13

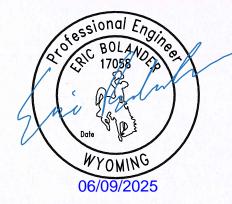
JOB TITLE	Wooden Spur	r Renovation		
	368 Wppdem Spur Dr. Alpine Wy.			
JOB NO.	225112	SHEET NO.	1.0	
CALCULATED BY	EVB	DATE	6/4/25	
CHECKED BY		DATE		

STRUCTURAL CALCULATIONS

FOR

Wooden Spur Renovation

368 Wppdem Spur Dr. Alpine Wy.



JOB TITLE Wooden Spur Renovation

 Wooden Spur Renovation

 368 Wppdem Spur Dr. Alpine Wy.

 305112

 SHEET NO.

 DATE

 JOB NO. 225112 CALCULATED BY EVB CHECKED BY DATE

CODE SUMMARY

<u>Code:</u>	momationa	I Building Co	
Live Loads:			
Roof 0 to 200 sf:			
		rea, but not l	ess than 12 psf
over 600 sf:	12 psf		
Typical Floor	40 psf		
Partitions	N/A		
All other residential areas except bal			
Habitable attics & sleeping areas	30 psf		
Attics without storage	10 psf		
Decks (1.5 times live load)	60 psf		
Dead Loads:			
Floor	10.0 psf		
Roof	15.0 psf		
	. 5.6 por		
Roof Snow Loads:			
Design Uniform Roof Snow load	=	100.0 psf	
Flat Roof Snow Load	Pf =	98.0 psf	
Balanced Snow Load	Ps =	98.0 psf	
Ground Snow Load	Pg =	140.0 psf	
Importance Factor	=	1.00	
Snow Exposure Factor	Ce =	1.00	
Thermal Factor	Ct =	1.00	
Sloped-roof Factor	Cs =	1.00	
Drift Surcharge load Width of Snow Drift	Pd = w =		
Width of Show Dhit	w –		
Earthquake Design Data:			
Risk Category	=	11	
Importance Factor	=	1.00	
Mapped spectral response accelerat		110.90	
	S1 =	34.10	
Site Class		code default	
Spectral Response Coef.	Sds =	0.887	
Colomia Daolan October	Sd1 =	0.445	
Seismic Design Category	=	D Bearing Wal	Systems
Basic Structural System			wood) walls with structural wood shear papels
Basic Structural System Seismic Resisting System	=	Light frame (wood) walls with structural wood shear panels
Basic Structural System Seismic Resisting System Seismic Response Coef.	= Cs =	Light frame (0.136	wood) walls with structural wood shear panels
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor	= Cs = R =	Light frame (0.136 6.5	
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor	= Cs = R =	Light frame (0.136 6.5	wood) walls with structural wood shear panels ateral-Force Analysis
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor Analysis Procedure	= Cs = R =	Light frame (0.136 6.5	
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor Analysis Procedure Rain Design Data:	= Cs = R =	Light frame (0.136 6.5	
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor Analysis Procedure <u>Rain Design Data:</u> Rain intensity	= Cs = R = =	Light frame (0.136 6.5 Equivalent L	
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor Analysis Procedure Rain Design Data: Rain intensity Rain Load	= Cs = R = = <i>i</i> =	Light frame (0.136 6.5 Equivalent L 7.23 in/hr	
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor Analysis Procedure <u>Rain Design Data:</u> Rain intensity Rain Load <u>Wind Design Data:</u>	= Cs = R = = <i>i</i> = R =	Light frame (0.136 6.5 Equivalent L 7.23 in/hr 24.4 psf	
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor Analysis Procedure Rain Design Data: Rain intensity Rain Load <u>Wind Design Data:</u> Ultimate Design Wind Speed	= Cs = R = = i = R =	Light frame (0.136 6.5 Equivalent L 7.23 in/hr 24.4 psf mph	
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor Analysis Procedure Rain Design Data: Rain intensity Rain Load <u>Wind Design Data:</u> Ultimate Design Wind Speed Nominal Design Wind Speed	= Cs = R = = i= R = 120 92.95	Light frame (0.136 6.5 Equivalent L 7.23 in/hr 24.4 psf mph	
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor Analysis Procedure Rain Design Data: Rain intensity Rain Load Wind Design Data: Ultimate Design Wind Speed Nominal Design Wind Speed Risk Category	= Cs = R = = R = 120 92.95 II	Light frame (0.136 6.5 Equivalent L 7.23 in/hr 24.4 psf mph	
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor Analysis Procedure Rain Design Data: Rain intensity Rain Load Wind Design Data: Ultimate Design Wind Speed Nominal Design Wind Speed Risk Category Mean Roof Ht (h)	= Cs = R = = R = 120 92.95 II 17.0 ft	Light frame (0.136 6.5 Equivalent L 7.23 in/hr 24.4 psf mph	
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor Analysis Procedure Rain Design Data: Rain intensity Rain Load Wind Design Data: Ultimate Design Wind Speed Nominal Design Wind Speed Risk Category Mean Roof Ht (h) Exposure Category	= Cs = R = = i = R = 120 92.95 II 17.0 ft C	Light frame (0.136 6.5 Equivalent L 7.23 in/hr 24.4 psf mph mph	
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor Analysis Procedure Rain Design Data: Rain intensity Rain Load Wind Design Data: Ultimate Design Wind Speed Nominal Design Wind Speed Risk Category Mean Roof Ht (h) Exposure Category Enclosure Classif.	= Cs = R = = i = R = 120 92.95 II 17.0 ft C Enclosed Bu	Light frame (0.136 6.5 Equivalent L 7.23 in/hr 24.4 psf mph mph	
Basic Structural System Seismic Resisting System Seismic Response Coef. Response Modification Factor Analysis Procedure Rain Design Data: Rain intensity Rain Load Wind Design Data: Ultimate Design Wind Speed Nominal Design Wind Speed Risk Category Mean Roof Ht (h) Exposure Category	= Cs = R = = i = R = 120 92.95 II 17.0 ft C	Light frame (0.136 6.5 Equivalent L 7.23 in/hr 24.4 psf mph mph	

JOB TITLE Wooden Spur Renovation 368 Wppdem Spur Dr. Alpine Wy.

JOB NO. 225112 SHEET NO. 26 39 CALCULATED BY EVB DATE 6/4/25 CHECKED BY DATE

Component and Cladding Nominal Wind Pressures

Roof	Surface Pressure (psf)							
Area	2 sf	10 sf	20 sf	50 sf	75 sf	100 sf	200 sf	250 sf
Negative Zone 1 & 2e	-29.1	-29.1	-29.1	-17.7	-12.7	-10.0	-10.0	-10.
Negative Zone 2n, 2r &3e	-42.4	-42.4	-36.7	-29.1	-25.7	-23.3	-17.6	-15.
Negative Zone 3r	-50.5	-50.5	-43.2	-33.7	-29.4	-26.4	-26.4	-26.4
Positive All Zones	11.7	10.0	10.0	10.0	10.0	10.0	10.0	10.0
Overhang Zone 1 & 2e	-33.4	-33.4	-33.4	-25.8	-22.4	-20.0	-20.0	-20.0
Overhang Zone 2n & 2r	-46.7	-46.7	-42.4	-36.7	-34.2	-32.4	-28.1	-26.
Overhang Zone 3e	-54.7	-54.7	-47.3	-37.4	-33.0	-29.9	-22.4	-20.0
Overhang Zone 3r	-62.7	-62.7	-53.1	-40.3	-34.7	-30.7	-30.7	-30.
				2 2 3 1 4				

Overhang soffit pressure equals adj wall pressure (which includes internal pressure of 2.4 psf)

	Parapet		Solid Pa	rapet Press	ure (psf)		
	Area	10 sf	20 sf	50 sf	100 sf	250 sf	500 sf
CASE A:	Zone 2e :	0.0	0.0	0.0	0.0	0.0	0.0
Zone	2n, 2r & 3e :	0.0	0.0	0.0	0.0	0.0	0.0
	Zone 3r :	0.0	0.0	0.0	0.0	0.0	0.0
CASE B: I	nterior zone :	0.0	0.0	0.0	0.0	0.0	0.0
(Corner zone :	0.0	0.0	0.0	0.0	0.0	0.0

Wall	Surfac	e Pressure	(psf)	
Area	10 sf	100 sf	200 sf	500 sf
Negative Zone 4	-15.6	-13.5	-12.9	-12.0
Negative Zone 5	-19.2	-15.0	-13.7	-12.0
Positive Zone 4 & 5	14.4	12.3	11.7	10.8

JOB TITLE Wooden Spur Renovation 368 Wppdem Spur Dr. Alpine Wy. SHEET NO. 306 39 JOB NO. 225112 CALCULATED BY EVB DATE 6/4/25 CHECKED BY DATE

Code Search

Code: International Building Code 2021

Occupancy:

R Occupancy Group = Residential

Risk Category & Importance Factors:

Risk Category =	П	
Wind factor =	1.00	use 0.60 NOTE: Output will be nominal wind pressures
Snow factor =	1.00	
Seismic factor =	1.00	

Type of Construction:

Fire Rating:		
	Roof =	0.0 hr
	Floor =	0.0 hr

Building Geometry:

·Roof angle (θ)	2.00 / 12	9.5 deg
Building length	26.0 ft	
Least width	38.0 ft	
Mean Roof Ht (h)	17.0 ft	
Parapet ht above grd	0.0 ft	
Minimum parapet ht	0.0 ft	

Live Loads:

Roof	0 to 200 sf:	20 psf
	200 to 600 sf:	24 - 0.02Area, but not less than 12 psf
	over 600 sf:	12 psf

Floor:

Typical Floor	40 psf
Partitions	N/A
All other residential areas except balc	40 psf
Habitable attics & sleeping areas	30 psf
Attics without storage	10 psf
Decks (1.5 times live load)	60 psf

TSE Engineers

136 S. State Street Shelley, Idaho 83274 208-357-2420

ē =

ł = $z_{min} =$ $c = g_Q, g_v =$

L_z =

Q =

 $I_z =$

G =

JOB TITLE Wooden Spur Renovation 368 Wppdem Spur Dr. Alpine Wy. SHEET NO. 4 07 39 DATE 6/4/25 JOB NO. 225112 CALCULATED BY EVB DATE CHECKED BY

Wind Loads :	ASCE 7- 16	
Ultimate Wind Speed Nominal Wind Speed Risk Category Exposure Category Enclosure Classif. Internal pressure Directionality (Kd) Kh case 1 Kh case 2	120 mph 93 mph II C Enclosed Building +/-0.18 0.85 0.872 0.872	
Type of roof	Gable	
Topographic Factor(TopographyHill Height(H)Half Hill Length (Lh)Actual H/Lh=Use H/Lh=Modified Lh=From top of crest: x =Bldg up/down wind?	K <u>zt)</u> Flat 80.0 ft 100.0 ft 0.80 0.50 160.0 ft 50.0 ft downwind	Z V(Z) x(upwind) H/2 H/2 H ESCARPMENT
H/Lh= 0.50	$K_1 = 0.000$	
x/Lh = 0.31 z/Lh = 0.11 At Mean Roof Ht: Kzt =	$K_2 = 0.792$ $K_3 = 1.000$ $(1+K_1K_2K_3)^2 = 1.00$	V(z) V(z) x(upwind) V(z) x(upwind) H/2 H/2 H 2D RIDGE or 3D AXISYMMETRICAL HILL
GustEffectFactorh =17.0B =38.0/z (0.6h) =15.0	ft ft	Flexible structure if natural frequency < 1 Hz (T > 1 second). If building h/B>4 then may be flexible and should be investigated. h/B = 0.45 Rigid structure (low rise bldg)

G = 0.85 Using rigid structure formula

Rigio	Structure	Flexible or Dyn	amically Ser	nsitive St	ructure			
ē =	0.20	Natural Frequency $(\eta_1) =$	0.0 Hz					
: = in =	500 ft 15 ft	Damping ratio (β) = /b =	0 0.65					
c = Iv =	0.20 3.4	/α = Vz =	0.15 101.3					
-z =	427.1 ft	N ₁ =	0.00					
Q =	0.92	$R_n =$	0.000					
_z =	0.23	R _h =	28.282	η =	0.000	h =	17.0 ft	
G =	0.88 use G = 0.85	R _B =	28.282	η =	0.000			
		$R_L =$	28.282	η =	0.000			
		g _R =	0.000					
		R =	0.000					
		Gf =	0.000					

JOB TITLE Wooden Spur Renovation 368 Wppdem Spur Dr. Alpine Wy. JOB NO. 225112 SHEET NO. Jok 39 CALCULATED BY EVB 6/4/25 DATE CHECKED BY DATE

Enclosure Classification

Test for Enclosed Building:

Ao < 0.01Ag or 4 sf, whichever is smaller

Test for Open Building:

All walls are at least 80% open. $Ao \ge 0.8Ag$

Test for Partially Enclosed Building: Predominately open on one side only

Test Input Ao 180.0 sf Ao ≥ 1.1Aoi NO Ao > 4' or 0.01Ag Ag 286.0 sf YES 308.0 sf Aoi / Agi ≤ 0.20 NO Building is NOT Aoi 858.0 sf Partially Enclosed Agi

Conditions to qualify as Partially Enclosed Building. Must satisfy all of the following:

Ao ≥ 1.1Aoi

Ao > smaller of 4' or 0.01 Ag

Aoi / Agi ≤ 0.20

Where:

Ao = the total area of openings in a wall that receives positive external pressure.

Ag = the gross area of that wall in which Ao is identified.

Aci = the sum of the areas of openings in the building envelope (walls and roof) not including Ac.

Agi = the sum of the gross surface areas of the building envelope (walls and roof) not including Ag.

Test for Partially Open Building:

A building that does not qualify as open, enclosed or partially enclosed. (This type building will have same wind pressures as an enclosed building.

Reduction Factor for large volume partially enclosed buildings (Ri) :

If the partially enclosed building contains a single room that is unpartitioned, the internal pressure coefficient may be multiplied by the reduction factor Ri.

Total area of all wall & roof openings (Aog):		0 sf
Unpartitioned internal volume (Vi) :		0 cf
	Ri =	1.00

Ground Elevation Factor (Ke)

Grd level above sea level = Constant =

5664.0 ft 0.00256

Adj Constant = 0.00209

Ke = 0.8146

TSE Engineers	JOB TITLE Wooden Spur Renovation		Renovation
136 S. State Street	368 Wppdem Spur Dr. Alpine Wy.		pur Dr. Alpine Wy.
Shelley, Idaho 83274	JOB NO.	225112	SHEET NO. 6 05 39
208-357-2420	CALCULATED BY	EVB	DATE 6/4/25
	CHECKED BY		DATE

Wind Loads - MWFRS h≤60' (Low-rise Buildings) except for open buildings

Kz = Kh (case 1) =	0.87	Edge St
Base pressure (qh) =	13.3 psf	End Zon
GCpi =	+/-0.18	Zone 2 I

Strip (a) = one (2a) = 3.0 ft 6.0 ft length = 13.0 ft

Wind Pressure Coefficients

	C	ASE A				CASE B	
		$\theta = 9.5 \deg$					
Surface	GCpf	w/-GCpi	w/+GCpi		GCpf	w/-GCpi	w/+GCpi
1	0.44	0.62	0.26		-0.45	-0.27	-0.63
2 3	-0.69	-0.51	-0.87		-0.69	-0.51	0.87
3	-0.40	-0.22	-0.58		-0.37	-0.19	-0.55
4	-0.33	-0.15	-0.51		-0.45	-0.27	-0.63
5					0.40	0.58	0.22
6					-0.29	-0.11	-0.47
1E	0.67	0.85	0.49		-0.48	-0.30	-0.66
2E	-1.07	-0.89	-1.25		-1.07	-0.89	-1.25
3E	-0.58	-0.40	-0.76		-0.53	-0.35	-0.71
4E	-0.49	-0.31	-0.67		-0.48	-0.30	-0.66
5E			알카일라	그는 것 같아요. 그는 것 같아요.	0.61	0.79	0.43
6E			elan de		-0.43	-0.25	-0.61

	Nominal	Wind	Surface	Pressures	(psf
--	---------	------	---------	-----------	------

1	8.3 3.5	-3.6	-8.4
2	-6.8 -11.6	-6.8	-11.6
3	-3.0 -7.8	-2.5 -3.6	-7.3
4	-2.0 -6.8	-3.6	-8.4
5	이 그 같은 것은 것 같은 것 같은 것 같은 것 같은 것 같은 것 같이 없다.	7.7	2.9
6		-1.5	-11.6 -7.3 -8.4 2.9 -6.3
1E	11.3 6.5	-4.0	-8.8 -16.7 -9.5
2E	-11.9 -16.7	-11.9	-16.7
3E	-5.3 -10.1	-4.7	-9.5
4E	-4.2 -9.0	-4.0	-8.8 5.7
2E 3E 4E 5E 6E		10.5	
6E		-3.3	-8.1

Parapet

0.0 psf (GCpn = +1.5) Windward parapet = Leeward parapet =

0.0 psf (GCpn = -1.0)

Horizontal MWFRS Simple Diaphragm Pressures (psf)

al WWW NO OIL	ipic Diap	mayminessures
Transverse d	irection	(normal to L)
Interior Zone:	Wall	10.3 psf
	Roof	-3.8 psf **
End Zone:	Wall	15.5 psf
	Roof	-6.6 psf **

Longitudinal direction (parallel to L)

Interior Zone:	Wall	9.2 psf	
End Zone:	Wall	13.9 psf	

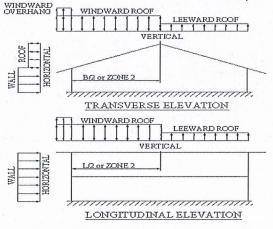
** NOTE: Total horiz force shall not be less than that determined by neglecting roof forces (except for MWFRS moment frames).

The code requires the MWFRS be designed for a min ultimate force of 16 psf multiplied by the wall area plus an 8 psf force applied to the vertical projection of the roof.

Windward roof

overhangs =

9.3 psf (upward) add to windward roof pressure



JOB TITLE	Wooden Spur F	Renovation	
	368 Wppdem S	pur Dr. Alpine Wy.	
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ASCE 7-16 procedure

Wind Loads - h≤60' Longitudinal Direction MWFRS On Open or Partially

Enclosed Buildings with Transverse Frames and Pitched Roofs

Base pressure (qh) = GCpi = Roof Angle (θ) =

13.3 psf +/-0.18 Enclosed bldg, procdure doesn't apply 9.5 deg

n=5 SHOWN Ae As SHOWN

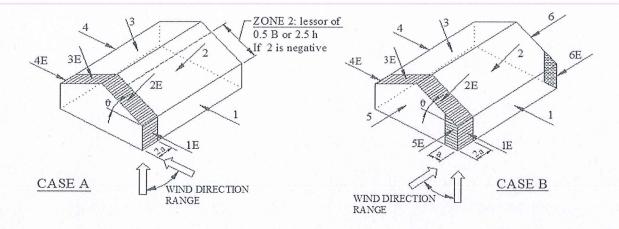
B=	38.0 ft
# of frames (n) =	5
Solid are of end wall including fascia (As) =	1,500.0 sf
Roof ridge height =	20.2 ft
Roof eave height =	17.0 ft
Total and wall area if saild (As) -	700.0

Total end wall area if soild (Ae) = 706.2 sf

	ngidinal Directional Force (F	-	
p= qh [(GC	pf)windward -(GCpf)leewar	d] $K_B K_S$	
	Solidarity ratio (Φ) =	2.124	
	n =	5	
	KB =	1.42	
	KS =	5.597	
	Zones 5 & 6 area =	653	sf
	5E & 6E area =	53	sf
(GCpf) wind	ward - (GCpf) leeward] =	0.716	
	p =	76.0	psf
1			
Total force to be r	esisted by MWFRS (F) =	53.7	kips applied at the centroid of the end wall area Ae

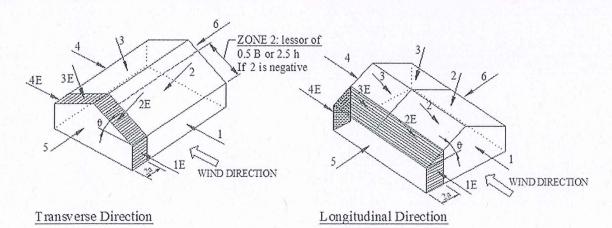
Note: The longidudinal force acts in combination with roof loads calculated elsewhere for an open or partially enclosed building.

JOB TITLE	Wooden Spur R	lenovation	
	368 Wppdem S	pur Dr. Alpine Wy.	
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	JOB NO. CALCULATED BY	368 Wppdem S JOB NO. 225112 CALCULATED BY EVB	CALCULATED BY EVB DATE



NOTE: Torsional loads are 25% of zones 1 - 6. See code for loading diagram. Exception: One story buildings h<30' and 1 to 2 storybuildings framed with light-frame construction or with flexible diaphragms need not be designed for the torsional load case.

ASCE 7-98 & ASCE 7-10 (& later) - MWFRS wind pressure zones



NOTE: Torsional loads are 25% of zones 1 - 4. See code for loading diagram. Exception: One story buildings h<30' and 1 to 2 storybuildings framed with light-frame construction or with flexible diaphragms need not be designed for the torsional load case.

