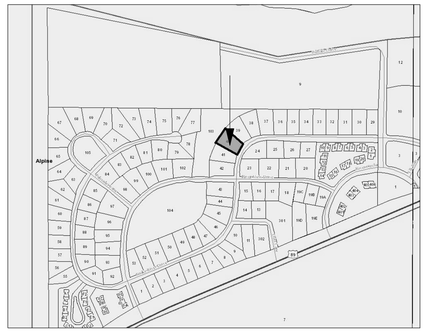
ABBREVIATIONS

APPROX. = APPROXIMATE
BOT. = BOTTOM
BRG = BEARING
CFM = CUBIC FEET PER MINUTE
CLR = CLEARANCE
CO = CARBON MONOXIDE
CONC. = CONCRETE
CONT. = CONTINUOUS
D = PENNY
DBL = DOUBLE
DECO = DECORATIVE
DEG. = DEGREE
DF = DOUGLAS FIR
DIA. = DIAMETER
DWG = DRAWING
EMBED. = EMBEDMENT
FND = FOUNDATION
FTG = FOOTING
GLB = GLULAM BEAM
GYP = GYPSUM
HORIZ = HORIZONTAL
MAX = MAXIMUM
MECH = MECHANICAL
MFGR = MANUFACTURER
MFGR'S = MANUFACTURER'S
MIN. = MINIMUM
NTS = NOT TO SCALE
O.C. = ON CENTER
PE = POLYETHYLENE
PT = PRESSURE TREATED
R = ROUND (IN LOG BEAM SCHEDULE)
REINF. = REINFORCE
REQ'D = REQUIRED
SEL. = SELECT
SF = SQUARE FEET
SQ. FT. = SQUARE FEET
SQR. = SQUARE
SS = SELECT STRUCTURAL
STRUCT. = STRUCTURAL
TBD = TO BE DETERMINED
TYP = TYPICAL
UNO = UNLESS NOTED OTHERWISE
UTIL = UTILITY
VERT = VERTICAL
VIF - VERIFY IN FIELD
W/ = WITH
WIC = WALK IN CLOSET
YR = YEAR

341 SNAKE RIVER DR LYTLE RESIDENCE, ALPINE, LINCOLN COUNTY, WYOMING



VICINITY MAP

PROJECT DATA

1. GOVERNING BUILDING CODE: IRC 2021

2. TYPE OF CONSTRUCTION: TYPE V-B

3. SPRINKLED: NO

PROJECT INFORMATION

BUILDING DEPARTMENT:

ALPINE, WYOMING

DRAWING INDEX

BUILDING SQ. FT.

LIVING SPACE :

BASEMENT = 754 SQ. FT.
MAIN FLOOR = 1532 SQ. FT.

TOTAL = 2286 SQ. FT.

NON LIVING SPACE :

UNFINISHED BASEMENT = 777 SQ. FT.
GARAGE = 446 SQ. FT.
DECK OR PORCH = 240 SQ. FT.

DESIGN NOTES

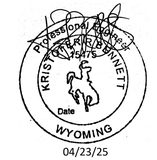
GROUND SNOW LOAD - 130 PSF
FLAT ROOF SNOW LOAD - 100 PSF
SNOW LOAD IMPORTANCE FACTOR - 1.0
SNOW EXPOSURE FACTOR - 1.0
THERMAL FACTOR - 1.1

OCCUPANCY CATEGORY - II
SOIL BEARING CAPACITY - 1500 PSF

ULTIMATE WIND SPEED - 115 MPH, EXP C
ASCE 7 DESIGN WIND SPEED - 105 MPH

SEISMIC DESIGN CATEGORY - D
SEISMIC SITE CLASS - D
RISK CATEGORY - II
SEISMIC COEFFICIENTS -
Sds: 0.88g Sd1: 0.44g R: 6.5 Cs: 0.14
SEISMIC ANALYSIS PROCEDURE -
EQUIVALENT LATERAL FORCE METHOD

