MARTHA J TRUSTEE	PO BOX 2063	APEX NC 27502-1154
MUDDASANI, SANDEEP REDDY MUDDASANI, SHILPA GOUD	2008 LEO DR	APEX NC 27502-4361
MUTHALIP ABUBAKKAR, SHEIK ABDUL BASHEER, ABIYABARVIN AHAMED	1983 CUSTOM LN	APEX NC 27502-4349
PAREEK, ABHISHEK	1967 FAHEY DR	APEX NC 27502-7093
PATURKAR, SHREYAS JITENDRA SHINTRE, PALLAVI SHRINIVAS	1957 FAHEY DR	APEX NC 27502-7093
RAGHUNATHAN, RAGHUNATHAN	2002 LEO DR	APEX NC 27502-4361
RAYALA, GIRIDHAR	1973 FAHEY DR	APEX NC 27502-7093
SAMANTA, SOURISHMOY SINHA, PARINITA	1959 FAHEY DR	APEX NC 27502-7093
SEARS, TONY C SEARS, JUDY T	2508 KELLY RD	APEX NC 27502-9563
SHARMA, SANJAY	1513 KIRBY LN	RALEIGH NC 27614
SHEN, SI	4009 RATCLIFFE	BENTONVILLE AR 72713-7650
SHIELDS LAND LLC	PO BOX 65	HOLLY SPRINGS NC 27540-0065
SKL REALTY LLC	3904 WEDONIA DR	CARY NC 27519-6629
SOTOS, GUSTAVO REZENDE DOS SGARBI, ROXANE TASSI	2006 LEO DR	APEX NC 27502-4361
STANLEY MARTIN HOMES LLC	11710 PLAZA AMERICA DR STE 1100	RESTON VA 20190-4771
THAKURI, SUMAN CHAND, ARPANA	1956 FAHEY DR	APEX NC 27502-7093
UNDEELA, SHASHANK THOTA, KIRANMAI	1972 FAHEY DR	APEX NC 27502-7093
VALENCIA, CAMILA ANDREA SAYDAH, JOHN DEMAREST	1968 FAHEY DR	APEX NC 27502-7093
ZISKIN, NATALIE PARHAM, ANDREW	1974 FAHEY DR	APEX NC 27502-7093
Current Tenant	1947 Fahey DR	APEX NC 27502
Current Tenant	1949 Fahey DR	APEX NC 27502
Current Tenant	1955 Fahey DR	APEX NC 27502
Current Tenant	1961 Fahey DR	APEX NC 27502
Current Tenant	1963 Fahey DR	APEX NC 27502
Current Tenant	1966 Fahey DR	APEX NC 27502
Current Tenant	1975 Fahey DR	APEX NC 27502
Current Tenant	2517 Kelly RD	APEX NC 27502
Current Tenant	2524 Kelly RD	APEX NC 27502
Current Tenant	2601 Kelly RD	APEX NC 27502
Current Tenant	2604 Kelly RD	APEX NC 27502
Current Tenant	2004 Leo DR	APEX NC 27502

APEX ENVIRONMENTAL ADVISORY BOARD Suggested Zoning Conditions



West Village PUD Amendment

2/3/2025

The Town of Apex Environmental Advisory Board offers this general list of suggested rezoning conditions for rezoning applicants to consider before filling a rezoning petition. The purpose of this list is to encourage and recommend implementation of exceptional environmental practices for future development that exceeds Town requirements. The Board will review each rezoning pre-application request and expand on suggested conditions by offering specific recommendations on a case-by-case basis.

The decision to include any of the recommendations below is voluntary by the applicant and the Board does not expect applicants to add all of the suggested conditions. Planning staff will include all zoning conditions suggested by this Board and will note which conditions have been added by the applicant in the staff reports to the Planning Board and Town Council. Applicants should review this list before meeting with the Board. NOTE: Text in green indicates suggested zoning condition language from Planning Staff. Underlined text indicates text or numbers that may be changed based on the specific project. Additional conditions may be suggested by the EAB at the meeting.

This document is divided into two parts:

- <u>Part I Residential</u> applies to single-family dwellings and townhome subdivisions, but does not include the parking lots, exterior building lights or exterior architecture.
- Part II Non-Residential includes condominiums, apartments, and multi-family, common areas
 in residential developments (e.g. amenity areas, parking lots, exterior building lights, and
 exterior architecture), commercial, office, and industrial areas. Your development may include
 elements of each part.

Please be sure to read and complete the entire document. Please provide a response to each goal and/or sub-goal. Any proposed modifications to the green zoning language should be listed in the section at the end of the document.

Part I – Residential

Single-family dwelling and townhome subdivisions (excluding parking lots, exterior building lights and exterior architecture).

STORMWATER AND WATER CONSERVATION – WATER QUALITY (1-5)		NO	N/A
Goal 1. Increase riparian buffer widths from surface waters in environmentally sensitive areas. The project shall increase the riparian buffer width by at least feet above the minimum required by the Unified Development Ordinance. The additional buffer width shall be measured from the top of bank on each side of the stream.			
Goal 2. Install signage near environmental sensitive areas in order to reduce pet waste and excess nutrient inputs near Stormwater Control Measure (SCM) drainage areas.			

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STORMWATER AND WATER CONSERVATION – WATER QUALITY (1-5)	YES	NO	N/A
The project shall install one (1) sign per SCM to reduce pet waste and prohibit fertilizer, in locations that are publicly accessible, such as adjacent to amenity centers, sidewalks, greenways, or side paths.			
Goal 3. Implement Low Impact Development (LID) techniques as defined by the NC Department of Environmental Quality. The project shall install a minimum of Low Impact Development Technique as defined and approved by the NC Department of Environmental Quality. The specific type of LID technique shall be reviewed and approved by the Water Resources Department at site or subdivision plan review.			
Goal 4. Increase pervious surface to reduce stormwater runoff and pollutant concentrations.			
<u>Option 5.1:</u> Install pervious pavements where practicable (e.g. when parking maximums are exceeded). The Department of Public Works & Transportation does not currently support these options within the right-of-way (ROW). These may be done on private sites, but not within the public ROW.			
a. The project shall utilize pervious pavement when constructing the parking spaces for parking lot-style townhomes. The specific type of pervious pavement system shall be reviewed and approved by the Water Resources Department at site or subdivision plan review. The selected system shall be maintained by the developer and/or owner's association.			
 AND/OR b. The project shall utilize pervious pavement when constructing the driveways for residential units. The specific type of pervious pavement system shall be reviewed and approved by the Water Resources Department at site or subdivision plan review. The selected system shall be maintained by the developer and/or owner's association. 			
Goal 5. Use the stormwater captured in the on-site SCM to irrigate landscaping within the development.			
At least SCM shall be designed and constructed to provide irrigation to the surrounding landscaping on site. The design shall be reviewed and approved by the Water Resources Department at site plan.			
PLANTING AND LANDSCAPING (6-13)	YES	NO	N/A
Goal 6. Preserve tree canopy and prioritize medium to large, healthy, desirable species.			
Option 6.1: Preserve existing trees (percentage-based). Numbers shown may be changed based on project. The project shall preserve a minimum of			
trees that must be replaced, a tree survey for the full property shall be provided to the Planning Department. The survey shall be independently verified by a third-party licensed arborist.			

PLANTING AND LANDSCAPING (6-13)	YES	NO	N/A
Goal 7. Plant trees to improve energy efficiency.			
Option 7.1: Plant deciduous shade trees on southern side of buildings. To improve energy efficiency, a combination of large and small deciduous shade trees shall be planted on the southern side of any buildings.			
Option 7.2: Plant evergreen trees as a windbreak on northern side of buildings. To improve energy efficiency, the project shall plant evergreen trees on the northern side of all buildings to act as a windbreak.			
Goal 8. Increase biodiversity.			
Note: Invasive species are prohibited. Please see the Town's <u>Design and Development Manual</u> for a link to the list of prohibited species. Option 8.1: Plant pollinator-friendly flora. Provide diverse and abundant pollinator and bird food sources (e.g. Snectar, pollen, and berries from blooming plants) that bloom in succession from spring to fall. (Refer to the Apex <u>Design & Development Manual</u> for suggested native species).			
a. The project shall ensure that% of the landscaping shall be native species, which shall provide diverse and abundant pollinator and bird food sources. Special attention shall be paid to providing diverse and abundant pollinator and bird food sources, including plants that bloom in succession from spring to fall. Landscaping shall be coordinated with and approved by the Planning Department at site or subdivision review.			
Option 8.2: Provide and allow for undisturbed spaces (e.g. leaf piles, unmown fields, fallen trees) for nesting and overwintering for native pollinators and wildlife. In order to support wildlife and pollinators, HOA covenants shall not require that fallen leaves or dormant plants be removed during the winter on areas without turf grass, including individual homes and HOA owned common areas.			
<u>Option 8.3:</u> Retain and protect old ponds if the dam is structurally sound. To preserve and protect existing species, existing ponds shall be preserved if structurally sound.			
 Option 8.4: Increase the number of native trees and shrubs. a. The project shall increase biodiversity within perimeter buffers, common owned open space, and other landscape areas by providing a variety of native and adaptive species for the canopy, understory and shrub levels. A minimum of% of the species selected shall be native or a native of North Carolina. AND/OR			
 b. No single species of native or adaptive vegetation shall constitute more than 20% of the plant material of its type within a single development site. 			
Goal 9. Implement xeriscaping in design, which will use landscaping that requires less irrigation and chemical use. Contact Planning for assistance, if needed.			
a. The project commits to planting only drought tolerant plants, of which% of the plants selected shall be native. Landscaping shall be coordinated with and approved by the Planning Department at site or subdivision review. OR			
 b. To reduce irrigation requirements, the project shall select and plant only warm season grasses. 			
Goal 10. Promote the benefits of native pollinators.			
The project shall plant at least native pollinator demonstration garden within the development. The developer shall coordinate with a local or state agency that specializes in the design or certification of such gardens. Informational signage regarding the purpose of the garden and selected vegetation shall be provided. The pollinator garden shall be maintained by the developer or HOA.			
Goal 11. Improve soil quality to be amenable for a variety of native and non-invasive plantings.			

PLANTING AND LANDSCAPING (6-13)	YES	NO	N/A
To encourage the establishment of healthy plants, reduce fertilizers, and reduce stormwater runoff, topsoil shall be retained on site and a minimum of 4 inches of topsoil shall be placed on each lot and within disturbed common areas.			
Goal 12. Increase perimeter buffer requirements, especially in transitional areas (nonresidential to residential areas).			
The UDO requires afoot buffer along theperimeter of the property. The applicant shall addfoot buffer in that location, which would be an increase offeet above the requirement.			
Goal 13. Reduce impacts to resource conservation Areas (RCAs). a. The project shall install signage adjacent to wooded or natural condition Resource Conservation Area. The signage shall indicate that the area is RCA and is to be preserved in perpetuity and not disturbed. OR			Ø
 A farm-style split rail fence shall be installed where wooded or natural condition Resource Conservation Area (RCA) abuts individual residential lots. 			
SUSTAINABLE BUILDINGS (14)	YES	NO	N/A
Goal 14. Apply for green building certifications, such as LEED, Energy Star, BREEAM,			
Green Globes, NGBS Green, or GreenGuard. The project shall be designed to meet the requirements for one of the green building certifications listed above. A third-party consultant shall be hired to evaluate the project and certify to the Town of Apex that the project meets the standards for the certification. The applicant shall forward a copy of the certification application to the Town of Apex Planning Department to verify that the application has been submitted.			
WASTE MANAGEMENT (15)	YES	NO	N/A
WASTE MANAGEMENT (15) Goal 15. Encourage the proper disposal of pet waste to reduce environmental impacts. Numbers shown may be changed based on project. The project shall install at least one (1) pet waste station per 25 residential units throughout the community in locations that are publicly accessible, such as adjacent to amenity centers, SCMs, sidewalks, greenways or side paths. If there fewer than 25 homes, at least one (1) pet waste station shall be installed.	YES	NO	N/A
Goal 15. Encourage the proper disposal of pet waste to reduce environmental impacts. Numbers shown may be changed based on project. The project shall install at least one (1) pet waste station per 25 residential units throughout the community in locations that are publicly accessible, such as adjacent to amenity centers, SCMs, sidewalks, greenways or side paths. If there fewer than 25		NO NO	N/A
Goal 15. Encourage the proper disposal of pet waste to reduce environmental impacts. Numbers shown may be changed based on project. The project shall install at least one (1) pet waste station per 25 residential units throughout the community in locations that are publicly accessible, such as adjacent to amenity centers, SCMs, sidewalks, greenways or side paths. If there fewer than 25 homes, at least one (1) pet waste station shall be installed.			Ø
Goal 15. Encourage the proper disposal of pet waste to reduce environmental impacts. Numbers shown may be changed based on project. The project shall install at least one (1) pet waste station per 25 residential units throughout the community in locations that are publicly accessible, such as adjacent to amenity centers, SCMs, sidewalks, greenways or side paths. If there fewer than 25 homes, at least one (1) pet waste station shall be installed. CLEAN ENERGY (16-18)			Ø

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,			
CLEAN ENERGY (16-18)	YES	NO	N/A
 The amenity center for the project shall include a rooftop solar PV system with a capacity of at least kWHs. 			
Goal 17. Include solar conduit in building design.			
All homes shall be pre-configured with conduit for a solar energy system.			
Goal 18. Encourage clean transportation.			
The developer shall install at leastelectric vehicle charging station in amenity	Ш		
centers or common area parking lots.			
Part II - Non-Residential			
Includes condominiums, apartments, and multi-family, common areas in residentic amenity areas, parking lots, exterior building lights, and exterior architecture), co industrial areas.		-	
STORMWATER AND WATER CONSERVATION – WATER QUANTITY (1)	YES	NO	N/A
Goal 1. Increase design storm for retention basin in flood-prone areas. The UDO requires that treatment for the first 1-inch of runoff will be provided such that the removal of 85% Total Suspended Solids is achieved. Each option is intended to be used as an improvement to the minimum UDO requirements. If an area is already required to mitigate the 25-year storm, option b should not be selected. a. Post-development peak runoff shall not exceed pre-development peak runoff for			
the 24-hour, 1-year, 10-year, 25-year and 100-year storm events in accordance with the Unified Development Ordinance. OR			
 Post development peak runoff shall not exceed pre-development peak runoff for the 24-hour, 1-year, 10-year, and <u>25-year storm events</u> in accordance with the Unified Development Ordinance. 			
STORMWATER AND WATER CONSERVATION – WATER QUALITY (2-7)	VEC	NO	NI/A
1 1	YES	NO	N/A
Goal 2. Increase riparian buffer widths from surface waters in environmentally sensitive areas. The project shall increase the riparian buffer width by at leastfeet above the minimum required by the Unified Development Ordinance. The additional buffer width shall be measured from the top of bank on each side of the stream.			
Goal 3. Limit tree clearing, stormwater control measures (SCM), or infrastructure in either zone of the riparian buffer. No clearing or land disturbance shall be permitted within the riparian buffer, except the minimum necessary to install required sewer infrastructure and SCM outlets. The SCM water storage and treatment area shall not be permitted within the riparian buffer. The sewer shall be designed to minimize impacts to the riparian buffer.			
Goal 4. Install signage near environmental sensitive areas in order to reduce pet waste			
and excess nutrient inputs near Stormwater Control Measure (SCM) drainage			
areas.			
The project shall install one (1) sign per SCM to reduce pet waste and prohibit fertilizer, in locations that are publicly accessible, such as adjacent to amenity centers, sidewalks, greenways, or side paths.			
Goal 5. Implement low impact development (LID) techniques as defined by the NC			
Department of Environmental Quality.			

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defined and approved by the NC Department of Environmental Quality. The specific

STORMWATER AND WATER CONSERVATION – WATER QUALITY (2-7)	YES	NO	N/A
type of LID technique shall be reviewed and approved by the Water Resources Department at site or subdivision plan review.			
Goal 6. Increase pervious surface to reduce stormwater runoff and pollutant concentrations. The Department of Public Works & Transportation does not currently support these options within the ROW. These may be done on private sites, but not within the public ROW. Option 6.1: Install pervious pavements where practicable (e.g. when parking	1		
 maximums are exceeded). a. The project shall utilize pervious pavement when constructing parking spaces that are in excess of the minimum parking requirement. The specific type of pervious pavement system shall be reviewed and approved by the Water Resources Department at site or subdivision plan review. 	1		V
AND/OR			
 b. The project shall utilize pervious pavement for all of the parking spaces provided. The specific type of pervious pavement system shall be reviewed and approved by the Water Resources Department at site or subdivision plan review. Option 6.2: Modify curb and gutters to provide stormwater infiltration and 			
evaporation, such as swale-only, reverse curbs, Silva cells, or curb cuts with rain gardens. To increase stormwater infiltration and evaporation, the project shall use modified curb and gutter designs to direct driveway runoff to one or more stormwater device, such as, but not limited to, bioswales, Silva cells, or rain gardens. The specific type and design shall be selected at site or subdivision plan review. The proposal shall be reviewed and approved by the Water Resources Department and Department of Public Works and Transportation. Option 6.3: Utilize green street design. May be done within the public ROW if it's in the form of a bioretention cell within a landscaped median or large roundabout. Will require approval by the Department of Public Works and Transportation. The project shall design and install one or more bioretention cells within the			
landscape median or roundabout along the primary road. The specific type and design shall be determined at site or subdivision plan review. The proposal shall be reviewed and approved by the Water Resources Department and Department of Public Works and Transportation.			
Goal 7. Stormwater re-use application: Integrate irrigation from the SCM (wet pond) on site.			
At least oneSCM shall be designed and constructed to provide irrigation to the surrounding landscaping on site. The design shall be reviewed and approved by the Water Resources Department at site plan.			
PLANTING AND LANDSCAPING (8-15)	YES	NO	N/A
Goal 8. Preserve tree canopy and prioritize medium to large, healthy, desirable			
species. Option 8.1: Preserve existing trees (percentage-based). Numbers shown may be changed based on project. The EAB's preference is for a minimum of 50%. a. The project shall preserve a minimum of% of the existing tree canopy. Preserved areas may include, but are not limited to, RCA, perimeter buffers, riparian buffers and/or HOA maintained open space throughout the neighborhood. OR			
b. The project shall preserve a minimum of% of the existing tree canopy.			
Where the project abuts adjacent developments, special effort shall be taken to			

PLANTING AND LANDSCAPING (8-15)	YES	NO	N/A
locate the preserved trees adjacent to areas of existing preserved open space, including but not limited to, RCA, perimeter landscape buffers, riparian buffers, and/or HOA maintained open spaces. Option 8.2: Replace canopy (percentage- or DBH size-based) where there is sufficient			
space. The project shall replace any large type trees, that measure 18-inches in caliper size or larger, and small type trees, that measure 8-inches in caliper size or larger, that are removed as a part of the development. The ratio of replacement shall be 1 large tree to 1 replacement tree. The UDO's required landscaping may be used to satisfy this requirement. To determine the number of trees that must be replaced, a tree survey for the full property shall be provided to the Planning Department. The survey shall be independently verified by a third-party licensed arborist.			
Goal 9. Plant trees for improved energy efficiency.			
<u>Option 9.1</u> : Plant deciduous shade trees on southern side of buildings. To improve energy efficiency, a combination of large and small deciduous shade trees shall be planted on the southern side of any buildings.			
<u>Option 9.2:</u> Plant evergreen trees as a windbreak on northern side of buildings. To improve energy efficiency, the project shall plant evergreen trees on the northern side of all buildings to act as a windbreak.			
Goal 10. Increase biodiversity.			
<u>Option 10.1:</u> Plant pollinator-friendly flora. Provide diverse and abundant pollinator and bird food sources (e.g. nectar, pollen, and berries from blooming plants) that bloom in succession from spring to fall. (Refer to the Apex <u>Design & Development Manual</u> for suggested native species).			
a. The project shall select and install tree, shrub and perennial species with special attention to providing diverse and abundant pollinator and bird food sources, including plants that bloom in succession from spring to fall.			
OR			
b. The project shall ensure that% of the landscaping shall be native species. Landscaping shall be coordinated with and approved by the Planning Department at site or subdivision review.			
<u>Option 10.2:</u> Retain and protect old ponds if the dam is structurally sound. To preserve and protect existing species, existing ponds shall be preserved if structurally sound.			
 Option 10.3: Increase the number of native tree and shrub species selected. a. The project shall increase biodiversity within perimeter buffers, common owned open space, and other landscape areas by providing a variety of native and adaptive species for the canopy, understory and shrub levels. A minimum of% of the species selected shall be native or a native of North Carolina. 			
 b. No invasive species shall be permitted. No single species of tree or shrub shall constitute more than 20% of the plant material of its type within a single development site. 			
Goal 11. Implement green infrastructure.			
Option 11.1: Plant rain gardens. The project shall install one or more rain gardens throughout the site.			
Option 11.2: Install vegetated rooftops.			
 a. The project shall install a vegetated rooftop, aka green roof, on each building. OR 			
 b. The project shall install a vegetated rooftop, aka green roof, on at leastft² of each building. 			

PLANTING AND LANDSCAPING (8-15)	YES	NO	N/A
Option 11.3: Implement xeriscaping in design.			
a. The project commits to planting% drought tolerant native plants. Landscaping shall be coordinated with and approved by the Planning Department at site or subdivision review.			
b. The project commits to planting only drought tolerant plants. At least% of the plants selected shall be native. Landscaping shall be coordinated with and approved by the Planning Department at site or subdivision review. OR			
 To reduce irrigation requirements, the project shall select and plant only warm season grasses. 			
Goal 12. Install community gardens and native pollinator demonstration gardens.			
The project shall plant at least native pollinator demonstration garden within the development. The developer shall coordinate with a local or state agency that specializes in the design or certification of such gardens. Informational signage regarding the purpose of the garden and selected vegetation shall be provided.			
Goal 13. Improve soil quality to be amenable for a variety of native and non-invasive			
plantings. To encourage the establishment of healthy plants, reduce fertilizers, and reduce stormwater runoff, topsoil shall be retained on site and a minimum of 4 inches of topsoil shall be placed within disturbed areas.			
Goal 14. Increase perimeter buffer requirements, especially in transitional areas			
(nonresidential to residential areas). The UDO requires afoot buffer along theperimeter of the property. The applicant is proposing afoot buffer in that location, which would be an increase offeet above the requirement.			
Goal 15. Add information signage or other marking at the boundary of lots when they are adjacent to a wooded or natural condition resource conservation area (RCA) indicating that the area beyond the sign is RCA and is not to be disturbed. a. The project shall install signage adjacent to wooded or natural condition Resource Conservation Area. The signage shall indicate that the area is RCA and is to be preserved in perpetuity and not disturbed. OR b. A farm-style split rail fence shall be installed where wooded or natural condition			
Resource Conservation Area (RCA) abuts individual residential lots.	Ш	Ш	
SUSTAINABLE BUILDINGS (16)	YES	NO	N/A
Goal 16. Apply for green building certifications, such as LEED, Energy Star, BREEAM, Green Globes, NGBS Green, or GreenGuard. The project shall be designed to meet the requirements forgreen building certification. A third-party consultant shall be hired to evaluate the project and certify to the Town of Apex that the project meets the standards for the certification. The applicant shall forward a copy of the certification application to the Town of Apex Planning Department to verify that the application has been submitted.			
WASTE REDUCTION (17)	YES	NO	N/A
Goal 17. Install pet waste stations in public areas for multi-family, apartments, or condominiums or dog friendly businesses.			

WASTE REDUCTION (17)			N/A
The project shall install at least pet waste stations throughout the community, in locations that are publicly accessible, such as adjacent to amenity centers, SCMs, sidewalks, greenways or side paths.			
CLEAN ENEDGY/40 20\	VEC	NO	21/2
CLEAN ENERGY (18-20)	YES	NO	N/A
Goal 18. Install rooftop solar on buildings.			
 a. A solar PV system shall be incorporated into buildings to be constructed on the property. Such PV systems shall have a capacity of not less than 2 kW/1,00 heated square feet of building floor area. OR 	l l		
b. A solar PV system of at least 3.5kW shall be installed on at least% of buildings within the development. All solar installation required by th condition shall be completed or under construction prior to % of the building permits being issued for the development. The buildings on which the PV systems are located shall be identified on the Site Plan, which may be amended from time to time.	ne		
OR			
 The amenity center for the project shall include a rooftop solar PV system with capacity of at least kWHs. 	a		
Goal 19. Include solar conduit in building design.			
The project shall install conduit for solar energy systems for all non-resident buildings. The roof shall also be engineered to support the weight of a future rooft solar PV system.			
Goal 20. Encourage clean transportation.			
 a. The installation of EV charging spaces shall not reduce the width of adjace sidewalk to less than 5 feet. 	nt 🗆		
AND/ORb. EV charging spaces shall be located such that the cords shall not cause a tr hazard.	ip 🗆		
AND/OR c. The developer shall provide 5% of all parking spaces as EV charging spaces.			
c. The developer shall provide 5% of all parking spaces as EV charging spaces.			
LIGHTING EFFICIENCY (21-24)	YES	NO	N/A
Goal 21. Include energy efficient lighting in building design. Option 21.1: Increase the use of LEDs. The exterior lighting for all multi-family and commercial buildings and parking lower will consist entirely of LED fixtures.	ts		
Option 21.2: Lower maximum foot-candles outside of buildings. On the lighting plan, the average footcandle measurement for parking, building lighting and driveways shall be at least 0.5 footcandles lower than the UDO requires.			
Goal 22. Install timers or light sensors or smart lighting technology.			
The project shall install light timers, motion sensors, or other smart lighting technology for all exterior lighting.	ng 🔲		
Goal 23. Include International Dark Sky Association compliance standards. The project shall use full cutoff LED fixtures that have a maximum color temperatu of 3000K for all exterior lighting, including, but not limited to, parking lot and building mounted fixtures.			

Applicant Clarification/Additional Language:
The proposed PUD Amendment is exempt from EAB review. UDO Section 2.1.9.A).2).a) states:
The following conditional rezoning requests shall be exempt from review by the Board:
a) Rezonings to amend zoning conditions which have no environmental impact on a site including but not limited to revisions to architectural standards, building height setbacks, and uses.
This PUD Amendment proposes revisions to existing PUD conditions related to transportation nfrastructure improvements, but does not propose any amendments to conditions which have an environmental impact.
Additional Board Recommendations:

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This or dis	closed to third parties. 1, 2025	RHOOD WIEETING North Carolina Public Records Act and may	be published on the Town's website
Dat	e		
You	Neighbor: are invited to a neighborhood mee 7 Kelly Road	ting to review and discuss the develop	ment proposal at
	Address(es)		PIN(s)
for the amail. Development mon	the applicant to discuss the projection has before the protection of the projection	eighborhood Meeting procedures. This sect and review the proposed plans is submittal of an application to the To uss any concerns about the impacts of please refer to the Project Contact Information equest that the applicant provide updated submitted to the Town, it may be evelopment Report located on the point for Rezoning must hold a second earing date.	with adjacent neighbors and wn. This provides neighbors an the project before it is officially mation page for ways to contact ates and send plans via email or tracked using the <u>Interactive</u> Town of Apex website at Neighborhood Meeting in the
_		ecause this project includes (check all t	Approving Authority
✓ Ap	plication Type Rezoning (including Planned Unit D	evelopment)	Town Council
	Major Site Plan		Technical Review Committee (staff)
	-	care facility", "Government service", urant, drive-through", or "Convenience	Technical Review Committee (staff)
	Special Use Permit		Board of Adjustment (QJPH*)
	Residential Master Subdivision Plan	n (excludes exempt subdivisions)	Technical Review Committee (staff)
*Qu	asi-Judicial Public Hearing: The Board	d of Adjustment cannot discuss the proje	ect prior to the public hearing.
The	following is a description of the pro	posal (also see attached map(s) and/o	r plan sheet(s)):
site		oning to amend conditions of the non-residential uses, including, be space.	•
Esti	mated submittal date: Submitted	on February 3, 2025	
MI	EETING INFORMATION:		
	operty Owner(s) name(s):	Fahey Family Farm, LLC	
	plicant(s):	Trilandco, LLC c/o Collier R. Marsh	25 4002
	ntact information (email/phone):	colliermarsh@parkerpoe.com; (919) 83 Virtual (Zoom) - See attached notice let	
	eeting Address: te/Time of meeting**:	April 15, 2025 (5:30 - 6:30 PM)	IIGI
Da	te, mile of meeting .		

Project Presentation: 5:30 PM

Welcome: 5:30 PM

Question & Answer: 6:00PM

^{**}Meetings shall occur between 5:00 p.m.-9:00 p.m. on a Monday through Thursday (excluding Town recognized holidays). If you have questions about the general process for this application, please contact the Planning Department at 919-249-3426. You may also find information about the Apex Planning Department and on-going planning efforts at http://www.apexnc.org/180.



To: Neighboring Property Owners and Tenants

From: Collier R. Marsh Date: April 1, 2025

Re: Notice of Second Virtual Neighborhood Meeting

You are invited to attend a second virtual neighborhood meeting on April 15, 2025 at 5:30 PM to discuss 25CZ05, the proposed rezoning and site plan applications for an approximately 6.19-acre property located at 2517 Kelly Road (PIN 0731434504). The rezoning will amend the West Village PUD zoning conditions that currently apply to the property and rezone the property from Planned Unit Development Conditional Zoning (PUD-CZ) to PUD-CZ with revised conditions. The site plan application will allow for the development of non-residential uses on the property, including, but not limited to, a day care facility and office space.

During the meeting, the applicant will describe the nature of the rezoning request, provide updates since the first neighborhood meeting, and field any questions from the public. Enclosed are: (1) a vicinity map outlining the location of the property; (2) a zoning map of the area; (3) a copy of the PUD Plan, (4) a draft site plan, (5) a Transportation Plan exhibit, (6) a project contact information sheet; and (7) a common construction issues & who to call information sheet.

The meeting will be held virtually. You can participate online via Zoom or by telephone. To participate in the Zoom online meeting:

Visit: https://zoom.us./join

Enter the following meeting ID: 875 3788 7197

Enter the following password: 180565

To participate by telephone:

Dial: 1 929 205 6099 Enter the following meeting ID: 875 3788 7197

Enter the Participant ID: #

Enter the Meeting password: 180565

If you have any questions about this rezoning, please contact me at (919) 835-4663 or via email at <u>colliermarsh@parkerpoe.com</u>.

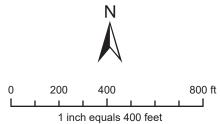
Sincerley,

Collier R. Marsh



2517 Kelly Road

Vicinity Map



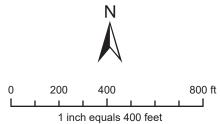
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2517 Kelly Road

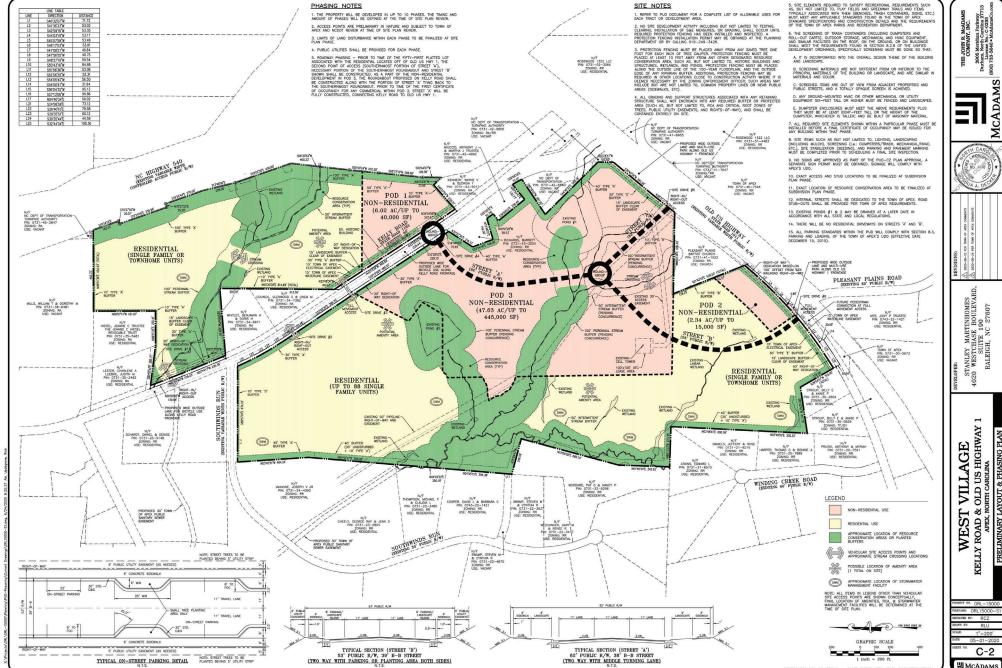
Zoning Map

Current Zoning: PUD-CZ

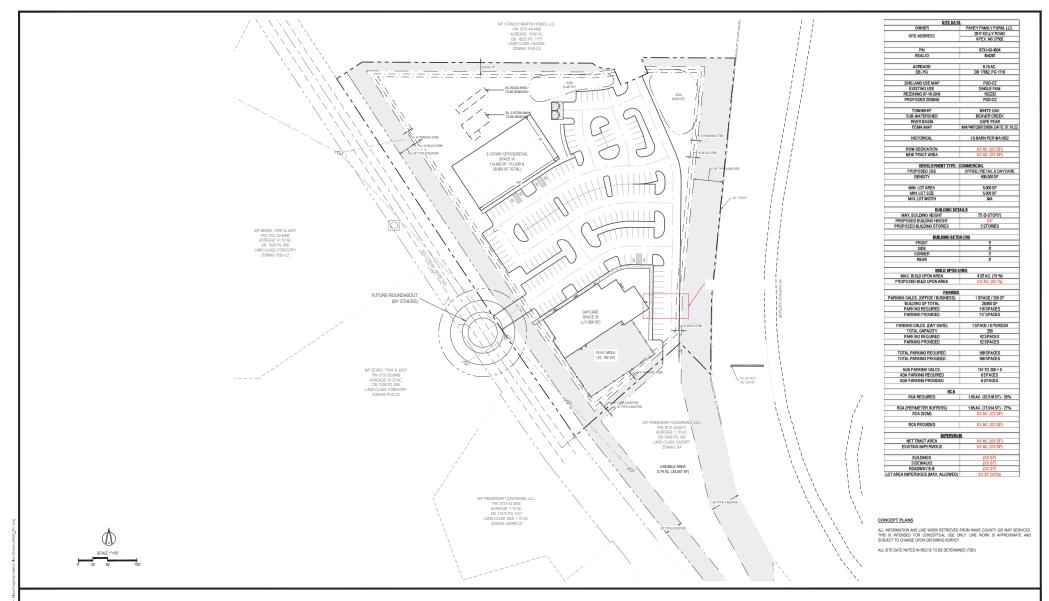


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MCADAMS



PEAK
Engineering & Design
1123 Apar Prokwor | Apar, NC 27502
phi 919-439-0100
vvv-PeakEn gineering.com

KELLY ROAD COMMERCIAL 2517 KELLY ROAD

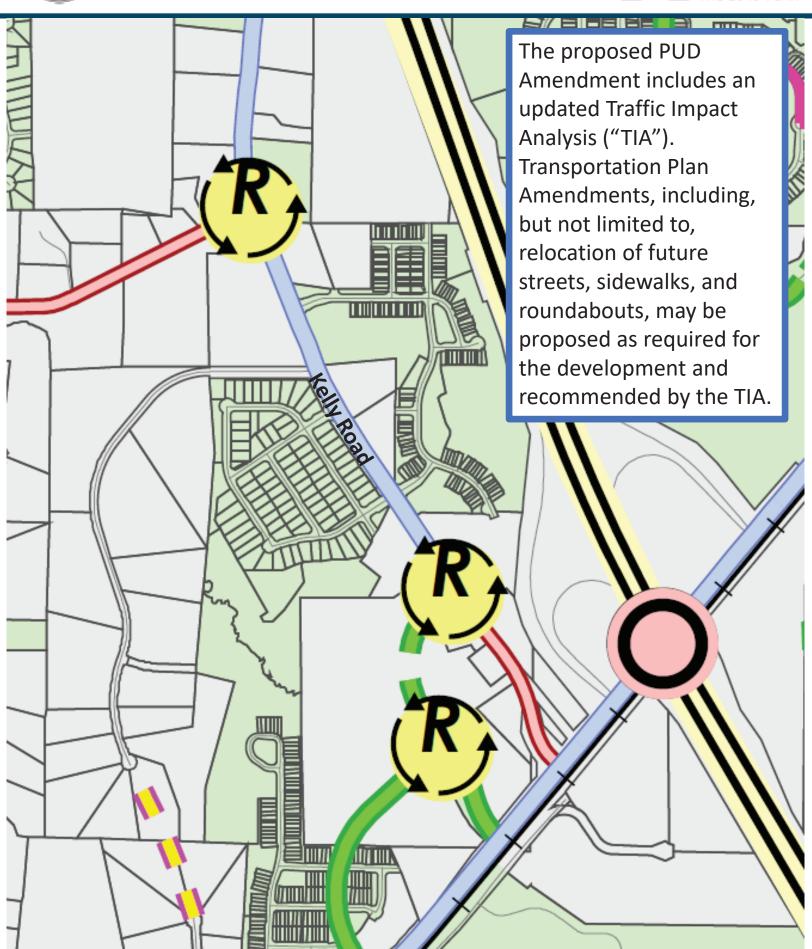
2517 KELLY ROAD APEX, NC WAKE COUNTY

DRAWN ST:	CHICKED IN	PROJECTION.	
MD	JR.	240403	
DATE		SHIEFHO.	
01.09.25		SP-1	



Thoroughfare and Collector Street Plan





PROJECT CONTACT INFORMATION

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Development Contacts:						
Project Name: West Village PUD Am	endment			Zc	oning: PUD-CZ	
Location: 2517 Kelly Road						
Property PIN(s): 0731434504	Acreage	e/Square	Fee	t: <u>6</u> .′	19 ac	
Property Owner: Fahey Family Farm, L	LC					
Address: 1115 Capitata Crossing						
City: Apex		State:	NC		zip: 27502-9011	
	nail:					
Developer: Trilandco, LLC c/o Collier R	Marsh					
Address: 301 Fayetteville Street, Suit	te 1400					
City: Raleigh	State:	NC			zip: 27601	
Phone: (919) 835-4663 Fax:	N/A			Email:	colliermarsh@parkerpoe.com	
Engineer: Peak Engineering & Design, PLLC, attn: Jeff Roach						
Address: 1125 Apex Peakway						
City: Apex		State:	NC	;	Zip: 27502	
Phone: (919) 439-0100 Fax:	N/A			Email:	jroach@peakengineering.com	
Builder (if known): Trilandco, LLC c/o Collier R. Marsh						
Address: 301 Fayetteville Street, Suite 1400						
City: Raleigh		State:	NC		Zip: 27601	
Phone: (919) 835-4663 Fax:	N/A			Email:	colliermarsh@parkerpoe.com	

Please note that Town staff will not have complete information about a proposed development until the application is submitted for review. If you have a question about Town development standards and how they relate to the proposed development, please contact the appropriate staff person listed below.

Town of Apex Department Contacts				
Planning Department Main Number				
(Provide development name or location to be routed to correct planner)	(919) 249-3426			
Parks, Recreation & Cultural Resources Department				
Angela Reincke, Parks Planning Project Manager	(919) 372-7468			
Transportation & Infrastructure Development				
Russell Dalton, Traffic Engineering Manager	(919) 249-3358			
Water Resources Department				
Jessica Bolin, Environmental Engineering Manager (Stormwater, Sedimentation &	(919) 249-3537			
Erosion Control)				
Matt Reker, Utility Engineer/FOG Program Manager (Water & Sewer)	(919) 946-4394			
Electric Utilities Division				
Rodney Smith, Electric Technical Services Manager	(919) 249-3342			

OWNER	MAILING ADDRESS	
ARYA, ANJALI	1963 FAHEY DR	APEX NC 27502-7093
BELLAM, ANUDEEP VUNNAM, SAI KAMALA	1971 FAHEY DR	APEX NC 27502-7093
BHAVSAR, DEWANG S BHAVSAR, DIPTI PATEL	2010 LEO DR	APEX NC 27502-4361
BRAHMANDAY, GOVINDA RAGHURAM PALURU, HIMABINDU	829 NIJINSKI WAY	CARY NC 27519-6740
COHEN, STEVEN A SLUSS-COHEN, ILDAURA TANISHA	608 PENINSULA FOREST PL	CARY NC 27519-1605
COUNCIL, GLENWOOD C COUNCIL, LINDA M	2411 KELLY RD	APEX NC 27502-9589
FAHEY FAMILY FARM LLC	1115 CAPITATA XING	APEX NC 27502-9011
FRIENDSHIP COWORKING LLC	2121 MCKENZIE RIDGE LN	APEX NC 27502-6629
GUPTA, MANIK GUPTA, RAJASAVI	842 PEPPER TREE LN	SANTA CLARA CA 95051-5227
GUPTA, SANDEEP RAMBACHAN RAI, SWETA	1952 FAHEY DR	APEX NC 27502-7093
HARGROVE, CATHERINE HEART HARGROVE, CHRISTOPHER EARL	2000 LEO DR	APEX NC 27502-4361
KARUPPASWAMY, RAMKUMAR KARUPPUSWAMY, RADHIKKAA	1950 FAHEY DR	APEX NC 27502-7093
LI, JUAN	1954 FAHEY DR	APEX NC 27502-7093
LU, YI-TIEN WANG, NAN-YI	1977 FAHEY DR	APEX NC 27502-7093
MALATHI, MOGAN RAJ NADARAJAN BABU, PRIYADARSHINI	1970 FAHEY DR	APEX NC 27502-7093
MIUCCIO, ANTHONY J TRUSTEE MIUCCIO, MARTHA J TRUSTEE	PO BOX 2063	APEX NC 27502-1154
MUDDASANI, SANDEEP REDDY MUDDASANI, SHILPA GOUD	2008 LEO DR	APEX NC 27502-4361
MUTHALIP ABUBAKKAR, SHEIK ABDUL BASHEER, ABIYABARVIN AHAMED	1983 CUSTOM LN	APEX NC 27502-4349
PAREEK, ABHISHEK	1967 FAHEY DR	APEX NC 27502-7093
PATURKAR, SHREYAS JITENDRA SHINTRE, PALLAVI SHRINIVAS	1957 FAHEY DR	APEX NC 27502-7093
RAGHUNATHAN, RAGHUNATHAN	2002 LEO DR	APEX NC 27502-4361
RAYALA, GIRIDHAR	1973 FAHEY DR	APEX NC 27502-7093
SAMANTA, SOURISHMOY SINHA, PARINITA	1959 FAHEY DR	APEX NC 27502-7093
SEARS, TONY C SEARS, JUDY T	2508 KELLY RD	APEX NC 27502-9563
SHEN, SI	4009 RATCLIFFE	BENTONVILLE AR 72713-7650
SHIELDS LAND LLC	PO BOX 65	HOLLY SPRINGS NC 27540-0065
SOTOS, GUSTAVO REZENDE DOS SGARBI, ROXANE TASSI	2006 LEO DR	APEX NC 27502-4361
STANLEY MARTIN HOMES LLC	11710 PLAZA AMERICA DR STE 1100	RESTON VA 20190-4771
THAKURI, SUMAN CHAND, ARPANA	1956 FAHEY DR	APEX NC 27502-7093
ZISKIN, NATALIE PARHAM, ANDREW	1974 FAHEY DR	APEX NC 27502-7093
Current Tenant	1947 Fahey DR	APEX NC 27502
Current Tenant	1949 Fahey DR	APEX NC 27502
Current Tenant	1966 Fahey DR	APEX NC 27502
Current Tenant	1975 Fahey DR	APEX NC 27502
Current Tenant	2517 Kelly RD	APEX NC 27502
Current Tenant	2524 Kelly RD	APEX NC 27502
Current Tenant	2601 Kelly RD	APEX NC 27502
Current Tenant	2604 Kelly RD	APEX NC 27502
Current Tenant	2004 Leo DR	APEX NC 27502

COMMON CONSTRUCTION ISSUES & WHO TO CALL

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Noise & Hours of Construction: Non-Emergency Police

Noise from tree removal, grading, excavating, paving, and building structures is a routine part of the construction process. The Town generally limits construction hours from 7:00 am to 8:30 pm so that there are quiet times even during the construction process. Note that construction outside of these hours is allowed with special permission from the Town when it makes more sense to have the construction occur at night, often to avoid traffic issues. In addition, the Town limits hours of blasting rock to Monday-Friday from 8:00 am to 5:00 pm. Report violations of construction hours and other noise complaints to the Non-Emergency Police phone number at 919-362-8661.

Construction Traffic:

Infrastructure Inspections

919-249-3386

Construction truck traffic will be heavy throughout the development process, including but not limited to removal of trees from site, loads of dirt coming in and/or out of the site, construction materials such as brick and wood brought to the site, asphalt and concrete trucks come in to pave, etc. The Town requires a construction entrance that is graveled to try to prevent as much dirt from leaving the site as possible. If dirt does get into the road, the Town can require they clean the street (see "Dirt in the Road" below).

Road Damage & Traffic Control:

Infrastructure Inspections

There can be issues with roadway damage, roadway improvements, and traffic control. Potholes, rutting, inadequate lanes/signing/striping, poor traffic control, blocked sidewalks/paths are all common issues that should be reported to Transportation & Infrastructure Development – Infrastructure Inspections at 919-249-1109. The Town will get NCDOT involved if needed.

Parking Violations:

Non-Emergency Police

Unless a neighbor gives permission, there should be no construction parking in neighbors' driveways or on their property. Note that parking in the right-of-way is allowed, but Town regulations prohibit parking within 15 feet of driveways so as not to block sight triangles. Trespassing and parking complaints should be reported to the Non-Emergency Police phone number at 919-362-8661.

Dirt in the Road:

Water Resources

Sediment (dirt) and mud gets into the existing roads due to rain events and/or vehicle traffic. These incidents should be reported by visiting the Report a Concern page at https://www.apexnc.org/1173/ or by calling the number listed. Staff will coordinate the cleaning of the roadways with the developer.

Dirt on Properties or in Streams:

Water Resources

919-362-8166

Sediment (dirt) can leave the site and get onto adjacent properties or into streams and stream buffers; it is typically transported off-site by rain events. These incidents should be reported by visiting the Report a Concern page at https://www.apexnc.org/1173/ or by calling the number listed so that staff can coordinate the appropriate repairs with the developer.

Dust:

Water Resources

919-362-8166

During dry weather dust often becomes a problem blowing into existing neighborhoods or roadways. These incidents should be reported by visiting the Report a Concern page at https://www.apexnc.org/1173/ or by calling the number listed so that staff can coordinate the use of water trucks onsite with the grading contractor to help control the dust.

Trash:

Water Resources

919-362-8166

Excessive garbage and construction debris can blow around on a site or even off of the site. These incidents should be reported by visiting the Report a Concern page at https://www.apexnc.org/1173/ or by calling the number listed. Staff will coordinate the cleanup and trash collection with the developer/home builder.

Temporary Sediment Basins:

Water Resources

Temporary sediment basins during construction (prior to the conversion to the final stormwater pond) are often quite unattractive. Concerns should be reported by visiting the Report a Concern page at https://www.apexnc.org/1173/ or by calling the number listed so that he can coordinate the cleaning and/or mowing of the slopes and bottom of the pond with the developer.

Stormwater Control Measures: Water Resources

Post-construction concerns related to Stormwater Control Measures (typically a stormwater pond) such as conversion and long-term maintenance should be reported by visiting the Report a Concern page at https://www.apexnc.org/1173/ or by calling the number listed.

Electric Utility Installation:

Rodney Smith

919-249-3342

Concerns with electric utility installation can be addressed by the Apex Electric Utilities Department. Contact Rodney Smith at 919-249-3342.

NEIGHBORHOOD MEETING SIGN-IN SHEET

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Meeting Address: Virtual via Zoom	
Date of meeting: April 15, 2025	Time of meeting: 5:30pm - 6:30pm
Property Owner(s) name(s): See attached	
Applicant(s): Trilandco, LLC	

Please <u>print</u> your name below, state your address and/or affiliation with a neighborhood group, and provide your phone number and email address. Providing your name below does not represent support or opposition to the project; it is for documentation purposes only. For virtual meetings, applicants must include all known participants and request the information below.

	NAME/ORGANIZATION	ADDRESS	PHONE #	EMAIL	SEND PLANS & UPDATES
1.	Brian Griffith	2524 & 2601 Kelly Rd, Apex, NC 25702			
2.	Pat Fahey	2517 Kelly Road			
3.	Phone attendee/name unknown				
4.					
5.					
6.					
7.					
8.					
9.					
10.					
11.					
12.					
13.					
14.					

Use additional sheets, if necessary.

SUMMARY OF DISCUSSION FROM THE NEIGHBORHOOD MEETING

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

Property Owner(s) name(s): See attached
Applicant(s): Trilandco, LLC
Contact information (email/phone): Collier R. Marsh; (919) 835-4663; colliermarsh@parkerpoe.com
Meeting Address: Virtual via Zoom
Date of meeting: April 15, 2025 Time of meeting: 5:30pm - 6:30pm
Please summarize the questions/comments and your responses from the Neighborhood Meeting or emails/phone calls received in the spaces below (attach additional sheets, if necessary). Please state if/how the project has been modified in response to any concerns. The response should not be "Noted" or "No Response". There has to be documentation of what consideration the neighbor's concern was given and justification for why no change was deemed warranted. Question/Concern #1: Will the project be extending utilities?
Applicant's Response: Yes, the POD 1 project will be extending utilities across the site.
Question/Concern #2: What frontage improvements will be provided? Applicant's Response: The Pod 1 project will include all of the Town's required improvements on the Kelly Road frontage and will dedicate right of way for a future roundabout.
dedicate right of way for a future roundabout.
Question/Concern #3: What is the anticipated hearing schedule?
Applicant's Response: The project is tracking for hearings in May.
Question/Concern #4:
Applicant's Response:

AFFIDAVIT OF CONDUCTING A NEIGHBORHOOD MEETING, SIGN-IN SHEET AND ISSUES/RESPONSES SUBMITTAL

This document is a public record under the North Carolina Public Records Act and may be published on the Town's website or disclosed to third parties.

I, Colli	er R. Marsh	, do hereby declare as follows:	
	Print Name		
1.		d Meeting for the proposed Rezoning, Major Site Plar Plan, or Special Use Permit in accordance with I	
2.	abutting and within 300 feet of th	iled to the Apex Planning Department, all property ow he subject property and any neighborhood associatio a first class mail a minimum of 14 days in advance of t	n that represents
3.	The meeting was conducted at _	virtually via Zoom	location/address
	on 4/15/25	(date) from5:30pm(start time) to6:30pm	n(end time).
4.	I have included the mailing list, m map/reduced plans with the appl	neeting invitation, sign-in sheet, issue/response sum lication.	mary, and zoning
5.	I have prepared these materials in	n good faith and to the best of my ability.	
4/	23/25 Date	By: Callier R. Marsh	*
	OF NORTH CAROLINA Y OF WAKE		
Sworn a	and subscribed before me, Amy, on this the 23rd day of A	w. Stephenson, a Notary Public for the a	above State and
	SEAL NOTARY OF PUBLIC CONTINUES AND TARY OF THE COUNTY OF	Any W. Stephenson Print Name My Commission Expires: October	2,2028



Matthew J. Carpenter

Attorney
t: 919-835-4032
MatthewCarpenter@parkerpoe.com

Atlanta, GA
Charleston, SC
Charlotte, NC
Columbia, SC
Greenville, SC
Raleigh, NC
Spartanburg, SC
Washington, DC

February 3, 2025

Via Town of Apex IDT

Planning Town of Apex 322 N. Mason Street Apex, NC 27502

RE: West Village PUD Amendment for 2517 Kelly Road; PIN 0731434504; West Village PUD POD 1 (the "Property")

To Whom It May Concern,

This letter is to inform you of a proposed PUD Amendment to rezone the Property from Planned Unit Development Conditional Zoning (PUD-CZ) to PUD-CZ with amended conditions to allow an approximately 40,000 square foot non-residential development (the "Project").

In 2016, the Property was rezoned to PUD-CZ as part of the overall West Village PUD development (Rezoning 15CZ33, the "2016 PUD") and in 2020, the PUD was amended (Rezoning 20CZ05) to modify buffers and other design standards (collectively, the "West Village PUD"). The West Village PUD permits up to 380 residential units and up to 500,000 square feet of non-residential uses across 163.34 acres as shown on the West Village PUD Preliminary Layout and Phasing Plan dated May 1, 2020 (the "Layout Plan"). Since 2016, several residential phases have been developed, but the non-residential phases have not developed.

The West Village PUD includes significant road improvements for residential and non-residential development. Though it specifies phasing for certain road improvements in the residential district, it lacks phasing for transportation infrastructure improvements in the commercial district and all road improvements are required prior to issuance of the first certificate of occupancy for a commercial building. Accordingly, as currently written, the PUD would require the Project - a relatively small 40,000 sf non-residential development - to construct additional through lanes, on ramps, and traffic signals that were required for 500,000 sf of non-residential uses approved in the West Village PUD.

To remedy this oversight, the developer completed an updated Traffic Impact Analysis ("TIA") which recommends road improvements based on the anticipated traffic impacts of the Project. This PUD Amendment proposes technical revisions to West Village PUD Section 13: Public Facilities, as set forth in the attached **Exhibit A**, to incorporate the updated TIA recommendations. Road improvements that were commitments of the original West Village PUD remain, but will be completed by subsequent non-residential phases.

A corresponding PUD-CZ rezoning application has been filed in the Town's development portal.

Sincerely,

Exhibit A

to

West Village PUD Amendment Letter Revisions to West Village PUD

Section 13: Public Facilities

The proposed PUD shall meet all Public Facilities requirements as set forth in UDO Section 2.3.4(F) (1) (f) and be designed according to sound engineering standards. Specifically, road and utility infrastructure shall be as follows:

General Roadway Infrastructure:

All proposed roadway infrastructure will be consistent with the Town of Apex UDO and Transportation Plan, and the Traffic Impact Analysis approved by the Town of Apex and NCDOT. An internal road network will be provided in accordance with the Town's UDO. All road networks will promote connectivity wherever possible to adjacent neighborhoods, undeveloped property, nearby points of interest, and municipal destinations. Further, cul-de-sacs will be avoided except where environmental features make through streets unfeasible.

Roadway Phasing – Prior to time of the fifty-first certificate of occupancy associated with the residential located off of Old US HWY 1, the second point of access (southernmost portion of Street A), necessary portion of the southernmost roundabout and Street B shown shall be constructed. As a part of the non-residential development in Pod 3, the roundabout proposed on Kelly Road shall be constructed along with the portion of Street A tying back to the southernmost roundabout. Prior to time of the first certificate of occupancy associated with POD 3, as part of the development of POD 3, Street A will be complete the connection from KellyRoad and Old US HWY 1.

Off-Site Transportation Conditions:

The project will also provide the following off-site transportation conditions:

All recommendations on state-maintained roadways are subject to NCDOT review and approval as part of the driveway permits and encroachment agreements. Build 2018 refers to the first plat of residential development. Build 2030 refers to the first plat of commercial development for POD 2 or POD 3 and does not include the development of POD 1, also identified as Build 2028, which is subject to separate recommendations set forth below. Please note that prior analysis and the original basis for build-out recommendations was based on a Build 2020 analysis. Internal Protected Storage Length (IPS) refers to the required minimum distance from the intersection along the proposed driveway or street before any full movement commercial driveway access or public street intersection will be allowed.

Developer shall provide right-of-way dedication along Kelly Road and Old US 1 based on a 100-foot right-of-way. Where Old US 1 abuts railroad right-of-way the developer shall be responsible for dedicating public right-of-way 70 feet from roadway centerline along the project frontage or as otherwise required to accommodate a 100-foot road right-of-way exclusive of railroad right-of-way.

Street 'A' and Street 'B' (including Kelly Road at Site Drive #4)

- Street 'A' shall be constructed as a 3-lane 38-foot curb and gutter street with 5-foot sidewalk on both sides on 62-foot public right-of-way.
- Street 'B' shall be constructed as a 2-lane 39'-foot curb and gutter street with onstreet parking and 6-foot sidewalk on both sides on 53-foot public right-of-way.
- Residential driveway access shall not be permitted along Streets 'A' and 'B'.
- Prior to platting the 51st residential unit in the Residential area located adjacent to Old US 1, developer shall construct and dedicate as public Street 'A' from Site Drive #5 to the roundabout at Street 'B', roundabout serving Street 'A' at Street 'B', and Street 'B' from Site Drive #6/Pleasant Plains Road to the roundabout at Street 'A'.
- Prior to the first certificate of occupancy within POD 3, developer shall construct and dedicate as public Street 'A' from the roundabout at Street 'B' to Kelly Road at Site Drive #4 and construct a roundabout on Kelly Road at Site Drive #4. The roundabout will serve a 4-lane divided roadway to the north and 2-lane roadway to the south.
 - POD 1: Kelly Road Roundabout. Developer shall prepare a preliminary design and engineer's estimate for review and approval and dedicate right of way and construction easements for the future construction of a roundabout at the intersection of the site driveway, Kelly Road, and Street A as shown on the Town of Apex Thoroughfare and Collector Street Plan (the "Roundabout"). Prior to site plan final plat for POD 1, Developer shall pay a fee in lieu for eight percent (8%) of the estimated costs to design and construct the Roundabout.

Kelly Road at Olive Chapel Road

- Developer shall construct a 200-foot eastbound right turn lane and a 300-foot additional westbound left turn lane on Olive Chapel Road (with southbound receiving through lane on Kelly Road) for Build 2030.
- POD 1: Developer shall provide a preliminary plan and engineer's estimate for review and approval and pay a fee in lieu in the amount of 8% of the total estimated cost of the aforementioned improvements prior to site plan final plat in POD 1.

Kelly Road at Apex Barbecue Road

Improvements have been completed by others satisfying the prior zoning requirements for a Build 2020 analysis including the following: construction of a 400-foot eastbound left turn lane, 350-foot westbound left turn lane, 350-foot northbound left turn lane, 150-foot northboundright turn lane, 350-foot southbound left turn lane, and 200-foot southbound right turn lane. No additional improvements are recommended for updated commercial build dates, Build 2028 (POD 1) and Build 2030 (PODS 2 & 3).

Kelly Road at Southwinds Run

Developer shall construct a 100-foot northbound left turn lane on Kelly Road, an additional (second) northbound through lane through the intersection to drop as aright turn at Site Drive #1, and begin an additional (second) southbound through lane immediately south of Southwinds Run for Build 2030.

Old US 1 at Kelly Road

- Developer shall construct a 100-foot westbound right turn lane on Old US 1 for Build 2018.
- Developer shall convert the intersection to right-in/right-out as well as construct an additional westbound through lane, beginning at the NC 540 Southbound offramp as a free-flow right turn exiting the ramp, along with a 200-foot westbound right turn lane on Old US 1 for Build 2030.

NC 540 Ramps at Old US 1

• Developer shall provide a free-flow right turn lane for the NC 540 Southbound offramp and additional receiving through lane continuing west to drop as a right turn at Site Drive #6 on Old US 1 for Build 2030.

Old US 1 at Pleasant Plains Road & Site Drive #6

- Developer shall construct Site Drive #6 with a through-right lane and a 200-foot left turn lane for Build 2018.
- Developer shall construct a 200-foot eastbound left turn lane and 200-foot westbound left turn lane on Old US 1 for Build 2018.
- Developer shall construct an additional westbound through lane on Old US 1 to drop as a right turn lane at Site Drive #6 for Build 2030.
- Developer shall construct an additional eastbound through lane on Old US 1 beginning 400 feet west of Site Drive #6 for Build 2030.

Kelly Road at Site Drive #1

 Developer shall construct Site Drive #1 as a right-in and right-out only providing 100 feet IPS and a 100-foot northbound right turn lane on Kelly Road for Build 2018. • Developer shall construct an additional northbound through lane on Kelly Road to drop as a right turn lane at Site Drive #1 for Build 2030.

Kelly Road at Site Drive #2

- Developer shall construct Site Drive #2 as a right-in and right-out only providing 100 feet IPS for Build 2018.
- Developer shall construct an additional northbound through lane and an additional southbound through lane on Kelly Road at Site Drive #2 for Build 2030.

Kelly Road at Site Drive #3

- Developer shall construct Site Drive #3 with a 100-foot eastbound right turn lane and a 100-foot westbound right turn lane on the Site Drive #3 approaches adjacent to through-right lanes with 200 feet IPS for Build 2018.
- Developer shall construct 100-foot northbound and 100-foot southbound left turn lanes on Kelly Road for Build 2018.
- Developer shall construct an additional (second) southbound through lane and additional (second) northbound through lane on Kelly Road at Site Drive #3 for Build 2030.

Old US 1 at Site Drive #5

- Developer shall convert Site Drive #5 from a right-in/right-out to a signalized full-movement intersection with 200-foot dual southbound left turn lanes and a 200-foot southbound right turn lane providing 300 feet IPS providing connectivity to both the residential and commercial phases for Build 2030.
- Developer shall construct an additional (second) westbound through lane and add a 200-foot westbound right turn lane on Old US 1 for Build 2030.
- Developer shall construct a 300-foot eastbound left turn lane and an additional (second) eastbound through lane on Old US 1 dropping 1000 feet east of the intersection for Build 2030.