ASCE 7-02 and ASCE 7-05 - MWFRS wind pressure zones

TSE Engineers

136 S. State Street Shelley, Idaho 83274

208-357-2420

JOB TITLE Wooden Spur Renovation

368 Wppdem Spur Dr. Alpine Wy. oh 39 JOB NO. 225112 SHEET NO. 7 CALCULATED BY EVB 6/4/25 DATE CHECKED BY

DATE

Nominal Wind Pressures

Wind Loads - Components & Cladding : h ≤ 60'

Kh (case 2) =	0.87	h =	17.0 ft
Base pressure (qh) =	13.3 psf	a =	3.0 ft
Minimum parapet ht =	0.0 ft	GCpi =	+/-0.18
Roof Angle (θ) =	9.5 deg	qi = qh =	13.3 psf
Type of roof = 0	Bable		

Roof					Surface Pr	ressure (psf))	
Area	2 sf	10 sf	20 sf	50 sf	75 sf	100 sf	200 sf	250 sf
Negative Zone 1 & 2e	-29.1	-29.1	-29.1	-17.7	-12.7	-10.0	-10.0	-10.0
Negative Zone 2n, 2r &3e	-42.4	-42.4	-36.7	-29.1	-25.7	-23.3	-17.6	-15.8
Negative Zone 3r	-50.5	-50.5	-43.2	-33.7	-29.4	-26.4	-26.4	-26.4
Positive All Zones	11.7	10	10	10	10.0	10.0	10.0	10.0
Overhang Zone 1 & 2e	-33.4	-33.4	-33.4	-25.8	-22.4	-20.0	-20.0	-20.0
Overhang Zone 2n & 2r	-46.7	-46.7	-42.4	-36.7	-34.2	-32.4	-28.1	-26.7
Overhang Zone 3e	-54.7	-54.7	-47.3	-37.4	-33.0	-29.9	-22.4	-20.0
Overhang Zone 3r	-62.7	-62.7	-53.1	-40.3	-34.7	-30.7	-30.7	-30.7

User	input
25 sf	50 sf
-26.3	-17.7
-34.8	-29.1
-40.9	-33.7
10.0	10.0
-31.5	-25.8
-41.0	-36.7
-44.8	-37.4
-50.0	-40.3

Overhang pressures in the table above assume an internal pressure coefficient (Gcpi) of 0.0 Overhang soffit pressure equals adj wall pressure (which includes internal pressure of 2.4 psf)

Parapet

Tapoe							
qp =	0.0 psf	Surface Pressure (psf)					
	Solid Parapet Pressure	10 sf	20 sf	50 sf	100 sf	250 sf	500 sf
	CASE A: Zone 2e :	0.0	0.0	0.0	0.0	0.0	0.0
	Zone 2n, 2r & 3e :	0.0	0.0	0.0	0.0	0.0	0.0
	Zone 3r :	0.0	0.0	0.0	0.0	0.0	0.0
	CASE B : Interior zone :	0.0	0.0	0.0	0.0	0.0	0.0
	Corner zone :	0.0	0.0	0.0	0.0	0.0	0.0

Use	r input
5	0 sf
1975	0.0
	0.0
	0.0
	0.0
	0.0

Walls	GCp +/- GCpi				Surface Pressure at h			
Area	10 sf	100 sf	200 sf	500 sf	10 sf	100 sf	200 sf	500 sf
Negative Zone 4	-1.17	-1.01	-0.96	-0.90	-15.6	-13.5	-12.9	-12.0
Negative Zone 5	-1.44	-1.12	-1.03	-0.90	-19.2	-15.0	-13.7	-12.0
Positive Zone 4 & 5	1.08	0.92	0.87	0.81	14.4	12.3	11.7	10.8

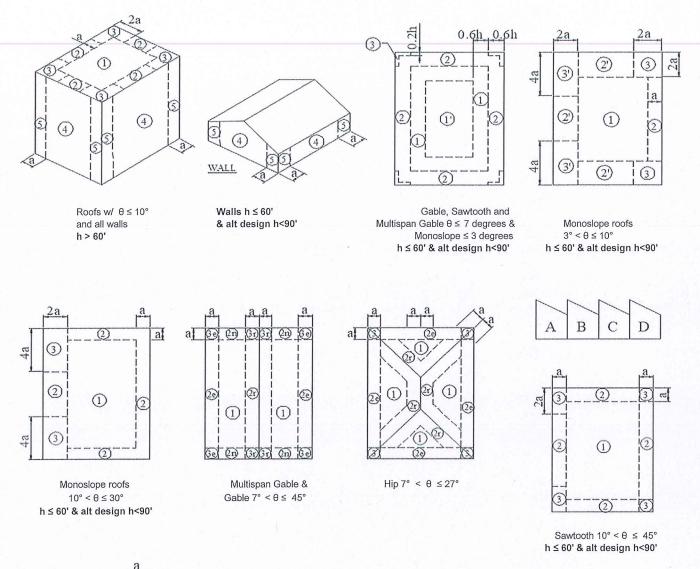
Γ	User i	nput
Γ	21 sf	50 sf
Г	-14.9	-14.1
1	-17.8	-16.3
	13.7	12.9

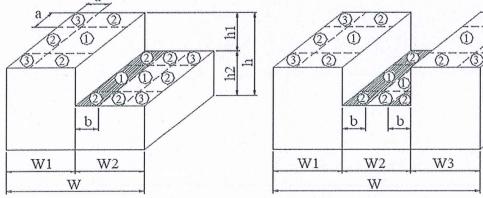
TSE Engineers 136 S. State Street Shelley, Idaho 83274

208-357-2420

JOB IIILE	Wooden Spur F	enovation	
	368 Wppdem S	pur Dr. Alpine Wy.	
JOB NO.	225112	SHEET NO.	1006 39
CALCULATED BY	EVB	DATE	6/4/25
CHECKED BY		DATE	

Location of C&C Wind Pressure Zones - ASCE 7-16





Stepped roofs $\theta \le 3^{\circ}$ h $\le 60'$ & alt design h<90'

JOB TITLE Wooden Spur Renovation

 368 Wppdem Spur Dr. Alpine Wy.

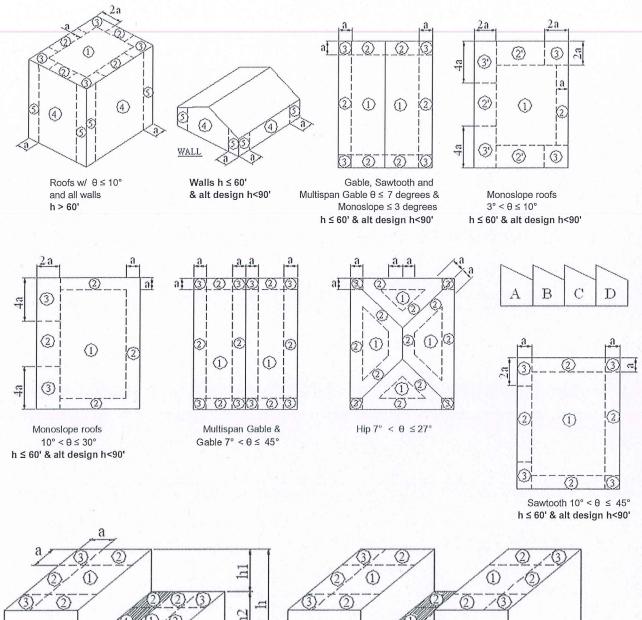
 JOB NO.
 225112

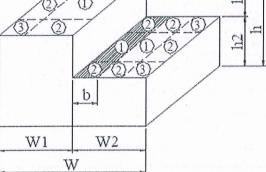
 SHEET NO.
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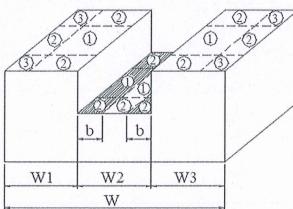
 CALCULATED BY
 EVB

 CHECKED BY
 DATE

Location of C&C Wind Pressure Zones - ASCE 7-10 & earlier







Stepped roofs $\theta \le 3^{\circ}$ h $\le 60'$ & alt design h<90'

TOP	- per		
ISE	- ⊢n	aine	ers
IOL		gin	1010
		•	

136 S. State Street

Shelley, Idaho 83274 208-357-2420

JOB TITLE Wooden Spur Renovation 368 Wppdem Spur Dr. Alpine Wy. SHEET NO. 1206 39 DATE 6/4/25 JOB NO. 225112 CALCULATED BY EVB

CHECKED BY

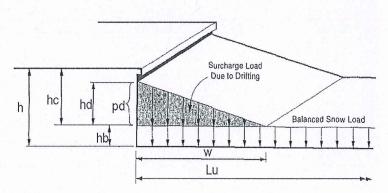
Snow Loads : ASCE 7-16

OHOW	Louds, MOOL /- I	0				Norminal Onow Forces
	Roof slo	ope	= 9.5 deg			
	Horiz. eave to ridge					
	Roof length parallel to r	. ,				
	Type of Roof		Hip and gable	e w/ trussed systems		
	Ground Snow Load	Pg				
	Risk Category	•	=			
	Importance Factor	1	= 1.0			
	Thermal Factor	Ct				
	Exposure Factor	Ce	= 1.0			
	Pf = 0.7*Ce*Ct*I*Pg		= 98.0 psf			
	Unobstructed Slippery Surfa	ace	no			
	Sloped-roof Factor	Cs	= 1.00			
	Balanced Snow Load		= 98.0 psf		Near ground level s	urface balanced snow load = 140.0 psf
	Rain on Snow Surcharge Ar		0.38 deg			
	Code Maximum Rain Surcha	arge	5.0 psf			
	Rain on Snow Surcharge		= 0.0 psf			
	Ps plus rain surcharge		= 98.0 psf			
	Minimum Snow Load	Pm	= 20.0 psf			NOTE: Alternate spans of continuous beams
						shall be loaded with half the design roof snow
	Uniform Roof Design Snov	v Load	= 98.0 psf	use 100.0		oad so as to produce the greatest possible effect - see code for loading diagrams and
						exceptions for gable roofs
Unbalar	nced Snow Loads - for	Hin &	Gable roofs	only		exceptions for gable roots
In the second se	Required if slope is between			<u>unit</u>		
		.38 deg		Unbalanced snow	loads must be app	lied
	Windward sn	•	•			
Lee	ward snow load from ridge to			= hdγ / √S + Ps		
	d snow load from 16.88' to th					
Windwar	rd Snow Drifts 1 - Agains	t walls,	parapets, etc			
	Up or downwind fetch	lu	= 40.0 ft			

Leeward snow load from ridge to 16.88' =	129.7 psf = hdγ / √S + Ps	
Leeward snow load from 16.88' to the eave =	98.0 psf = Ps	

Windw

		the second second
Up or downwind fetch	lu =	40.0 ft
Projection height	h =	6.0 ft
Projection width/length	lp =	18.0 ft
Snow density	g =	30.0 pcf
Balanced snow height	hb =	3.27 ft
	hd =	2.73 ft
	hc =	2.73 ft
hc/hb > 0.2 = 0.8	Therefore, c	lesign for drif
Drift height (hc)		2.73 ft
Drift width	w =	10.95 ft
Surcharge load:	$pd = \gamma^{*}hd =$	82.0 psf
Balanced Snow load:	=_	98.0 psf
		180.0 psf
indward Snow Drifts 2 - Aga	ainst walls, pa	rapets, etc
Up or downwind fetch	lu =	12.0 ft
Projection height	h =	6.0 ft
Projection width/length	lp =	18.0 ft
Snow density	g =	30.0 pcf
Balanced snow height	hb =	3.27 ft
	hd =	1.94 ft
	hc =	2.73 ft
hc/hb >0.2 = 0.8	Therefore, d	esign for drift
Drift height (hd)	=	1.94 ft
Drift width	w =	7.75 ft
Surcharge load:	$pd = \gamma^*hd =$	58.2 psf
Balanced Snow load:	=	98.0 psf
		156.2 psf



Note: If bottom of projection is at least 2 feet above hb then snow drift is not required.

Nominal Snow Forces

DATE

TSE Engineers JOB TITLE Wooden Spur Renovation 136 S. State Street 368 Wppdem Spur Dr. Alpine Wy. 39 Shelley, Idaho 83274 JOB NO. 225112 SHEET NO. 208-357-2420 CALCULATED BY EVB DATE 6/4/25 CHECKED BY DATE Seismic Loads: IBC 2021 Strength Level Forces **Risk Category:** Ш Importance Factor (le) : 1.00 Site Class : D - code default Ss (0.2 sec) = 110.90 %g 34.10 %g S1 (1.0 sec) = A site specific ground motion analysis is required for seismically isolated structures or with damping systems, see ASCE7 11.4.8 S_{DS} = 1.331 0.887 Design Category = D Fa = 1.200 Sms = D Fv = 1.959 Sm1 = 0.668 $S_{D1} =$ 0.445 Design Category = Seismic Design Category = D Redundancy Coefficient p = 1.30 Number of Stories: 1 Structure Type: All other building systems Horizontal Struct Irregularities: No plan Irregularity Vertical Structural Irregularities: No vertical Irregularity Flexible Diaphragms: Yes Building System: Bearing Wall Systems Seismic resisting system: Light frame (wood) walls with structural wood shear panels System Structural Height Limit: 65 ft Actual Structural Height (hn) = 17.0 ft See ASCE7 Section 12.2.5 for exceptions and other system limitations DESIGN COEFFICIENTS AND FACTORS Response Modification Coefficient (R) = 6.5 Over-Strength Factor (Ωo) = 2.5 Deflection Amplification Factor (Cd) = 4 S_{DS} = 0.887 $S_{D1} =$ 0.445 Q_E = horizontal seismic force Seismic Load Effect (E) = Eh +/-Ev = ρQ_E +/- 0.2S_{DS} D = 1.3Qe +/ 0.177D Special Seismic Load Effect (Em) = Emh +/- Ev = $\Omega o Q_E$ +/- $0.2S_{DS}D$ = 2.5Qe +/ 0.177D D = dead load PERMITTED ANALYTICAL PROCEDURES **Simplified Analysis** - Use Equivalent Lateral Force Analysis Equivalent Lateral-Force Analysis - Permitted Building period coef. $(C_T) =$ 0.020 Cu = 1.40 $C_T h_n^{+} =$ Tmax = CuTa = 0.234 sec Approx fundamental period (Ta) = 0.167 sec x= 0.75 User calculated fundamental period = 0.500 s T = 0.234 sec Long Period Transition Period (TL) = ASCE7 map = 8 sec 0.136 Seismic response coef. (Cs) = Sdsl/R = ASCE7 11.4.8 exception 2 equations used but not less than Cs = 0.044Sdsl = 0.039 USE Cs = 0.136 Design Base Shear V = 0.136W

Model & Seismic Response Analysis

- Permitted (see code for procedure)

ALLOWABLE STORY DRIFT

Structure Type: All other structures

Allowable story drift $\Delta a = 0.020$ hsx where hsx is the story height below level x

Total	Stories =	1	Floor Dead Load =	40.0 psf	Roof Dead Load =	20.0 psf	
Building le	ength L =	26.0 ft	Floor LL to include =	0.0 psf	Roof Snow Load =	20.0 psf	
Building w	vidth W =	38.0 ft	Floor Equip wt =	0.0 kips	Roof Equip wt =	0.0 kips	
	hn =	17.0 ft	Partition weight =	10.0 psf	Parapet weight =	0.0 psf	
	k =	1.000	Ext Wall Weight =	0.0 psf	Parapet height =	0.0 ft	
	V =	0.136W					
Bottom Floor is a s	slab on grad	е			Diaphragm shall be d but not less than F	0	vpx, but :

Fpx min = $0.2S_{DS}$ le wpx = 0.177 wpx Fpx max = $0.4S_{DS}$ le wpx = 0.355 wpx

	Seismic F	orces Norm	nal to Buildir	ng Length					px max = 0.23	00 1	Sector Contraction Sector Sector
S	EL above eismic Base	Level Weight	Wx hx⁵	$Cvx = Wx hx^{\kappa}$	V = t Base S	5.4k hear Distri	bution		Diaph	ragm Force	Fpx
Level (x)	hx (ft)	Wx (kips)	(ft-kips)	Σ Wi hi ^κ	Fx=CvxV	Σ Fx (k)	Story M		Σ Wi (k)	Fpx	Design Fpx
Roof	15.00	40	593	1.000	5.39	5.4	0		40	5.4	7.0
1	2.00	0	0	0.000	0.00	0.0	0		0	0.0	0.0
Base		40		1.000	ing the set	5.4	81		/		
							0.4	D 11			

81 = Base M

	Seismic F	orces Paral	lel to Buildin	g Length	V =	5.4k					
					Base S	Shear Distri	bution		Diaph	ragm Force	Fpx
Level (x)	hx (ft)	Wx (kips)	Wx hx ^k	Cvx =	Fx=CvxV	Σ Fx (k)	Story M		Σ Wi (k)	Fpx	Design Fpx
Roof	15.00	40	593	1.000	5.39	5.4	0		40	5.4	7.0
1	2.00	0	0	0.000	0.00	0.0	0		0	0.0	0.0
Base	0.00	40		1.000		5.4	81				
							81	= Base M			

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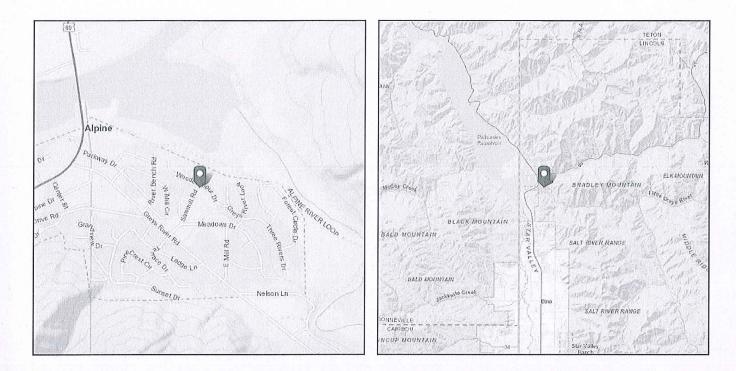


Address: 368 Wooden Spur Dr Alpine, Wyoming 83128

ASCE Hazards Report

Standard:ASCE/SEI 7-16Risk Category:IISoil Class:D - Default (see
Section 11.4.3)

Latitude: 43.16238 Longitude: -111.008836 Elevation: 5666.142041756172 ft (NAVD 88)





Site Soil Class: Results:	D - Default (s	ee Section 11.4.3)		
S _s :	1.109	S _{D1} :	N/A	
S ₁ :	0.341	T _L :	6	
F _a :	1.2	PGA :	0.477	
F _v :	N/A	PGA _M :	0.572	
S _{MS} :	1.331	F _{PGA} :	1.2	
S _{M1} :	N/A	l _e :	1	
S _{DS} :	0.888	C _v :	1.322	
Ground motion hazard ar	nalysis may be required	. See ASCE/SEI 7-16 Se	ection 11.4.8.	
Data Accessed:	Wed Jun 04 2	2025		
Date Source:	USGS Seism	ic Design Maps		

JOB TITLE Wooden Spur Renovation 368 Wppdem Spur Dr. Alpine Wy. JOB NO. 225112 CALCULATED BY EVB DATE DATE DATE

Roof Design Loads

ltems	Description	Multiple	psf (max)	psf (min)
Roofing	Asphalt Shingles w/roll roofing		3.0	2.0
Decking	5/8" plywood/OSB		2.2	1.8
Framing	Wood Trusses @ 24"		3.0	2.5
Insulation	R-40 Fiberglass insul.	x 1.4	1.8	1.7
Ceiling	5/8" gypsum		2.8	2.5
	말 말 수많은 것 같아요.		0.0	0.0
			0.0	0.0
a Sherry Cak	전 물질 것 같아요. 같아요. 것 같다.		0.0	0.0
	Actual I	Dead Load	D 12.8	O 10.5
	Use this I	DL instead	15.0	9.0
		Live Load	20.0	0.0
		Snow Load	100.0	0.0
	Ultimate Wind (zo	ne 2 - 100sf)	16.0	-38.9
ASD Loading		D + S	115.0	-
	D + 0.75(0.6*W + S)	97.2	-
a state of the second second second	0.6	6*D + 0.6*W	-	-17.9
LRFD Loading	1.2D + 1.	6S + 0.5W	186.0	-
	1.2D + 1	.0W + 0.5S	84.0	-
	C).9D + 1.0W	-	-30.8

Roof Live Load Reduction

Roof angle 2.00 / 12 9.5 deg

0 to 200 sf: 20.0 psf 200 to 600 sf: 24 - 0.02Area, but not less than 12 psf over 600 sf: 12.0 psf

	300 sf	18.0 psf
	400 sf	16.0 psf
	500 sf	14.0 psf
User Input:	450 sf	15.0 psf

JOB TITLE Wooden Spur Renovation 368 Wppdem Spur Dr. Alpine Wy. JOB NO. 225112 SHEET NO. 18

 JOB NO. 225112
 SHEET NO. 18
 39

 CALCULATED BY EVB
 DATE
 6/4/25

 CHECKED BY
 DATE
 0

IBC alternate procedure

Items Description Multiple psf (max) psf (min) Flooring Carpet & pad 1.0 1.0 None 0.0 0.0 Decking 3/4" plywood/OSB 2.7 2.3 Framing TJI @ 24" x 1.4 2.8 1.4 None 0.0 0.0 5/8" gypsum 2.8 2.5 Ceiling 0.0 0.0 None None 0.0 0.0 None 0.0 0.0 ۲ 7.2 Actual Dead Load C 9.3 0 Use this DL instead 10.0 8.0 Partitions 15.0 0.0 40.0 Live Load 0.0 Total Live Load 55.0 0.0 Total Load 65.0 7.2

Floor Design Loads

FLOOR LIVE LOAD REDUCTION (not including partitions)

NOTE: Not allowed for assembly occupancy or LL>100psf or passenger car garages, except may reduce members supporting 2 or more floors & non-assembly 20%.