FLOOR LIVE LOAD - 40 PSF
FLOOR DEAD LOAD - 15 PSF
ROOF DEAD LOAD - 15 PSF



04/23/25

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EMAIL: JOSH@DESIGNINTEL.COM

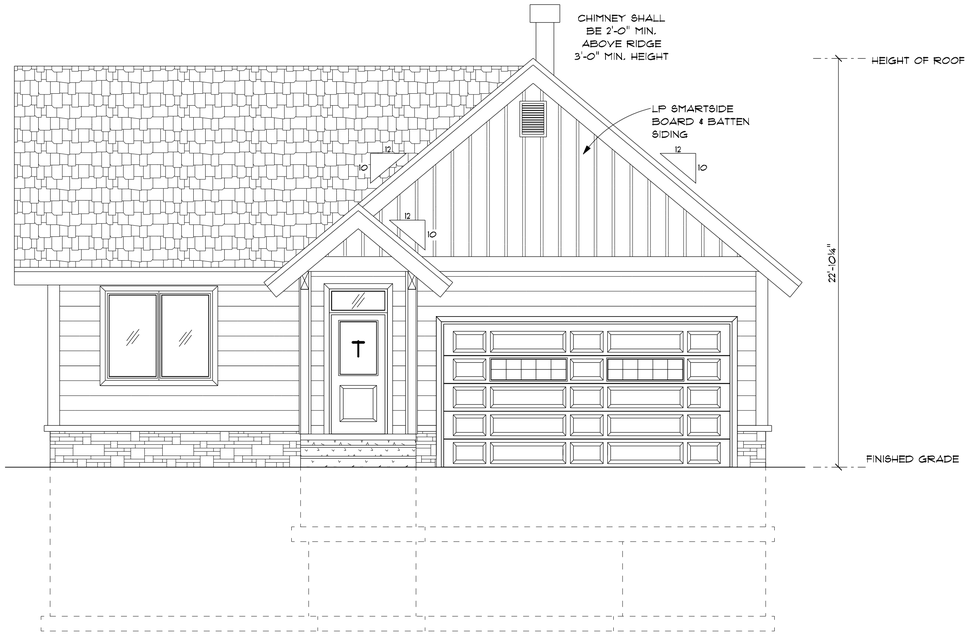
DATE 4/23/2025

SCALE AS NOTED
DRAWN BY MDW
2024-224

DESIGN INTELLIGENCE LLC THE RIGHT FIT

341 SNAKE RIVER DR LYTLE RESIDENCE
ALPINE, LINCOLN COUNTY, WYOMING

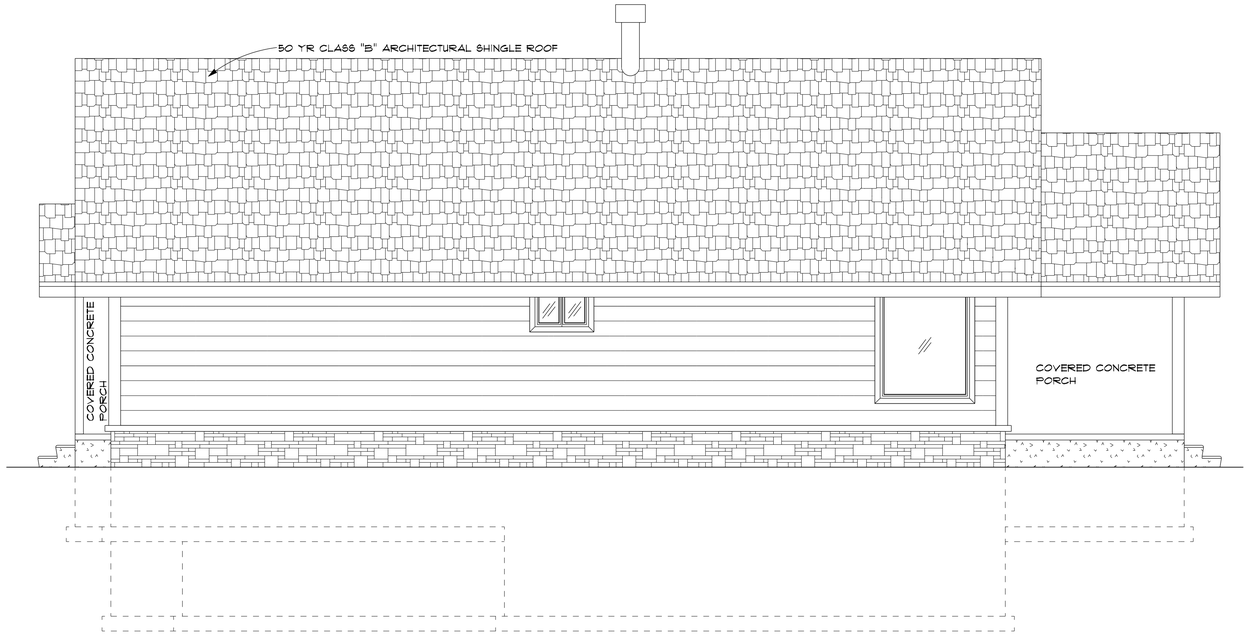
A0



FRONT ELEVATION

1/4" = 1'-0"

STEP FOOTING AS REQUIRED
BOTTOM OF FOOTING SHALL BE
3'-0" BELOW FINISHED GRADE
AND BELOW LOCAL FROST

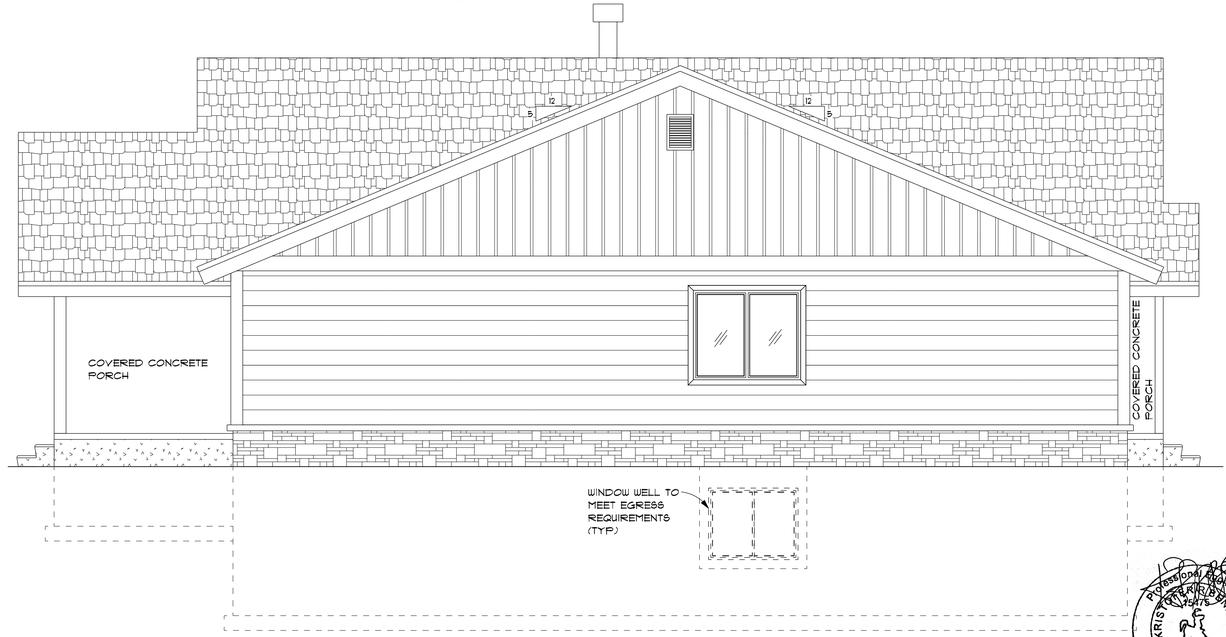


RIGHT ELEVATION

1/4" = 1'-0"