Construction of the preceding roadway infrastructure improvements shall not be required for the development of POD 1. Fee in lieu payments and additional right of way dedication where required for POD 1 are noted in the above sections where applicable. Development of POD 1 shall include the below roadway infrastructure improvements which shall be consistent with the Traffic Impact Analysis on file with the Town of Apex prepared by DRMP dated 1/30/2025 and the Town of Apex Transportation Plan. The road improvements shall be subject to Town of Apex and North Carolina Department of Transportation approval.

 Kelly Road Widening. Developer shall dedicate right of way for the length of the property's Kelly Road frontage, measured a minimum of 55 feet from the existing centerline of Kelly Road, and widen and improve Kelly Road for the length of the property's Kelly Road frontage based on an 84-foot back-to-back curb and gutter 4-lane divided roadway with 5-foot sidewalks and 6-foot bike lanes in a 110-foot right of way.

• Kelly Road and Site Driveway. Developer shall construct:

- the Site Driveway with a stop-controlled approach, one ingress lane, and one egress lane; and
- a southbound left turn lane on Kelly Road with a minimum of 75 feet of storage.

• Electric Charging Stations:

Developer shall provide 2 charging stations, one within the residential and one within the non-residential for electric vehicles within the overall project. In addition to these committed stations, two additional charging stations will be installed as part of the overall project.

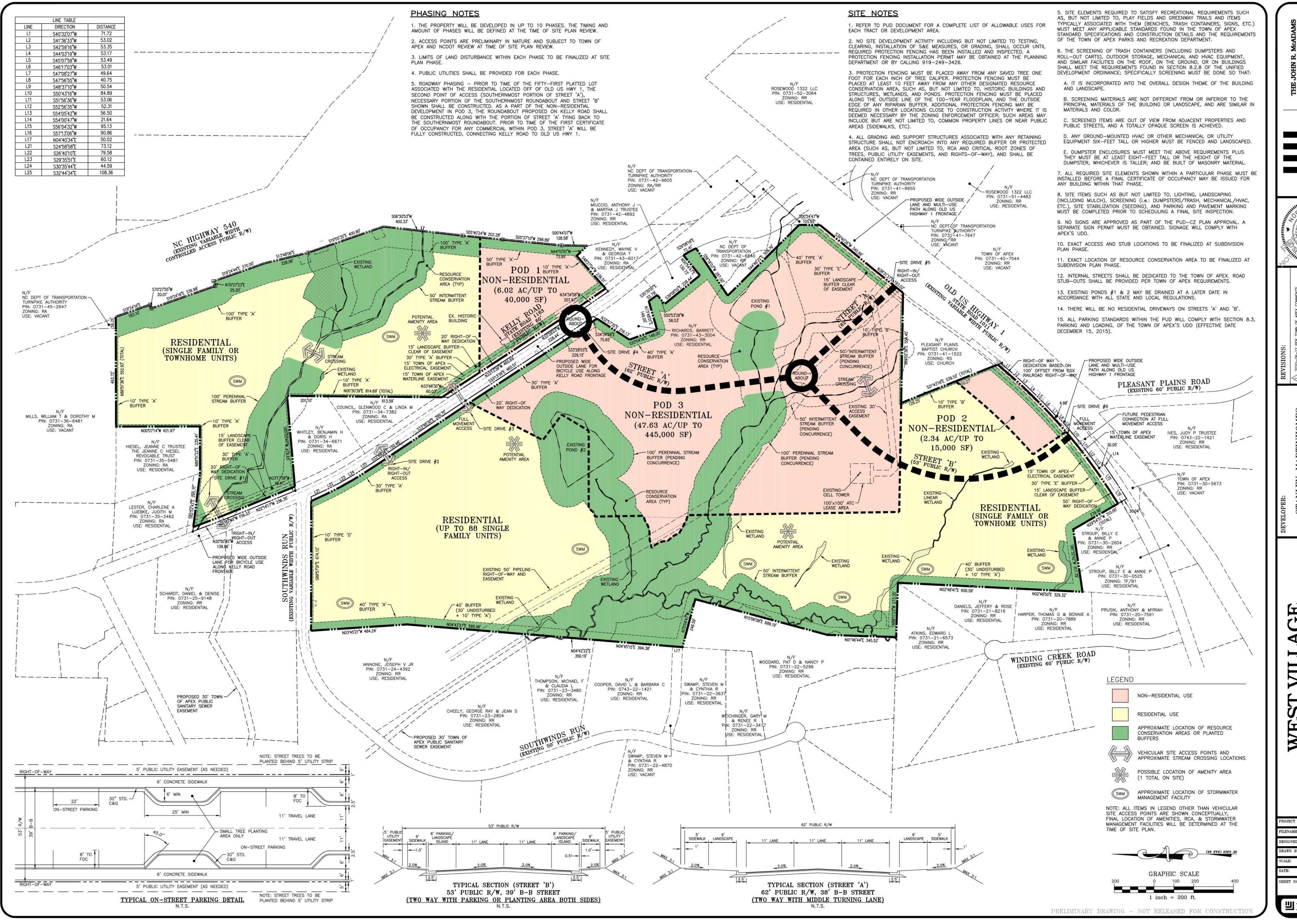
Water & Sewer Utilities:

All water and sanitary sewer service will be provided by the developer and conform to the Town of Apex Public Works and Utilities Department requirements. Preliminary location and tie in points are shown on sheet C-3 and C-4 of the PUD plans. The water extension shown along Kelly Road to Old US HWY 1, alternatively, could be located through Pod – 3 Commercial and southernmost residential accomplishing the intent of the routing shown on sheet C-3 and C-4. The ultimate routing will be dictated by timing of commercial development, roadway construction internal to the site and timing commitment related to the extension. However, this will be coordinated with the Town of Apex at time of site plan and construction documents.

Developer to provide \$75,000 in escrow for use by the Westwinds community for insurance associated with community wells, for use in drilling new wells or to extend public water into the community as a primary or secondary water source. Ultimately, the well-related use of the funds will be determined by the Westwinds community. Land owners will be required to annex into the Town of Apex prior to making any connections to public water services provided by the Town.

Other Utilities:

Electricity will be provided by Apex Electric. Phone, cable and gas will be provided by the developer and shall meet the Town of Apex standards as outlined in the UDO



ROJECT NO. ORL-15000 FILENAME: ORL15000-ESIGNED BY: RCZ RLU 1"=200

05-01-202

■ McAdams



January 30, 2025

DRMP #:2400935

Sajid Hassan
Town of Apex Transportation
105-B Upchurch Street
Apex, NC 27502
919-372-7360
Sajid.Hassan@apexnc.org

Subject: Phasing Study for Off-Site Improvements
West Village Development - Apex

Dear Mr. Hassan:

This letter provides a summary of the phasing study prepared for the initial phase of commercial development in the West Village PUD to determine off-site transportation improvements required for this phase of development. A Traffic Impact Analysis (TIA) was prepared in 2015 for the overall development as part of the PUD approval. This TIA was reviewed by the Town and NCDOT and required transportation improvements were identified in the conditions of approval at the Town (see attached). The improvements were not phased for the commercial portion of the PUD; therefore, this phasing study is requesting the improvements be phased.

Initial Commercial Phase

The initial phase of commercial development is proposed along the east side of Kelly Road and would include a day care (\sim 12,130 s.f. building) and a two-story building with office / retail space (\sim 29,000 s.f. total). The day care building would be completed initially, and the mixed-use building would be completed afterwards. This development is the first phase of the commercial portion of the West Village PUD. The subject property is approximately 8% of the overall West Village development.

Access would be provided via one new full movement driveway on Kelly Road and a connection to the north to Fahey Drive. The new full movement driveway is ultimately planned for a roundabout, but the roundabout would not be constructed with the initial phase of development. The connection to Fahey Drive would provide an indirect second access to Kelly Road via Leo Drive.



Scope of Study

The phasing study scope is based on the MOU submitted and approved by the Town. A copy of the approved MOU (dated October 30, 2024) is attached.

The study area includes intersections from the original TIA and PUD that have required improvements that are not constructed and that would be relevant to this phase. The study intersections include:

- Olive Chapel Road & Kelly Road (signalized)
- Apex Barbeque Road & Kelly Road (signalized)
- Leo Drive and Kelly Road (unsignalized)
- Kelly Road and Proposed Site Driveway (Site Driveway #4 in TIA)
- Old US 1 and Kelly Road

Traffic Volumes

Existing peak hour traffic volumes are based on traffic counts collected at the existing study intersections above during the weekday AM (7:00 AM – 9:00 AM) and PM (4:00 PM – 6:00 PM) peak periods in November 2024. Count data is attached. Existing 2024 lanes and peak hour traffic volumes are shown in the attached figures.

No-Build Traffic Volumes

No-build traffic volumes were calculated by growing the existing peak hour traffic volumes to the future years using an annual growth rate of 4%.

In addition to annual traffic growth, the following adjacent development trips were added to the future year traffic volumes:

- Depot 499
- Townes at Pleasant Park (fka Sears Property)
- Friendship Village
- Holland Road Assembly

Adjacent development trips were taken from the TIA reports prepared for each development. Some homes within the Depot 499 development were occupied and the trips captured in the traffic counts; however, to be conservative, no reduction in the adjacent development trips was made in this study. Residential trips for the Depot 499 development were included as adjacent development for future 2028 conditions, while full build out trips were included in future 2030 conditions.

Adjacent development information is provided in the attachments. Refer to the attached figures for the projected (2028 and 2030) peak hour traffic volumes, adjacent development trips, and no-build (2028 and 2030) peak hour traffic volumes.



Site Trip Generation

Average weekday daily, AM peak hour, and PM peak hour trips for the initial commercial development phase were estimated using methodology contained within the ITE *Trip Generation Manual*, 11th Edition. Table 1 provides a summary of the trip generation potential for this phase of development.

Table 1: Trip Generation Summary - Initial Commercial Phase

Land Use (ITE Code)	Intensity	Daily Traffic (vpd)		Weekday ak Houi (vph)		Weekday PM Peak Hour Trips (vph)			
		(vpu)	Enter	Exit	Total	Enter	Exit	Total	
Day Care (565)	12,130 SF	576	71	62	133	63	72	135	
General Office (710)	14,400 SF	215	28	4	32	6	27	33	
Retail Shopping Center (822)	14,400 SF	837	22	15	37	50	51	101	
Total Trip	s	1,628	121	81	202	119	150	269	
Internal Captur AM (4% Enter, 10 PM (3% Enter, 2)% Exit)		-2	-2	-4	-2	-2	-4	
Pass-By Tri (Retail - 31% AM,			-6	-5	-11	-20	-20	-40	
Primary (New)	Trips	1,628	113	74	187	97	128	225	

Internal capture trips were determined based on the NCHRP Report 684. As shown, a relatively low number of internal capture trips are expected within the initial commercial phase. Pass-by trips were calculated for the retail space using the ITE *Trip Generation Manual*.

Refer to Table 2, on the following page, for a summary of the full build out trip generation from the original TIA. The trip generation for full build out includes assumptions for the phase 1 commercial area as well as trips from some residential development that is already completed.



Table 2: Trip Generation Summary - Full Build (from Original TIA)

Description	Weekday 24-Hour Volumes	AM I Hour	kday Peak Trips oh)	PM Pea	kday ak Hour (vph)
		Enter	Exit	Enter	Exit
East of Kelly Road	2,498	176	154	163	163
West of Kelly Road	31,192	1,087	723	1,237	1,434
Fast Food Outparcel	2,481	116	111	85	78
Total Site Trips	36,171	1,379	988	1,485	1,675
East of Kelly Road	214	-	1	2	2
West of Kelly Road	10,956	82	82	<i>7</i> 9	<i>7</i> 9
Fast Food Outparcel	1,240	18	18	4	4
Pass-By Trips	12,410	100	100	85	85
Total Primary (New) Trips	23,761	1,279	888	1,400	1,590

As shown in Tables 1-2, the initial commercial phase is expected to generate approximately 8% of the total full build out trips analyzed in the TIA.

Site Trip Distribution and Assignment

Site trips are distributed based on the regional trip distribution from the original TIA report and modified as appropriate near the site driveway. Refer to the attached site trip distribution and assignment figures. Although there is interconnectivity to the north, this traffic study assigns all development trips to the new site access to be conservative.



Capacity Analysis

Study intersections were analyzed during the weekday AM and PM peak hours for the following traffic scenarios:

- Existing 2024 Conditions
- No-Build 2028 (Phase 1 Build Year) Conditions
- Build 2028 Phase 1 Conditions
- No-Build 2030 (Full Build Year) Conditions
- Build 2030 (Full Build Out) Conditions with All Required Improvements

Capacity analyses were performed utilizing Synchro (Version 11). Synchro reports and intersection results summary tables are attached. Tables 3 –7, attached, provide the intersection capacity analysis results for the study intersections.

This study assumes the initial commercial development (Phase 1) would be completed by 2028. Although a schedule is unknown for the remaining commercial development within the PUD, this study assumes a full build out year of 2030. Additional intersections beyond the study intersections are included in the Synchro analysis models for reference, but results are not provided for these locations in this study.

Future conditions analyses consider trips generated by numerous adjacent developments, several of which consist of residential uses. When trips for commercial and residential developments are added to each other, there is some double-counting of trips that can overestimate traffic volumes. Trips between residential and commercial uses are complementary and would not be added separately to the roadway network. While this study considers all trips as new added traffic, it should be noted that this is not expected to be the case.

Furthermore, the traffic growth projected for Kelly Road and reflected in this study is not likely to occur if conditions along the corridor are congested. There are alternative north-south routes to Kelly Road that can be utilized if/when congestion occurs on Kelly Road. Future traffic volumes and the following analysis results are therefore considered conservative.

Kelly Road and Olive Chapel Road

Analysis indicates that Phase 1 site trips are expected to have minimal impact on intersection operations. Overall intersection delay is expected to increase by approximately two seconds or less, when compared to 2028 no-build conditions, and queue lengths are expected to increase by no more than one vehicle length for any movement. No improvements are recommended for Phase 1.



2030 build conditions were analyzed with the required intersection improvements consisting of the following:

- Provide dual westbound left turn lanes.
- Provide an exclusive eastbound right turn lane.
- Signal phasing and timing adjustments to accommodate new lanes.

With the improvements above, the intersection is expected to operate at an overall LOS E during the AM peak hour and LOS F during the PM peak hour. Overall intersection delay is expected to be less than under 2030 no-build conditions.

Kelly Road and Apex Barbecue Road

This intersection was analyzed with existing lanes and traffic control. All improvements previously required of the development have been recently constructed by others.

Analysis indicates that Phase 1 site trips are expected to have minimal impact on intersection operations. Under 2028 build conditions, the intersection is expected to operate at an overall LOS C during the AM peak hour and LOS D during the PM peak hour.

Future 2030 conditions analyses indicate the potential for significant congestion on the Kelly Road approaches. As discussed previously, however, this analysis does not take into consideration complementary trips between developments and the likelihood that some traffic would choose an alternative route to avoid congestion.

Kelly Road and Leo Drive

The minor-street approaches of Leo Drive and the major-street left turn movements on Kelly Road are expected to operate at LOS C or better under future 2028 conditions. Phase 1 site trips are not expected to have a noticeable impact on intersection operations, and no improvements are recommended.

2030 Build conditions were analyzed with Kelly Road as a four-lane divided roadway. Analysis indicates the Leo Drive approaches may experience longer delays, particularly in the PM; however, queues are projected to be short (one or two vehicles).

Kelly Road and Old US 1

The southbound approach of Kelly Road currently operates at LOS F and is expected to continue to do so under future 2028 conditions. The addition of Phase 1 site trips is projected to increase queue lengths on the Kelly Road approach by approximately 105 feet (about four vehicles) in the AM peak hour and 152 feet (about six vehicles) in the PM peak hour.



A traffic signal was considered to reduce delays on Kelly Road. Due to the proximity of this intersection to the NC 540 interchange, NCDOT is not expected to allow signalization of the intersection. Extension of the existing southbound right turn lane was also considered. Southbound left turn movement queues are longer than the right turn movement queues, however, and a 100-150 foot extension of the right turn lane may provide limited benefit.

Under 2030 build conditions, the intersection was analyzed as a right-in/right-out (RIRO) and Old US 1 was analyzed with two eastbound and westbound through lanes. The southbound approach of Kelly Road is expected to operate at LOS D in the AM peak hour and LOS F in the PM peak hour as a RIRO. If delay and queues are long on Kelly Road, it is likely traffic will divert to alternative routes. The RIRO configuration at the intersection is not recommended until future development occurs within the West Village PUD to provide an alternate connection for Kelly Road to Old US 1 with a traffic signal that has more separation from the NC 540 interchange.

Kelly Road and Site Driveway

The site driveway and major-street left turn movement on Kelly Road are projected to operate at LOS C or better under 2028 no-build conditions.

Auxiliary turn lanes were considered based on the NCDOT *Policy on Street and Driveway Access to North Carolina Highways*. An exclusive southbound left turn lane is warranted on Kelly Road with a minimum of 75 feet of storage. A short right turn lane on Kelly Road is marginally warranted; however, it is not recommended due to future plans to install a roundabout at this intersection.

Analysis of 2030 build conditions with the roundabout project overall LOS B conditions in the AM peak hour and LOS F in the PM. The eastbound approach is expected to operate at LOS F in the PM. Provision of a slip lane on this approach may improve operations in the future and can be considered at the time the roundabout is constructed.



Summary

This phasing study was prepared to identify improvements that would be necessary for the initial phase of commercial development within the West Village PUD. The original West Village PUD included transportation improvements that were assigned to the commercial phase of development; however, the PUD considered the improvements and all commercial development would occur at the same time. The initial phase of commercial development includes limited uses and accounts for only approximately 8% of the West Village PUD.

Based on the analysis results of Build (2028) Phase 1 traffic, the initial commercial development will not have a significant impact on the study area intersections. Improvements were recently constructed at the Apex Barbeque Road/Kelly Road intersection.

Although the Kelly Road approach at Old US 1 operates at LOS F and is expected to continue to operate with long delays during peak times, the initial phase of commercial development is not expected to have a negative impact on the intersection. Queues and delays on Kelly Road are expected to increase slightly with the initial phase. Traffic signals to the east and west of the intersection at NC 540 and at Pleasant Plains Road will create some gaps in through traffic on Old US 1. A traffic signal was considered for the intersection; however, it is not expected to be approved due to the spacing with the NC 540 interchange ramp intersection. The intersection will be restricted to right-in/right-out movements in the future with commercial development in West Village that will provide a roadway connection between existing Kelly Road and Old US 1 that provides increased spacing with the NC 540 interchange. It is not recommended to restrict the Kelly Road intersection until the additional roadway infrastructure can be constructed.

Roadway improvements are required to be constructed in the future as the West Village commercial development occurs. Many improvements are required when network connections are made within the commercial development area.

Future traffic projections in this phasing study should be considered conservative (high). This study includes traffic projections for several adjacent developments in addition to a 4% annual growth rate. Adjacent development trips are expected to interact with each other instead of being added to each other, which would result in less traffic growth. In addition, alternate routes are available for traffic on Kelly Road as well as Old US 1 if congestion increases.



Roadway Improvements Required of Phase 1

Kelly Road and Site Driveway

- Construct the site driveway as a stop-controlled approach with one ingress lane and one egress lane (striped as a shared left-right turn lane).
- Construct a left turn lane with a minimum of 75 feet of storage on the southbound approach
 of Kelly Road.
- Provide right-of-way necessary for the future construction of a roundabout at the intersection.

If you should have any questions or comments regarding this letter, please feel free to contact me at rstephenson@drmp.com or by phone at 919-872-5115.

Sincerely,

Rynal Stephenson, P.E. Chief Traffic Analysis Engineer

DRMP, Inc.

Corporate License #F-1524

Attachments: Intersection Capacity Analysis Tables

Figures

Approved Scope Letter

Traffic Counts

Signal Plans

Adjacent Development Information

Synchro Reports

Original PUD Approval Plan and Requirements

cc: Daniel Boulware, PE, NCDOT District

Amanda Bunce, Town of Apex Planning

Russell Dalton, PE, Town of Apex

Rehab Hamad, MBA

Jessica McClure, PE, Town of Apex

Jeff Roach, PE, Peak Engineering





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Table 3: Analysis Summary of Kelly Road and Olive Chapel Road AM Peak Hour Lane Group EBL EBTR EBR WBL WBT WBR NBL NBT NBR SBL SBT SBR Overatt

AM Peak Hour	Lane Group	EBL	EBTR	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Overall
	Total Delay	35	38.4		18.4	40.1	0.1	21.3	53.1	20	34.1	36.4	15	
	LOS	C	D		В	D	Α	C C	D	C	C	D	В	LOS C
		C			В	23.3	A	C	40.4	C	C	27.7	В	33.1
Existing (2024)	Approach Delay		36.6											33.1
	Approach LOS	400	D		00	C	•	0.5	D			С	0.4	
	Queue Length 50th (ft)	122	223		28	123	0	35	187	44	50	89	64	
	Queue Length 95th (ft)	188	332		54	206	0	80	#389	97	107	159	109	
	Total Delay	37.5	39.7		21.4	42.8	0.1	28.7	174.1	23.5	44.9	43.6	14.9	
	LOS	D	D		C	D	A	C	F	C	D	D	В	LOSE
No-Build (2028)	Approach Delay		38.6		0	25.2		-	122.5	-		34	-	56.1
140-Baila (2020)	Approach LOS		D			C C			F			C		30.1
		234	400		60	248	0	102	#660	125	#159	#269	107	
	Queue Length 95th (ft)	234	400		60	248	U	102	#000	125	#159	#209	137	
	Total Delay	37.2	39.8		24.3	43.2	0.1	30.2	186.5	24.1	45.5	45.7	14.9	
	LOS	D	D		С	D	Α	С	F	С	D	D	В	LOSE
Build (2028)	Approach Delay		38.5			25.8			129.5			35.2		58.4
Jana (2020)	Approach LOS		D			C			F			D		00.4
	Queue Length 95th (ft)	234	407		69	249	0	105	#680	136	#161	#296	138	
	Queue Length 95th (it)	234	407		09	243	U	103	#000	130	#101	#230	130	
	Total Delay	39.1	40.2		24	44.2	0.2	41.1	256.1	25.6	51.5	78.7	15.9	
	LOS	D	D		С	D	Α	D	F	С	D	Е	В	LOS E
No-Build (2030)	Approach Delay		39.6			26.3			180.2			53.4		75.1
	Approach LOS		D			С			F			D		
	Queue Length 95th (ft)	262	442		64	271	0	115	#789	142	#198	#481	156	
	Total Delay	72.3	66.8	27.3	68.6	74	0.2	48.5	63.3	19.5	100.6	57.4	16.3	
	LOS	E	E	С	Е	E	Α	D	E	В	F	E	В	LOSE
Build (2030) w/ Improvements	Approach Delay		63.9			54.4			49.4			55.2		56
	Approach LOS		E			D			D			Е		
	Queue Length 95th (ft)	#405	488	168	#241	#404	0	#149	#824	228	#276	547	148	
														_
PM Peak Hour	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	Overall
PM Peak Hour	Total Delay	108.9	39	EBR	23.4	37.6	0.1	20.7	53.9	15.9	33.7	37.7	29.4	
PM Peak Hour				EBR										Overal l
PM Peak Hour Existing (2024)	Total Delay	108.9	39	EBR	23.4	37.6	0.1	20.7	53.9	15.9	33.7	37.7	29.4	
	Total Delay LOS	108.9	39 D	EBR	23.4	37.6 D	0.1	20.7	53.9 D	15.9	33.7	37.7 D	29.4	LOS D
	Total Delay LOS Approach Delay	108.9	39 D 78.9	EBR	23.4	37.6 D 26.3	0.1	20.7	53.9 D 40.8	15.9	33.7	37.7 D 32.8	29.4	LOS D
	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	108.9 F #259	39 D 78.9 E 252	EBR	23.4 C	37.6 D 26.3 C 324	0.1 A	20.7 C	53.9 D 40.8 D #392	15.9 B	33.7 C	37.7 D 32.8 C #311	29.4 C #445	LOS D
	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	108.9 F #259	39 D 78.9 E 252	EBR	23.4 C 106	37.6 D 26.3 C 324	0.1 A 0	20.7 C 50	53.9 D 40.8 D #392	15.9 B 95	33.7 C 103	37.7 D 32.8 C #311	29.4 C #445	LOS D 43.8
Existing (2024)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS	108.9 F #259	39 D 78.9 E 252 40.3	EBR	23.4 C	37.6 D 26.3 C 324	0.1 A	20.7 C	53.9 D 40.8 D #392	15.9 B	33.7 C	37.7 D 32.8 C #311	29.4 C #445	LOS D 43.8
	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay	108.9 F #259	39 D 78.9 E 252 40.3 D 136.4	EBR	23.4 C 106	37.6 D 26.3 C 324 38.8 D 28.2	0.1 A 0	20.7 C 50	53.9 D 40.8 D #392 142 F 102	15.9 B 95	33.7 C 103	37.7 D 32.8 C #311 65.7 E 51.6	29.4 C #445	LOS D 43.8
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Existing (2024)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS	108.9 F #259 208.7 F	39 D 78.9 E 252 40.3 D 136.4 F 301	EBR	23.4 C 106 28.2 C	37.6 D 26.3 C 324 38.8 D 28.2 C	0.1 A 0	20.7 C 50 30.8 C	53.9 D 40.8 D #392 142 F 102	15.9 B 95 18.2 B	33.7 C 103 42.8 D	37.7 D 32.8 C #311 65.7 E 51.6	29.4 C #445 43.3 D	LOS D 43.8
Existing (2024)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	108.9 F #259 208.7 F	39 D 78.9 E 252 40.3 D 136.4 F 301	EBR	23.4 C 106 28.2 C	37.6 D 26.3 C 324 38.8 D 28.2 C 390	0.1 A 0 0.2 A	20.7 C 50 30.8 C	53.9 D 40.8 D #392 142 F 102 F #601	15.9 B 95 18.2 B	33.7 C 103 42.8 D #148	37.7 D 32.8 C #311 65.7 E 51.6 D #541	29.4 C #445 43.3 D	LOS D 43.8 LOS E 75.3
Existing (2024) No-Build (2028)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Company Total Delay Approach LOS Company Total Delay LOS	108.9 F #259 208.7 F #338	39 D 78.9 E 252 40.3 D 136.4 F 301	EBR	23.4 C 106 28.2 C	37.6 D 26.3 C 324 38.8 D 28.2 C 390	0.1 A 0 0.2 A	20.7 C 50 30.8 C	53.9 D 40.8 D #392 142 F 102 F #601	15.9 B 95 18.2 B	33.7 C 103 42.8 D	37.7 D 32.8 C #311 65.7 E 51.6 D #541	29.4 C #445 43.3 D #617	LOS D 43.8 LOS E 75.3
Existing (2024)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Approach LOS Approach Delay LOS Approach Delay	108.9 F #259 208.7 F #338	39 D 78.9 E 252 40.3 D 136.4 F 301 40.3 D 136	EBR	23.4 C 106 28.2 C	37.6 D 26.3 C 324 38.8 D 28.2 C 390	0.1 A 0 0.2 A	20.7 C 50 30.8 C	53.9 D 40.8 D #392 142 F 102 F #601	15.9 B 95 18.2 B	33.7 C 103 42.8 D #148	37.7 D 32.8 C #311 65.7 E 51.6 D #541 69.8 E 53.2	29.4 C #445 43.3 D #617	LOS D 43.8 LOS E 75.3
Existing (2024) No-Build (2028)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Company Total Delay Approach LOS Company Total Delay LOS	108.9 F #259 208.7 F #338	39 D 78.9 E 252 40.3 D 136.4 F 301	EBR	23.4 C 106 28.2 C	37.6 D 26.3 C 324 38.8 D 28.2 C 390	0.1 A 0 0.2 A	20.7 C 50 30.8 C	53.9 D 40.8 D #392 142 F 102 F #601	15.9 B 95 18.2 B	33.7 C 103 42.8 D #148	37.7 D 32.8 C #311 65.7 E 51.6 D #541	29.4 C #445 43.3 D #617	LOS E 75.3
Existing (2024) No-Build (2028)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	108.9 F #259 208.7 F #338 208.7 F	39 D 78.9 E 252 40.3 D 136.4 F 301 40.3 D 136 F 303	EBR	23.4 C 106 28.2 C 123	37.6 D 26.3 C 324 38.8 D 28.2 C 390 38.8 D 28.7 C 390	0.1 A 0 0.2 A 0	20.7 C 50 30.8 C 62 31.7 C	53.9 D 40.8 D #392 142 F 102 F #601 154.1 F 107.9 F #620	15.9 B 95 18.2 B 120 18.7 B	33.7 C 103 42.8 D #148 42.8 D	37.7 D 32.8 C #311 65.7 E 51.6 D #541 69.8 E 53.2 D #556	29.4 C #445 43.3 D #617	LOS D 43.8 LOS E 75.3
Existing (2024) No-Build (2028)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay LOS Approach Delay LOS Approach Delay Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft)	108.9 F #259 208.7 F #338 208.7 F	39 D 78.9 E 252 40.3 D 136.4 F 301 40.3 D 136 F 303	EBR	23.4 C 106 28.2 C 123 30 C	37.6 D 26.3 C 324 38.8 D 28.2 C 390 38.8 D 28.7 C 390	0.1 A 0 0.2 A 0 0.2 A	20.7 C 50 30.8 C 62 31.7 C	53.9 D 40.8 D #392 142 F 102 F #601 154.1 F 107.9 F #620	15.9 B 95 18.2 B 120 18.7 B	33.7 C 103 42.8 D #148 42.8 D	37.7 D 32.8 C #311 65.7 E 51.6 D #541 69.8 E 53.2 D #556	29.4 C #445 43.3 D #617 43.3 D	LOS D 43.8 LOS E 75.3
Existing (2024) No-Build (2028) Build (2028)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft)	108.9 F #259 208.7 F #338 208.7 F	39 D 78.9 E 252 40.3 D 136.4 F 301 40.3 D 136 F 303	EBR	23.4 C 106 28.2 C 123 30 C	37.6 D 26.3 C 324 38.8 D 28.2 C 390 38.8 D 28.7 C 390	0.1 A 0 0.2 A 0 0.2 A	20.7 C 50 30.8 C 62 31.7 C	53.9 D 40.8 D #392 142 F 102 F #601 154.1 F 107.9 F #620	15.9 B 95 18.2 B 120 18.7 B	33.7 C 103 42.8 D #148 42.8 D	37.7 D 32.8 C #311 65.7 E 51.6 D #541 69.8 E 53.2 D #556	29.4 C #445 43.3 D #617	LOS D 43.8 LOS E 75.3
Existing (2024) No-Build (2028)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft)	108.9 F #259 208.7 F #338 208.7 F	39 D 78.9 E 252 40.3 D 136.4 F 301 40.3 D 136 F 303	EBR	23.4 C 106 28.2 C 123 30 C	37.6 D 26.3 C 324 38.8 D 28.2 C 390 38.8 D 28.7 C 390	0.1 A 0 0.2 A 0 0.2 A	20.7 C 50 30.8 C 62 31.7 C	53.9 D 40.8 D #392 142 F 102 F #601 154.1 F 107.9 F #620	15.9 B 95 18.2 B 120 18.7 B	33.7 C 103 42.8 D #148 42.8 D	37.7 D 32.8 C #311 65.7 E 51.6 D #541 69.8 E 53.2 D #556	29.4 C #445 43.3 D #617 43.3 D	LOS D 43.8 LOS E 75.3
Existing (2024) No-Build (2028) Build (2028)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft)	108.9 F #259 208.7 F #338 208.7 F	39 D 78.9 E 252 40.3 D 136.4 F 301 40.3 D 136 F 303	EBR	23.4 C 106 28.2 C 123 30 C	37.6 D 26.3 C 324 38.8 D 28.2 C 390 38.8 D 28.7 C 390	0.1 A 0 0.2 A 0 0.2 A	20.7 C 50 30.8 C 62 31.7 C	53.9 D 40.8 D #392 142 F 102 F #601 154.1 F 107.9 F #620	15.9 B 95 18.2 B 120 18.7 B	33.7 C 103 42.8 D #148 42.8 D	37.7 D 32.8 C #311 65.7 E 51.6 D #541 69.8 E 53.2 D #556	29.4 C #445 43.3 D #617 43.3 D	LOS D 43.8 LOS E 75.3
Existing (2024) No-Build (2028) Build (2028)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft)	108.9 F #259 208.7 F #338 208.7 F	39 D 78.9 E 252 40.3 D 136.4 F 301 40.3 D 136 F 303	EBR	23.4 C 106 28.2 C 123 30 C	37.6 D 26.3 C 324 38.8 D 28.2 C 390 38.8 D 28.7 C 390	0.1 A 0 0.2 A 0 0.2 A	20.7 C 50 30.8 C 62 31.7 C	53.9 D 40.8 D #392 142 F 102 F #601 154.1 F 107.9 F #620	15.9 B 95 18.2 B 120 18.7 B	33.7 C 103 42.8 D #148 42.8 D	37.7 D 32.8 C #311 65.7 E 51.6 D #541 69.8 E 53.2 D #556	29.4 C #445 43.3 D #617 43.3 D	LOS D 43.8 LOS E 75.3
Existing (2024) No-Build (2028) Build (2028)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft)	108.9 F #259 208.7 F #338 208.7 F #338 4388 273.9 F	39 D 78.9 E 252 40.3 D 136.4 F 301 40.3 D 136 F 303 40.7 D 174 F 330		23.4 C 106 28.2 C 123 30 C 130	37.6 D 26.3 C 324 38.8 D 28.2 C 390 38.8 D 28.7 C 390 29.2 C 429	0.1 A 0 0.2 A 0 0.2 A 0 0.2 A	20.7 C 50 30.8 C 62 31.7 C 67 33.6 C	53.9 D 40.8 D #392 142 F 102 F #601 154.1 F 107.9 F #620 301.5 F 219.5 F	15.9 B 95 18.2 B 120 18.7 B 135	33.7 C 103 42.8 D #148 42.8 D #148 48.9 D	37.7 D 32.8 C #311 65.7 E 51.6 D #541 69.8 E 53.2 D #556 123.1 F 83.6 F	29.4 C #445 43.3 D #617 43.3 D #617 60.4 E	LOS D 43.8 LOS E 75.3
Existing (2024) No-Build (2028) Build (2028)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	108.9 F #259 208.7 F #338 208.7 F #338 273.9 F	39 D 78.9 E 252 40.3 D 136.4 F 301 40.3 D 136 F 303 40.7 D 174 F 330	33.6	23.4 C 106 28.2 C 123 30 C 130 31.2 C	37.6 D 26.3 C 324 38.8 D 28.2 C 390 38.8 D 28.7 C 390 29.2 C 429	0.1 A 0 0.2 A 0 0.2 A 0 0.2 A 0 0.2 A	20.7 C 50 30.8 C 62 31.7 C 67 33.6 C	53.9 D 40.8 D #392 142 F 102 F #601 154.1 F 107.9 F #620 301.5 F 219.5 F #840	15.9 B 95 18.2 B 120 18.7 B 135 19.6 B	33.7 C 103 42.8 D #148 42.8 D #148 48.9 D	37.7 D 32.8 C #311 65.7 E 51.6 D #541 69.8 E 53.2 D #556 123.1 F 83.6 F	29.4 C #445 43.3 D #617 43.3 D #617 60.4 E	LOS D 43.8 LOS E 75.3
Existing (2024) No-Build (2028) Build (2028) No-Build (2030)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	108.9 F #259 208.7 F #338 208.7 F #338 4388 273.9 F	39 D 78.9 E 252 40.3 D 136.4 F 301 40.3 D 136 F 303 40.7 D 174 F 330		23.4 C 106 28.2 C 123 30 C 130	37.6 D 26.3 C 324 38.8 D 28.2 C 390 38.8 D 28.7 C 390 39.3 D 29.2 C 429	0.1 A 0 0.2 A 0 0.2 A 0 0.2 A	20.7 C 50 30.8 C 62 31.7 C 67 33.6 C	53.9 D 40.8 D #392 142 F 102 F #601 154.1 F 107.9 F #620 301.5 F 219.5 F #840	15.9 B 95 18.2 B 120 18.7 B 135	33.7 C 103 42.8 D #148 42.8 D #148 48.9 D	37.7 D 32.8 C #311 65.7 E 51.6 D #541 69.8 E 53.2 D #556 123.1 F 83.6 F #695	29.4 C #445 43.3 D #617 43.3 D #617 60.4 E	LOS D 43.8 LOS E 75.3 LOS E 77.1
Existing (2024) No-Build (2028) Build (2028)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	108.9 F #259 208.7 F #338 208.7 F #338 213.9 F #387	39 D 78.9 E 252 40.3 D 136.4 F 301 40.3 D 136 F 303 40.7 D 174 F 330	33.6	23.4 C 106 28.2 C 123 30 C 130 31.2 C	37.6 D 26.3 C 324 38.8 D 28.2 C 390 38.8 D 28.7 C 390 39.3 D 29.2 C 429	0.1 A 0 0.2 A 0 0.2 A 0 0.2 A 0 0.2 A	20.7 C 50 30.8 C 62 31.7 C 67 33.6 C	53.9 D 40.8 D #392 142 F 102 F #601 154.1 F 107.9 F #620 301.5 F 219.5 F #840	15.9 B 95 18.2 B 120 18.7 B 135 19.6 B	33.7 C 103 42.8 D #148 42.8 D #148 48.9 D	37.7 D 32.8 C #311 65.7 E 51.6 D #541 69.8 E 53.2 D #556 123.1 F 83.6 F #695	29.4 C #445 43.3 D #617 43.3 D #617 60.4 E	LOS D 43.8 LOS E 75.3
Existing (2024) No-Build (2028) Build (2028) No-Build (2030)	Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	108.9 F #259 208.7 F #338 208.7 F #338 213.9 F #387	39 D 78.9 E 252 40.3 D 136.4 F 301 40.3 D 136 F 303 40.7 D 174 F 330	33.6	23.4 C 106 28.2 C 123 30 C 130 31.2 C	37.6 D 26.3 C 324 38.8 D 28.2 C 390 38.8 D 28.7 C 390 39.3 D 29.2 C 429	0.1 A 0 0.2 A 0 0.2 A 0 0.2 A 0 0.2 A	20.7 C 50 30.8 C 62 31.7 C 67 33.6 C	53.9 D 40.8 D #392 142 F 102 F #601 154.1 F 107.9 F #620 301.5 F 219.5 F #840	15.9 B 95 18.2 B 120 18.7 B 135 19.6 B	33.7 C 103 42.8 D #148 42.8 D #148 48.9 D	37.7 D 32.8 C #311 65.7 E 51.6 D #541 69.8 E 53.2 D #556 123.1 F 83.6 F #695	29.4 C #445 43.3 D #617 43.3 D #617 60.4 E	LOS D 43.8 LOS E 75.3 LOS E 77.1



Table 4: Analysis Summary of Kelly Road and Olive Chapel Road

AM Peak Hour	Lane Group	EBL	EBTR	WBL	WBTR	NBL	NBTR	SBL	SBT	SBR	Overal
AFFEARTION											Overac
	Total Delay	12.5	18	10.9	26.6	19.2	30.4	21.9	28	12.1	
	LOS	В	В	В	С	В	С	С	С	В	LOS C
Existing (2024)	Approach Delay		16.1		25.3		29.5		23.4		23.7
	Approach LOS		В		С		С		С		
	Queue Length 95th (ft)	56	136	21	235	29	120	79	114	26	
	Total Delay	12.9	19	10.4	31	23.7	38.1	31.9	35.2	15.7	
	LOS	В	В	В	С	С	D	С	D	В	LOSC
No-Build (2028)	Approach Delay		17		29.4		37		31.7		29
110 24114 (2020)	Approach LOS		В		C		D		C		
	Queue Length 95th (ft)	63	158	24	324	37	178	116	187	35	
	Quede Length com (it)		100		024		170	110	107		
	Total Delay	13.1	19.5	10.7	31.4	24.2	38.9	33.1	36.2	15.7	
	LOS	В	В	В	C	C C	D	C	D	В	LOS C
Build (2028)	Approach Delay	ь	17.4	В	29.3	C	37.6	C	33	В	29.7
Dullu (2028)			17.4 B		29.3 C		37.6 D		C		29.7
	Approach LOS			20		40		110		0.5	
	Queue Length 95th (ft)	63	168	30	324	43	194	116	226	35	
	T			40 -			40 -				
	Total Delay	19.3	27.8	10.6	32.7	26.9	43.5	60.2	39.1	25.4	
	LOS	В	С	В	С	С	D	E	D	С	LOS D
No-Build (2030)	Approach Delay		25.5		31.2		42.2		48.5		36.4
	Approach LOS		С		С		D		D		
	Queue Length 95th (ft)	66	295	25	403	41	204	#305	212	52	
	Total Delay	17.9	32.6	20.9	33.1	44.1	123.9	97.7	333.3	26.4	
	LOS	В	С	С	С	D	F	F	F	С	LOS F
			29.5		30.1		113		253.6		117.8
Build (2030)	Approach Delay		23.3								
Build (2030)	Approach Delay Approach LOS		C		С		F		F		
Build (2030)		66		85	C 403	118	F #511	#346	F #900	52	
Build (2030)	Approach LOS	66	С	85		118		#346		52	
Build (2030) PM Peak Hour	Approach LOS	66 EBL	С	85 WBL		118 NBL		#346 SBL			Overall
	Approach LOS Queue Length 95th (ft) Lane Group	EBL	C 373 EBT	WBL	403 WBT	NBL	#511 NBT	SBL	#900 SBT	SBR	Overall
	Approach LOS Queue Length 95th (ft) Lane Group Total Delay	EBL 12.8	C 373 EBT 21.2	WBL 12.2	403 WBT 28.4	NBL 19.5	#511 NBT 30.4	SBL 22.5	#900 SBT 34.4	SBR 12.2	
PM Peak Hour	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS	EBL	C 373 EBT 21.2 C	WBL	403 WBT 28.4 C	NBL	#511 NBT 30.4 C	SBL	#900 SBT 34.4 C	SBR	LOSC
	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay	EBL 12.8	C 373 EBT 21.2 C 18.6	WBL 12.2	403 WBT 28.4 C 25.9	NBL 19.5	#511 NBT 30.4 C 28.8	SBL 22.5	#900 SBT 34.4 C 27.3	SBR 12.2	
PM Peak Hour	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS	12.8 B	C 373 EBT 21.2 C 18.6 B	WBL 12.2 B	403 WBT 28.4 C 25.9 C	NBL 19.5 B	#511 NBT 30.4 C 28.8 C	SBL 22.5 C	#900 SBT 34.4 C 27.3 C	SBR 12.2 B	LOSC
PM Peak Hour	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay	EBL 12.8	C 373 EBT 21.2 C 18.6	WBL 12.2	403 WBT 28.4 C 25.9	NBL 19.5	#511 NBT 30.4 C 28.8	SBL 22.5	#900 SBT 34.4 C 27.3	SBR 12.2	LOSC
PM Peak Hour	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	12.8 B	C 373 EBT 21.2 C 18.6 B	WBL 12.2 B	WBT 28.4 C 25.9 C 215	NBL 19.5 B	#511 30.4 C 28.8 C 125	SBL 22.5 C	#900 SBT 34.4 C 27.3 C #276	SBR 12.2 B	LOSC
PM Peak Hour	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS	12.8 B	C 373 EBT 21.2 C 18.6 B	WBL 12.2 B	403 WBT 28.4 C 25.9 C 215	NBL 19.5 B	#511 NBT 30.4 C 28.8 C	SBL 22.5 C	#900 SBT 34.4 C 27.3 C #276	SBR 12.2 B	LOSC
PM Peak Hour	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS	EBL 12.8 B	C 373 EBT 21.2 C 18.6 B 110	WBL 12.2 B	WBT 28.4 C 25.9 C 215	NBL 19.5 B	#511 30.4 C 28.8 C 125	SBL 22.5 C	#900 SBT 34.4 C 27.3 C #276	SBR 12.2 B	LOSC
PM Peak Hour	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay	EBL 12.8 B 42	C 373 EBT 21.2 C 18.6 B 110	WBL 12.2 B 34	403 WBT 28.4 C 25.9 C 215	NBL 19.5 B 47	#511 30.4 C 28.8 C 125	SBL 22.5 C 105	#900 SBT 34.4 C 27.3 C #276	SBR 12.2 B 47	LOS C 26
PM Peak Hour Existing (2024)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach Delay Approach Delay	EBL 12.8 B 42	C 373 EBT 21.2 C 18.6 B 110	WBL 12.2 B 34	WBT 28.4 C 25.9 C 215	NBL 19.5 B 47	#511 30.4 C 28.8 C 125 35.3 D	SBL 22.5 C 105	#900 SBT 34.4 C 27.3 C #276	SBR 12.2 B 47	LOS C 26
PM Peak Hour Existing (2024)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay	EBL 12.8 B 42	C 373 EBT 21.2 C 18.6 B 110 21.8 C 18.9	WBL 12.2 B 34	WBT 28.4 C 25.9 C 215 31 C 28.2	NBL 19.5 B 47	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1	SBL 22.5 C 105	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6	SBR 12.2 B 47	LOS C 26
PM Peak Hour Existing (2024)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach Delay Approach Delay	12.8 B 42 12.8 B	C 373 EBT 21.2 C 18.6 B 110 21.8 C 18.9 B	WBL 12.2 B 34 11.8 B	WBT 28.4 C 25.9 C 215 31 C 28.2 C	NBL 19.5 B 47 27.1 C	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C	SBL 22.5 C 105 35.2 D	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D	SBR 12.2 B 47 14.4 B	LOS C 26
PM Peak Hour Existing (2024)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach Delay Approach Delay	12.8 B 42 12.8 B	C 373 EBT 21.2 C 18.6 B 110 21.8 C 18.9 B	WBL 12.2 B 34 11.8 B	WBT 28.4 C 25.9 C 215 31 C 28.2 C	NBL 19.5 B 47 27.1 C	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C	SBL 22.5 C 105 35.2 D	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D	SBR 12.2 B 47 14.4 B	LOS C 26
PM Peak Hour Existing (2024)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach Delay Approach LOS Queue Length 95th (ft)	12.8 B 42 12.8 B	C 373 EBT 21.2 C 18.6 B 110 21.8 C 18.9 B 125	WBL 12.2 B 34 11.8 B	403 WBT 28.4 C 25.9 C 215 31 C 28.2 C 271	NBL 19.5 B 47 27.1 C	#511 30.4 C 28.8 C 125 35.3 D 34.1 C 173	\$BL 22.5 C 105 35.2 D	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450	SBR 12.2 B 47 14.4 B	LOS C 26
PM Peak Hour Existing (2024)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft)	12.8 B 42 12.8 B 47	C 373 21.2 C 18.6 B 110 21.8 C 18.9 B 125	WBL 12.2 B 34 11.8 B	403 WBT 28.4 C 25.9 C 215 31 C 28.2 C 271	NBL 19.5 B 47 27.1 C	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173	\$BL 22.5 C 105 35.2 D #165	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450	SBR 12.2 B 47 14.4 B	LOS C 26
PM Peak Hour Existing (2024) No-Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft) Total Delay LOS	12.8 B 42 12.8 B 47	C 373 21.2 C 18.6 B 110 21.8 C 18.9 B 125	WBL 12.2 B 34 11.8 B	403 WBT 28.4 C 25.9 C 215 31 C 28.2 C 271 31.4 C	NBL 19.5 B 47 27.1 C	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D	\$BL 22.5 C 105 35.2 D #165	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450	SBR 12.2 B 47 14.4 B	LOS C 26 LOS C 33.0
PM Peak Hour Existing (2024) No-Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft) Total Delay Approach Delay Approach Delay Approach Delay LOS Approach Delay	12.8 B 42 12.8 B 47	C 373 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C	WBL 12.2 B 34 11.8 B	403 WBT 28.4 C 25.9 C 215 31 C 28.2 C 271 31.4 C 28.2	NBL 19.5 B 47 27.1 C	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 35.6	\$BL 22.5 C 105 35.2 D #165	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7	SBR 12.2 B 47 14.4 B	LOS C 26 LOS C 33.0
PM Peak Hour Existing (2024) No-Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft) Total Delay Approach Delay Approach Delay LOS Approach Delay LOS Approach Delay Approach LOS	12.8 B 42 12.8 B 47 12.9	C 373 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C	WBL 12.2 B 34 11.8 B 37 12.1 B	403 WBT 28.4 C 25.9 C 215 31 C 28.2 C 271 31.4 C 28.2 C	NBL 19.5 B 47 27.1 C 58 30.9 C	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 35.6 D	\$BL 22.5 C 105 35.2 D #165	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7 D	SBR 12.2 B 47 14.4 B 63 14.3 B	LOS C 26 LOS C 33.0
PM Peak Hour Existing (2024) No-Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft) Total Delay LOS Queue Length 95th (ft)	12.8 B 42 12.8 B 47 12.9 B	C 373 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C	WBL 12.2 B 34 11.8 B 37 12.1 B	31.4 C 28.2 C 271	NBL 19.5 B 47 27.1 C 58 30.9 C	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 35.6 D 196	\$BL 22.5 C 105 35.2 D #165	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7 D #494	SBR 12.2 B 47 14.4 B 63 14.3 B	LOS C 26 LOS C 33.0
PM Peak Hour Existing (2024) No-Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay Approach LOS Queue Length 95th (ft)	12.8 B 42 12.8 B 47 12.9 B	C 373 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C 19.4 B	WBL 12.2 B 34 11.8 B 37 12.1 B	31.4 C 28.2 C 271 31.4 C 28.2 C 271	19.5 B 47 27.1 C 58 30.9 C	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 35.6 D 196	\$BL 22.5 C 105 35.2 D #165 37.8 D	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7 D #494	SBR 12.2 B 47 14.4 B 63 14.3 B 29.1	LOS C 26 LOS C 33.0
PM Peak Hour Existing (2024) No-Build (2028) Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft)	12.8 B 42 12.8 B 47 12.9 B	C 373 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C 19.4 B 132	WBL 12.2 B 34 11.8 B 37 12.1 B	31.4 C 28.2 C 271 31.4 C 28.2 C 271	NBL 19.5 B 47 27.1 C 58 30.9 C	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 196 48.7 D	\$BL 22.5 C 105 35.2 D #165 37.8 D	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7 D #494	SBR 12.2 B 47 14.4 B 63 14.3 B	LOS C 26 LOS C 33.0
PM Peak Hour Existing (2024) No-Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	12.8 B 42 12.8 B 47 12.9 B	C 373 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C 19.4 B 132	WBL 12.2 B 34 11.8 B 37 12.1 B	31.4 C 28.2 C 271 31.4 C 28.2 C 271 33.4	19.5 B 47 27.1 C 58 30.9 C	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 35.6 D 196 48.7 D 47.7	\$BL 22.5 C 105 35.2 D #165 37.8 D	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7 D #494 97.9 F 92.9	SBR 12.2 B 47 14.4 B 63 14.3 B 29.1	LOS C 26 LOS C 33.0
PM Peak Hour Existing (2024) No-Build (2028) Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	12.8 B 42 12.8 B 47 12.9 B	C 373 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C 19.4 B 132	WBL 12.2 B 34 11.8 B 37 12.1 B	31.4 C 28.2 C 271 31.4 C 28.2 C 271 33.4 C 28.2	19.5 B 47 27.1 C 58 30.9 C 41.4 D	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 196 48.7 D 47.7 D	\$BL 22.5 C 105 35.2 D #165 37.8 D #179	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7 D #494 97.9 F 92.9 F	SBR 12.2 B 47 14.4 B 63 14.3 B 63 29.1 C	LOS C 26 LOS C 33.0
PM Peak Hour Existing (2024) No-Build (2028) Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	12.8 B 42 12.8 B 47 12.9 B	C 373 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C 19.4 B 132	WBL 12.2 B 34 11.8 B 37 12.1 B	31.4 C 28.2 C 271 31.4 C 28.2 C 271 33.4	19.5 B 47 27.1 C 58 30.9 C	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 35.6 D 196 48.7 D 47.7	\$BL 22.5 C 105 35.2 D #165 37.8 D	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7 D #494 97.9 F 92.9	SBR 12.2 B 47 14.4 B 63 14.3 B 29.1	LOS C 26 LOS C 33.0
PM Peak Hour Existing (2024) No-Build (2028) Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	12.8 B 42 12.8 B 47 12.9 B 47 16.4 B	C 373 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C 19.4 B 132	WBL 12.2 B 34 11.8 B 37 12.1 B 42 10.5 B	31.4 C 28.2 C 271 31.4 C 28.2 C 271 36.1 D 33.4 C 504	NBL 19.5 B 47 27.1 C 58 30.9 C 41.4 D	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 196 48.7 D 47.7 D 216	\$BL 22.5 C 105 35.2 D #165 37.8 D #179 110.8 F	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7 D #494 97.9 F 92.9 F #582	SBR 12.2 B 47 14.4 B 63 14.3 B 63 29.1 C	LOS C 26 LOS C 33.0
PM Peak Hour Existing (2024) No-Build (2028) Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	12.8 B 42 12.8 B 47 12.9 B 47 16.4 B	C 373 EBT 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C 19.4 B 132 37 D 31.5 C 229 36.5	WBL 12.2 B 34 11.8 B 37 12.1 B 42 10.5 B 38	31.4 C 28.2 C 271 31.4 C 28.2 C 271 36.1 D 33.4 C 504	NBL 19.5 B 47 27.1 C 58 30.9 C 67 41.4 D	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 196 48.7 D 47.7 D 216 387.6	\$BL 22.5 C 105 35.2 D #165 37.8 D #179 110.8 F	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7 D #494 97.9 F 92.9 F #582	SBR 12.2 B 47 14.4 B 63 14.3 B 63 29.1 C	LOS C 26 LOS C 33.0 LOS D 35
PM Peak Hour Existing (2024) No-Build (2028) Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft)	12.8 B 42 12.8 B 47 12.9 B 47 16.4 B	C 373 EBT 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C 19.4 B 132 37 D 31.5 C 229 36.5 D	WBL 12.2 B 34 11.8 B 37 12.1 B 42 10.5 B	31.4 C 28.2 C 271 31.4 C 28.2 C 271 36.1 D 33.4 C 504	NBL 19.5 B 47 27.1 C 58 30.9 C 41.4 D	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 196 48.7 D 47.7 D 216 387.6 F	\$BL 22.5 C 105 35.2 D #165 37.8 D #179 110.8 F	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7 D #494 97.9 F 92.9 F #582	SBR 12.2 B 47 14.4 B 63 14.3 B 63 29.1 C	LOS C 26 LOS C 33.0 LOS D 35
PM Peak Hour Existing (2024) No-Build (2028) Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft)	12.8 B 42 12.8 B 47 12.9 B 47 16.4 B	C 373 EBT 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C 19.4 B 132 37 D 31.5 C 229 36.5 D 32.7	WBL 12.2 B 34 11.8 B 37 12.1 B 42 10.5 B 38	31.4 C 28.2 C 271 31.4 C 28.2 C 271 36.1 D 33.4 C 504	NBL 19.5 B 47 27.1 C 58 30.9 C 67 41.4 D	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 196 48.7 D 47.7 D 216 387.6 F 339.5	\$BL 22.5 C 105 35.2 D #165 37.8 D #179 110.8 F	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7 D #494 97.9 F 92.9 F #582 697.2 F 518.5	SBR 12.2 B 47 14.4 B 63 14.3 B 63 29.1 C	LOS C 26 LOS C 33.0 LOS D 35
PM Peak Hour Existing (2024) No-Build (2028) Build (2028)	Approach LOS Queue Length 95th (ft) Lane Group Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach Delay Approach LOS Queue Length 95th (ft) Total Delay LOS Approach LOS Queue Length 95th (ft)	12.8 B 42 12.8 B 47 12.9 B 47 16.4 B	C 373 EBT 21.2 C 18.6 B 110 21.8 C 18.9 B 125 22.2 C 19.4 B 132 37 D 31.5 C 229 36.5 D	WBL 12.2 B 34 11.8 B 37 12.1 B 42 10.5 B 38	31.4 C 28.2 C 271 31.4 C 28.2 C 271 36.1 D 33.4 C 504	NBL 19.5 B 47 27.1 C 58 30.9 C 67 41.4 D	#511 NBT 30.4 C 28.8 C 125 35.3 D 34.1 C 173 36.5 D 196 48.7 D 47.7 D 216 387.6 F	\$BL 22.5 C 105 35.2 D #165 37.8 D #179 110.8 F	#900 SBT 34.4 C 27.3 C #276 50.6 D 40.6 D #450 55.8 E 44.7 D #494 97.9 F 92.9 F #582	SBR 12.2 B 47 14.4 B 63 14.3 B 63 29.1 C	LOS C 26 LOS C 33.0 LOS D 35 LOS E 56.8



Table 5: Analysis Summary of Kelly Road and Leo Drive

AM Peak Hour	Lane Group	EB ²	WB ²	NB ¹	SB ¹	Overall
	HCM Control Delay	11.5	11.4	7.6	7.7	N/A
Existing (2024)	HCM Lane/Approach LOS	В	В	Α	Α	
	HCM 95th- Queue (ft)	5	3	0	0	
	HCM Control Delay	12	11.8	7.6	7.7	N/A
No-Build (2028)	HCM Lane/Approach LOS	В	В	Α	Α	
	HCM 95th- Queue (ft)	5	5	0	0	
	HCM Control Delay	13.6	13.3	7.8	7.8	N/A
Build (2028)	HCM Lane/Approach LOS	В	В	Α	Α	
	HCM 95th- Queue (ft)	8	5	0	0	
	HCM Control Delay	12.7	12.5	7.7	7.8	N/A
No-Build (2030)	HCM Lane/Approach LOS	В	В	Α	Α	
	HCM 95th- Queue (ft)	8	5	0	0	
	HCM Control Delay	44.8	34.7	9.8	9.1	N/A
Build (2030)	HCM Lane/Approach LOS	Е	D	Α	Α	
	HCM 95th- Queue (ft)	35	23	0	3	
		2	2	1	1	
PM Peak Hour	Lane Group	EB ²	WB^2	NB ¹	SB ¹	Overal
	HCM Control Delay	13.9	13.2	7.8	7.9	N/A
Existing (2024)	HCM Control Delay HCM Lane/Approach LOS	В	В	Α	Α	N/A
Existing (2024)	HCM Control Delay					N/A
Existing (2024)	HCM Control Delay HCM Lane/Approach LOS	В	В	Α	Α	N/A N/A
	HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft)	B 5	B 3	A 0	A 3	
	HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay	B 5 15.6	B 3 14.6	A 0 7.9	A 3 8.1	
	HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft)	B 5 15.6 C	B 3 14.6 B 3	7.9 A	A 3 8.1 A	
No-Build (2028)	HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay	B 5 15.6 C 8	B 3 14.6 B 3	7.9 A 0	A 3 8.1 A 3	N/A
No-Build (2028)	HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft)	15.6 C 8	B 3 14.6 B 3	7.9 A	8.1 A 3	N/A
No-Build (2028)	HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft)	15.6 C 8 17.8 C	B 3 14.6 B 3 16.3 C	7.9 A 0 8 A	A 3 8.1 A 3 A 3	N/A
No-Build (2028) Build (2028)	HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft)	15.6 C 8 17.8 C 10	B 3 14.6 B 3 16.3 C 5	7.9 A 0 8 A 0	A 3 8.1 A 3 A 3 8.2	N/A
No-Build (2028) Build (2028)	HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft)	15.6 C 8 17.8 C	B 3 14.6 B 3 16.3 C	7.9 A 0 8 A	A 3 8.1 A 3 A 3	N/A
No-Build (2028) Build (2028)	HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft)	15.6 C 8 17.8 C 10 16.9 C	B 3 14.6 B 3 16.3 C 5	A 0 7.9 A 0 7.9 A 0	A 3 8.1 A 3 8.3 A 3 8.2 A 3	N/A N/A
No-Build (2028) Build (2028) No-Build (2030)	HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft)	15.6 C 8 17.8 C 10 16.9 C 10	B 3 14.6 B 3 16.3 C 5 15.3 C 78.6	7.9 A 0 8 A 0 7.9 A 0	8.1 A 3 8.3 A 3 8.2 A 3	N/A
Existing (2024) No-Build (2028) Build (2028) No-Build (2030)	HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft)	15.6 C 8 17.8 C 10 16.9 C	B 3 14.6 B 3 16.3 C 5	A 0 7.9 A 0 7.9 A 0	A 3 8.1 A 3 8.3 A 3 8.2 A 3	N/A N/A