		Smallest of:
L=Lo(0	.25+15/√K _{LL} A _T)	R= .08%(SF - 150)
Unreduced design live load: Lo =	40 psf	R= 23.1(1+D/L) = 28.9% R= 40% member supports 1 floor
Floor member & 1 floor cols K_{LL} =	2	R= 60% member supports ≥2 floors
Tributary Area A _T =	300 sf	R = 12.0%
Reduced live load: L =	34.5 psf	Reduced live load: L = 35.2 psf
Columns (2 or more floors) $K_{LL} =$	4	
Tributary Area A _T =	500 sf	R = 28.0%
Reduced live load: L =	23.4 psf	Reduced live load: L = 28.8 psf

		TSE Engin 136 S State S Shelley, ID 8 208-357-2	Street 33274			JOB TITLE <u>368 Wooden Spur D</u> JOB NO. CALCULATED BY CHECKED BY	225112	ning	_SHEET NO. _DATE _DATE	6/4/2025
			Wood He		ood Beam (No NDS 2018	t Laterally Supported)		4 16 17 <u>-</u>	4	
Mark:		Load F	actors				Loading	2 W		
Span(ft)=	18	Dead	1.00		DL (psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)
LL Deflection <l <="" th=""><th></th><th>Floor Live</th><th>1.00</th><th>root</th><th>and the second s</th><th>100</th><th>16</th><th>240</th><th>1600</th><th>1840</th></l>		Floor Live	1.00	root	and the second s	100	16	240	1600	1840
Total Deflection <l <="" td=""><td></td><td>Roof Live/Snow</td><td>1.00</td><td>floor</td><td>Manufacture based on the second</td><td></td><td></td><td>0</td><td>0</td><td>0</td></l>		Roof Live/Snow	1.00	floor	Manufacture based on the second			0	0	0
CD=		Misc.	1.00	wal misc.	Sector and the sector of the s			0	0 0	0 0
				THISE.	· Same and the second s		Total	240.0		1600.0 1
					oad Factors Ap		Reactions		alen di	
	Load	DL(lbs)	LL(lbs)	x(ft)(left)	DL	LL			Left (lbs)	Right (lbs)
	roof		contr.		0	0	Uniform	RDL=		2160
oint Load 2	roof				0	0		RLL=		14400
							Point Loads	P1DL=	• 0	0
	DIM	TIMBER	LVL	GLB				P1LL=	: 0	0
								P2DL=	• 0	0
			and the second states we		4		111111	P2LL=		0
Wood Species=	Douglas Fir #2	Douglas Fir #1	LVL	24F-V4			7.7	RTL=	16560	16560
Option 1	No Single Ply	4x None	No Single Ply	3.125 x None				Max Shear=	: 16.6	k
Option 2	No Double Ply	6x None	No Double Ply	5.125x22.5						
Option 3	No Triple Ply	8x None	No Triple Ply	6.75x19.5	1		Moment	1.1		
Option 4	No Four Ply	10x None	(4) 1.75x18	8.75x18			, and a first of	Center	Point Load 1	Point Load
Option 5		12x None	-	10.75×16.5			RDL=	9720	0	0
Option 6	-	14x None	-	-			RLL=	64800	0	0
Option 7	<u>.</u>	16x18		-			P1DL=	0	0	0
Option /	States and the second second	10410			1				0	0
							P1LL= P2DI=	0		
							P1LL= P2DL= 	0 0 74520 Max M=	0 0 0 74.5	0 0 0 k-ft
	UDD2						P2DL= P2LL= MTL=	0 0 74520	0 0 0	0 0 0
Mark:		Load F					P2DL= P2LL= MTL= Loading	0 0 74520 Max M=	0 0 74.5	0 0 k-ft
Span(ft)=[3	Dead	1.00		DL (psf)	LL(psf)	P2DL= P2LL= MTL= Loading trib (ft)	0 0 74520 Max M=	0 0 74.5	0 0 k-ft TLw(plf)
Span(ft)=[_LL Deflection <l <="" td=""><td>3 360</td><td>Dead Floor Live</td><td>1.00 1.00</td><td>roof</td><td>Concert Industry on Decision of Lines and</td><td>LL(psf) 100</td><td>P2DL= P2LL= MTL= Loading</td><td>0 0 74520 Max M= DLw(plf) 60</td><td>0 0 74.5 <u>LLw(plf)</u> 400</td><td>0 0 k-ft TLw(pif) 460</td></l>	3 360	Dead Floor Live	1.00 1.00	roof	Concert Industry on Decision of Lines and	LL(psf) 100	P2DL= P2LL= MTL= Loading	0 0 74520 Max M= DLw(plf) 60	0 0 74.5 <u>LLw(plf)</u> 400	0 0 k-ft TLw(pif) 460
Span(ft)=[LL Deflection <l <br=""></l>	3 360 240	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	floor	Concert Industry on Decision of Lines and		P2DL= P2LL= MTL= Loading trib (ft)	0 0 74520 Max M= DLw(plf) 60 0	0 0 74.5 <u>LLw(plf)</u> 400 0	0 0 k-ft <u>TLw(plf)</u> 460 0
Span(ft)=[_LL Deflection <l <="" td=""><td>3 360 240</td><td>Dead Floor Live</td><td>1.00 1.00</td><td>floor wall</td><td>Concert Industry on Decision of Lines and</td><td></td><td>P2DL= P2LL= MTL= Loading trib (ft)</td><td>0 0 74520 Max M=</td><td>0 0 74.5 LLw(plf) 400 0 0</td><td>0 0 k-ft <u>TLw(plf)</u> 460 0 0</td></l>	3 360 240	Dead Floor Live	1.00 1.00	floor wall	Concert Industry on Decision of Lines and		P2DL= P2LL= MTL= Loading trib (ft)	0 0 74520 Max M=	0 0 74.5 LLw(plf) 400 0 0	0 0 k-ft <u>TLw(plf)</u> 460 0 0
Span(ft)= 	3 360 240	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	floor	Concert Industry on Decision of Lines and		P2DL= P2LL= MTL= Loading trib (ft)	0 0 74520 Max M=	0 0 74.5 <u>LLw(plf)</u> 400 0 0 0	0 0 k-ft <u>TLw(plf)</u> 460 0 0 0
Span(ft)=[LL Deflection <l <br=""></l>	3 360 240	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	floor wall misc.	15	100	P2DL= P2LL= MTL= Loading trib (ft) 4 Total	0 0 74520 Max M=	0 0 74.5 <u>LLw(plf)</u> 400 0 0 0	0 0 k-ft <u>TLw(plf)</u> 460 0 0 0
Span(ft)=[LL Deflection <l <br="">fotal Deflection<l <br="">CD=]</l></l>	3 360 240	Dead Floor Live Roof Live/Snow Misc.	1.00 1.00 1.00 1.00	floor wall misc.	Concert Industry on Decision of Lines and	100	P2DL= P2LL= MTL= Loading trib (ft) 4	0 0 74520 Max M=	0 0 74.5 <u>LLw(plf)</u> 400 0 0 0	0 0 k-ft <u>TLw(plf)</u> 460 0 0 0
Span(ft)=[LL Deflection <l <br="">otal Deflection<l <br="">CD=</l></l>	3 360 240 1	Dead Floor Live Roof Live/Snow Misc.	1.00 1.00 1.00 1.00	floor wall misc. Lo	15 Dad Factors Ap	100 Iplied	P2DL= P2LL= MTL= Loading trib (ft) 4 Total	0 0 74520 Max M=	0 0 74.5	0 0 k-ft TLw(plf) 460 0 0 0 434.8
Span(ft)=[LL Deflection <l <br="">otal Deflection<l <br="">CD= pint Load 1</l></l>	3 360 240 1	Dead Floor Live Roof Live/Snow Misc.	1.00 1.00 1.00 1.00	floor wall misc. Lo	15 Dad Factors Ap DL	100 pplied LL	P2DL= P2LL= MTL= Loading trib (ft) 4 Total Reactions	0 0 74520 Max M=	0 0 74.5 LLw(pif) 400 0 0 0 0 0 2 2 2 2 2 2 2 2 2 2 2 2 2	0 0 k-ft TLw(plf) 460 0 0 434.8 Right (lbs)
Span(ft)=[LL Deflection <l <br="">otal Deflection<l <br="">CD= pint Load 1</l></l>	3 360 240 1 Load roof roof	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00	floor wall misc. Lc <(ft)(left)	15 Dad Factors Ap DL	100 pplied LL	P2DL= P2LL= MTL= Loading trib (ft) 4 Total Reactions	0 0 74520 Max M=	0 0 74.5 LLw(plf) 400 0 0 0 0 0 8 0 98 652	0 0 k-ft <u>TLw(plf)</u> 460 0 0 0 434.8 <u>Right (lbs)</u> 98
Span(ft)=[LL Deflection <l <br="">otal Deflection<l <br="">CD= pint Load 1</l></l>	3 360 240 1 Load roof	Dead Floor Live Roof Live/Snow Misc.	1.00 1.00 1.00 1.00	floor wall misc. Lo	15 Dad Factors Ap DL	100 pplied LL	P2DL= P2LL= MTL= Loading trib (ft) 4 Total Reactions	0 0 74520 Max M=	0 0 74.5 LLw(plf) 400 0 0 0 0 0 0 0 98 652 0	0 0 k-ft 7Lw(plf) 460 0 0 434.8 8 8434.8 98 652
Span(ft)=[LL Deflection <l <br="">otal Deflection<l <br="">CD= pint Load 1</l></l>	3 360 240 1 Load roof roof	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00	floor wall misc. La <(ft)(left)	15 Dad Factors Ap DL	100 pplied LL	P2DL= P2LL= MTL= Loading trib (ft) 4 Total Reactions	0 0 74520 Max M=	0 0 74.5 LLw(plf) 400 0 0 0 0 0 0 0 98 652 0 0 0	0 0 k-ft 1Lw(plf) 460 0 0 434.8 Right (lbs) 98 652 0
Span(ft)=[LL Deflection <l <br="">otal Deflection<l <br="">CD= pint Load 1</l></l>	3 360 240 1 Load roof roof	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00	floor wall misc. La <(ft)(left)	15 Dad Factors Ap DL	100 pplied LL	P2DL= P2LL= MTL= Loading trib (ft) 4 Total Reactions	0 0 74520 Max M= DLw(plf) 60 0 0 65,2 RDL= RLL= P1DL= P1LL= P2DL=	0 0 74.5	0 0 k-ft 460 0 0 434.8 Right (lbs) 98 652 0 0 0 0 0
Span(ft)=[LL Deflection <l <br="">otal Deflection<l <br="">CD= pint Load 1</l></l>	3 360 240 1 Load roof roof DIM	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00	floor wall misc. (ft)(left) GLB	15 Dad Factors Ap DL	100 pplied LL	P2DL= P2LL= MTL= Loading trib (ft) 4 Total Reactions	0 0 74520 Max M=	0 0 74.5	0 0 k-ft 460 0 0 434.8 Right (lbs) 98 652 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Span(ft)=[LL Deflection <l <br="">otal Deflection<l <br="">CD= point Load 1 point Load 2</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1	1.00 1.00	floor wall misc. (ft)(left) GLB 24F-V4	15 Dad Factors Ap DL	100 pplied LL	P2DL= P2LL= MTL= Loading trib (ft) 4 Total Reactions	0 0 74520 Max M=	0 0 74.5 LLw(plf) 400 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 k-ft 7Lw(plf) 460 0 0 0 434.8 8 8 652 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
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CD= 1 Misc. 1.00 wall misc. 0	Mark: Span(ft)=[3		1.00				P2DL= P2LL= MTL= Loading trib (ft)	0 0 12880 Max M=	0 0 12.9	0 0 k-ft TLw(plf)
Image: 0 <td>Mark: Span(ft)= LL. Deflection<l <="" td=""><td>3 360</td><td>Dead Floor Live</td><td>1.00</td><td>roof</td><td></td><td></td><td>P2DL= P2LL= MTL= Loading trib (ft)</td><td>0 0 12880 Max M= DLw(plf) 210</td><td>0 0 12.9 LLw(plf) 1400</td><td>0 0 k-ft TLw(plf) 1610</td></l></td>	Mark: Span(ft)= LL. Deflection <l <="" td=""><td>3 360</td><td>Dead Floor Live</td><td>1.00</td><td>roof</td><td></td><td></td><td>P2DL= P2LL= MTL= Loading trib (ft)</td><td>0 0 12880 Max M= DLw(plf) 210</td><td>0 0 12.9 LLw(plf) 1400</td><td>0 0 k-ft TLw(plf) 1610</td></l>	3 360	Dead Floor Live	1.00	roof			P2DL= P2LL= MTL= Loading trib (ft)	0 0 12880 Max M= DLw(plf) 210	0 0 12.9 LLw(plf) 1400	0 0 k-ft TLw(plf) 1610
Load DL(lbs) LL(lbs) x/ft)(left) DL LL Reactions oint Load 1 roof 0 <td< td=""><td>Mark: Span(ft)=[LL Deflection<l <br="">iotal Deflection<l <="" td=""><td>3 360 240</td><td>Dead Floor Live Roof Live/Snow</td><td>1.00 1.00 1.00</td><td>roof floor</td><td></td><td></td><td>P2DL= P2LL= MTL= Loading trib (ft)</td><td>0 0 12880 Max M= DLw(plf) 210 0</td><td>0 0 12.9 LLw(plf) 1400 0</td><td>0 0 k-ft <u>TLw(plf)</u> 1610 0</td></l></l></td></td<>	Mark: Span(ft)=[LL Deflection <l <br="">iotal Deflection<l <="" td=""><td>3 360 240</td><td>Dead Floor Live Roof Live/Snow</td><td>1.00 1.00 1.00</td><td>roof floor</td><td></td><td></td><td>P2DL= P2LL= MTL= Loading trib (ft)</td><td>0 0 12880 Max M= DLw(plf) 210 0</td><td>0 0 12.9 LLw(plf) 1400 0</td><td>0 0 k-ft <u>TLw(plf)</u> 1610 0</td></l></l>	3 360 240	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	roof floor			P2DL= P2LL= MTL= Loading trib (ft)	0 0 12880 Max M= DLw(plf) 210 0	0 0 12.9 LLw(plf) 1400 0	0 0 k-ft <u>TLw(plf)</u> 1610 0
Load DL(lbs) LL(lbs) x(ft)(left) DL LL Left (lbs) Right (lbs) bint Load 1 roof 0	Mark: Span(ft)=[LL Deflection <l <br="">jotal Deflection<l <="" td=""><td>3 360 240</td><td>Dead Floor Live Roof Live/Snow</td><td>1.00 1.00 1.00</td><td>roof floor wall</td><td></td><td></td><td>P2DL= P2LL= MTL= Loading trib (ft)</td><td>0 0 12880 Max M= DLw(plf) 210 0 0</td><td>0 0 12.9 LLw(plf) 1400 0 0</td><td>0 0 k-ft TLw(plf) 1610 0 0</td></l></l>	3 360 240	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	roof floor wall			P2DL= P2LL= MTL= Loading trib (ft)	0 0 12880 Max M= DLw(plf) 210 0 0	0 0 12.9 LLw(plf) 1400 0 0	0 0 k-ft TLw(plf) 1610 0 0
Load DL(lbs) LL(lbs) x(ft)(left) DL LL Left (lbs) Right (lbs) bint Load 1 roof 0	Mark: Span(ft)=[LL Deflection <l <br="">jotal Deflection<l <="" td=""><td>3 360 240</td><td>Dead Floor Live Roof Live/Snow</td><td>1.00 1.00 1.00</td><td>roof floor wall</td><td></td><td></td><td>P2DL= P2LL= MTL= Loading trib (ft) 14</td><td>0 0 12880 Max M= DLw(plf) 210 0 0 0</td><td>0 0 12.9 LLw(plf) 1400 0 0 0</td><td>0 0 k-ft TLw(plf) 161(0 0 0</td></l></l>	3 360 240	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	roof floor wall			P2DL= P2LL= MTL= Loading trib (ft) 14	0 0 12880 Max M= DLw(plf) 210 0 0 0	0 0 12.9 LLw(plf) 1400 0 0 0	0 0 k-ft TLw(plf) 161(0 0 0
Dim Load 2 roof RLL= 2100 2100 DIM TIMBER LVL GLB Point Loads P1DL= 0 0 Wood Species= Douglas Fir #2 Douglas Fir #1 LVL 24F-V4 P2DL= 0 0 Wood Species= Douglas Fir #2 Douglas Fir #1 LVL 24F-V4 RTL= 2415 2415 Option 1 No Single Ply 4x8 (1) 1.75x9.5 3.125x9 Max Shear= 2.4 k Option 2 (2) 2x8 6x6 (2) 1.75x5.5 6.75x9 Moment 245 k Option 3 (3) 2x6 8x8 (3) 1.75x5.5 8.75x9 Moment 236 0 0 Option 4 (4) 2x6 10x10 (4) 1.75x12 - 10.75x12 RDL= 236 0 0 Option 4 - - - - 0 0 0 Option 5 - 16x16 - - - 0 0 0	Mark: Span(ft)=[LL Deflection <l <br="">jotal Deflection<l <="" td=""><td>3 360 240</td><td>Dead Floor Live Roof Live/Snow</td><td>1.00 1.00 1.00</td><td>roof floor wall misc.</td><td>15</td><td>100</td><td>P2DL= P2LL= MTL= Loading trib (ft) 14 Total</td><td>0 0 12880 Max M= DLw(plf) 210 0 0 0</td><td>0 0 12.9 LLw(plf) 1400 0 0 0</td><td>0 0 k-ft TLw(plf) 161(0 0 0</td></l></l>	3 360 240	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	roof floor wall misc.	15	100	P2DL= P2LL= MTL= Loading trib (ft) 14 Total	0 0 12880 Max M= DLw(plf) 210 0 0 0	0 0 12.9 LLw(plf) 1400 0 0 0	0 0 k-ft TLw(plf) 161(0 0 0
DIM TIMBER LVL GLB Point Loads P1DL= 0 0 Vood Species= Douglas Fir #2 Douglas Fir #1 LVL 24F-V4 P1UL= 0 0 Vood Species= Douglas Fir #2 Douglas Fir #1 LVL 24F-V4 RTL= 2415 2415 Option 1 No Single Ply 4x8 (1) 1.75x9.5 3.125x9 Max Shear= 2.4 k Option 2 (2) 2x8 6x6 (2) 1.75x5.5 6.75x9 Moment 2.4 k Option 4 (4) 2x6 10x10 (4) 1.75x5.5 8.75x9 Moment Point Load 1 Point Load 1 Option 5 - 10.75x12 - 10.75x12 0 0 Option 6 - 14x14 - - 0 0 0 Option 7 16x16 - - - 0 0 0	Mark: Span(ft)=[LL Deflection <l <br="">otal Deflection<l <br="">CD=]</l></l>	3 360 240 1	Dead Floor Live Roof Live/Snow Misc,	1.00 1.00 1.00 1.00	roof floor wall misc, Lo	15 ad Factors App	100	P2DL= P2LL= MTL= Loading trib (ft) 14 Total	0 0 12880 Max M= DLw(plf) 210 0 0 0	0 0 12.9 LLw(pif) 1400 0 0 0 1	0 0 k-ft <u>TLw(plf)</u> 1610 0 0 0
DIM TIMBER LVL GLB P1LL= 0 0 P2DL= 0 0 P2DL= 0 0 Wood Species= Douglas Fir #2 Douglas Fir #1 LVL 24F-V4 RTL= 2415 2415 Option 1 No Single Ply 4x8 (1) 1.75x9.5 3.125x9 Max Shear= 2.4 k Option 2 (2) 2x8 6x6 (2) 1.75x5.5 5.125x6 Moment 2.4 k Option 3 (3) 2x6 8x8 (3) 1.75x5.5 6.75x9 Moment Voment Point Load Point Load Option 4 (4) 2x6 10x1.0 (4) 1.75x5.5 8.75x9 RDL= 236 0 0 Option 5 - 12x12 - 10.75x12 RDL= 236 0 0 Option 6 - 14x14 - - 0 0 0 0 Option 7 - 16x16 - - 0 0 0 0 <td>Mark: Span(ft)=[LL Deflection<l <br="">otal Deflection<l <br="">CD=]</l></l></td> <td>3 360 240 1</td> <td>Dead Floor Live Roof Live/Snow Misc,</td> <td>1.00 1.00 1.00 1.00</td> <td>roof floor wall misc, Lo</td> <td>15 ad Factors App DL</td> <td>100 plied LL</td> <td>P2DL= P2LL= MTL= trib (ft) 14 Total Reactions</td> <td>0 0 12880 Max M= DLw(plf) 210 0 0 0 210.0</td> <td>0 0 12.9 LLw(plf) 1400 0 0 0 0 1 Left (lbs)</td> <td>0 0 k-ft 1610 0 0 1400.0 Right (lbs</td>	Mark: Span(ft)=[LL Deflection <l <br="">otal Deflection<l <br="">CD=]</l></l>	3 360 240 1	Dead Floor Live Roof Live/Snow Misc,	1.00 1.00 1.00 1.00	roof floor wall misc, Lo	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= trib (ft) 14 Total Reactions	0 0 12880 Max M= DLw(plf) 210 0 0 0 210.0	0 0 12.9 LLw(plf) 1400 0 0 0 0 1 Left (lbs)	0 0 k-ft 1610 0 0 1400.0 Right (lbs