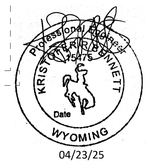
REAR ELEVATION

1/4" = 1'-0"



LEFT ELEVATION

1/4" = 1'-0"



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341 SNAKE RIVER DR LYTLE RESIDENCE
ALPINE, LINCOLN COUNTY, WYOMING

DATE 4/23/2025
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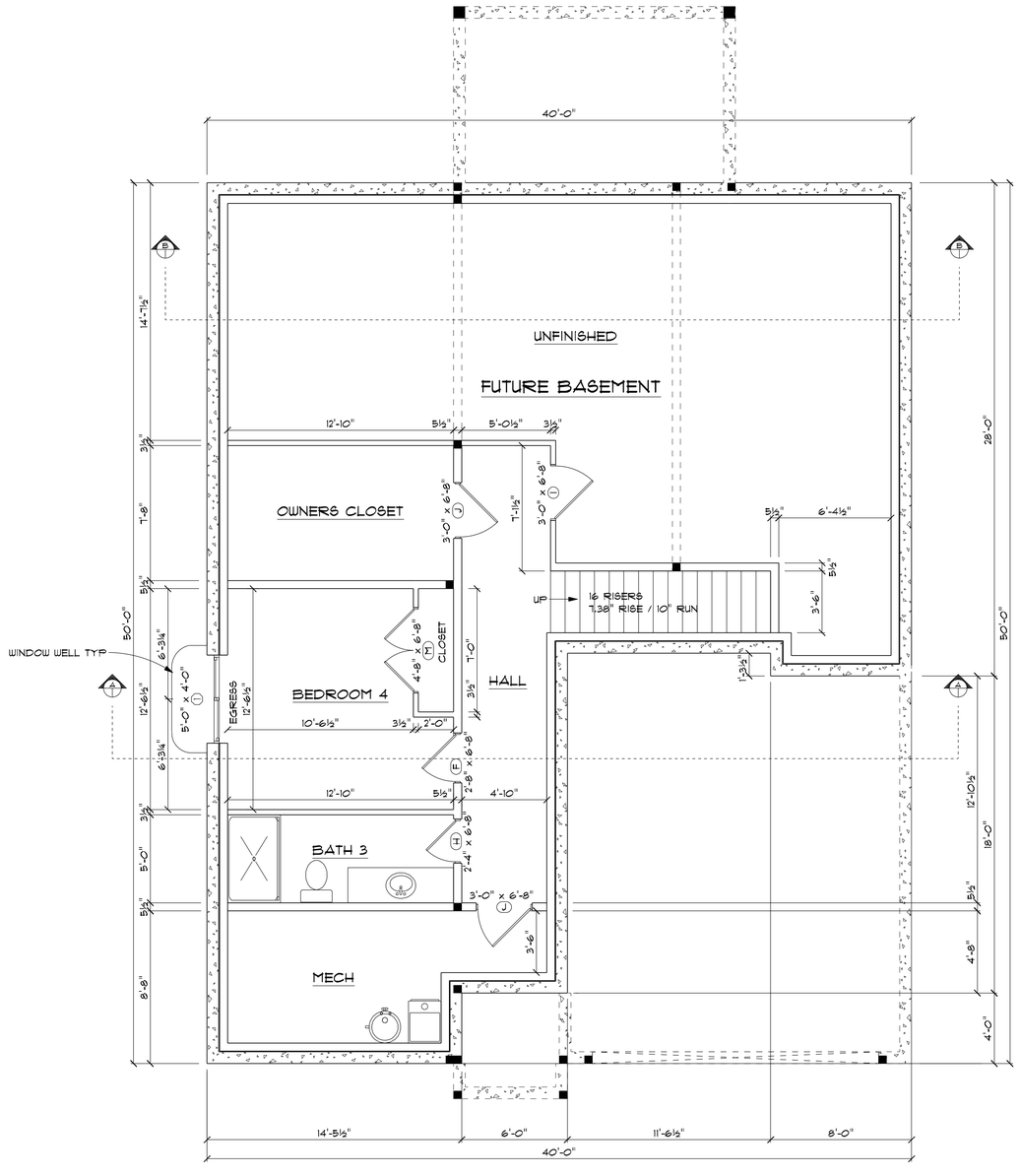
2024-224

DESIGN INTELLIGENCE LLC
THE RIGHT FIT

A1

A1



BASEMENT PLAN

1/4" = 1'-0"

FUTURE BASEMENT = 754 SQ. FT.
UNFINISHED BASEMENT = 777 SQ. FT.

LEGEND

■ STRUCTURAL POST

THE MECHANICAL IS NOT ENGINEERED. IT IS THE OWNERS RESPONSIBILITY TO HAVE THE MECHANICAL DESIGNED BY A MECHANICAL CONTRACTOR PRIOR TO CONSTRUCTION AND NOTIFY THE ENGINEER OF ANY POTENTIAL PROBLEMS.

