# FJS

Memo	
Date:	Friday, September 01, 2023
Project:	Mulberry Fields Subdivision CAF (Capacity Acquisition Fee)
To:	Otis Spriggs, Director of Development Services
From:	Javier Vasquez, P.E., CFM
Subiect:	Water and Wastewater Capacity Acquisition Fee (41 ESUs)

The City of Angleton has coordinated with a Developer for Mulberry Fields subdivision, a proposed subdivision located on the west side of the City near the SH 288 and SH 35 (W. Mulberry St.) intersection. The proposed development of 41 single-family homes is generally bound by W. Live Oak Street to the north, W. Mulberry Street to the south, N. Walker Street to the east and Western Avenue to the west. Based on this information and using the planning criteria for water demand and sewer loading from the Utility Master Plan, below is the summary of the assumptions, analysis and model results. Capacity demand for this development was assessed based solely on the number of proposed residences and does not include loading from the proposed recreational center that is proposed for the subdivision.

## **Capacity Verification**

- Water Demand
  - Average Daily Demand (ADD): 300 gallons per day per connection, 41 x 300 = 12,300 gpd or 8.54 gpm
  - Max Daily Demand (MDD): 1.7 x ADD = 14.52 gpm
  - Peak Hour Demand (PHD): 1.25 x MDD = 18.15 gpm
- Water Model Run
  - The Subdivision will be required to have two points of connection to the existing water system. The Subdivision will propose a connection point at the existing 6-inch water line along the east side of Walker Street and shall coordinate a connection to the existing 8-inch water line along the south side of W. Mulberry Street in order to create a looped system for the Subdivision.
  - The existing water model was run for the above noted scenario. The model shows that there is sufficient pressure and fire flow when the system is looped as noted (See Exhibit #2).
- Wastewater Flows
  - Average Daily Flow (ADF): 255 gallons per day per connection, 41 x 255 = 10,455 gpd or 7.26 gpm

- Peak Hour Wet Weather Flow (PWF): 4 x ADF = 29.04 gpm
- Wastewater Model Run
  - The existing model was run for PWF scenario, which uses a peaking factor of 4.
  - It was assumed in the model that the wastewater for the subdivision will be collected and discharged at the manhole located on the west side of N. Walker Street. This manhole discharges into a 12" sewer line that runs south along N Walker Street towards W. Mulberry Street (SH 35). The line crosses W. Mulberry Street (SH 35) and continues as an 8-inch to 12-inch gravity main south along S. Walker Street, and discharges into Lift Station No. 3 (900 S Walker).
  - Wastewater is pumped from Lift Station No. 3 and collected into an existing 30-inch gravity sewer main along Munson Place. This existing 30-inch gravity main runs west along Munson Place and continues west towards and crossing SH 288 through easements along an existing drainage channel and discharges at the Oyster Creek WWTP along Sebesta Road.
  - Based on the model run, there is an existing section of 24-inch pipe within the WWTP that will require upsizing. This is an item identified in the utility master plan and is recommended to be upsized in the near future.

## Capacity Acquisition Fee:

Please see Appendix "A" for the calculations for the Capacity Acquisition Fee.

- Water Service
  - The City has adopted a flat fee of \$536.70 per ESU for water service throughout the City.
- Wastewater Service
  - Total Capacity of 8" Sanitary Sewer set at TCEQ minimum slope is approximately 314 gpm
    - Percentage utilization of 8" sanitary sewer is 9.24% (peak flow)
  - Total Capacity of 12" Sanitary Sewer set at TCEQ minimum slope is approximately 715 gpm
    - Percentage utilization of 12" sanitary sewer is 4.06% (peak flow)
  - Total Capacity of 24" Sanitary Sewer set at TCEQ minimum slope is approximately 2,871 gpm
    - Percentage utilization of 30" sanitary sewer is 1.01% (peak flow)
  - Total Capacity of 30" Sanitary Sewer set at TCEQ minimum slope is approximately 4,508 gpm
    - Percentage utilization of 30" sanitary sewer is 0.64% (peak flow)
  - Total Capacity of 36" Sanitary Sewer set at TCEQ minimum slope is approximately 6,348 gpm

- Percentage utilization of 36" sanitary sewer is 0.46% (peak flow)
- Total Firm Capacity (taken from drawdown test) of Lift Station No. 3 is approximately 280 gpm.
  - Based on the assumed capacity of the lift station, the percent utilization of LS No. 3 pumping capacity and 4" force main is 10.37% (peak flow).
- Fee for sewer service is \$861.58 per ESU.

Therefore, the combined cost per ESU (water and wastewater) will be approximately \$1,398.28. The total fee for the projected 41 homes for Mulberry Fields Subdivision is approximately \$57,329.48.