Mod Species Douglas Fir #2 Douglas Fir #1 LVL 24F-V4 RTL= 2415 2415 Option 1 No Single Ply 4x8 (1) 1.75x9.5 3.125x9 Max Shear= 2.4 k Option 2 (2) 2x8 6x6 (2) 1.75x5.5 5.125x6 Moment k Option 3 (3) 2x6 8x8 (3) 1.75x5.5 6.75x9 Moment v v v Option 4 (4) 2x6 10x10 (4) 1.75x5.5 8.75x9 RDL= 236 0 0 Option 5 - 12x12 - 10.75x12 RDL= 236 0 0 Option 6 - 14x14 - - P10L= 0 0 0 Option 7 - 16x16 - - P10L= 0 0 0	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD=</l></l>	3 360 240 1 Load	Dead Floor Live Roof Live/Snow Misc,	1.00 1.00 1.00 1.00	roof floor wall misc, Lo	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= trib (ft) 14 Total Reactions	0 0 12880 Max M= DLw(plf) 210 0 0 0 210.0 RDL=	0 0 12.9 LLw(plf) 1400 0 0 0 1 Left (lbs) 315	0 0 k-ft 1610 0 0 0 1400.0 Right (lbs 315
Wood Species Douglas Fir #2 Douglas Fir #1 LVL 24F-V4 RTL= 2415 2415 2415 Option 1 No Single Ply 4x8 (1) 1.75x9.5 3.125x9 Max Shear= 2.4 k Option 2 (2) 2x8 6x6 (2) 1.75x5.5 5.125x6 Moment k Option 3 (3) 2x6 8x8 (3) 1.75x5.5 6.75x9 Moment k No Option 4 (4) 2x6 10x1.0 (4) 1.75x5.5 8.75x9 RDL 236 0 0 Option 5 - 12x12 - 10.75x12 RDL 236 0 0 Option 6 - 14x14 - - 1075x12 0 0 0 Option 7 - 16x16 - - 0 0 0 0	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD=</l></l>	3 360 240 1 Load	Dead Floor Live Roof Live/Snow Misc,	1.00 1.00 1.00 1.00	roof floor wal misc. (ft)(left)	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= trib (ft) 14 Total Reactions Uniform	0 0 12880 Max M= DLw(plf) 210 0 0 210.0 RDL= RLL=	0 0 12.9 LLw(plf) 1400 0 0 0 2 0 1 2 100 2 100	0 0 k-ft 1610 0 0 0 1400,0 Right (lbs 315 2100
Wood Species= Douglas Fir #2 Douglas Fir #1 LVL 24F-V4 RTL= 2415 2415 2415 Option 1 No Single Ply 4x8 (1) 1.75x9.5 3.125x9 Max Shear= 2.4 k Option 2 (2) 2x8 6x6 (2) 1.75x5.5 5.125x6 Moment k Option 3 (3) 2x6 8x8 (3) 1.75x5.5 6.75x9 Moment k Option 4 (4) 2x6 10x10 (4) 1.75x5.5 8.75x9 Moment k Option 5 - 12x12 - 10.75x12 RDL= 236 0 0 Option 6 - 14x14 - - P1DL= 0 0 0 Option 7 - 16x16 - - P1DL= 0 0 0	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD=</l></l>	3 360 240 1 Load roof roof	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00 1.00	roof floor wal misc. (ft)(left)	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= trib (ft) 14 Total Reactions Uniform	0 0 12280 Max M= DLw(plf) 210 0 0 210.0 8 210.0 8 210.0 9 10L= 91DL=	0 0 12.9 LLw(plf) 1400 0 0 0 0 1 2 100 0 0 0	0 0 k-ft 1610 0 0 0 1400,0 Right (lbs 315 2100 0
Option 1 No Single Ply 4x8 (1) 1.75x9.5 3.125x9 Max Sheare 2.4 k Option 2 (2) 2x8 6x6 (2) 1.75x5.5 5.125x6 6.75x9 6.76x9 6.75x9 6.75x19 6.75x19 6.75x19	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD=</l></l>	3 360 240 1 Load roof roof	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00 1.00	roof floor wal misc. (ft)(left)	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= trib (ft) 14 Total Reactions Uniform	0 0 12280 Max M= DLw(plf) 210 0 0 210.0 210.0 RDL= RLL= P1DL= P1LL=	0 0 12.9 LLw(plf) 1400 0 0 0 0 2 1 2 100 0 0 0 0 0 0 0 0 0 0	0 0 k-ft 1610 0 0 0 0 1400.0 Right (lbs 2100 0 0 0 0 0
Option 1 No Single Ply 4x8 (1) 1.75x9,5 3.125x9 Max Shears 2.4 k Option 2 (2) 2x8 6x6 (2) 1.75x5,5 5.125x6 k Option 3 (3) 2x6 8x8 (3) 1.75x5,5 6.75x9 Moment Point Loa Point Loa Point Loa Option 4 (4) 2x6 10x10 (4) 1.75x5,5 8.75x9 Moment Option 5 - 10x12 - 10.75x12 RDL 236 0	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD=</l></l>	3 360 240 1 Load roof roof	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00 1.00	roof floor wal misc. (ft)(left)	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= trib (ft) 14 Total Reactions Uniform	0 0 12280 Max M= DLw(plf) 210 0 0 210.0 210.0 8 CL= RLL= P1DL= P1LL= P2DL=	0 0 12.9 LLw(plf) 1400 0 0 0 0 1 Left (lbs) Left (lbs) 2100 0 0 0	0 0 k-ft 1610 0 0 0 1400.0 Right (lbs 2100 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Option 2 (2) 2x8 6x6 (2) 1.75x5.5 5.125x6 Option 3 (3) 2x6 8x8 (3) 1.75x5.5 6.75x9 Option 4 (4) 2x6 10x10 (4) 1.75x5.5 8.75x9 Option 5 - 12x12 - 10.75x12 Option 6 - 14x14 - - Option 7 - 16x16 - - Option 7 - 0 0 0	Mark: Span(ft)=[LL Deflection <l <br="">otal Deflection<l <br="">CD= Dint Load 1</l></l>	3 360 240 1 Load roof roof DIM	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER	1.00 1.00 1.00 1.00 1.00 X LU(lbs) ×	(ft)(left)	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= trib (ft) 14 Total Reactions Uniform	0 0 12280 Max M= DLw(plf) 210 0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 0 210.0 0 0 210.0 0 0 0 210.0 0 0 210.0 0 0 0 210.0 0 0 0 0 210.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 12.9 LLw(plf) 1400 0 0 0 0 1 Left (lbs) 2100 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 k-ft 1610 0 0 1400.0 Right (lbs 3115 2.100 0 0 0 0 0 0 0 0
Option 3 (3) 2x6 8x8 (3) 1.75x5.5 6.75x9 Option 4 (4) 2x6 10x10 (4) 1.75x5.5 8.75x9 Option 5 - 12x12 - 10.75x12 Option 6 - 14x14 - RLL= 1575 0 Option 7 - 16x16 - - 0 0 PIDL= 0 0 0 0 0	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD= pint Load 1 pint Load 2</l></l>	3 360 240 1 Load roof roof DIM Douglas Fir #2	Dead Floor Live Roof Live/Snow Misc. DL{Ibs) TIMBER Douglas Fir #1	1.00 1.00 1.00 1.00 1.00 LU(lbs) x	(ft)(left) GLB 24F-V4	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= trib (ft) 14 Total Reactions Uniform	0 0 12280 Max M= DLw(plf) 210 0 0 210.0 21	0 0 12.9 LLw(plf) 1400 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 k-ft 1610 0 0 1400.0 Right (lbs 315 2100 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Option 5 - 12x12 - 10.75x12 RDL= 236 0 0 Option 6 - 14x14 - - RLL= 1575 0 0 Option 7 - 16x16 - - P1DL= 0 0 0 P1LL= 0	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD= Dint Load 1 Dint Load 1 Dint Load 2 System 1</l></l>	3 360 240 1 Load roof roof DIM Douglas Fir #2 No Single Ply	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x8	1.00 1.00 1.00 1.00 1.00 1.00 LU(lbs) × LVL LVL (1) 1.75x9.5	(ft)(left) GLB 24F-V4 3.125x9	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= trib (ft) 14 Total Reactions Uniform	0 0 12280 Max M= DLw(plf) 210 0 0 210.0 21	0 0 12.9 LLw(plf) 1400 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 k-ft 1610 0 0 1400.0 Right (lbs 315 2100 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Option 6 14x14 - RLL= 1575 0 0 Option 7 16x16 - - P1DL= 0 0 0 P1UL= 0 0 0 0 0 0 0	Mark: Span(ft)= LL Deflection <l <br="">CD= Dint Load 1 Dint Load 2 Wood Species= Option 1 Option 2</l>	3 360 240 1 Load roof roof DIM Douglas Fir #2 No Single Ply (2) 2x8	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x3 6x5 8x8	1.00 1.00	roof floor wall misc. (ft)(left) GLB 24F-V4 3.125x9 5.125x6	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= trib (ft) 14 Total Reactions Uniform Point Loads	0 0 12280 Max M= DLw(plf) 210 0 0 210.0 21	0 0 12.9 LLw(plf) 1400 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 k-ft 1610 0 0 1400.0 Right (lbs 315 2100 0 0 0 0 0 0 0 0 0 0 0
Option 7 16x16 - P1DL= 0 0 0 P1IL= 0 0 0 0 0 0 0	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD= bint Load 1 bint Load 2 Wood Species= Option 1 Option 2 Option 3 Option 4</l></l>	3 360 240 1 1 Load roof roof DIM Douglas Fir #2 No Single Ply (2) 2x8 (3) 2x6	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x8 6x6 8x8 10x10	1.00 1.05 1	roof floor wall misc. (ft)(left) GLB 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL=	0 0 12280 Max M= DLw(plf) 210 0 0 210.0 21	0 0 12.9 1400 0 0 0 0 0 1 1400 0 0 0 0 0 1 2 100 0 0 0 0 0 0 0 0 0 0	0 0 0 k-ft 1610 0 0 0 0 1400.0 7 Right (lbs 315 2100 0 0 0 0 0 2415 k
PILL= 0 0 0	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD= bint Load 1 bint Load 2 Wood Species= Option 1 Option 2 Option 3 Option 4 Option 5</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 No Single Ply (2) 2x8 (3) 2x6 (4) 2x6 -	Dead Floor Live Roof Live/Snow Misc. DL{lbs} TIMBER Douglas Fir #1 4x8 6x6 8x8 10x10 12x12	1.00 1.05 1.05 1.05 1.75x5.5 [3] 1.75x5.5 [4] 1.75x	roof floor wall misc. (ft)(left) GLB 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 10.75x12	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= trib (ft) 14 Total Reactions Uniform Point Loads Moment RDL=	0 0 12280 Max M= DLw(plf) 210 0 0 210.0 210.0 210.0 210.0 210.0 210.0 210.0 210.0 210.0 210.0 210.0 0 0 210.0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 0 0 210.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 12.9 1400 0 0 0 0 0 1 1400 0 0 0 0 1 2415 2.00 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 k-ft 1610 0 0 0 1400.0
	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD= Dint Load 1 Dint Load 2 Wood Species= Option 1 Option 2 Option 3 Option 4 Option 5 Option 6</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 No Single Ply (2) 2x8 (3) 2x6 (4) 2x6 (4) 2x6 -	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x3 6x6 8x8 10x10 12x12 14x14	1.00 1.00 1.00 1.00 1.00 1.00 1.00 LUL (lbs) x LVL (1) 1.75x9.5 (2) 1.75x5.5 (3) 1.75x5.5 (4) 1.75x5.5 (4) 1.75x5.5	roof floor wall misc, (ft)(left) GLB 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 8.75x9 8.75x9	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL=	0 0 12880 Max M= DLw(plf) 210 0 0 210.0 210.0 210.0 8 RDL= RLL= P1DL= P2DL= P2DL= P2DL= P2DL= P2DL= P2DL= P2DL= P2DL= Center 236 1575	0 0 12.9 1400 0 0 0 0 0 1 1400 0 0 0 0 0 0 0 0 0	0 0 0 k-ft 1610 0 0 0 1400.0 Right (lbs 315 2100 0 0 0 0 0 2415 k k Point Loa 0 0
	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD= Dint Load 1 Dint Load 2 Wood Species= Option 1 Option 2 Option 4 Option 5 Option 6</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 No Single Ply (2) 2x8 (3) 2x6 (4) 2x6 (4) 2x6 -	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x3 6x6 8x8 10x10 12x12 14x14	1.00 1.00 1.00 1.00 1.00 1.00 1.00 LUL (lbs) x LVL (1) 1.75x9.5 (2) 1.75x5.5 (3) 1.75x5.5 (4) 1.75x5.5 (4) 1.75x5.5	roof floor wall misc, (ft)(left) GLB 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 8.75x9 8.75x9	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= Interpretations Total Reactions Uniform Point Loads Moment RDL= RLL= P1DL=	0 0 12880 Max M= DLw(plf) 210 0 0 210.0 210.0 210.0 210.0 8 RDL= RLL= P1DL= P2DL= P2L= P2L= RTL= Max Shear= 236 1575 0	0 0 12.9 1400 0 0 0 0 1 1400 0 0 0 1 1 1400 0 0 0	0 0 0 k-ft 1610 0 0 0 1400,0 1400,0 1400,0 1400,0 0 1400,0 0 0 0 0 0 0 0 2415 k k
P2II= 0 0 0	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD= Dint Load 1 Dint Load 2 Wood Species= Option 1 Option 2 Option 4 Option 5 Option 6</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 No Single Ply (2) 2x8 (3) 2x6 (4) 2x6 (4) 2x6 -	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x3 6x6 8x8 10x10 12x12 14x14	1.00 1.00 1.00 1.00 1.00 1.00 1.00 LUL (lbs) x LVL (1) 1.75x9.5 (2) 1.75x5.5 (3) 1.75x5.5 (4) 1.75x5.5 (4) 1.75x5.5	roof floor wall misc, (ft)(left) GLB 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 8.75x9 8.75x9	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= Itrib (ft) 14 Total Reactions Uniform Point Loads Uniform Point Loads	0 0 12880 Max M= DLw(plf) 210 0 0 210.0 0 210.0 0 210.0 0 210.0 0 0 210.0 0 0 210.0 0 210.0 0 0 210.0 0 0 210.0 0 0 210.0 0 0 210.0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 0 210.0 0 0 0 0 0 210.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 12.9 1400 0 0 0 0 0 1 2 1 5 100 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 0 k-ft 161(0 0 0 0 1400.0 1400.0 1400.0 1400.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
1212 0 0 0	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD= Dint Load 1 Dint Load 2 Wood Species= Option 1 Option 2 Option 3 Option 4 Option 5 Option 6</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 No Single Ply (2) 2x8 (3) 2x6 (4) 2x6 (4) 2x6 -	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x3 6x6 8x8 10x10 12x12 14x14	1.00 1.00 1.00 1.00 1.00 1.00 1.00 LUL (lbs) x LVL (1) 1.75x9.5 (2) 1.75x5.5 (3) 1.75x5.5 (4) 1.75x5.5 (4) 1.75x5.5	roof floor wall misc, (ft)(left) GLB 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 8.75x9 8.75x9	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= intermediate intermediate intermediate Point Loads Morent RDL= RLL= P1DL= P1DL= P1LL= P2DL=	0 0 12280 Max M= DLw(plf) 210 0 0 210.0 0 0 210.0 0 210.0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 0 210.0 0 0 0 0 0 0 0 0 0 210.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 12.9 1400 0 0 0 0 0 0 1 1400 0 0 0 0 0 0 0 0	0 0 0 k-ft 1610 0 0 0 0 1400.0 2415 2100 0 0 0 0 0 2415 k k Point Loa 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
MTL= 1811 0 0	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD= oint Load 1 oint Load 2 Wood Species= Option 1 Option 2 Option 4 Option 5 Option 6</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 No Single Ply (2) 2x8 (3) 2x6 (4) 2x6 (4) 2x6 -	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x3 6x6 8x8 10x10 12x12 14x14	1.00 1.00 1.00 1.00 1.00 1.00 1.00 LU(lbs) x LVL (1) 1.75x9.5 (2) 1.75x5.5 (3) 1.75x5.5 (4) 1.75x5.5 (4) 1.75x5.5 (5) 1.75x5.5 (5) 1.75x5.5 (5) 1.75x5.5 (6) 1.05x5.5 (7) 1.05x5.5 (7) 1.05x5.5 (7) 1.05x5.5 (8) 1.05x5.5 (7) 1.05x5.5	roof floor wall misc, (ft)(left) GLB 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 8.75x9 8.75x9	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= Interpretations Interpretations Uniform Point Loads Uniform Point Loads Moment RDL= RLL= P1DL= P1LL= P2DL= P2L=	0 0 12280 Max M= DLw(plf) 210 0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 0 210.0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 0 210.0 0 0 0 0 210.0 0 0 0 0 0 0 0 0 210.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 12.9 1400 0 0 0 0 0 0 1401 0 0 0 0 0 0 0 0 0	0 0 0 k-ft 1610 0 0 0 0 1400.0 2415 2100 0 0 0 0 0 0 0 2415 k k Point Loa 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
	Mark: Span(ft)= LL Deflection <l <br="">otal Deflection<l <br="">CD= oint Load 1 oint Load 2 Wood Species= Option 1 Option 2 Option 4 Option 5 Option 6</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 No Single Ply (2) 2x8 (3) 2x6 (4) 2x6 (4) 2x6 -	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x3 6x6 8x8 10x10 12x12 14x14	1.00 1.00 1.00 1.00 1.00 1.00 1.00 LU(lbs) x LVL (1) 1.75x9.5 (2) 1.75x5.5 (3) 1.75x5.5 (4) 1.75x5.5 (4) 1.75x5.5 (5) 1.75x5.5 (5) 1.75x5.5 (5) 1.75x5.5 (6) 1.05x5.5 (7) 1.05x5.5 (7) 1.05x5.5 (7) 1.05x5.5 (8) 1.05x5.5 (7) 1.05x5.5	roof floor wall misc, (ft)(left) GLB 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 8.75x9 8.75x9	15 ad Factors App DL	100 plied LL	P2DL= P2LL= MTL= Interpretations Interpretations Uniform Point Loads Uniform Point Loads Moment RDL= RLL= P1DL= P1LL= P2DL= P2L=	0 0 12280 Max M= DLw(plf) 210 0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 210.0 0 0 210.0 0 0 210.0 0 0 0 210.0 0 0 0 210.0 0 0 0 0 210.0 0 0 0 0 210.0 0 0 0 0 0 0 0 0 210.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0 12.9 1400 0 0 0 0 0 0 1401 0 0 0 0 0 0 0 0 0	0 0 0 k-ft 1610 0 0 0 0 1400.0 2415 2100 0 0 0 0 0 0 0 2415 k k Point Loa 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0