NOTES

1. A SMOKE DETECTOR IS REQUIRED IN ALL ROOMS USED FOR SLEEPING, SMOKE AND CO DETECTOR ARE REQUIRED IN THE IMMEDIATE VICINITY OUTSIDE THE SLEEPING AREA, AND ON EACH LEVEL, HARD WIRED TOGETHER WITH BATTERY BACKUP.
2. ALL BATHROOMS SHALL HAVE A PROGRAMMABLE CEILING VENTILATION FAN WITH A MINIMUM CAPACITY OF 50 CFM AND A PASSIVE MAKE UP AIR INLET.
3. PROVIDE SEISMIC RESTRAINT STRAPPING FOR ALL WATER HEATERS.
4. SEE SHEET S2 FOR STRUCTURAL POST SIZES.
5. TYPICAL WINDOW HEADER HEIGHT 6'-8" UNO.

Professional Engineer
KRISTOPHER K. BENNETT
19475
Date
WYOMING
04/23/25

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341 SNAKE RIVER DR LYTLE RESIDENCE
ALPINE, LINCOLN COUNTY, WYOMING

SCALE	AS NOTED
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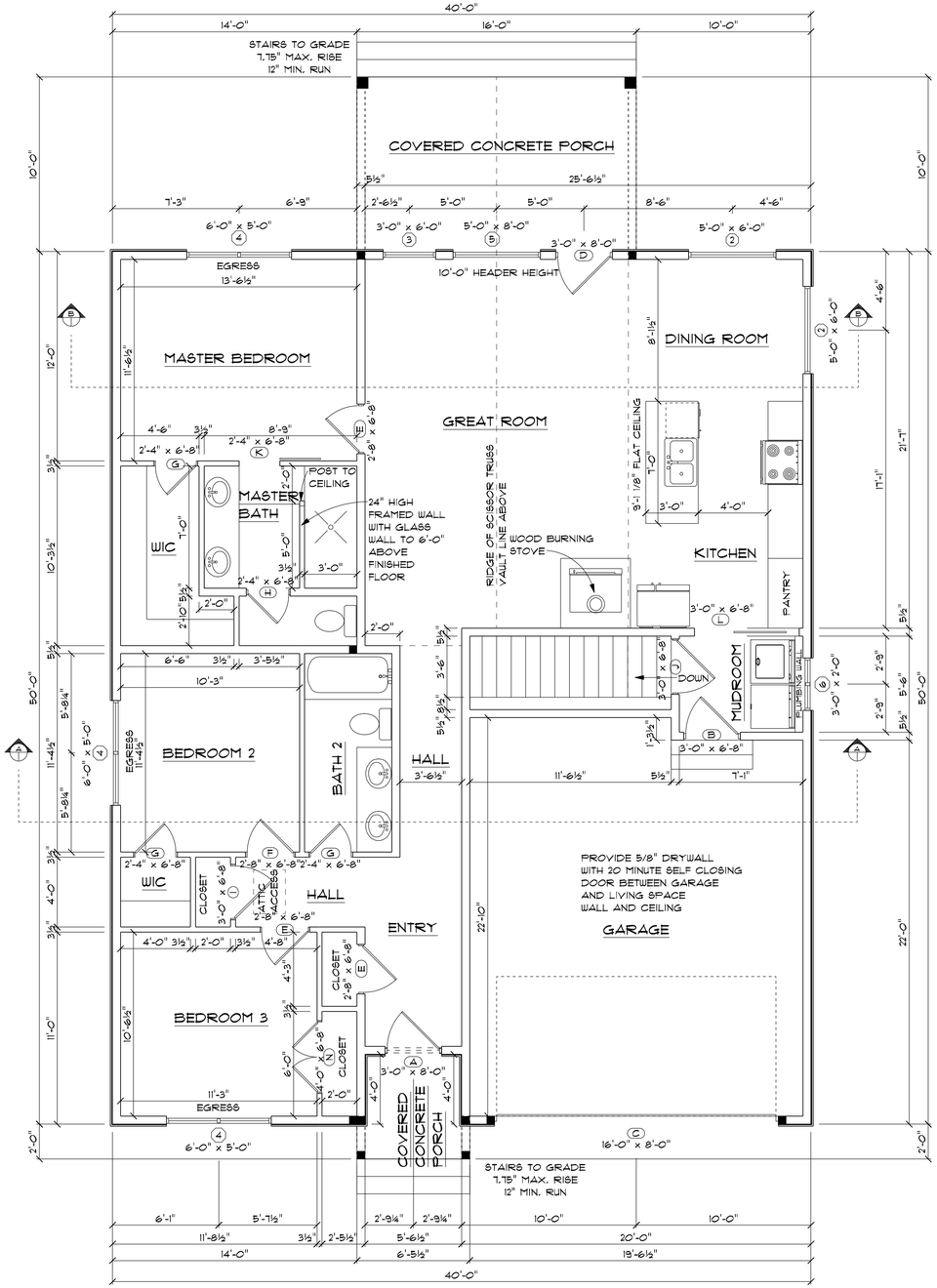
	2024-224
DATE	4/23/2025

DESIGN INTELLIGENCE LLC THE RIGHT FIT

A2

A2



MAIN FLOOR PLAN

1/4" = 1'-0"

LIVING SPACE = 1532 SQ. FT.
GARAGE = 446 SQ. FT.
DECK & PORCH = 240 SQ. FT.

LEGEND

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3. PROVIDE SEISMIC RESTRAINT STRAPPING FOR ALL WATER HEATERS.
4. SEE SHEET S2 FOR STRUCTURAL POST SIZES.
5. TYPICAL WINDOW HEADER HEIGHT 8'-0" UNO.
6. PROVIDE ATTIC ACCESS (22"x30" MIN.).