Table 6: Analysis Summary of Kelly Road and Old US 1

AM Peak Hour	Lane Group	EB ¹	SB ²	Overall
	HCM Control Delay	9.3	84.2	N/A
Existing (2024)	HCM Lane/Approach LOS	Α	F	
	HCM 95th- Queue (ft)	10	158	
	HCM Control Delay	11	Long Delays	N/A
No-Build (2028)	HCM Lane/Approach LOS	В	F	
	HCM 95th- Queue (ft)	15	410	
	HCM Control Delay	11.5	Long Delays	N/A
Build (2028)	HCM Lane/Approach LOS	В	F	
	HCM 95th- Queue (ft)	20	515	
	HCM Control Delay	11.7	Long Delays	N/A
No-Build (2030)	HCM Lane/Approach LOS	В	F	
	HCM 95th- Queue (ft)	18	490	
	HCM Control Delay	-	26.2	N/A
Build (2030)	HCM Lane/Approach LOS	-	D	
	HCM 95th- Queue (ft)	-	100	
			_	
PM Peak Hour	Lane Group	EB ²	SB ¹	Overall
	HCM Control Delay	10.5	67.7	N/A
Existing (2024)	HCM Lane/Approach LOS	В	F	
	HCM 95th- Queue (ft)	15	153	
	HCM Control Delay	14.3	Long Delays	N/A
No-Build (2028)	HCM Lane/Approach LOS	В	F	
	HCM 95th- Queue (ft)	28	383	
	HCM Control Delay	15	Long Delays	N/A
Build (2028)	HCM Control Delay HCM Lane/Approach LOS		Long Delays	N/A
Build (2028)	HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft)	15 C 33		N/A
Build (2028)	HCM Lane/Approach LOS HCM 95th- Queue (ft)	C 33	F 535	
	HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay	C 33	F 535 Long Delays	
Build (2028) No-Build (2030)	HCM Lane/Approach LOS HCM 95th- Queue (ft)	C 33	F 535	
	HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS HCM 95th- Queue (ft)	C 33 17.1 C	F 535 Long Delays F 455	N/A
	HCM Lane/Approach LOS HCM 95th- Queue (ft) HCM Control Delay HCM Lane/Approach LOS	C 33 17.1 C 38	F 535 Long Delays F	



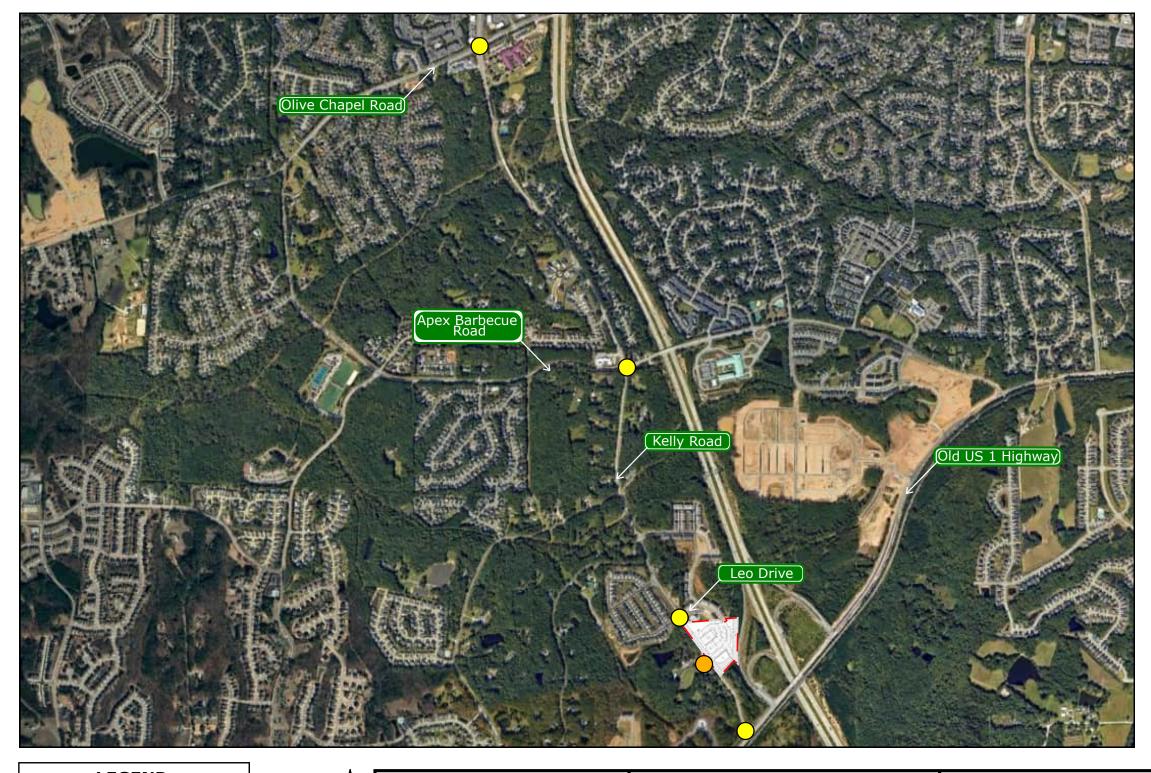
Table 7: Analysis Summary of Kelly Road and Site Driveway

AM Peak Hour	Lane Group	EB ²	WB ²	NB ¹	SB ¹	Overall
	HCM Control Delay	-	12.9	-	8	N/A
Build (2028)	HCM Lane/Approach LOS	-	В	-	Α	
,	HCM 95th- Queue (ft)	-	15	-	5	
D.::Id (0000)	HCM Control Delay	12.5	7.7	10.2	8.7	В
Build (2030)	HCM Lane/Approach LOS	В	Α	В	Α	10.2
Roundabout	HCM 95th- Queue (ft)	197	14	47	111	
PM Peak Hour	Lane Group	EB ²	WB ²	NB	SB ¹	Overall
	HCM Control Delay	-	17.8	-	8.3	N/A
Build (2028)	HCM Lane/Approach LOS	-	С	-	Α	
	HCM 95th- Queue (ft)	-	43	-	5	
D (11/0000)	HCM Control Delay	109.2	16.9	25	12.1	F
Build (2030)	HCM Lane/Approach LOS	F	С	D	В	56.2
Roundabout	HCM 95th- Queue (ft)	Long	46	133	282	



FIGURES





LEGEND



Study Intersection Proposed Site Access



Proposed Si Study Area

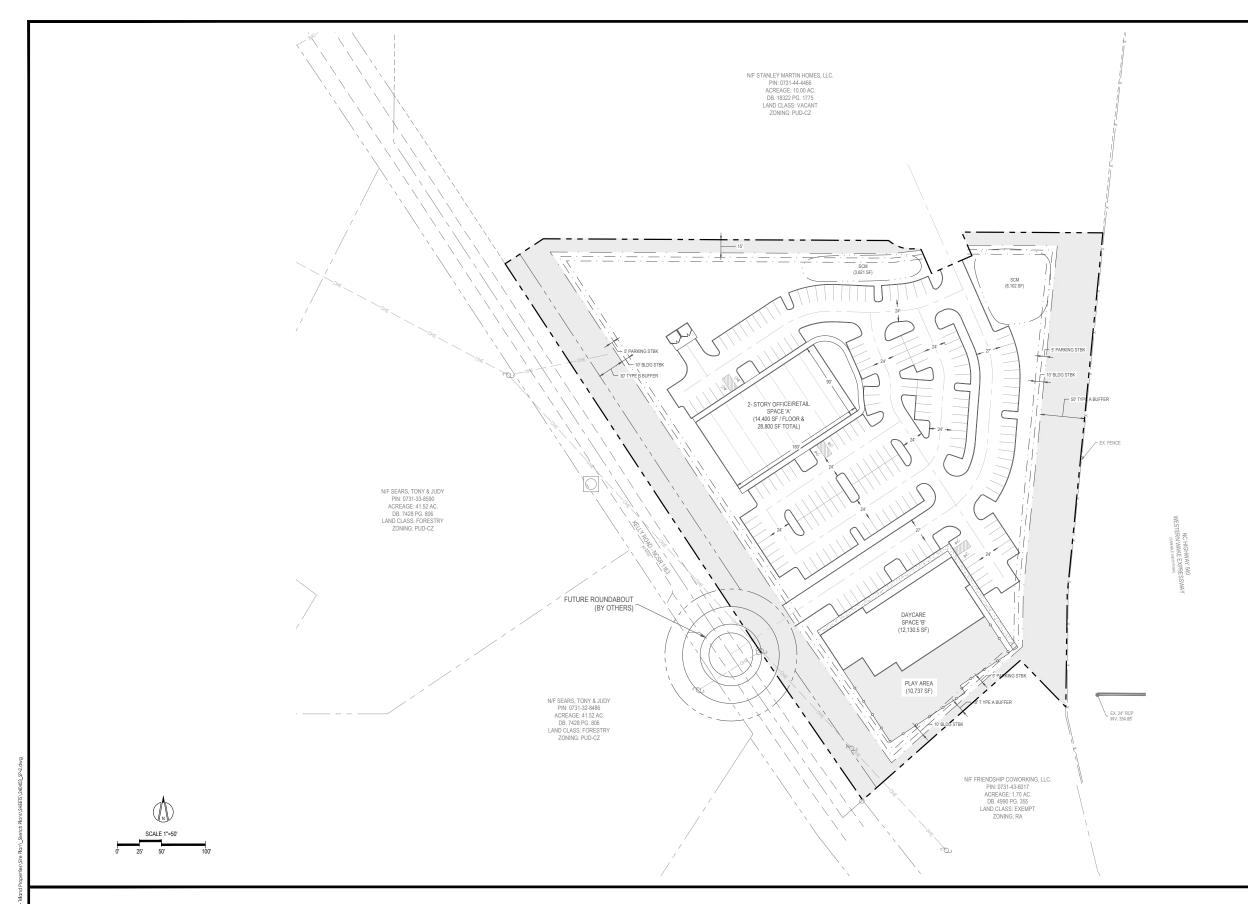




Commercial Development West Village PUD Apex, NC

Site Location Map

Scale: Not to Scale Figure 1



SITE DATA	
OWNER	FAHEY FAMILY FARM, LLC.
SITE ADDRESS	2517 KEILY ROAD
SI E ADDRESS	APEX NC 27502
PN	0731-43-45D4
REAL ID	454267
ACREAGE	6.19 AC.
DB. PG	DB 1/062 PG 1/16
2045 JAND USE MAP	PUD-02
EXISTING USE	SINGLE FAM
REZONING 67-19-2016	15CZ33
FROPOSED ZONING	PUD ŒZ
TOWNSHIP	WHITE OAK
SUB-WATERSHED	BEAVER CREEK
RIVERBASIN	CAFE FEAR
FEMA MAP	MAP#3720073100K DATE 07.18.22
	1
HISTORICAL	(1) BARN PER WA1052
ROW DEDICATION	XX AC. (XX SF)
NEW TRACTIAREA	XX AC. (XX SF)
DEVELOPMENT TYPE - (OMMERCIAL
PROPOSED USE	OFFICE / RETAIL & DAYCARE
DENSITY	50Q ODC SF
MIN. LOT AREA	5,000 SF
MIN.LO SIZE	5,000 SF
MIN LOT WIDTH	N/A
BULLDING DET	ALS
MAX, BUILDING HEIGHT	75 (6-STORY)
PROPOSED BUILDING HEIGHT	XX'
PROFOSED BUILDING STORIES	2 STORIES
PROPOSED BOILDING S GIVES	231011123
BUILDING SETTE	A CMS
FRONT	5
SIDE	5
CORNER	ร์
REAR	5
NO-N	, ,
BUILD UPON A	DCA
MAX. BUILD UPON AREA	4.33° AC. (70 %)
PROPOSED BULD UPON AREA	XX'AC. (XX %)
The beat bet a crime.	The state of the state of
PARKING	
PARKING CALCS. (MIXED USE)	1SPACE / 250 SF
BUILDING SETOTAL	28.800 SF
PARKING REQUIRED	11BSPACES
	133 SPACES
PARKING PROVIDED	I IOUU, NOLU
PARKING PROVIDED	
	1 SPACE (SPERSON
PARKING CALCS. (DAY CARE)	1 SPAGE / BPERSON 300
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PARKING CALOS. (DAY CARE) TOTAL CAPACITY PARKING REQUIRED	300 50 SPACES
PARKING CALCS. (DAY CARE) TOTAL CAPACITY	300
PARKING CALOS. (DAY CARE) TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED	300 90 SPACES 30 SPACES
PARKING CALOS. (DAY CARE) TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING REQUIRED	300 50 SPACES 30 SPACES 166 SPACES
PARKING CALOS. (DAY CARE) TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED	300 90 SPACES 30 SPACES
PARKING CALOS. (DAY CARE) TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING REQUIRED TOTAL PARKING PROVIDED	300 50 SPACES 30 SPACES 188 SPACES 193 SPACES
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PARKING CALOS. (DAY CARE) TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING REQUIRED TOTAL PARKING PROVIDED ADA PARKING ALCS. ADA PARKING REQUIRED	300 30 SPACES 30 SPACES 168 SPACES 193 SPACES 191 TO 200 = 6 6 SPACES
PARTING CALCS. (DAY CARE) TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING REQUIRED TOTAL PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING CALCS.	300 90 SPACES 30 SPACES 168 SPACES 193 SPACES 151 TO 200 = 6
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PARKING CALOS. (DAY CARE) TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING REQUIRED TOTAL PARKING OF OWNER ADA PARKING OLOS. ADA PARKING COULIED ADA PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING PROVIDED	300 50 SPACES 30 SPACES 168 SPACES 193 SPACES 151 TO 200 = 6 6 SPACES 6 SPACES
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PARKING CALCS (DAY CARE) TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING REQUIRED TOTAL PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING PROVIDED RCA RCA RECUIRED RCA PCA PERIMETER SUFFERS)	300 30 SPACES 30 SPACES 188 SPACES 193 SPACES 191 TO 200 = 6 6 SPACES 6 SPACES 1.55 AC (57,518 SF) - 25%
PARKING CALCS (DAY CARE) TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING PROVIDED RCA RCA RECUIRED RCA RCA (PERIMETER BUFFERS) RCA (SCIM)	300 30 SPACES 30 SPACES 108 SPACES 1.55 AC (57,518 SF) - 25% 1.55 AC (27,518 SF) - 25%
PARKING CALCS. (DAY CARE) TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING PROVIDED RCA RCA RECUIRED RCA RCA (PERIMETER BUFFERS) RCA (STRAM FILFFERS)	300 90 SPACES 30 SPACES 198 SPACES 198 SPACES 193 SPACES 191 SPACES 6 SPACES 6 SPACES 1.55 AC (57,518 SP) - 25% 1.55 AC (77,914 SP) - 27% 27 AC (XX SP)
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PARKING CALCS (DAY CARE) TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING REQUIRED TOTAL PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING PROVIDED RCA RCA PARKING PROVIDED RCA RCA RECUIRED RCA (STREAM BIFFERS) RCA (SCIM) RCA PREVIDED OS REQUIRED OS REQUIRED OS PROVIDED	300 30 SPACES 30 SPACES 188 SPACES 188 SPACES 193 SPACES 193 SPACES 151 TO 200 = 0 6 G 3 PACES 6 S PACES 1.55 AC (57,518 SF) - 25% 1.55 AC (57,518 SF) - 25% 1.55 AC (57,518 SF) - 27% 1.55 AC (57,518 SF) - 27% 1.55 AC (57,518 SF) - 25% 1.55 AC (57,518 S
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CONCEPT PLANS

ALL INFORMATION AND LINE WORK RETRIEVED FROM WAKE COUNTY GIS MAP SERVICES. THIS IS INTENDED FOR CONCEPTUAL USE ONLY. LINE WORK IS APPROXIMATE AND SUBJECT TO CHANGE UPON OBTAINING SURVEY.

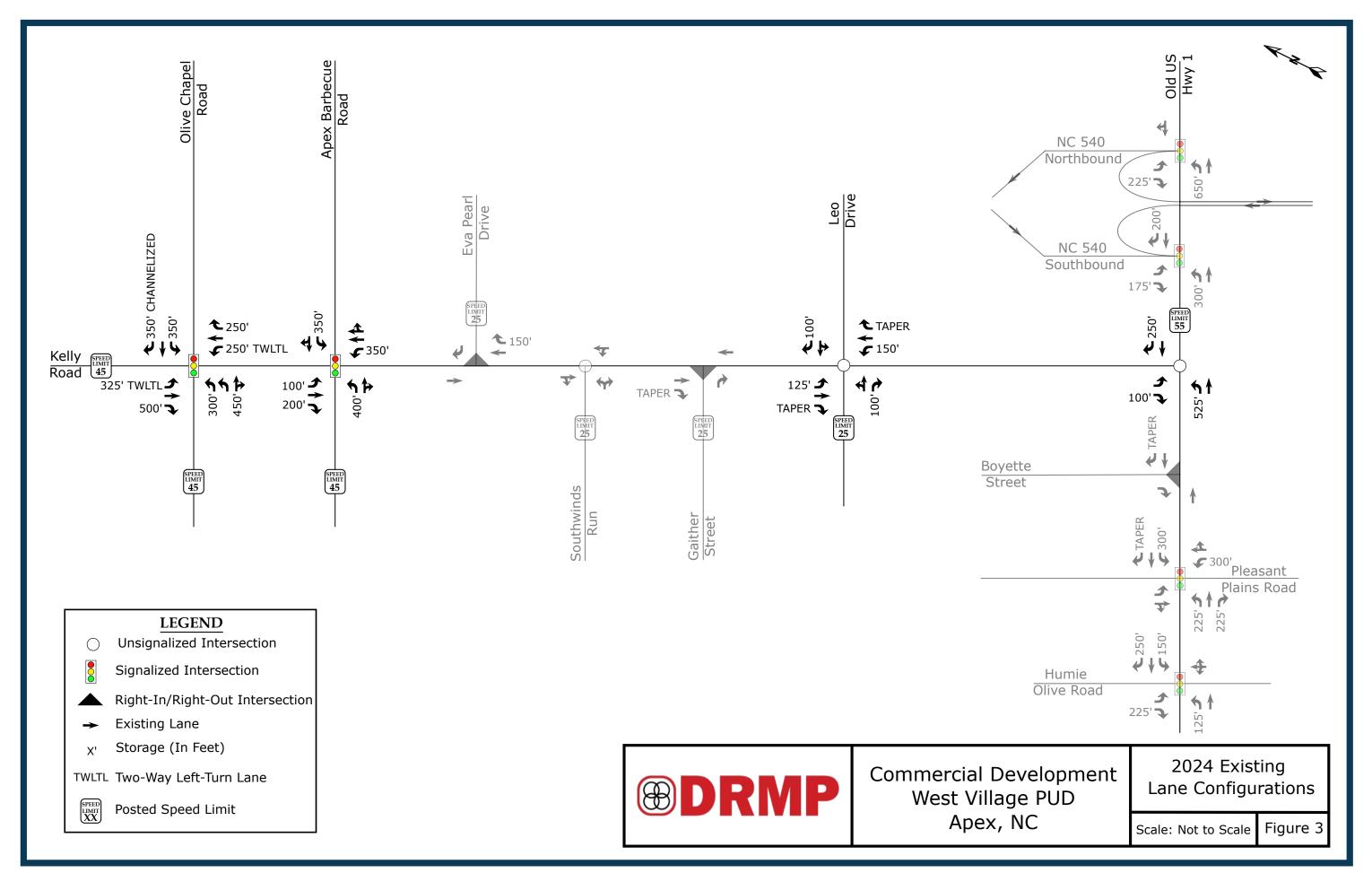
ALL SITE DATE NOTED IN RED IS TO BE DETERMINED (TBD).

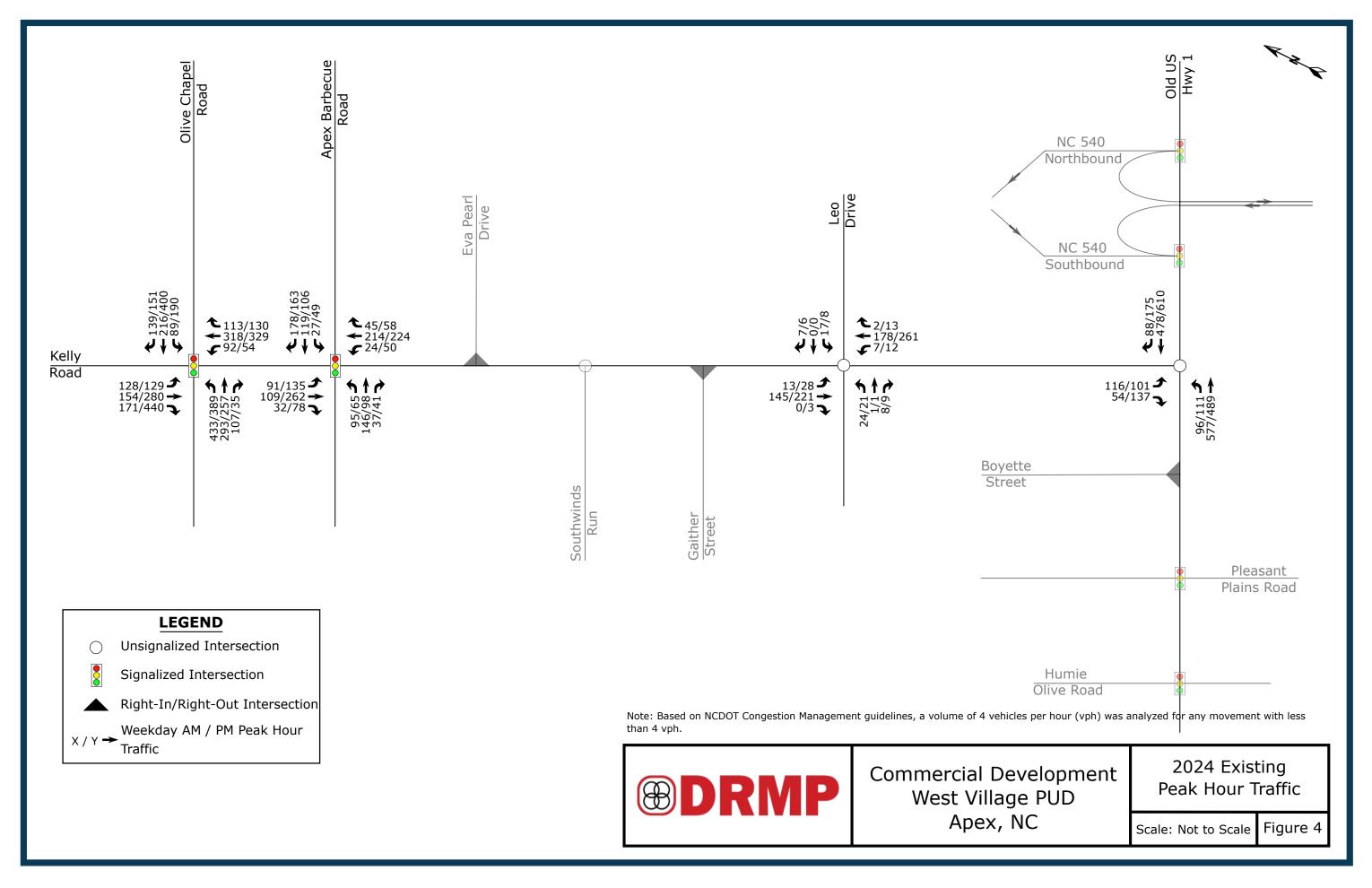


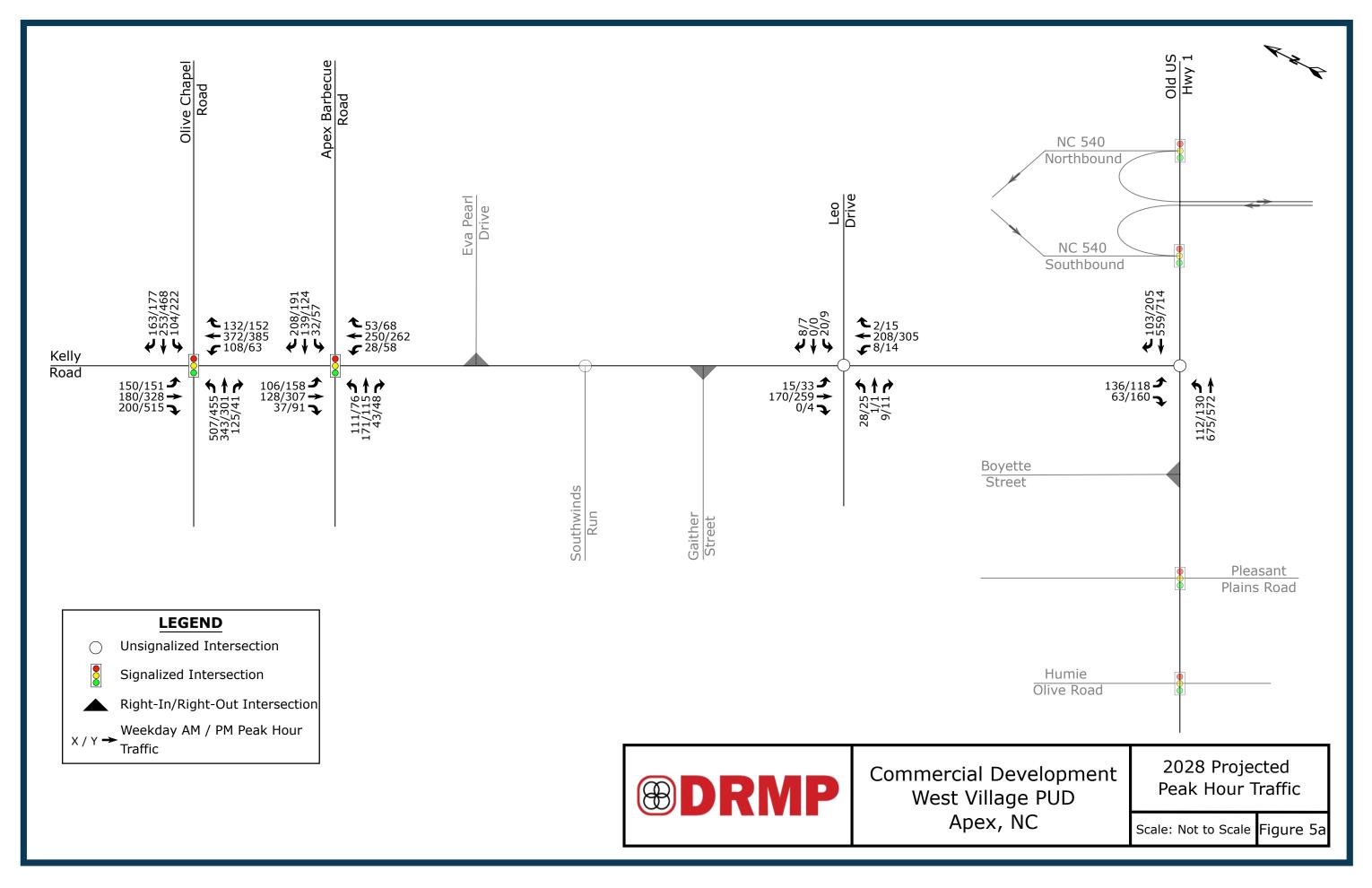
KELLY ROAD COMMERCIAL 2517 KELLY ROAD

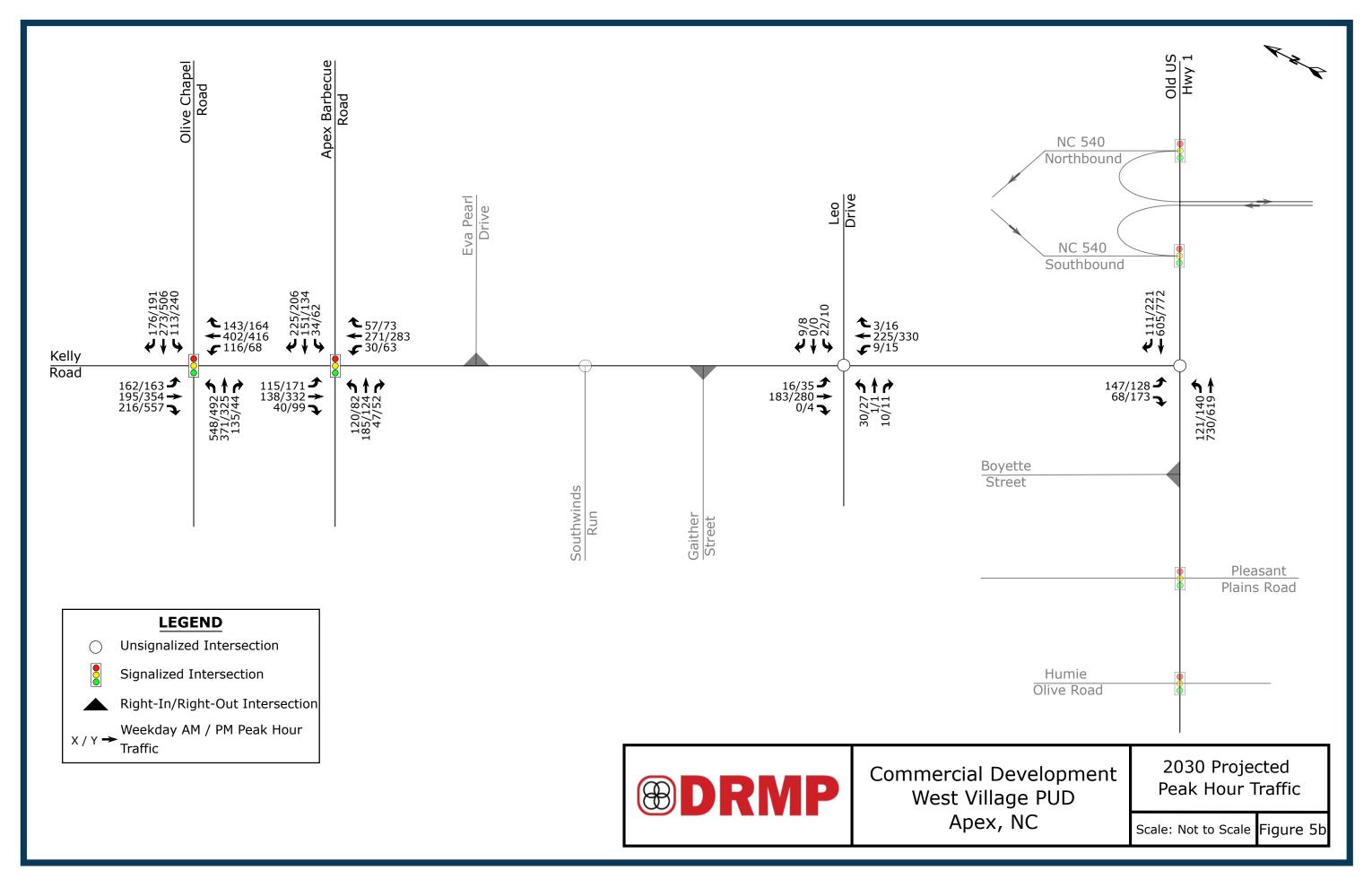
2517 KELLY ROAD APEX NC WAKE COUNTY

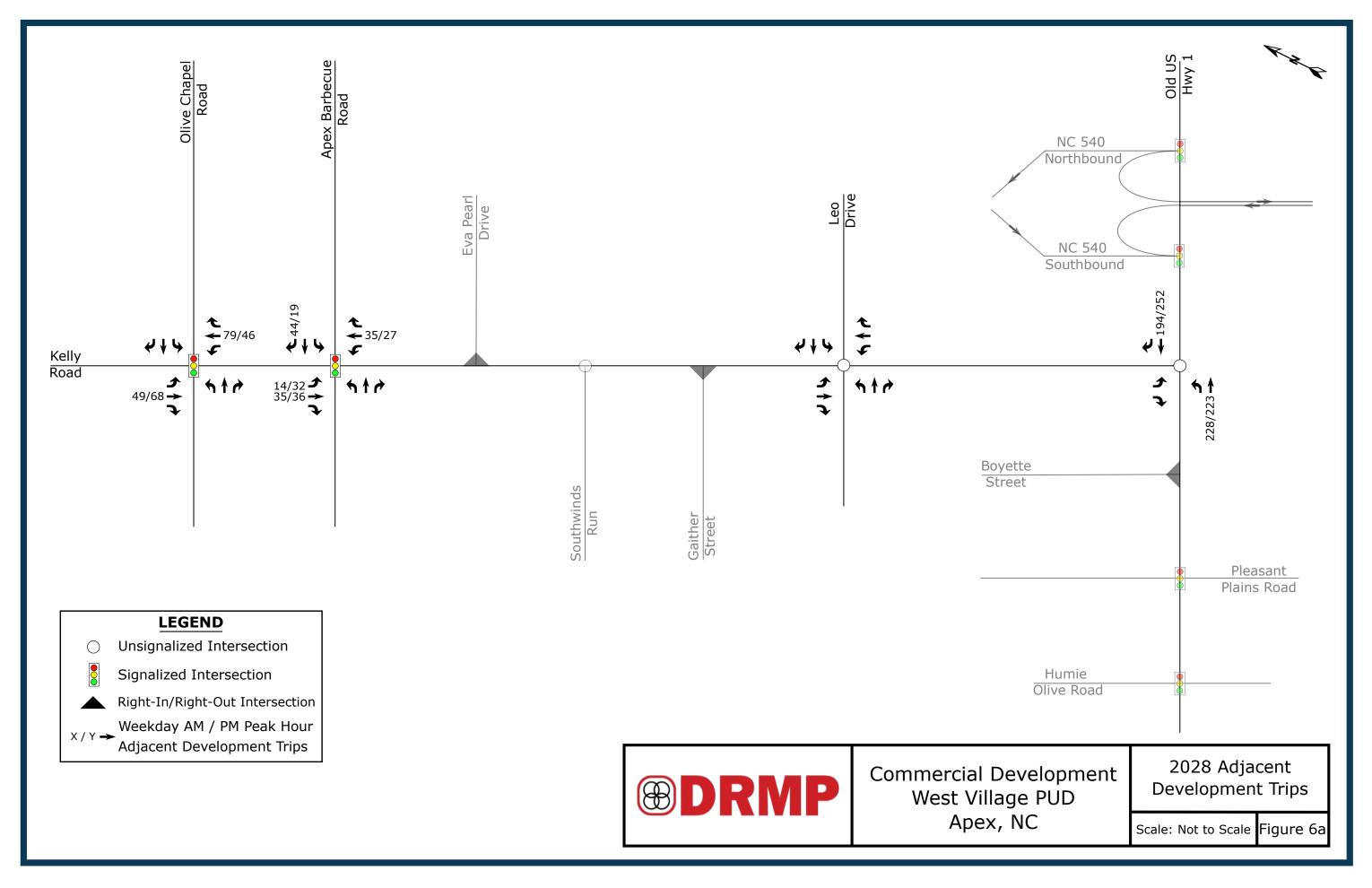
DRAWN BY:	CHECKED BY:	PROJECT NO.
MD	JR	240306
DATE		SHEET NO.
O	3.12.24	SP-2