368 Wooden Spur D JOB NO.	r. Alpine, Wyoming 225112	SHEET NO.	21 120
JOB NO.	225112	CHEET NO	21 170
	AL 4. 4 AL 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4.	SHEET NO.	610151
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t Laterally Supported)			
	and the second se	CHECKED BY 0	CHECKED BY 0 DATE

Span(ft)=	4	Dead	1.00	C	DL (psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)
LL Deflection <l <="" th=""><th></th><th>Floor Live</th><th>1.00</th><th>roof</th><th>15</th><th>100</th><th>4</th><th>60</th><th>400</th><th></th></l>		Floor Live	1.00	roof	15	100	4	60	400	
otal Deflection <l <="" th=""><th></th><th>Roof Live/Snow</th><th></th><th>floor</th><th></th><th></th><th></th><th>0</th><th>0</th><th>0</th></l>		Roof Live/Snow		floor				0	0	0
CD=		Misc.		wall				0	0	Ö
				misc.	en same			0	0	0
				-		1997 - 1998 - 1998 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 -	Total	65.	2	434.8
				Loa	d Factors A	pplied	Reaction	15		
11.15.2	Load	DL(lbs)	LL(lbs)	x(ft)(left)	DL	L			Left (lbs)	Right (lb
oint Load 1	roof				0	0	Uniforr	n RDL:	= 130	13
int Load 2	roof				0	0		RLL		87
							Point Loa	ds P1DL:	= 0	0
	DIM	TIMBER	LVL	GLB				P1LL:	= 0	0
								P2DL:	= 0	0
		2		1				P2LL:		0
Wood Species=	Douglas Fir #2	Douglas Fir #1	LVL	24F-V4				RTL:		
Option 1	(1) 2x8	4x4	(1) 1.75x5.5	3.125x9				Max Shear	= 1.0	k
Option 2	(2) 2x6	6x6	(2) 1.75x5.5	5.125x6						
Option 3	(3) 2x4	8x8	(3) 1.75x5.5	6.75x9			Momen	t	14 3. C 14 3	Constant State
Option 4	(4) 2x4	10x10	(4) 1.75x5.5	8.75x9				Center	Point Load 1	Point Lo
Option 5	-	12x12	-	10.75x12			RI	DL= 130	0	0
Option 6	-	14x14		-			R	L= 870	0	0
Option 7	-	16x16	-	-			P11	0L= 0	0	0
L				and and the state of the state			P1	.L= 0	0	0
							P2I		0	0
							P2	.L= 0	0	0
								rL= 1000 Max M=	0 1.0	0 k-f
Mark:		Load F					M	rL= 1000 Max M=	1.0	k-f
Span(ft)=	3	Dead	1.00	and the second	IL (psf)	LL(psf)	M Loading trib (ft)	TL= 1000 Max M= DLw(plf)	1.0 LLw(plf)	k-f
Span(ft)=	3 360	Dead Floor Live	1.00 1.00	roof	1L (psf) 15	LL(psf) 100	M	TL= 1000 Max M= DLw(plf) 60	1.0 	K-f
Span(ft)= LL Deflection <l <br="">al Deflection<l <="" td=""><td>3 360 240</td><td>Dead Floor Live Roof Live/Snow</td><td>1.00 1.00 1.00</td><td>roof floor</td><td></td><td></td><td>M Loading trib (ft)</td><td>TL= 1000 Max M= DLw(plf) 60 0</td><td>1.0 </td><td>TLw(plf) 46(0</td></l></l>	3 360 240	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	roof floor			M Loading trib (ft)	TL= 1000 Max M= DLw(plf) 60 0	1.0 	TLw(plf) 46(0
Span(ft)=	3 360 240	Dead Floor Live	1.00 1.00	roof			M Loading trib (ft)	TL= 1000 Max M= DLw(plf) 60	1.0 	K-f
Span(ft)= LL Deflection <l al Deflection<l< th=""><th>3 360 240</th><th>Dead Floor Live Roof Live/Snow</th><th>1.00 1.00 1.00</th><th>roof floor wall</th><th></th><th></th><th>M Loading trib (ft)</th><th>TL= 1000 Max M= DLw(plf) 60 0 0</th><th>1.0 LLw(plf) 400 0 0 0 0 0</th><th>TLw(pif) 46(0 0</th></l<></l 	3 360 240	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	roof floor wall			M Loading trib (ft)	TL= 1000 Max M= DLw(plf) 60 0 0	1.0 LLw(plf) 400 0 0 0 0 0	TLw(pif) 46(0 0
Span(ft)= LL Deflection <l <br="">al Deflection<l <="" th=""><th>3 360 240</th><th>Dead Floor Live Roof Live/Snow</th><th>1.00 1.00 1.00</th><th>roof floor wall misc.</th><th></th><th>100</th><th>M Loading trib (ft) 4</th><th>TL= 1000 Max M= DLw(plf) 60 0 0 0 65.2</th><th>1.0 LLw(plf) 400 0 0 0 0 0</th><th>TLw(pif) 460 0 0 0</th></l></l>	3 360 240	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	roof floor wall misc.		100	M Loading trib (ft) 4	TL= 1000 Max M= DLw(plf) 60 0 0 0 65.2	1.0 LLw(plf) 400 0 0 0 0 0	TLw(pif) 460 0 0 0
Span(ft)= LL Deflection <l <br="">al Deflection<l <br="">CD=</l></l>	3 360 240 1	Dead Floor Live Roof Live/Snow Misc.	1,00 1.00 1.00 1.00	roof floor wall misc.	15 15 d Factors Ap DL	pplied	M Loading trib (ft) 4 Total Reaction	rL= 1000 Max M= DLw(plf) 60 0 0 0 65.2 s	1.0 LLw(plf) 400 0 0 2 Left (lbs)	k-f
Span(ft)= LL Deflection <l <br="">al Deflection<l <br="">CD= the Load 1</l></l>	3 360 240 1 Load	Dead Floor Live Roof Live/Snow Misc.	1,00 1.00 1.00 1.00	roof floor wall misc, Loac	15 d Factors Ap DL 0	pplied UL 0	Loading trib (ft) 4 Total	TL= 1000 Max M= DLw(plf) 60 0 0 0 65.2 s RDL=	1.0 LLw(plf) 400 0 0 2 Left (lbs) 98	TLw(plf) 460 0 0 434.8 Right (lb: 98
Span(ft)= LL Deflection <l <br="">al Deflection<l <br="">CD= the Load 1</l></l>	3 360 240 1	Dead Floor Live Roof Live/Snow Misc.	1,00 1.00 1.00 1.00	roof floor wall misc, Loac	15 15 d Factors Ap DL	pplied	M Loading trib (ft) 4 Total Reaction Uniform	TL= 1000 Max M= DLw(plf) 60 0 0 65,2 s RDL= RLL=	1.0 LLw(plf) 400 0 0 0 2 Left (lbs) 98 552	TLw(pif) TLw(pif) 460 0 0 0 434.8 Right (lb: 98 652
Span(ft)= L Deflection <l i<br="">al Deflection<l i<br="">CD= L t Load 1</l></l>	3 360 240 1 Load roof roof	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00 1.00	roof floor wall misc. x(ft)(left)	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction	TL= 1000 Max M= DLw(plf) 60 0 0 65,2 s RDL= RLL= RLL=	1.0 LLw(plf) 400 0 0 0 2 Left (lbs) 98 552 0	TLw(pif) TLw(pif) 460 0 0 0 434.8 Right (lb: 98 652 0
Span(ft)= L Deflection <l <br="">al Deflection<l <br="">CD= t Load 1</l></l>	3 360 240 1 Load	Dead Floor Live Roof Live/Snow Misc.	1,00 1.00 1.00 1.00	roof floor wall misc, Loac	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform	TL= 1000 Max M= DLw(pif) 60 0 0 0 65,2 s s RDL= RLL= RLL= p1LL=	1.0 LLw(plf) 400 0 0 0 2 Left (lbs) 5 98 652 0 0 0 0 0 0 0 0 0 0 0 0 0	TLw(pif) 466 0 0 434.8 Right (lb: 98 652 0 0 0
Span(ft)= L Deflection <l i<br="">al Deflection<l i<br="">CD= L t Load 1</l></l>	3 360 240 1 Load roof roof	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00 1.00	roof floor wall misc. x(ft)(left)	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform	TL= 1000 Max M= DLw(pif) 60 0 0 65,2 5 s RDL= RLE RLE RLE P1DL= P1LL= P2DL=	1.0 LLw(plf) 400 0 0 2 Left (lbs) 5 8 652 0 0 0 0 0 0 0 0 0 0 0 0 0	TLw(plf) 460 0 434.8 Right (lbs 652 0 0
Span(ft)= LL Deflection <l <br="">al Deflection<l <br="">CD= the Load 1 the Load 2 the Load 2 the Load 2</l></l>	3 360 240 1 Load roof roof DIM	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00 1.00	x(ft)(left)	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform	TL= 1000 Max M= DLw(plf) 60 0 0 65.2 s s RDL= RLL= RLL= P1DL= P1LL= P2DL= P2LL=	1.0 LLw(plf) 400 0 0 2 Left (lbs) 98 652 0 0 0 0 0 0 0 0 0 0 0 0 0	K-f
Span(ft)= L Deflection <l 2<br="">al Deflection<l 2<br="">CD= t Load 1 t Load 2 Wood Species=</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1	1.00 1.00 1.00 1.00 1.00 LU(lbs)	x(ft)(left) GLB 24F-V4	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform	TL= 1000 Max M= DLw(plf) 60 0 0 65.2 s s r tLL= P1DL= P1LL= P2DL= P2LL= RTL=	1.0 LLw(plf) 400 0 0 2 Left (lbs) 98 652 98 652 0 0 0 0 0 0 0 750	k-f
Span(ft)= LL Deflection <l <br="">al Deflection<l <br="">CD= t t Load 1 t Load 2 Wood Species= Option 1</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 (1) 2x6	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x4	1.00 1.00 1.00 1.00 1.00 1.00 LU(lbs)	x(ft)(left) GLB 24F-V4 3.125x9	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform	TL= 1000 Max M= DLw(plf) 60 0 0 65.2 s s RDL= RLL= RLL= P1DL= P1LL= P2DL= P2LL=	1.0 LLw(plf) 400 0 0 2 Left (lbs) 98 652 98 652 0 0 0 0 0 0 0 750	K-f
Span(ft)= LL Deflection <l <br="">al Deflection<l <br="">CD= t t Load 1 t Load 2 Wood Species= Option 1 Option 2</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 (1) 2x6 (2) 2x4	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x4 6x6	1,00 1,00 1,00 1,00 1,00 LL(lbs) LVL (l) 1,75x5.5 (2) 1,75x5.5	x(ft)(left) 24F-V4 3.125x9 5.125x6	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform Point Loa	TL= 1000 Max M= DLw(plf) 60 0 0 65.2 s RDL= RLL= 91DL= P1LL= P2LL= P2LL= RTL= Max Shear=	1.0 LLw(plf) 400 0 0 2 Left (lbs) 98 652 98 652 0 0 0 0 0 0 0 750	k-f
Span(ft)= LL Deflection <l <br="">cD= cD= tal Deflection<l <br="">cD= tal Load 1 tal Load 2 Wood Species= Option 1 Option 2 Option 3</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 (1) 2x6 (2) 2x4 (3) 2x4	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x4 6x6 8x8	1,00 1,00 1,00 1,00 1,00 LUL LVL (1) 1,75x5.5 (2) 1,75x5.5 (3) 1,75x5.5	x(ft)(left) 24F-V4 3.125x9 5.125x6 6.75x9	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform	TL= 1000 Max M= DLw(pif) 60 0 0 65.2 s s RDL= RLL= P1L= P2L= P2L= RTL= Max Shear=	1.0 LLw(plf) 400 0 0 2 Left (lbs) 2 5 98 652 5 0 0 0 0 0 0 0 0 0 0 0 0 0	k-f
Span(ft)= LL Deflection <l <br="">al Deflection<l <br="">CD= the Load 1 the Load 2 Wood Species= Option 1 Option 2 Option 3 Option 4</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 (1) 2x6 (2) 2x4 (3) 2x4 (4) 2x4	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x4 6x6 8x8 10x10	1,00 1,00 1,00 1,00 1,00 1,00 LVL LVL (1) 1,75x5.5 (2) 1,75x5.5 (3) 1,75x5.5 (4) 1,75x5.5	Coar roof floor wall misc. Loar x(ft)(left) GLB 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 8.75x9	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform Point Loa	TL= 1000 Max M= DLw(plf) 60 0 0 65,2 5 5 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	1.0 LLw(plf) 400 0 0 2 Left (lbs) 98 652 98 652 0 0 0 0 0 0 0 0 0 0 0 0 0	TLw(plf) 460 0 434.8 Right (lbs) 98 655 0 0 0 756 k Point Los
Span(ft)= LL Deflection <l <br="">al Deflection<l <br="">CD= the Load 1 the Load 1 the Load 2 Wood Species= Option 1 Option 3 Option 4 Option 5</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 (1) 2x6 (2) 2x4 (3) 2x4 (4) 2x4 -	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x4 6x6 8x8 10x10 12x12	1.00 1.00 1.00 1.00 1.00 1.00 LUL (I) 1.75x5.5 (2) 1.75x5.5 (3) 1.75x5.5 (4) 1.75x5.5 (4) 1.75x5.5 (5) 1.75x5.5 (4) 1.75x5.5 (5)	Coar roof floor wall misc. Loac x(ft)(left) GLB 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 10.75x12	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform Point Loa Momen	TL= 1000 Max M= DLw(plf) 60 0 0 65.2 s s RDL= RLL= P1DL= P1LL= P2DL= P2LL= RTL= Max Shear= Center L= 73	1.0 LLw(plf) 400 0 0 2 Left (lbs) 98 652 98 652 0 0 0 0 0 0 0 0 0 0 0 0 0	k-f
Span(ft)= LL Deflection <l <br="">tal Deflection<l <br="">CD= T t Load 1 t Load 2 Wood Species= Option 1 Option 2 Option 3 Option 4 Option 5 Option 6</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 (1) 2x6 (2) 2x4 (3) 2x4 (4) 2x4 - -	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x4 6x6 8x8 10x10 12x12 14x14	1.00 1.00 1.00 1.00 1.00 1.00 LUL (lbs) LVL (1) 1.75x5.5 (2) 1.75x5.5 (3) 1.75x5.5 (4) 1.75x5.5 (4) 1.75x5.5 (5) 1.75x5.5 (5) 1.75x5.5 (5) 1.75x5.5 (6) 1.75x5.5 (7) 1.75x5.5 (7) 1.75x5.5 (8) 1.75x5.5 (7) 1.75x5.5 (8) 1.75x5.5 (9) 1	x(ft)(left) 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 1.75x12 -	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform Point Loa Momen Rt R	TL= 1000 Max M= DLw(plf) 60 0 0 65.2 s s RDL= RLL= P1DL= P1DL= P2DL= P2LL= RTL= Max Shear=	1.0 LLw(plf) 400 0 0 2 2 2 2 2 3 400 0 0 2 3 400 0 0 2 3 400 0 0 2 3 400 0 0 0 0 0 0 0 0 0 0 0 0	k-f
Span(ft)= LL Deflection <l <br="">al Deflection<l <br="">CD= the Load 1 the Load 1 the Load 2 Wood Species= Option 1 Option 3 Option 4 Option 5</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 (1) 2x6 (2) 2x4 (3) 2x4 (4) 2x4 -	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x4 6x6 8x8 10x10 12x12	1.00 1.00 1.00 1.00 1.00 1.00 LUL (I) 1.75x5.5 (2) 1.75x5.5 (3) 1.75x5.5 (4) 1.75x5.5 (4) 1.75x5.5 (5) 1.75x5.5 (4) 1.75x5.5 (5)	Coar roof floor wall misc. Loac x(ft)(left) GLB 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 10.75x12	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform Point Loa Momen RE RI RI	TL= 1000 Max M= DLw(plf) 60 0 0 65.2 s s RDL= RLL= 91DL= P1LL= P1LL= P1LL= P1LL= P1LL= Center L= 73 L= 73 L= 789 L= 0	1.0 LLw(plf) 400 0 0 2 Left (lbs) 2 Left (lbs) 3 8 652 0 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0	k-f
Span(ft)= LL Deflection <l <br="">tal Deflection<l <br="">CD= T t Load 1 t Load 2 Wood Species= Option 1 Option 2 Option 3 Option 4 Option 5 Option 6</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 (1) 2x6 (2) 2x4 (3) 2x4 (4) 2x4 - -	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x4 6x6 8x8 10x10 12x12 14x14	1.00 1.00 1.00 1.00 1.00 1.00 LUL (lbs) LVL (1) 1.75x5.5 (2) 1.75x5.5 (3) 1.75x5.5 (4) 1.75x5.5 (4) 1.75x5.5 (5) 1.75x5.5 (5) 1.75x5.5 (5) 1.75x5.5 (6) 1.75x5.5 (7) 1.75x5.5 (7) 1.75x5.5 (8) 1.75x5.5 (7) 1.75x5.5 (8) 1.75x5.5 (9) 1	x(ft)(left) 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 1.75x12 -	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform Point Loa Momen Rt Rt Rt P1I	TL= 1000 Max M= DLw(plf) 60 0 0 65.2 s s RDL= RTL= P1LL= P2LL= P2LL= RTL= RTL= RTL= Aax Shear=	1.0 LLw(plf) 400 0 0 2 Left (lbs) 2 2 400 0 0 2 2 400 0 0 2 2 400 0 0 2 2 400 0 0 0 0 0 0 0 0 0 0 0 0	k-f
Span(ft)= LL Deflection <l <br="">tal Deflection<l <br="">CD= nt Load 1 nt Load 2 Wood Species= Option 1 Option 2 Option 3 Option 4 Option 5 Option 6</l></l>	3 360 240 1 Load roof DIM Douglas Fir #2 (1) 2x6 (2) 2x4 (3) 2x4 (4) 2x4 - -	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER Douglas Fir #1 4x4 6x6 8x8 10x10 12x12 14x14	1.00 1.00 1.00 1.00 1.00 1.00 LUL (lbs) LVL (1) 1.75x5.5 (2) 1.75x5.5 (3) 1.75x5.5 (4) 1.75x5.5 (4) 1.75x5.5 (5) 1.75x5.5 (5) 1.75x5.5 (5) 1.75x5.5 (6) 1.75x5.5 (7) 1.75x5.5 (7) 1.75x5.5 (8) 1.75x5.5 (7) 1.75x5.5 (8) 1.75x5.5 (9) 1	x(ft)(left) 24F-V4 3.125x9 5.125x6 6.75x9 8.75x9 1.75x12 -	15 d Factors Ap DL 0	pplied UL 0	M Loading trib (ft) 4 Total Reaction Uniform Point Loa Momen RE RI RI	TL= 1000 Max M= DLw(plf) 60 0 0 65,2 s s RDL= RTL= P1LL= P2LL= P2LL= P2LL= RTL= Max Shear= Center L= 73 L= 489 L= 0 L= 0 L= 0	1.0 LLw(plf) 400 0 0 2 Left (lbs) 2 Left (lbs) 3 8 652 0 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0	k-f

		TSE Eng					JOB TITLE	Wooden Spu			
		136 S Stat					368 Wooden Spur D		ming		00 22
		Shelley, Il					JOB NO.	225112		SHEET NO.	22 003
		208-357	-2420				CALCULATED BY	EVB		DATE	6/4/2025
							CHECKED BY	()	DATE	
			1	Wood Header Desig	n Or Wo	od Beam (Not	: Laterally Supported)		1.1		
					N	DS 2018					
Mar	K: HDR7	Load	Factors					Loading			
Span(f	:)= 3	Dea	d	1.00		DL (psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)
LL Deflection<	L/ 360	Floor Liv	e	1.00	roof	15	100	7	105	700	805
otal Deflection<	L/ 240	Roof Live/Sno	N	1.00	floor				0	0	0
C	D= 1	Mis	c.	1.00	wall				0	0	0
					misc.				0	0	0
								Total	105.0		700.0 805
					Lo	ad Factors Ap	plied	Reactions			
	Load	DL(lbs)	LL(lbs)	x(ft)(left)		DL	LL			Left (lbs)	Right (lbs)
oint Load 1	roof					0	0	Uniform	RDL=	158	158
oint Load 2	roof	direction de la company				0	0		RLL=	1050	1050
					1.00			Point Loads	P1DL=	0	0
	DIM	TIMBER	LV	L GLE	3				P1LL=	0	0

LVL DIM TIMBER GLB

Wood Species=	Douglas Fir #2	Douglas Fir #1	LVL	24F-V4
Option 1	(1) 2×8	4x4	(1) 1.75×5.5	3.125х9
Option 2	(2) 2x6	6x6	(2) 1.75x5.5	5.125x6
Option 3	(3) 2x4	8x8	(3) 1.75x5.5	6.75x9
Option 4	(4) 2x4	10x10	(4) 1.75×5.5	8.75x9
Option 5	10 10 - 10 10 10	12x12	-	10.75x12
Option 6	-	14x14	-	-
Option 7		16x16	-	-

	Center	Point Load 1	Point Load 2
RDL=	118	0	0
RLL=	788	0	0
P1DL=	0	0	0
P1LL=	0	0	0
P2DL=	0	0	0
P2LL=	0	0	0
MTL=	906	0	0
	Max M=	0.9	k-ft

0 0

1208

1.2

0

k

1208

P2DL= P2LL=

RTL=

Max Shear=

Mark: HDR8	Load Factors					Loading			Sec. 1
Span(ft)= 3	Dead	1.00	DL	(psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)
LL Deflection <l 360<="" td=""><td>Floor Live</td><td>1.00</td><td>roof</td><td>15</td><td>100</td><td>9</td><td>135</td><td>900</td><td>1035</td></l>	Floor Live	1.00	roof	15	100	9	135	900	1035
Total Deflection <l 240<="" td=""><td>Roof Live/Snow</td><td>1.00</td><td>floor</td><td></td><td></td><td></td><td>0</td><td>0</td><td>0</td></l>	Roof Live/Snow	1.00	floor				0	0	0
CD= 1	Misc.	1.00	wall	1.00			0	0	0
			misc.	ALC: NO			0	0	0
					In the second	Total	135	.0 9	00.0 10

					Load Factors Appl	ied		Reactions	Land and		1. J. M. 18.
	Load	DL(lbs)	LL(lbs)	x(ft)(left)	DL		LL			Left (lbs)	Right (lbs)
Point Load 1	roof				0		0	Uniform	RDL=	203	203
Point Load 2	roof			8	0		0		RLL=	1350	1350
	18 C 19 C			7. P. B. C.				Point Loads	P1DL=	0	0
	DIM	TIMBER	LVL	GLB					P1LL=	0	0
									P2DL=	0	0
									P2LL=	0	0
Wood Species=	Douglas Fir #2	Douglas Fir #1	LVL	24F-V4					RTL=	1553	1553
Option 1	(1) 2x10	4x6	(1) 1.75x5.5	3.125x9	5.05				Max Shear=	1.6	k
Option 2	(2) 2x6	6x6	(2) 1.75×5.5	5.125x6							
Option 3	(3) 2x6	8x8	(3) 1.75x5.5	6.75x9				Moment	Alter Smille	1 10 - 3 - 1 × 1 4	
Option 4	(4) 2x4	10x10	(4) 1.75×5.5	8.75x9				and service of the	Center	Point Load 1	Point Load
Option 5		12x12		10.75x12				RDL=	152	0	0
Option 6	- 1998 - 1998 - 1998 - 1998 - 1998 - 1998 - 1998 - 1998 - 1998 - 1998 - 1998 - 1998 - 1998 - 1998 - 1998 - 199	14x14		-				RLL=	1013	0	0
Option 7	-	16x16	-	-				P1DL=	0	0	0
								P1LL=	0	0	0
								P2DL=	0	0	0
								P2LL=	0	0	0
								MTL=	1164	0	0
									Max M=	1.2	k-ft

1.2

		TSE Engin				JOB TITLE		pur Renova	tion	
		136 S State S				368 Wooden Spur I		Vyoming		00 1
		Shelley, ID 8				JOB NO.	225112	A		0.2300
		208-716-24	126			CALCULATED BY CHECKED BY	EVB		DATE DATE	6/4/2025
			Wood Bea	m Design (Later	ral Support	ed Fully)				
				NDS 201						
Mark: E			Factors				ading			
Span(ft)= <u>1</u>		Dead			DL (psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)
LL Deflection <l 3<="" td=""><td></td><td>Floor Live</td><td></td><td>roof</td><td>15</td><td>100</td><td>8.5</td><td>127.5</td><td>850</td><td>977.5</td></l>		Floor Live		roof	15	100	8.5	127.5	850	977.5
Total Deflection <l 2<br="">CD= 1</l>		Roof Live/Snow	1.00	floor wall				0	0	0
CD= <u>1</u>	built contractor	Misc.	1.00	j wan misc.				0	0 0	0 0
				mise.			Total	127.5		
				Load	Factors A	pplied	Reactions			
Le	oad	DL(lbs)	LL(lbs)	x(ft)(left)	DL	LL			Left (lbs)	Right (Ibs)
Point Load 1 ro	oof			Constant and	0	0	Uniform	RDL=	765	765
Point Load 2 fl	oor			1893	0	0		RLL=		5100
							Point Loads	P1DL=	0	0
	DIM	TIMBER	LVL	GLB				P1LL=	0	0
	Ī			and so that				P2DL=		0
							$\sum_{i=1}^{n} a_{ii}$	P2LL=		0
Wood Species= D)ouglas Fir #2	Douglas Fir #2	LVL	24F-V4	Section .			RTL=	5865	5865
Option 1	No Single Ply	4x None	(1) 1.75x18	3.125x13.5	1		1	Max Shear=	5.9	k
ance	lo Double Ply	6x None	(2) 1.75×14	5.125x12	te da					
and a second sec	No Triple Ply	8x16	(3) 1.75x11.875	6.75x10.5	Re to t		Moment			
presen	No Four Ply	10x14	(4) 1.75x11.875	8.75x10.5			moment	Center	Pointlord	Point Load 2
Succes	no rou riy		(4) 1.7 3 11.873				DDI			
Option 5		12x14		10.75x12			RDL=	2295	0	0
Option 6 Option 7	-	14x14 16x16	-				RLL= P1DL=	15300 0	0 0	0 0
Option /		TOVIO					PIDL=	0	0	0
							P2DL=	0	0	0
							P2LL=	0	0	0
								17595	0	
							MTL=			0
							IVI I L=	Max M=	17.6	0 k-ft
							IVI I L=	Max M=		
							INT L=	Max M=		
Mark: B			Factors				ading		17.6	k-ft
Span(ft)= <u>1</u> 2	2	Dead	1.00	2	DL (psf)	LL(psf)	ading trib (ft)	DLw(plf)	17.6 LLw(plf)	k-ft TLw(plf)
Span(ft)= <u>12</u> LL Deflection <l <u="">36</l>	2 60	Dead Floor Live	1.00 1.00	roof	DL (psf) 15		ading	DLw(plf) 86.25	17.6 LLw(plf) 575	k-ft TLw(plf) 661.25
Span(ft)= 12 LL Deflection <l 36<br="">otal Deflection<l 24<="" td=""><td>2 60 40</td><td>Dead Floor Live Roof Live/Snow</td><td>1.00 1.00 1.00</td><td>roof floor</td><td></td><td>LL(psf)</td><td>ading trib (ft)</td><td>DLw(plf) 86.25 0</td><td>17.6 LLw(plf) 575 0</td><td>k-ft TLw(plf) 661.25 0</td></l></l>	2 60 40	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	roof floor		LL(psf)	ading trib (ft)	DLw(plf) 86.25 0	17.6 LLw(plf) 575 0	k-ft TLw(plf) 661.25 0
Span(ft)= <u>12</u> LL Deflection <l <u="">36</l>	2 60 40	Dead Floor Live	1.00 1.00	roof floor wall		LL(psf)	ading trib (ft)	DLw(plf) 86.25 0 0	17.6 LLw(plf) 575 0 0	k-ft TLw(plf) 661.25 0 0
Span(ft)= 12 LL Deflection <l 36<br="">otal Deflection<l 24<="" td=""><td>2 60 40</td><td>Dead Floor Live Roof Live/Snow</td><td>1.00 1.00 1.00</td><td>roof floor</td><td></td><td>LL(psf) 100</td><td>ading trib (ft)</td><td>DLw(plf) 86.25 0</td><td>17.6 LLw(plf) 575 0</td><td>k-ft TLw(plf) 661.25 0 0 0</td></l></l>	2 60 40	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	roof floor		LL(psf) 100	ading trib (ft)	DLw(plf) 86.25 0	17.6 LLw(plf) 575 0	k-ft TLw(plf) 661.25 0 0 0
Span(ft)= 12 LL Deflection <l 36<br="">otal Deflection<l 24<="" td=""><td>2 60 40</td><td>Dead Floor Live Roof Live/Snow</td><td>1.00 1.00 1.00</td><td>roof floor wall misc.</td><td>15</td><td>LL(psf) 100</td><td>ading trib (ft) 5.75 Total</td><td>DLw(plf) 86.25 0 0 0</td><td>17.6 LLw(plf) 575 0 0 0 0</td><td>k-ft TLw(plf) 661.25 0 0 0</td></l></l>	2 60 40	Dead Floor Live Roof Live/Snow	1.00 1.00 1.00	roof floor wall misc.	15	LL(psf) 100	ading trib (ft) 5.75 Total	DLw(plf) 86.25 0 0 0	17.6 LLw(plf) 575 0 0 0 0	k-ft TLw(plf) 661.25 0 0 0
Span(ft)= <u>12</u> LL Deflection <l <u="">36 Datal Deflection<l <u="">24 CD= <u>1</u></l></l>	2 60 40	Dead Floor Live Roof Live/Snow Misc.	1.00 1.00 1.00 1.00	roof floor wall misc.		LL(psf) 100	ading trib (ft) 5.75	DLw(plf) 86.25 0 0 0 86.25	17.6 LLw(plf) 575 0 0 0 0 575	k-ft TLw(plf) 661.25 0 0 0
Span(ft)= 12 LL Deflection <l 36<br="">otal Deflection<l 24<br="">CD= 1</l></l>	2 60 40 60 60 60 60 60 60 60 60 60 60 60 60 60	Dead Floor Live Roof Live/Snow Misc.	1.00 1.00 1.00 1.00	roof floor wall misc.	15 Factors Ap DL	LL(psf) 100 plied LL	ading trib (ft) 5.75 Total Reactions	DLw(plf) 86.25 0 0 86.25	17.6 LLw(plf) 575 0 0 0 575 Left (lbs)	k-ft TLw(plf) 661.25 0 0 661.23 Right (lbs)
Span(ft)= 12 LL Deflection <l 36<br="">otal Deflection<l 24<br="">CD= 1</l></l>	2 60 40 60 60 60 60 60 60 60 60 60 60 60 60 60	Dead Floor Live Roof Live/Snow Misc.	1.00 1.00 1.00 1.00	roof floor wall misc.	15 Factors Ap DL 0	LL(psf) 100 pplied LL 0	ading trib (ft) 5.75 Total	DLw(plf) 86.25 0 0 86.25 86.25 RDL=	17.6 LLw(plf) 575 0 0 0 575 0 2 575 Left (lbs) 518	k-ft TLw(plf) 661.25 0 0 661.2 661.2 Right (lbs)
Span(ft)= 12 LL Deflection <l 36<br="">otal Deflection<l 24<br="">CD= 1</l></l>	2 60 40 60 60 60 60 60 60 60 60 60 60 60 60 60	Dead Floor Live Roof Live/Snow Misc.	1.00 1.00 1.00 1.00	roof floor wall misc.	15 Factors Ap DL	LL(psf) 100 •plied LL 0 0	ading trib (ft) 5,75 Total Reactions Uniform	DLw(plf) 86.25 0 0 86.25 86.25 RDL= RLL=	17.6 LLw(plf) 575 0 0 0 575 Left (lbs) 518 3450	k-ft TLw(plf) 661.25 0 0 661.2 661.2 Right (lbs) 518 3450
Span(ft)= 12 LL Deflection <l 36<br="">otal Deflection<l 24<br="">CD= 1</l></l>	2 60 60 40 boad [boof boor	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00 1.00	roof floor wall misc. Load x(ft)(left)	15 Factors Ap DL 0	LL(psf) 100 •plied LL 0 0	ading trib (ft) 5.75 Total Reactions	DLw(plf) 86.25 0 0 86.25 86.25 RDL= RLL= P1DL=	17.6 LLw(plf) 575 0 0 0 0 575 0 0 575 575 518 3450 0	k-ft TLw(plf) 661.25 0 0 661.23 661.23 Right (lbs) 518 3450 0 0
Span(ft)= 12 LL Deflection <l 36<br="">otal Deflection<l 24<br="">CD= 1</l></l>	2 60 40 60 60 60 60 60 60 60 60 60 60 60 60 60	Dead Floor Live Roof Live/Snow Misc.	1.00 1.00 1.00 1.00	roof floor wall misc.	15 Factors Ap DL 0	LL(psf) 100 •plied LL 0 0	ading trib (ft) 5,75 Total Reactions Uniform	DLw(plf) 86.25 0 0 86.25 86.25 RDL= RLL= P1DL= P1DL= P1LL=	17.6 LLw(plf) 575 0 0 0 0 575 Left (lbs) 518 3450 0 0 0 0 0 0 0 0 0 0 0 0 0	k-ft TLw(plf) 661.25 0 0 661.23 661.23 0 0 818 3450 0 0 0 0 0 0 0 0 0 0 0 0 0
Span(ft)= 12 LL Deflection <l 36<br="">otal Deflection<l 24<br="">CD= 1</l></l>	2 60 60 40 boad [boof boor	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00 1.00	roof floor wall misc. Load x(ft)(left)	15 Factors Ap DL 0	LL(psf) 100 •plied LL 0 0	ading trib (ft) 5,75 Total Reactions Uniform	DLw(plf) 86.25 0 0 86.25 RDL= RLL= P1DL= P1DL= P1LL= P2DL=	17.6 LLw(plf) 575 0 0 0 0 575 575 Left (lbs) 518 3450 0 0 0 0 0 0 0 0 0 0 0 0 0	k-ft TLw(plf) 661.25 0 0 0 661.23 0 0 8 3450 0 0 0 0 0 0 0 0 0 0 0 0 0
Span(ft)= 12 LL Deflection <l 36<br="">otal Deflection<l 22<br="">CD= 1 Lo Point Load 1 ro Point Load 2 ftc</l></l>	2 60 40 for the second	Dead Floor Live Roof Live/Snow Misc. DL(lbs) TIMBER	1.00 1.00 1.00 1.00 1.00	roof floor wall misc. x(ft)(left) GLB	15 Factors Ap DL 0	LL(psf) 100 •plied LL 0 0	ading trib (ft) 5,75 Total Reactions Uniform	DLw(plf) 86.25 0 0 86.25 RDL= RLL= P1DL= P1DL= P1LL= P2DL= P2L=	17.6 LLw(plf) 575 0 0 0 0 575 575 Left (lbs) 518 3450 0 0 0 0 0 0 0 0 0 0 0 0 0	k-ft TLw(plf) 661.25 0 0 0 661.25 0 0 518 3450 0 0 0 0 0 0 0 0 0 0 0 0 0
Span(ft)= 12 LL Deflection <l 36<br="">otal Deflection<l 24<br="">CD= 1</l></l>	2 60 60 40 bad I boor DIM	Dead Floor Live Roof Live/Snow Misc. DL(lbs)	1.00 1.00 1.00 1.00 1.00	roof floor wall misc. Load x(ft)(left)	15 Factors Ap DL 0	LL(psf) 100 •plied LL 0 0	ading trib (ft) 5.75 Total Reactions Uniform Point Loads	DLw(plf) 86.25 0 0 86.25 RDL= RLL= P1DL= P1DL= P1LL= P2DL=	17.6 LLw(plf) 575 0 0 0 0 575 575 Left (lbs) 518 3450 0 0 0 0 0 0 0 0 0 0 0 0 0	k-ft TLw(plf) 661.25 0 0 0 661.25 0 0 661.25 0 0 0 0 0 0 0 0 0 0 0 0 0