DOOR SCHEDULE				
LABEL	QTY	SIZE	HINGE DIR	TYPE
A	1	3'-0" x 8'-0"	R	Exterior Door\Country W/ 16" Transom In Top
B	1	3'-0" x 6'-8"	R	Exterior Door\Colonial 20 Minute Fire Rated
C	1	16'-0" x 8'-0"	U	Exterior Door\Garage
D	1	3'-0" x 8'-0"	L	Exterior Door\French
E	3	2'-8" x 6'-8"	L	Interior Door\Colonial
F	2	2'-8" x 6'-8"	R	Interior Door\Colonial
G	3	2'-4" x 6'-8"	L	Interior Door\Colonial
H	2	2'-4" x 6'-8"	R	Interior Door\Colonial
I	2	3'-0" x 6'-8"	R	Interior Door\Colonial
J	3	3'-0" x 6'-8"	L	Interior Door\Colonial
K	1	2'-4" x 6'-8"	R	Interior Door\Barn Doors
L	1	3'-0" x 6'-8"	N	Interior Door\Pocket
M	1	4'-8" x 6'-8"	LR	Interior Door\Colonial
N	1	4'-0" x 6'-8"	LR	Interior Door\Colonial

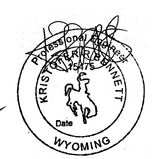
WINDOW SCHEDULE			
LABEL	QTY	SIZE	TYPE
1	1	5'-0" x 4'-0"	Window\Slider Egress
2	2	5'-0" x 6'-0"	Window\Casement
3	1	3'-0" x 6'-0"	Window\Casement
4	3	6'-0" x 5'-0"	Window\Casement Egress
5	1	5'-0" x 8'-0"	Window\Casement (T) W/ 24" Transom In Top
6	1	3'-0" x 2'-0"	Window\Casement

DOOR AND WINDOW NOTE:

CONTRACTOR SHALL VERIFY ALL WINDOW AND DOOR ROUGH OPENING SIZES AND LOCATIONS AS SIZES VARY BY MANUFACTURER.

U-FACTOR OF 0.29 FOR ALL EXTERIOR OPENINGS UNO.

(T) TEMPERED GLASS



04/23/25

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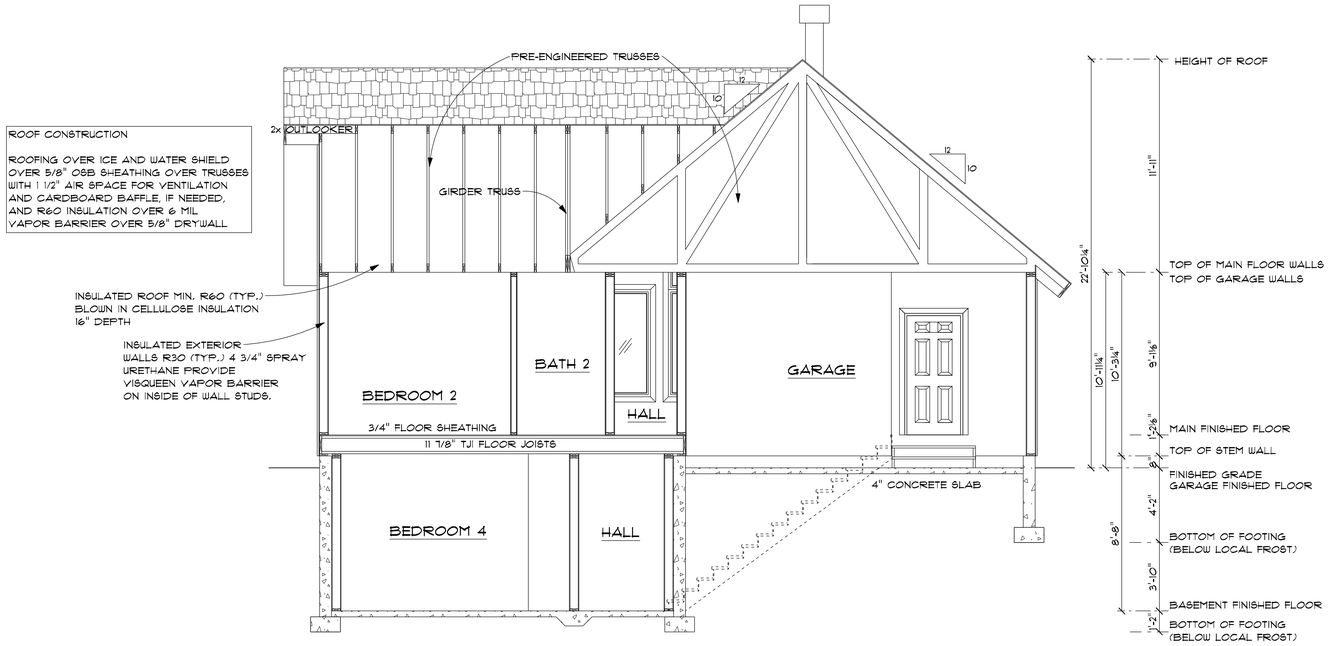
341 SNAKE RIVER DR LYTLE RESIDENCE
ALPINE, LINCOLN COUNTY, WYOMING