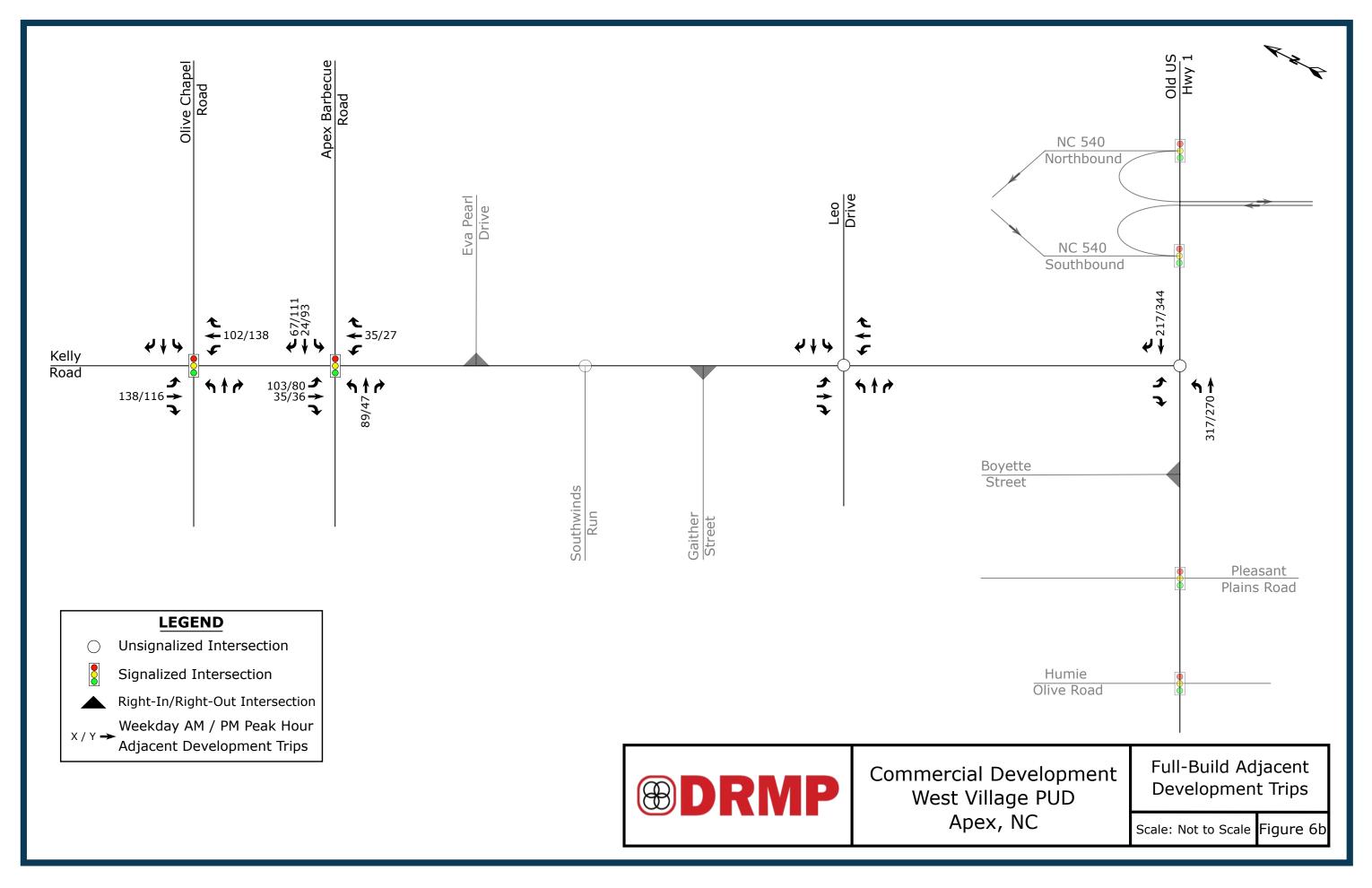


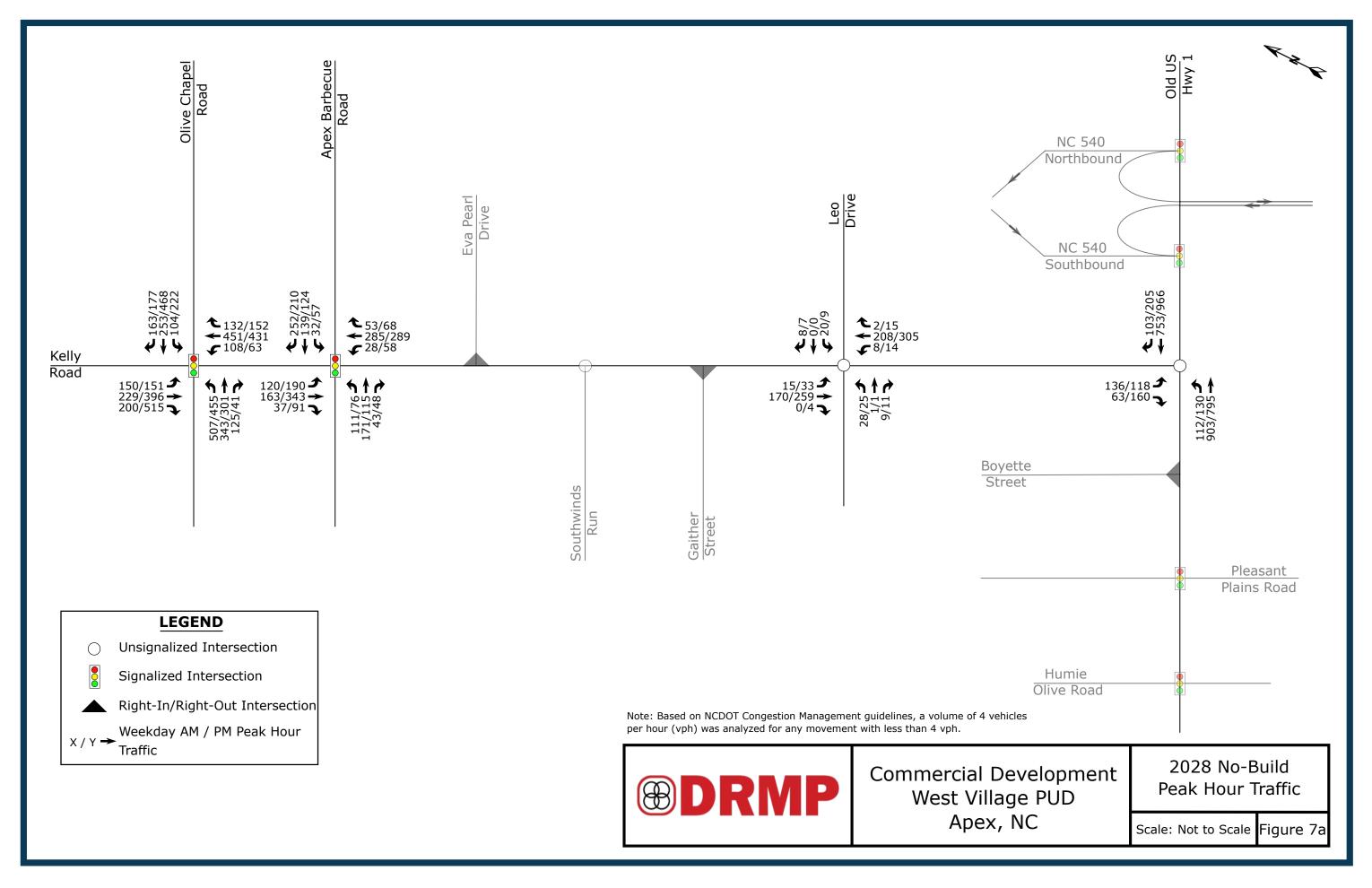


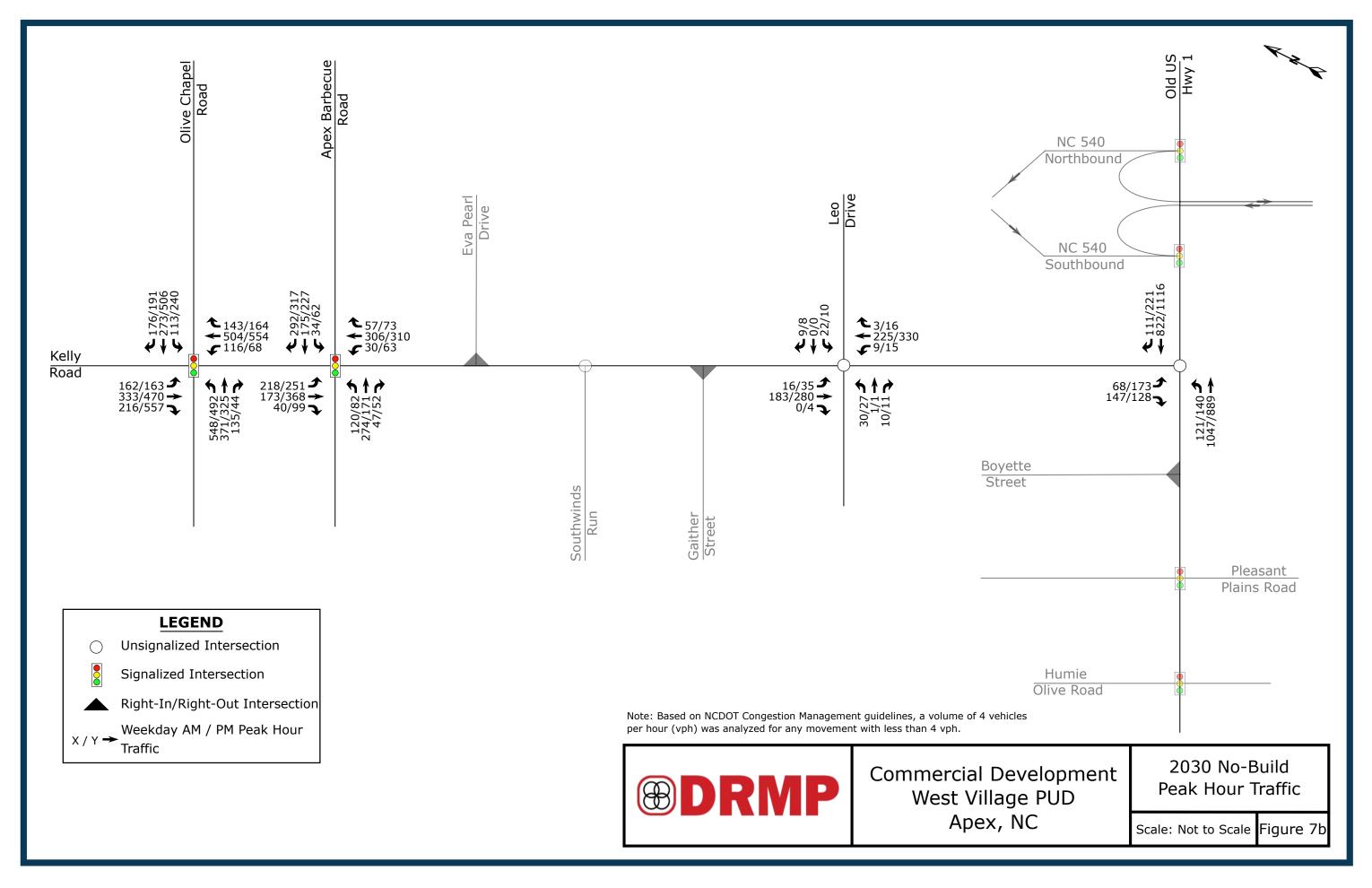


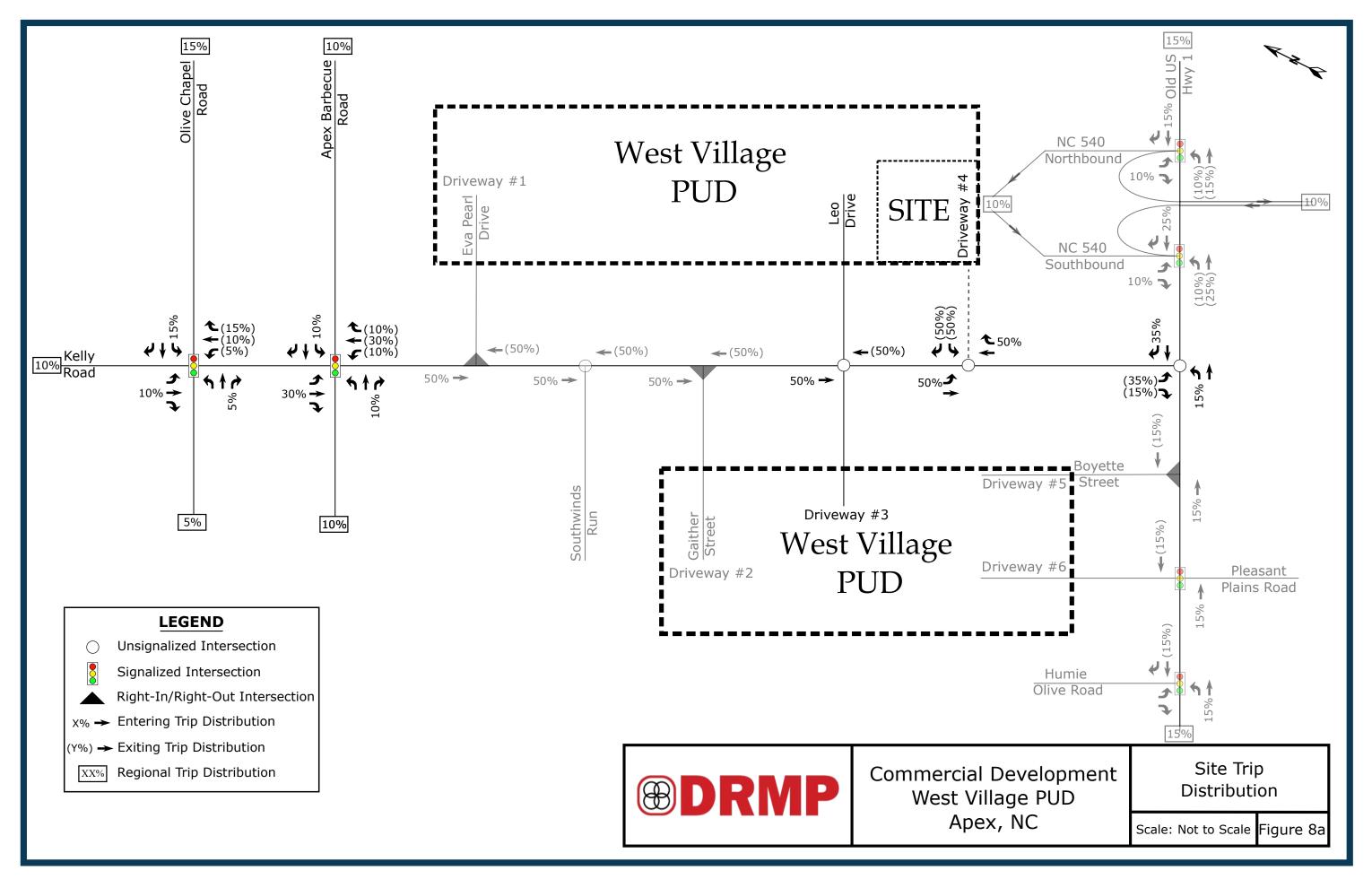


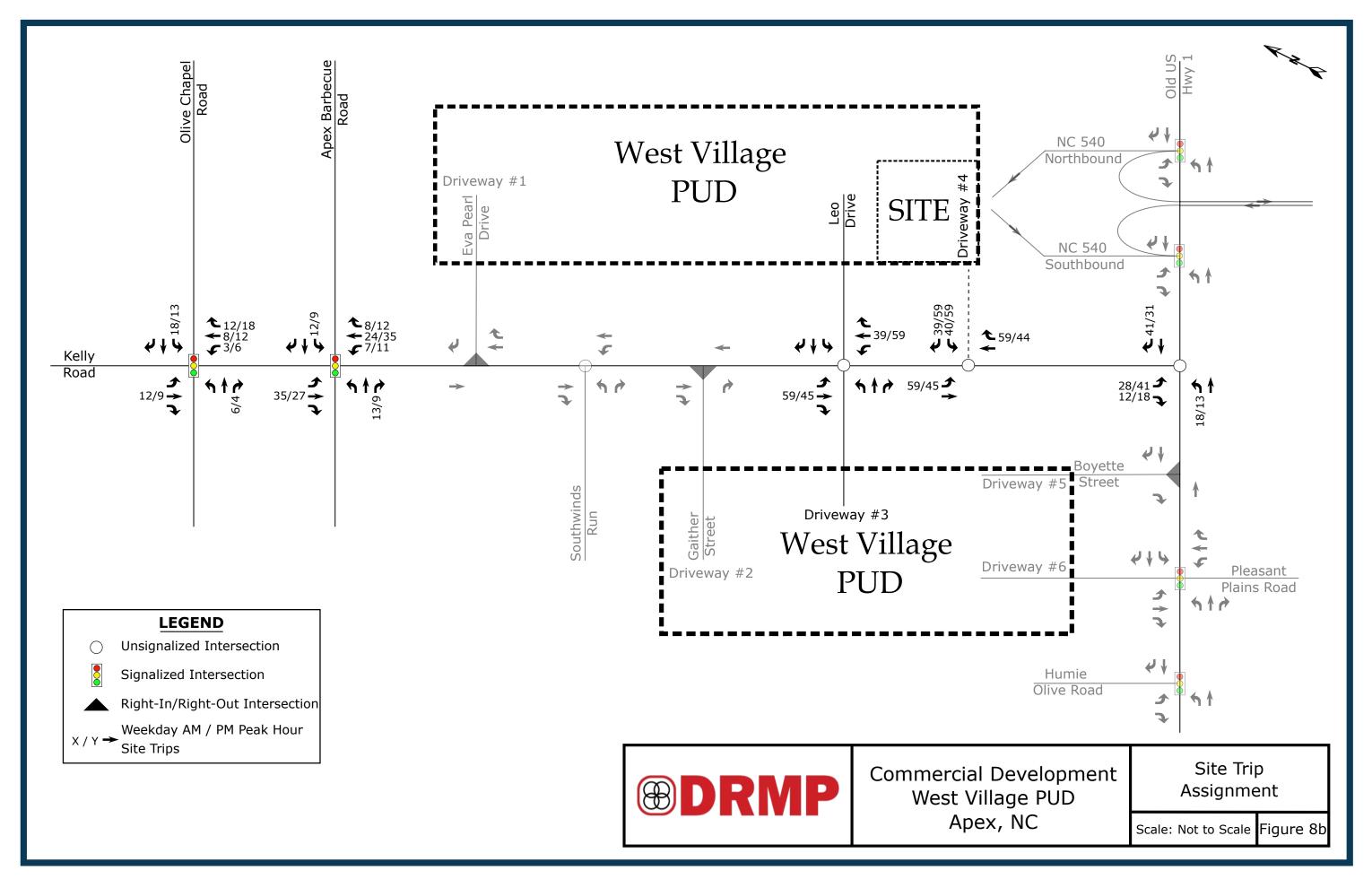


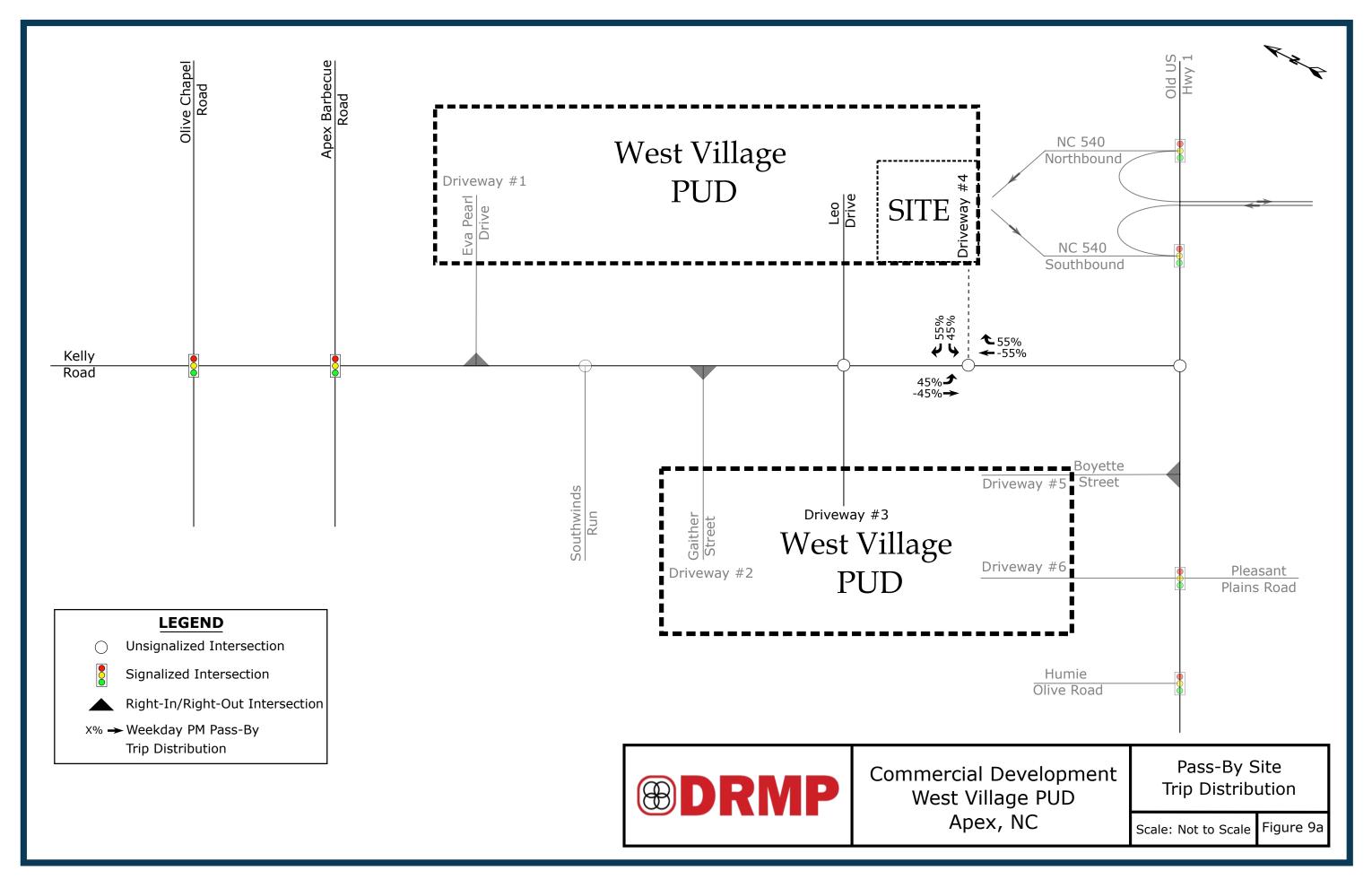


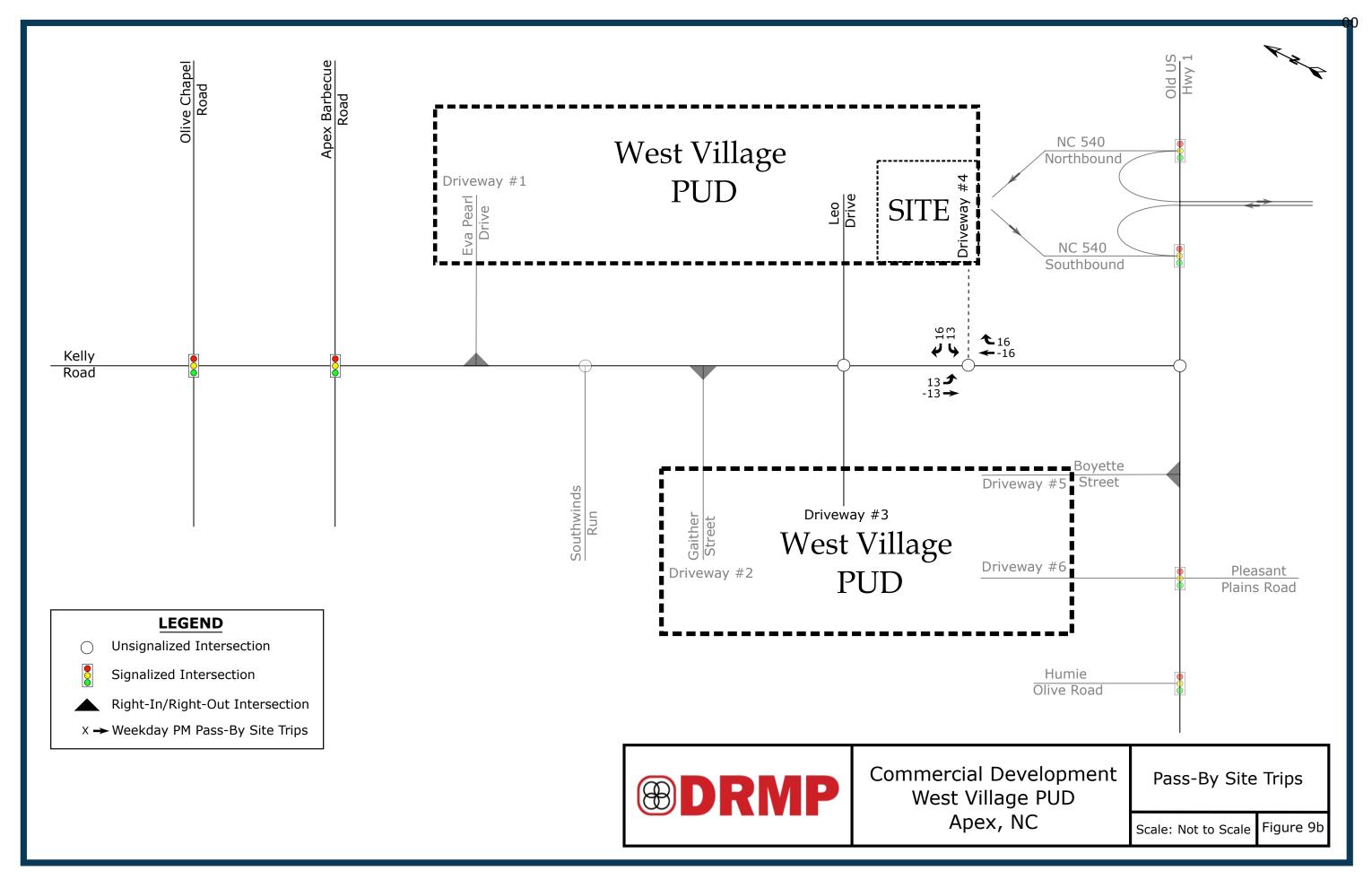


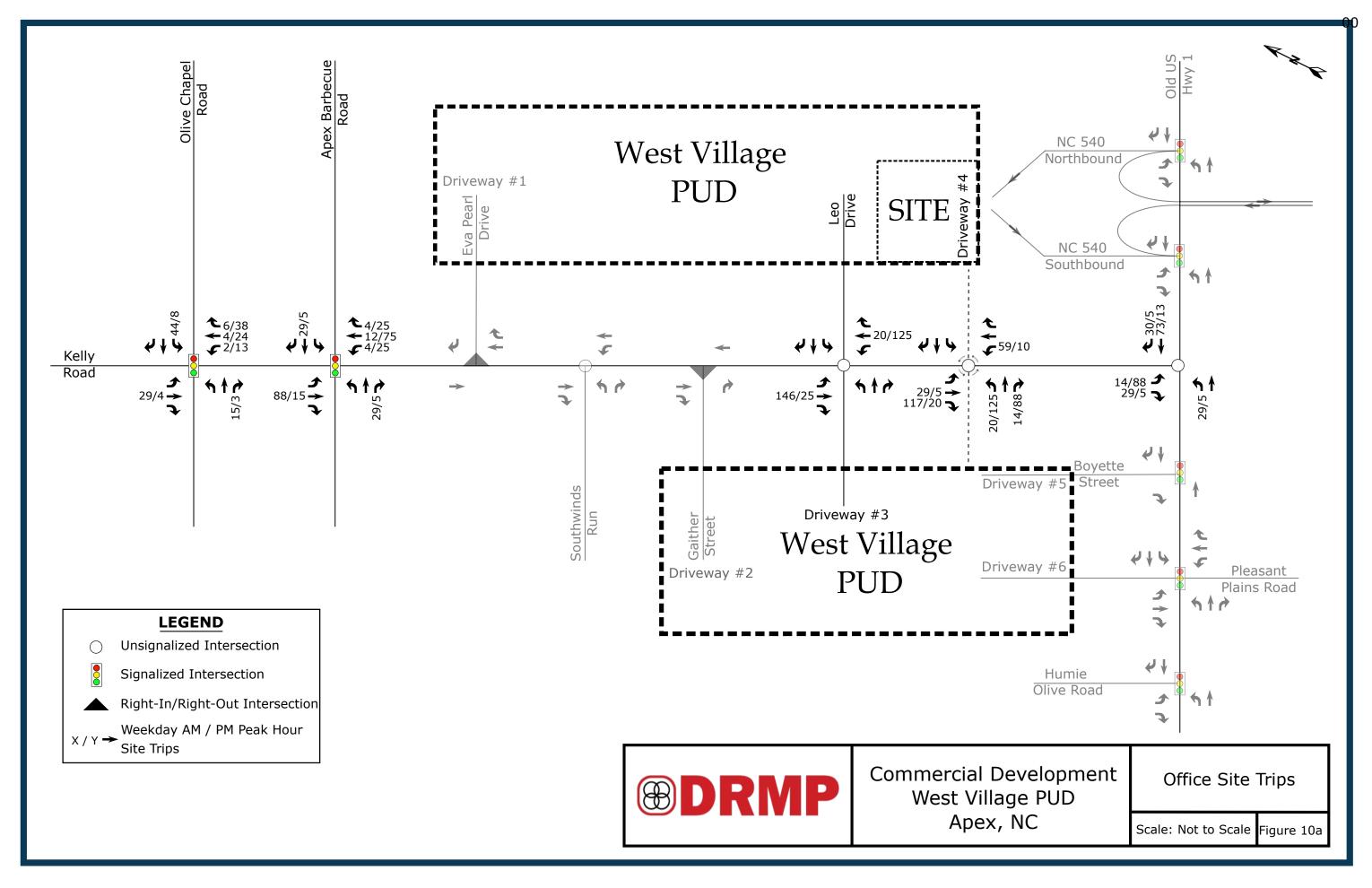


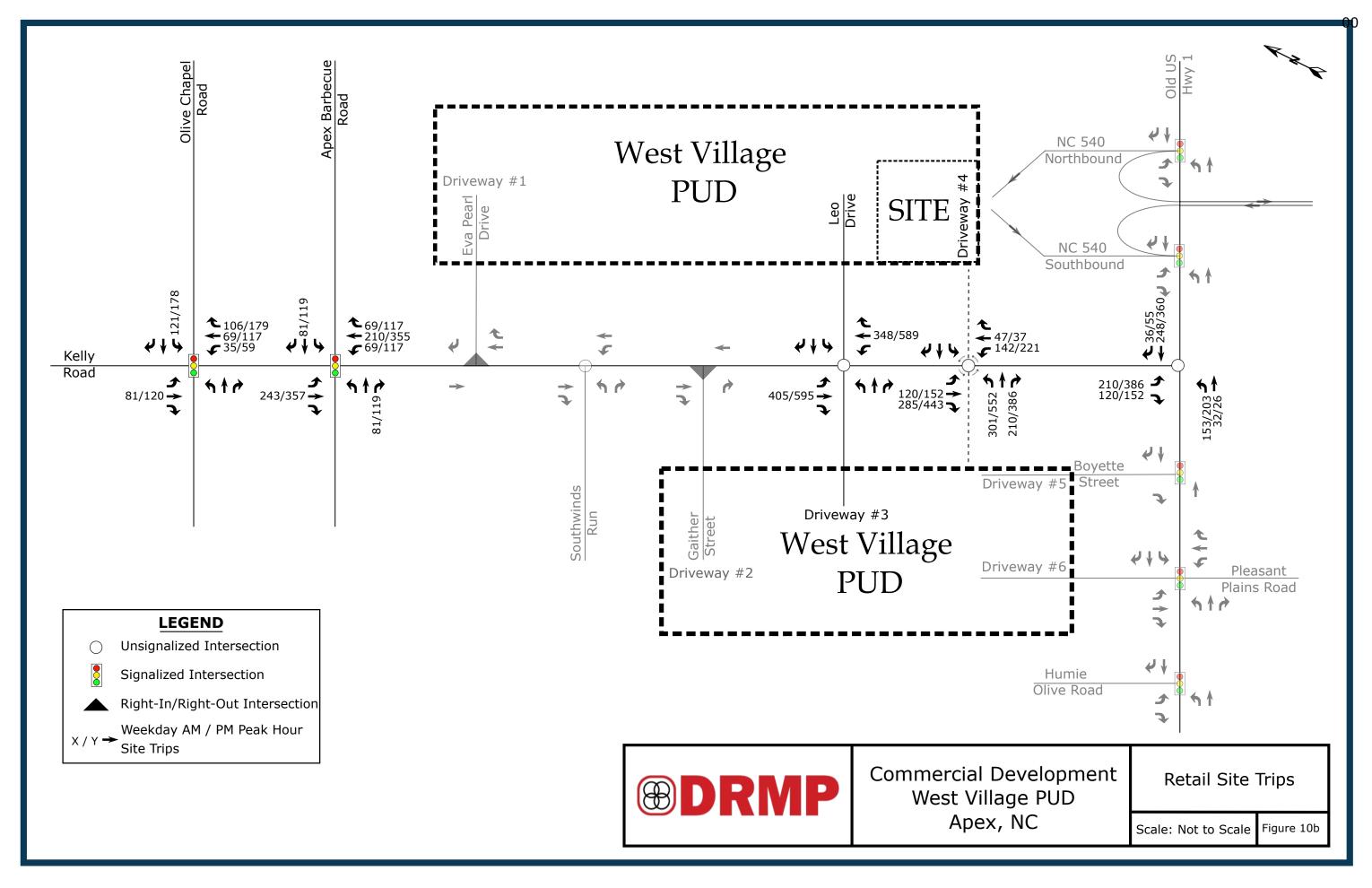


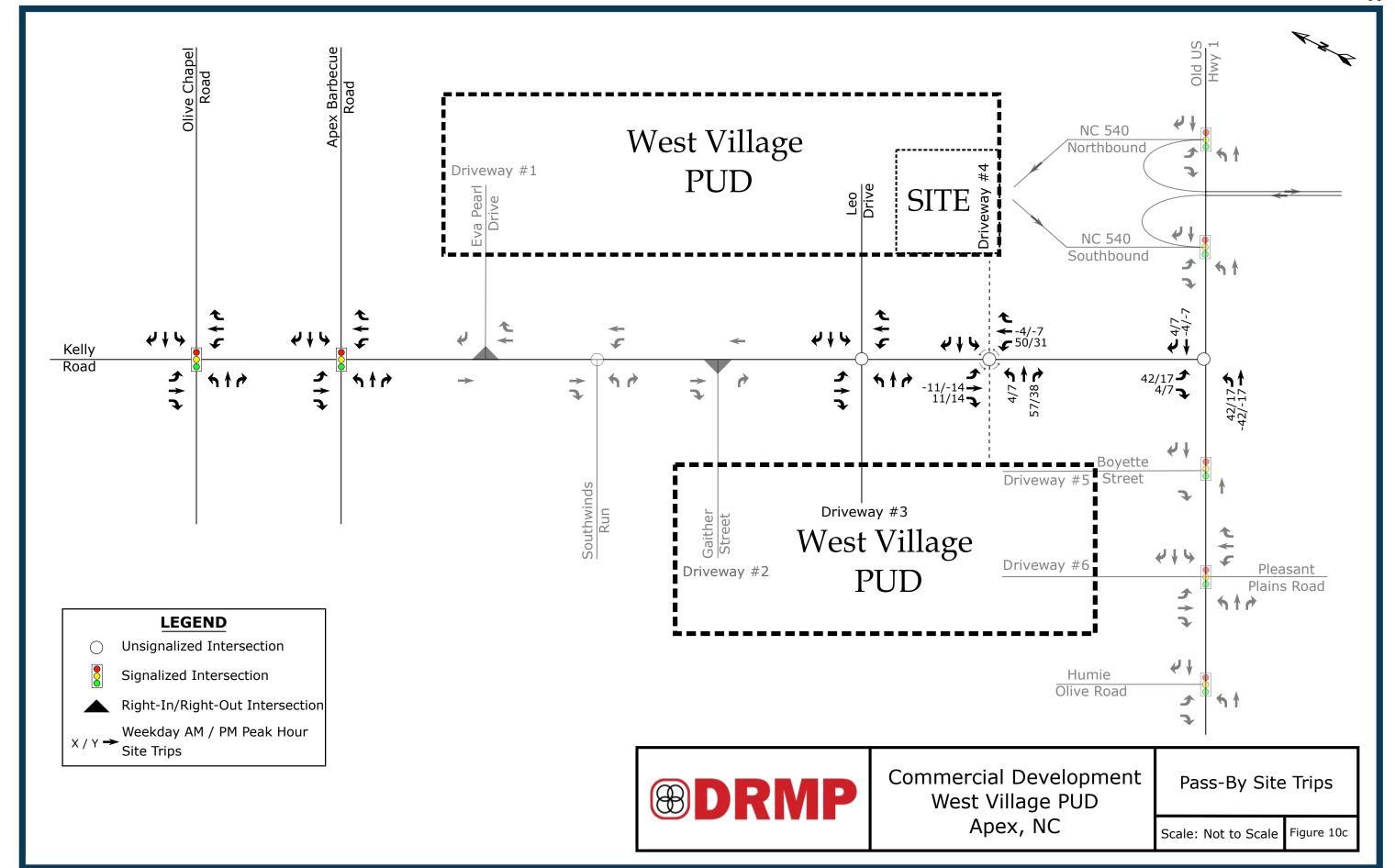


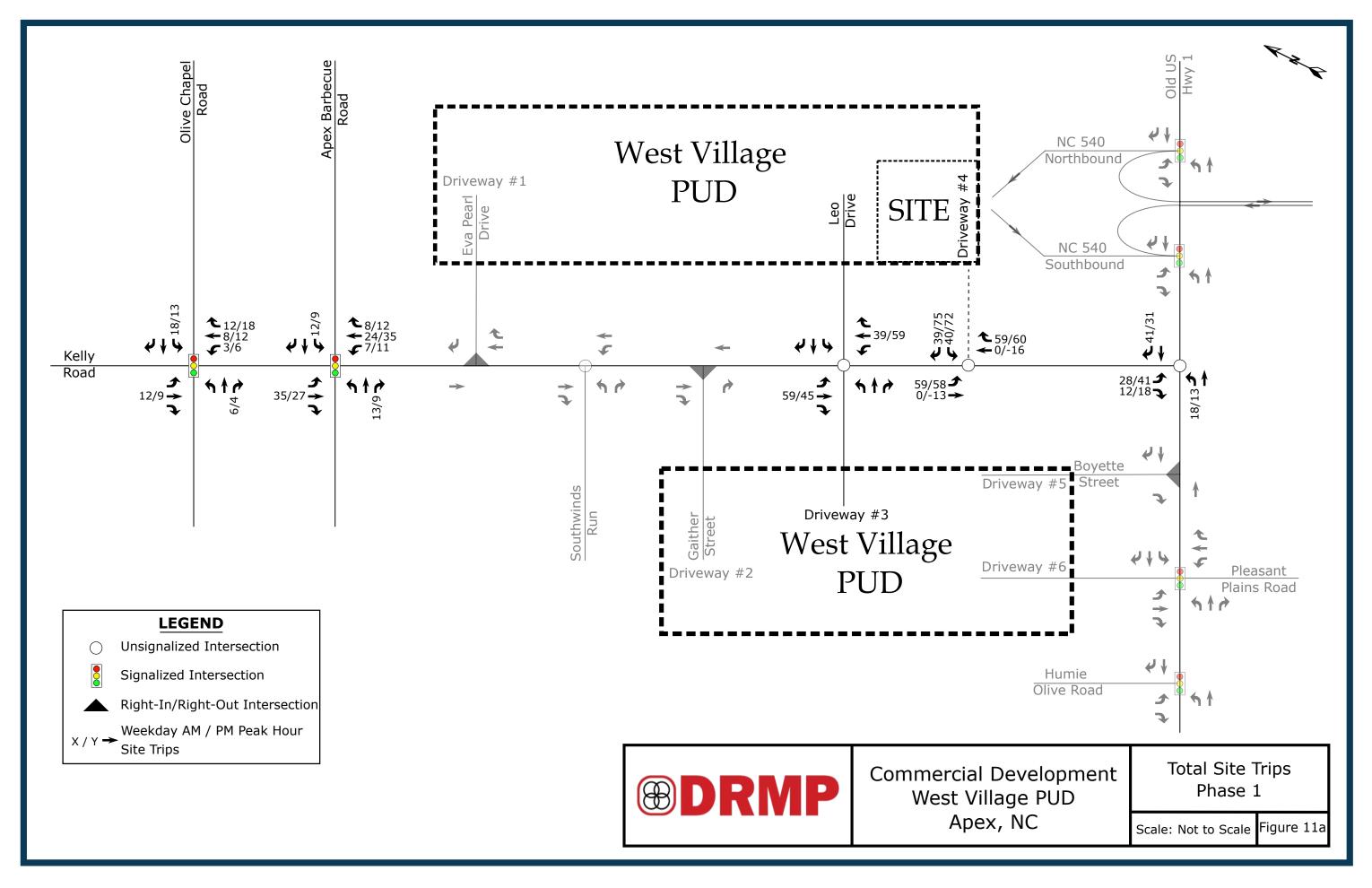


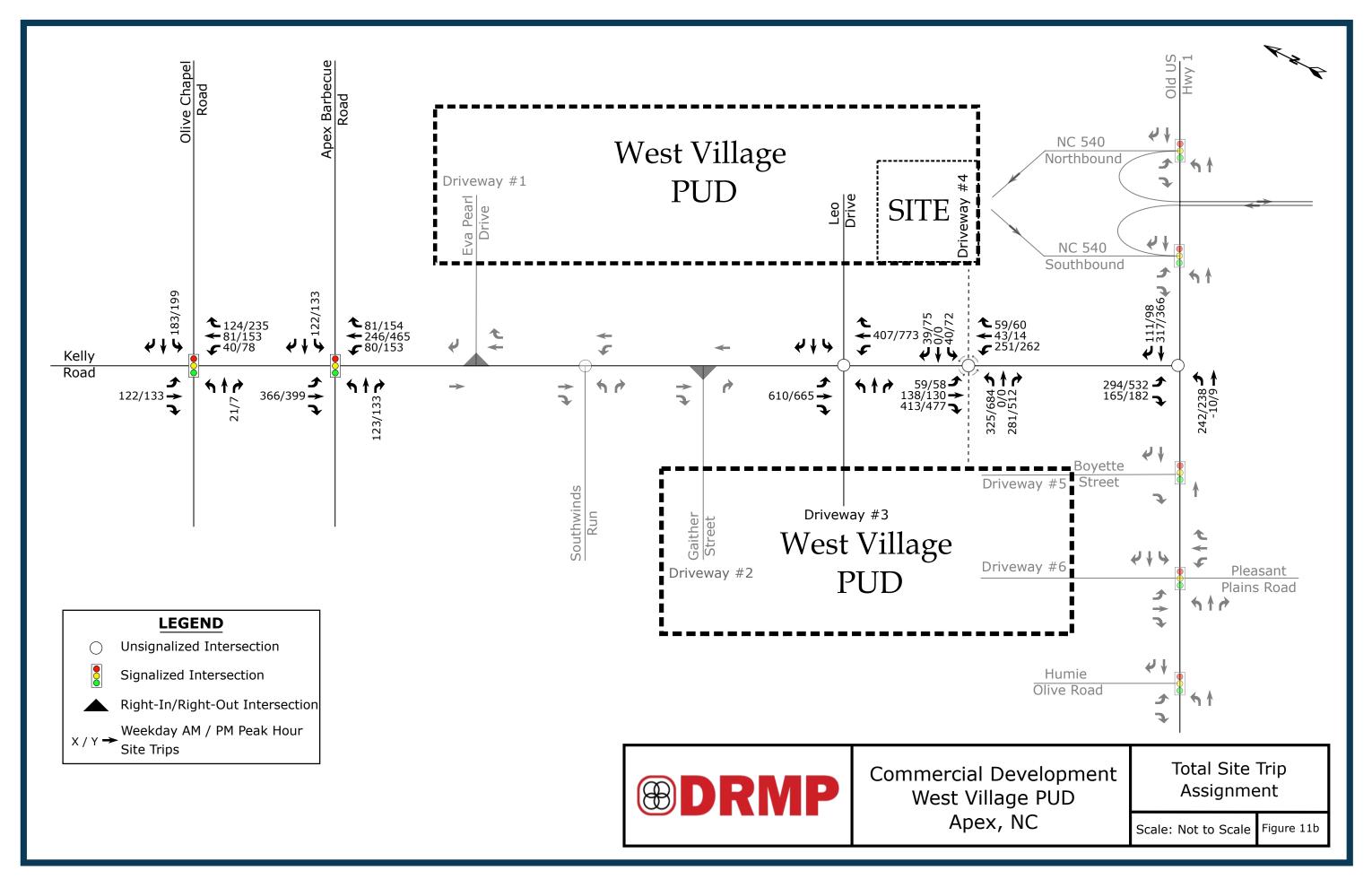


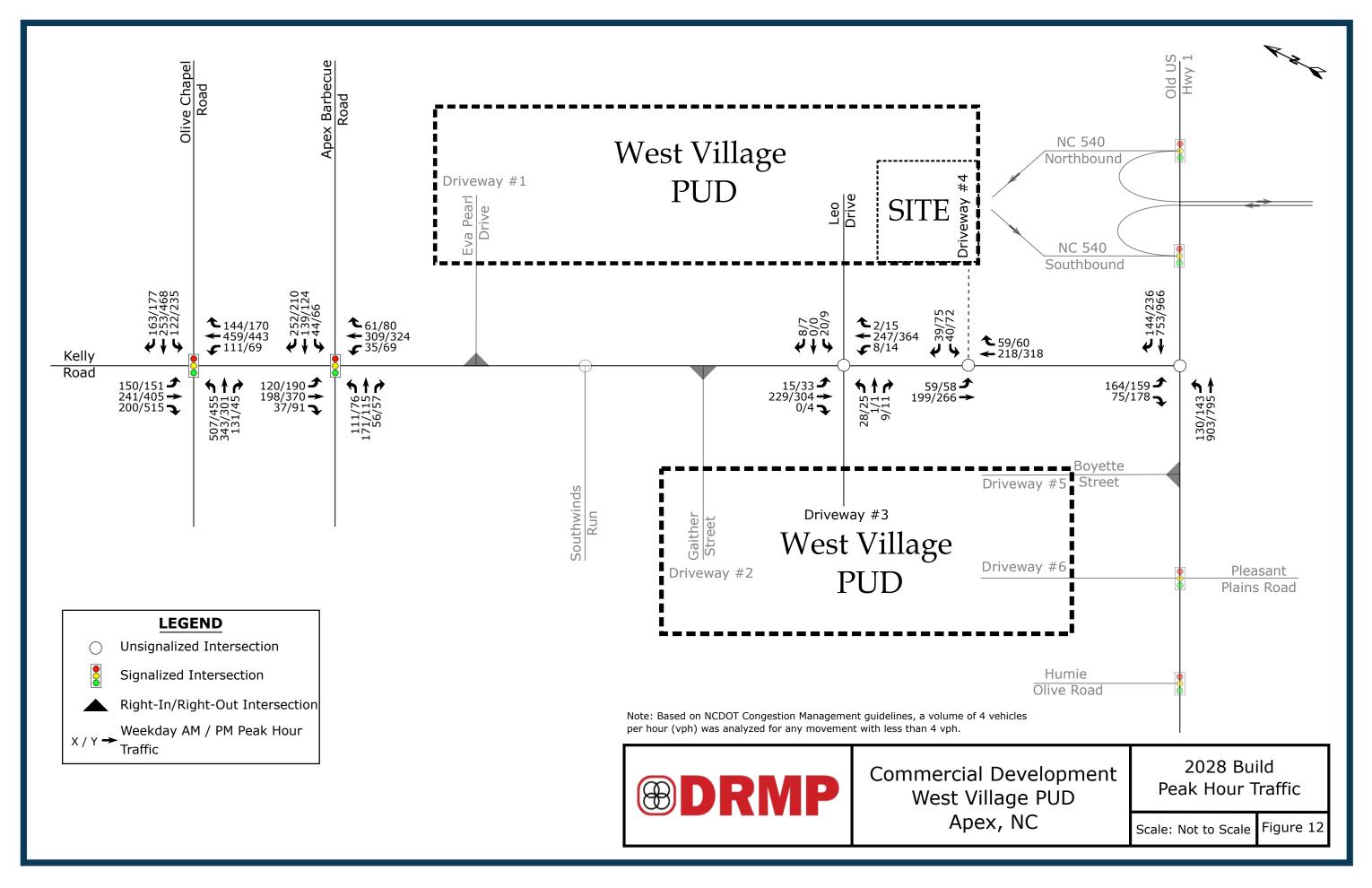


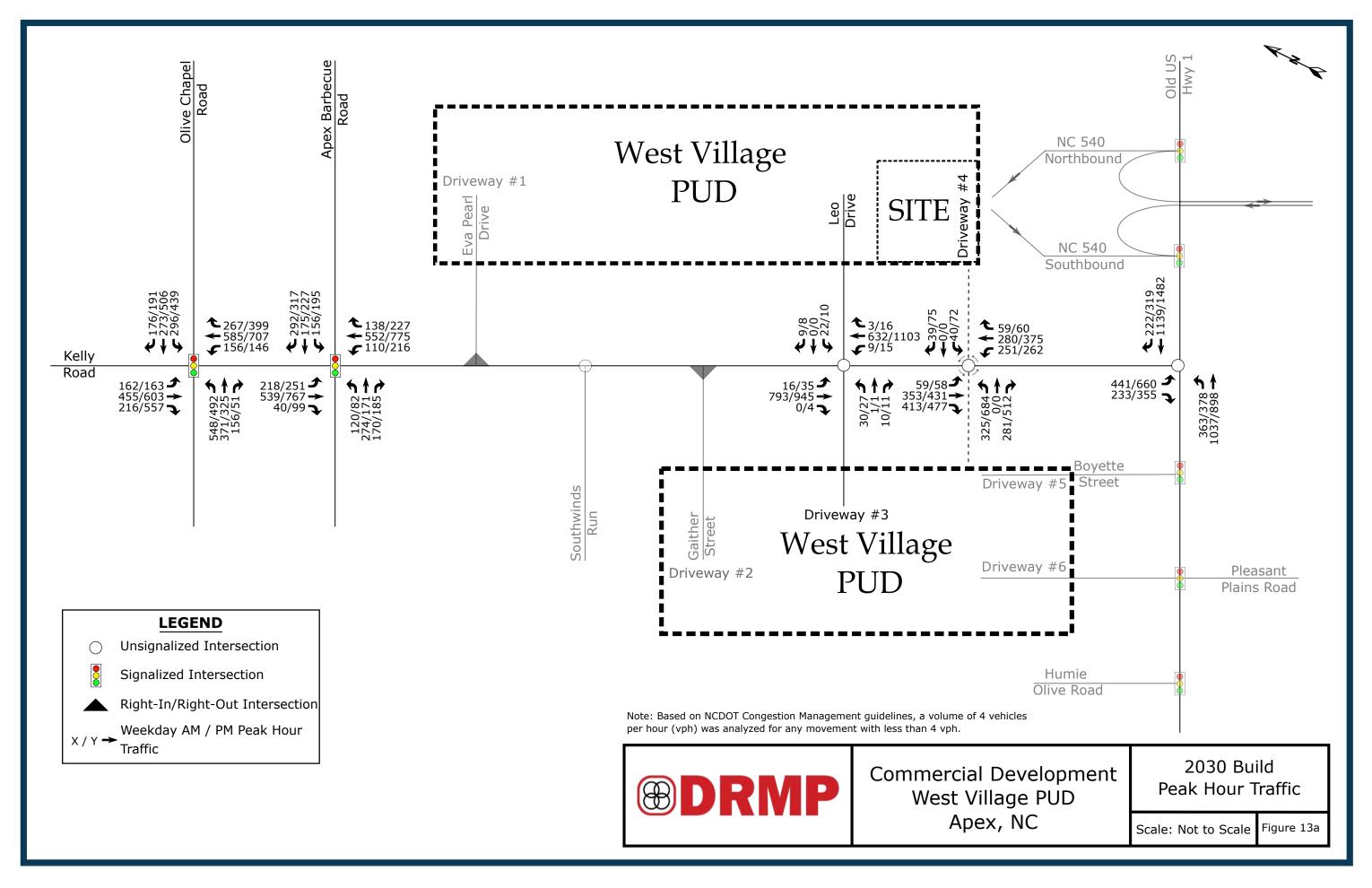


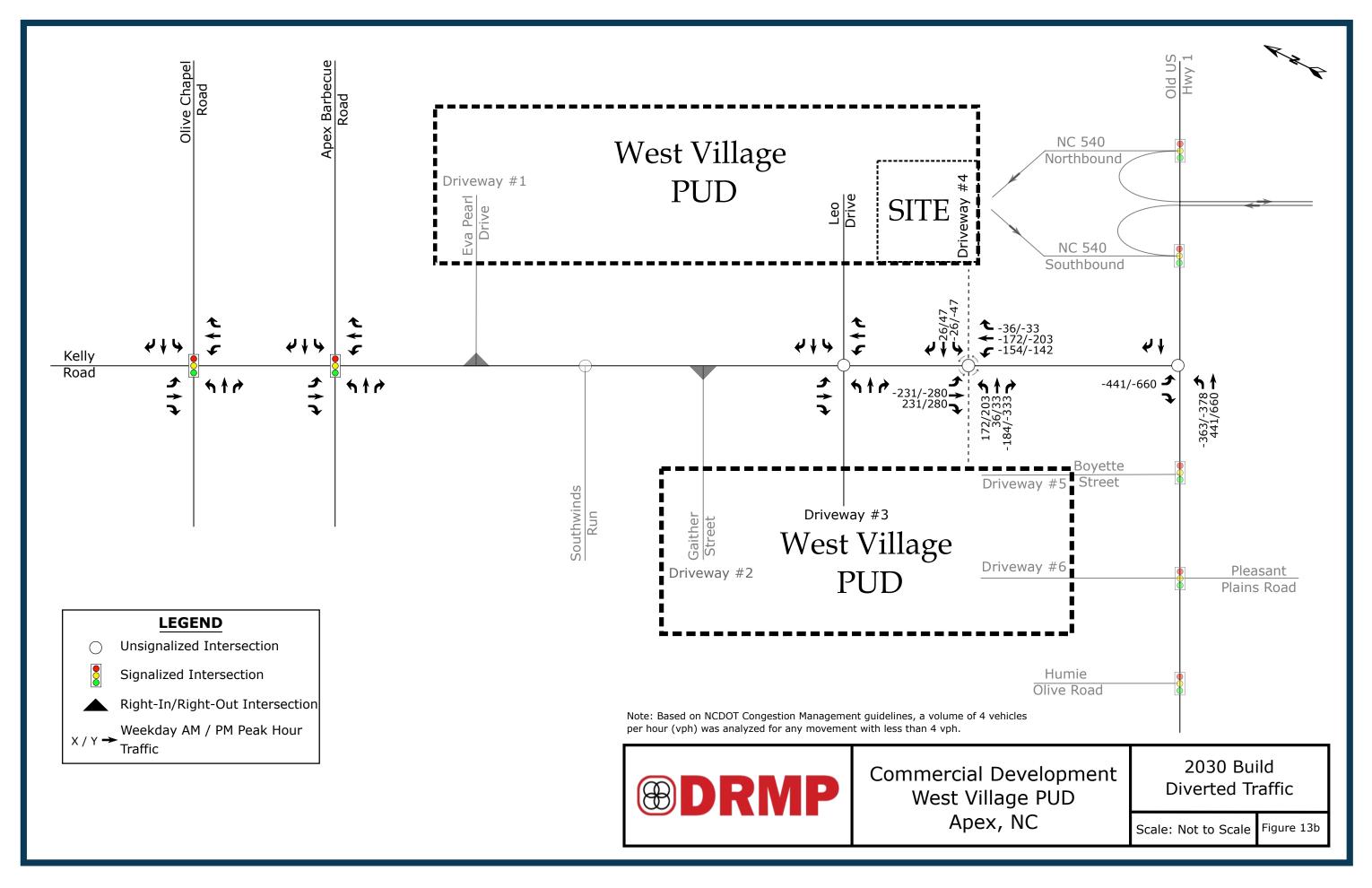


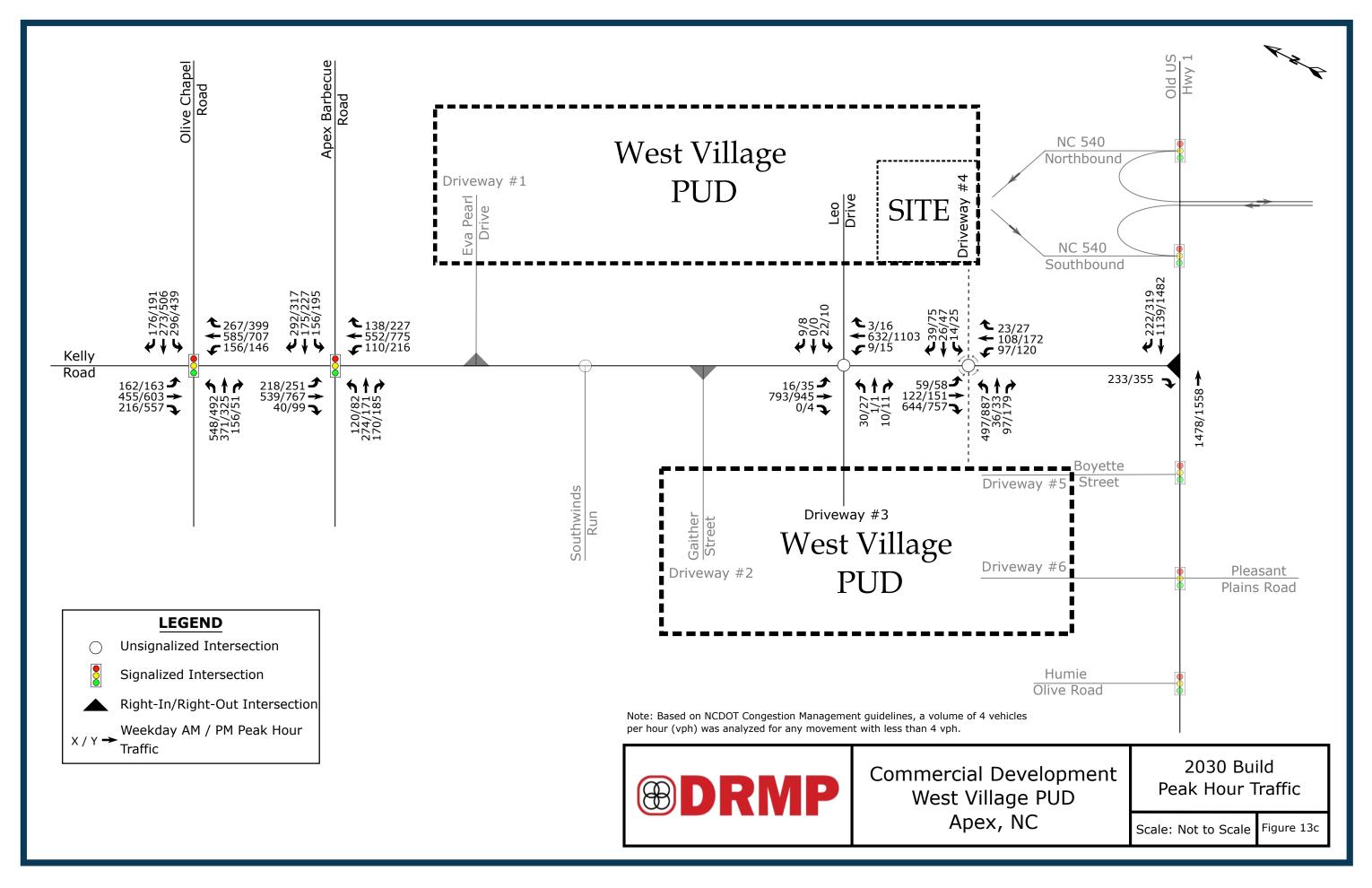














SCOPE LETTER



October 30, 2024

Sajid Hassan

Town of Apex Transportation

105-B Upchurch Street

Apex, NC 27502

919-372-7360

Sajid.Hassan@apexnc.org

[Sent via Email]

Reference: West Village Commercial Development

Subject: Updated MOU for Traffic Phasing Study

Dear Mr. Hassan:

The following is an updated Memorandum of Understanding (MOU) outlining the proposed scope of work and assumptions related to the traffic phasing study to be prepared for the development of the initial phase of commercial development in the West Village PUD. This MOU is based on an initial virtual meeting on September 27th and incorporates comments received from the Town's initial review of the MOU. It is our understanding that a phasing study would be required to address impacts of the initial phase of the commercial development.

The initial phase of commercial development is proposed along the east side of Kelly Road and would include a day care ($\sim 12,130 \text{ s.f.}$ building) and a two-story building with office / retail space ($\sim 29,000 \text{ s.f.}$ total). The day care building would be completed initially, and the mixed-use building would be completed afterwards. This development is the first phase of the commercial portion of the West Village PUD. The property is approximately 8% of the overall West Village development.

Access would be provided via one new full movement driveway on Kelly Road and a connection to the north to Fahey Drive. The new full movement driveway is ultimately planned for a roundabout, but the roundabout would not be constructed with the initial phase of development. The connection to Fahey Drive would provide an indirect second access to Kelly Road via Leo Drive.



Study Area

Based on the call with City staff on September 27th and follow up coordination, it is our understanding the study area would need to include intersections from the original TIA and PUD that have required improvements that are not constructed and that would be relevant to this phase. intersections will include:

- Olive Chapel Road & Kelly Road (signalized)
- Apex Barbeque Road & Kelly Road (signalized)
- Leo Drive and Kelly Road (unsignalized)
- Kelly Road and Proposed Site Driveway (Site Driveay #4 in TIA)
- Old US 1 and Kelly Road

Other intersections **NOT** included in the phasing study scope include:

- Kelly Road and Eva Pearl Drive (Site Driveway #1) already built
- Southwinds Run and Kelly Road not relevant
- NC 540 Northbound ramps and Old US 1 No improvements remain
- NC 540 Southbound ramps and Old US 1 No improvements remain
- Old US 1 and Boyette Street (Site Driveway #5) not relevant
- Old US 1 and Pleasant Plains Road / Site Driveway #6) not relevent
- Old US 1 and Humie Olive Roak not relevant

Existing Traffic Volumes

Current traffic counts would be required for the existing study intersections above during a typical weekday AM (7:00 AM - 9:00 AM) and PM (4:00 PM - 6:00 PM) peak periods, while schools are in session.

Background Traffic Volumes

Background traffic volumes will be determined by projecting 2024 existing traffic volumes to the year Phase 1 build year using an annual growth rate of 4% as required by the Town. The following adjacent developments are to be considered:

- o Depot 499
- Townes at Pleasant Park (fka Sears Property)



- o Friendship Village
- o Holland Road Assembly

Future Roadway Improvements

It was discussed that other developments could be completing roadway improvements in the area. No other improvements are planned by the Town of Apex or NCDOT at the study intersections.

Trip Generation

Average weekday daily, AM peak hour, and PM peak hour trips for the initial phase of commercial development were estimated using methodology contained within the ITE Trip Generation Manual, 11th Edition. Refer to Table 1 for a summary of the trip generation for the initial commercial phase.

Table 1: Trip Generation Summary - Initial Commercial Phase

Land Use (ITE Code)	Intensity	Daily Traffic (vpd)	AM Peak Hour Trips ic (vph)		ır Trips	PM Peak	Weekda Hour Tri	ay ips (vph)
		(TPG)	Enter	Exit	Total	Enter	Exit	Total
Day Care (565)	12,130 SF	576	71	62	133	63	72	135
General Office (710)	14,400 SF	215	28	4	32	6	27	33
Retail Shopping Center (822)	14,400 SF	837	22	15	37	50	51	101
Total Trip	S	1,628	121	81	202	119	150	269
Internal Capture Trips AM (4% Enter, 10% Exit) PM (3% Enter, 2% Exit)			-2	-2	-4	-2	-2	-4
Pass-By Trips (31% AM, 40% PM)			-6	-5	-11	-20	-20	-40
Primary (New) Trips		1,628	113	74	187	97	128	225



Internal capture trips were calculated based on the NCHRP 684 for the office and retail uses. The day care is not included in the internal capture calculations; however, it is likely that some day care trips will be internal capture. Refer to the attached NCHRP reports for the internal capture calculations.

In addition, pass-by trips for the retail space will be calculated using the ITE Trip Generation manual, which includes pass-by trip rates of 31% in the AM and 40% in the PM.

Trip Distribution and Assignment

Site trips are distributed based on the regional trip distribution from the original TIA report and modified as appropriate near the site driveway. Refer to the attached site trip distribution figure.

Although there is interconnectivity to the north, this traffic study assigns all development trips to the new site access to be conservative.

Analysis Scenarios

All capacity analyses will be performed utilizing Synchro (Version 11). All study intersections will be analyzed during the weekday AM and PM peak hours under the following proposed traffic scenarios:

- Existing 2024 Conditions
- No-Build 2028 (Phase 1 Build Year) Conditions
- Build 2028 Phase 1 Conditions
- Build 2028 Phase 1 Conditions With Necessary Required Improvements
- No-Build 2030 (Full Build Year) Conditions
- Build 2030 (Full Build Out) Conditions with All Required Improvements

This study assumes the initial commercial development (Phase 1) would be completed by 2028. Although a schedule is unknown for the remaining commercial development within the PUD, this scope assumes a full build out year of 2030.

Report

The Traffic Phasing Study will be prepared based on Town and NCDOT requirements.



If you find this memorandum of understanding acceptable, please let me know. If you should have any questions or comments regarding this letter, please feel free to contact me at rstephenson@drmp.com or by phone at 919-872-5115.

Sincerely,

Rynal Stephenson, P.E.

Chief Traffic Analysis Engineer

DRMP, Inc.

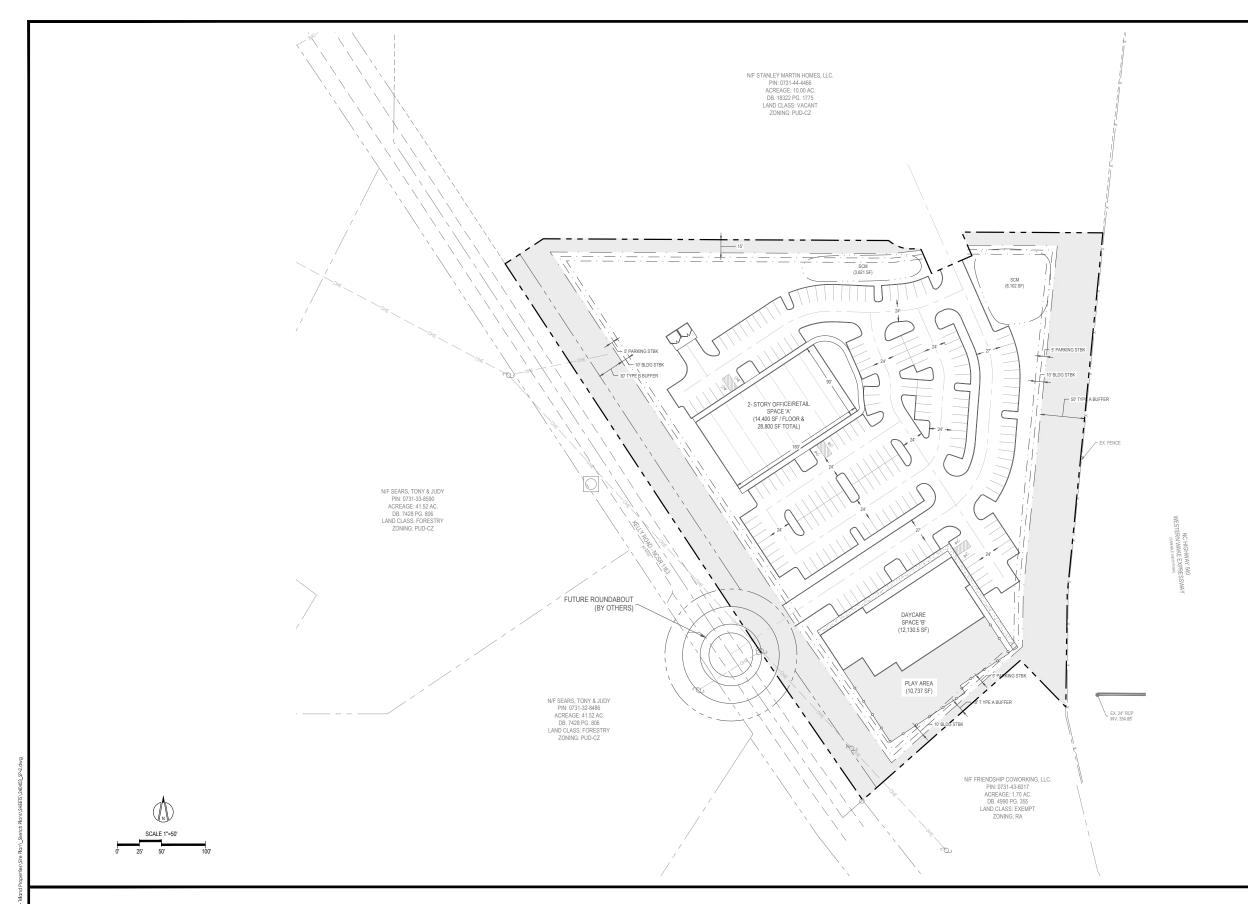
Attachments:

Site Plan

Site Trip Distribution Figure

NCHRP Internal Capture Reports

cc: Amanda Bunce, Town of Apex Planning Russell Dalton, PE, Town of Apex Rehab Hamad, MBA Jessica McClure, PE, Town of Apex Jeff Roach, PE, Peak Engineering



SITE DATA	
OWNER	FAHEY FAMILY FARM, LLC.
SITE ADDRESS	2517 KEILY ROAD
SI E ADDRESS	APEX NC 27502
PN	0731-43-45D4
REAL ID	454267
ACREAGE	6.19 AC.
DB. PG	DB 1/062 PG 1/16
2045 JAND USE MAP	PUD-02
EXISTING USE	SINGLE FAM
REZONING 67-19-2016	15CZ33
FROPOSED ZONING	PUD ŒZ
	•
TOWNSHP	WHITE OAK
SUB-WATERSHED	BEAVER CREEK
RIVERBASIN	CAFE FEAR
FEMA MAP	MAP#3720073100K DATE 07.18.22
	1
HISTORICAL	(1) BARN PER (VA 1052
DOMESTICAL	NOV NO. AGY OFF
ROWDEDICATION	XX AC. (XX SF)
NEW TRACT AREA	XX AC. (XX 9F)
DEVELOPMENT TYPE - (
PROPOSED USE	OFFICE / RETAIL & DAYCARE
DENSITY	50Q ODC SF
MIN. LOT AREA	5,000 SF
MIN.LO SIZE	5,000 SF
MIN LOT WIDTH	N/A
Building det	<u>ALS</u>
MAX. BUILDING HEIGHT	75 (5-STORY)
PROPOSED BUILDING HEIGHT	XX'
PROFOSED BUILDING STORIES	2 STORIES
	-
BUILDING SETE	A.C.KS
FRONT	5
SIDE	5
CORNER	s
REAR	5
	-
BUILD UPON A	RFA
MAX. BUILD UPON AREA	4.33° A.C. (70 %)
PROPOSED BULD UPON AREA	XX1AC. (XX %)
PARKING	
PARKING CALCS. (MIXED USE)	1SPACE / 250 SF
BUILDING SETOTAL	28.800 SF
PARKING REQUIRED	11B SPACES
PARKING PROVIDED	133 SPACES
	1000111000
PARKING CALCS (DAY CARE)	1 SPACE / SPERSON
PARKING CALCS, (DAY CARE) TOTAL CAPACITY	1 SPAGE / GPERSON 300
TOTAL CAPACITY	300
TOTAL CAPACITY PARKING REQUIRED	300 50 SPACES
TOTAL CAPACITY	300
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED	300 90 SPACES 30 SPACES
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING REQUIRED	300 50 SPACES 30 SPACES 166 SPACES
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED	300 90 SPACES 30 SPACES
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED	300 50 SPACES 30 SPACES 166 SPACES 193 SPACES
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING REQUIRED TOTAL PARKING PROVIDED ADA PARKING OALCS.	300 90 SPACES 30 SPACES 168 SPACES 193 SPACES 151 TO 200 = 6
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED	300 30 SPACES 30 SPACES 168 SPACES 193 SPACES 191 TO 200 = 6 6 SPACES
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROWIDED ADA PARKING ALCS. ADA PARKING GEOLUIED	300 90 SPACES 30 SPACES 168 SPACES 193 SPACES 151 TO 200 = 6
TOTAL CAPACITY PARKING REQUIRED PARKING REQUIRED TOTAL PARKING REQUIRED TOTAL PARKING PROVIDED ADA PARKING ALCS. ADA PARKING REQUIRED ADA PARKING PROVIDED	300 30 SPACES 30 SPACES 168 SPACES 193 SPACES 191 TO 200 = 6 6 SPACES
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING GALCS. ADA PARKING GALCS. ADA PARKING GALCS. ADA PARKING PROVIDED RCA	300 50 SPACES 30 SPACES 168 SPACES 193 SPACES 151 TO 200 = 6 6 SPACES 6 SPACES
TOTAL CAPACITY PARKING REQUIRED PARKING REQUIRED TOTAL PARKING REQUIRED TOTAL PARKING PROVIDED ADA PARKING ALCS. ADA PARKING REQUIRED ADA PARKING PROVIDED	300 30 SPACES 30 SPACES 168 SPACES 193 SPACES 191 TO 200 = 6 6 SPACES
TOTAL CAPACITY PARKING REQUIRED PARKING SEQUIRED TOTAL PARKING SEQUIRED TOTAL PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING GALGS, ADA PARKING REQUIRED ADA PARKING PROVIDED RGA RCA REQUIRED	300 30 SPACES 30 SPACES 168 SPACES 163 SPACES 151 TO 200 = 6 6 SPACES 6 SPACES
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING GALCS, ADA PARKING GALCS, ADA PARKING FROWDED RCA RCA PECUIRED RCA RCA PECUIRED	300 30 SPACES 30 SPACES 188 SPACES 193 SPACES 191 TO 200 = 6 6 SPACES 6 SPACES 1.55 AC (57,518 SF) - 25%
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING GALCS, ADA PARKING GALCS, ADA PARKING FEQUIRED ADA PARKING PROVIDED RCA RCA RECUIRED RCA RCA (PERIMETER SUFFERS) RCA (STRFAM FILIFFERS)	300 90 SPACES 30 SPACES 198 SPACES 198 SPACES 193 SPACES 191 SPACES 6 SPACES 6 SPACES 1.55 AC (57,518 SP) - 25% 1.55 AC (77,914 SP) - 27% 27 AC (XX SP)
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING GALCS, ADA PARKING GALCS, ADA PARKING FROWDED RCA RCA PECUIRED RCA RCA PECUIRED	300 30 SPACES 30 SPACES 188 SPACES 193 SPACES 191 TO 200 = 6 6 SPACES 6 SPACES 1.55 AC (57,518 SF) - 25%
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING GALCS. ADA PARKING GALCS. ADA PARKING PROVIDED RCA RCA RECUIRED RCA RCA (PERIMETER BUFFERS) RCA (SCIM)	300 30 SPACES 30 SPACES 108 SPACES 1.55 AC (57,518 SF) - 25% 1.55 AC (27,518 SF) - 25%
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING GALCS, ADA PARKING GALCS, ADA PARKING FEQUIRED ADA PARKING PROVIDED RCA RCA RECUIRED RCA RCA (PERIMETER SUFFERS) RCA (STRFAM FILIFFERS)	300 90 SPACES 30 SPACES 198 SPACES 198 SPACES 193 SPACES 191 SPACES 6 SPACES 6 SPACES 1.55 AC (57,518 SP) - 25% 1.55 AC (77,914 SP) - 27% 27 AC (XX SP)
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TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING GALOS, ADA PARKING GALOS, ADA PARKING GALOS, ADA PARKING FROUITED RCA RCA RECUIRED RCA RCA (PERIMETER BUFFERS) RCA (SCIM) RCA PROVIDED OPEN SPACE OPEN SPACE	300 90 SPACES 30 SPACES 198 SPACES 198 SPACES 193 SPACES 151 TO 200 = 6 6 G PACES 6 S PACES 1.55 AC (37,518 SP) - 25% 1.65 AC (71,914 SP) - 27% 1.75 AC (XX SP)
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING REQUIRED TOTAL PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING PROVIDED RCA RCA RECUIRED RCA (STREAM RIFFERS) RCA (STREAM RIFFERS) RCA PROVIDED OS REQUIRED OPEN SPAC OS REQUIRED	300 30 SPACES 30 SPACES 30 SPACES 168 SPACES 168 SPACES 163 SPACES 151 TO 200 = 0 6 3 SPACES 6 SPACES 1.55 AC (37,518 SF) - 25% 27 AC (27,518 SF) - 27% 27 AC (27,515 SF) 27 AC (27,515 SF)
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TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING GALCS, ADA PARKING GALCS, ADA PARKING PROVIDED RCA RCA RECUIRED RCA (PERIMETER SUFFERS) RCA (SCIM) RCA PROVIDED OS REQUIRED OS REQUIRED OS REQUIRED OS PROVIDED	300 30 SPACES 30 SPACES 188 SPACES 193 SPACES 193 SPACES 151 TO 200 = 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING GALGS. AND PARKING GALGS. AND PARKING FROUDIED RCA RCA PARKING PROVIDED RCA RCA RECUIRED RCA (STREAM FILIFERS) RCA (SCM) RCA PROVIDED OS REQUIRED OS PROMIDED OS PROMIDED	300 30 SPACES 30 SPACES 10 SPACES 151TO 200 = 0 0 SPACES 155AC (57,518 SF) - 25% 1.55 AC (57,518 SF) - 25% 1.55 AC (57,518 SF) - 27% 1.55 AC (57,518 SF) - 27% 1.55 AC (57,518 SF) - 25% 1.55 AC
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING GALGS, ADA PARKING GALGS, ADA PARKING GALGS, ADA PARKING FROUITED RCA RCA RECUIRED RCA RCA RECUIRED RCA (STREAM FLIFFERS) RCA (SCIM) RCA PROVIDED OS REQUIRED OS REQUIRED OS REQUIRED OS PROMIDED NET TRACT AREA	300 90 SPACES 30 SPACES 188 SPACES 198 SPACES 193 SPACES 151 TO 200 = 0 0 3 PACES 6 S PACES 1.55 AC (57,518 SP) - 25% 1.55 AC (71,914 SP) - 27% 1.55 AC (71,914 SP) - 27% 1.55 AC (200 SP)
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TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING GROUIFED ADA PARKING GALCS, ADA PARKING GALCS, ADA PARKING FROUDED RCA RCA PECUIRED RCA RCA PECUIRED RCA (STREAM FLIFFERS) RCA (STREAM FLIFFERS) RCA SEGUING OS REQUIRED OS REQUIRED OS PROVIDED IMPERVIOUS NET TRACT AREA EXISTING INITIALITY	300 30 SPACES 31 SPACES 188 SPACES 193 SPACES 193 SPACES 191 SPACES 151 TO 200 = 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
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TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING REQUIRED TOTAL PARKING PROVIDED ADA PARKING GALGS. AND PARKING FROUDED RCA RCA PARKING PROVIDED RCA RCA RECUIRED RCA (STRAW RIFFERS) RCA (SCM) RCA PROVIDED OS REQUIRED OS REQUIRED OS REQUIRED METTRACT AREA EXISTING IMPERMICUS BUILDINGS BUILDINGS	300 30 SPACES 30 SPACES 30 SPACES 168 SPACES 168 SPACES 151 TO 200 = 0 6 G SPACES 6 SPACES 151 TO 200 = 0 155 AC (57,518 SF) - 25% 1.55 AC (57,518 SF) - 25% 1.55 AC (27,514 SF) - 27% 1.55 AC (27,514 SF) - 27% 1.55 AC (27,515 SF) 1.55 AC (27,515 S
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING GALCS. ADA PARKING GALCS. ADA PARKING GALCS. ADA PARKING FROUDED RCA RCA RECUIRED RCA RCA (SERMETER BUFFERS) RCA (SCIM) RCA PROVIDED OS REQUIRED OS REQUIRED OS REQUIRED OS PROMDED NETTRACT AREA EXISTINS IMPERSIOUS LOT AREA MAX. BJILDIAMSX SIDEMALKS	300 30 SPACES 30 SPACES 30 SPACES 198 SPACES 198 SPACES 193 SPACES 151 TO 200 = 0 6 SPACES 6 SPACES 1.55 AC (37,518 SF) - 25% 1.55 AC (37,518 SF) -
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING PROVIDED ADA PARKING PROVIDED RCA RCA PARKING PROVIDED RCA RCA RECUIRED RCA (STREAU RIFFERS) RCA (STREAU RIFFERS) RCA (STREAU RIFFERS) RCA PROVIDED OS REQUIRED OS PROVIDED OS PROVIDED OS PROVIDED NET TRACT AREA EXISTING IMPERADOUS LOT AREA MAX. B JILDINGS SIDENAL-S RCADWAY B B	300 30 SPACES 30 SPACES 30 SPACES 188 SPACES 198 SPACES 193 SPACES 191 SPACES 193 SPACES 191 SPACES
TOTAL CAPACITY PARKING REQUIRED PARKING PROVIDED TOTAL PARKING PROVIDED TOTAL PARKING PROVIDED ADA PARKING GALCS. ADA PARKING GALCS. ADA PARKING GALCS. ADA PARKING FROUDED RCA RCA RECUIRED RCA RCA (SERMETER BUFFERS) RCA (SCIM) RCA PROVIDED OS REQUIRED OS REQUIRED OS REQUIRED OS PROMDED NETTRACT AREA EXISTINS IMPERSIOUS LOT AREA MAX. BJILDIAMSX SIDEMALKS	300 30 SPACES 30 SPACES 30 SPACES 188 SPACES 198 SPACES 193 SPACES 191 SPACES 193 SPACES 191 SPACES

CONCEPT PLANS

ALL INFORMATION AND LINE WORK RETRIEVED FROM WAKE COUNTY GIS MAP SERVICES. THIS IS INTENDED FOR CONCEPTUAL USE ONLY. LINE WORK IS APPROXIMATE AND SUBJECT TO CHANGE UPON OBTAINING SURVEY.

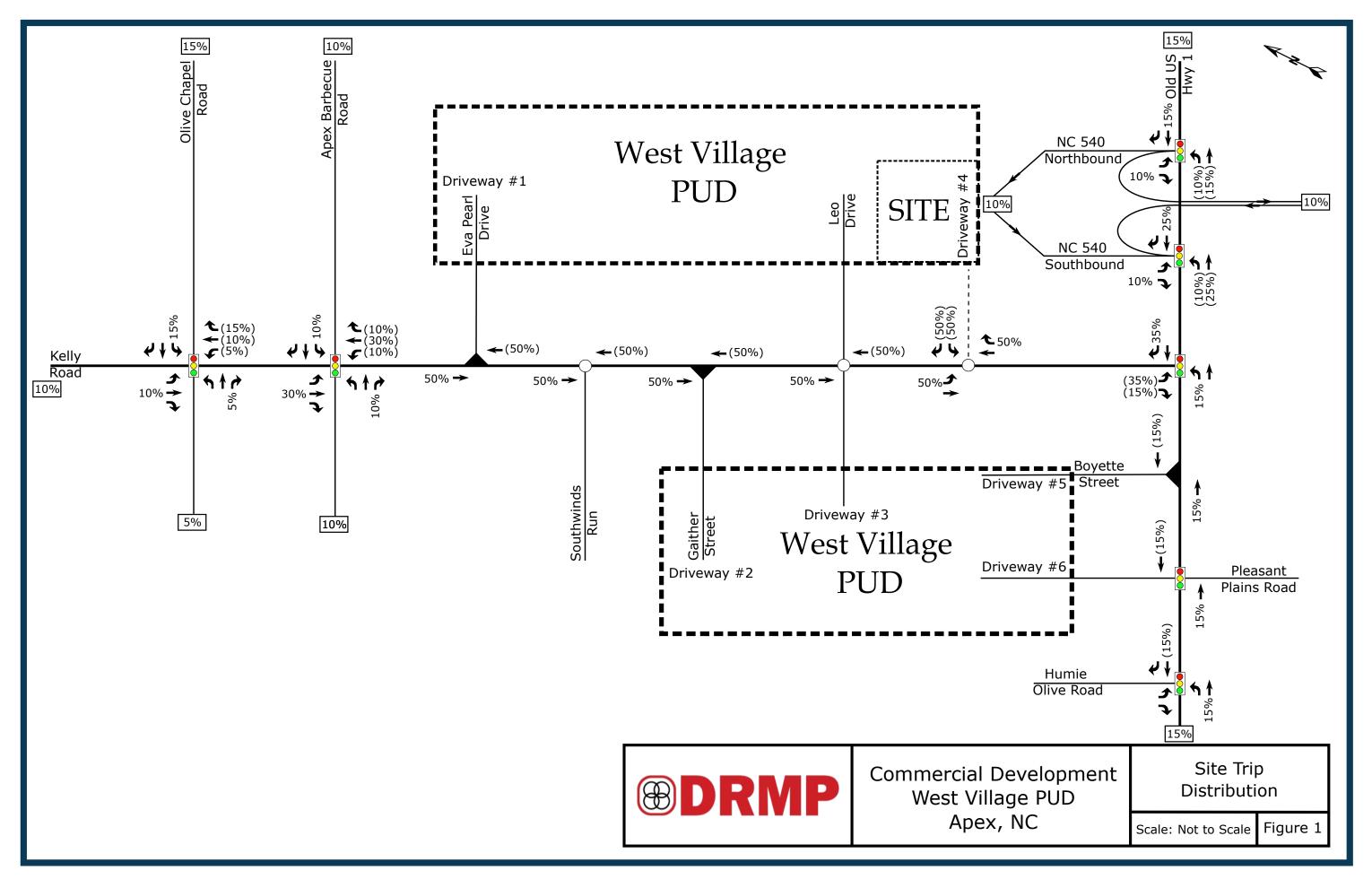
ALL SITE DATE NOTED IN RED IS TO BE DETERMINED (TBD).