6.75x9

8.75x9

10.75x12

-

(4) 1.75x9.5

-

-

Moment	Center Point Load Point Load 2 RDL= 1553 0 0 RLL= 10350 0 0 P1DL= 0 0 0 P1LL= 0 0 0 P2DL= 0 0 0						
1 - U 250	Center	Point Load	Point Load 2				
RDL=	1553	0	0				
RLL=	10350	0	0				
P1DL=	0	0	0				
P1LL=	0	0	0				
P2DL=	0	0	0				
P2LL=	0	0	0				
MTL=	11903	0	0				
	Max M=	11.9	k-ft				

Option 3 No Triple Ply 8x14 (3) 1.75×9.5

10x12

14x14

16x16

Option 4

Option 5

Option 6

Option 7

No Four Ply

-

TSE EngineersJOB TITLEWooden Spur Renovation136 S State Street368 Wooden Spur Dr. Alpine, WyomingShelley, ID 83274JOB NO.225112208-357-2420CALCULATED BYEVBDATECHECKED BY0DATE

Mark:	RFT1				NDS 2018				DIM	ILT	BC
Span(ft)=		C _D =	1	0.00							
Span(It)= Spacing (in)=			1.15								
Spacing (in)= LL Deflection <l <="" th=""><th></th><th>C_=</th><th>1.15</th><th></th><th></th><th></th><th></th><th>Ward Caralian</th><th>Douglas Fir #1</th><th></th><th></th></l>		C_=	1.15					Ward Caralian	Douglas Fir #1		
and the second se	A REAL PROPERTY AND A REAL							Wood Species= Single Ply		9 1/2" 110	91/2"5
Total Deflection <l <="" td=""><td>240</td><td></td><td>Loading</td><td></td><td></td><td></td><td></td><td>Double Ply</td><td>(1) 2x8 (2) 2x6</td><td>9 1/2" 210</td><td>91/2 9</td></l>	240		Loading					Double Ply	(1) 2x8 (2) 2x6	9 1/2" 210	91/2 9
	DL (psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)	_	4x	4x6	9 1/2" 230	91/2"6
roof	and the second se	100	1.33	20.0	133.3	153.3	alendara -	6x	6x6	117/8" 360	117/8
floor		100	1.33	0.0	0.0	0.0		8x	8x8	117/8" 560	117/8
misc.			1.33	0.0	0.0	0.0		10x	10×10		
	1 Mar 1 - A 2 - A		Total		133.3		153.3	12x	12x12		
								14x	14×14		
			Reactions		N	loment		16x	16x16		
			Left (lbs)	Right (lbs)				1. h añ	Intervention and a second of the second of t		
		RDL=	85	85	RDL=	181					
		RLL=	567	567	RLL=	1204					
		RTL=	652	652	MTL=	1385					
		Max Shear (k)=	0.7		Max M (k-ft)=	1.4					
Mark:	DET 2								DIM	ILT ·	BC
								A second states			
Span(ft)=		C _D =	Long the second second second second								
Spacing (in)=	16	C _r =	1.15								
LL Deflection <l <="" td=""><td>360</td><td></td><td></td><td></td><td></td><td></td><td></td><td>Wood Species=</td><td>Douglas Fir #1</td><td>-</td><td>-</td></l>	360							Wood Species=	Douglas Fir #1	-	-
Total Deflection <l <="" td=""><td>240</td><td></td><td></td><td></td><td></td><td></td><td></td><td>Single Ply</td><td>(1) 2x12</td><td>117/8" 110</td><td>11 7/8"</td></l>	240							Single Ply	(1) 2x12	117/8" 110	11 7/8"
	Property of the second s		Loading					Double Ply	(2) 2x8	9 1/2" 210	9 1/2" 6
1000	DL (psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)		4x	4x8	9 1/2" 230	9 1/2" 6
roof	15	100	1.33	20.0	133.3	153.3	CARGE A	6x	6x8	11 7/8" 360	117/8"
floor		100	1.33	0.0	0.0	0.0		8x	8x8	11 7/8" 560	117/8"
misc.		60	1.33	0.0	80.0	80.0		10x	10x10	1176 300	11770
Tillac.					0.0 213.3	0010	233.3		12x12		
			Total	2	0.0 215.5		233.5	12x 14x	12x12 14x14		
			Reactions		N	loment		14x 16x	16x16		
			Left (lbs)	Right (lbs)		ioment		ION	10/10		
		RDL=	100	100	RDL=	250					
		RLL=	1067	1067	RLL=	2667					
		RTL=	1167	1167	MTL=	2917	10000				
		Max Shear (k)=	1.2		Max M (k-ft)=	2.9					
	FID 4								DIM	711	DC
Mark:				Distance in the second				Charles States	DIM	TJI	BC
Span(ft)=		C _D =									
Spacing (in)=	16	C _r =	1.15								
LL Deflection <l <="" td=""><td>480</td><td></td><td></td><td></td><td></td><td></td><td></td><td>Wood Species=</td><td>Douglas Fir #2</td><td>-</td><td>-</td></l>	480							Wood Species=	Douglas Fir #2	-	-
Total Deflection <l <="" td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>Single Ply</td><td>(1) 2x8</td><td>9 1/2" 110</td><td>9 1/2" 5</td></l>								Single Ply	(1) 2x8	9 1/2" 110	9 1/2" 5
		$T = c_1 f_2 \dots f_n$	Loading	and the star	a fail a line a	S. Anna S.	ALC: N	Double Ply	(2) 2x6	9 1/2" 210	9 1/2" 6
	DL (psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)	() () () () () () () () () ()	4x	4x6	9 1/2" 230	9 1/2" 6
roof			1.33	0.0	, 0.0	0.0		6х	6х6	11 7/8" 360	117/8"
floor	10	40	1.33	13.3	53.3	66.7		8x	8x8	117/8"560	117/8"
misc.			1.33	0.0	0.0	0.0	SISTER.	10x	10x10		
			Total	1:	3.3 53.3		66.7	12x	12x12		
					1			14x	14x14		
			Reactions	in the second second	M	oment	No. 10	16x	16x16		
			Left (lbs)	Right (lbs)							
		RDL=	67	67	RDL=	167					
		RLL=	267	267	RLL= MTL=	667 833					

		TSE Engir 136 S State S Shelley, ID 8 208-716-24	Street 33274			JOB TITLE 368 Wooden Spur I JOB NO. CALCULATED BY	225112		SHEET NO	0. 75 of 6/4/2025
		200 710 2	120			CHECKED BY			DATE	0/4/2023
			Wood Pop	ım Design (Late	ral Cuppor	tod Fully)				
			wood Bea	NDS 20:		tea Fully)				
Mark:	BM21	Load	Factors			Lo	ading			
Span(ft)=	18	Dead	1.00		DL (psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)
LL Deflection <l <="" td=""><td></td><td>Floor Live</td><td></td><td>200.00</td><td>-</td><td>100</td><td>7</td><td>105</td><td>700</td><td>805</td></l>		Floor Live		200.00	-	100	7	105	700	805
otal Deflection <l <br="">CD=</l>	and the set of the set of the set of the	Roof Live/Snow Misc.	Provide and the second s		10	40	5	50 0	0 0	50 0
CD=	1	Iviise.	1.00	misc.	1	60	5	0	300	300
							Total	155		
							1			
	Load	DL(lbs)	LL(lbs)	Load x(ft)(left)	Factors A	pplied LL	Reactions	1	Left (lbs)	Right (lbs)
Point Load 1		DL(IDS)			0	0	Uniform	RDL=	1395	1395
Point Load 1 Point Load 2					0	0	onnorm	RLL=	9000	9000
							Point Loads	P1DL=	0	0
	DIM	TIMBER	LVL	GLB				P1LL=	0	0
								P2DL=	0	0
		1 22					C. Let Let	P2LL=	0	0
Wood Species=		Douglas Fir #2	LVL	24F-V4				RTL=	10395	10395
Option 1	No Single Ply	4x None	No Single Ply	3.125 x None			N	/lax Shear=	10.4	k
	No Double Ply	6x None	No Double Ply	5.125x18						
	No Triple Ply	8x None	(3) 1.75x18	6.75x16.5			Moment			
Option 4	No Four Ply	10x None	(4) 1.75x16	8.75x15				Center		Point Load
Option 5	-	12x None	-	10.75x13.5			RDL=	6278	0	0
Option 6 Option 7	-	14x18 16x18	-	-	6		RLL= P1DL=	40500 0	0 0	0 0
Option /		10/10					P1LL=	0	0	0
							P2DL=	0	0	0
							P2LL=	0	0	0
							MTL=	46778 Max M=	0 46.8	0 k-ft
									40.0	K-IL
								252.5		
	BM 21		Factors				ading			
Span(ft)=		Dead Floor Live	1.00		DL (psf) 15	LL(psf) 100			LLw(plf)	TLw(plf)
LL Deflection <l <="" td=""><td></td><td>Roof Live/Snow</td><td>0.75</td><td>roof floor</td><td>10</td><td>40</td><td>7</td><td>105 50</td><td>525 150</td><td>630 200</td></l>		Roof Live/Snow	0.75	roof floor	10	40	7	105 50	525 150	630 200
CD=		Misc.	0.75	wall				0	0	0
				misc.		60	5	0	225	225
							Total	155	900	10
				Load	Factors A	pplied	Reactions			
	Load	DL(lbs)	LL(lbs)	x(ft)(left)	DL	LL	3 8 4 20	J-1255	Left (lbs)	Right (Ibs)
Point Load 1	roof				0	0	Uniform	RDL=	1395	1395
Point Load 2	floor				0	0		RLL=	8100	8100
							Point Loads	P1DL=	0	x 0
r	DIM	TIMBER	LVL	GLB				P1LL=	0	0
			19 19 C	6				P2DL=	0	0
	NA WALLSON DO	1 B						P2LL=	0	0
-				Charles and the second s				DTI	0.405	
Wood Species=	NATURAL CONTRACTOR OF CONTRACTOR	Douglas Fir #2	LVL	24F-V4			- 1900 - T	RTL=	9495	9495
Option 1	No Single Ply	4x None	No Single Ply	3.125x21			Ν	ax Shear=	9495 9.5	9495 k
Option 1 Option 2	No Single Ply No Double Ply	4x None 6x None	No Single Ply No Double Ply	3.125x21 5.125x18						
Option 1	No Single Ply	4x None	No Single Ply	3.125x21			Moment	lax Shear=	9.5	

10.75x13.5

-

-

12x None

14x18

16x18

Option 5

Option 6

Option 7

-

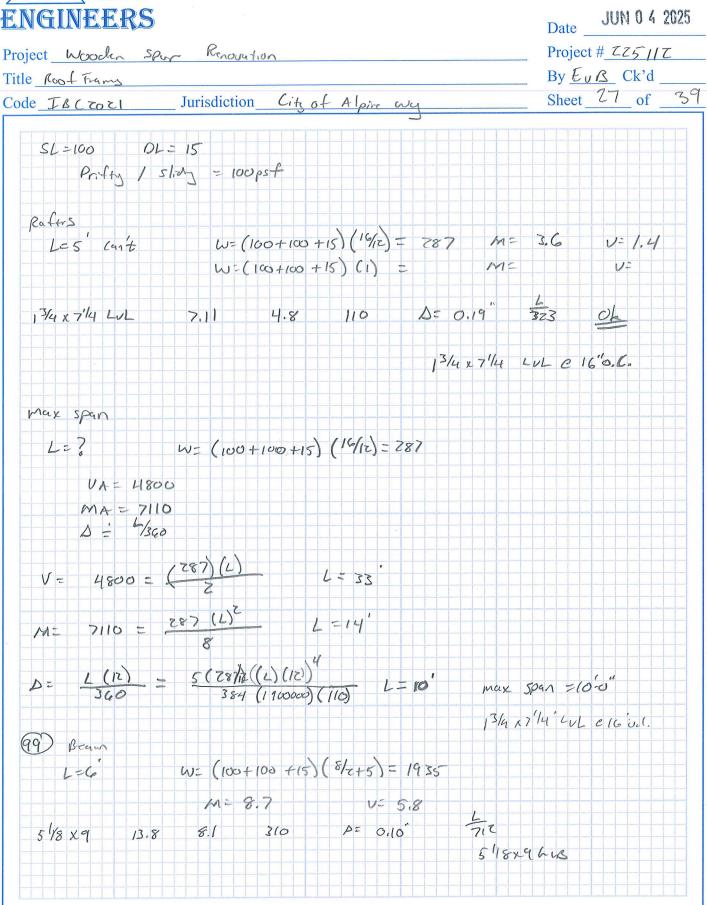
Moment			
, S. 2	Center	Point Load	Point Load 2
RDL=	6278	0	0
RLL=	36450	0	0
P1DL=	0	.0	0
P1LL=	0	0	0
P2DL=	0	0	0
P2LL=	0	0	0
MTL=	42728	0	0
	Max M=	42.7	k-ft

		208-716-24	3274 126			JOB NO. CALCULATED BY CHECKED BY	225112 EVB		DATE DATE DATE	0. Zle 03 2 6/4/2025
			Wood Bea	m Design (Late		ted Fully)			-	
	DI 422			NDS 202	18					
Mark:	and the second se		Factors				ading			
Span(ft)=		Dead	Contraction of the second second	-	DL (psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)
LL Deflection <l <="" td=""><td></td><td>Floor Live</td><td></td><td>roof</td><td></td><td>100</td><td>4</td><td>60</td><td>400</td><td>460</td></l>		Floor Live		roof		100	4	60	400	460
otal Deflection <l <="" td=""><td></td><td>Roof Live/Snow</td><td></td><td>floor</td><td></td><td>40</td><td>1</td><td>10</td><td>0</td><td>10</td></l>		Roof Live/Snow		floor		40	1	10	0	10
CD=	1	Misc.	1.00	wall	Enternances Extension	60	4	0	0 240	0 240
				misc.	E BRANDERS	60	4 Total	70		
				103	d Factors A	nnlied	Reactions	10		, , , , , , , , , , , , , , , , , , , ,
1	Load	DL(lbs)	LL(lbs)	x(ft)(left)	DL	LL	Redectoris		Left (lbs)	Right (lbs)
Point Load 1					0	0	Uniform	RDL=	350	350
Point Load 1				<u></u>	0	0	onnorm	RLL=		3200
Tome Loud 2			Language and constrained of the				Point Loads			0
	DIM	TIMBER	LVL	GLB				P1LL=		0
		THUBER		GLD				P2DL=		0
-								P2DL= P2LL=	0	0
Wood Species=	Douglas Fir #2	Douglas Fir #2	LVL	24F-V4			1	RTL=	3550	3550
	No Single Ply	4x None	(1) 1.75x11.875	3.125×10.5			1	Max Shear=		k
	No Double Ply	6x14	(2) 1.75x9.5	5.125x9						
	No Triple Ply	8x12	(3) 1.75x9.5	6.75x9			Moment			
Option 4	(4) 2x12	10x10	(4) 1.75x9.5	8.75x9				Center	Point Load	Point Load 2
Option 5		12x12	(1)	10.75x12			RDL=	875	0	0
Option 6	_	14x14	_	-			RLL=	8000	0	0
Option 7	-	16x16	-				P1DL=	0	0	0
option /		TONIO	L		1. 1. 1.		P1LL=	0	0	0
							P2DL=	0	0	0
							P2LL=	0	0	0
							MTL=	8875	0	0
								Max M=	8.9	k-ft

Mark: BM 22	Load Factors					Loading			10.321
Span(ft)= 10	Dead	1.00		DL (psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)
LL Deflection <l 360<="" td=""><td>Floor Live</td><td>0.75</td><td>roof</td><td>15</td><td>100</td><td>4</td><td>60</td><td>300</td><td>360</td></l>	Floor Live	0.75	roof	15	100	4	60	300	360
Total Deflection <l 240<="" td=""><td>Roof Live/Snow</td><td>0.75</td><td>floor</td><td>10</td><td>40</td><td>1</td><td>10</td><td>3,0</td><td>40</td></l>	Roof Live/Snow	0.75	floor	10	40	1	10	3,0	40
CD= 1	Misc.	0.75	wall				0	0	0
			misc.		60	4	0	180	180
						Total	7	0 51	0 58

				Load	Factors Appli	ed	Reactions	1.1.2.2	Post first	1.5. 0. 1
	Load	DL(lbs)	LL(lbs)	x(ft)(left)	DL	LL		200	Left (Ibs)	Right (lbs)
Point Load 1	roof				0	0	Uniform	RDL=	350 ′	350
Point Load 2	floor				0	0		RLL=	2550	2550
							Point Loads	P1DL=	0	0
	DIM	TIMBER	LVL	GLB				P1LL=	0	0
								P2DL=	0	0
								P2LL=	0	0
Vood Species=	Douglas Fir #2	Douglas Fir #2	LVL	24F-V4				RTL=	2900	2900
Option 1	No Single Ply	4x None	(1) 1.75x11.875	3.125x10.5			N	/lax Shear=	2.9	k
Option 2	No Double Ply	6x12	(2) 1.75x9.5	5.125x9						
Option 3	No Triple Ply	8x10	(3) 1.75x9.5	6.75x9			Moment		1	
Option 4	(4) 2x12	10x10	(4) 1.75x7.25	8.75x9			- 16.13	Center	Point Load	Point Load
Option 5	100000-00000	12x12	-	10.75x12			RDL=	875	0	0
Option 6	-	14x14	-				RLL=	6375	0	0
Option 7	-	16x16	-	-			P1DL=	0	0	0
		Landan da yang mangan kangan kangan da ka		Remotion contraction of the cont			P1LL=	0	0	0
							P2DL=	0	0	0
							P2LL=	0	0	0
							MTL=	7250	0	0
								Max M=	7.3	k-ft