ATTACHMENTS

Appendix "A" – Capacity Acquisition Fee Calculations

Exhibit 1 – Water System Map (Existing)

Exhibit 2 – Water Model Map (Proposed)

Exhibit 3 – Wastewater System Sewer Trace and Flow Capacity

APPENDIX A - PROPOSED COST PER CONNECTION
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		Current		ENR Value for	Ectimated			Total Estimated			
		Construction	Voar			Istinated	Number of			Production	Cost por ESU
Accet Name		Cost Estimate	Constructed	Vear	Vear of Construction			Cost		(gnd)	(1  FSU = 300  gnd)
Asset Name		Cost Estimate	constructed	Tear			Assets	6031		(694)	(1 130 - 300 860)
		2 000 000	1099	/510	ć	825 002	1	ć	825 002		
		5 2,000,000	2006	4319	ې د	26 204	2	ې د	72 609		
850 gpm pumps		51,250	2008	8802	ې د	30,304 /1 227	2	ې د	123 680		
Total Henderson Water Plant	, 	5 51,250	2010	8602	ې ا	41,227		ې د	1 022 280	3 672 000	\$83.52
Chenango Water Plant								<b>Ý</b>	1,022,200	3,072,000	<del>,03.52</del>
		2 000 000	1053	600	ć	109 669	1	ć	109 669		
850 gpm pumps		5 2,000,000 5 51 250	2005	7446	ې د	34 875	3	\$ 104,626			
Total Chenango Water Plant		51,230	2003	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Ŷ	51,675		\$	214.296	3.672.000	\$17.51
Jamison Water Plant									,	-,,	
450k GST		987.500	2009	8570	Ś	773.430	1	Ś	\$ 773.430		
850 gpm pumps		51.250	2015	10035	Ś	47.002	3	\$ 141.005			
10k Hydro Tanks		5 77,500	2009	8570	\$	60,700	2	\$	121,399		
Total Jamison Water Plant								\$	1,035,835	3,672,000	\$84.63
Water Well #11		1,062,500	1985	4195	\$	407,347	1	\$	407,347	1,224,000	\$99.84
		Current		ENR Value for	ENR Value for <sup>1</sup> Estimated			Total Estimated			
		Construction	Year	Construction Construction Cost in		Number of	Construction		Production	Cost per ESU	
Asset Name	Name Cost Estimate Constructed Year Year of Construct		ar of Construction	Assets		Cost	(gpd)	(1 ESU = 200 gpd)			
Northside EST	4	2,000,000	1961	847	\$	154,816	1	\$	154,816	500,000	\$61.93
Southside EST	4	2,000,000	1977	2576	\$	470,846	1	\$	470,846	500,000	\$188.34
<sup>1</sup> Total Cost Per Connection for Water Purchased From Brazosport Water Authority (BWA)									\$0.94		
2											
<sup>2</sup> Total Estimated Cost Per Water Connection									\$536.70		

#### Wastewater Plants

	Current Construction	Year	ENR Value for Construction	<sup>3</sup> Estimated Construction Cost in	Number of	Total Estimated Construction		Cost per ESU
Asset Name	Cost Estimate	Constructed	Year	Year of Construction	Assets	Cost	Production (gpd)	(1 ESU = 255 gpd)
Oyster Creek Sanitary Sewer Treatment Plant	\$ 36,000,000	1980	3237	\$ 9,214,201	1	\$ 9,214,201	3,600,000	\$ 652.67

#### Wastewater Infrastructure

		Current		ENR Value for	<sup>3</sup> Estimated		Tot	al Estimated			
		Construction	Est. Year	Construction Construction Cost in		% of	Construction		Development	Cost per ESU	
Asset Name		Cost Estimate	Constructed	Year	Year of Construction	Capacity		Cost	ESU's	(1 ESU = 255 gpd)	
Gravity Sewer											
8" Main (2,610 feet)	\$	195,750	1970	1381	\$ 21,375	9.24%	\$	1,976		\$	48.19
12" Main (1,055 feet)	\$	131,875	1970	1381	\$ 14,400	4.06%	\$	585		\$	14.27
24" Main (560 feet)	\$	155,000	1970	1381	\$ 16,925	1.01%	\$	171		\$	4.18
30" Main (5,870 feet)	\$	2,233,150	1970	1381	\$ 243,851	0.64%	\$	1,571		\$	38.32
36" Main (20 feet)	\$	7,000	1970	1381	\$ 764	0.46%	\$	3		\$	0.09
Total Gravity Sewer							\$	4,307	] '	\$	105.04
Force Main									41		
4" Force Main (11 feet)	\$	1,000	1970	1381	\$ 109	10.37%	\$	11		\$	0.28
Total Force Main							\$	11		\$	0.28
Lift Station									]		
No. 3	\$	375,000	1970	1381	\$ 40,948	10.37%	\$	4,247		\$	103.59
Total Lift Station							\$	4,247	]	\$	103.59
Total Wastewater Infrastructure			Γ				\$	8,565	1	\$	208.91
					Total	Fatimated	Cost F				<u> </u>
					IUldi	Estimateu v	COSLE	er wastewa	iter connection		<b>2001.20</b>

<sup>1</sup> The City purchases approximately 1.8 MGD from BWA which is provided at a rate of \$3.12 per 1,000 gallons. Therefore, one (1) ESU or 300 gallons, is approixmately \$0.94.

<sup>2</sup> The cost shown is the adopted flat fee per ESU for water service.

<sup>3</sup> The cost shown is taken by dividing the current construction cost estimate by the Nov 2021 ENR Value of 12647.





1 inch equals 400 feet 0 200 400 800 Feet

Mulberry Fields Subdivision - Existing System Model System Pressure and Available Fire Flow **EXHIBIT 1** 





1 inch equals 400 feet 0 200 400 800 Feet

City of Angleton, Texas - Water System Modeling Mulberry Fields Subdivision - Post Development System Model System Pressure and Available Fire Flow **EXHIBIT 2** 



# **EXHIBIT 3**