KELLY ROAD COMMERCIAL 2517 KELLY ROAD

2517 KELLY ROAD APEX NC WAKE COUNTY

DRAWN BY:	CHECKED BY:	PROJECT NO.
MD	JR	240306
DATE		SHEET NO.
O	3.12.24	SP-2





Traffic Counts

	NCHRP 684 Internal Trip Capture Estimation Tool								
Project Name:	West Village Commercial Development		Organization:	DRMP					
Project Location:	Apex NC		Performed By:	RGS					
Scenario Description:			Date:	10/1/2024					
Analysis Year:	2028		Checked By:						
Analysis Period:	AM Street Peak Hour		Date:						

	Table 1	-A: Base Vehicl	e-Trip Generation	ı Es	timates (Single-Use Sit	e Estimate)	
Land Use	Developm	ent Data (For Inf	ormation Only)			Estimated Vehicle-Trips ³	
Land OSE	ITE LUCs1	Quantity	Units		Total	Entering	Exiting
Office	710	14	KSF		32	28	4
Retail	822	14	KSF		37	22	15
Restaurant					0	0	0
Cinema/Entertainment					0	0	0
Residential					0	0	0
Hotel					0	0	0
All Other Land Uses ²					0	0	0
					69	50	19

		Table 2-A:	Mode Split and Veh	nicl	e Occupancy Estimates			
Land Use		Entering Trip	os		Exiting Trips			
Land Ose	Veh. Occ.4	c.4 % Transit % Non-Motorized			Veh. Occ.4	% Transit	% Non-Motorized	
Office	1.10	0%	0%		1.10	0%	0%	
Retail	1.10	0%	0%		1.10	0%	0%	
Restaurant	1.10	0%	0%		1.10	0%	0%	
Cinema/Entertainment	1.10	0%	0%		1.10	0%	0%	
Residential	1.10	0%	0%		1.10	0%	0%	
Hotel	1.10	0%	0%		1.10	0%	0%	
All Other Land Uses ²	1.10	0%	0%		1.10	0%	0%	

	Table 3-A: Average Land Use Interchange Distances (Feet Walking Distance)									
Origin (From)		Destination (To)								
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel				
Office										
Retail										
Restaurant										
Cinema/Entertainment										
Residential										
Hotel										

Table 4-A: Internal Person-Trip Origin-Destination Matrix*								
Origin (Fram)	Destination (To)							
Origin (From)	Office	Office Retail Restaurant Cinema/Ent		Cinema/Entertainment	a/Entertainment Residential			
Office		1	0	0	0	0		
Retail	1		0	0	0	0		
Restaurant	0	0		0	0	0		
Cinema/Entertainment	0	0	0		0	0		
Residential	0	0	0	0		0		
Hotel	0	0	0	0	0			

Table 5-A	Table 5-A: Computations Summary								
Total Entering Exiting									
All Person-Trips	76	55	21						
Internal Capture Percentage	5%	4%	10%						
External Vehicle-Trips ⁵	66	48	18						
External Transit-Trips ⁶	0	0	0						
External Non-Motorized Trips ⁶	0	0	0						

Table 6-A: Internal Trip Capture Percentages by Land Use									
Land Use	Entering Trips	Exiting Trips							
Office	3%	25%							
Retail	4%	6%							
Restaurant	N/A	N/A							
Cinema/Entertainment	N/A	N/A							
Residential	N/A	N/A							
Hotel	N/A	N/A							

¹Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

⁴Enter vehicle occupancy assumed in Table 1-A vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made to Tables 5-A, 9-A (O and D). Enter transit, non-motorized percentages that will result with proposed mixed-use project complete.

⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-A.

⁶Person-Trips

*Indicates computation that has been rounded to the nearest whole number.

Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

	NCHRP 684 Internal Trip Capture Estimation Tool								
Project Name:	West Village Commercial Development		Organization:	DRMP					
Project Location:	Apex NC		Performed By:	RGS					
Scenario Description:			Date:	10/1/2024					
Analysis Year:	2028		Checked By:						
Analysis Period:	PM Street Peak Hour		Date:						

	Table 1	-P: Base Vehicl	e-Trip Generation	ı Es	timates (Single-Use Sit	e Estimate)	
Land Use	Developm	ent Data (For Inf	ormation Only)			Estimated Vehicle-Trips ³	
Land USE	ITE LUCs1	Quantity	Units		Total	Entering	Exiting
Office	710	14	KSF		33	6	27
Retail	822	14	KSF		101	50	51
Restaurant					0	0	0
Cinema/Entertainment					0	0	0
Residential					0	0	0
Hotel					0	0	0
All Other Land Uses ²					0	0	0
					134	56	78

	Table 2-P: Mode Split and Vehicle Occupancy Estimates									
Land Use		Entering Trip	os			Exiting Trips				
Land Ose	Veh. Occ.4	Veh. Occ. ⁴ % Transit % Non-Motorized			Veh. Occ.4	% Transit	% Non-Motorized			
Office	1.10	0%	0%		1.10	0%	0%			
Retail	1.10	0%	0%		1.10	0%	0%			
Restaurant	1.10	0%	0%		1.10	0%	0%			
Cinema/Entertainment	1.10	0%	0%		1.10	0%	0%			
Residential	1.10	0%	0%		1.10	0%	0%			
Hotel	1.10	0%	0%		1.10	0%	0%			
All Other Land Uses ²	1.10	0%	0%		1.10	0%	0%			

Table 3-P: Average Land Use Interchange Distances (Feet Walking Distance)														
Origin (From)		Destination (To)												
Oligili (Floili)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel								
Office		2000	2000		2000									
Retail					2000									
Restaurant					2000									
Cinema/Entertainment					2000									
Residential		2000	2000											
Hotel					2000									

		Table 4-P: Ir	nternal Person-Trip	Origin-Destination Matrix	*				
Destination (To)									
Origin (From)	Office	Retail	Restaurant	Cinema/Entertainment	Residential	Hotel			
Office		1	0	0	0	0			
Retail	1		0	0 0		0			
Restaurant	0	0		0	0	0			
Cinema/Entertainment	0	0	0		0	0			
Residential	0	0	0	0		0			
Hotel	0	0	0	0	0				

Table 5-P: Computations Summary										
Total Entering Exiting										
All Person-Trips	148	62	86							
Internal Capture Percentage	3%	3%	2%							
External Vehicle-Trips ⁵	130	54	76							
External Transit-Trips ⁶	0	0	0							
External Non-Motorized Trips ⁶	0	0	0							

Table 6-P: Internal Trip Capture Percentages by Land Use								
Land Use	Entering Trips	Exiting Trips						
Office	14%	3%						
Retail	2%	2%						
Restaurant	N/A	N/A						
Cinema/Entertainment	N/A	N/A						
Residential	N/A	N/A						
Hotel	N/A	N/A						

¹Land Use Codes (LUCs) from *Trip Generation Manual*, published by the Institute of Transportation Engineers.

²Total estimate for all other land uses at mixed-use development site is not subject to internal trip capture computations in this estimator.

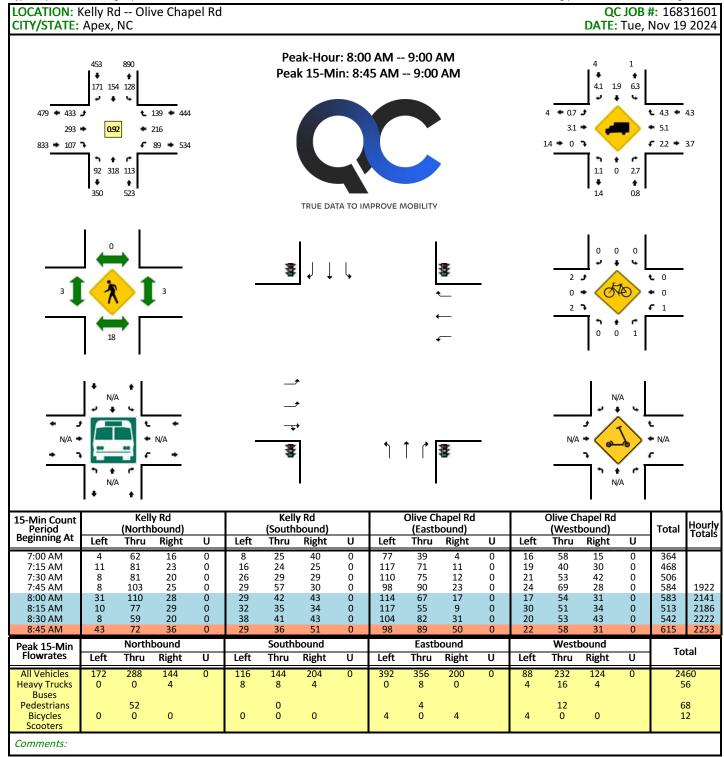
³Enter trips assuming no transit or non-motorized trips (as assumed in ITE *Trip Generation Manual*).

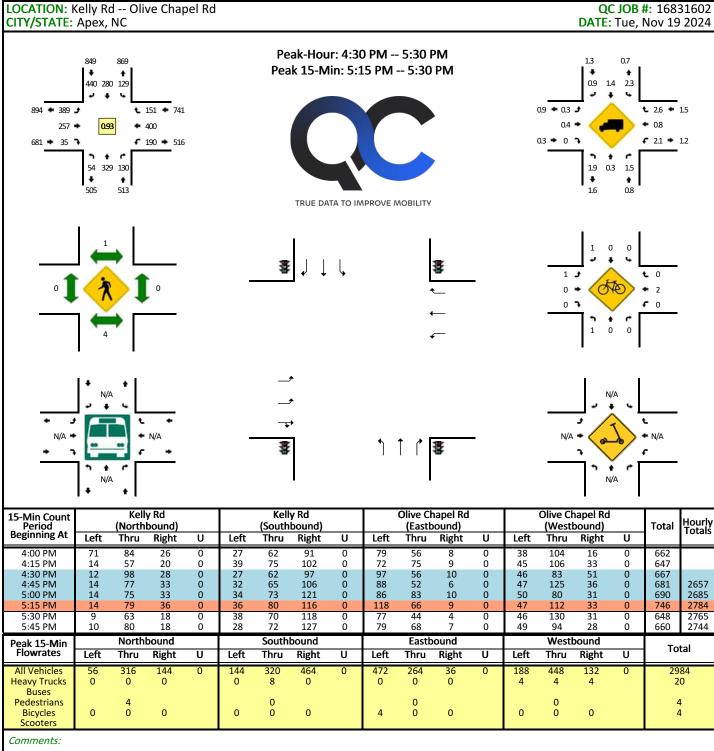
⁴Enter vehicle occupancy assumed in Table 1-P vehicle trips. If vehicle occupancy changes for proposed mixed-use project, manual adjustments must be made ⁵Vehicle-trips computed using the mode split and vehicle occupancy values provided in Table 2-P.

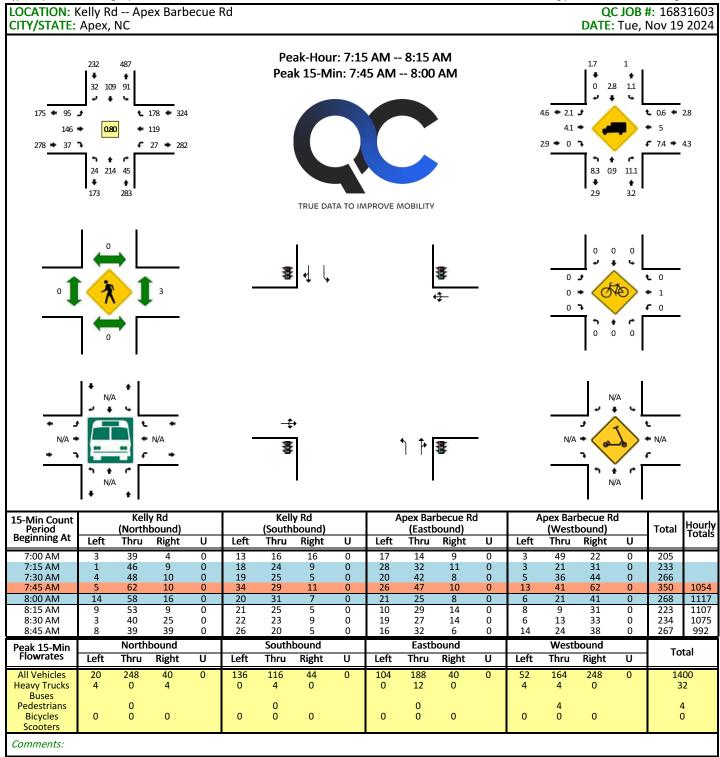
Person-Trips

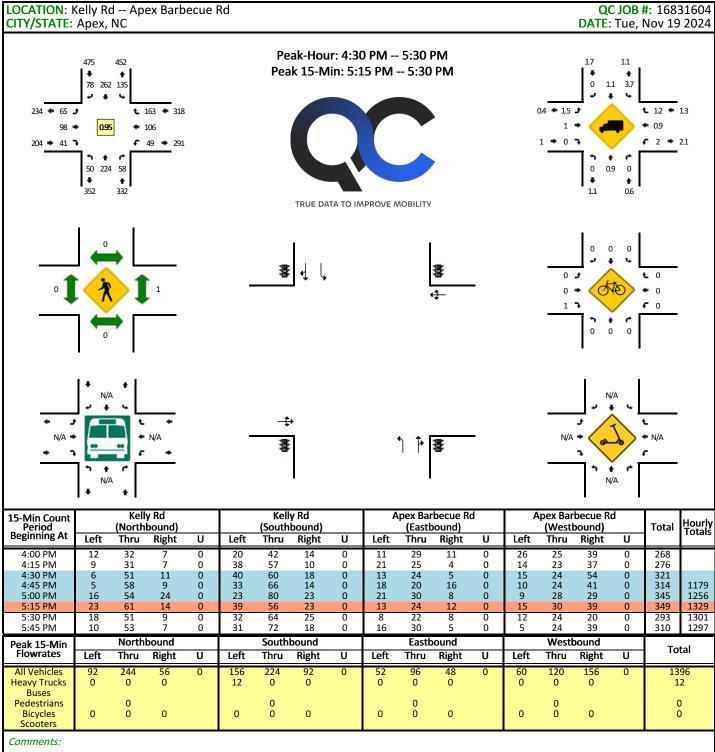
Indicates computation that has been rounded to the nearest whole number.

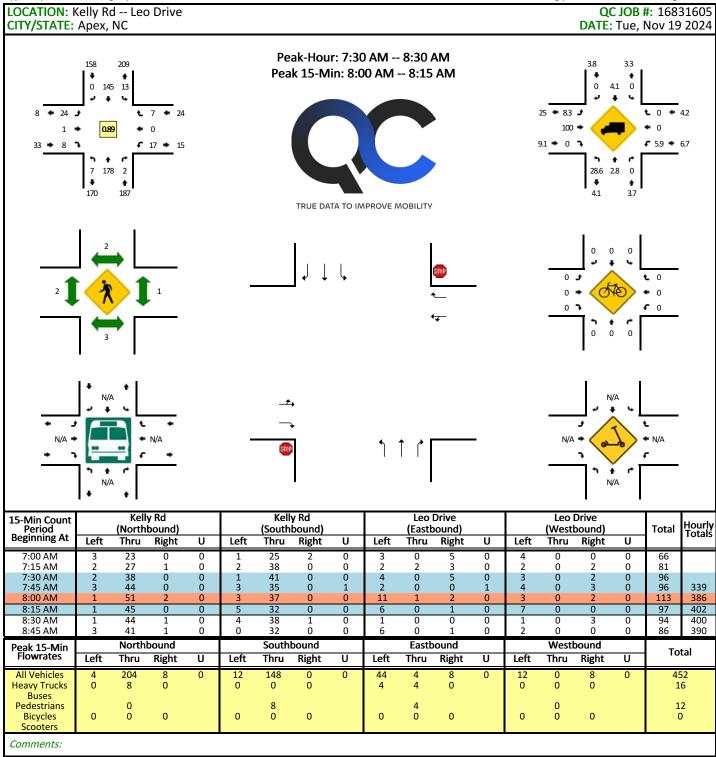
Estimation Tool Developed by the Texas A&M Transportation Institute - Version 2013.1

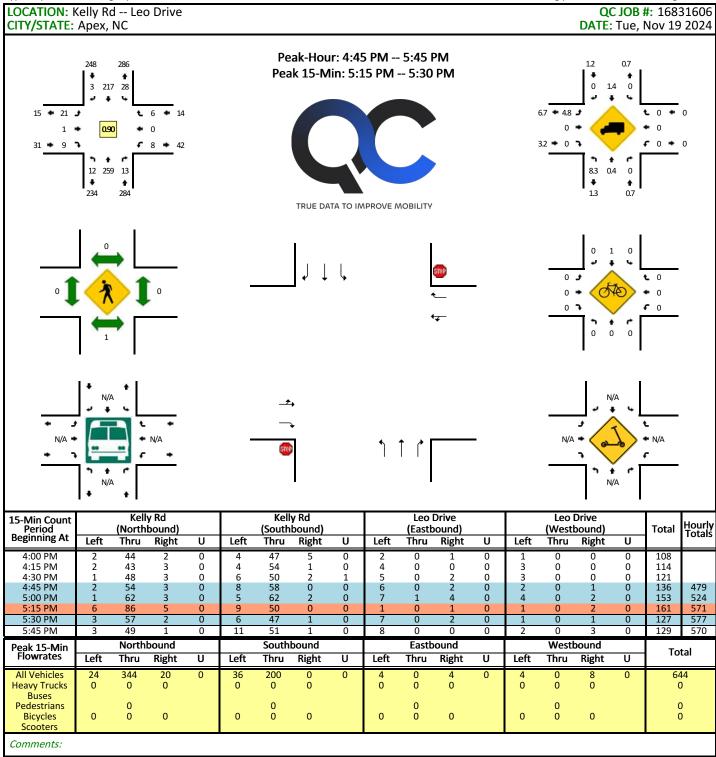


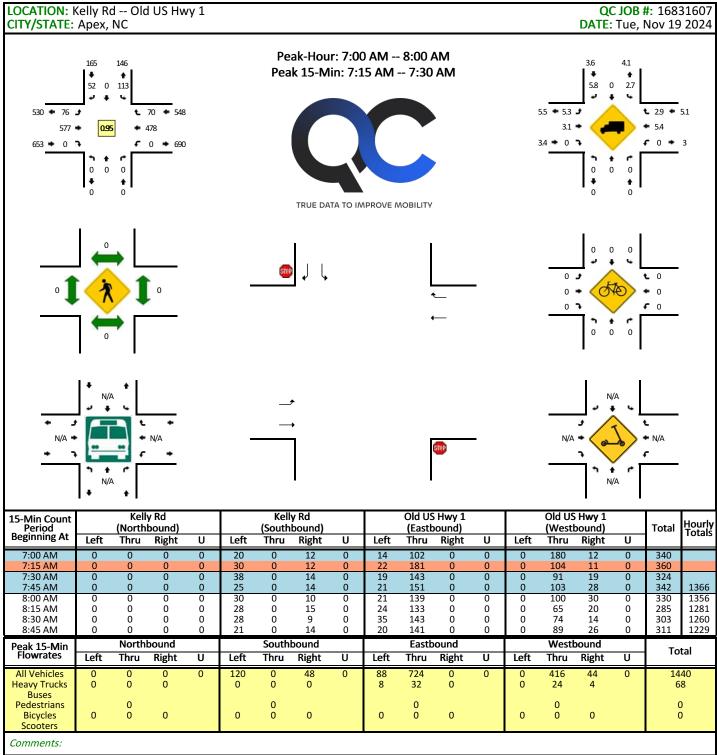












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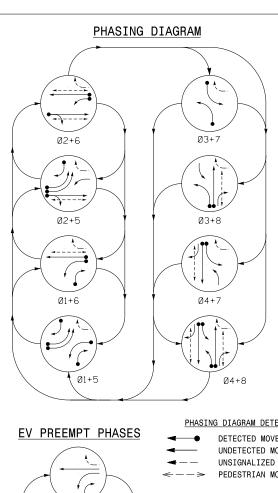
5:00 PM 5:15 PM 5:30 PM 0

0

Intersectio Kelly Rd Old US Hwy 1 City/State: Apex NC Lane Configuration: SBLane1 SBLane2 SBLane3 SBLane4 SBLane5 SBLane6 SBLane7 QCJobNo: 16831608 QCJobno. ClientID: WBLane1 EBLane6 EBLane5 WBLane2 WBLane3 Comments: Latitude/Lc 35.70681 -78.8824 EBLane4 WBLane4 PEAK HOU 4:45 PM PEAK HOU 5:45 PM EBLane3 EBLane2 WBLane5 PEAK 15-M 5:15 PM EBLane1 T WBLane7 PEAK 15-M 5:30 PM NBLane7 NBLane6 NBLane5 NBLane4 NBLane3 NBLane2 NBLane1 STOP PEAK-HOUR VOLUMES NBLeft NBThru NBRight SBLeft SBThru SBRight EBLeft E
0 0 0 101 0 137 111 EBThru EBRight WBLeft WBThru WBRight NBEnterin; SBEntering EBEntering WBEnterin; NBLeaving SBLeaving EBLeaving WBLeaving
11 489 0 0 610 175 0 238 600 785 286 0 590 747 PERCENT HEAVY VEHICLES NBLeft NBThru NBRight SBLeft SBThru SBRight EBLeft EBThru EBRight WBLeft WBThru WBRight NBEntering SBEntering EBEntering WBLeaving SBLeaving SBLeaving SBLeaving WBLeaving FDLeaving WBLeaving FDLeaving WBLeaving FDLeaving WBLeaving FDLeaving WBLeaving FDLeaving FDLea HEAVY VEH PEAK-HOUR VOLUMES - PEDESTRIANS Leg/CrossvSouth North 0 0 PEAK-HOUR VOLUMES - MICROMOBILITY
NBLeft NBThru NBRight SBLeft SBThru SBRight EBLeft EBThru EBRIght WBLeft WBThru WBRight Bicycles 0 0 0 0 0 0 0 0 0 PEAK 15-MIN FLOWRATES VehicleTyp NBLeft NBThru NBRight NBU-Turn NBRTOR SBLeft SBThru SBRight SBU-Turn SBRTOR EBLeft EBThru EBRight EBU-Turn EBRTOR WBLeft WBThru WBRight WBU-Turn WBRTOR Total All Vehicle: 0 0 0 0 644 264 0 0 Heavy Truc 24 Buses Pedestrians 0 0 0 0 0 0 Bicycles Scooters ALL-VEHICLE VOLUMES NB Right NB U-Turn NB RTOR SB Left
0 0
0 0 Time Perio NB Left 4:00 PM SB Right SB U-Turn SB RTOR EB Left 0 32 0 21 EB Thru 1 118 WB Thru WB Right WB U-Turn WB RTOR Total SB Thru EB Right EB U-Turn EB RTOR WB Left Hourly Totals 4:15 PM 24 32 24 120 0 116 26 342 4:30 PM 4:45 PM 35 27 98 112 120 160 322 391 16 34 36 14 17 20 24 28 32 27 33 34 37 66 38 35 44 37 29 37 133 114 130 5:00 PM 0 0 141 419 1474 161 148 424 389 1556 1623 5:15 PM 5:30 PM 5:45 PM 377 1609 HEAVY-VEHICLE VOLUMES Time Perio NB Left NB Thru NB Right SB Left SB Thru SB Right EB Left EB Thru EB Right WB Left 4:15 PM 11 4:30 PM 11 5:00 PM 5:15 PM 0 5:45 PM NB Thru NB Right SB Left SB Thru SB Right EB Left EB Thru EB Right WB Left WB Thru WB Right Total Time Perio NB Left 4:00 PM 4:30 PM 4:45 PM 5:00 PM 5:15 PM 5:30 PM 5:45 PM PEDESTRIAN VOLUMES Time PeriorSouth Leg North Leg West Leg East Leg Total 4:00 PM 0 0 0 0 4:15 PM 4:30 PM 5:00 PM 5:15 PM 5:45 PM BICYCLE VOLUMES Time Perio NB Left NB Thru NB Right SB Left 4:00 PM 0 0 0 0 SB Thru SB Right EB Left EB Thru EB Right WB Left WB Thru WB Right Total 4:15 PM 4:30 PM



SIGNAL PLANS



OASIS 2070 E	V PRE	EMPT
FUNCTION	EVP 3	EVP 5
Interval 1 – Dwell Green	255	255
Interval 1 — Dwell Yellow	0.0*	0.0*
Interval 1 — Dwell Red	0.0*	0.0*
Interval 5 — Exit Green	1	1
Interval 5 - Yellow	0.0	0.0
Interval 5 — Red	0.0	0.0
Exit Phase(s)	2+6	4+8
Priority	MEDIUM	MEDIUM
Delay Time	0.0	0.0
Min Green Before Pre	1	1
Ped Clear Before Pre	10	10
Yellow Clear Before Pre	0.0*	0.0*
Red Clear Before Pre	0.0*	0.0*
Dwell Min Time	7	7
Enable Backup Protection	N	N
Ped Clear Through Yellow	Y	Y
Omit Overlaps	-	-
Preempt Extend**	2	2

*				time	used	for	phase	during	
••	Progr	al ope	ration Timing	on	Optice	ıl De	tection	Unit	

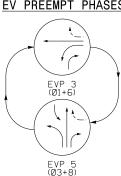
T	AΒ	LE	01	= ()PE	RA	TI	NC			
	PHASE										
SIGNAL FACE	Ø 1 + 5	Ø 1 + 6	Ø2+5	Ø 2 + 6	Ø 3 + 7	Ø 3 + 8	Ø 4 + 7	Ø 4 + 8	E V P	EVP 5	FLASH
11	-	-	- F	÷	₹R	₹R	₹R	- ₽	-	₹R	-¥
21, 22	R	R	G	G	R	R	R	R	R	R	Υ
31	₹R	-R	-R	-R	-	-	Ŧ	Ŧ	-R	-	-R
41	R	R	R	R	R	R	G	G	R	R	R
42	<u>R</u>	R	<u>R/</u>	R	R	R	G	G	R	R	R
51, 52	-	-R	-	-R	-R	-R	-R	- ₽	-R	-R	→R
61, 62	R	G	R	G	R	R	R	R	G	R	Υ
7:1	-R	-R	-R	₹	-	÷	-	÷	₹	÷	-R
81	R	R	R	R	R	G	R	G	R	G	R
82	P Z	<u>PZ</u>	R	R	R	G	R	G	<u>R</u>	G	R
P21, P22	DW	D₩	W	W	DW	DW	D₩	D₩	D _W	D₩	DRK
P41, P42	D₩	D₩	D₩	D₩	D₩	D₩	W	W	D₩	D₩	DRK
P61, P62	D₩	W	D₩	W	D₩	D₩	D₩	D₩	D₩	D₩	DRK
P81, P82	D₩	D₩	D₩	D₩	D₩	W	DW	W	D₩	DW	DRK

I	NDUCTI	VE LOC)PS		DETI	ECT	OR	PΙ	ROGRAM	MMING		
LOOP	SIZE (FT)	DISTANCE FROM STOPBAR (FT)	TURNS	NEW LOOP	PHASE	CALLING	EXTENSION	FULL TIME DELAY	STRETCH TIME	DELAY TIME	SYSTEM LOOP	0010
1.4	CV40	0	2 4 2	Y	1	Υ	Υ	-	-	15	-	
1 A	6X40	0	2-4-2	ī	6	Υ	Υ	Υ	-	3	-	
1B	6X40	0	2-4-2	Υ	1	Υ	Υ	-	-	15	-	,
2A	6X6	300	5	Υ	2	Υ	Υ	-	-	-	-	
7.	CV 40	_	0.4.0	.,	3	Υ	Υ	-	-	15	-	
3A	6X40	0	2-4-2	Υ	8	Υ	Υ	-	-	3	-	
4A	6X40	0	2-4-2	Υ	4	Υ	Υ	-	-	-	-	
5A	6X40	0	2-4-2	Υ	5	Υ	Υ	-	-	3	-	-
5B	6X40	0	2-4-2	Υ	5	Υ	Υ	-	-	-	-	,
5C	6X40	0	2-4-2	Υ	5	Υ	Υ	-	-	15	-	,
6A	6X6	300	5	Υ	6	Υ	Υ	-	-	-	-	
7.	61/40		0.4.0	.,	7	Υ	Υ	-	-	15	-	
7A	6X40	0	2-4-2	Υ	4	Υ	Υ	-	-	3	-	
8.8	6X40	0	2-4-2	Υ	8	Υ	Υ	-	-	-	-	

8 Phase Fully Actuated (Isolated)

- 1. Refer to "Roadway Standard Drawings NCDDT" dated January 2012 and "Standard Specifications for Roads and Structures" dated January 2012 and all applicable sections of the latest version of the generic Project Special Provisions. The PSP can be accessed at the following website:

 http://connect.ncdot.gov/resources/safety/Pages/ITSDesign-Resources.aspx
- 2. Do not program signal for late night flashing operation unless otherwise directed by the Engineer.
- 3. Phase 1 and/or phase 5 may be lagged.
- 4. Phase 3 and/or phase 7 may be lagged.
- 5. Reposition all existing signal heads as shown.
- 6. Set all detector units to presence mode.
- Refer to the roadway design provided by Town of Apex for pavement markings.
- 8. Omit "WALK" and flashing "DON'T WALK" with no pedestrian calls.
- 9. Program pedestrian heads to countdown the flashing "Don't Walk" time only.
- 10. Pedestrian pedestals are conceptual and shown for reference only. See sheets P1-P3 for pushbutton location details.
- 11. This intersection features an optical preemption system. Shown location of optical detector are conceptual only.
- 12. Optical detector 10 calls EVP 3. Optical detector 20 calls EVP 5.



FEATURE Min Green 1 *

Extension 1 *

Max Green 1 *

Red Clearan

Red Revert Walk 1 * Don't Walk 1

Walk Advance Time

Seconds Per Actuation

Max Variable Initial*

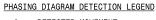
Time Before Reduction

Time To Reduce

Vehicle Call Memory

Recall Mode

Dual Entry



DETECTED MOVEMENT UNDETECTED MOVEMENT (OVERLAP) UNSIGNALIZED MOVEMENT PEDESTRIAN MOVEMENT

OASIS 2070 TIMING CHART

2.0

25

4.3

1.4

2.0

12

ON

2.0

15

3.0

2.6

2.0

12

6.0

90

4.7

1.8

2.0

19

2.5

34

15

30

3.0

MIN RECALL

YELLOW

ON

2.0

15

3.0

3.3

2.0

PHASE

2.0

15

3.0

3.3

2.0

ON

6.0

90

1.8

2.0

20

2.5

34

15

30

3.0

MIN RECALL

YELLOW

ON



SR 1160 (Olive Chapel Road)

+2% Grade

2.0

25

4.3

1.4

2.0

11

5

ON

45 MPH

2.0

15

3.0

2.3

2.0

ON



SIGNAL FACE I.D.

All Heads L.E.D.

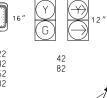


STDEWALK

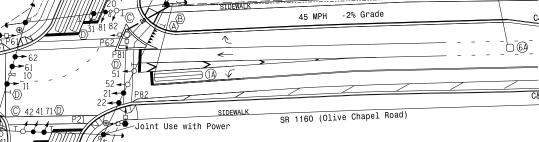
Accessible

Pedestrian Signal

SIDEWALK



DW - Don't Walk DRK - Dark



			ACCESSIBLE PEDES	TRIAN SIGNAL OPERATION
SIGNAL FACE	VOICE	TONES	INTERVAL	SPEECH MESSAGE
P21	_	х	Walk	(Rapid Ticks)
F21	_	^	Flashing Don't Walk/Don't Walk	-
P22		х	Walk	(Rapid Ticks)
P22 - /		^	Flashing Don't Walk/Don't Walk	-
P41	_	х	Walk	(Rapid Ticks)
P41	P41 -		Flashing Don't Walk/Don't Walk	-
P42	_	х	Walk	(Rapid Ticks)
P42	-	^	Flashing Don't Walk/Don't Walk	-
P61	_	х	Walk	(Rapid Ticks)
1 01		<	Flashing Don't Walk/Don't Walk	-
P62	x	-	Walk	Kelly. Walk sign is on to cross Kelly.
F 62	_		Flashing Don't Walk/Don't Walk	Wait. Wait to cross Kelly.
P81	l _x	_	Walk	Olive Chapel. Walk sign is on to cross Olive Chapel.
1 01	l^		Flashing Don't Walk/Don't Walk	Wait. Wait to cross Olive Chapel.
P82	_	x	Walk	(Rapid Ticks)
F 02		^	Flashing Don't Walk/Don't Walk	-

PROPOSED	!	<u>EXISTING</u>
\bigcirc	Traffic Signal Head	•
0->	Modified Signal Head	N/A
	Sign	-
†	Pedestrian Signal Head With Push Button & Sign	+
<u>~</u>	Signal Pole with Guy	•
$\cup \bigcup$	Signal Pole with Sidewalk Guy	
	Inductive Loop Detector	
\boxtimes	Controller & Cabinet	[×]
	Junction Box	
	2-in Underground Conduit -	
	- Right of Way -	
\longrightarrow	Directional Arrow	\longrightarrow
\bigcirc	Type II Signal Pedestal	•
⊗	Type I Pushbutton Post	€
	Curb Ramp	
∞	Optical Detector	•
$\langle \! A \! \rangle$	"YIELD" Sign (R1-2)	A
⟨B⟩	Ped Crossing Sign (W11-2)	₿
(C)	Right Arrow "ONLY" Sign (R3-5R	

Street Name Sign (D3-1)

LEGEND

PROJECT REFERENCE NO.

U-5518 AF

SHEET NO.

Sig 2 0



Signal Upgrade-Final Design

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



SR 1160 (Olive Chapel Road) at SR 1163 (Kelly Road)

Division 5 Wake County PLAN DATE: Jan. 2017 REVIEWED BY: J.L. Lewis VHB PROJECT NO.: 38545.00

CAR 033108

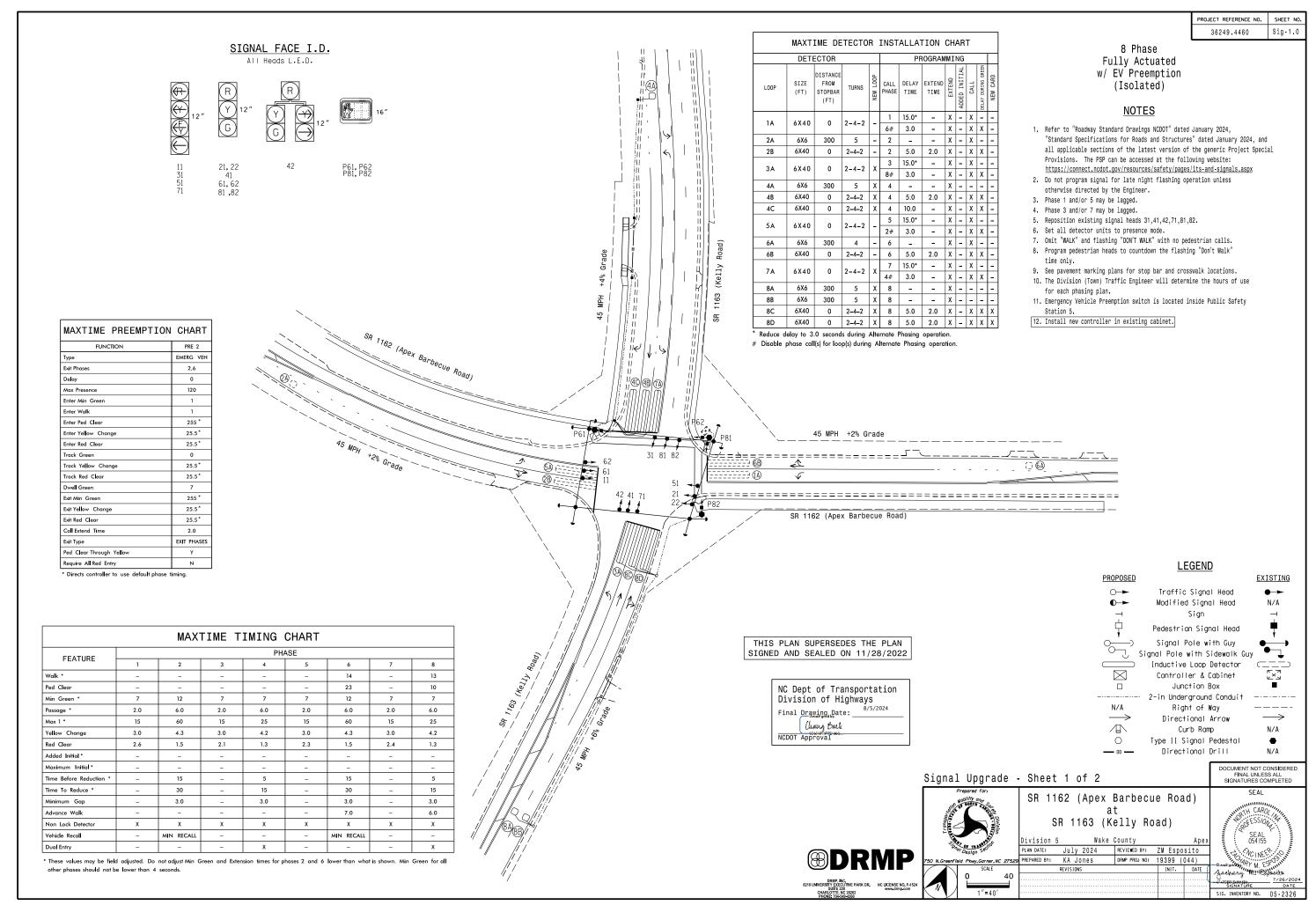
SIG. INVENTORY NO. 05-2214

ON * These values may be field adjusted. Do not adjust Min Green and Extension times for phases 2 and 6 lower than what is shown Min Green for all other phases should not be lower than 4 seconds.

NC Dept of Transportation Division of Highways Final Drawing Date: ______1/18/2017

ITS & Signal's Unit

Pkwy.Garner.NC 27529 PREPARED BY: J. Ma SCALE REVISIONS INIT. DATE



PROJECT REFERENCE NO. SHEET NO. 36249.4460 Sig 1 1

8 Phase Fully Actuated w/ EV Preemption (Isolated)

<u>NOTES</u>

- 1. Refer to "Roadway Standard Drawings NCDOT" dated January 2024, "Standard Specifications for Roads and Structures" dated January 2024, and all applicable sections of the latest version of the generic Project Special Provisions. The PSP can be accessed at the following website: https://connect.ncdot.gov/resources/safety/pages/its-and-signals.aspx
- 2. Do not program signal for late night flashing operation unless otherwise directed by the Engineer.
- 3. Phase 1 and/or 5 may be lagged.
- 4. Phase 3 and/or 7 may be lagged.
- 5. Reposition existing signal heads 31,41,42,71,81,82.
- 6. Set all detector units to presence mode.
- 7. Omit "WALK" and flashing "DON'T WALK" with no pedestrian calls.
- 8. Program pedestrian heads to countdown the flashing "Don't Walk" time only
- 9. See pavement marking plans for stop bar and crosswalk locations.
- 10. The Division (Town) Traffic Engineer will determine the hours of use
- for each phasing plan.

 11. Emergency Vehicle Preemption switch is located inside Public Safety Station 5.

12. Install new controller in existing cabinet.

EV PREEMPT DEFAULT PHASE (Medium Priority)



1			ILT OF					٧		
					PHA	SE				
SIGNAL FACE	Ø 1 + 5	Ø 1 + 6	Ø2+5	Ø2+6	Ø 3 + 7	Ø 3 + 8	Ø 4 + 7	Ø 4 + 8	E V P 2	FLASH
11	+	-	Ŧ	₹	-R	-R	-R	-R	÷	-R
21,22	R	R	G	G	R	R	R	R	G	R
31	#	₹R	-R	- R	-	•	- F	₹	-R	₹
41	R	R	R	R	R	R	G	G	R	R
42	<u>RZ</u>	R	RZ.	R	R	R	G	G	RZ.	R
51	-	Ŧ	-	÷	-R	-R	∗R	∗R	-	-R
61,62	R	G	R	G	R	R	R	R	R	R
71	#	₹R	-R	- R	+	Ŧ	-	₹	-R	₹
81,82	R	R	R	R	R	G	R	G	R	R
P61,P62	DW	W	DW	W	DW	DW	DW	DW	DW	DRK
P81,P82	DW	DW	DW	DW	DW	W	DW	W	DW	DRK

PHASING DIAGRAM DETECTION LEGEND

DEFAULT PHASING DIAGRAM

02+6

02+5

01+6

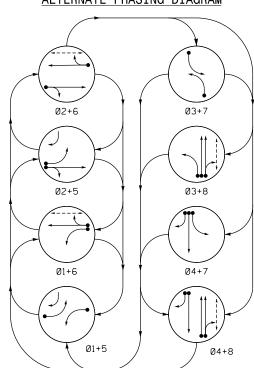
Ø3+7

03+8

04+7

- DETECTED MOVEMENT
- UNDETECTED MOVEMENT (OVERLAP)
- UNSIGNALIZED MOVEMENT
- \leftarrow --> PEDESTRIAN MOVEMENT

ALTERNATE PHASING DIAGRAM



PHASING DIAGRAM DETECTION LEGEND

UNSIGNALIZED MOVEMENT

UNDETECTED MOVEMENT (OVERLAP)

■ DETECTED MOVEMENT

 \leftarrow --> PEDESTRIAN MOVEMENT

EV PREEMPT ALTERNATE PHASE (Medium Priority)



TABLE OF OPERATION												
					PHA	SE						
SIGNAL FACE	Ø 1 + 5	Ø 1 + 6	Ø2+5	Ø2+6	Ø 3 + 7	Ø 3 + 8	Ø 4 + 7	Ø 4 + 8	E V P 2	FLASH		
11	+	-	-R	-R	₩	₩	-R	-R	-R	-R		
21,22	R	R	G	G	R	R	R	R	G	R		
31	- R	₹R	-R	-R	-	-	₹R	-R	-R	- ₽		
41	R	R	R	R	R	R	G	G	R	R		
42	RZ	R	<u>R</u>	R	R	R	G	G	<u>R</u> Z	R		
51	-	- ₽	-	-R	-R	₹	- ₽	∗R	-	 }		
61,62	R	G	R	G	R	R	R	R	R	R		
71	- R	₹R	∗R	-R	-	-R	-	-R	-R	-R		
81,82	R	R	R	R	R	G	R	G	R	R		
P61,P62	DW	W	DW	W	DW	DW	DW	DW	D₩	DRK		
P81,P82	DW	DW	DW	DW	DW	W	DW	W	DW	DRK		

ALTERNATE PHASING

THIS PLAN SUPERSEDES THE PLAN SIGNED AND SEALED ON 11/28/2022

> NC Dept of Transportation Division of Highways Final Drawing Date: ____8/5/2024 Chang Back

NCDOT Approval

Signal Upgrade - Sheet 2 of 2 SR 1163 (Kelly Road)

SIGNAL FACE I.D.

All Heads L.E.D.

P61, P62 P81, P82

<u></u>

21, 22 41

61, 62 81, 82

SR 1162 (Apex Barbecue Road) SEAL 054 155 Wake County Division 5 PLAN DATE: July 2024 REVIEWED BY: ZM Esposito D N. Greenfield Pkwy, Garner, NC 27529 PREPARED BY: KA Jones DRMP PROJ NO: 19399 (044)

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



ADJACENT DEVELOPMENT INFORMATION

TRAFFIC IMPACT ANALYSIS UPDATE

FOR

FRIENDSHIP STATION

LOCATED

IN

APEX, NORTH CAROLINA

Prepared For:
Parkside Development Group, LLC
PO Box 1438
Apex, NC 27502

Prepared By: Ramey Kemp & Associates, Inc. 5808 Faringdon Place, Suite 100 Raleigh, NC 27609 License #C-0910

March 2017

Prepared By: <u>DBL</u>

Reviewed By: TAA

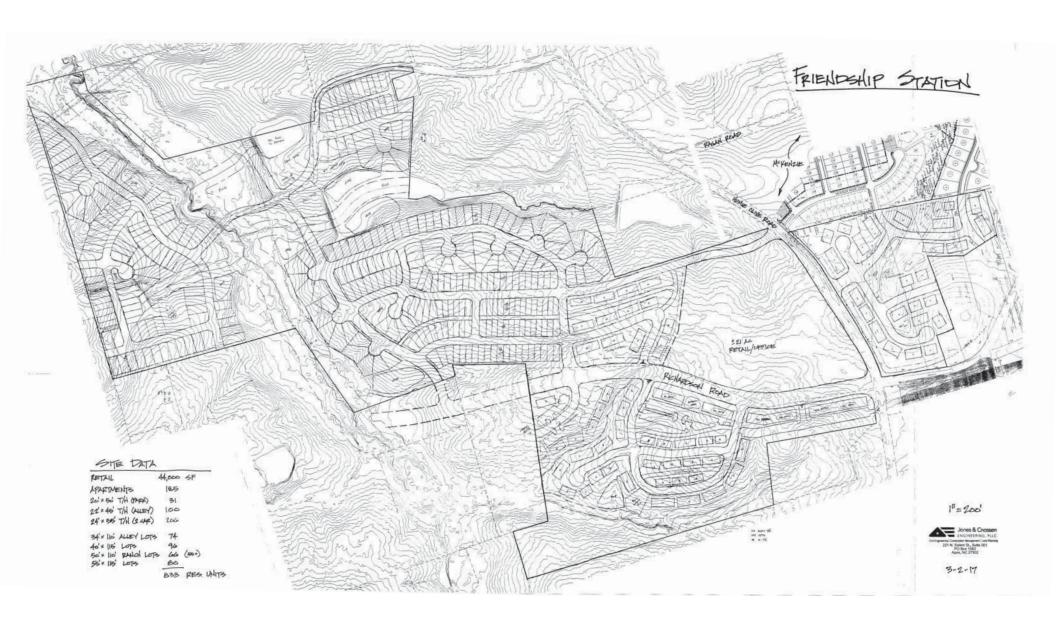


Table 2: Trip Generation Summary – Phase 2

		Land Use (ITE Code)	Intensity	Daily Traffic	AM Pea Trips		PM Peak Hour Trips (vph)		
		(TE Code)		(vpd)	Enter	Exit	Enter	Exit	
ASS	UME	Single Family Detached Housing (210)	316 Units	3,010	59	178	199	117	
	STLY	Townhomes (230)	238 Units	1,390	18	87	84	40	
		Townhomes (230) ¹	99 Units	580	7	37	34	17	
		Apartments (220)	185 Units	1,250	19	75	77	42	
		Shopping Center (820)	44,000 sq. ft.	3,990	58	36	166	180	
		Shopping Center (820)	100,000 sq. ft.	6,800	97	59	288	311	
		General Office Building (710)	68,000 sq. ft.	980	124	17	26	129	
		Total Trips		18,000	382	489	874	836	
		Internal Capture (8% Daily, 8% I	PM)	770			42	29	
		Total External Trips		17,230	382	489	832	807	
		Shopping Center Pass-By Trip	s (34% PM)		-1-		-156	-156	
		Total Primary Trips		17,230	382	489	676	651	

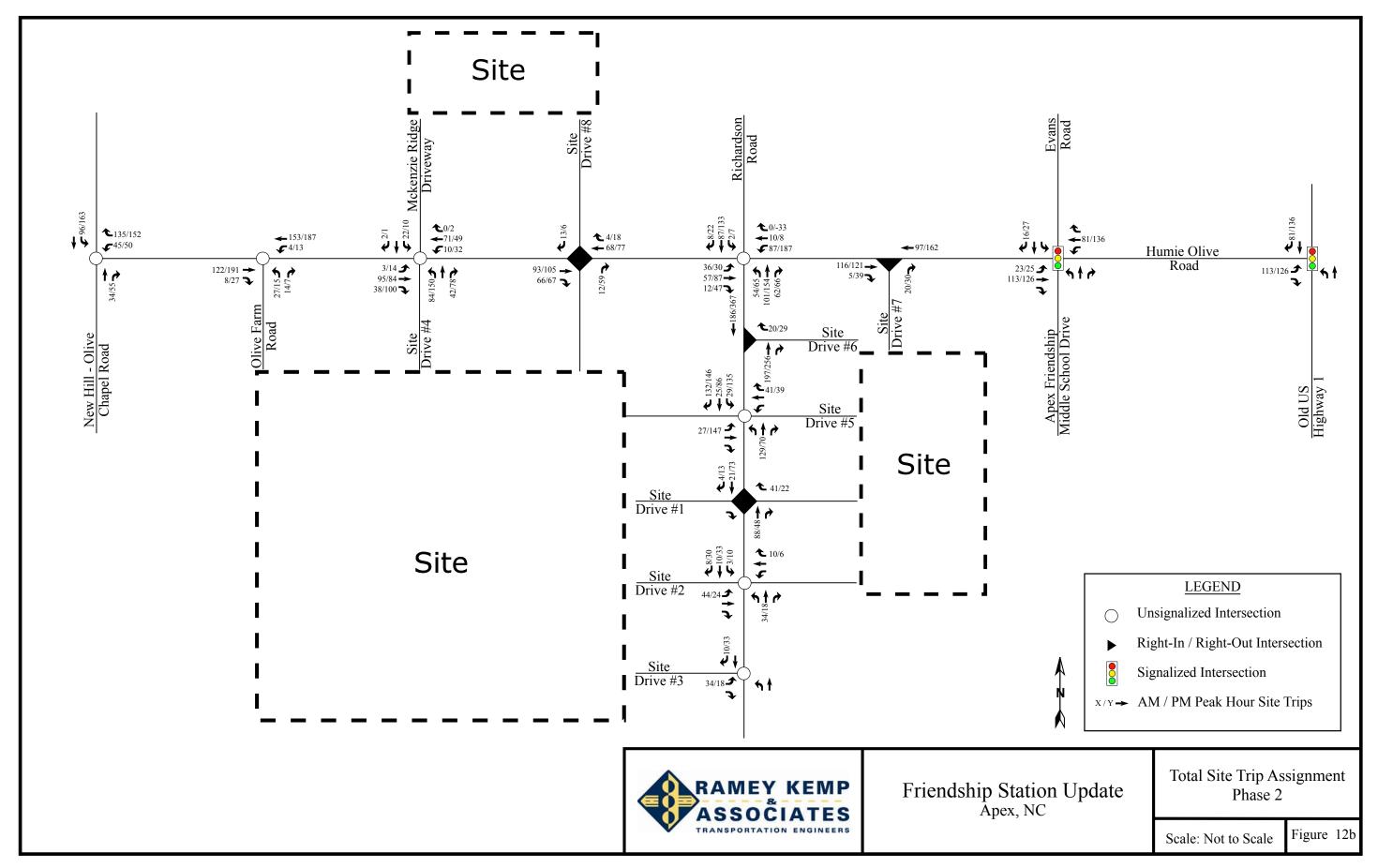
^{1.} Trip Generation for the townhomes north of Humie Olive Road were calculated separately. These trips were not included in the internal capture calculations.

44% of Total Built (PM)

It is estimated that Phase 2 of the proposed development will generate approximately 18,000 total site trips on the roadway network during a typical 24-hour weekday period. Of the daily traffic volume, it is anticipated that 871 total trips (382 entering and 489 exiting) will occur during the weekday AM peak hour and 1,710 (874 entering and 836 exiting) will occur during the weekday PM peak hour.

Internal capture of trips between the residential and retail uses were considered in this study. It is worth noting that the 99 townhomes north of Humie Olive Road were not included in the internal capture calculations due to their separation from the retail land uses. Based on ITE 9th





TRAFFIC IMPACT ANALYSIS

FOR

DEPOT 499

LOCATED

IN

Apex, North Carolina

Prepared For:

Lennar Corporation 1100 Perimeter Park Drive, Suite 112 Morrisville, North Carolina 25760

> Prepared By: Ramey Kemp & Associates, Inc. 5808 Faringdon Place, Suite 100 Raleigh, NC 27609 License #C-0910

> > January 2020

Prepared By: NB

Reviewed By: RS

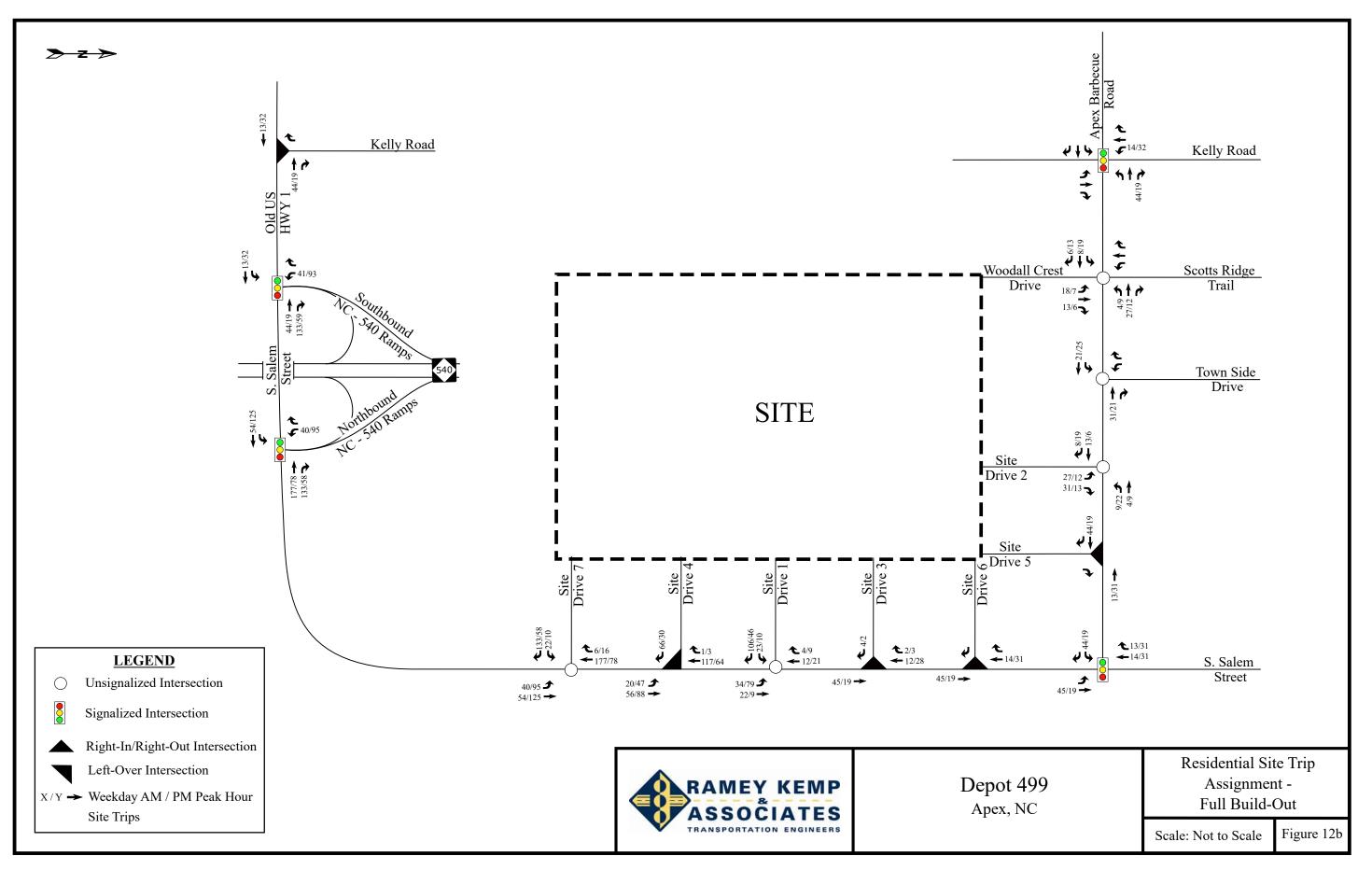
Table 2B: Trip Generation Summary – Full Buildout

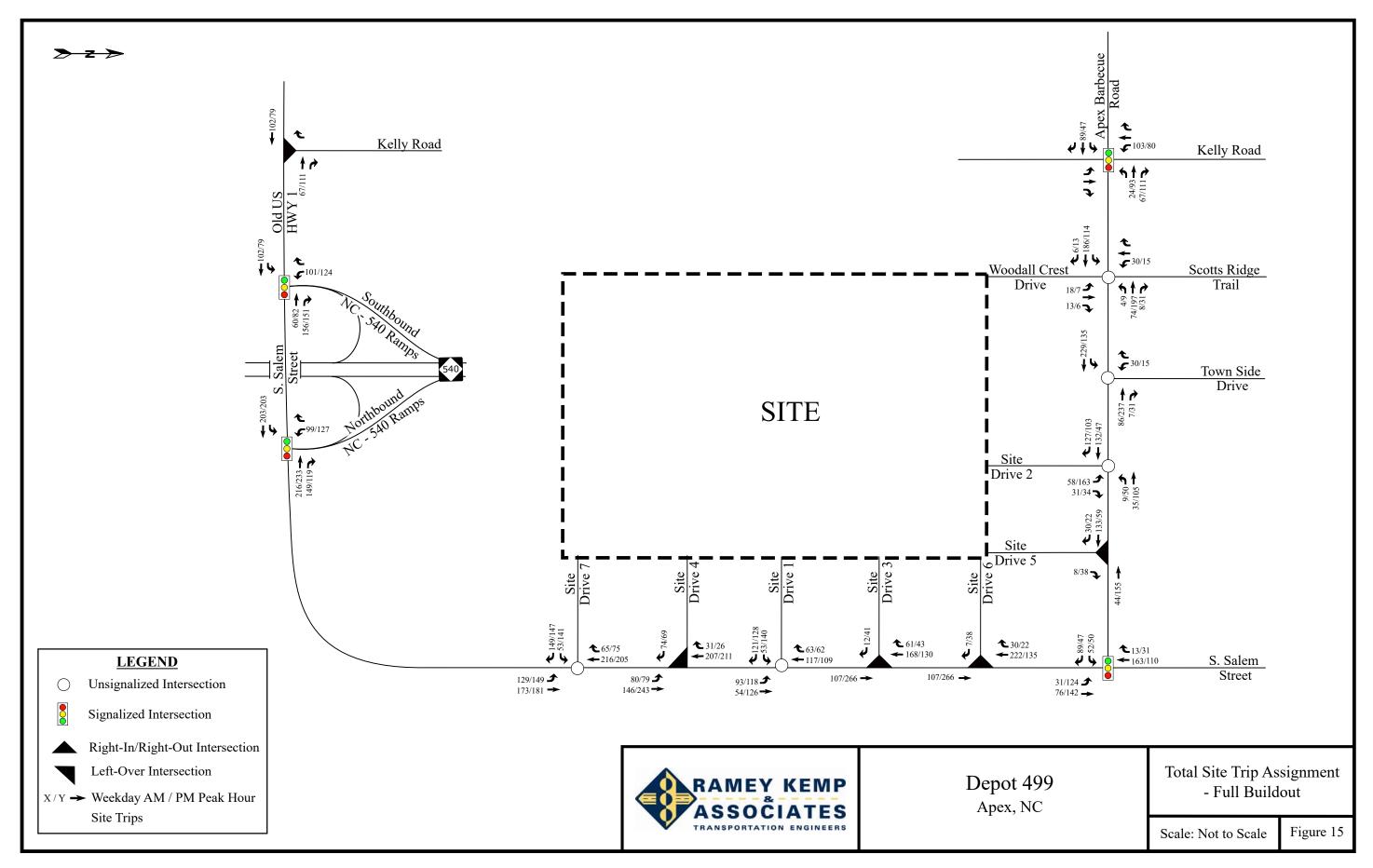
Land Use (ITE Code)	Intensity	Daily Traffic	Weekday Hour Tri		Hour Trips (vph)		
(III code)		(vpd)	Enter	Exit	Enter	Exit	
Multifamily Housing (Low-Rise) (220)	1,500 units	11,300	144	481	415	243	
General Office Building (710)	375,000 s.f.	3,820	467	64	86	392	
Shopping Center (820)	250,000 s.f.	11,210	172	105	514	556	
Total		26,330	783	650	1,015	1,191	
Internal Capt (7% Entering AM, 8% 24% Entering PM, 20%	Exiting AM		-55	-52	-244	-238	
Total External	Trips		728	598	771	953	
Pass-By Trips: Shopp (34% PM)	-		0	0	-142	-142	
Total Primary	Trips		728	598	629	811	

It is estimated that the proposed development will generate approximately 26,330 total site trips on the roadway network during a typical 24-hour weekday period. Of the daily traffic volume, it is anticipated that 1,433 trips (783 entering and 650 exiting) will occur during the weekday AM peak hour and 2,206 (1,015 entering and 1,191 exiting) will occur during the weekday PM peak hour.

Internal capture of trips between the residential and retail uses was considered in this study. Internal capture is the consideration for trips that will be made within the site between different land uses, so the vehicle never leaves the internal site but can still be considered as a trip to that specific land use. Internal capture typically only considers trips between residential, office, and retail/restaurant land uses. Based on the NCHRP Internal Capture methodology, an AM peak hour internal capture rate of 7% entering and 8% exiting was applied to the total trips. Also, a PM peak hour internal capture rate of 24% entering and 20% exiting was applied to the total trips. The internal capture reductions are expected to account for 107 (55 entering







TRAFFIC IMPACT ANALYSIS

FOR

SEARS PROPERTY

LOCATED

IN

APEX, NC

Prepared For: Beazer Homes - Raleigh Division 5400 Trinity Road, Suite 313 Raleigh, NC 27607

Prepared By: Ramey Kemp & Associates, Inc. 5808 Faringdon Place, Suite 100 Raleigh, NC 27609 License #C-0910

NOVEMBER 2021

While A. BOURING

Prepared By: DT

Reviewed By: NB

4. SITE TRIP GENERATION AND DISTRIBUTION

4.1. Trip Generation

The proposed development is assumed to consist of 160 townhomes and an 11,000 s.f. daycare center. Average weekday daily, AM peak hour, and PM peak hour trips for the proposed development were estimated using methodology contained within the ITE *Trip Generation Manual*, 10th Edition. Table 3 provides a summary of the trip generation potential for the site.

Weekday Weekday **PM Peak** AM Peak Daily **Land Use Intensity Traffic Hour Trips Hour Trips** (ITE Code) (vpd) (vph) (vph) **Enter Exit Enter** Exit Multifamily Housing (Low-Rise) 160 units 1,170 17 58 57 33 (220)Daycare Center 11,000 s.f. 520 64 57 57 65 (565)**Total Trips** 1,690 81 115 114 98

Table 3: Trip Generation Summary

It is estimated that the proposed development will generate approximately 1,690 total site trips on the roadway network during a typical 24-hour weekday period. Of the daily traffic volume, it is anticipated that 196 trips (81 entering and 115 exiting) will occur during the weekday AM peak hour and 212 trips (114 entering and 98 exiting) will occur during the weekday PM peak hour. It should be noted that the proposed development is anticipated to be below the typical threshold for NCDOT to require a TIA (3,000 trips per day); however, a copy of the TIA will be provided to NCDOT for courtesy review.