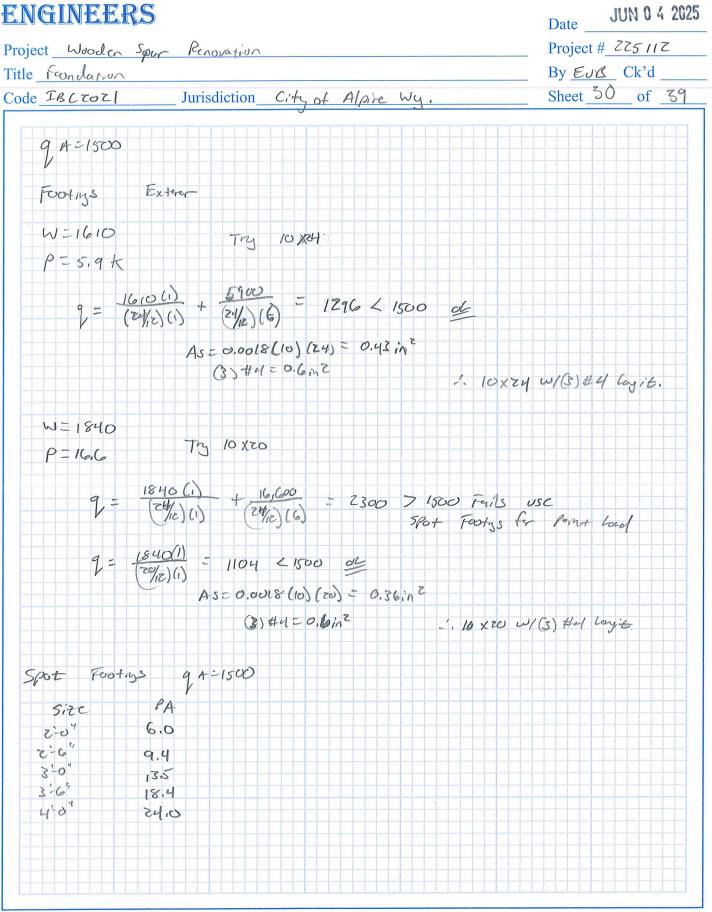




TSE Engineers	JOB TITLE	Wooden Spur Renovation				
136 S State Street	368 Wooden Spur Dr. Alpine, Wyoming					
Shelley, ID 83274	JOB NO.	225112	SHEET NO.	2906 39		
208-357-2420	CALCULATED BY	EVB	DATE	6/4/2025		
	CHECKED BY	0	DATE			

			nood	Joist/Rafters (simp	e span)				
Mark: FLR 2				NDS 2018			DIM	TJI	BC
BANDESCORPORT OF CONTRACTOR OF CONTRACTOR							DIN	131	
Span(ft)= 14	C_=	1.15							
Spacing (in)= 12 LL Deflection <l 360<="" th=""><th>ـــــــــــــــــــــــــــــــــــــ</th><th>1.15</th><th>560</th><th></th><th></th><th>Wood Species=</th><th>Douglas Fir S.S.</th><th></th><th></th></l>	ـــــــــــــــــــــــــــــــــــــ	1.15	560			Wood Species=	Douglas Fir S.S.		
Total Deflection <l 240<="" th=""><th></th><th></th><th></th><th></th><th></th><th>Single Ply</th><th>(1) 2x10</th><th>9 1/2" 110</th><th>91/2"</th></l>						Single Ply	(1) 2x10	9 1/2" 110	91/2"
	-	Loading				Double Ply	(2) 2x8	9 1/2" 210	91/2"
DL (psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)	4x	4x8	9 1/2" 230	91/2"
roof 10	60	1.00	10.0	60.0	70.0	6x	6x6	117/8"360	117/8
floor		1.00	0.0	0.0	0.0	8x	8x8	117/8" 560	117/8
misc.		1.00	0.0	0.0	0.0	10x	10x10		
		Total	1	.0.0 60.0	70,0	12x	12x12		
	Ш					14x	14x14		
	L	Reactions		r	Noment	16x	16x16		
	RDL=	Left (lbs)	Right (lbs) 70	RDL=	245				
	RLL=	70 420	420	RLL=					
	RTL=	490	490	MTL=	and the second				
	Max Shear (k)=	0.5		Max M (k-ft)=	1.7				
Mark:							DIM	ILT	вс
	C _D =	4					Dim	131	
Span(ft)=	02310	Contraction in the second second							
Spacing (in)= 16	C_r=	1.15							
LL Deflection <l 360<="" td=""><td></td><td></td><td></td><td></td><td></td><td>Wood Species=</td><td>Douglas Fir #1</td><td></td><td></td></l>						Wood Species=	Douglas Fir #1		
Total Deflection <l 240<="" td=""><td></td><td></td><td></td><td></td><td></td><td>Single Ply</td><td>(1) 2x4</td><td>9 1/2" 110</td><td>9 1/2"</td></l>						Single Ply	(1) 2x4	9 1/2" 110	9 1/2"
		Loading	riallie and the	and the second second	and a state of the	Double Ply	(2) 2x4	9 1/2" 210	9 1/2" (
DL (psf)	LL(psf)	trib (ft)	DLw(plf)	LLw(plf)	TLw(plf)	4x	4x4	9 1/2" 230	9 1/2"
roof 15	100	1.33	20.0	133.3	153.3	6х	6x6	11 7/8" 360	117/8
floor		1.33	0.0	0.0	0.0	8x	8x8	11 7/8" 560	117/8
	60	1.33	0.0	80.0	80.0	10x	10x10		
misc.	00								
misc.		Total	2	0.0 213.3	233.3	12x	12x12		
misc.			2			14x	14x14		
misc.		Reactions			233.3 1oment				
misc		Reactions Left (lbs)	Right (lbs)	N	1oment	14x	14x14		
misc	LRDL=	Reactions Left (Ibs) 0	Right (lbs) 0		1oment 0	14x	14x14		
misc	RDL= RLL=	Reactions Left (lbs) 0 0	Right (Ibs) 0 0		10ment 0 0	14x	14x14		
misc	LRDL=	Reactions Left (Ibs) 0	Right (lbs) 0		1oment 0	14x	14x14		
	RDL= 	Reactions Left (lbs) 0 0 0	Right (Ibs) 0 0		10ment 0 0 0	14x	14x14 16x16	TII	BC
Mark:	RDL= RLL= RTL= Max Shear (k)=	Reactions 0 0 0 0 0 0 0	Right (Ibs) 0 0		10ment 0 0 0	14x	14x14	ILT	BC
Mark: Span(ft)=	RDL= RLL= RTL= Max Shear (k)= C _D =	Reactions Left (lbs) 0 0 0 0.0	Right (Ibs) 0 0		10ment 0 0 0	14x	14x14 16x16	ונד	BC
Mark: Span(ft)= Spacing (in)=	RDL= RLL= RTL= Max Shear (k)=	Reactions Left (lbs) 0 0 0 0.0	Right (Ibs) 0 0		10ment 0 0 0	14x 16x	14x14 16x16 DIM	<u></u>	BC
Mark: Span(ft)= Spacing (in)= LL Deflection <l 480<="" td=""><td>RDL= RLL= RTL= Max Shear (k)= C_D=</td><td>Reactions Left (lbs) 0 0 0 0.0</td><td>Right (Ibs) 0 0</td><td></td><td>10ment 0 0 0</td><td>14x 16x</td><td>14x14 16x16 DIM Douglas Fir #2</td><td>÷</td><td>-</td></l>	RDL= RLL= RTL= Max Shear (k)= C _D =	Reactions Left (lbs) 0 0 0 0.0	Right (Ibs) 0 0		10ment 0 0 0	14x 16x	14x14 16x16 DIM Douglas Fir #2	÷	-
Mark: Span(ft)= Spacing (in)=	RDL= RLL= RTL= Max Shear (k)= C _D =	Reactions Left (lbs) 0 0 0 0 0 0 0.0	Right (Ibs) 0 0		10ment 0 0 0	14x 16x Wood Species= Single Ply	14x14 16x16 DIM Douglas Fir #2 (1) 2x4	- 9 1/2" 110	- 9 1/2" 5
Mark: Span(ft)= Spacing (in)= LL Deflection <l 480<br="">Fotal Deflection<l 240<="" td=""><td>RDL= RLL= RTL= Max Shear (k)= C_D= C_r=</td><td>Reactions Left (Ibs) 0 0 0 0.0 1 1 1.15 Loading</td><td>Right (lbs) 0 0</td><td>RDL= RLL= MTL= Max M (k-ft)=</td><td>10ment 0 0 0,0</td><td>14x 16x Wood Species= Single Ply Double Ply</td><td>14x14 16x16 DIM Douglas Fir #2 (1) 2x4 (2) 2x4</td><td>9 1/2" 110 9 1/2" 210</td><td>- 9 1/2" 5 9 1/2" 6</td></l></l>	RDL= RLL= RTL= Max Shear (k)= C _D = C _r =	Reactions Left (Ibs) 0 0 0 0.0 1 1 1.15 Loading	Right (lbs) 0 0	RDL= RLL= MTL= Max M (k-ft)=	10ment 0 0 0,0	14x 16x Wood Species= Single Ply Double Ply	14x14 16x16 DIM Douglas Fir #2 (1) 2x4 (2) 2x4	9 1/2" 110 9 1/2" 210	- 9 1/2" 5 9 1/2" 6
Mark: Span(ft)= Spacing (in)= LL Deflection <l 480<br="">Fotal Deflection<l 240<br="">DL (psf)</l></l>	RDL= RLL= RTL= Max Shear (k)= C _D = C _r =	Reactions Left (lbs) 0 0 0 0 0 0 0 1 1.15 Loading Loading trib (ft)	Right (lbs) 0 0	RDL= RLT= MTL= Max M (k-ft)=	10ment 0 0 0.0	14x 16x Wood Species= Single Ply Double Ply 4x	14x14 16x16 Dim Douglas Fir #2 (1) 2x4 (2) 2x4 4x4	9 1/2" 110 9 1/2" 210 9 1/2" 230	9 1/2" 9 9 1/2" 6 9 1/2" 6
Mark: Span(ft)= Spacing (in)= LL Deflection <l 480<br="">Fotal Deflection<l 240<br="">DL (psf) roof</l></l>	RDL= RLL= RTL= Max Shear (k)= C ₀ = C ₇ =	Reactions Left (lbs) 0 0 0 0 0 0 0.0	Right (lbs) 0 0	RDL= RLT= MTL= Max M (k-ft)= LLw(plf) 0.0	10ment 0 0 0.0 0.0 TLw(pif) 0.0	14x 16x Wood Species= Single Ply Double Ply 4x 6x	14x14 16x16 Douglas Fir #2 (1) 2x4 (2) 2x4 4x4 6x6	9 1/2" 110 9 1/2" 210 9 1/2" 230 11 7/8" 360	9 1/2" 5 9 1/2" 6 9 1/2" 6 11 7/8'
Mark: Span(ft)= Spacing (in)= LL Deflection <l <br="">Fotal Deflection<l <br="">Z40 DL (psf) roof floor 10</l></l>	RDL= RLL= RTL= Max Shear (k)= C _D = C _r =	Reactions Left (lbs) 0 0 0 0 0 0 0 1 1.15 Loading Loading trib (ft)	Right (lbs) 0 0 0 0 0 0 0 0 0 0 0 0.0 13.3	RDL= RLL= MTL= Max M (k-ft)= LLw(plf) 0.0 53.3	10ment 0 0 0.0	14x 16x Wood Species= Single Ply Double Ply 4x	14x14 16x16 Dim Douglas Fir #2 (1) 2x4 (2) 2x4 4x4	9 1/2" 110 9 1/2" 210 9 1/2" 230	9 1/2" 5 9 1/2" 6 9 1/2" 6 11 7/8'
Mark: Span(ft)= Spacing (in)= LL Deflection <l 480<br="">Fotal Deflection<l 240<br="">DL (psf) roof</l></l>	RDL= RLL= R1L= RTL= Max Shear (k)= C ₀ = C ₁ = LL(psf)	Reactions 0 0 0 0 0 0.0 1 1.15 Loading trib (ft) 1.33 1.33	Right (lbs) 0 0 0 0 0 0 0 0 0 0 0 0 13.3 0.0	RDL= RLT= MTL= Max M (k-ft)= LLw(plf) 0.0	10ment 0 0 0.0 0.0 10.0 66.7	14x 16x Wood Species= Single Ply Double Ply 4x 6x 8x	14x14 16x16 DIM Douglas Fir #2 (1) 2x4 (2) 2x4 (2) 2x4 4x4 6x6 6x6 8x8	9 1/2" 110 9 1/2" 210 9 1/2" 230 11 7/8" 360	9 1/2" 5 9 1/2" 6 9 1/2" 6 11 7/8'
Mark: Span(ft)= Spacing (in)= LL Deflection <l <br="">Fotal Deflection<l <br="">Z40 DL (psf) roof floor 10</l></l>	RDL= RLL= R1L= RTL= Max Shear (k)= C ₀ = C ₁ = LL(psf)	Reactions 0 0 0 0 0 0.0 1 1.15 Loading trib (ft) 1.33 1.33	Right (lbs) 0 0 0 0 0 0 0 0 0 0 0 0 13.3 0.0	RDL= RLT= MTL= Max M (k-ft)= LLw(plf) 0.0 53.3 0.0 3.3 53.3	10ment 0 0 0 0 0 0.0 5 0.0 66.7 0.0 66.7 0.0 66.7	14x 16x Wood Species= Single Ply Double Ply 4x 6x 8x 10x	14x14 16x16 DIM Douglas Fir #2 (1) 2x4 (2) 2x4 (2) 2x4 4x4 6x6 6x6 8x8 10x10	9 1/2" 110 9 1/2" 210 9 1/2" 230 11 7/8" 360	9 1/2" 5 9 1/2" 6 9 1/2" 6 11 7/8"
Mark: Span(ft)= Spacing (in)= LL Deflection <l <br="">Fotal Deflection<l <br="">Z40 DL (psf) roof floor 10</l></l>	RDL= RLL= RTL= Max Shear (k)= C ₀ = C ₇ = LL(psf)	Reactions Left (lbs) 0 1.15 Loading trib (ft) 1.33 1.33 1.33 1.33 1.33 Fotal Reactions	Right (lbs) 0.0 13.3 0.0 11	RDL= RLT= MTL= Max M (k-ft)= LLw(plf) 0.0 53.3 0.0 3.3 53.3	10ment 0 0 0.0 0.0 0.0 TLw(plf) 0.0 66.7 0.0	14x 16x Wood Species= Single Ply Double Ply 4x 6x 8x 10x 10x 12x	14x14 16x16 Douglas Fir #2 (1) 2x4 (2) 2x4 4x4 4x4 6x6 8x8 10x10 12x12	9 1/2" 110 9 1/2" 210 9 1/2" 230 11 7/8" 360	9 1/2" 5 9 1/2" 6 9 1/2" 6 11 7/8"
Mark: Span(ft)= Spacing (in)= LL Deflection <l <br="">Fotal Deflection<l <br="">Z40 DL (psf) roof floor 10</l></l>	RDL= RLL= RTL= Max Shear (k)= C _p = C _r = LL(psf) 40	Reactions Left (lbs) 0 1.15 Loading trib (ft) 1.33 1.33 1.33 Total Reactions .eft (lbs)	Right (lbs) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0.0 13.3 0.0 13 Right (lbs)	N RDL= RLT= Max M (k-ft)= LLw(plf) 0.0 53.3 0.0 3.3 53.3 M	10ment 0 0 0 0 0 0 0 0 0 0 0 6 6 7 0 0 6 6 7 0 0 6 6 7 0 0 0 0 0 0 0 0 0 0 0 0 0	14x 16x Wood Species= Single Ply Double Ply 4x 6x 8x 10x 12x 12x 14x	14x14 16x16 Douglas Fir #2 (1) 2x4 (2) 2x4 (2) 2x4 4x4 6x6 8x8 10x10 12x12 14x14	9 1/2" 110 9 1/2" 210 9 1/2" 230 11 7/8" 360	9 1/2" 5 9 1/2" 6 9 1/2" 6 11 7/8'
Mark: Span(ft)= Spacing (in)= LL Deflection <l <br="">Fotal Deflection<l <br="">Z40 DL (psf) roof floor 10</l></l>	RDL= RLL= RLL= RTL= Max Shear (k)= C ₀ = C _r = LL(psf) 40 RDL=	Reactions Left (lbs) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0.0 0 1.15 Loading trib (ft) 1.33 1.33 1.33 Total Reactions eft (lbs) 0	Right (lbs) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0.0 13.3 0.0 13.3 0.0 12 Right (lbs) 0	N RDL= RLT= Max M (k-ft)= LLw(plf) 0.0 53.3 0.0 3.3 53.3 N RDL=	Aoment 0 0 0 0 0 0 0 0 0 0 0 0 0	14x 16x Wood Species= Single Ply Double Ply 4x 6x 8x 10x 12x 12x 14x	14x14 16x16 Douglas Fir #2 (1) 2x4 (2) 2x4 (2) 2x4 4x4 6x6 8x8 10x10 12x12 14x14	9 1/2" 110 9 1/2" 210 9 1/2" 230 11 7/8" 360	9 1/2" 5 9 1/2" 6 9 1/2" 6 11 7/8'
Mark: Span(ft)= Spacing (in)= LL Deflection <l <br="">Fotal Deflection<l <br="">Z40 DL (psf) roof floor 10</l></l>	RDL= RLL= RTL= Max Shear (k)= C _p = C _r = LL(psf) 40	Reactions Left (lbs) 0 1.15 Loading trib (ft) 1.33 1.33 1.33 Total Reactions .eft (lbs)	Right (lbs) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0.0 13.3 0.0 13 Right (lbs)	N RDL= RLT= Max M (k-ft)= LLw(plf) 0.0 53.3 0.0 3.3 53.3 M	10ment 0 0 0 0 0 0 0 0 0 0 0 6 6 7 0 0 6 6 7 0 0 6 6 7 0 0 0 0 0 0 0 0 0 0 0 0 0	14x 16x Wood Species= Single Ply Double Ply 4x 6x 8x 10x 12x 12x 14x	14x14 16x16 Douglas Fir #2 (1) 2x4 (2) 2x4 (2) 2x4 4x4 6x6 8x8 10x10 12x12 14x14	9 1/2" 110 9 1/2" 210 9 1/2" 230 11 7/8" 360	BCI 9 1/2" 5 9 1/2" 6 9 1/2" 6 11 7/8" 11 7/8"

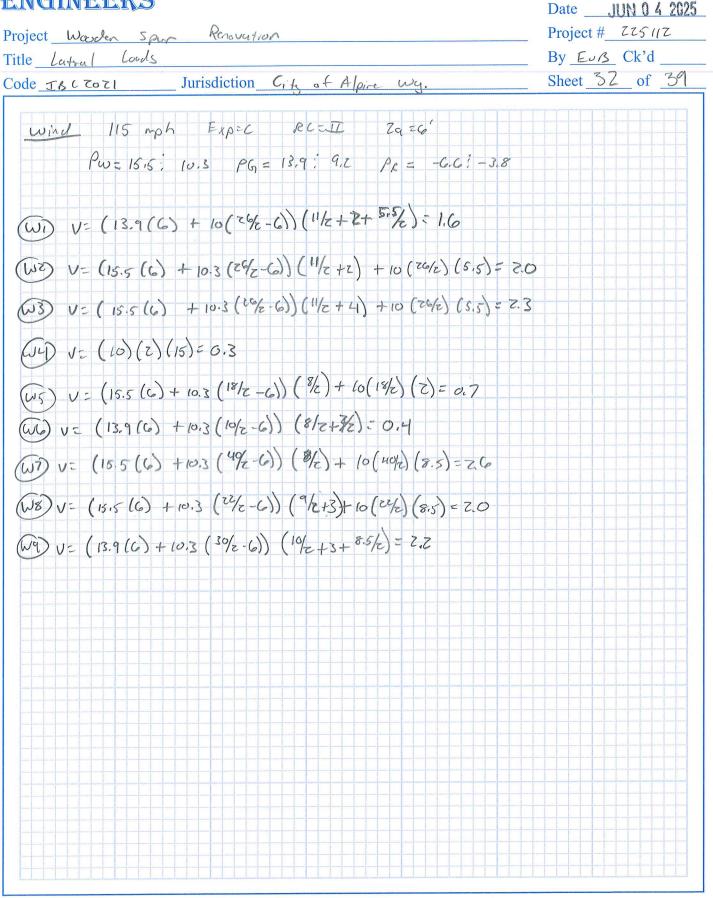






INGINEERS	DateJ	UN 0 4	2025
oject wooden Spor Penovation	Project #_	22511	2
tle Lateral Louds	By EVS	Ck'd	
ode traczozi Jurisdiction City of Alpre Wy.	Sheet 31	of	39
$\frac{Se_{1}Sm_{1}C}{Sd_{1}} = 0.887$ $\frac{Se_{1}Sm_{1}C}{V=0.156} = 0.0000000000000000000000000000000000$) s = 0,13		
Grange W= (29) (29) (15) + (29) (29) (100) (0.2) = 29 435 V=0.136 (29435) /1.4/2 = 1.4			
Courd Pario w= (15) (29) (15) + (15) (29) (100) (0,2) = 15225 V=0.136 (15225) 11.4 = 1.5 / 2= 0.8			
Son Room W= $(z_1)(1z)(1z)(1z) + (z_1)(1z)(1z)(0,z) = 8820$ V=0.136 (8820) /1.4 /2 = 0.4			
$E_{xisty} House$ $W= (40)(30)(15) + (40)(30)(100)(0.2) = 42000$ $V=0.136(42000)/1.4/2 = 2.0$ $E_{xisty} G_{ye}$ $W= (30)(22)(20) + (30)(22)(100)(0.2) = 26400$			
V=0.136 (Z6400) /1.4/Z=1.3			





JOB TITLE	Wooden Spur Renovation		
368 Wooden Spur D	r. Alpine, Wyoming		
JOB NO.	225112	SHEET NO.	3305 39
CALCULATED BY	EVB	DATE	6/4/2025
CHECKED BY	0	DATE	