DATE 4/23/2025
SCALE AS NOTED
DRAWN BY MDW
2024-224

DESIGN INTELLIGENCE LLC THE RIGHT FIT

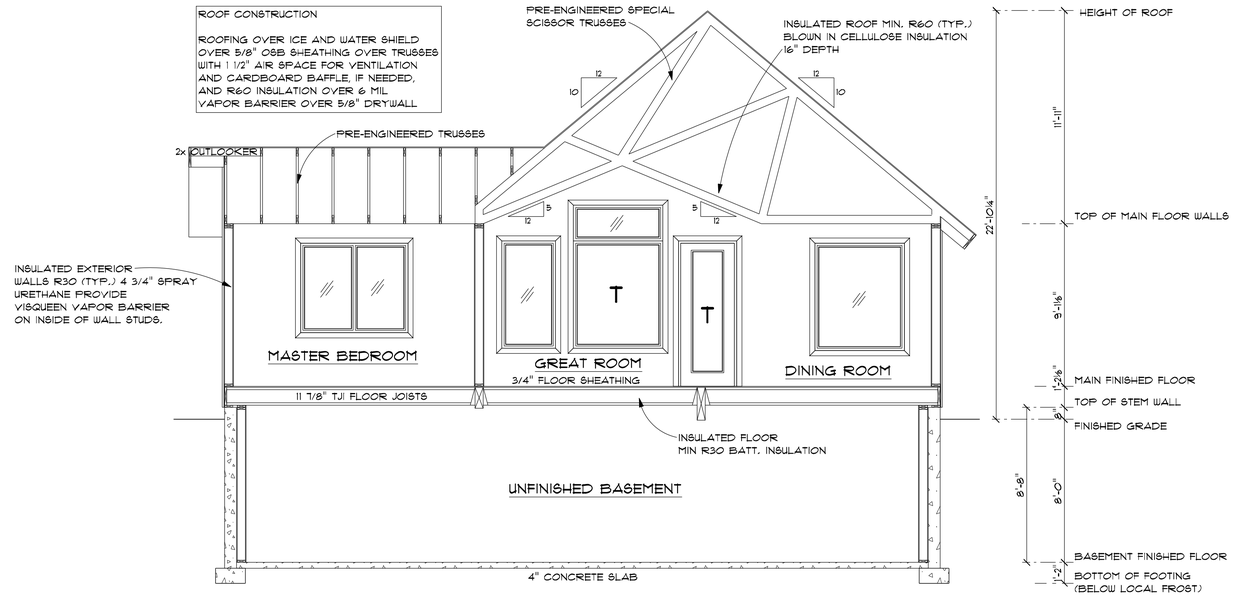
A3

A3



SECTION AA

1/4" = 1'-0"



SECTION BB

1/4" = 1'-0"

04/23/25

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341 SNAKE RIVER DR LYTLE RESIDENCE
ALPINE, LINCOLN COUNTY, WYOMING

A4

DATE 4/23/2025
SCALE AS NOTED
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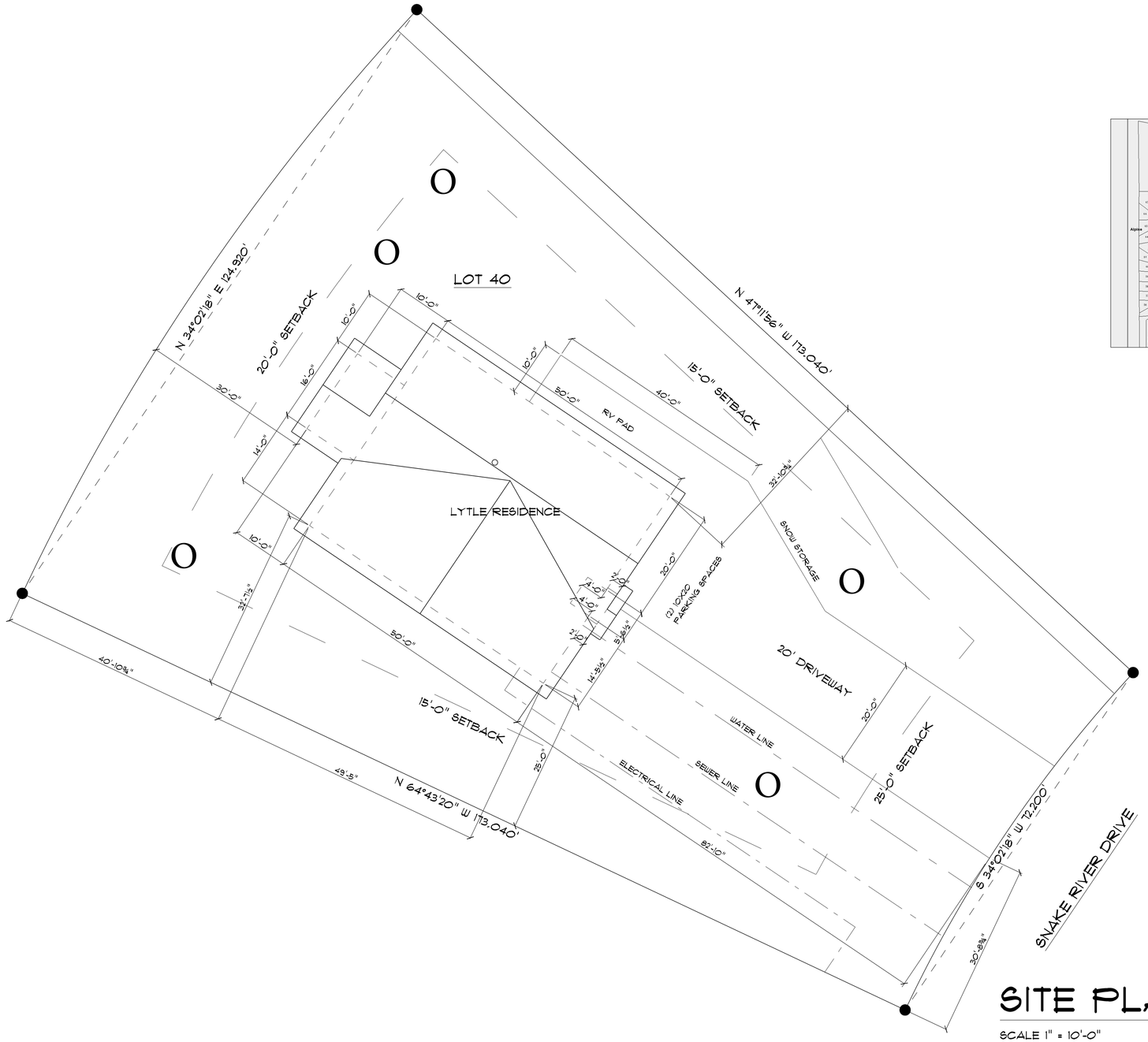
2024-224