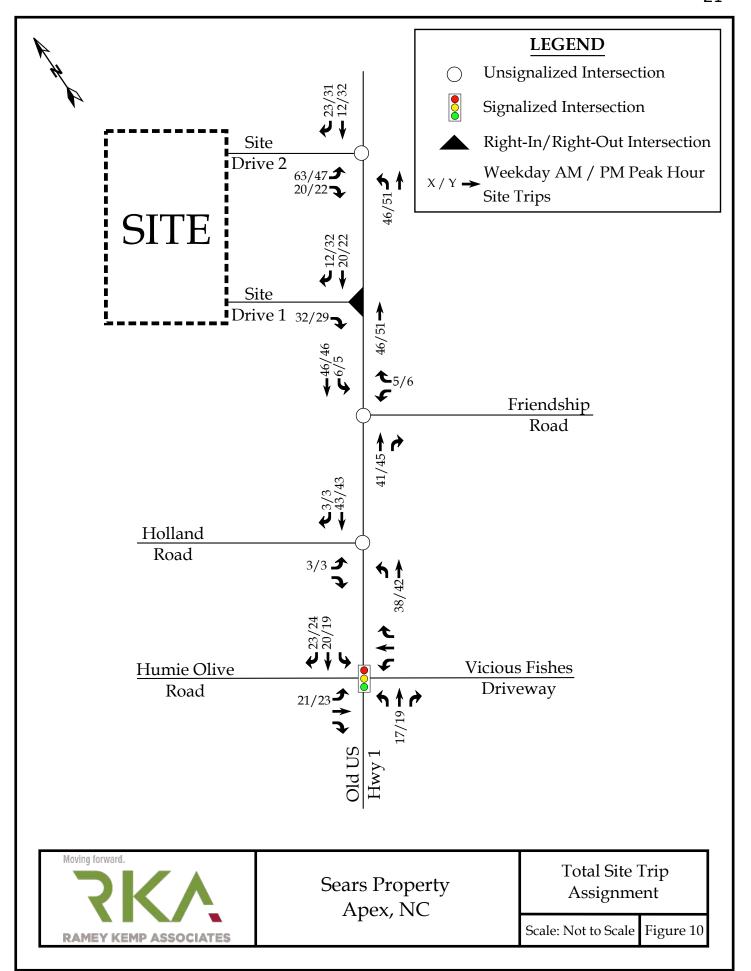
4.2. Site Trip Distribution and Assignment

Trip distribution percentages used in assigning site traffic for this development were estimated based on a combination of existing traffic patterns, population centers adjacent to the study area, and engineering judgment.

It is estimated that the residential site trips will be regionally distributed as follows:

• 75% to/from the north via Old US Hwy 1





TRAFFIC IMPACT ANALYSIS

FOR

HOLLAND ROAD MIXED-USE

LOCATED

IN

APEX, NC

Prepared For: Peak Engineering & Design, PLLC 1125 Apex Peakway Apex, NC 27502

Prepared By: Ramey Kemp & Associates, Inc. 5808 Faringdon Place, Suite 100 Raleigh, NC 27609 License #C-0910

APRIL 2021

SEAL 050502
4/29/21

ENGINEER ONLY

MANUAL A. BOUNDER

RKA Project No. 21015

Prepared By: <u>DT</u>

Reviewed By: NB

4. SITE TRIP GENERATION AND DISTRIBUTION

4.1. Trip Generation

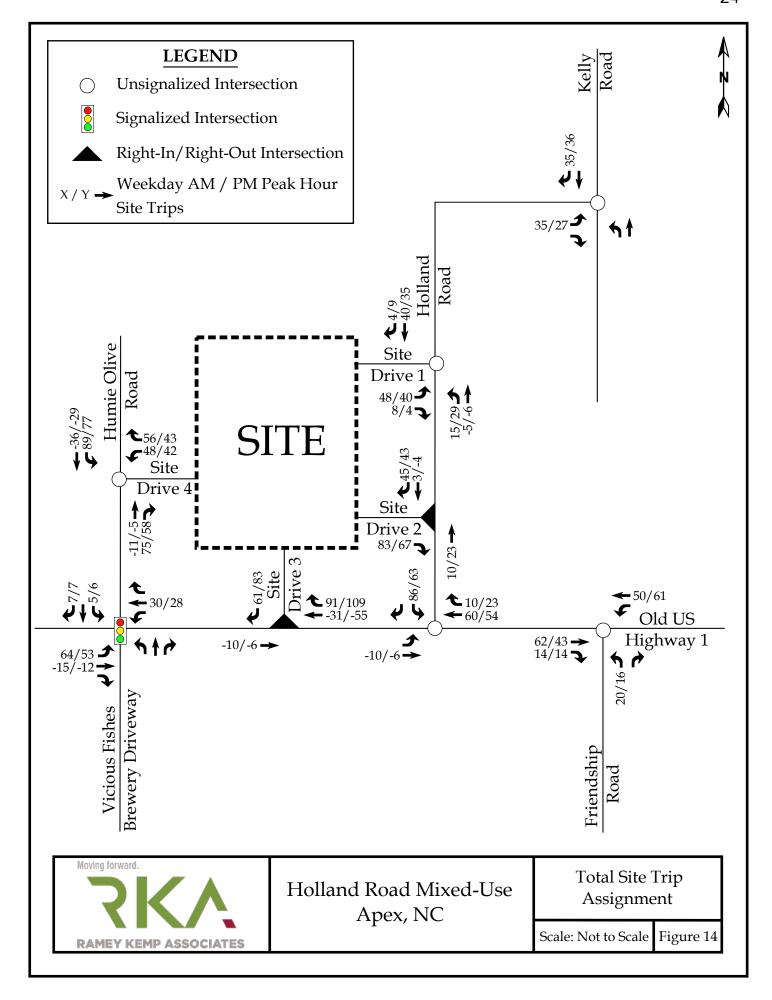
The proposed development is assumed to consist of a maximum of 110 single-family homes, a 60,000 s.f. shopping center, two (2) 4,000 s.f. fast-food restaurants with drive-thru, an 8,500 s.f. quality restaurant, and a 10 f.p. gas station with convenience market. Average weekday daily, AM peak hour, and PM peak hour trips for the proposed development were estimated using methodology contained within the ITE *Trip Generation Manual*, 10th Edition. Table 3 provides a summary of the trip generation potential for the site.

Table 3: Trip Generation Summary

Land Use (ITE Code)	Intensity	Daily Traffic (vpd)	Weel AM Pea Trips	k Hour (vph)	Weekday PM Peak Hour Trips (vph)		
		(Vpu)	Enter	Exit	Enter	Exit	
Single Family Homes (210)	110 units	1,140	21	62	70	41	
Shopping Center (820)	60,000 s.f.	4,250	113	69	179	193	
Quality Restaurant (931)	8,500 s.f.	710	**	**	44	22	
Fast-Food Restaurant w/ Drive-Thru (934)	8,000 s.f.	3,770	164	158	136	125	
Gas Station w/ Convenience Market (945)	10 f.p.	2,050	64	61	71	69	
Total Trips	•	11,920	362	350	500	450	
Internal Capture (12% AM Entering & 13% AM (35% PM Entering, 38% PM E			-43	-46	-175	-171	
Total External Trips			319	304	325	279	
Pass-By Trips: Fast-Food Restaurant wit (49% AM, 50% PM)	th Drive-Throug	h	-69	-69	-41	-41	
Pass-By Trips: Quality Resta (44% PM)	urant				-9	-9	
Pass-By Trips: Gas Station w/ Conve (62% AM, 56% PM)	nience Market		-34	-34	-25	-25	
Pass-By Trips: Shopping Co (34% PM)	enter				-40	-40	
Total Primary Trips			216	201	210	164	

^{**}No trips are expected to be generated during the weekday AM peak hour, as this land use is not typically open during this peak hour.







SYNCHRO AND SIMTRAFFIC REPORTS

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	f)		ሻ	1	7	ሻ	†	7	ኻ	†	7
Traffic Volume (vph)	433	293	107	89	216	139	92	318	113	128	154	171
Future Volume (vph)	433	293	107	89	216	139	92	318	113	128	154	171
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			-2%			4%			3%	
Storage Length (ft)	300		450	350		350	250		250	325	- 7.	500
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.0.	0.960				0.850			0.850			0.850
Flt Protected	0.950	0.000		0.950		0.000	0.950		0.000	0.950		0.000
Satd. Flow (prot)	3399	1770	0	1787	1881	1599	1734	1825	1552	1743	1835	1560
Flt Permitted	0.950	1110	v	0.229	1001	1000	0.574	1020	1002	0.229	1000	1000
Satd. Flow (perm)	3399	1770	0	431	1881	1599	1048	1825	1552	420	1835	1560
Right Turn on Red	0000	1770	No	701	1001	No	1040	1020	No	720	1000	No
Satd. Flow (RTOR)			140			140			110			110
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		2183			2240			1627			1614	
Travel Time (s)		33.1			33.9			24.7			24.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	481	326	119	99	240	154	102	353	126	142	171	190
Shared Lane Traffic (%)	404	445	0	00	040	454	400	252	400	4.40	474	400
Lane Group Flow (vph)	481	445	0	99	240	154	102	353	126	142	171	190
Turn Type	Prot	NA		D.P+P	NA	Free	D.P+P	NA	pm+ov	D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8	1	7	4	5
Permitted Phases	_	•		2	•	Free	4	•	8	8		4
Detector Phase	5	2		1	6		3	8	1	7	4	5
Switch Phase		40.0			40.0			- 0			- 0	
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0	14.0	14.0	14.0	14.0
Total Split (s)	15.0	90.0		15.0	90.0		15.0	25.0	15.0	15.0	25.0	15.0
Total Split (%)	10.3%	62.1%		10.3%	62.1%		10.3%	17.2%	10.3%	10.3%	17.2%	10.3%
Maximum Green (s)	8.0	83.0		8.0	83.0		8.0	18.0	8.0	8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lag	Lead		Lag	Lead		Lag	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	2.0	2.0	2.0	2.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	2.0	2.0	2.0	2.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min		None	Min		None	None	None	None	None	None
Act Effct Green (s)	19.7	27.6		37.4	17.8	87.3	30.9	20.1	34.9	29.8	19.6	44.3
Actuated g/C Ratio	0.23	0.32		0.43	0.20	1.00	0.35	0.23	0.40	0.34	0.22	0.51
v/c Ratio	0.63	0.79		0.29	0.63	0.10	0.21	0.84	0.20	0.49	0.42	0.24
Control Delay	35.0	38.4		18.4	40.1	0.10	21.3	53.1	20.0	34.1	36.4	15.0
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Quodo Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	35.0	38.4		18.4	40.1	0.1	21.3	53.1	20.0	34.1	36.4	15.0
LOS	С	D		В	D	Α	С	D	С	С	D	В
Approach Delay		36.6			23.3			40.4			27.7	
Approach LOS		D			С			D			С	
Queue Length 50th (ft)	122	223		28	123	0	35	187	44	50	89	64
Queue Length 95th (ft)	188	332		54	206	0	80	#389	97	107	159	109
Internal Link Dist (ft)		2103			2160			1547			1534	
Turn Bay Length (ft)	300			350		350	250		250	325		500
Base Capacity (vph)	765	1690		342	1796	1599	476	420	587	297	489	791
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.63	0.26		0.29	0.13	0.10	0.21	0.84	0.21	0.48	0.35	0.24

Intersection Summary

Area Type: Other

Cycle Length: 145

Actuated Cycle Length: 87.3

Natural Cycle: 75

Control Type: Actuated-Uncoordinated

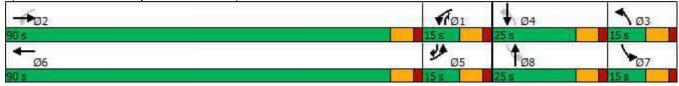
Maximum v/c Ratio: 0.84 Intersection Signal Delay: 33.1 Intersection Capacity Utilization 68.3%

Intersection LOS: C ICU Level of Service C

Analysis Period (min) 15

Queue shown is maximum after two cycles.

Splits and Phases: 1: Kelly Road & Olive Chapel Road



^{# 95}th percentile volume exceeds capacity, queue may be longer.

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Lane Group EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL	SBT SE	BR
		_
Lane Configurations 77 5 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	†	7
Traffic Volume (vph) 389 257 35 190 400 151 54 329 130 129		140
Future Volume (vph) 389 257 35 190 400 151 54 329 130 129		140
Ideal Flow (vphpl) 1900 1900 1900 1900 1900 1900 1900 190		900
Grade (%) 2% -2% 4%	3%	-00
Storage Length (ft) 300 450 350 250 250 325	5	500
Storage Lanes 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		1
Taper Length (ft) 100 100 100 100	4.00	
Lane Util. Factor 0.97 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0		.00
Frt 0.982 0.850 0.850	0.8	350
Fit Protected 0.950 0.950 0.950 0.950	1005 15	
Satd. Flow (prot) 3399 1811 0 1787 1881 1599 1734 1825 1552 1743	1835 15	60
Flt Permitted 0.950 0.313 0.365 0.215		
Satd. Flow (perm) 3399 1811 0 589 1881 1599 666 1825 1552 394		60
Right Turn on Red No No No	I	No
Satd. Flow (RTOR)		
Link Speed (mph) 45 45 45	45	
Link Distance (ft) 2183 2240 1627	1614	
Travel Time (s) 33.1 33.9 24.7	24.5	
Peak Hour Factor 0.90		.90
Adj. Flow (vph) 432 286 39 211 444 168 60 366 144 143	311 4	189
Shared Lane Traffic (%)	044	
Lane Group Flow (vph) 432 325 0 211 444 168 60 366 144 143		189
Turn Type Prot NA D.P+P NA Free D.P+P NA pm+ov D.P+P	NA pm+	
Protected Phases 5 2 1 6 3 8 1 7	4	5
Permitted Phases 2 Free 4 8 8 Detector Phase 5 2 1 6 3 8 1 7	4	4
	4	5
Switch Phase	70 7	7.0
Minimum Initial (s) 7.0 12.0 7.0 12.0 7.0 7.0 7.0 7.0 7.0 Minimum Split (s) 14.0 19.0 14.0 19.0 14.0 14.0 14.0		7.0
		4.0 5.0
	25.0 15 17.2% 10.3	
Total Split (%) 10.3% 62.1% 10.3% 62.1% 10.3% 17.2% 10.3% 10.3% Maximum Green (s) 8.0 83.0 8.0 83.0 8.0 18.0 8.0		3% 8.0
		5.0 5.0
Yellow Time (s) 5.0		2.0
Lost Time Adjust (s) -2.0 -2.0 -2.0 -2.0 -2.0 -2.0 -2.0 -2.0		2.0
Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0 5.0 5.0 5.0		5.0
Lead/Lag Lag Lead Lag Lead Lag Lead Lag Lag Lead Lag		.ag ₋ag
Lead-Lag Optimize? Yes Yes Yes Yes Yes Yes Yes Yes		'es
Vehicle Extension (s) 2.0 6.0 2.0 6.0 2.0 2.0 2.0 2.0		2.0
Minimum Gap (s) 2.0 3.0 2.0 3.0 2.0 2.0 2.0 2.0		2.0
Time Before Reduce (s) 0.0 15.0 0.0 15.0 0.0 0.0 0.0 0.0		0.0
Time To Reduce (s) 0.0 30.0 0.0 30.0 0.0 0.0 0.0 0.0		0.0
Recall Mode None Min None Min None None None None	None No	
Act Effet Green (s) 10.0 21.4 36.0 26.0 85.8 30.8 20.1 39.7 29.7		8.3
Actuated g/C Ratio 0.12 0.25 0.42 0.30 1.00 0.36 0.23 0.46 0.35		.45
v/c Ratio 1.09 0.72 0.47 0.78 0.11 0.17 0.86 0.20 0.50		.70
Control Delay 108.9 39.0 23.4 37.6 0.1 20.7 53.9 15.9 33.7		9.4
Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.		0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	108.9	39.0		23.4	37.6	0.1	20.7	53.9	15.9	33.7	37.7	29.4
LOS	F	D		С	D	Α	С	D	В	С	D	С
Approach Delay		78.9			26.3			40.8			32.8	
Approach LOS		Ε			С			D			С	
Queue Length 50th (ft)	~137	161		65	218	0	19	191	44	49	156	222
Queue Length 95th (ft)	#259	252		106	324	0	50	#392	95	103	#311	#445
Internal Link Dist (ft)		2103			2160			1547			1534	
Turn Bay Length (ft)	300			350		350	250		250	325		500
Base Capacity (vph)	398	1753		450	1821	1599	367	427	717	296	495	695
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.09	0.19		0.47	0.24	0.11	0.16	0.86	0.20	0.48	0.63	0.70

Intersection Summary

Area Type: Other

Cycle Length: 145

Actuated Cycle Length: 85.8

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

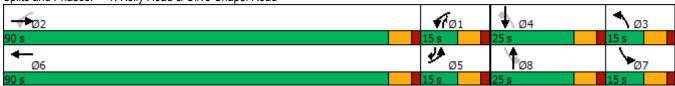
Maximum v/c Ratio: 1.09 Intersection Signal Delay: 43.8 Intersection Capacity Utilization 73.3%

Intersection LOS: D ICU Level of Service D

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Kelly Road & Olive Chapel Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	f)		ሻ	†	7	ሻ	1	7	ሻ	1	7
Traffic Volume (vph)	507	343	125	104	253	163	108	451	132	150	229	200
Future Volume (vph)	507	343	125	104	253	163	108	451	132	150	229	200
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			-2%			4%			3%	
Storage Length (ft)	300		450	350		350	250		250	325		500
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.960				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3399	1770	0	1787	1881	1599	1734	1825	1552	1743	1835	1560
Flt Permitted	0.950			0.193			0.393			0.199		
Satd. Flow (perm)	3399	1770	0	363	1881	1599	717	1825	1552	365	1835	1560
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		2183			2240			1627			1614	
Travel Time (s)		33.1			33.9			24.7			24.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	563	381	139	116	281	181	120	501	147	167	254	222
Shared Lane Traffic (%)	000	001	100		20.		0	00.			20 .	
Lane Group Flow (vph)	563	520	0	116	281	181	120	501	147	167	254	222
Turn Type	Prot	NA	•	D.P+P	NA	Free	D.P+P	NA	pm+ov	D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8	1	7	4	5
Permitted Phases		_		2	· ·	Free	4	·	8	8	•	4
Detector Phase	5	2		1	6		3	8	1	7	4	5
Switch Phase	Ū	_		•	ŭ		Ū	ŭ	•		•	Ū
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0	14.0	14.0	14.0	14.0
Total Split (s)	15.0	90.0		15.0	90.0		15.0	25.0	15.0	15.0	25.0	15.0
Total Split (%)	10.3%	62.1%		10.3%	62.1%		10.3%	17.2%	10.3%	10.3%	17.2%	10.3%
Maximum Green (s)	8.0	83.0		8.0	83.0		8.0	18.0	8.0	8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lag	Lead		Lag	Lead		Lag	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	2.0	2.0	2.0	2.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	2.0	2.0	2.0	2.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min		None	Min		None	None	None	None	None	None
Act Effct Green (s)	22.5	32.9		42.8	20.3	93.1	30.2	20.1	35.0	30.2	20.1	47.6
Actuated g/C Ratio	0.24	0.35		0.46	0.22	1.00	0.32	0.22	0.38	0.32	0.22	0.51
v/c Ratio	0.24	0.83		0.40	0.22	0.11	0.32	1.27	0.36	0.32	0.22	0.31
					42.8	0.11	28.7	1.2 <i>1</i> 174.1	23.5	44.9		
Control Delay	37.5	39.7		21.4							43.6	14.9
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	37.5	39.7		21.4	42.8	0.1	28.7	174.1	23.5	44.9	43.6	14.9
LOS	D	D		С	D	Α	С	F	С	D	D	В
Approach Delay		38.6			25.2			122.5			34.0	
Approach LOS		D			С			F			С	
Queue Length 50th (ft)	154	276		34	153	0	46	~376	58	67	137	70
Queue Length 95th (ft)	234	400		60	248	0	102	#660	125	#159	#269	137
Internal Link Dist (ft)		2103			2160			1547			1534	
Turn Bay Length (ft)	300			350		350	250		250	325		500
Base Capacity (vph)	820	1613		321	1714	1599	342	394	568	267	396	798
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.69	0.32		0.36	0.16	0.11	0.35	1.27	0.26	0.63	0.64	0.28

Intersection Summary

Area Type: Other

Cycle Length: 145

Actuated Cycle Length: 93.1

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.27 Intersection Signal Delay: 56.1 Intersection Capacity Utilization 80.2%

Intersection LOS: E ICU Level of Service D

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Kelly Road & Olive Chapel Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1,1	f)		ሻ	†	7	ሻ	†	7	ሻ	1	7
Traffic Volume (vph)	455	301	41	222	468	177	63	431	152	151	396	515
Future Volume (vph)	455	301	41	222	468	177	63	431	152	151	396	515
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			-2%			4%			3%	
Storage Length (ft)	300		450	350		350	250		250	325		500
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.982				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3399	1811	0	1787	1881	1599	1734	1825	1552	1743	1835	1560
Flt Permitted	0.950			0.261			0.171			0.199		
Satd. Flow (perm)	3399	1811	0	491	1881	1599	312	1825	1552	365	1835	1560
Right Turn on Red	0000		No			No	V		No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		2183			2240			1627			1614	
Travel Time (s)		33.1			33.9			24.7			24.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	506	334	46	247	520	197	70	479	169	168	440	572
Shared Lane Traffic (%)	000	001	10		020	107	10	170	100	100	110	012
Lane Group Flow (vph)	506	380	0	247	520	197	70	479	169	168	440	572
Turn Type	Prot	NA	v	D.P+P	NA	Free	D.P+P	NA	pm+ov	D.P+P	NA	pm+ov
Protected Phases	5	2		1	6	1100	3	8	1	7	4	5
Permitted Phases	J	_		2	·	Free	4	Ū	8	8		4
Detector Phase	5	2		1	6	1100	3	8	1	7	4	5
Switch Phase	U	_			Ū		J	O		•	7	O
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0	14.0	14.0	14.0	14.0
Total Split (s)	15.0	90.0		15.0	90.0		15.0	25.0	15.0	15.0	25.0	15.0
Total Split (%)	10.3%	62.1%		10.3%	62.1%		10.3%	17.2%	10.3%	10.3%	17.2%	10.3%
Maximum Green (s)	8.0	83.0		8.0	83.0		8.0	18.0	8.0	8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lag	Lead		Lag	Lead		Lag	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	2.0	2.0	2.0	2.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	2.0	2.0	2.0	2.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min			Min					None		None
				None		01.1	None	None	None		None	
Act Effct Green (s)	10.1	25.3		40.8	30.8	91.1	31.3	20.1	40.7	30.2	23.4	38.5
Actuated g/C Ratio	0.11	0.28		0.45	0.34	1.00	0.34	0.22	0.45	0.33	0.26	0.42
v/c Ratio	1.35	0.76		0.56	0.82	0.12	0.27	1.19	0.24	0.62	0.93	0.87
Control Delay	208.7	40.3		28.2	38.8	0.2	30.8	142.0	18.2	42.8	65.7	43.3
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	208.7	40.3		28.2	38.8	0.2	30.8	142.0	18.2	42.8	65.7	43.3
LOS	F	D		С	D	Α	С	F	В	D	Ε	D
Approach Delay		136.4			28.2			102.0			51.6	
Approach LOS		F			С			F			D	
Queue Length 50th (ft)	~197	199		78	270	0	25	~334	58	64	~286	311
Queue Length 95th (ft)	#338	301		123	390	0	62	#601	120	#148	#541	#617
Internal Link Dist (ft)		2103			2160			1547			1534	
Turn Bay Length (ft)	300			350		350	250		250	325		500
Base Capacity (vph)	375	1685		441	1750	1599	263	402	692	272	472	659
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.35	0.23		0.56	0.30	0.12	0.27	1.19	0.24	0.62	0.93	0.87

Intersection Summary

Area Type: Other

Cycle Length: 145

Actuated Cycle Length: 91.1

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.35 Intersection Signal Delay: 75.3 Intersection Capacity Utilization 85.3%

Intersection LOS: E ICU Level of Service E

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Kelly Road & Olive Chapel Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1,4	f)		7	<u></u>	7	7	<u></u>	7	7	†	7
Traffic Volume (vph)	507	343	131	122	253	163	111	459	144	150	241	200
Future Volume (vph)	507	343	131	122	253	163	111	459	144	150	241	200
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			-2%			4%			3%	
Storage Length (ft)	300		450	350		350	250		250	325		500
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.958				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3399	1767	0	1787	1881	1599	1734	1825	1552	1743	1835	1560
Flt Permitted	0.950			0.191			0.362			0.199		
Satd. Flow (perm)	3399	1767	0	359	1881	1599	661	1825	1552	365	1835	1560
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		2183			2240			1627			1614	
Travel Time (s)		33.1			33.9			24.7			24.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	563	381	146	136	281	181	123	510	160	167	268	222
Shared Lane Traffic (%)	303	301	140	130	201	101	125	310	100	101	200	222
Lane Group Flow (vph)	563	527	0	136	281	181	123	510	160	167	268	222
Turn Type	Prot	NA	U	D.P+P	NA	Free	D.P+P	NA	pm+ov	D.P+P	NA	
Protected Phases	5	2		D.Г+Г	6	riee	D.F+F	8	ριτι τ ον 1	D.F+F	4	pm+ov 5
Permitted Phases	5	۷		2	U	Free	4	O	8	8	4	4
Detector Phase	5	2		1	6	riee	3	8	1	7	4	5
Switch Phase	5	2		1	O		3	0	1	1	4	5
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0	7.0	7.0	7.0	7.0
` ,	14.0	19.0		14.0	19.0		14.0	14.0	14.0	14.0	14.0	14.0
Minimum Split (s)												
Total Split (s)	15.0	90.0 62.1%		15.0	90.0		15.0	25.0	15.0	15.0	25.0	15.0
Total Split (%)	10.3%			10.3%	62.1%		10.3%	17.2%	10.3%	10.3%	17.2%	10.3%
Maximum Green (s)	8.0	83.0		8.0	83.0		8.0	18.0	8.0	8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lag	Lead		Lag	Lead		Lag	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	2.0	2.0	2.0	2.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	2.0	2.0	2.0	2.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min		None	Min		None	None	None	None	None	None
Act Effct Green (s)	23.0	33.5		43.4	20.4	93.7	30.2	20.1	35.0	30.2	20.1	48.1
Actuated g/C Ratio	0.25	0.36		0.46	0.22	1.00	0.32	0.21	0.37	0.32	0.21	0.51
v/c Ratio	0.68	0.84		0.43	0.69	0.11	0.38	1.30	0.28	0.63	0.68	0.28
Control Delay	37.2	39.8		24.3	43.2	0.1	30.2	186.5	24.1	45.5	45.7	14.9
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	37.2	39.8		24.3	43.2	0.1	30.2	186.5	24.1	45.5	45.7	14.9
LOS	D	D		С	D	Α	С	F	С	D	D	В
Approach Delay		38.5			25.8			129.5			35.2	
Approach LOS		D			С			F			D	
Queue Length 50th (ft)	154	282		40	154	0	48	~391	65	67	147	70
Queue Length 95th (ft)	234	407		69	249	0	105	#680	136	#161	#296	138
Internal Link Dist (ft)		2103			2160			1547			1534	
Turn Bay Length (ft)	300			350		350	250		250	325		500
Base Capacity (vph)	832	1602		320	1706	1599	328	392	564	265	394	801
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.68	0.33		0.42	0.16	0.11	0.38	1.30	0.28	0.63	0.68	0.28

Intersection Summary

Area Type: Other

Cycle Length: 145

Actuated Cycle Length: 93.7

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

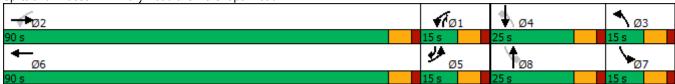
Maximum v/c Ratio: 1.30 Intersection Signal Delay: 58.4 Intersection Capacity Utilization 81.9%

Intersection LOS: E ICU Level of Service D

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Kelly Road & Olive Chapel Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1/4	f)		ሻ	<u></u>	7	ሻ	<u></u>	7	ሻ	<u></u>	7
Traffic Volume (vph)	455	301	45	235	468	177	69	443	170	151	405	515
Future Volume (vph)	455	301	45	235	468	177	69	443	170	151	405	515
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			-2%			4%			3%	
Storage Length (ft)	300		450	350		350	250		250	325		500
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.980				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3399	1807	0	1787	1881	1599	1734	1825	1552	1743	1835	1560
Flt Permitted	0.950			0.258			0.171			0.199		
Satd. Flow (perm)	3399	1807	0	485	1881	1599	312	1825	1552	365	1835	1560
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		2183			2240			1627			1614	
Travel Time (s)		33.1			33.9			24.7			24.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	506	334	50	261	520	197	77	492	189	168	450	572
Shared Lane Traffic (%)												
Lane Group Flow (vph)	506	384	0	261	520	197	77	492	189	168	450	572
Turn Type	Prot	NA		D.P+P	NA	Free	D.P+P	NA	pm+ov	D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8	່ 1	7	4	5
Permitted Phases				2		Free	4		8	8		4
Detector Phase	5	2		1	6		3	8	1	7	4	5
Switch Phase												
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0	7.0	7.0	7.0	7.0
` ,	14.0	19.0		14.0	19.0		14.0	14.0	14.0	14.0	14.0	14.0
	10.3%	62.1%		10.3%	62.1%		10.3%	17.2%	10.3%	10.3%	17.2%	10.3%
,		83.0										
` ,	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
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Minimum Split (s) Total Split (s) Total Split (%) Maximum Green (s) Yellow Time (s) All-Red Time (s) Lost Time Adjust (s) Total Lost Time (s) Lead/Lag Lead-Lag Optimize? Vehicle Extension (s) Minimum Gap (s) Time Before Reduce (s) Time To Reduce (s) Recall Mode Act Effct Green (s) Actuated g/C Ratio v/c Ratio Control Delay Queue Delay	14.0 15.0 10.3% 8.0 5.0 2.0 -2.0 5.0 Lag Yes 2.0 0.0 0.0 None 10.1 1.35 208.7 0.0	19.0 90.0		14.0 15.0 10.3% 8.0 5.0 2.0 -2.0 5.0 Lag Yes 2.0 0.0 0.0 None 40.8 0.45 0.60 30.0 0.0	19.0 90.0	91.1 1.00 0.12 0.2 0.0	14.0 15.0 10.3% 8.0 5.0 2.0 -2.0 5.0 Lag Yes 2.0 0.0 0.0 None 31.3 0.34 0.30 31.7 0.0	14.0 25.0 17.2% 18.0 5.0 2.0 -2.0 5.0 Lead Yes 2.0 0.0 0.0 None 20.1 0.22 1.22 154.1 0.0	14.0 15.0 10.3% 8.0 5.0 2.0 -2.0 5.0 Lag Yes 2.0 0.0 0.0 None 40.5 0.44 0.27 18.7 0.0	14.0 15.0	14.0 25.0 17.2% 18.0 5.0 2.0 -2.0 5.0 Lead Yes 2.0 0.0 0.0 None 23.4 0.26 0.95 69.8 0.0	14.0 15.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	208.7	40.3		30.0	38.8	0.2	31.7	154.1	18.7	42.8	69.8	43.3
LOS	F	D		С	D	Α	С	F	В	D	Ε	D
Approach Delay		136.0			28.7			107.9			53.2	
Approach LOS		F			С			F			D	
Queue Length 50th (ft)	~197	201		83	270	0	28	~350	66	64	~298	311
Queue Length 95th (ft)	#338	303		130	390	0	67	#620	135	#148	#556	#617
Internal Link Dist (ft)		2103			2160			1547			1534	
Turn Bay Length (ft)	300			350		350	250		250	325		500
Base Capacity (vph)	375	1682		436	1750	1599	263	402	689	272	472	659
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.35	0.23		0.60	0.30	0.12	0.29	1.22	0.27	0.62	0.95	0.87

Intersection Summary

Area Type: Other

Cycle Length: 145

Actuated Cycle Length: 91.1

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.35 Intersection Signal Delay: 77.1 Intersection Capacity Utilization 86.0%

Intersection LOS: E ICU Level of Service E

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Kelly Road & Olive Chapel Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	44	f)		ሻ	†	7	ሻ	<u></u>	7	ሻ	†	7
Traffic Volume (vph)	548	371	135	113	273	176	116	504	143	162	333	216
Future Volume (vph)	548	371	135	113	273	176	116	504	143	162	333	216
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			-2%			4%			3%	
Storage Length (ft)	300		450	350		350	250		250	325		500
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.960				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3399	1770	0	1787	1881	1599	1734	1825	1552	1743	1835	1560
Flt Permitted	0.950			0.174			0.199			0.199		
Satd. Flow (perm)	3399	1770	0	327	1881	1599	363	1825	1552	365	1835	1560
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		2183			2240			1627			1614	
Travel Time (s)		33.1			33.9			24.7			24.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	609	412	150	126	303	196	129	560	159	180	370	240
Shared Lane Traffic (%)												
Lane Group Flow (vph)	609	562	0	126	303	196	129	560	159	180	370	240
Turn Type	Prot	NA		D.P+P	NA	Free	D.P+P	NA	pm+ov	D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8	1	7	4	5
Permitted Phases				2		Free	4		8	8		4
Detector Phase	5	2		1	6		3	8	1	7	4	5
Switch Phase												
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0	14.0	14.0	14.0	14.0
Total Split (s)	15.0	90.0		15.0	90.0		15.0	25.0	15.0	15.0	25.0	15.0
Total Split (%)	10.3%	62.1%		10.3%	62.1%		10.3%	17.2%	10.3%	10.3%	17.2%	10.3%
Maximum Green (s)	8.0	83.0		8.0	83.0		8.0	18.0	8.0	8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lag	Lead		Lag	Lead		Lag	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	2.0	2.0	2.0	2.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	2.0	2.0	2.0	2.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min		None	Min	00.4	None	None	None	None	None	None
Act Effet Green (s)	24.1	36.2		46.0	21.9	96.4	30.2	20.1	35.1	30.2	20.1	49.3
Actuated g/C Ratio	0.25	0.38		0.48	0.23	1.00	0.31	0.21	0.36	0.31	0.21	0.51
v/c Ratio	0.72	0.85		0.41	0.71	0.12	0.50	1.47	0.28	0.70	0.97	0.30
Control Delay	39.1	40.2		24.0	44.2	0.2	41.1	256.1	25.6	51.5	78.7	15.9
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	39.1	40.2		24.0	44.2	0.2	41.1	256.1	25.6	51.5	78.7	15.9
LOS	D	D		С	D	Α	D	F	С	D	Е	В
Approach Delay		39.6			26.3			180.2			53.4	
Approach LOS		D			С			F			D	
Queue Length 50th (ft)	174	309		37	171	0	53	~473	68	77	225	80
Queue Length 95th (ft)	262	442		64	271	0	115	#789	142	#198	#481	156
Internal Link Dist (ft)		2103			2160			1547			1534	
Turn Bay Length (ft)	300			350		350	250		250	325		500
Base Capacity (vph)	850	1569		309	1668	1599	257	381	549	258	383	797
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.72	0.36		0.41	0.18	0.12	0.50	1.47	0.29	0.70	0.97	0.30

Intersection Summary

Area Type: Other

Cycle Length: 145

Actuated Cycle Length: 96.4

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

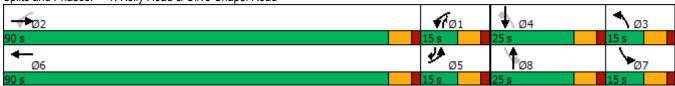
Maximum v/c Ratio: 1.47 Intersection Signal Delay: 75.1 Intersection Capacity Utilization 86.2%

Intersection LOS: E ICU Level of Service E

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Kelly Road & Olive Chapel Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	44	f)		ሻ	†	7	ሻ	<u></u>	7	ሻ	†	7
Traffic Volume (vph)	492	325	44	240	506	191	68	554	164	163	470	557
Future Volume (vph)	492	325	44	240	506	191	68	554	164	163	470	557
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			-2%			4%			3%	
Storage Length (ft)	300		450	350		350	250		250	325		500
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.982				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3399	1811	0	1787	1881	1599	1734	1825	1552	1743	1835	1560
Flt Permitted	0.950			0.238			0.170			0.199		
Satd. Flow (perm)	3399	1811	0	448	1881	1599	310	1825	1552	365	1835	1560
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		2183			2240			1627			1614	
Travel Time (s)		33.1			33.9			24.7			24.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	547	361	49	267	562	212	76	616	182	181	522	619
Shared Lane Traffic (%)												
Lane Group Flow (vph)	547	410	0	267	562	212	76	616	182	181	522	619
Turn Type	Prot	NA		D.P+P	NA	Free	D.P+P	NA	pm+ov	D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8	1	7	4	5
Permitted Phases				2		Free	4		8	8		4
Detector Phase	5	2		1	6		3	8	1	7	4	5
Switch Phase												
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0	14.0	14.0	14.0	14.0
Total Split (s)	15.0	90.0		15.0	90.0		15.0	25.0	15.0	15.0	25.0	15.0
Total Split (%)	10.3%	62.1%		10.3%	62.1%		10.3%	17.2%	10.3%	10.3%	17.2%	10.3%
Maximum Green (s)	8.0	83.0		8.0	83.0		8.0	18.0	8.0	8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lag	Lead		Lag	Lead		Lag	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	2.0	2.0	2.0	2.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	2.0	2.0	2.0	2.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min		None	Min	04.0	None	None	None	None	None	None
Act Effct Green (s)	10.1	27.6		43.7	33.6	94.0	31.3	20.1	41.3	30.2	23.5	38.6
Actuated g/C Ratio	0.11	0.29		0.46	0.36	1.00	0.33	0.21	0.44	0.32	0.25	0.41
v/c Ratio	1.51	0.77		0.61	0.84	0.13	0.30	1.58	0.27	0.69	1.14	0.97
Control Delay	273.9	40.7		31.2	39.3	0.2	33.6	301.5	19.6	48.9	123.1	60.4
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	273.9	40.7		31.2	39.3	0.2	33.6	301.5	19.6	48.9	123.1	60.4
LOS	F	D		С	D	Α	С	F	В	D	F	Е
Approach Delay		174.0			29.2			219.5			83.6	
Approach LOS		F			С			F			F	
Queue Length 50th (ft)	~234	221		85	301	0	29	~523	66	73	~404	~406
Queue Length 95th (ft)	#387	330		132	429	0	70	#840	137	#184	#695	#722
Internal Link Dist (ft)		2103			2160			1547			1534	
Turn Bay Length (ft)	300			350		350	250		250	325		500
Base Capacity (vph)	363	1643		437	1706	1599	255	390	681	264	458	640
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.51	0.25		0.61	0.33	0.13	0.30	1.58	0.27	0.69	1.14	0.97

Intersection Summary

Area Type: Other

Cycle Length: 145 Actuated Cycle Length: 94 Natural Cycle: 120

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.58 Intersection Signal Delay: 119.0 Intersection Capacity Utilization 95.5%

Intersection LOS: F ICU Level of Service F

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Kelly Road & Olive Chapel Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	*	7	ሻሻ	^	7	ሻ	†	7	ሻ	1	7
Traffic Volume (vph)	548	371	156	296	273	176	156	585	267	162	455	216
Future Volume (vph)	548	371	156	296	273	176	156	585	267	162	455	216
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			-2%			4%			3%	
Storage Length (ft)	300	_,,	200	325	_,-	350	250	.,.	250	325		500
Storage Lanes	2		1	2		1	1		1	1		1
Taper Length (ft)	100			100		•	100			100		-
Lane Util. Factor	0.97	1.00	1.00	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850	0.0.		0.850			0.850			0.850
Flt Protected	0.950		0.000	0.950		0.000	0.950		0.000	0.950		0.000
Satd. Flow (prot)	3399	1844	1567	3467	1881	1599	1734	1825	1552	1743	1835	1560
Flt Permitted	0.950			0.950			0.143			0.077		
Satd. Flow (perm)	3399	1844	1567	3467	1881	1599	261	1825	1552	141	1835	1560
Right Turn on Red	0000		No	0.01		No	20.	1020	No		1000	No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		2183			2240			1168			1614	
Travel Time (s)		33.1			33.9			17.7			24.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	609	412	173	329	303	196	173	650	297	180	506	240
Shared Lane Traffic (%)	000	112	170	020	000	100	110	000	201	100	000	210
Lane Group Flow (vph)	609	412	173	329	303	196	173	650	297	180	506	240
Turn Type	Prot	NA	pm+ov	Prot	NA	Free	D.P+P	NA	pm+ov	D.P+P	NA	pm+ov
Protected Phases	5	2	3	1	6	1100	3	8	1	7	4	5
Permitted Phases	Ū	_	2	•	Ū	Free	4	Ū	8	8		4
Detector Phase	5	2	3	1	6	1100	3	8	1	7	4	5
Switch Phase	Ū	_	Ū		O		Ū	J		,	7	O
Minimum Initial (s)	7.0	12.0	7.0	7.0	12.0		7.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	14.0	19.0	14.0	14.0	19.0		14.0	14.0	14.0	14.0	14.0	14.0
Total Split (s)	33.0	47.0	17.0	21.0	35.0		17.0	61.0	21.0	16.0	60.0	33.0
Total Split (%)	22.8%	32.4%	11.7%	14.5%	24.1%		11.7%	42.1%	14.5%	11.0%	41.4%	22.8%
Maximum Green (s)	26.0	40.0	10.0	14.0	28.0		10.0	54.0	14.0	9.0	53.0	26.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lag	Lead	Lag	Lag	Lead		Lag	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0	2.0	2.0	6.0		2.0	2.0	2.0	2.0	2.0	2.0
Minimum Gap (s)	2.0	3.0	2.0	2.0	3.0		2.0	2.0	2.0	2.0	2.0	2.0
Time Before Reduce (s)	0.0	15.0	0.0	0.0	15.0		0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	30.0	0.0	0.0	30.0		0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode		Min			Min					None		None
Act Effct Green (s)	None 27.3	35.7	None 59.6	None 18.0	26.5	137.3	None 63.4	None 52.3	None 75.4	63.4	None 44.6	76.9
	0.20			0.13						0.46		
Actuated g/C Ratio		0.26	0.43		0.19	1.00	0.46	0.38	0.55		0.32	0.56
v/c Ratio	0.90	0.86	0.25	0.72	0.83	0.12	0.54	0.94	0.35	0.93	0.85	0.27
Control Delay	72.3	66.8	27.3	68.6	74.0	0.2	48.5	63.3	19.5	100.6	57.4	16.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	72.3	66.8	27.3	68.6	74.0	0.2	48.5	63.3	19.5	100.6	57.4	16.3
LOS	Ε	Ε	С	Е	Е	Α	D	Ε	В	F	Ε	В
Approach Delay		63.9			54.4			49.4			55.2	
Approach LOS		Ε			D			D			Ε	
Queue Length 50th (ft)	293	369	102	155	273	0	89	578	149	117	444	115
Queue Length 95th (ft)	#405	488	168	#241	#404	0	#149	#824	228	#276	547	148
Internal Link Dist (ft)		2103			2160			1088			1534	
Turn Bay Length (ft)	300		200	325		350	250		250	325		500
Base Capacity (vph)	699	568	679	455	414	1599	321	750	851	194	741	871
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.87	0.73	0.25	0.72	0.73	0.12	0.54	0.87	0.35	0.93	0.68	0.28

Intersection Summary

Area Type: Other

Cycle Length: 145

Actuated Cycle Length: 137.3

Natural Cycle: 90

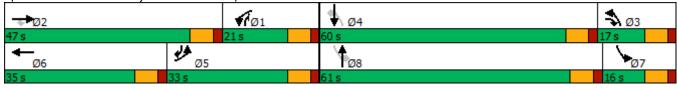
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.94 Intersection Signal Delay: 56.0 Intersection Capacity Utilization 86.4%

Intersection LOS: E ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: Kelly Road & Olive Chapel Road



^{# 95}th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	1	7	1,1	^	7	ሻ	1	7	*	1	7
Traffic Volume (vph)	492	325	51	439	506	191	146	707	399	163	603	557
Future Volume (vph)	492	325	51	439	506	191	146	707	399	163	603	557
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			-2%			4%			3%	
Storage Length (ft)	300		200	325		350	250		250	325		500
Storage Lanes	2		1	2		1	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	0.97	1.00	1.00	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.01	1.00	0.850	0.01	1.00	0.850			0.850	1.00	1.00	0.850
Flt Protected	0.950		0.000	0.950		0.000	0.950		0.000	0.950		0.000
Satd. Flow (prot)	3399	1844	1567	3467	1881	1599	1734	1825	1552	1743	1835	1560
Flt Permitted	0.950	1011	1007	0.950	1001	1000	0.073	1020	1002	0.073	1000	1000
Satd. Flow (perm)	3399	1844	1567	3467	1881	1599	133	1825	1552	134	1835	1560
Right Turn on Red	0000	1011	No	0401	1001	No	100	1020	No	10-1	1000	No
Satd. Flow (RTOR)			140			110			140			110
Link Speed (mph)		45			45			45			45	
Link Opeca (mpn) Link Distance (ft)		2183			2240			1168			1614	
Travel Time (s)		33.1			33.9			17.7			24.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
								786				
Adj. Flow (vph)	547	361	57	488	562	212	162	700	443	181	670	619
Shared Lane Traffic (%)	E 4.7	204	- 7	400	FC0	040	400	700	440	404	670	C40
Lane Group Flow (vph)	547	361	57	488	562	212	162	786	443	181	670	619
Turn Type	Prot	NA	pm+ov	Prot	NA	Free	D.P+P	NA	pm+ov	D.P+P	NA	pm+ov
Protected Phases	5	2	3	1	6	_	3	8	1	7	4	5
Permitted Phases	_	•	2	4	•	Free	4	•	8	8	4	4
Detector Phase	5	2	3	1	6		3	8	1	7	4	5
Switch Phase					40.0							
Minimum Initial (s)	7.0	12.0	7.0	7.0	12.0		7.0	7.0	7.0	7.0	7.0	7.0
Minimum Split (s)	14.0	19.0	14.0	14.0	19.0		14.0	14.0	14.0	14.0	14.0	14.0
Total Split (s)	25.0	42.0	14.0	29.0	46.0		14.0	60.0	29.0	14.0	60.0	25.0
Total Split (%)	17.2%	29.0%	9.7%	20.0%	31.7%		9.7%	41.4%	20.0%	9.7%	41.4%	17.2%
Maximum Green (s)	18.0	35.0	7.0	22.0	39.0		7.0	53.0	22.0	7.0	53.0	18.0
Yellow Time (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag	Lag	Lead	Lag	Lag	Lead		Lag	Lead	Lag	Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0	2.0	2.0	6.0		2.0	2.0	2.0	2.0	2.0	2.0
Minimum Gap (s)	2.0	3.0	2.0	2.0	3.0		2.0	2.0	2.0	2.0	2.0	2.0
Time Before Reduce (s)	0.0	15.0	0.0	0.0	15.0		0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce (s)	0.0	30.0	0.0	0.0	30.0		0.0	0.0	0.0	0.0	0.0	0.0
Recall Mode	None	Min	None	None	Min		None	None	None	None	None	None
Act Effct Green (s)	20.0	32.9	47.4	28.1	41.0	145.0	64.0	55.0	88.1	64.0	54.5	79.5
Actuated g/C Ratio	0.14	0.23	0.33	0.19	0.28	1.00	0.44	0.38	0.61	0.44	0.38	0.55
v/c Ratio	1.17	0.23	0.33	0.73	1.06	0.13	0.99	1.14	0.47	1.14	0.97	0.72
Control Delay	149.7	74.2	33.6	62.6	104.5	0.13	122.8	119.2	18.4	162.4	72.4	30.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

1: Kelly Road & Olive Chapel Road

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	149.7	74.2	33.6	62.6	104.5	0.2	122.8	119.2	18.4	162.4	72.4	30.5
LOS	F	Е	С	Ε	F	Α	F	F	В	F	Е	С
Approach Delay		114.6			70.8			87.5			65.8	
Approach LOS		F			Ε			F			Е	
Queue Length 50th (ft)	~315	326	37	228	~580	0	~112	~862	226	~148	616	421
Queue Length 95th (ft)	#435	441	70	#318	#811	0	#265	#1113	329	#309	#874	579
Internal Link Dist (ft)		2103			2160			1088			1534	
Turn Bay Length (ft)	300		200	325		350	250		250	325		500
Base Capacity (vph)	468	470	512	672	531	1599	164	692	943	159	696	854
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.17	0.77	0.11	0.73	1.06	0.13	0.99	1.14	0.47	1.14	0.96	0.72

Intersection Summary

Area Type: Other

Cycle Length: 145 Actuated Cycle Length: 145 Natural Cycle: 150

Control Type: Actuated-Uncoordinated

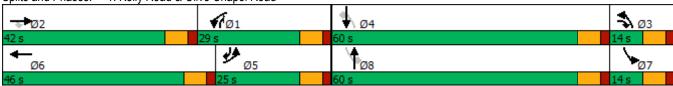
Maximum v/c Ratio: 1.17 Intersection Signal Delay: 82.2 Intersection Capacity Utilization 103.6%

Intersection LOS: F
ICU Level of Service G

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Kelly Road & Olive Chapel Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	, j	f)		¥	£		7	∱ }		¥	†	7
Traffic Volume (vph)	95	146	37	27	119	178	24	214	45	91	109	32
Future Volume (vph)	95	146	37	27	119	178	24	214	45	91	109	32
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			2%			6%			4%	
Storage Length (ft)	400		0	350		0	350		800	100		200
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Frt		0.970			0.910			0.974				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1789	0	1752	1678	0	1717	3344	0	1734	1825	1552
Flt Permitted	0.424			0.600			0.679			0.537		
Satd. Flow (perm)	782	1789	0	1106	1678	0	1227	3344	0	980	1825	1552
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1161			1113			2466			922	
Travel Time (s)		17.6			16.9			37.4			14.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	106	162	41	30	132	198	27	238	50	101	121	36
Shared Lane Traffic (%)												
Lane Group Flow (vph)	106	203	0	30	330	0	27	288	0	101	121	36
Turn Type	D.P+P	NA		D.P+P	NA		D.P+P	NA		D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8		7	4	5
Permitted Phases	6			2			4			8		4
Detector Phase	5	2		1	6		3	8		7	4	5
Switch Phase												
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0		7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0		14.0	14.0	14.0
Total Split (s)	15.0	60.0		15.0	60.0		15.0	25.0		15.0	25.0	15.0
Total Split (%)	13.0%	52.2%		13.0%	52.2%		13.0%	21.7%		13.0%	21.7%	13.0%
Maximum Green (s)	8.0	53.0		8.0	53.0		8.0	18.0		8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Lead/Lag	Lead	Lag		Lead	Lag		Lag	Lead		Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	6.0		2.0	6.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	3.0		2.0	3.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	5.0		0.0	5.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	15.0		0.0	15.0	0.0
Recall Mode	None	Min		None	Min		None	None		None	None	None
Act Effct Green (s)	34.7	33.6		36.6	27.8		26.5	18.1		27.9	23.1	37.1
Actuated g/C Ratio	0.45	0.43		0.47	0.36		0.34	0.23		0.36	0.30	0.48
v/c Ratio	0.22	0.26		0.05	0.55		0.06	0.37		0.23	0.22	0.05
Control Delay	12.5	18.0		10.9	26.6		19.2	30.4		21.9	28.0	12.1
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	12.5	18.0		10.9	26.6		19.2	30.4		21.9	28.0	12.1
LOS	В	В		В	С		В	С		С	С	В
Approach Delay		16.1			25.3			29.5			23.4	
Approach LOS		В			С			С			С	
Queue Length 50th (ft)	28	56		7	141		9	68		34	41	8
Queue Length 95th (ft)	56	136		21	235		29	120		79	114	26
Internal Link Dist (ft)		1081			1033			2386			842	
Turn Bay Length (ft)	400			350			350			100		200
Base Capacity (vph)	490	1277		619	1198		493	943		473	609	757
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.22	0.16		0.05	0.28		0.05	0.31		0.21	0.20	0.05

Intersection Summary

Area Type: Other

Cycle Length: 115

Actuated Cycle Length: 77.4

Natural Cycle: 65

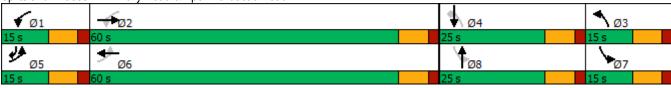
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.55 Intersection Signal Delay: 23.7 Intersection Capacity Utilization 52.9%

Intersection LOS: C
ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 2: Kelly Road & Apex Barbecue Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĥ		ሻ	f)		ሻ	∱ }		ሻ	†	7
Traffic Volume (vph)	65	98	41	49	106	163	50	224	58	135	262	78
Future Volume (vph)	65	98	41	49	106	163	50	224	58	135	262	78
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			2%			6%			4%	
Storage Length (ft)	400		0	350		0	350		800	100		200
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Frt		0.955			0.909			0.969				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1761	0	1752	1676	0	1717	3327	0	1734	1825	1552
Flt Permitted	0.437			0.652			0.408			0.508		
Satd. Flow (perm)	806	1761	0	1202	1676	0	737	3327	0	927	1825	1552
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1161			1113			2466			922	
Travel Time (s)		17.6			16.9			37.4			14.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	72	109	46	54	118	181	56	249	64	150	291	87
Shared Lane Traffic (%)												
Lane Group Flow (vph)	72	155	0	54	299	0	56	313	0	150	291	87
Turn Type	D.P+P	NA		D.P+P	NA		D.P+P	NA		D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8		7	4	5
Permitted Phases	6			2			4			8		4
Detector Phase	5	2		1	6		3	8		7	4	5
Switch Phase												
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0		7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0		14.0	14.0	14.0
Total Split (s)	15.0	60.0		15.0	60.0		15.0	25.0		15.0	25.0	15.0
Total Split (%)	13.0%	52.2%		13.0%	52.2%		13.0%	21.7%		13.0%	21.7%	13.0%
Maximum Green (s)	8.0	53.0		8.0	53.0		8.0	18.0		8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Lead/Lag	Lead	Lag		Lead	Lag		Lag	Lead		Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	6.0		2.0	6.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	3.0		2.0	3.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	5.0		0.0	5.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	15.0		0.0	15.0	0.0
Recall Mode	None	Min		None	Min		None	None		None	None	None
Act Effct Green (s)	32.2	28.2		33.2	25.3		31.1	17.9		28.4	23.0	33.9
Actuated g/C Ratio	0.40	0.35		0.42	0.32		0.39	0.22		0.36	0.29	0.42
v/c Ratio	0.16	0.25		0.10	0.56		0.14	0.42		0.34	0.55	0.13
Control Delay	12.8	21.2		12.2	28.4		19.5	30.4		22.5	34.4	12.2
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	12.8	21.2		12.2	28.4		19.5	30.4		22.5	34.4	12.2
LOS	В	С		В	С		В	С		С	С	В
Approach Delay		18.6			25.9			28.8			27.3	
Approach LOS		В			С			С			С	
Queue Length 50th (ft)	20	61		15	131		17	74		49	138	19
Queue Length 95th (ft)	42	110		34	215		47	125		105	#276	47
Internal Link Dist (ft)		1081			1033			2386			842	
Turn Bay Length (ft)	400			350			350			100		200
Base Capacity (vph)	453	1253		578	1192		422	860		446	525	675
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.16	0.12		0.09	0.25		0.13	0.36		0.34	0.55	0.13

Intersection Summary

Area Type: Other

Cycle Length: 115

Actuated Cycle Length: 79.9

Natural Cycle: 65

Control Type: Actuated-Uncoordinated

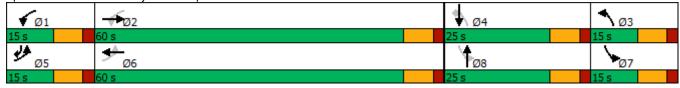
Maximum v/c Ratio: 0.56 Intersection Signal Delay: 26.0 Intersection Capacity Utilization 57.7%

Intersection LOS: C ICU Level of Service B

Analysis Period (min) 15

Queue shown is maximum after two cycles.

Splits and Phases: 2: Kelly Road & Apex Barbecue Road



^{# 95}th percentile volume exceeds capacity, queue may be longer.

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĥ		ሻ	ĥ		ሻ	∱ }		ሻ	<u></u>	7
Traffic Volume (vph)	111	171	43	32	139	252	28	285	53	120	163	37
Future Volume (vph)	111	171	43	32	139	252	28	285	53	120	163	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			2%			6%			4%	
Storage Length (ft)	400		0	350		0	350		800	100		200
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Frt		0.970			0.903			0.976				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1789	0	1752	1665	0	1717	3351	0	1734	1825	1552
Flt Permitted	0.317		•	0.556		•	0.560		•	0.392		
Satd. Flow (perm)	585	1789	0	1025	1665	0	1012	3351	0	716	1825	1552
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1161			1113			2466			922	
Travel Time (s)		17.6			16.9			37.4			14.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	123	190	48	36	154	280	31	317	59	133	181	41
Shared Lane Traffic (%)							-	•	-			
Lane Group Flow (vph)	123	238	0	36	434	0	31	376	0	133	181	41
Turn Type	D.P+P	NA		D.P+P	NA		D.P+P	NA		D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8		7	4	5
Permitted Phases	6			2			4			8		4
Detector Phase	5	2		1	6		3	8		7	4	5
Switch Phase												
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0		7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0		14.0	14.0	14.0
Total Split (s)	15.0	60.0		15.0	60.0		15.0	25.0		15.0	25.0	15.0
Total Split (%)	13.0%	52.2%		13.0%	52.2%		13.0%	21.7%		13.0%	21.7%	13.0%
Maximum Green (s)	8.0	53.0		8.0	53.0		8.0	18.0		8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Lead/Lag	Lead	Lag		Lead	Lag		Lag	Lead		Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	6.0		2.0	6.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	3.0		2.0	3.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	5.0		0.0	5.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	15.0		0.0	15.0	0.0
Recall Mode	None	Min		None	Min		None	None		None	None	None
Act Effct Green (s)	44.3	41.5		46.6	34.7		30.4	18.8		28.1	25.0	36.9
Actuated g/C Ratio	0.48	0.45		0.50	0.37		0.33	0.20		0.30	0.27	0.40
v/c Ratio	0.31	0.30		0.06	0.70		0.08	0.55		0.42	0.37	0.07
Control Delay	12.9	19.0		10.4	31.0		23.7	38.1		31.9	35.2	15.7
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	12.9	19.0		10.4	31.0		23.7	38.1		31.9	35.2	15.7
LOS	В	В		В	С		С	D		С	D	В
Approach Delay		17.0			29.4			37.0			31.7	
Approach LOS		В			С			D			С	
Queue Length 50th (ft)	35	99		10	215		12	103		53	95	11
Queue Length 95th (ft)	63	158		24	324		37	178		116	187	35
Internal Link Dist (ft)		1081			1033			2386			842	
Turn Bay Length (ft)	400			350			350			100		200
Base Capacity (vph)	410	1075		600	1001		411	732		334	496	626
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.30	0.22		0.06	0.43		0.08	0.51		0.40	0.36	0.07

Intersection Summary

Area Type: Other

Cycle Length: 115

Actuated Cycle Length: 92.7

Natural Cycle: 65

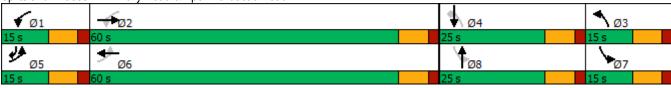
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.70 Intersection Signal Delay: 29.0 Intersection Capacity Utilization 61.8%

Intersection LOS: C ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 2: Kelly Road & Apex Barbecue Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĥ		ሻ	f)		ሻ	∱ }		ሻ	†	7
Traffic Volume (vph)	76	115	48	57	124	210	58	289	68	190	343	91
Future Volume (vph)	76	115	48	57	124	210	58	289	68	190	343	91
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			2%			6%			4%	
Storage Length (ft)	400		0	350		0	350		800	100		200
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Frt		0.956			0.906			0.971				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1763	0	1752	1671	0	1717	3333	0	1734	1825	1552
Flt Permitted	0.359			0.614			0.216			0.396		
Satd. Flow (perm)	662	1763	0	1132	1671	0	390	3333	0	723	1825	1552
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1161			1113			2466			922	
Travel Time (s)		17.6			16.9			37.4			14.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	84	128	53	63	138	233	64	321	76	211	381	101
Shared Lane Traffic (%)												
Lane Group Flow (vph)	84	181	0	63	371	0	64	397	0	211	381	101
Turn Type	D.P+P	NA		D.P+P	NA		D.P+P	NA		D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8		7	4	5
Permitted Phases	6			2			4			8		4
Detector Phase	5	2		1	6		3	8		7	4	5
Switch Phase												
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0		7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0		14.0	14.0	14.0
Total Split (s)	15.0	60.0		15.0	60.0		15.0	25.0		15.0	25.0	15.0
Total Split (%)	13.0%	52.2%		13.0%	52.2%		13.0%	21.7%		13.0%	21.7%	13.0%
Maximum Green (s)	8.0	53.0		8.0	53.0		8.0	18.0		8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Lead/Lag	Lead	Lag		Lead	Lag		Lag	Lead		Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	6.0		2.0	6.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	3.0		2.0	3.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	5.0		0.0	5.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	15.0		0.0	15.0	0.0
Recall Mode	None	Min		None	Min		None	None		None	None	None
Act Effet Green (s)	39.0	33.1		40.2	29.7		31.0	19.1		28.9	22.5	33.1
Actuated g/C Ratio	0.44	0.38		0.46	0.34		0.35	0.22		0.33	0.26	0.38
v/c Ratio	0.21	0.27		0.11	0.66		0.23	0.55		0.61	0.82	0.17
Control Delay	12.8	21.8		11.8	31.0		27.1	35.3		35.2	50.6	14.4
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	12.8	21.8		11.8	31.0		27.1	35.3		35.2	50.6	14.4
LOS	В	С		В	С		С	D		D	D	В
Approach Delay		18.9			28.2			34.1			40.6	
Approach LOS		В			С			С			D	
Queue Length 50th (ft)	24	74		18	176		22	103		80	209	25
Queue Length 95th (ft)	47	125		37	271		58	173		#165	#450	63
Internal Link Dist (ft)		1081			1033			2386			842	
Turn Bay Length (ft)	400			350			350			100		200
Base Capacity (vph)	423	1111		596	1053		291	764		355	466	595
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.20	0.16		0.11	0.35		0.22	0.52		0.59	0.82	0.17

Intersection Summary

Area Type: Other

Cycle Length: 115

Actuated Cycle Length: 88.1

Natural Cycle: 75

Control Type: Actuated-Uncoordinated

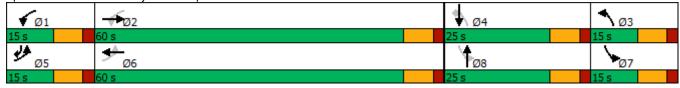
Maximum v/c Ratio: 0.82 Intersection Signal Delay: 33.0 Intersection Capacity Utilization 65.8%

Intersection LOS: C ICU Level of Service C

Analysis Period (min) 15

Queue shown is maximum after two cycles.