Shear Line: SW1 Sesimic Load (Ib): 1400			w/ Wood Studs (Top Story) 21 (ASD Loading) s (ft) = 8.00					
S _{DS} 0.887		Segment 1	Segment 2	Segment 3	Segment 4	Segment 5	Segment 6	Segment
Wind Load (lb): 1600	length (ft)	4	4	and the search of	Hattor History			orginetic
	height (ft)	11	11					
Roof DL (psf) 15	roof trib. (ft)	16	16			1911 C 1917 1843		
Floor DL (psf) 10	floor trib. (ft)	0	0	2. Sector			A CONTRACTOR OF THE	
Wall Self (psf) 8	Distance from HD to end of wall (in)	3	3					
	Aspect Ratio:	2.75	2.75			The second s	ADDRESS CONTRACTORS AND ADDRESS OF ADDRESS ADDRESS ADDRESS ADDRESS ADD	APPART CONTINUE
	Aspect Ratio Factor (WSP):	0.91	0.91					
Seismic	shear flow (plf)	175.0	175.0					
	F (lbs)	700	700					
	Seismic Factor	0.73	0.73					
	Adjusted Shear Flow (plf)	240.63	240.63					
	MOT (ft-lb)	7700	7700					
	DL Factor A=	1.12	1.12					
	A x wDL (plf)	369	369					
	End Post Compression (Ib)	2175	2175					
	DL Factor B=	0.48	0.48	the second second second second second				
	B x wDL (plf)	156	156					
and the second	End Post Uplift for HDs (lb)	1035	1035					
				10 T T T T T T T T T T T T T T T T T T T	Ender the second second			
Wind	shear flow (plf)	200	200					1000
이 이번 것 같은 것 같은 것 같아요.	F (lb)	800	800					
	MOT (ft-lb)	8800	8800					
	wDL (plf)	328	328					
	End Post Compression (lb)	3003	3003					
	DL Factor	0.6	0.6			12 12 12 12 12 12 12 12 12 12 12 12 12 1		
	wDL (plf)	196.8	197					
	End Post Uplift for HDs (Ib)	2121	2121					
	Max Compression	3003	3003					
	Max Uplift	2121	2121					
	Max Shear Flow	241	241					
Max S	hear Floow Due to Wind or Seismic (W/S)	S	S					
	Sheathing Nails 7,	16 w/ 8d @ 4" O.C.	7/16 w/ 8d @ 4" O.C.	建制建筑和中国的 中国	·····································	Sector Restance	1999年1月1日日日日日	SAN TOPS
	# of Layers	1	1					
	Shear Flow Allowabel (plf)	317	317	1. 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	A			
	Check	Adequate	Adequate					
	Sheathing Staples 7	16 w/ 1 1/2" 16 ga. @ 3" O.C.	7/16 w/ 1 1/2" 16 ga. @ 3" O.C.		Same Real and the	ALL HOLD OF A		10.000
	Shar Flow Allowable (plf)	310	310			14/ 18 19 19 19 19		
	Check	Adequate	Adequate					
Recor	nmended Holdown @ Foundation 8" wall	HDU2	HDU2		a robalia - Casta		State In 1971 1987	Shi Carto
	Strap Style (Midwall)	LSTHD8	LSTHD8		a second second			
	Strap Style (End Wall)	LSTHD8	LSTHD8					
	Recommended Holdown @ Wood Wall	MSTC40	MSTC40					
	Strap Nailing Req'd	(32) 16d	(32) 16d					
Provide and the second s	# OF Minimum End Post	2	2	and the later of t			the first of the second se	

JOB TITLE	Wooden Spur Renovation		
368 Wooden Spur Dr	. Alpine, Wyoming		
JOB NO.	225112	SHEET NO.	340139
CALCULATED BY	EVB	DATE	6/4/2025
CHECKED BY	0	DATE	

Shear Line:	SW2 2900		Wood Shear Walls w/ NDS 2021 (/ Total Length of walls (ft	ASD Loading)					
Sos	0.887		Segment 1	Segment 2	Segment 3	Segment 4	Segment 5	Segment 6	Segment
Wind Load (lb):	1900	length (ft)	9		CONTRACTOR OF A	PERMIT			
., _		height (ft)	11	President and the second of the	ALC: NUMBER OF PARTY			No. No. Manager	
Roof DL (psf)	15	roof trib. (ft)	13	A PROPORTION OF THE PARTY		and includence and		and high second	INANO DI CREMA
Floor DL (psf)	10	floor trib. (ft)	0	The second second			Marine Statistics		AN AS BUT
Wall Self (psf)	8	Distance from HD to end of wall (in)	3	A CONTRACTOR					
		Aspect Ratio:	1.22						
s	eismic	shear flow (plf)	322.2						in the second
		F (lbs)	2900						
×		Seismic Factor	1.00						
		Adjusted Shear Flow (plf)	322.22		영화 전 소리 가슴 소리 위험			nd. a. Shi	M = 2N
		MOT (ft-lb)	31900						
1 A. 1 A. 1 A.		DL Factor A=	1.12						
		A x wDL (plf)	318						
1		End Post Compression (Ib)	3984				and the second		
		DL Factor B=	0.48						
s de la contra de		B x wDL (plf)	135						
L		End Post Uplift for HDs (lb)	1875						des and
Γ ν	Vind	shear flow (plf)	211						
1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		F (lb)	1900	Margaret A. P. Stratt	15 Martines	Salahan Mariaka			
10 1 at 1.00		MOT (ft-lb)	20900						
		wDL (plf)	283						
1		End Post Compression (Ib)	3662			1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.		Sec.	
		DL Factor	0.6						
h		wDL (plf)	169.8						
9 S. C	Part of the	End Post Uplift for HDs (lb)	1695				a State State State		
		Max Compression	3984						
		Max Uplift	1875						
		Max Shear Flow	322						
	Ma	x Shear Floow Due to Wind or Seismic (W/S)	S						1. St. 1.
		Sheathing Nails 7/16		The Constant States		88 8 S S S S S S S S S S S S S S S S S	and the second states	建立是的目光的 同时间的。	STATISTICS.
		# of Layers	1				service processes	Real Property and the	
		Shear Flow Allowabel (plf)	450	N					
	Annie Fritzen.	Check	Adequate				1		
		Sheathing Staples 7/16		10 各地址的复数形式的	引起的"新兴"的"新兴"(1)	· 通知: 通知: · · · · · · · · · · · · · · · · · · ·	Line of the second second	AND	新加加利用
		Shar Flow Allowable (plf)	395						
- 11 - <u>-</u>	Local Soft	Check	Adequate	08			- Contraction of the second		
	Red	commended Holdown @ Foundation 8" wall	HDU2						a distance
		Strap Style (Midwall)	LSTHD8						
1 <u></u>	-	Strap Style (End Wall)	LSTHD8						
		Recommended Holdown @ Wood Wall	MSTA49						
	Indiana and a second	Strap Nailing Req'd	(26) 10d						
		# OF Minimum End Post	2				and the second second second	and the second second	

JOB TITLE	Wooden Spur Renovation		
368 Wooden Spur Dr	. Alpine, Wyoming		
JOB NO.	225112	SHEET NO.	35 06 39
CALCULATED BY	EVB	DATE	6/4/2025
CHECKED BY	0	DATE	

				/ Wood Studs (Top Story) (ASD Loading)	-				
Shear Line:	SW3		Total Length of walls (f						
Sesimic Load (Ib):	2200		Total congen of Walls (
S _{DS}	0.887		Segment 1	Segment 2	Segment 3	Segment 4	Segment 5	Segment 6	Segment 7
Wind Load (Ib):	2000	length (ft)	20			The second second			
, ,		height (ft)	11	r in state state sade sa					
Roof DL (psf)	15	roof trib. (ft)	4	and the share the second second		The second second	And the Property of the	Contraction of the second	Sub-state average
Floor DL (psf)	10	floor trib. (ft)	0						
Wall Self (psf)	8	Distance from HD to end of wall (in)	3						Designed and
wan sen (psi)	0	Aspect Ratio:	0.55			Checker Constants and acceler			Salata and an and the state
Se	ismic	shear flow (plf)	110.0						
		F (lbs)	2200						
		Seismic Factor	1.00						
		Adjusted Shear Flow (plf)	110.00						
1 N N N N		MOT (ft-lb)	24200						
and the first of the		DL Factor A=	1.12						
		A x wDL (plf)	166						
		End Post Compression (lb)	2522						
	() 1 100 100 100 100 100 100 100 100 100	DL Factor B=	0.48						
		B x wDL (plf)	70						
1		End Post Uplift for HDs (lb)	143						
			143		and the second	And the second second			
Wi	ind	shear flow (plf)	100						
		F (Ib)	2000						
		MOT (ft-lb)	22000						1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
		wDL (plf)	148						
a sa 1936 ili		End Post Compression (lb)	2594						
		DL Factor	0.6						
in g n		wDL (plf)	88.8						
Prof. (La		End Post Uplift for HDs (lb)	240						
		Max Compression	2594				THE NEW THE VERY STATE		
		Max Uplift	240						
		Max Shear Flow	110						
	Max Sh	near Floow Due to Wind or Seismic (W/S)	S						
	maxon	Sheathing Nails				The design of the second s	SALVE SALVES	THE AND STATES OF	STORE STORES
		# of Layers	1			A DOUBLE PARTY OF	A CONTRACTOR OF A		Quille Valence
		Shear Flow Allowabel (plf)	240					La ligada da la compete	The second second
		Check	Adequate						
			7/16 w/ 1 1/2" 16 ga. @ 6" O.C.			Subsequences and	A RESERVE MARKED	T. MARY NEWSFROM	development and the
		Shar Flow Allowable (plf)	155	TAN TRADUCTOR WARRANGED IN		CONVERTING AND INTO CONVERT	Constant of the Carlot	A UNIO A HELING MARKAGE CONC. M	<u> </u>
		Check	Adequate						
	Recom	imended Holdown @ Foundation 8" wall	not regd.	All and a second se					
12 S 1 S 1	Necom	Strap Style (Midwall)	not regd.						151
		Strap Style (Midwall) Strap Style (End Wall)	not reqd.						
		Recommended Holdown @ Wood Wall	not requ.						
		Strap Nailing Req'd	not reqd.						
	all and the second	# OF Minimum End Post	and the second se	and the second second second second					and the second s
		# OF WINIMUM ENd Post	not reqd	and a second			and the second second	and the second second	

JOB TITLE	Wooden Spur Renovation		
368 Wooden Spur Dr	. Alpine, Wyoming		
JOB NO.	225112	SHEET NO.	360339
CALCULATED BY	EVB	DATE	6/4/2025
CHECKED BY	0	DATE	

Shear Line: SW4 Sesimic Load (lb): 2200			w/ Wood Studs (Top Story) 21 (ASD Loading) s (ft) = 15.00					
S _{DS} 0.887		Segment 1	Segment 2	Segment 3	Segment 4	Segment 5	Segment 6	Segmen
Wind Load (lb): 2000	length (ft)	9	6		Service and the service	12 Million Children		S PARENS
	height (ft)	11	11		TE STORE STORE	CARLES STATE		
Roof DL (psf) 15	roof trib. (ft)	4	4		to Participante and	n network and the		
Floor DL (psf) 10	floor trib. (ft)	0	0					CARE LAN
Wall Self (psf) 8	Distance from HD to end of wall (in)	3	3					No. of the second
	Aspect Ratio:	1.22	1.83					
Seismic	shear flow (plf)	146.7	146.7					
bershite .	F (lbs)	1320	880					
	Seismic Factor	1.00	1.00					
	Adjusted Shear Flow (plf)	146.67	146.67					
-	MOT (ft-lb)	14520	9680					
	DL Factor A=	1.12	1.12					
	A x wDL (plf)	166	166					
	End Post Compression (lb)	1910	1678					
	DL Factor B=	0.48	0.48					
	B x wDL (plf)	70	70					
	End Post Uplift for HDs (lb)	812	918				1.4	
Wind	shear flow (plf)	133	133					
wind	F (lb)	1200	800					
	MOT (ft-lb)	13200	8800					
	wDL (plf)	148	148					
	End Post Compression (lb)	2175	1974					
	DL Factor	0.6	0.6					
the second se	wDL (plf)	88.8	89					
	End Post Uplift for HDs (lb)	1153	1334					
	Max Compression	2175	1974					
	Max Uplift	1153	1334					
	Max Shear Flow	147	147					
Max Sl	near Floow Due to Wind or Seismic (W/S)	S	S					
	Sheathing Nails 7/16		7/16 w/ 8d @ 6" O.C.	and the second second	CIMPERCIPACION		BERE MIRANSURAL	THE REAL
	# of Layers	1	1			STATISTICS.	State of the state of the state	100003160
	Shear Flow Allowabel (plf)	240	240			and a second sec	State of the state	
	Check	Adequate	Adequate					
	Sheathing Staples 7/16		7/16 w/ 1 1/2" 16 ga. @ 6" O.C.	AN AN AN AN AN		The second second		THE REAL PROPERTY IN
	Shar Flow Allowable (plf)	155	155					
	Check	Adequate	Adequate					
Recon	mended Holdown @ Foundation 8" wall	HDU2	HDU2			The state of the	Burt and the state	1.2 1.0 9.000
	Strap Style (Midwall)	LSTHD8	LSTHD8			No. of Contraction		6 In 77 p 20
	Strap Style (End Wall)	LSTHD8	LSTHD8					
	Recommended Holdown @ Wood Wall	MSTC28	MSTC28					
	Strap Nailing Req'd	(16) 16d	(16) 16d					
	# OF Minimum End Post	2	2		And the second sec	Non-statistication and statistication and	and the second sec	

JOB TITLE	Wooden Spur Renovation		
368 Wooden Spur Dr	. Alpine, Wyoming		
JOB NO.	225112	SHEET NO.	37 08 39
CALCULATED BY	EVB	DATE	6/4/2025
CHECKED BY	0	DATE	

Shear Line:	SW5			w/ Wood Studs (Top Story) 21 (ASD Loading) s (ft) = 5.00					
Sesimic Load (lb):	400								and the second
S _{DS}	0.887		Segment 1	Segment 2	Segment 3	Segment 4	Segment 5	Segment 6	Segment
Wind Load (Ib):	400	length (ft)	2.5	2.5		A. A. A.			
	1	height (ft)	7	7					and the start of
Roof DL (psf)	15	roof trib. (ft)	7	7		In the state of the	Contraction of the		and the second second
Floor DL (psf)	10	floor trib. (ft)	0	0	esting and the point				
Wall Self (psf)	8	Distance from HD to end of wall (in)	3	3			122010122		
		Aspect Ratio:	2.80	2.80					
		Aspect Ratio Factor (WSP):	0.90	0.90					
9	Seismic	shear flow (plf)	80.0	80.0					
		F (lbs)	200	200					
		Seismic Factor	0.71	0.71					
		Adjusted Shear Flow (plf)	112.00	112.00	A second s		l est i se		alter a p
		MOT (ft-lb)	1400	1400					
1.12		DL Factor A=	1.12	1.12					
		A x wDL (plf)	181	181					
		End Post Compression (Ib)	662	662					
		DL Factor B=	0.48	0.48					
		B x wDL (plf)	77	77					
		End Post Uplift for HDs (lb)	296	296	فتسبيها والمعادلي		9 ₅ - 1		10.000
L.	Wind	shear flow (plf)	80	80					
1		F (lb)	200	200					
		MOT (ft-lb)	1400	1400					
1		wDL (plf)	161	161					
		End Post Compression (lb)	823	823					
		DL Factor	0.6	0.6				(Carles to a second second	
		wDL (plf)	96.6	97					
		End Post Uplift for HDs (lb)	579	579					
		Max Compression	823	823					
		Max Compression Max Uplift	579	579					
		Max Opint Max Shear Flow	112	112					
	Max Shear Floow Due to Wind or Seismic (W/S)		S	S					
	IVIDA	Sheat ridow Due to wind of Seisnic (W/3) Sheathing Nails 7/16 v			AND A CONTRACTOR OF A CONTRACTOR	and the second second second	Childen Borner and All	A RO-STRUCTURE DI VERSION	C. C
		# of Layers	/ 8d @ 6" O.C. 1	7/16 w/ 8d @ 6" O.C. 1				Waterwy rest. Starparty	
			216	216			100.00 (A. 100.00 (A. 20)		UTS HALL THE
		Shear Flow Allowabel (plf)							
N 2 5 🛁		Check	Adequate	Adequate					
		Sheathing Staples 7/16 w		7/16 w/ 1 1/2" 16 ga. @ 6" O.C.	的研究性系统的主义是否的	WHEN SPIRE AN OLD DO	The Cost of States and States	建制性的复数形式 包括	The second second second
		Shar Flow Allowable (plf)	155	155					
		Check	Adequate	Adequate			-		
1	Reco	ommended Holdown @ Foundation 8" wall	not reqd.	not reqd.			بالأعالية المسالية الأكتاب		Part Nulling
		Strap Style (Midwall)	not reqd.	not reqd.					
		Strap Style (End Wall)	not reqd.	not reqd.	Section and the			Sec Alder was	
		Recommended Holdown @ Wood Wall	not reqd.	not reqd.					
		Strap Nailing Req'd	not reqd.	not reqd.	A COLUMN STATES				1.5
		# OF Minimum End Post	not reqd	not reqd				1 K	

JOB TITLE	Wooden Spur Renovation		
368 Wooden Spur Dr	. Alpine, Wyoming		
JOB NO.	225112	SHEET NO.	380139
CALCULATED BY	EVB	DATE	6/4/2025
CHECKED BY	0	DATE	

Shear Line:	SW6 1300	Wood Shear Walls w/ Wood Studs (Top Story) NDS 2021 (ASD Loading) Total Length of walls (ft) = 8.00							
S _{DS}	0.887		Segment 1	Segment 2	Segment 3	Segment 4	Segment 5	Segment 6	Segment
Wind Load (Ib):	2000	length (ft)	8		这些的"自己 "				
		height (ft)	9		Res advertised and a second		a the second second		
Roof DL (psf)	15	roof trib. (ft)	4		Protection and the second		and the second second	And the first state	
Floor DL (psf)	10	floor trib. (ft)	1						
Wall Self (psf)	8	Distance from HD to end of wall (in)	3						
		Aspect Ratio:	1.13						
Se	eismic	shear flow (plf)	162.5			,			1911
		F (lbs)	1300						
10.00 A 10.00 A 10.00 A		Seismic Factor	1.00						
		Adjusted Shear Flow (plf)	162.50			Loc – fing		<u>s</u>	8
		MOT (ft-lb)	11700						
		DL Factor A=	1.12						
		A x wDL (plf)	160						
		End Post Compression (Ib)	1695	in the second					- (4) (4) (4)
The second second		DL Factor B=	0.48						
and the second second		B x wDL (plf)	68						
L		End Post Uplift for HDs (Ib)	753						1
W	'ind	shear flow (plf)	250			15 2			1.1.1.1.1.1
		F (lb)	2000				Section of the second		in the
		MOT (ft-lb)	18000						
		wDL (plf)	142						
		End Post Compression (Ib)	2891						2.
		DL Factor	0.6						
		wDL (plf)	85.2						
	general Marchine	End Post Uplift for HDs (Ib)	2059		the state of the second	and the second second	2. a. X. L		
		Max Compression	2891						
		Max Uplift	2059						
		Max Shear Flow	250						
	Ma	x Shear Floow Due to Wind or Seismic (W/S)	W		Market and an Armiter Street and an Armiter	The second s	The second second second	The second second second second	PROFESSION STREET
		Sheathing Nails 7/16	w/ 8d @ 6" O.C. 1						
		# of Layers Shear Flow Allowabel (plf)	335		1999年1999年1999年1999年1999		A STATE OF THE STATE OF THE STATE		CASE AND CONTRACT
		Check	Adequate						
		Sheathing Staples 7/16			THE REPORT OF THE PARTY	Sector de la Constancia de	Transferration and the	NERVICE AND DESCRIPTION OF A	Superior and the second
		Shar Flow Allowable (plf)	322		A CHORE AND A CHEER WAY	A STATES AND REAL REAL PROPERTY.	Lesson et de la lese	A REAL PROPERTY AND A REAL	All Contractions and
		Check	Adequate						
-	Rec	commended Holdown @ Foundation 8" wall	HDU2		and the second	and the second second second			
	Net	Strap Style (Midwall)	LSTHD8	Contraction of the second					
		Strap Style (Midwall) Strap Style (End Wall)	LSTHD8						
	and a single start in the second	Recommended Holdown @ Wood Wall	MSTC40						
		Strap Nailing Reg'd	(32) 16d						
		# OF Minimum End Post	2						

JOB TITLE	Wooden Spur Renovation		
368 Wooden Spur Dr	Alpine, Wyoming		
JOB NO.	225112	SHEET NO.	390% 39
CALCULATED BY	EVB	DATE	6/4/2025
CHECKED BY	0	DATE	

Shear Line: simic Load (Ib):		Wood Shear Walls w/ Wood Studs (Top Story) NDS 2021 (ASD Loading) Total Length of walls (ft) = 16.50							
Sps	0.887		Segment 1	Segment 2	Segment 3	Segment 4	Segment 5	Segment 6	Segment
Wind Load (Ib):		length (ft)	8	8.5		1 Company and the set	(Providenting)	NAME OF COMPANY	AND DE DE DE
	Low and the second second	height (ft)	8	8			A CONTRACTOR OF THE		C DIMPOS
Roof DL (psf)	15	roof trib. (ft)	4	4	en gest gen gik en en angen en a	and a set of the set o		eters are and a striket sector (se	and the second of
Floor DL (psf)		floor trib. (ft)	2	2	W. Lie Marca			Repair Contractor	
Wall Self (psf)		Distance from HD to end of wall (in)	3	3					
wan sen (psi)	United and the second	Aspect Ratio:	1.00	0.94		CONTRACTOR AND A DESCRIPTION		APROPERTY AND	E SEADTE CON
		Aspect huno.	1.00	0.54					
	Seismic	shear flow (plf)	200.0	200.0					
		F (lbs)	1600	1700					
		Seismic Factor	1.00	1.00					
		Adjusted Shear Flow (plf)	200.00	200.00					
1990 - N. 1		MOT (ft-lb)	12800	13600					
		DL Factor A=	1.12	1.12					
		A x wDL (plf)	162	1.12					
		End Post Compression (lb)	1804	1842					
		DL Factor B=	0.48	0.48					- Second second
	10 I.	B x wDL (plf)	69	0.48 69					
			846	829					
		End Post Uplift for HDs (lb)	040	829	And the second second second	and the second second		L. U.S. Sanata	
	Wind	shear flow (plf)	279	279					
	wind		2230						
	Sec	F (lb)		2370					A STATEMENT
1.1.1.1.1		MOT (ft-lb)	17842	18958					
1.1.1		wDL (plf)	144	144					
		End Post Compression (Ib)	2878	2910				A STOCK HE HAD	
1 A 12		DL Factor	0.6	0.6					
	1	wDL (plf)	86.4	86					
		End Post Uplift for HDs (lb)	2033	2002		and the second sec			
		Max Compression	2878	2910					
		Max Uplift	2033	2002					
	and the state of t	Max Shear Flow	279	279					
	Max	Shear Floow Due to Wind or Seismic (W/S)	W	W			Land the second second		2.011.5.8.8.4
		Sheathing Nails 7/1		7/16 w/ 8d @ 6" O.C.					19,54010,392
		# of Layers	1	1			可以在自己的问题。		and the second second
		Shear Flow Allowabel (plf)	335	335					
		Check	Adequate	Adequate					아이 집에 걸려 봐요.
		Sheathing Staples 7/1		7/16 w/ 1 1/2" 16 ga. @ 4" O.C.		S IN NO SA SERIE		相關國際的關係	1.1.4 11.4 11.4
		Shar Flow Allowable (plf)	322	322					
	and the second	Check	Adequate	Adequate			and the second second	and the state of the	
	Rec	ommended Holdown @ Foundation 8" wall	HDU2	HDU2			Stand States	and a start of the start	
		Strap Style (Midwall)	LSTHD8	LSTHD8			영영 영화 가 모양 감독 감독		15" N (17 2008)
1.5.14	n nuel te	Strap Style (End Wall)	LSTHD8	LSTHD8	전체 제가 많은 것			en an	
. 1913 A.		Recommended Holdown @ Wood Wall	MSTC40	MSTA49					
-	1.5.5.1.7.1.5	Strap Nailing Req'd	(32) 16d	(26) 10d	Contraction of the second				
		# OF Minimum End Post	2	2					