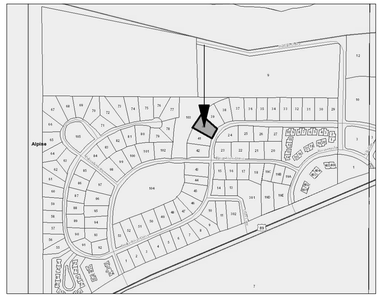
DESIGN INTELLIGENCE
THE RIGHT FIT

A4



SITE PLAN

SCALE 1" = 10'-0"



VICINITY MAP

LEGAL DESCRIPTION

341 SNAKE RIVER DR LYTLE RESIDENCE,
LOT 40, RIVER VIEW MEADOWS ADDN,
ALPINE, WYOMING

TOTAL LOT AREA = 0.39 ACRES = 16,988 SF

LANDSCAPING = 12,180 SF
DRIVEWAY : 2,211 SF
SNOW STORAGE = 443 SF - 20% OF DRIVEWAY

O TREE TYP.

Professional Engineer
KRISTOPHER J. BENNETT
15475
Date
WYOMING
04/23/25

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ALPINE, LINCOLN COUNTY, WYOMING

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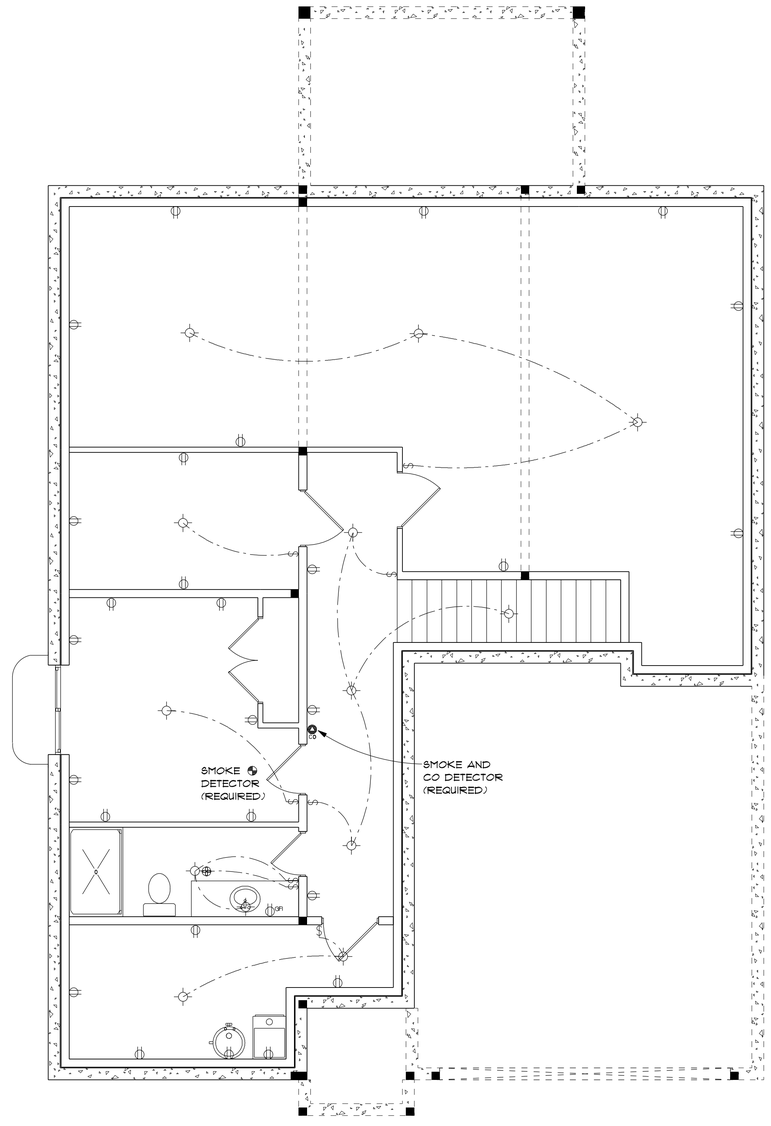
DESIGN INTELLIGENCE LLC THE RIGHT FIT

C1

C1



BASEMENT ELECTRICAL

1/4" = 1'-0"

THIS BASIC ELECTRICAL PLAN IS INTENDED TO REPRESENT THE OWNERS INTENT AND DOES NOT REPRESENT AN ENGINEERED SYSTEM. ALL FEATURES SHALL BE VERIFIED WITH THE OWNER.