Splits and Phases: 2: Kelly Road & Apex Barbecue Road



^{# 95}th percentile volume exceeds capacity, queue may be longer.

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	, j	£		, j	£		¥	↑ ↑		7	†	7
Traffic Volume (vph)	111	171	56	44	139	252	35	309	61	120	198	37
Future Volume (vph)	111	171	56	44	139	252	35	309	61	120	198	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			2%			6%			4%	
Storage Length (ft)	400		0	350		0	350		800	100		200
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Frt		0.963			0.903			0.975				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1776	0	1752	1665	0	1717	3347	0	1734	1825	1552
Flt Permitted	0.314			0.539			0.494			0.354		
Satd. Flow (perm)	579	1776	0	994	1665	0	893	3347	0	646	1825	1552
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1161			1113			2466			922	
Travel Time (s)		17.6			16.9			37.4			14.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	123	190	62	49	154	280	39	343	68	133	220	41
Shared Lane Traffic (%)												
Lane Group Flow (vph)	123	252	0	49	434	0	39	411	0	133	220	41
Turn Type	D.P+P	NA		D.P+P	NA		D.P+P	NA		D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8		7	4	5
Permitted Phases	6			2			4			8		4
Detector Phase	5	2		1	6		3	8		7	4	5
Switch Phase												
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0		7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0		14.0	14.0	14.0
Total Split (s)	15.0	60.0		15.0	60.0		15.0	25.0		15.0	25.0	15.0
Total Split (%)	13.0%	52.2%		13.0%	52.2%		13.0%	21.7%		13.0%	21.7%	13.0%
Maximum Green (s)	8.0	53.0		8.0	53.0		8.0	18.0		8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Lead/Lag	Lead	Lag		Lead	Lag		Lag	Lead		Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	6.0		2.0	6.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	3.0		2.0	3.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	5.0		0.0	5.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	15.0		0.0	15.0	0.0
Recall Mode	None	Min		None	Min		None	None		None	None	None
Act Effct Green (s)	44.4	41.5		46.6	34.7		30.9	19.4		28.7	25.7	37.5
Actuated g/C Ratio	0.48	0.44		0.50	0.37		0.33	0.21		0.31	0.28	0.40
v/c Ratio	0.31	0.32		0.09	0.70		0.10	0.59		0.43	0.44	0.07
Control Delay	13.1	19.5		10.7	31.4		24.2	38.9		33.1	36.2	15.7
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
- Guodo Dolay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	13.1	19.5		10.7	31.4		24.2	38.9		33.1	36.2	15.7
LOS	В	В		В	С		С	D		С	D	В
Approach Delay		17.4			29.3			37.6			33.0	
Approach LOS		В			С			D			С	
Queue Length 50th (ft)	35	106		13	215		15	114		53	116	11
Queue Length 95th (ft)	63	168		30	324		43	194		116	226	35
Internal Link Dist (ft)		1081			1033			2386			842	
Turn Bay Length (ft)	400			350			350			100		200
Base Capacity (vph)	405	1059		584	993		388	726		322	501	632
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.30	0.24		0.08	0.44		0.10	0.57		0.41	0.44	0.06

Intersection Summary

Area Type: Other

Cycle Length: 115

Actuated Cycle Length: 93.3

Natural Cycle: 65

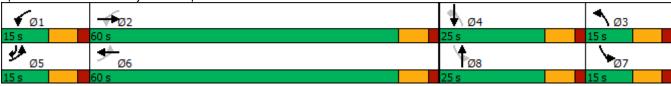
Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.70 Intersection Signal Delay: 29.7 Intersection Capacity Utilization 62.7%

Intersection LOS: C ICU Level of Service B

Analysis Period (min) 15





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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	f)		ሻ	f)		ሻ	↑ ↑		ሻ	†	7
Traffic Volume (vph)	76	115	57	66	124	210	69	324	80	190	370	91
Future Volume (vph)	76	115	57	66	124	210	69	324	80	190	370	91
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			2%			6%			4%	
Storage Length (ft)	400		0	350		0	350		800	100		200
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Frt		0.951			0.906			0.970				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1754	0	1752	1671	0	1717	3330	0	1734	1825	1552
Flt Permitted	0.356		•	0.599			0.174		·	0.345		
Satd. Flow (perm)	657	1754	0	1105	1671	0	314	3330	0	630	1825	1552
Right Turn on Red	001		No	1100		No	011	0000	No	000	1020	No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1161			1113			2466			922	
Travel Time (s)		17.6			16.9			37.4			14.0	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	84	128	63	73	138	233	77	360	89	211	411	101
Shared Lane Traffic (%)	01	120	00	10	100	200		000	00		• • • • • • • • • • • • • • • • • • • •	101
Lane Group Flow (vph)	84	191	0	73	371	0	77	449	0	211	411	101
Turn Type	D.P+P	NA	v	D.P+P	NA	v	D.P+P	NA	Ū	D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8		7	4	5
Permitted Phases	6	_		2	·		4	Ū		8		4
Detector Phase	5	2		1	6		3	8		7	4	5
Switch Phase	U	_			Ū		J	O		,		O
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0		7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0		14.0	14.0	14.0
Total Split (s)	15.0	60.0		15.0	60.0		15.0	25.0		15.0	25.0	15.0
Total Split (%)	13.0%	52.2%		13.0%	52.2%		13.0%	21.7%		13.0%	21.7%	13.0%
Maximum Green (s)	8.0	53.0		8.0	53.0		8.0	18.0		8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Lead/Lag	Lead	Lag		Lead	Lag		Lag	Lead		Lag	Lead	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	6.0		2.0	6.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	3.0		2.0	3.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	5.0		0.0	5.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	15.0		0.0	15.0	0.0
Recall Mode	None	Min		None 40.2	Min		None	None		None 29.7	None 23.0	None
Act Effct Green (s)	39.1	33.0			29.7		31.1	19.7				33.5
Actuated g/C Ratio	0.44	0.37		0.45	0.33		0.35	0.22		0.33	0.26	0.38
v/c Ratio	0.21	0.29		0.13	0.66		0.29	0.61		0.63	0.87	0.17
Control Delay	12.9	22.2		12.1	31.4		30.9	36.5		37.8	55.8	14.3
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	12.9	22.2		12.1	31.4		30.9	36.5		37.8	55.8	14.3
LOS	В	С		В	С		С	D		D	Е	В
Approach Delay		19.4			28.2			35.6			44.7	
Approach LOS		В			С			D			D	
Queue Length 50th (ft)	24	79		21	176		27	118		80	231	25
Queue Length 95th (ft)	47	132		42	271		67	196		#179	#494	63
Internal Link Dist (ft)		1081			1033			2386			842	
Turn Bay Length (ft)	400			350			350			100		200
Base Capacity (vph)	418	1092		580	1041		269	754		335	472	598
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.20	0.17		0.13	0.36		0.29	0.60		0.63	0.87	0.17

Intersection Summary

Area Type: Other

Cycle Length: 115

Actuated Cycle Length: 88.9

Natural Cycle: 75

Control Type: Actuated-Uncoordinated

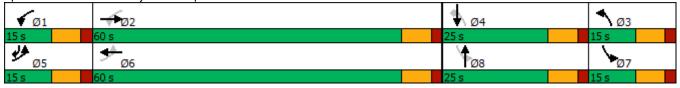
Maximum v/c Ratio: 0.87 Intersection Signal Delay: 35.0 Intersection Capacity Utilization 67.2%

Intersection LOS: D ICU Level of Service C

Analysis Period (min) 15

Queue shown is maximum after two cycles.

Splits and Phases: 2: Kelly Road & Apex Barbecue Road



^{# 95}th percentile volume exceeds capacity, queue may be longer.

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	f)		7	f)		7	∱ ∱		7	†	7
Traffic Volume (vph)	120	274	47	34	175	292	30	306	57	218	173	40
Future Volume (vph)	120	274	47	34	175	292	30	306	57	218	173	40
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			2%			6%			4%	
Storage Length (ft)	400		0	350		0	350		800	100		200
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Frt		0.978			0.906			0.977				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1804	0	1752	1671	0	1717	3354	0	1734	1825	1552
Flt Permitted	0.229			0.400			0.525			0.339		
Satd. Flow (perm)	422	1804	0	738	1671	0	949	3354	0	619	1825	1552
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1161			1113			913			906	
Travel Time (s)		17.6			16.9			13.8			13.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	133	304	52	38	194	324	33	340	63	242	192	44
Shared Lane Traffic (%)												
Lane Group Flow (vph)	133	356	0	38	518	0	33	403	0	242	192	44
Turn Type	D.P+P	NA		D.P+P	NA		D.P+P	NA		D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8		7	4	5
Permitted Phases	6			2			4			8		4
Detector Phase	5	2		1	6		3	8		7	4	5
Switch Phase												
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0		7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0		14.0	14.0	14.0
Total Split (s)	15.0	60.0		15.0	60.0		15.0	25.0		15.0	25.0	15.0
Total Split (%)	13.0%	52.2%		13.0%	52.2%		13.0%	21.7%		13.0%	21.7%	13.0%
Maximum Green (s)	8.0	53.0		8.0	53.0		8.0	18.0		8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Lead/Lag	Lag	Lead		Lag	Lead		Lag	Lead		Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	6.0		2.0	6.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	3.0		2.0	3.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	5.0		0.0	5.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	15.0		0.0	15.0	0.0
Recall Mode	None	Min		None	Min		None	None		None	None	None
Act Effct Green (s)	50.7	41.1		52.9	41.3		31.7	19.4		29.5	26.1	40.5
Actuated g/C Ratio	0.50	0.41		0.53	0.41		0.32	0.19		0.29	0.26	0.40
v/c Ratio	0.39	0.48		0.07	0.75		0.09	0.62		0.82	0.41	0.07
Control Delay	19.3	27.8		10.6	32.7		26.9	43.5		60.2	39.1	25.4
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	19.3	27.8		10.6	32.7		26.9	43.5		60.2	39.1	25.4
LOS	В	С		В	С		С	D		Ε	D	С
Approach Delay		25.5			31.2			42.2			48.5	
Approach LOS		С			С			D			D	
Queue Length 50th (ft)	39	196		11	276		14	124		115	110	19
Queue Length 95th (ft)	66	295		25	403		41	204		#305	212	52
Internal Link Dist (ft)		1081			1033			833			826	
Turn Bay Length (ft)	400			350			350			100		200
Base Capacity (vph)	350	998		553	925		377	675		294	474	573
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.38	0.36		0.07	0.56		0.09	0.60		0.82	0.41	0.08

Intersection Summary

Area Type: Other

Cycle Length: 115

Actuated Cycle Length: 100.4

Natural Cycle: 70

Control Type: Actuated-Uncoordinated

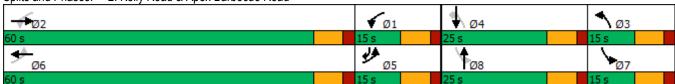
Maximum v/c Ratio: 0.82 Intersection Signal Delay: 36.4 Intersection Capacity Utilization 72.8%

Intersection LOS: D ICU Level of Service C

Analysis Period (min) 15

Queue shown is maximum after two cycles.

Splits and Phases: 2: Kelly Road & Apex Barbecue Road



^{# 95}th percentile volume exceeds capacity, queue may be longer.

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	(î		ሻ	1		ሻ	↑ ↑		ሻ	1	7
Traffic Volume (vph)	82	171	52	62	227	317	63	310	73	251	368	99
Future Volume (vph)	82	171	52	62	227	317	63	310	73	251	368	99
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			2%			6%			4%	
Storage Length (ft)	400		0	350		0	350		800	100		200
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Frt		0.965			0.913			0.971				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1780	0	1752	1684	0	1717	3333	0	1734	1825	1552
FIt Permitted	0.187			0.468			0.173			0.303		
Satd. Flow (perm)	345	1780	0	863	1684	0	313	3333	0	553	1825	1552
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1161			1113			913			906	
Travel Time (s)		17.6			16.9			13.8			13.7	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	91	190	58	69	252	352	70	344	81	279	409	110
Shared Lane Traffic (%)												
Lane Group Flow (vph)	91	248	0	69	604	0	70	425	0	279	409	110
Turn Type	D.P+P	NA		D.P+P	NA		D.P+P	NA		D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8		7	4	5
Permitted Phases	6	_		2			4	-		8	-	4
Detector Phase	5	2		1	6		3	8		7	4	5
Switch Phase	•	_		·	•		•	•		•	·	•
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0		7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0		14.0	14.0	14.0
Total Split (s)	15.0	60.0		15.0	60.0		15.0	25.0		15.0	25.0	15.0
Total Split (%)	13.0%	52.2%		13.0%	52.2%		13.0%	21.7%		13.0%	21.7%	13.0%
Maximum Green (s)	8.0	53.0		8.0	53.0		8.0	18.0		8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Lead/Lag	Lag	Lead		Lag	Lead		Lag	Lead		Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	6.0		2.0	6.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	3.0		2.0	3.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	5.0		0.0	5.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	15.0		0.0	15.0	0.0
Recall Mode	None	Min		None	Min		None	None		None	None	None
Act Effct Green (s)	56.6	32.4		57.8	46.7		31.2	19.6		29.6	23.1	38.0
Actuated g/C Ratio	0.53	0.30		0.54	0.44		0.29	0.18		0.28	0.22	0.36
v/c Ratio	0.33	0.30		0.34	0.44		0.29	0.18		1.05	1.03	0.30
Control Delay	16.4	37.0		10.5	36.1		41.4	48.7		110.8	97.9	29.1
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	16.4	37.0		10.5	36.1		41.4	48.7		110.8	97.9	29.1
LOS	В	D		В	D		D	D		F	F	С
Approach Delay		31.5			33.4			47.7			92.9	
Approach LOS		С			С			D			F	
Queue Length 50th (ft)	26	156		20	353		35	148		~175	~355	56
Queue Length 95th (ft)	48	229		38	504		73	216		#396	#582	109
Internal Link Dist (ft)		1081			1033			833			826	
Turn Bay Length (ft)	400			350			350			100		200
Base Capacity (vph)	317	928		700	878		225	631		265	396	540
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.29	0.27		0.10	0.69		0.31	0.67		1.05	1.03	0.20

Intersection Summary

Area Type: Other

Cycle Length: 115

Actuated Cycle Length: 106.4

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

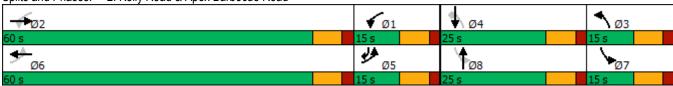
Maximum v/c Ratio: 1.05 Intersection Signal Delay: 56.8 Intersection Capacity Utilization 79.1%

Intersection LOS: E ICU Level of Service D

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 2: Kelly Road & Apex Barbecue Road



Lane Configurations \\ \bar{\bar{\bar{\bar{\bar{\bar{\bar{	SBR 7 40
	40
Future Volume (vph) 120 274 170 156 175 292 110 552 138 218 539	40
	1900
Grade (%) 2% 2% 6% 4%	
Storage Length (ft) 400 0 350 0 350 800 100 20	200
Storage Lanes 1 0 1 1 1 1	1
Taper Length (ft) 100 100 100 100	
Lane Util. Factor 1.00 1.00 1.00 1.00 1.00 1.00 0.95 0.95 1.00 1.00 1.00	1.00
Frt 0.942 0.906 0.970 0.85	.850
Flt Protected 0.950 0.950 0.950 0.950	
Satd. Flow (prot) 1752 1737 0 1752 1671 0 1717 3330 0 1734 1825 155	1552
Flt Permitted 0.255 0.261 0.198 0.198	
Satd. Flow (perm) 470 1737 0 481 1671 0 358 3330 0 361 1825 155	1552
Right Turn on Red No No No No	No
Satd. Flow (RTOR)	
Link Speed (mph) 45 45 45 45	
Link Distance (ft) 1161 1113 2992 1087	
Travel Time (s) 17.6 16.9 45.3 16.5	
Peak Hour Factor 0.90 0.90 0.90 0.90 0.90 0.90 0.90 0.9	0.90
Adj. Flow (vph) 133 304 189 173 194 324 122 613 153 242 599	44
Shared Lane Traffic (%)	
	44
Turn Type D.P+P NA D.P+P NA D.P+P NA D.P+P NA pm+c	1+0V
Protected Phases 5 2 1 6 3 8 7 4	5
Permitted Phases 6 2 4 8	4
Detector Phase 5 2 1 6 3 8 7 4	5
Switch Phase	
	7.0
$\mathcal{N}_{\mathcal{N}}$	14.0
	15.0
	3.0%
	8.0
	5.0
	2.0
	-2.0
	5.0
	Lag
	Yes
· ·	2.0
	2.0
	0.0
	0.0
	lone
	34.6
	0.34
· · · · · · · · · · · · · · · · · · ·	0.08
	26.4
•	0.0

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	17.9	32.6		20.9	33.1		44.1	123.9		97.7	333.3	26.4
LOS	В	С		С	С		D	F		F	F	С
Approach Delay		29.5			30.1			113.0			253.6	
Approach LOS		С			С			F			F	
Queue Length 50th (ft)	39	263		52	276		54	~304		115	~556	19
Queue Length 95th (ft)	66	373		85	403		118	#511		#346	#900	52
Internal Link Dist (ft)		1081			1033			2912			1007	
Turn Bay Length (ft)	400			350			350			100		200
Base Capacity (vph)	366	952		382	916		242	663		244	363	477
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.36	0.52		0.45	0.57		0.50	1.16		0.99	1.65	0.09

Intersection Summary

Area Type: Other

Cycle Length: 115

Actuated Cycle Length: 101.2

Natural Cycle: 100

Control Type: Actuated-Uncoordinated

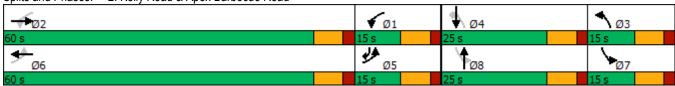
Maximum v/c Ratio: 1.65
Intersection Signal Delay: 117.8
Intersection Capacity Utilization 84.9%

Intersection LOS: F
ICU Level of Service E

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 2: Kelly Road & Apex Barbecue Road



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	ĵ.		ሻ	ĵ.		ሻ	↑ ↑		ሻ	†	7
Traffic Volume (vph)	82	171	185	195	227	317	216	775	227	251	767	99
Future Volume (vph)	82	171	185	195	227	317	216	775	227	251	767	99
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Grade (%)		2%			2%			6%			4%	
Storage Length (ft)	400		0	350		0	350		800	100		200
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Frt		0.922			0.913			0.966				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1700	0	1752	1684	0	1717	3316	0	1734	1825	1552
FIt Permitted	0.200			0.312			0.199			0.199		
Satd. Flow (perm)	369	1700	0	575	1684	0	360	3316	0	363	1825	1552
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		45			45			45			45	
Link Distance (ft)		1161			1113			2992			1087	
Travel Time (s)		17.6			16.9			45.3			16.5	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	91	190	206	217	252	352	240	861	252	279	852	110
Shared Lane Traffic (%)												
Lane Group Flow (vph)	91	396	0	217	604	0	240	1113	0	279	852	110
Turn Type	D.P+P	NA		D.P+P	NA		D.P+P	NA		D.P+P	NA	pm+ov
Protected Phases	5	2		1	6		3	8		7	4	· 5
Permitted Phases	6			2			4			8		4
Detector Phase	5	2		1	6		3	8		7	4	5
Switch Phase												
Minimum Initial (s)	7.0	12.0		7.0	12.0		7.0	7.0		7.0	7.0	7.0
Minimum Split (s)	14.0	19.0		14.0	19.0		14.0	14.0		14.0	14.0	14.0
Total Split (s)	15.0	60.0		15.0	60.0		15.0	25.0		15.0	25.0	15.0
Total Split (%)	13.0%	52.2%		13.0%	52.2%		13.0%	21.7%		13.0%	21.7%	13.0%
Maximum Green (s)	8.0	53.0		8.0	53.0		8.0	18.0		8.0	18.0	8.0
Yellow Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0		-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Lead/Lag	Lag	Lead		Lag	Lead		Lag	Lead		Lag	Lead	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	6.0		2.0	6.0		2.0	6.0		2.0	6.0	2.0
Minimum Gap (s)	2.0	3.0		2.0	3.0		2.0	3.0		2.0	3.0	2.0
Time Before Reduce (s)	0.0	15.0		0.0	15.0		0.0	5.0		0.0	5.0	0.0
Time To Reduce (s)	0.0	30.0		0.0	30.0		0.0	15.0		0.0	15.0	0.0
Recall Mode	None	Min		None	Min		None	None		None	None	None
Act Effct Green (s)	56.6	36.4		56.6	46.8		30.2	20.1		30.2	20.1	35.0
Actuated g/C Ratio	0.53	0.34		0.53	0.44		0.28	0.19		0.28	0.19	0.33
v/c Ratio	0.28	0.69		0.41	0.82		1.05	1.78		1.21	2.48	0.22
Control Delay	15.8	36.5		19.0	36.5		116.2	387.6		165.5	697.2	29.7
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0

	۶	-	•	•	•	•	4	†	~	>	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Total Delay	15.8	36.5		19.0	36.5		116.2	387.6		165.5	697.2	29.7
LOS	В	D		В	D		F	F		F	F	С
Approach Delay		32.7			31.9			339.5			518.5	
Approach LOS		С			С			F			F	
Queue Length 50th (ft)	26	240		67	353		~147	~625		~203	~1005	56
Queue Length 95th (ft)	48	314		105	504		#345	#803		#391	#1312	109
Internal Link Dist (ft)		1081			1033			2912			1007	
Turn Bay Length (ft)	400			350			350			100		200
Base Capacity (vph)	326	880		527	871		229	624		231	343	494
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.28	0.45		0.41	0.69		1.05	1.78		1.21	2.48	0.22

Intersection Summary

Area Type: Other

Cycle Length: 115 Actuated Cycle Length: 107 Natural Cycle: 150

Control Type: Actuated-Uncoordinated

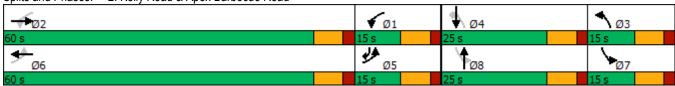
Maximum v/c Ratio: 2.48 Intersection Signal Delay: 293.4 Intersection Capacity Utilization 106.2%

Intersection LOS: F
ICU Level of Service G

Analysis Period (min) 15

- Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
- # 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 2: Kelly Road & Apex Barbecue Road



Intersection												
Int Delay, s/veh	2.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स	7		ર્ન	7	ሻ	†	7	ሻ	†	7
Traffic Vol, veh/h	24	4	8	17	4	7	7	178	4	13	145	4
Future Vol, veh/h	24	4	8	17	4	7	7	178	4	13	145	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	- -	-	None	- -	- -	None	-	-	None	- 1	-	None
Storage Length	_	_	100	_	_	100	150	_	50	125	_	50
Veh in Median Storage	e.# -	0	-	_	0	-	-	0	-	-	0	-
Grade, %	<i>-</i>	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	90	90	90	90	90	90	90	90	90		90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2		2	2
Mymt Flow	27	4	9	19	4	8	8	198	4	14	161	4
WALLE TOW	21	7	3	13	7	J	0	100	7	17	101	7
Major/Minor	Minor2			Minor1		ı	Major1			Major2		
Conflicting Flow All	411	407	161	412	407	198	165	0	0		0	0
Stage 1	189	189	-	214	214	130	100	U	U	202	U	U
Stage 2	222	218	-	198	193	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	0.22	6.12	5.52	0.22	4.12	-	-	4.12	-	_
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	_	-	-	-	-	-	-
, ,	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Follow-up Hdwy Pot Cap-1 Maneuver		533	884	550	533	843	1413	-	-	1370	-	-
	551					043	1413	-	-	1370	-	-
Stage 1	813	744	-	788	725	-	-	-	-	-	-	-
Stage 2	780	723	-	804	741	-	-	-	-	-	-	-
Platoon blocked, %	ESC	ΕOΛ	001	ESE	E01	042	1//12	-	-	1370	-	-
Mov Cap-1 Maneuver	536	524	884	535 535	524 524	843	1413	-	-	13/0	-	-
Mov Cap-2 Maneuver	536	524	-	535	524	-	-	-	-	-	-	-
Stage 1	808	737	-	783	721	-	-	-	-	-	-	-
Stage 2	764	719	-	783	734	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	11.5			11.4			0.3			0.6		
HCM LOS	П.5			В			0.0			0.0		
	J			5								
Minor Lane/Major Mvn	nt	NBL	NBT	NBR	EBLn1	EBLn2\	VBLn1V	VBLn2	SBL	SBT	SBR	
Capacity (veh/h)		1413	-	-	534	884	533	843	1370	-	-	
HCM Lane V/C Ratio		0.006	_	_	0.058	0.01		0.009		_	_	
HCM Control Delay (s)	7.6	_	_	12.2	9.1	12.1	9.3	7.7	_	_	
HCM Lane LOS	,	Α	_	_	В	A	В	A	Α		_	
HCM 95th %tile Q(veh	1)	0	_	_	0.2	0	0.1	0	0		_	
3 2.2 /0 a/1011	,	•				J	•	J	•			

Intersection												
Int Delay, s/veh	1.7											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स	7		ર્ન	7	ሻ	↑	7	ሻ	†	7
Traffic Vol, veh/h	21	4	9	8	4	6	12	261	13		221	4
Future Vol, veh/h	21	4	9	8	4	6	12	261	13		221	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free		Free	Free
RT Channelized	Stop -	Stop	None	Stop -	Stop -	None	-	-	None	-	-	None
Storage Length	-	-	100	-	-	100	150	-	50		-	50
	_ ш -	-		-	-			-		123	_	50
Veh in Median Storage	∋,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-		0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90		90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2		2	2
Mvmt Flow	23	4	10	9	4	7	13	290	14	31	246	4
Major/Minor	Minor2			Minor1		į	Major1			Major2		
Conflicting Flow All	637	638	246	633	628	290	250	0	0	304	0	0
Stage 1	308	308	_	316	316	_	-	_	_	_	_	_
Stage 2	329	330	_	317	312	_	_	_	_	_	_	_
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	_	_	4.12	_	_
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	0.22	2	_	_		_	_
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52							
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	_	_	2.218	_	_
Pot Cap-1 Maneuver	390	394	793	392	4.010	749	1316	-	-	1257	-	-
						749	1310	-	-	1237	-	-
Stage 1	702	660	-	695	655	-	-	-	-	-	-	-
Stage 2	684	646	-	694	658	-	-	-	-	-	-	-
Platoon blocked, %	070	000		o= :	000	- 46	1010	-	-	40==	-	-
Mov Cap-1 Maneuver	373	380	793	374	386	749	1316	-	-	1257	-	-
Mov Cap-2 Maneuver	373	380	-	374	386	-	-	-	-	-	-	-
Stage 1	695	644	-	688	648	-	-	-	-	-	-	-
Stage 2	667	640	-	664	642	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	13.9			13.2			0.3			0.9		
HCM LOS	В			В			0.0			0.0		
HOW LOO	ט			ט								
Minan Lang (NA) 1 - NA	-1	NDI	NOT	NDD	EDL 4	EDL A	MDL A	NDL 0	ODI	007	000	
Minor Lane/Major Mvn	nt	NBL	NBT	NBK		EBLn2\			SBL	SBT	SBR	
Capacity (veh/h)		1316	-	-	374	793	378	749	1257	-	-	
HCM Lane V/C Ratio		0.01	-	-		0.013		0.009		-	-	
HCM Control Delay (s))	7.8	-	-	15.4	9.6	14.9	9.9	7.9	-	-	
HCM Lane LOS		Α	-	-	С	Α	В	Α	Α	-	-	
HCM 95th %tile Q(veh	1)	0	-	-	0.2	0	0.1	0	0.1	-	-	
·												

Intersection												
Int Delay, s/veh	2.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7		4	7	ሻ	†	7	ሻ	†	7
Traffic Vol, veh/h	28	4	9	20	4	8	8	208	4		170	4
Future Vol, veh/h	28	4	9	20	4	8	8	208	4	4	170	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	_	-	None	_	-	None
Storage Length	_	_	100	_	_	100	150	_	50	125	_	50
Veh in Median Storage	.# -	0	_	_	0	-	-	0	_	_	0	-
Grade, %	, -	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2		2	2
Mvmt Flow	31	4	10	22	4	9	9	231	4		189	4
				_ _		,	,		·	•		•
Major/Minor I	Minor2			Minor1		I	Major1			Major2		
Conflicting Flow All	455	450	189	455	450	231	193	0	0	235	0	0
Stage 1	197	197	109	249	249	231	133	U	U	233	U	U
Stage 1 Stage 2	258	253	-	249	249	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52		6.12	5.52	0.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
, ,	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Follow-up Hdwy								-	-	1332	-	-
Pot Cap-1 Maneuver	515	504 738	853	515 755	504 701	808	1380	-	-	1332	-	-
Stage 1	805		-	755 706	701	-	-	-	-	-	-	-
Stage 2	747	698	-	796	735	-	-	-	-	-	-	-
Platoon blocked, %	E00	400	0.50	E00	400	000	1200	-	-	1220	-	-
Mov Cap-1 Maneuver	502	499	853	502	499	808	1380	-	-	1332	-	-
Mov Cap-2 Maneuver	502	499	-	502	499	-	-	-	-	-	-	-
Stage 1	799	736	-	750	696	-	-	-	-	-	-	-
Stage 2	729	693	-	780	733	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
Approach												
HCM Control Delay, s	12			11.8			0.3			0.2		
HCM LOS	В			В								
Minor Lane/Major Mys	, ‡	NBL	NDT	NDD	EDI 51	בטו אט	MDI 541	MDI 52	C DI	SBT	SBR	
Minor Lane/Major Mvm	IL		NBT	INDK		EBLn2\			SBL	SDI	SDK	
Capacity (veh/h)		1380	-	-	502	853	501	808	1332	-	-	
HCM Cantral Dalay (a)		0.006	-	-	0.0.		0.053			-	-	
HCM Control Delay (s)		7.6	-	-	12.7	9.3	12.6	9.5	7.7	-	-	
HCM Lane LOS		A	-	-	В	A	В	A	A	-	-	
HCM 95th %tile Q(veh))	0	-	-	0.2	0	0.2	0	0	-	-	

Intersection												
Int Delay, s/veh	1.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स	7		4	7	ሻ		7	ሻ	1	7
Traffic Vol, veh/h	25	4	11	9	4	7	14	305	15	33	259	4
Future Vol, veh/h	25	4	11	9	4	7	14	305	15	33	259	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	Olop	Otop -	None	Otop -	Olop -	None	-	-	None	-	-	None
Storage Length	_	_	100	_	_	100	150	_	50	125	_	50
Veh in Median Storage	`#	0	100	_	0	-	130	0	-	123	0	30
Grade, %	;, # -	0	-	-	0	-				-	0	-
	-		-	-			-	0	-	-		-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	28	4	12	10	4	8	16	339	17	37	288	4
Major/Minor	Minor2			Minor1			Major1		1	Major2		
		750			707			^			^	
Conflicting Flow All	748	750	288	743	737	339	292	0	0	356	0	0
Stage 1	362	362	-	371	371	-	-	-	-	-	-	-
Stage 2	386	388	-	372	366	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518		3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	329	340	751	331	346	703	1270	-	-	1203	-	-
Stage 1	657	625	-	649	620	-	-	-	-	-	-	-
Stage 2	637	609	-	648	623	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	312	325	751	312	331	703	1270	-	-	1203	-	-
Mov Cap-2 Maneuver	312	325	-	312	331	-	-	-	-	-	-	-
Stage 1	648	606	-	641	612	-	-	-	-	-	-	-
Stage 2	617	601	-	613	604	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	15.6			14.6			0.3			0.9		
HCM LOS	С			В								
Minantan (MA)	.1	NDI	NOT	NDD		EDL O	MDL &	MDL - C	001	ODT	000	
Minor Lane/Major Mvm	IT	NBL	NBT	NRK	EBLn1				SBL	SBT	SBR	
Capacity (veh/h)		1270	-	-	314	751	318	703	1203	-	-	
HCM Lane V/C Ratio		0.012	-	-		0.016			0.03	-	-	
HCM Control Delay (s)		7.9	-	-	17.8	9.9	16.9	10.2	8.1	-	-	
HCM Lane LOS		Α	-	-	С	Α	С	В	Α	-	-	
HCM 95th %tile Q(veh)	0	-	-	0.3	0.1	0.1	0	0.1	-	-	

Intersection												
Int Delay, s/veh	2										·	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7		ર્ન	7	ሻ	†	7	ሻ	†	7
Traffic Vol, veh/h	28	4	9	20	4	8	8	247	4	15	229	4
Future Vol, veh/h	28	4	9	20	4	8	8	247	4	15	229	4
Conflicting Peds, #/hi		0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	_	_	100	_	_	100	150	_	50	125	_	50
Veh in Median Storag	ne.# -	0	-	_	0	-	-	0	-	-	0	-
Grade, %	,o, <i>''</i> -	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	31	4	10	22	4	9	9	274	4	17	254	4
	01	r			r	3	3	'	·	.,	_0 1	r
Major/Minor	Minor			Minor1		1	Major1			Major?		
Major/Minor	Minor2	E0.4			E0.4		Major1			Major2		
Conflicting Flow All	589	584	254	589	584	274	258	0	0	278	0	0
Stage 1	288	288	-	292	292	-	-	-	-	-	-	-
Stage 2	301	296	-	297	292	-	4.40	-	-	4.40	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318		4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver		423	785	420	423	765	1307	-	-	1285	-	-
Stage 1	720	674	-	716	671	-	-	-	-	-	-	-
Stage 2	708	668	-	712	671	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuve		415	785	405	415	765	1307	-	-	1285	-	-
Mov Cap-2 Maneuve		415	-	405	415	-	-	-	-	-	-	-
Stage 1	715	665	-	711	666	-	-	-	-	-	-	-
Stage 2	690	663	-	689	662	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay,	s 13.6			13.3			0.2			0.5		
HCM LOS	В			В								
Minor Lane/Major Mv	mt	NBL	NBT	NRD	ERI n1	EBLn2\	MRI n1\	VRI n2	SBL	SBT	SBR	
	1111		INDI	NDR						JDI	SDR	
Capacity (veh/h)		1307	-	-	406	785	407 0.066	765 0.012	1285	-	-	
HCM Cantral Dalay (0.007	-	-		0.013				-	-	
HCM Control Delay (S)	7.8	-	-	14.7	9.6	14.5	9.8	7.8	-	-	
HCM Lane LOS	L)	A	-	-	В	A	В	Α	A	-	-	
HCM 95th %tile Q(ve	11)	0	-	-	0.3	0	0.2	0	0	-	-	

Intersection													
Int Delay, s/veh	1.8												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		4	7		4	7	ሻ	↑	7	ሻ	†	7	
Traffic Vol, veh/h	25	4	11	9	4	7	14	364	15	33	304	4	
Future Vol, veh/h	25	4	11	9	4	7	14	364	15	33	304	4	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	_	None	
Storage Length	_	_	100	_	_	100	150	_	50	125	_	50	
Veh in Median Storage	.# -	0	-	_	0	-	-	0	-	-	0	-	
Grade, %	_	0	_	_	0	_	_	0	_	_	0	_	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	28	4	12	10	4	8	16	404	17	37	338	4	
	_*	•	. =			•						•	
Major/Minor I	Minor2			Minor1			Major1			Major2			
Conflicting Flow All	863	865	338	858	852	404	342	0	0	421	0	0	_
Stage 1	412	412	-	436	436	404	342	U	U	421	U	U	
Stage 2	451	453	-	422	416	-	-	-	-	-	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	0.22	6.12	5.52	0.22	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52	-	-	-	-	-	-	-	
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-	
Pot Cap-1 Maneuver	275	292	704	277	297	647	1217	-	-	1138	-	-	
Stage 1	617	594		599	580	047	1217	-	-	1130	-	-	
•	588	570	-	609	592	-	-	-	-	-	-	-	
Stage 2 Platoon blocked, %	500	370	-	009	592	-	-	-	-	-	-	-	
· ·	250	270	704	260	202	647	1017	-	-	1138	-	-	
Mov Cap 2 Manager	259 259	279 279		260	283 283	047	1217	-	-	1130	-	-	
Mov Cap-2 Maneuver			-	260 591		-	-	-	-	-	-	-	
Stage 1	609	574	-		572	-	-	-	-	-	-	-	
Stage 2	569	563	-	574	572	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	17.8			16.3			0.3			0.8			_
HCM LOS	17.0 C			10.3 C			0.5			0.0			
I IOWI LOG	C			C									
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1	EBLn2\	VBLn1\	NBL n2	SBL	SBT	SBR		
Capacity (veh/h)	-	1217		-	262	704	267	647	1138		-		_
HCM Lane V/C Ratio		0.013	_	_				0.012		_	-		
HCM Control Delay (s)		8	-	-	20.7	10.2	19.3	10.6	8.3	_	-		
HCM Lane LOS		A	-	-	20.7 C	В	19.5 C	В	0.5 A	-	-		
HCM 95th %tile Q(veh)	١	0	-	-	0.4	0.1	0.2	0	0.1	_	-		
TOW JOHN JOHN W(VEIL)	<i>'</i>	J	_	_	0.4	0.1	0.2	J	0.1		_		

Intersection												
Int Delay, s/veh	2.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7		र्स	7	ሻ	†	7	ሻ	†	7
Traffic Vol, veh/h	30	4	10	22	4	9	9	225	4		183	4
Future Vol, veh/h	30	4	10	22	4	9	9	225	4	16	183	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	·-		None	·-	·-	None .	-	-	None	_	-	None
Storage Length	_	-	100	-	-	100	150	-	50	125	-	50
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	_	0	-
Grade, %	· -	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	33	4	11	24	4	10	10	250	4		203	4
Major/Minor	Minor2			Minor1		I	Major1			Major2		
Conflicting Flow All	518	513	203	519	513	250	207	0	0	254	0	0
Stage 1	239	239	-	270	270	-	-	-	-		-	-
Stage 2	279	274	_	249	243	_	_	_	_	_	_	_
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	_	_	4.12	_	_
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	_	_	-	_	_
Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52	_	_	_	_	_	_	_
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	_	_	2.218	_	_
Pot Cap-1 Maneuver	468	465	838	467	465	789	1364	_	_	1311	_	_
Stage 1	764	708	-	736	686	-	-	_	_	-	_	_
Stage 2	728	683	_	755	705	_	_	_	_	_	_	_
Platoon blocked, %	3	300						_	_		_	_
Mov Cap-1 Maneuver	451	455	838	450	455	789	1364	_	_	1311	_	_
Mov Cap-2 Maneuver	451	455	-	450	455	-	-	_	_	-	_	_
Stage 1	759	698	_	731	681	_	_	_	_	_	_	_
Stage 2	709	678	-	730	695	-	-	_	_	-	-	-
J -												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.7			12.5			0.3			0.6		
HCM LOS	12.7 B			12.3 B			0.0			0.0		
TIOWI LOO	D			ט								
Minor Lanc/Major Muse	nt.	NBL	NDT	NDD	EDI 51	בטו אט	//DI ∽1\	رم اط/۸	CDI	CDT	SBR	
Minor Lane/Major Mvm	IL		NBT	INDK	451	EBLn2\			SBL	SBT	SDK	
Capacity (veh/h)		1364	-	-		838	451	789	1311	-	-	
HCM Control Doloy (a)		0.007	-	-		0.013		0.013		-	-	
HCM Long LOS	1	7.7	-	-	13.7	9.4	13.5	9.6	7.8	-	-	
HCM Lane LOS HCM 95th %tile Q(veh	١	A 0	-	-	B 0.3	A 0	B 0.2	A 0	A 0	-	-	
TION SOUL WINE CIVEN)	U	-	-	0.3	U	0.2	U	U	-	-	

Intersection													
Int Delay, s/veh	1.9												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		ન	7		4	7	ሻ	↑	7	ሻ	†	7	
Traffic Vol, veh/h	27	4	11	10	4	8	15	330	16	35	280	4	
Future Vol, veh/h	27	4	11	10	4	8	15	330	16	35	280	4	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	_	_	100	_	_	100	150	_	50	125	_	50	
Veh in Median Storage	.# -	0	-	_	0	-	-	0	-	-	0	-	
Grade, %	, _	0	_	_	0	_	_	0	_	_	0	_	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	30	4	12	11	4	9	17	367	18	39	311	4	
				• •		J	• •					•	
Major/Minor	Minor2			Minor1		I	Major1		ı	Major2			
Conflicting Flow All	806	808	311	800	794	367	315	0	0	385	0	0	
Stage 1	389	389	-	401	401	307	313	U	U	303	U	U	
Stage 2	417	419	-	399	393	-	-	-	-	-	-	-	
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-	
Critical Hdwy Stg 1	6.12	5.52	0.22	6.12	5.52	0.22	4.12	_	-	4.12	-	_	
Critical Hdwy Stg 1 Critical Hdwy Stg 2	6.12	5.52	_	6.12	5.52	_	-	_	-	_	-	_	
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	_	-	2.218	-	_	
Pot Cap-1 Maneuver	300	315	729	303	321	678	1245	_	-	1173	-	_	
Stage 1	635	608	123	626	601	070	1243	_	-	1173	-	_	
Stage 2	613	590	_	627	606	_	-	_	-	_	-	_	
Platoon blocked, %	013	550	-	UZI	000	-	-	-	-	-	-		
Mov Cap-1 Maneuver	282	300	729	284	306	678	1245	-	-	1173	-		
Mov Cap-1 Maneuver	282	300	129	284	306	010	1240	-	-	1113	-		
Stage 1	626	588	-	617	593	-	-	-	-	-	-	-	
Stage 2	592	582	_	591	586	-	-	-	-	-	-	-	
Olaye 2	JJZ	302	-	Jai	500	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Control Delay, s	16.9			15.3			0.3			0.9			_
HCM LOS	10.9 C			13.3 C			0.5			0.0			
I IOWI LOO	C			C									
Minor Lane/Major Mvm	t	NBL	NBT	NRR	FBI n1	EBLn2\	VBI n1\	NBI n2	SBL	SBT	SBR		
Capacity (veh/h)		1245			284	729	290	678	1173	-	-		_
HCM Lane V/C Ratio		0.013	-	-		0.017				-	-		
HCM Control Delay (s)		7.9	-	-	19.4	10	18.1	10.4	8.2	-	-		
HCM Lane LOS		7.9 A	-	-	19.4 C	В	10.1 C	10.4 B	0.2 A	-	-		
HCM 95th %tile Q(veh)	1	0	-	-	0.4	0.1	0.2	0	0.1	-	-		
TOW JOHT JUHE Q(VEIT)	1	U	-	-	0.4	0.1	0.2	U	0.1	_	-		

Intersection												
Int Delay, s/veh	2.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7		र्स	7	ř	ħβ		Ť	ħβ	
Traffic Vol, veh/h	30	4	10	22	4	9	9	632	4	16	793	4
Future Vol, veh/h	30	4	10	22	4	9	9	632	4	16	793	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	_	None	_	-	None
Storage Length	_	_	100	_	_	100	150	_	-	125	_	-
Veh in Median Storage	e.# -	0	_	_	0	-	-	0	_	-	0	_
Grade, %	-	0	_	_	0	_	_	0	_	_	0	_
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	33	4	11	24	4	10	10	702	4	18	881	4
						. •			•	.0	50 1	•
Major/Minor	Minor2		ľ	Minor1		ľ	Major1		ľ	Major2		
Conflicting Flow All	1292	1645	443	1203	1645	353	885	0	0	706	0	0
Stage 1	919	919	-	724	724	-	-	-	-	-	-	-
Stage 2	373	726	_	479	921	_	_	_	_	_	_	_
Critical Hdwy	7.54	6.54	6.94	7.54	6.54	6.94	4.14	_	_	4.14	_	_
Critical Hdwy Stg 1	6.54	5.54	-	6.54	5.54	J.J-	-	_	_		_	_
Critical Hdwy Stg 2	6.54	5.54	_	6.54	5.54	_	_	_	_	_	_	_
Follow-up Hdwy	3.52	4.02	3.32	3.52	4.02	3.32	2.22	_	_	2.22	_	_
Pot Cap-1 Maneuver	120	99	562	140	99	643	760	_	_	888	_	_
Stage 1	292	348	-	383	429	-		_	_	-	_	_
Stage 2	620	428	-	537	347	_	_	_	_	_	_	_
Platoon blocked, %	520	720	_	001	071	_	_	_	_	_	_	_
Mov Cap-1 Maneuver	111	96	562	129	96	643	760	-	_	888	-	-
Mov Cap-1 Maneuver	111	96	-	129	96	U 4 J	100	-	<u>-</u>	-	-	-
Stage 1	288	341	-	378	423	-		-	<u>-</u>	-	-	-
Stage 2	596	422	-	509	340	-	-	-	-	-	-	-
Stage 2	330	422	-	503	J40	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	44.8			34.7			0.1			0.2		
HCM LOS	E			D			•			J. <u> </u>		
	_			_								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR I	EBLn1 I	EBLn2V	VBLn1V	VBLn2	SBL	SBT	SBR	
Capacity (veh/h)		760	_	_	109	562	123	643	888	_	_	
HCM Lane V/C Ratio		0.013	_	_	0.347		0.235		0.02	_	_	
HCM Control Delay (s)		9.8	_	_	54.6	11.5	43	10.7	9.1	_	_	
HCM Lane LOS		A	_	_	F	В	E	В	A	_	_	
HCM 95th %tile Q(veh)	0	_	_	1.4	0.1	0.9	0	0.1	_	_	
	,	J				J	3.3	J	J			

Intersection													
Int Delay, s/veh	5.1												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		र्स	7		र्स	7	ሻ	∱ }		ሻ	∱ }		
Traffic Vol, veh/h	27	4	11	10	4	8	15	1103	16	35	945	4	
Future Vol, veh/h	27	4	11	10	4	8	15	1103	16	35	945	4	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	·-	None	-	·-	None	-	-	None	-	-	None	
Storage Length	-	-	100	-	-	100	150	-	-	125	-	-	
Veh in Median Storage	e,# -	0	-	-	0	_	-	0	_	-	0	_	
Grade, %	_	0	-	-	0	_	-	0	_	-	0	_	
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2	
Mvmt Flow	30	4	12	11	4	9	17	1226	18	39	1050	4	
			•		-				-				
Major/Minor	Minor2		ı	Minor1		i	Major1		ı	Major2			
Conflicting Flow All	1779	2408	527	1874	2401	622	1054	0	0	1244	0	0	
Stage 1	1130	1130	-	1269	1269	-	-	-	_	-	-	-	
Stage 2	649	1278	_	605	1132	_	_	_	_	_	_	_	
Critical Hdwy	7.54	6.54	6.94	7.54	6.54	6.94	4.14	_	_	4.14	_	_	
Critical Hdwy Stg 1	6.54	5.54	-	6.54	5.54	- 0.01		_	_	-	_	_	
Critical Hdwy Stg 2	6.54	5.54	_	6.54	5.54	_	_	_	_	_	_	_	
Follow-up Hdwy	3.52	4.02	3.32	3.52	4.02	3.32	2.22	_	_	2.22	_	_	
Pot Cap-1 Maneuver	52	33	496	44	33	430	656	_	_	555	_	_	
Stage 1	217	277	-	178	238	-	-	_	_	-	_	_	
Stage 2	425	235	_	451	276	_	_	_	_	_	_	_	
Platoon blocked, %	120	200		101	210			_	_		_	_	
Mov Cap-1 Maneuver	42	30	496	35	30	430	656	_	_	555	_	_	
Mov Cap-1 Maneuver	42	30	-30	35	30	-	-	_	_	-	_	_	
Stage 1	211	258	_	173	232	_	_	_	_	_	_	_	
Stage 2	398	229	_	402	257	_	_	_	_	_	_	_	
Jugo L	330			102	201								
Approach	EB			WB			NB			SB			
HCM Control Delay, s				124.7			0.1			0.4			
HCM LOS	F			F			0.1			J			
	•			•									
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1	EBLn2V	VBLn1\	WBLn2	SBL	SBT	SBR		
Capacity (veh/h)		656	_	_	40	496	33	430	555	_	_		
HCM Lane V/C Ratio		0.025	_	_	0.861				0.07	_	_		
HCM Control Delay (s))	10.6	_	_	253.1		188.3	13.5	12	_	_		
HCM Lane LOS	,	В	_	_	F	В	F	В	В	_	_		
HCM 95th %tile Q(veh)	0.1	_	_	3.3	0.1	1.6	0.1	0.2	_	_		
	,												

Intersection						
Int Delay, s/veh	2.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations		WDIX		NDIX		
	**	20	}	E 0	ሻ	100
Traffic Vol, veh/h	40	39	218	59	59	199
Future Vol, veh/h	40	39	218	59	59	199
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	44	43	242	66	66	221
			- :-			:
	Minor1		/lajor1		Major2	
Conflicting Flow All	628	275	0	0	308	0
Stage 1	275	-	-	-	-	-
Stage 2	353	-	-	_	_	_
Critical Hdwy	6.42	6.22	_	_	4.12	_
Critical Hdwy Stg 1	5.42	-	_	_	-	_
Critical Hdwy Stg 2	5.42	_	_	_	_	_
Follow-up Hdwy		3.318		_	2.218	
	447	764	-	_	1253	-
Pot Cap-1 Maneuver	771		-	-	1255	-
Stage 1		-	-	-	-	-
Stage 2	711	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	423	764	-	-	1253	-
Mov Cap-2 Maneuver	423	-	-	-	-	-
Stage 1	771	-	-	-	-	-
Stage 2	673	-	-	-	-	-
.						
Approach	WB		NB		SB	
HCM Control Delay, s	12.9		0		1.8	
HCM LOS	12.3 B		U		1.0	
I IOWI LOG	D					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		_	-	543	1253	_
HCM Lane V/C Ratio		_	_	0.162		_
HCM Control Delay (s))	_	_	12.9	8	_
HCM Lane LOS	,	_	_	В	Ā	_
HCM 95th %tile Q(veh)	_	_	0.6	0.2	_
	,			5.0	٥.٢	

Intersection						
Int Delay, s/veh	3.7					
•		WDD	NDT	NDD	CDI	CDT
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	70	7.5	\$	00	\	↑
Traffic Vol, veh/h	72	75	318	60	58	266
Future Vol, veh/h	72	75	318	60	58	266
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	100	-
Veh in Median Storage	e, # 0	-	0	-	-	0
Grade, %	0	_	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	80	83	353	67	64	296
	00	00	000	0.	٠.	200
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	811	387	0	0	420	0
Stage 1	387	-	-	-	-	-
Stage 2	424	-	-	-	-	-
Critical Hdwy	6.42	6.22	_	_	4.12	_
Critical Hdwy Stg 1	5.42	-	_	_	-	_
Critical Hdwy Stg 2	5.42	_	_	_	_	_
Follow-up Hdwy		3.318	_	_	2.218	_
Pot Cap-1 Maneuver	349	661			1139	
•	686	001	_	_	1133	-
Stage 1		-	-	-	-	-
Stage 2	660	-	-	-	-	-
Platoon blocked, %	000	004	-	-	4.400	-
Mov Cap-1 Maneuver	329	661	-	-	1139	-
Mov Cap-2 Maneuver	329	-	-	-	-	-
Stage 1	686	-	-	-	-	-
Stage 2	623	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	17.8		0		1.5	
HCM LOS	17.0 C		U		1.5	
I IOWI LOS	C					
Minor Lane/Major Mvn	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	442	1139	-
HCM Lane V/C Ratio		_	_		0.057	_
HCM Control Delay (s)	١	_	_	17.8	8.3	_
HCM Lane LOS	'	_	_	17.0 C	Α	_
HCM 95th %tile Q(veh	١	-	-	1.7	0.2	-
TION JULY /OUIE Q(VEH	,	-	-	1.7	0.2	-

MOVEMENT SUMMARY

♥ Site: 1 [01 2030 Build AM (Site Folder: General)]

Kelly Road and Site Driveway Site Category: Existing Design

Roundabout

Vehi	cle M	ovemen	t Perfo	rmance										
Mov ID	Turn	INP VOLU [Total veh/h		DEM/ FLO¹ [Total veh/h		Deg. Satn v/c		Level of Service	95% BA QUE [Veh. veh	ACK OF EUE Dist] ft	Prop. I Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed mph
South	n: Kelly	y Road												
179	L2	97	2.0	108	2.0	0.371	10.2	LOS B	1.8	47.0	0.69	0.74	0.80	21.4
300	T1	108	2.0	120	2.0	0.371	10.2	LOS B	1.8	47.0	0.69	0.74	0.80	20.1
59	R2	23	2.0	26	2.0	0.371	10.2	LOS B	1.8	47.0	0.69	0.74	0.80	20.9
Appro	oach	228	2.0	253	2.0	0.371	10.2	LOS B	1.8	47.0	0.69	0.74	0.80	20.8
East:	Site D	Priveway												
118	L2	14	2.0	16	2.0	0.146	7.7	LOSA	0.6	14.4	0.64	0.64	0.64	23.1
273	T1	26	2.0	29	2.0	0.146	7.7	LOS A	0.6	14.4	0.64	0.64	0.64	22.8
62	R2	39	2.0	43	2.0	0.146	7.7	LOSA	0.6	14.4	0.64	0.64	0.64	21.8
Appro	oach	79	2.0	88	2.0	0.146	7.7	LOSA	0.6	14.4	0.64	0.64	0.64	22.4
North	: Kelly	Road												
49	L2	59	2.0	66	2.0	0.166	4.4	LOSA	0.7	18.3	0.30	0.17	0.30	24.2
466	T1	122	2.0	136	2.0	0.166	4.4	LOSA	0.7	18.3	0.30	0.17	0.30	23.1
258	R2	644	2.0	716	2.0	0.584	9.9	LOSA	4.4	111.2	0.48	0.31	0.48	20.3
Appro	oach	825	2.0	917	2.0	0.584	8.7	LOSA	4.4	111.2	0.44	0.28	0.44	21.0
West	: Site I	Driveway												
140	L2	497	2.0	552	2.0	0.648	12.5	LOS B	7.8	196.9	0.69	0.66	0.88	19.8
116	T1	36	2.0	40	2.0	0.648	12.5	LOS B	7.8	196.9	0.69	0.66	0.88	20.8
123	R2	97	2.0	108	2.0	0.648	12.5	LOS B	7.8	196.9	0.69	0.66	0.88	18.9
Appro	oach	630	2.0	700	2.0	0.648	12.5	LOS B	7.8	196.9	0.69	0.66	0.88	19.7
All Vehic	les	1762	2.0	1958	2.0	0.648	10.2	LOS B	7.8	196.9	0.57	0.49	0.65	20.5

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6). Roundabout Capacity Model: US HCM 6.

Delay Model: HCM Delay Formula (Geometric Delay is not included).

Queue Model: HCM Queue Formula. Gap-Acceptance Capacity: Traditional M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: P:\Projects24\301.2400935.000_West_Village_Phase_2-Apex_NC\Carolinas_VA\Congestion Management\TIA\Analysis\SIDRA\Kelly Road and Site Driveway.sip9

MOVEMENT SUMMARY

♥ Site: 1 [02 2030 Build PM (Site Folder: General)]

Kelly Road and Site Driveway Site Category: Existing Design

Roundabout

Vehi	cle M	ovement	Perfo	rmance										
Mov ID	Turn	INP VOLU [Total veh/h		DEM/ FLO¹ [Total veh/h		Deg. Satn v/c		Level of Service		ACK OF EUE Dist] ft	Prop. Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed mph
South	n: Kelly	/ Road												
179	L2	120	2.0	133	2.0	0.695	25.0	LOS D	5.2	133.3	0.85	1.23	1.71	17.1
300	T1	172	2.0	191	2.0	0.695	25.0	LOS D	5.2	133.3	0.85	1.23	1.71	15.5
59	R2	27	2.0	30	2.0	0.695	25.0	LOS D	5.2	133.3	0.85	1.23	1.71	17.5
Appro	oach	319	2.0	354	2.0	0.695	25.0	LOS D	5.2	133.3	0.85	1.23	1.71	16.3
East:	Site D	riveway												
118	L2	25	2.0	28	2.0	0.406	16.9	LOS C	1.8	45.5	0.79	0.91	1.09	20.3
273	T1	47	2.0	52	2.0	0.406	16.9	LOS C	1.8	45.5	0.79	0.91	1.09	20.3
62	R2	75	2.0	83	2.0	0.406	16.9	LOS C	1.8	45.5	0.79	0.91	1.09	19.1
Appro	oach	147	2.0	163	2.0	0.406	16.9	LOS C	1.8	45.5	0.79	0.91	1.09	19.7
North	: Kelly	Road												
49	L2	58	2.0	64	2.0	0.203	5.0	LOSA	0.9	22.8	0.37	0.25	0.37	24.1
466	T1	151	2.0	168	2.0	0.203	5.0	LOSA	0.9	22.8	0.37	0.25	0.37	22.9
258	R2	757	2.0	841	2.0	0.718	14.0	LOS B	11.1	281.6	0.70	0.70	0.94	18.8
Appro	oach	966	2.0	1073	2.0	0.718	12.1	LOS B	11.1	281.6	0.62	0.60	0.81	19.7
West	: Site [Driveway												
140	L2	887	2.0	986	2.0	1.183	109.2	LOS F	105.6	2681.3	1.00	4.15	5.79	7.5
116	T1	33	2.0	37	2.0	1.183	109.2	LOS F	105.6	2681.3	1.00	4.15	5.79	9.6
123	R2	179	2.0	199	2.0	1.183	109.2	LOS F	105.6	2681.3	1.00	4.15	5.79	7.6
Appro	oach	1099	2.0	1221	2.0	1.183	109.2	LOS F	105.6	2681.3	1.00	4.15	5.79	7.6
All Vehic	eles	2531	2.0	2812	2.0	1.183	56.2	LOS F	105.6	2681.3	0.83	2.24	3.10	11.4

Site Level of Service (LOS) Method: Delay & v/c (HCM 6). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Roundabout LOS Method: Same as Sign Control.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Intersection and Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 6). Roundabout Capacity Model: US HCM 6.

Delay Model: HCM Delay Formula (Geometric Delay is not included).

Queue Model: HCM Queue Formula. Gap-Acceptance Capacity: Traditional M1.

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: P:\Projects24\301.2400935.000_West_Village_Phase_2-Apex_NC\Carolinas_VA\Congestion Management\TIA\Analysis\SIDRA\Kelly Road and Site Driveway.sip9

Intersection							
Int Delay, s/veh	10.8						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	ሻ	†	†	7	ሻ	7	
Traffic Vol, veh/h	96	577	478	88	116	54	
Future Vol, veh/h	96	577	478	88	116	54	
Conflicting Peds, #/hr	_ 0	_ 0	_ 0	_ 0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	525	-	-	250	0	100	
Veh in Median Storage		0	0	-	0	-	
Grade, % Peak Hour Factor	90	0 90	0 90	90	0 90	90	
Heavy Vehicles, %	90	90	90	90	90	90	
Mymt Flow	107	641	531	98	129	60	
WWITH TOW	107	U T 1	JJ 1	30	123	00	
Majar/Mins=	Mai4		Anie -O		Min.s.=0		
	Major1		Major2		Minor2	E24	
Conflicting Flow All	629	0	-	0	1386 531	531	
Stage 1 Stage 2	-	-	-	-	855	-	
Critical Hdwy	4.12	-	-	-	6.42	6.22	
Critical Hdwy Stg 1	7.12	_	_	_	5.42	0.22	
Critical Hdwy Stg 2	_	_	_	_	5.42	_	
Follow-up Hdwy	2.218	_	_	_	3.518	3.318	
Pot Cap-1 Maneuver	953	_	_	_	158	548	
Stage 1	-	-	-	-	590	-	
Stage 2	-	-	-	-	417	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver		-	-	-	140	548	
Mov Cap-2 Maneuver	-	-	-	-	140	-	
Stage 1	-	-	-	-	524	-	
Stage 2	-	-	-	-	417	-	
Approach	EB		WB		SB		
HCM Control Delay, s			0		84.2		
HCM LOS					F		
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR :	SBLn1 S	SBLn2
Capacity (veh/h)		953			-	140	548
HCM Lane V/C Ratio		0.112	_	_	_	0.921	
HCM Control Delay (s)	9.3	_	_		117.6	12.4
HCM Lane LOS	,	A	_	_	_	F	В
HCM 95th %tile Q(veh	1)	0.4	-	-	-	6.3	0.4
•							

Intersection							
Int Delay, s/veh	10.6						
Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	ሻ	↑	↑	7	ሻ	7	
Traffic Vol, veh/h	111	489	610	175	101	137	
Future Vol, veh/h	111	489	610	175	101	137	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	525	-	-	250	0	100	
Veh in Median Storage		0	0	-	0	-	
Grade, %	-	0	0	-	0	-	
Peak Hour Factor	90	90	90	90	90	90	
Heavy Vehicles, %	2	2	2	2	2	2 450	
Mvmt Flow	123	543	678	194	112	152	
Major/Minor	Major1	<u> </u>	Major2		Minor2		
Conflicting Flow All	872	0	-	0	1467	678	
Stage 1	-	-	-	-	678	-	
Stage 2	-	-	-	-	789	-	
Critical Hdwy	4.12	-	-	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	2.218	-	-	-	3.518		
Pot Cap-1 Maneuver	773	-	-	-	141	452	
Stage 1	-	-	-	-	504	-	
Stage 2	-	-	-	-	448	-	
Platoon blocked, %		-	-	-			
Mov Cap-1 Maneuver		-	-	-	119	452	
Mov Cap-2 Maneuver	-	-	-	-	119	-	
Stage 1	-	-	-	-	424	-	
Stage 2	-	-	-	-	448	-	
Approach	EB		WB		SB		
HCM Control Delay, s	1.9		0		67.7		
HCM LOS					F		
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR	SBLn1 S	BLn2
Capacity (veh/h)		773		-	-	119	452
HCM Lane V/C Ratio		0.16	_	_	_	0.943	
HCM Control Delay (s)	10.5	_	_		136.5	17
HCM Lane LOS	,	В	_	_	_	F	C
HCM 95th %tile Q(veh	1)	0.6	_	_	_	6.1	1.5
2 222. /00 5/10/	,	2.0				2	

-									
Intersection									
Int Delay, s/veh	76.4								
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations	- ኝ			7	<u>ነ</u>	7			
Traffic Vol, veh/h	112	903	753	103	136	63			
Future Vol, veh/h	112	903	753	103	136	63			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Free	Free	Free	Free	Stop	Stop			
RT Channelized	-	None	-	None	-	None			
Storage Length	525	-	_	250	0	100			
Veh in Median Storag		0	0		0	-			
Grade, %	-	0	0	_	0	_			
Peak Hour Factor	90	90	90	90	90	90			
Heavy Vehicles, %	2	2	2	2	2	2			
Mvmt Flow	124	1003	837	114	151	70			
MINITE FIOW	124	1003	031	114	101	70			
Major/Minor	Major1		Major2		Minor2				
Conflicting Flow All	951	0	-	0	2088	837			
Stage 1	-	-	-	-	837	-			
Stage 2	-	-	-	-	1251	-			
Critical Hdwy	4.12	-	-	-	6.42	6.22			
Critical Hdwy Stg 1	-	-	-	-	5.42	-			
Critical Hdwy Stg 2	-	-	-	-	5.42	-			
Follow-up Hdwy	2.218	_	_	-	3.518	3.318			
Pot Cap-1 Maneuver	722	_	_	_	~ 58	367			
Stage 1	_	_	_	_	425	-			
Stage 2	_	_	_	_	270	_			
Platoon blocked, %		_	_	_	•				
Mov Cap-1 Maneuver	722	_	_	_	~ 48	367			
Mov Cap-2 Maneuver		_	_	_	~ 48	-			
Stage 1	_	_	_	_	352	_			
Stage 2					270				
Stage 2					210				
A I	- -D		MD		OD				
Approach	EB		WB		SB				
HCM Control Delay, s	1.2		0	\$	788.8				
HCM LOS					F				
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR	SBLn1 S	BLn2		
Capacity (veh/h)		722	-	-	-	48	367		
HCM Lane V/C Ratio		0.172	-	-	-	3.148			
HCM Control Delay (s	;)	11	-	-		1146.3	17.1		
HCM Lane LOS	,	В	_	_	-	F	C		
HCM 95th %tile Q(veh	1)	0.6	_	_	_	16.4	0.7		
Notes	,	2.3							
	naoitre	¢. D.	dov. ove	20d2 31	200	Com=	utation Not Defined	*: All major valuma in plata ==	
~: Volume exceeds ca	apacity	φ: D6	ay exc	eeds 30	JUS .	+. Comp	utation Not Defined	*: All major volume in platoon	

Intersection									
Int Delay, s/veh	79.4								
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations	"			7	<u>ነ</u>	7			
Traffic Vol, veh/h	130	795	966	205	118	160			
Future Vol, veh/h	130	795	966	205	118	160			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Free	Free	Free	Free	Stop	Stop			
RT Channelized	-	None	-	None	-	None			
Storage Length	525	-	_	250	0	100			
Veh in Median Storage		0	0	-	0	-			
Grade, %	-	0	0	_	0	_			
Peak Hour Factor	90	90	90	90	90	90			
		2	2	2	2	2			
Heavy Vehicles, %	2								
Mvmt Flow	144	883	1073	228	131	178			
	Major1		Major2		Minor2				
Conflicting Flow All	1301	0	-	0	2244	1073			
Stage 1	-	-	-	-	1073	-			
Stage 2	-	-	-	-	1171	-			
Critical Hdwy	4.12	-	-	-	6.42	6.22			
Critical Hdwy Stg 1	-	-	-	-	5.42	-			
Critical Hdwy Stg 2	-	-	_	-	5.42	-			
Follow-up Hdwy	2.218	_	-	_	3.518	3.318			
Pot Cap-1 Maneuver	532	_	_	_	~ 46	268			
Stage 1	_	_	_	_	328	-			
Stage 2	_	_	_	_	295	_			
Platoon blocked, %		_	_	_					
Mov Cap-1 Maneuver	532	_	_	_	~ 34	268			
Mov Cap-2 Maneuver	-	_	_	_	~ 34	200			
Stage 1	_				239				
Stage 2	_	_	_	_	295	_			
Staye 2	-	-	-	-	233	-			
Approach	EB		WB	_	SB				
HCM Control Delay, s	2		0	\$	671.6				
HCM LOS					F				
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR	SBLn1 S	BLn2		
Capacity (veh/h)		532	-	-	-	34	268		
HCM Lane V/C Ratio		0.272	-	-	-	3.856			
HCM Control Delay (s)	14.3	_	_		\$ 1526	41.4		
HCM Lane LOS	,	В	_	_	_	F .020	E		
HCM 95th %tile Q(veh	1)	1.1	_	_	_	15.3	4.3		
`	.,					. 5.0			
Notes	••							* **	
~: Volume exceeds ca	pacity	\$: De	elay exc	eeds 30	JUS ·	+: Comp	utation Not Defined	*: All major volume in platoon	

Intersection									
Int Delay, s/veh	122.4								
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations	ሻ	†		7	ነ	7			
Traffic Vol, veh/h	130	903	753	144	164	75			
Future Vol, veh/h	130	903	753	144	164	75			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Free	Free	Free	Free	Stop	Stop			
RT Channelized	_	None	-	None	-	None			
Storage Length	525	-	_	250	0	100			
Veh in Median Storage		0	0		0	-			
Grade, %	-	0	0	_	0	_			
Peak Hour Factor	90	90	90	90	90	90			
Heavy Vehicles, %	2	2	2	2	2	2			
Mvmt Flow	144	1003	837	160	182	83			
WIVIII I IOW	144	1003	031	100	102	03			
Major/Minor	Major1	ľ	Major2	l	Minor2				
Conflicting Flow All	997	0	-	0	2128	837			
Stage 1	-	-	-	-	837	-			
Stage 2	-	-	-	-	1291	-			
Critical Hdwy	4.12	-	-	-	6.42	6.22			
Critical Hdwy Stg 1	-	-	-	-	5.42	-			
Critical Hdwy Stg 2	-	-	-	-	5.42	-			
Follow-up Hdwy	2.218	_	_	_	3.518	3.318			
Pot Cap-1 Maneuver	694	_	_	_	~ 55	367			
Stage 1	_	_	_	_	425	_			
Stage 2	_	_	_	_	258	_			
Platoon blocked, %		_	_	_					
Mov Cap-1 Maneuver	694	_	_	_	~ 44	367			
Mov Cap-2 Maneuver		_	_	_	~ 44	-			
Stage 1	_	_	_	_	337	_			
Stage 2	_	_	_	_	258	_			
Olage 2					200				
Annroach	EB		WB		SB				
Approach				Φ.					
HCM Control Delay, s	1.5		0	\$ '	1104.2				
HCM LOS					F				
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR	SBLn1 S			
Capacity (veh/h)		694	-	-	-	44	367		
HCM Lane V/C Ratio		0.208	-	-			0.227		
HCM Control Delay (s)	11.5	-	-	\$	1601.1	17.7		
HCM Lane LOS		В	-	-	-	F	С		
HCM 95th %tile Q(veh	1)	0.8	-	-	-	20.6	0.9		
Notes									
~: Volume exceeds ca	nacity	\$· Do	lav ava	eeds 30	ηne	T. Comp	utation Not Defined	*: All major volume in plato	nn .
. Volume exceeds Ca	ipacity	φ. De	ay exc	ccus 30	005	+. Comp	utation Not Delined	. Ali major volume in piato	JII

West Village - Apex, NC
DRMP
Synchro 11 Report
Page 1

Intersection										
Int Delay, s/veh	162.3									
Movement	EBL	EBT	WBT	WBR	SBL	SBR				
Lane Configurations	7	↑		7	ሻ	7				
Traffic Vol, veh/h	143	795	966	236	159	178				
Future Vol, veh/h	143	795	966	236	159	178				
Conflicting Peds, #/hr	0	0	0	0	0	0				
Sign Control	Free	Free	Free	Free	Stop	Stop				
RT Channelized	-	None	-	None	-	None				
Storage Length	525	-	-	250	0	100				
Veh in Median Storag	e,# -	0	0	-	0	-				
Grade, %	-	0	0	-	0	-				
Peak Hour Factor	90	90	90	90	90	90				
Heavy Vehicles, %	2	2	2	2	2	2				
Mvmt Flow	159	883	1073	262	177	198				
Major/Minor	Major1	ı	Major2	ı	Minor2					
Conflicting Flow All	1335	0	-	0	2274	1073				
Stage 1	-	-	_	-	1073	-				
Stage 2	-	_	-	_	1201	_				
Critical Hdwy	4.12	_	_	_	6.42	6.22				
critical Hdwy Stg 1	_	_	_	_	5.42	-				
Critical Hdwy Stg 2	_	_	_	_	5.42	_				
ollow-up Hdwy	2.218	_	_	_		3.318				
Pot Cap-1 Maneuver	517	_	_	_	~ 44	268				
Stage 1	-	_	_	_	328					
Stage 2	_	_	_	_	285	_				
Platoon blocked, %		_	_	_						
Nov Cap-1 Maneuver	517	_	_	_	~ 30	268				
Nov Cap-2 Maneuver		_	_	_	~ 30					
Stage 1	_	_	_	_	227	_				
Stage 2	_	_	_	_	285	_				
					,					
Approach	EB		WB		SB					
HCM Control Delay, s			0	\$	1186.8					
HCM LOS			,	•	F					
Minor Lane/Major Mvı	mt	EBL	EBT	WBT	WBR	SBLn1 S	BLn2			
Capacity (veh/h)		517	_	_	_	30	268			
HCM Lane V/C Ratio		0.307	_	_	_		0.738			
HCM Control Delay (s	s)	15	_	_		2461.1	48.6			
ICM Lane LOS	,	C	_	_	-	F	E			
HCM 95th %tile Q(vel	h)	1.3	-	_	_	21.4	5.3			
,	,	-								
Notes		ф г	.1=		20-	0	.d.eta N. (D	C I	* All	
~: Volume exceeds ca	apacity	⊅: De	elay exc	eeds 30	JUS -	+: Comp	utation Not De	Tined	*: All major volume in platoon	

Intersection									
Int Delay, s/veh	134.1								
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations	<u>ነ</u>			7	<u>ነ</u>	7			
Traffic Vol, veh/h	121	1047	822	111	147	68			
Future Vol, veh/h	121	1047	822	111	147	68			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Free	Free	Free	Free	Stop	Stop			
RT Channelized	-	None	-	None	·-	None			
Storage Length	525	-	-	0	0	100			
Veh in Median Storage		0	0	_	0	-			
Grade, %	<i>'</i>	0	0	_	0	_			
Peak Hour Factor	90	90	90	90	90	90			
Heavy Vehicles, %	2	2	2	2	2	2			
Mvmt Flow	134	1163	913	123	163	76			
			0.10	.20	.00	. •			
Major/Minor	Major1		Major2	,	Minor2				
			viajuiz			012			
Conflicting Flow All	1036	0	-	0	2344 913	913			
Stage 1	-	-	-	-		-			
Stage 2	-	-	-	-	1431	-			
Critical Hdwy	4.12	-	-	-	6.42	6.22			
Critical Hdwy Stg 1	-	-	-	-	5.42	-			
Critical Hdwy Stg 2	-	-	-	-	5.42	-			
Follow-up Hdwy	2.218	-	-	-	3.518	3.318			
Pot Cap-1 Maneuver	671	-	-	-	~ 40	331			
Stage 1	-	-	-	-	391	-			
Stage 2	-	-	-	-	220	-			
Platoon blocked, %		-	-	-					
Mov Cap-1 Maneuver	671	-	-	-	~ 32	331			
Mov Cap-2 Maneuver	-	-	-	-	~ 32	-			
Stage 1	_	_	_	_	313	_			
Stage 2	_	_	_	_	220	_			
5 ta g =									
Approach	EB		WB		SB				
HCM Control Delay, s			0	¢ ⁄	1438.4				
	1.2		U	Ф					
HCM LOS					F				
Minor Lane/Major Mvr	nt	EBL	EBT	WBT	WBR	SBLn1 SE			
Capacity (veh/h)		671	-	-	-	32	331		
HCM Lane V/C Ratio		0.2	-	-	-	5.104 0	.228		
HCM Control Delay (s)	11.7	-	-	- (\$ 2095	19.1		
HCM Lane LOS		В	-	-	-	F	С		
HCM 95th %tile Q(veh	1)	0.7	-	-	-	19.6	0.9		
•									
Notes		ф. D.	lavi arri	d - 0/	20-	04:	tation Not Defined	*. All manion values - ! lata - !	
~: Volume exceeds ca	ipacity	φ: D6	ay exc	eeds 30	JUS -	+. Compu	tation Not Defined	*: All major volume in platoon	

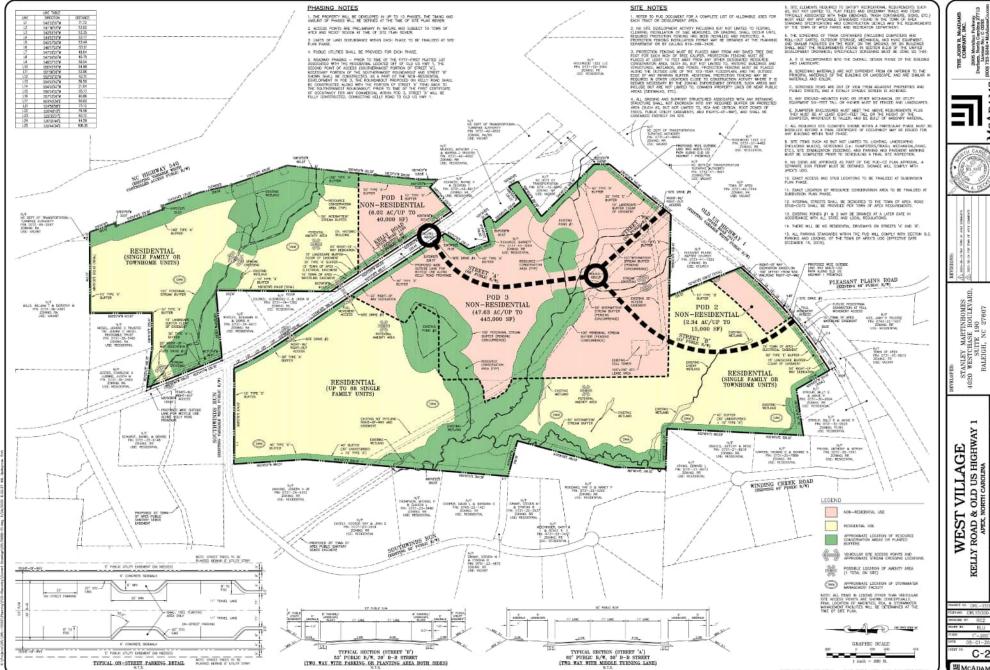
Intersection									
Int Delay, s/veh	156.6								
Movement	EBL	EBT	WBT	WBR	SBL	SBR			
Lane Configurations	ሻ	†	<u></u>	7	ሻ	7			
Traffic Vol, veh/h	140	889	1116	221	128	173			
Future Vol, veh/h	140	889	1116	221	128	173			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Free	Free	Free	Free	Stop	Stop			
RT Channelized	-	None	-	None	-	None			
Storage Length	525	-	-	0	0	100			
Veh in Median Storag	e,# -	0	0	-	0	-			
Grade, %	-	0	0	-	0	-			
Peak Hour Factor	90	90	90	90	90	90			
Heavy Vehicles, %	2	2	2	2	2	2			
Mvmt Flow	156	988	1240	246	142	192			
Major/Minor	Major1	ı	Major2	ı	Minor2				
Conflicting Flow All	1486	0	-	0	2540	1240			
Stage 1	-	-	-	-	1240	-			
Stage 2	-	_	-	-	1300	_			
Critical Hdwy	4.12	_	_	_	6.42	6.22			
Critical Hdwy Stg 1	_	_	-	-	5.42	-			
Critical Hdwy Stg 2	_	_	-	-	5.42	-			
Follow-up Hdwy	2.218	_	-	-	3.518	3.318			
Pot Cap-1 Maneuver	452	_	-	-	~ 30	214			
Stage 1	-	_	-	-	273	_			
Stage 2	-	-	-	-	255	-			
Platoon blocked, %		-	-	-					
Mov Cap-1 Maneuver	452	-	-	-	~ 20	214			
Mov Cap-2 Maneuver		-	-	-	~ 20	-			
Stage 1	-	-	-	-	179	-			
Stage 2	-	-	-	-	255	-			
3									
Approach	EB		WB		SB				
HCM Control Delay, s	2.3		0	\$ '	1379.8				
HCM LOS				•	F				
Minor Lane/Major Mvr	mt	EBL	EBT	WBT	WBR	SBLn1 S	SBLn2		
Capacity (veh/h)		452	-	-	-	20	214		
HCM Lane V/C Ratio		0.344	_	_	_		0.898		
HCM Control Delay (s	s)	17.1	_	_		3130.5	84.5		
HCM Lane LOS	,	С	_	_	-	F	F		
HCM 95th %tile Q(veh	h)	1.5	-	-	-	18.2	7.2		
,	,								
Notes		ф. D -	day av-		200	Cama	utation Net Defire	*. All major velvese in -1-1-	<u> </u>
~: Volume exceeds ca	apacity	⊅: De	elay exc	eeds 30	JUS ·	+: Comp	utation Not Defined	*: All major volume in plato	ON

Intersection						
Int Delay, s/veh	2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		^	^	7		7
Traffic Vol, veh/h	0	1478	1139	222	0	233
Future Vol, veh/h	0	1478	1139	222	0	233
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	_	-	_	225	_	-
Veh in Median Storage		0	0	-	0	_
Grade, %	-, "	0	0	_	0	_
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	1642	1266	247	0	259
IVIVIIIL FIUW	U	1042	1200	24 1	U	209
	Major1	ľ	Major2	N	/linor2	
Conflicting Flow All	-	0	-	0	-	633
Stage 1	-	-	-	-	-	-
Stage 2	_	_	-	-	-	-
Critical Hdwy	_	-	-	-	-	6.94
Critical Hdwy Stg 1	_	_	_	_	-	_
Critical Hdwy Stg 2	_	_	_	_	_	_
Follow-up Hdwy	_	_	_	_	_	3.32
Pot Cap-1 Maneuver	0	_	_	_	0	422
Stage 1	0	_	_	_	0	-
Stage 2	0	_	_	_	0	_
Platoon blocked, %	Ū	_	_	_	Ů	
Mov Cap-1 Maneuver	_	_	_	_	_	422
Mov Cap-1 Maneuver	_	_	_	_	_	722
Stage 1	-	-	-	-	-	-
_	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		26.2	
HCM LOS					D	
Minor Lane/Major Mvm	nt	EBT	WBT	WBR S	SBLn1	
Capacity (veh/h)					422	
HCM Lane V/C Ratio		-	-	_	0.613	
HCM Control Delay (s)		-	-	-	26.2	
HCM Lane LOS		-	-	-	20.2 D	
HCM 95th %tile Q(veh)	١	-	-	-	4	
TION SOUT /OUR Q(VEII)	/	-	-	-	4	

ntersection										
t Delay, s/veh	16.2									
ovement	EBL	EBT	WBT	WBR	SBL	SBR				
ne Configurations		^	^	7		7				
fic Vol, veh/h	0	1558	1482	319	0	355				
re Vol, veh/h	0	1558	1482	319	0	355				
flicting Peds, #/hr	0	0	0	0	0	0				
Control	Free	Free	Free	Free	Stop	Stop				
Channelized	_	None	_	None	·-	None .				
age Length	_	_	-	225	-	_				
in Median Storag	ie.# -	0	0	_	0	_				
de, %	-	0	0	_	0	_				
k Hour Factor	90	90	90	90	90	90				
y Vehicles, %	2	2	2	2	2	2				
t Flow	0	1731	1647	354	0	394				
	J				J					
r/Minor	Major1	ı	Major2	N	/linor2					
flicting Flow All		0	-	0	-	824				
Stage 1	_	-	_	-	_	-				
Stage 2										
al Hdwy	_	_	_	_	_	6.94				
al Hdwy Stg 1	_	_	_	_	-	0.34				
	-	-	-	-	-	-				
cal Hdwy Stg 2	-	-	-	-	-	3.32				
w-up Hdwy	-	-	-	-	-	~ 316				
Cap-1 Maneuver	0	-	-	-		~ 310				
Stage 1	0	-	-	-	0	-				
Stage 2	0	-	-	-	0	-				
oon blocked, %	_	-	-	-		040				
Cap-1 Maneuver		-	-	-	-	~ 316				
Cap-2 Maneuver	-	-	-	-	-	-				
Stage 1	-	-	-	-	-	-				
Stage 2	-	-	-	-	-	-				
o o o b	ED		WD		CD					
oach	EB		WB		SB					
M Control Delay, s	0		0		169.8					
M LOS					F					
1 184 1 22			14/5-	14/55	DDI 1					
r Lane/Major Mvi	mt	EBT	WBT	WBR S						
acity (veh/h)		-	-	-	316					
Lane V/C Ratio		-	-		1.248					
Control Delay (s	S)	-	-	-	169.8					
Lane LOS		-	-	-	F					
95th %tile Q(vel	h)	-	-	-	18					
;										
ume exceeds ca	apacity	\$: De	elav exc	eeds 30	00s -	+: Comr	utation Not Defined	*: All major v	olume in nla	toon
57.55545 00	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	ψ. Δ0	, one	2000 00		. 501116	atation not bonnou	. 7 iii 1110joi V	c.ao iii pia	.55.1



ORIGINAL PUD APPROVAL PLAN AND REQUIREMENTS



MCADAMS

C-2

□ McAdams

General Roadway Infrastructure:

All proposed roadway infrastructure will be consistent with the Town of Apex UDO and Transportation Plan (updated in 2011), and the Traffic Impact Analysis approved by the Town of Apex and NCDOT. An internal road network will be provided in accordance with the Town's UDO. All road networks will promote connectivity wherever possible to adjacent neighborhoods, undeveloped property, nearby points of interest, and municipal destinations. Further, cul-de-sacs will be avoided except where environmental features make through streets unfeasible.

Roadway Phasing – Prior to time of the fifty-first certificate of occupancy associated with the residential located off of Old US HWY 1, the second point of access (southernmost portion of Street A), necessary portion of the southernmost roundabout and Street B shown shall be constructed. As a part of the non-residential development in Pod 3, the roundabout proposed on Kelly Road shall be constructed along with the portion of Street A tying back to the southernmost roundabout. Prior to time of the first certificate of occupancy associated with Pod 3, Street A will be complete the connection from Kelly Road and Old US HWY 1.

Off-Site Transportation Conditions:

The project will also provide the following off-site transportation conditions:

All recommendations on state maintained roadways are subject to NCDOT review and approval as part of the driveway permits and encroachment agreements. Build 2018 refers to the first plat of residential development or as otherwise determined by Apex Town Council during the review and approval of subdivision plans. Build 2020 refers to the first plat of commercial development or as otherwise determined by Apex Town Council during the review and approval of commercial site plans. Internal Protected Storage Length (IPS) refers to the required minimum distance from the intersection along the proposed driveway or street before any full movement commercial driveway access or public street intersection will be allowed.

Developer shall provide right-of-way dedication along Kelly Road and Old US 1 based on a 100-foot right-of-way. Where Old US 1 abuts railroad right-of-way the developer shall be responsible for dedicating public right-of-way 70 feet from roadway centerline along the project frontage or as otherwise required to accommodate a 100-foot road right-of-way exclusive of railroad right-of-way.

Street 'A' and Street 'B' (including Kelly Road at Site Drive #4)

 Street 'A' shall be constructed as a 3-lane 38-foot curb and gutter street with 5foot sidewalk on both sides on 62-foot public right-of-way.

- Street 'B' shall be constructed as a 2-lane 39'-foot curb and gutter street with onstreet parking and 6-foot sidewalk on both sides on 53-foot public right-of-way.
- Residential driveway access shall not be permitted along Streets 'A' and 'B'.
- Prior to platting the 51st residential unit in the Residential area located adjacent to Old US 1, developer shall construct and dedicate as public Street 'A' from Site Drive #5 to the roundabout at Street 'B', roundabout serving Street 'A' at Street 'B', and Street 'B' from Site Drive #6/Pleasant Plains Road to the roundabout at Street 'A'.
- Prior to the first certificate of occupancy within POD 3, developer shall construct
 and dedicate as public Street 'A' from the roundabout at Street 'B' to Kelly Road
 at Site Drive #4 and construct a roundabout on Kelly Road at Site Drive #4. The
 roundabout will serve a 4-lane divided roadway to the north and 2-lane roadway
 to the south for Build 2020.

Kelly Road at Olive Chapel Road

 Developer shall construct a 200-foot eastbound right turn lane and a 300-foot additional westbound left turn lane on Olive Chapel Road (with southbound receiving through lane on Kelly Road) for Build 2020.

Kelly Road at Apex Barbecue Road

 Developer shall construct a 400-foot eastbound left turn lane, 350-foot westbound left turn lane, 350-foot northbound left turn lane, 150-foot northbound right turn lane, 350-foot southbound left turn lane, and 200-foot southbound right turn lane for Build 2020.

Kelly Road at Southwinds Run

 Developer shall construct a 100-foot northbound left turn lane on Kelly Road, an additional (second) northbound through lane through the intersection to drop as a right turn at Site Drive #1, and begin an additional (second) southbound through lane immediately south of Southwinds Run for Build 2020.

Old US 1 at Kelly Road

- Developer shall construct a 100-foot westbound right turn lane on Old US 1 for Build 2018.
- Developer shall convert the intersection to right-in/right-out as well as construct an additional westbound through lane, beginning at the NC 540 Southbound offramp as a free-flow right turn exiting the ramp, along with a 200-foot westbound right turn lane on Old US 1 for Build 2020.
- Prior to platting the 300th residential unit, the developer will complete a signal warrant analysis at the intersection of Old US 1 and Kelly Road to determine if a

traffic signal is warranted at the intersection. If the signal is warranted and approved for installation by NCDOT, the developer will permit and install the traffic signal. However, if Street "A" through POD 3 is under construction prior to installation of the signal, then the requirement for the signal shall be waived and the Kelly Road / Old US 1 intersection shall be converted to a Right-In/Right-Out as required in the improvements for the commercial development.

NC 540 Ramps at Old US 1

 Developer shall provide a free-flow right turn lane for the NC 540 Southbound offramp and additional receiving through lane continuing west to drop as a right turn at Site Drive #6 on Old US 1 for Build 2020.

Old US 1 at Pleasant Plains Road & Site Drive #6

- Developer shall construct Site Drive #6 with a through-right lane and a 200-foot left turn lane for Build 2018.
- Developer shall construct a 200-foot eastbound left turn lane and 200-foot westbound left turn lane on Old US 1 for Build 2018.
- Developer shall install a traffic signal once warranted and approved by NCDOT and install communication with the NC 540 traffic signals.
- Developer shall construct an additional westbound through lane on Old US 1 to drop as a right turn lane at Site Drive #6 for Build 2020.
- Developer shall construct an additional eastbound through lane on Old US 1 beginning 400 feet west of Site Drive #6 for Build 2020.

Kelly Road at Site Drive #1

- Developer shall construct Site Drive #1 as a right-in and right-out only providing 100 feet IPS and a 100-foot northbound right turn lane on Kelly Road for Build 2018.
- Developer shall construct an additional northbound through lane on Kelly Road to drop as a right turn lane at Site Drive #1 for Build 2020.

Kelly Road at Site Drive #2

- Developer shall construct Site Drive #2 as a right-in and right-out only providing 100 feet IPS for Build 2018.
- Developer shall construct an additional northbound through lane and an additional southbound through lane on Kelly Road at Site Drive #2 for Build 2020.

Kelly Road at Site Drive #3

- Developer shall construct Site Drive #3 with a 100-foot eastbound right turn lane and a 100-foot westbound right turn lane on the Site Drive #3 approaches adjacent to through-right lanes with 200 feet IPS for Build 2018.
- Developer shall construct 100-foot northbound and 100-foot southbound left turn lanes on Kelly Road for Build 2018.
- Developer shall construct an additional (second) southbound through lane and additional (second) northbound through lane on Kelly Road at Site Drive #3 for Build 2020.

Old US 1 at Site Drive #5

- Developer shall construct Site Drive #5 as a full-movement intersection with 200foot dual southbound left turn lanes and a 200-foot southbound right turn lane providing 300 feet IPS providing connectivity to both the residential and commercial phases for Build 2020.
- Developer shall construct an additional (second) westbound through lane and add a 200-foot westbound right turn lane on Old US 1 for Build 2020.
- Developer shall construct a 300-foot eastbound left turn lane and an additional (second) eastbound through lane on Old US 1 dropping 1000 feet east of the intersection for Build 2020.

Electric Charging Stations:

Developer shall provide 2 charging stations, one within the residential and one within the non-residential for electric vehicles within the overall project. In addition to these committed stations, two additional charging stations will be installed as part of the overall project.

Water & Sewer Utilities:

All water and sanitary sewer service will be provided by the developer and conform to the Town of Apex Public Works and Utilities Department requirements. Preliminary location and tie in points are shown on sheet C-3 and C-4 of the PUD plans. The water extension shown along Kelly Road to Old US HWY 1, alternatively, could be located through Pod – 3 Commercial and southernmost residential accomplishing the intent of the routing shown on sheet C-3 and C-4. The ultimate routing will be dictated by timing of commercial development, roadway construction internal to the site and timing commitment related to the extension. However, this will be coordinated with the Town of Apex at time of site plan and construction documents.



PUBLIC NOTIFICATION OF PUBLIC HEARINGS

CONDITIONAL ZONING #25CZ05 West Village PUD Amendment

Pursuant to the provisions of North Carolina General Statutes §160D-602 and to the Town of Apex Unified Development Ordinance (UDO) Section 2.2.11, notice is hereby given of public hearings before the Planning Board of the Town of Apex. The purpose of these hearings is to consider the following:

Applicants: Trilandco, LLC

Authorized Agent: Matthew Carpenter, Parker Poe & Adams & Bernstein LLP

Property Address: 2517 Kelly Road

Acreage: ±5.8591 acres

Property Identification Number (PIN): 0731434504

2045 Land Use Map Designation: High Density Residential/Office Employment/Commercial Services **Existing Zoning of Property:** Planned Unit Development-Conditional Zoning (PUD-CZ #15CZ33) **Proposed Zoning of Property:** Planned Unit Development-Conditional Zoning (PUD-CZ)

Public Hearing Location: Apex Town Hall

Council Chamber, 2nd Floor

73 Hunter Street, Apex, North Carolina

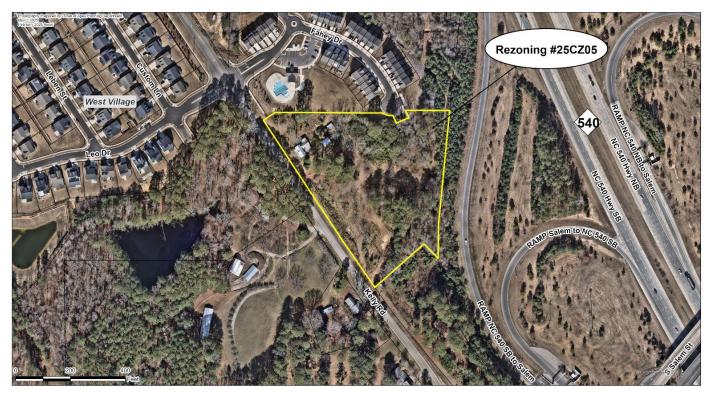
Planning Board Public Hearing Date and Time: May 12, 2025 4:30 PM

You may attend the meeting in person or view the meeting through the Town's YouTube livestream at: https://www.youtube.com/c/townofapexgov.

If you are unable to attend, you may provide a written statement by email to public.hearing@apexnc.org, or submit it to the clerk of the Planning Board, Jeri Pederson (322 N. Mason Street or USPS mail - P.O. Box 250, Apex, NC 27502), at least two business days prior to the Planning Board vote. You must provide your name and address for the record. The written statements will be delivered to the Planning Board prior to their vote. Please include the Public Hearing name in the subject line.

A separate notice of the Town Council public hearing on this project will be mailed and posted in order to comply with State public notice requirements.

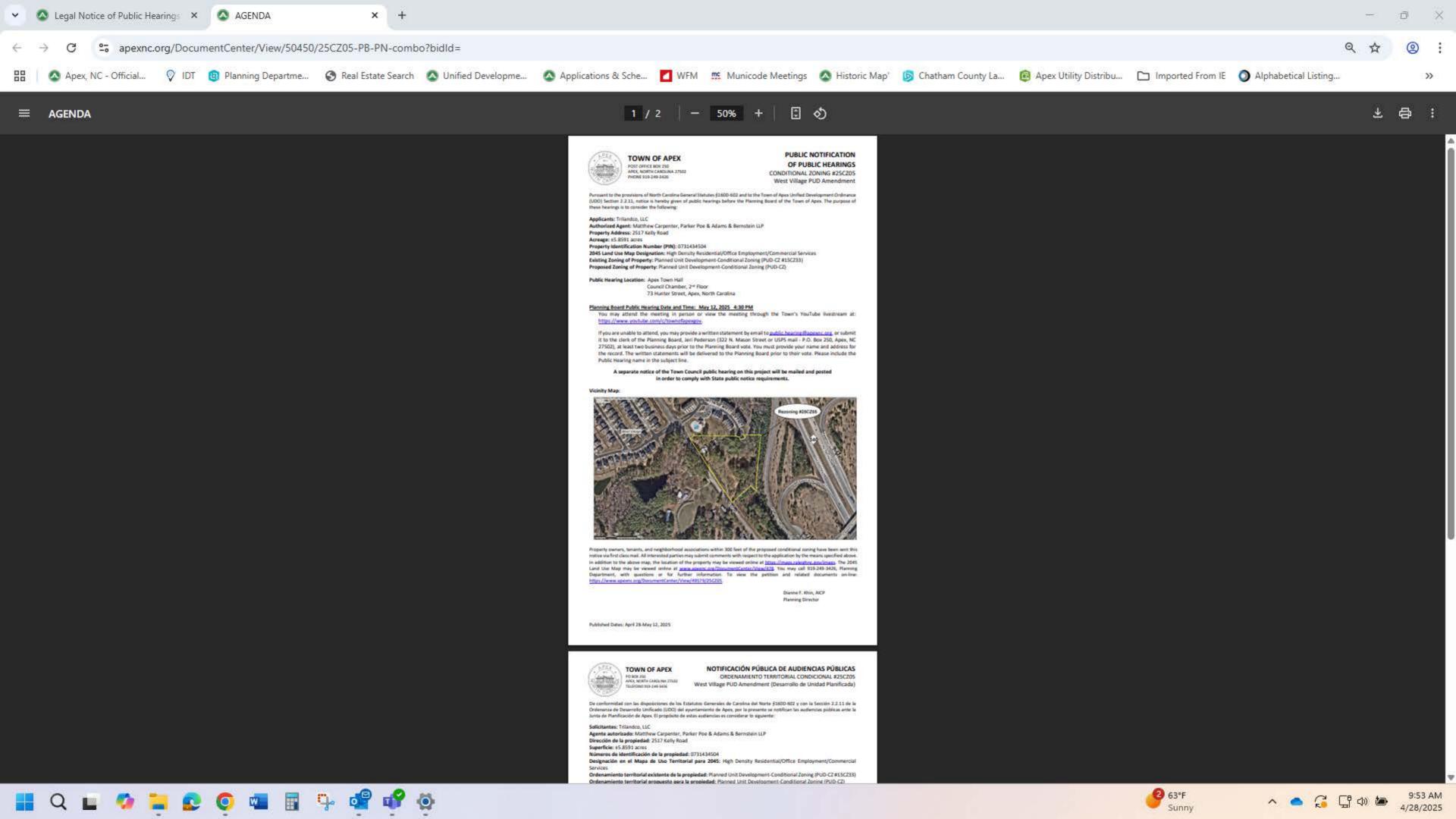
Vicinity Map:



Property owners, tenants, and neighborhood associations within 300 feet of the proposed conditional zoning have been sent this notice via first class mail. All interested parties may submit comments with respect to the application by the means specified above. In addition to the above map, the location of the property may be viewed online at https://maps.raleighnc.gov/imaps. The 2045 Land Use Map may be viewed online at www.apexnc.org/DocumentCenter/View/478. You may call 919-249-3426, Planning Department, with questions or for further information. To view the petition and related documents on-line: https://www.apexnc.org/DocumentCenter/View/49579/25CZ05.

Dianne F. Khin, AICP Planning Director

Published Dates: April 28-May 12, 2025



TOWN OF APEX PO BOX 250 APEX, NORTH CAROLINA 27502 TELÉFONO 919-249-3426

NOTIFICACIÓN PÚBLICA DE AUDIENCIAS PÚBLICAS

ORDENAMIENTO TERRITORIAL CONDICIONAL #25CZ05 West Village PUD Amendment (Desarrollo de Unidad Planificada)

De conformidad con las disposiciones de los Estatutos Generales de Carolina del Norte §160D-602 y con la Sección 2.2.11 de la Ordenanza de Desarrollo Unificado (UDO) del ayuntamiento de Apex, por la presente se notifican las audiencias públicas ante la Junta de Planificación de Apex. El propósito de estas audiencias es considerar lo siguiente:

Solicitantes: Trilandco, LLC

Agente autorizado: Matthew Carpenter, Parker Poe & Adams & Bernstein LLP

Dirección de la propiedad: 2517 Kelly Road

Superficie: ±5.8591 acres

Números de identificación de la propiedad: 0731434504

Designación en el Mapa de Uso Territorial para 2045: High Density Residential/Office Employment/Commercial

Services

Ordenamiento territorial existente de la propiedad: Planned Unit Development-Conditional Zoning (PUD-CZ #15CZ33)
Ordenamiento territorial propuesto para la propiedad: Planned Unit Development-Conditional Zoning (PUD-CZ)

Lugar de la audiencia pública: Ayuntamiento de Apex

Cámara del Consejo, 2º piso

73 Hunter Street, Apex, Carolina del Norte

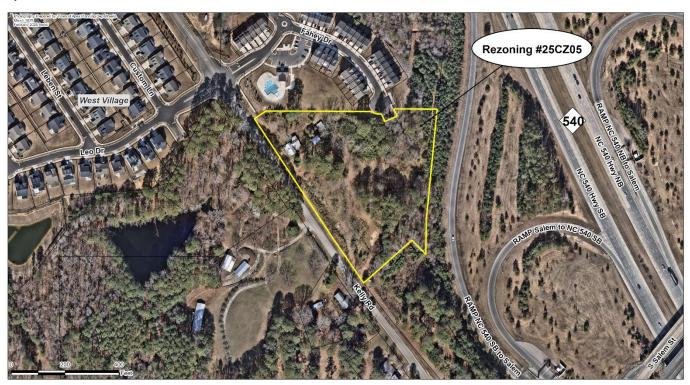
Fecha y hora de la audiencia pública de la Junta de Planificación: 12 de mayo de 2025 4:30 P.M.

Puede asistir a la reunión de manera presencial o seguir la transmisión en directo por YouTube a través del siguiente enlace: https://www.youtube.com/c/townofapexgov.

Si no puede asistir, puede enviar una declaración escrita por correo electrónico a <u>public.hearing@apexnc.org</u>, o presentarla a la secretaría de la Junta de Planificación, Jeri Pederson (322 N. Mason Street o por correo USPS a P.O. Box 250, Apex, NC 27502), al menos dos días hábiles antes de la votación de la Junta de Planificación. Debe proporcionar su nombre y dirección para que conste en el registro. Las declaraciones escritas se entregarán a la Junta de Planificación antes de la votación. No olvide incluir el nombre de la audiencia pública en el asunto.

De conformidad con los requisitos estatales de notificaciones públicas, se enviará por correo y se publicará por separado una notificación de la audiencia pública del Consejo Municipal sobre este proyecto.

Mapa de las inmediaciones:

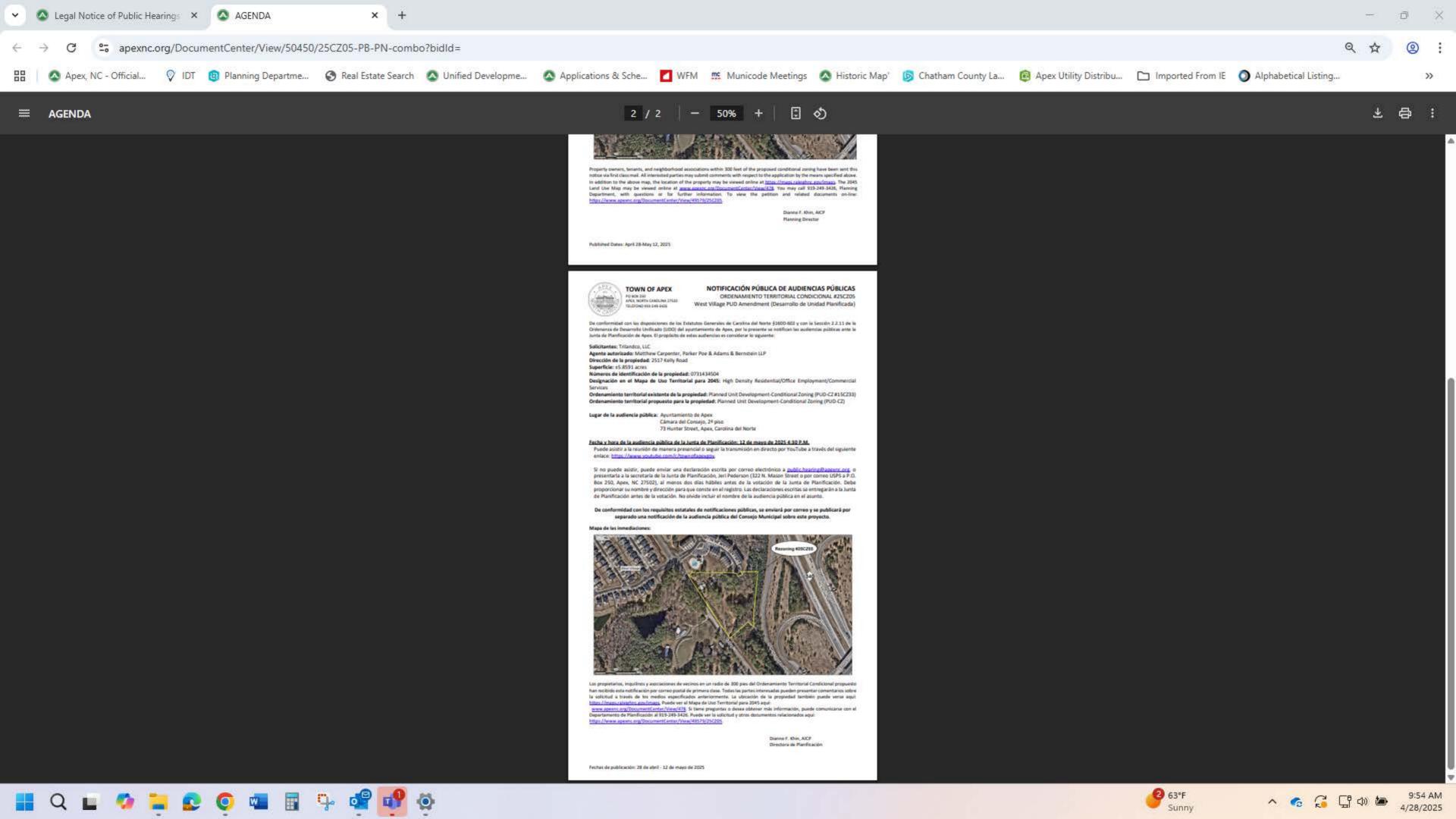


Los propietarios, inquilinos y asociaciones de vecinos en un radio de 300 pies del Ordenamiento Territorial Condicional propuesto han recibido esta notificación por correo postal de primera clase. Todas las partes interesadas pueden presentar comentarios sobre la solicitud a través de los medios especificados anteriormente. La ubicación de la propiedad también puede verse aquí: https://maps.raleighnc.gov/imaps. Puede ver el Mapa de Uso Territorial para 2045 aquí:

<u>www.apexnc.org/DocumentCenter/View/478</u>. Si tiene preguntas o desea obtener más información, puede comunicarse con el Departamento de Planificación al 919-249-3426. Puede ver la solicitud y otros documentos relacionados aquí: https://www.apexnc.org/DocumentCenter/View/49579/25CZ05.

Dianne F. Khin, AICP Directora de Planificación

Fechas de publicación: 28 de abril - 12 de mayo de 2025





TOWN OF APEX

POST OFFICE BOX 250 APEX, NORTH CAROLINA 27502 PHONE 919-249-3426

AFFIDAVIT CERTIFYING Public Notification - Written (Mailed) Notice

Section 2.2.11

Town of Apex Unified Development Ordinance

Project Name:

Conditional Zoning # 25CZ05 West Village PUD Amendment

Project Location:

2517 Kelly Road

Authorized Agent:

Matthew Carpenter

Firm:

Parker Poe & Adams Bernstein LLP

Planning Board

May 12, 2025

Public Hearing Date: Project Planner:

Lauren Staudenmaier

This is to certify that I, as Planning Director, mailed or caused to have mailed by first class postage for the above mentioned project on April 28, 2025, a notice containing the time and place, location, nature and scope of the application, where additional information may be obtained, and the opportunity for interested parties to be heard, to the property owners and tenants within 300' of the land subject to notification. I further certify that I relied on information from the Wake County Tax Assessor and the Town of Apex Master Address Repository provided to me by Town of Apex GIS Staff as to accuracy of the list and accuracy of mailing addresses of property owners and tenants within 300' of the land subject to notification.

4/28/2025

Starre

STATE OF NORTH CAROLINA **COUNTY OF WAKE**

Sworn and subscribed before me,

LAURENT SISSON, a Notary Public for the above

28th day of APRIL , 2025.

State and County, this the

Notary Public

LAUREN J SISSON Notary Public - North Carolina **Wake County** Commission Expires Oct 3, 2027

My Commission Expires: 10 / 03 / 2027



PUBLIC NOTIFICATION OF PUBLIC HEARINGS

CONDITIONAL ZONING #25CZ05
West Village PUD Amendment

Pursuant to the provisions of North Carolina General Statutes §160D-602 and to the Town of Apex Unified Development Ordinance (UDO) Section 2.2.11, notice is hereby given of public hearings before the Town Council of the Town of Apex. The purpose of these hearings is to consider the following:

Applicants: Trilandco, LLC

Authorized Agent: Matthew Carpenter, Parker Poe

Property Address: 2517 Kelly Road

Acreage: ±5.8591 acres

Property Identification Number (PIN): 0731434504

2045 Land Use Map Designation: High Density Residential/Office Employment/Commercial Services **Existing Zoning of Property:** Planned Unit Development- Conditional Zoning (PUD-CZ #15CZ33) **Proposed Zoning of Property:** Planned Unit Development-Conditional Zoning (PUD-CZ)

Public Hearing Location: Apex Town Hall

Council Chamber, 2nd Floor

73 Hunter Street, Apex, North Carolina

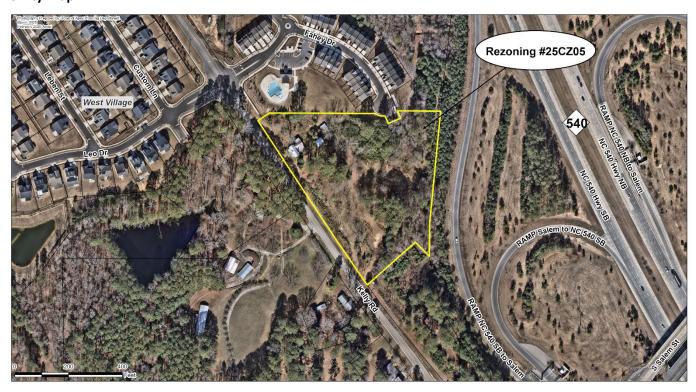
Comments received prior to the Planning Board public hearing will not be provided to the Town Council. Separate comments for the Town Council public hearing must be provided by the deadline specified below.

Town Council Public Hearing Date and Time: May 27, 2025 6:00 PM

You may attend the meeting in person or view the meeting through the Town's YouTube livestream at: https://www.youtube.com/c/townofapexgov.

If you are unable to attend, you may provide a written statement by email to public.hearing@apexnc.org, or submit it to the Office of the Town Clerk (73 Hunter Street or USPS mail - P.O. Box 250, Apex, NC 27502), at least two business days prior to the Town Council vote. You must provide your name and address for the record. The written statements will be delivered to the Town Council prior to their vote. Please include the Public Hearing name in the subject line.

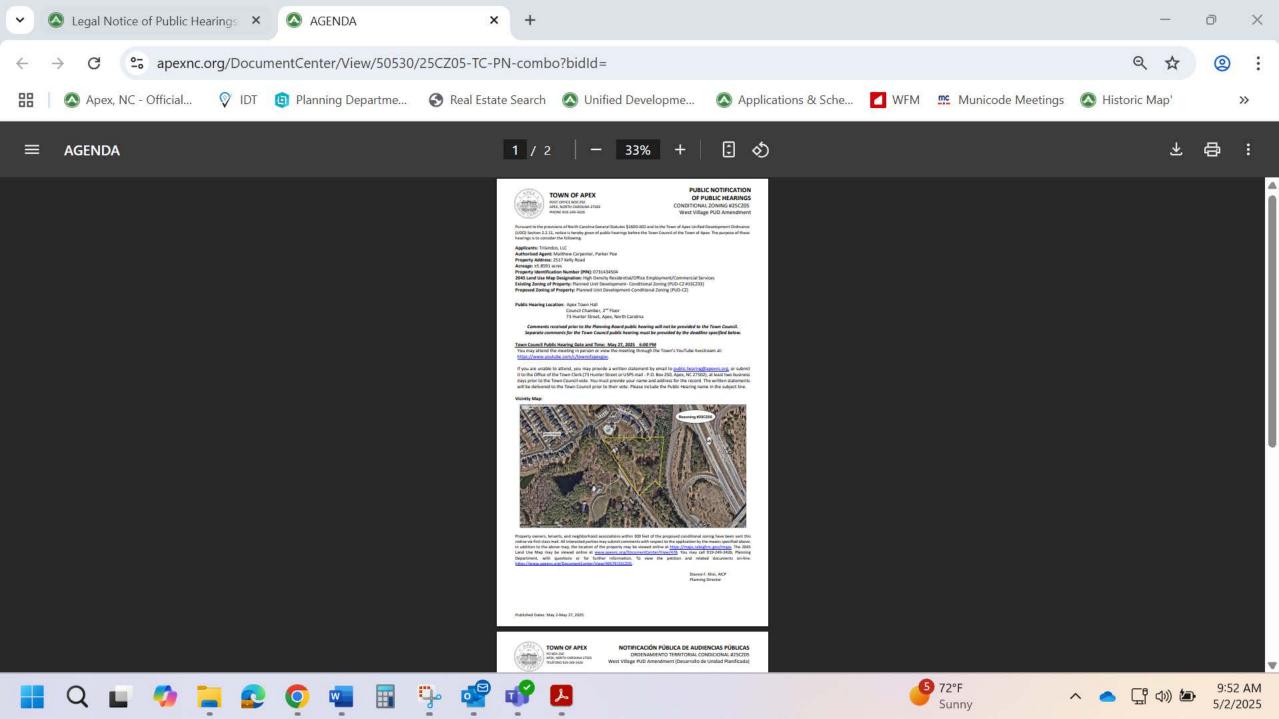
Vicinity Map:



Property owners, tenants, and neighborhood associations within 300 feet of the proposed conditional zoning have been sent this notice via first class mail. All interested parties may submit comments with respect to the application by the means specified above. In addition to the above map, the location of the property may be viewed online at https://maps.raleighnc.gov/imaps. The 2045 Land Use Map may be viewed online at https://www.apexnc.org/DocumentCenter/View/apexnc.org/DocumentCenter/View/aps/25CZ05. You may call 919-249-3426, Planning Department, with questions or for further information. To view the petition and related documents on-line: https://www.apexnc.org/DocumentCenter/View/49579/25CZ05.

Dianne F. Khin, AICP Planning Director

Published Dates: May 2-May 27, 2025



TOWN OF APEX PO BOX 250 APEX, NORTH CAROLINA 27502 TELÉFONO 919-249-3426

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Solicitantes: Trilandco, LLC

Agente autorizado: Matthew Carpenter, Parker Poe

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Lugar de la audiencia pública: Ayuntamiento de Apex Cámara del Consejo, 2º piso

73 Hunter Street, Apex, Carolina del Norte

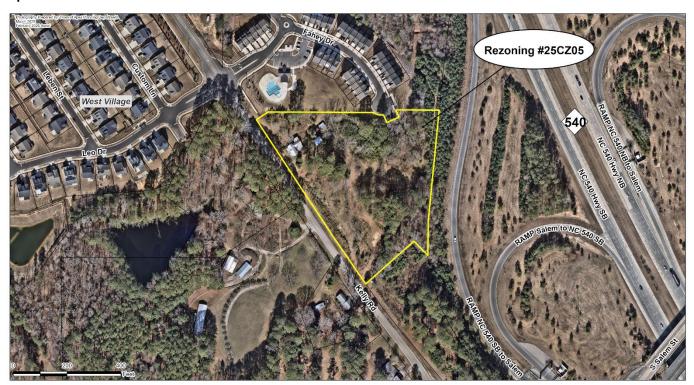
Los comentarios recibidos antes de la audiencia pública de la Junta de Planificación no se proporcionarán al Consejo Municipal. Los comentarios para la audiencia pública del Consejo Municipal deben presentarse por separado en el plazo especificado a continuación.

Fecha y hora de la audiencia pública del Consejo Municipal: 27 de mayo de 2025 6:00 P.M.

Puede asistir a la reunión de manera presencial o seguir la transmisión en directo por YouTube a través del siguiente enlace: https://www.youtube.com/c/townofapexgov.

Si no puede asistir, puede enviar una declaración escrita por correo electrónico a public.hearing@apexnc.org, o presentarla a la oficina del Secretario Municipal (73 Hunter Street o por correo USPS a P.O. Box 250, Apex, NC 27502), al menos dos días hábiles antes de la votación del Consejo Municipal. Debe proporcionar su nombre y dirección para que conste en el registro. Las declaraciones escritas se entregarán al Consejo Municipal antes de la votación. No olvide incluir el nombre de la audiencia pública en el asunto.

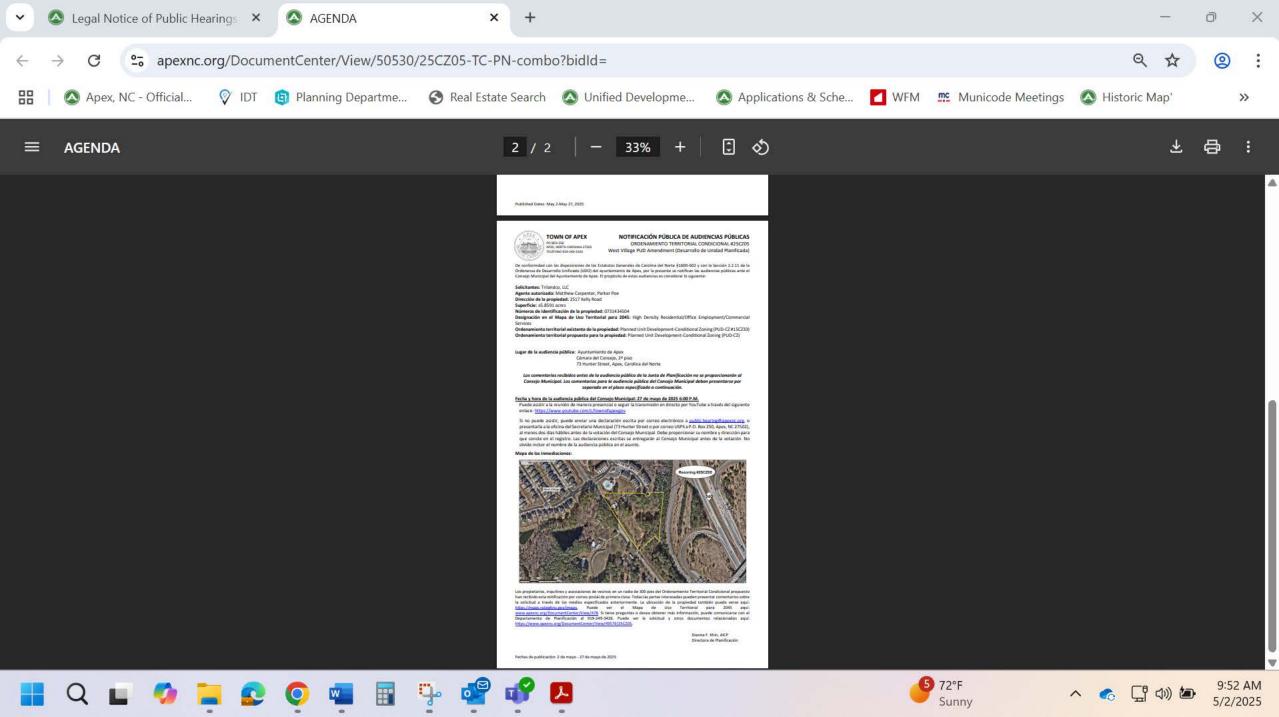
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Dianne F. Khin, AICP Directora de Planificación

Fechas de publicación: 2 de mayo - 27 de mayo de 2025





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Project Location:

2517 Kelly Road

Authorized Agent:

Matthew Carpenter

Firm:

Parker Poe

Town Council

May 27, 2025

Public Hearing Date:

Project Planner:

Lauren Staudenmaier

This is to certify that I, as Planning Director, mailed or caused to have mailed by first class postage for the above mentioned project on May 2, 2025, a notice containing the time and place, location, nature and scope of the application, where additional information may be obtained, and the opportunity for interested parties to be heard, to the property owners and tenants within 300' of the land subject to notification. I further certify that I relied on information from the Wake County Tax Assessor and the Town of Apex Master Address Repository provided to me by Town of Apex GIS Staff as to accuracy of the list and accuracy of mailing addresses of property owners and tenants within 300' of the land subject to notification.

5/5/2025

Channe F. Khin

STATE OF NORTH CAROLINA **COUNTY OF WAKE**

Sworn and subscribed before me,

uren J Sissan , a Notary Public for the above

State and County, this the

5th day of May

, 202 5 .

LAUREN J SISSON Notary Public - North Carolina Wake County My Commission Expires Oct 3, 2027 **Notary Public**

My Commission Expires: (0 / 03 / 2021