NOTES

1. A SMOKE DETECTOR IS REQUIRED IN ALL ROOMS USED FOR SLEEPING, SMOKE AND CO DETECTOR ARE REQUIRED IN THE IMMEDIATE VICINITY OUTSIDE THE SLEEPING AREA, AND ON EACH LEVEL, HARD WIRED TOGETHER WITH BATTERY BACKUP.
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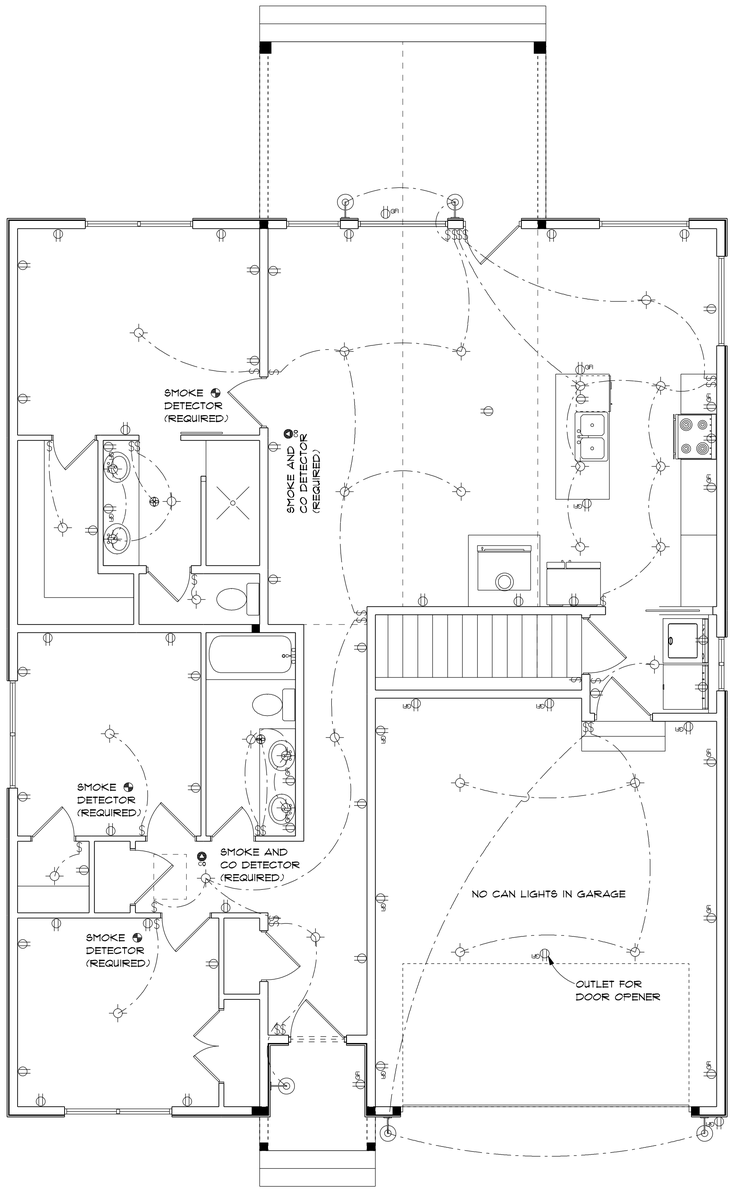
E1

341 SNAKE RIVER DR LYTLE RESIDENCE
ALPINE, LINCOLN COUNTY, WYOMING

E1



MAIN FLOOR ELECTRICAL

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ALPINE, LINCOLN COUNTY, WYOMING

SCALE	AS NOTED
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	2024-224

DATE 4/23/2025

THE RIGHT FIT

E2

E2

GENERAL STRUCTURAL NOTES

GENERAL

1. The Structural drawing shall be used in conjunction with the drawings of all other disciplines and the project specifications. The contractor shall verify the requirements of the other trades as to sleeves, chases, hangers, inserts, anchors, holes and other items to be placed or set in the structural work.

2. The contactor shall be responsible for complying with all safety precautions and regulations during the work. The engineer will not advise on nor issue direction as to safety precautions and programs.

3. The structural drawings herein represent the finished structure. The contractor shall provide all temporary bracing required to erect and hold the structure in proper alignment until all structural work and connections have been completed. The investigation, design, safety, adequacy and inspection of erection bracing, shoring, temporary supports, etc. is the sole responsibility of the contractor.

4. The engineer shall not be responsible for the methods, techniques and sequences of procedures to perform the work. The supervision of the work is the sole responsibility of the contractor.

5. Drawings indicate general and typical details of construction. Where conditions are not specifically shown, similar details of construction shall be used, subject to approval by the engineer.

6. All structural systems which are to be composed of components to be field erected shall be supervised by the supplier during manufacturing, delivery, handling, storage and erection in accordance with the suppliers instructions and requirements.

7. Loading applied to the structure during the process of construction shall not exceed the safe load carrying capacity of the structural members. The live loadings used in the design of the structure are indicated in the "Design Criteria Notes". Do not apply any construction loads until structural framing is properly connected together and until all temporary bracing is in place.

8. All ASTM and other references are per the latest editions of these standards, unless otherwise noted.

9. Shop drawings and other items shall be submitted to the engineer for review prior to fabrication. All shop drawings shall be reviewed by the general contractor before submittal. The engineer's review is to be for conformance with the design concept and general compliance with the relevant contract documents. The engineer's review does not relieve the contractor of the sole responsibility to review, check and coordinate the shop drawings prior to submission. The contractor remains solely responsible for errors and omissions associated with the preparation of the shop drawings as they pertain to member sizes, details, dimensions, etc.

10. Submit shop drawings to the Engineer. In no case shall reproduction of the contract drawings be used as shop drawings. Submit the following items for review:

    A. Concrete mix design(s) - NOT REQUIRED.
    B. Reinforcing steel shop drawings - NOT REQUIRED
    C. Structural steel shop drawings - NOT REQUIRED
    D. Steel Joist / Girder shop drawings - NOT REQUIRED
    E. Metal decking shop drawings - NOT REQUIRED
    F. Pre-manuf. wood system / truss shop drawings - NOT REQUIRED
    G. Pre-engineered metal building system - NOT REQUIRED

Other submittals may be required per the "Schedule of Special Inspections" or the separate notes contained herein.

11. Special inspections are not required on projects with an IRC governing building code (see cover sheet). Special inspections are required on IBC projects as noted below:

    A. Concrete - NOT REQUIRED
    B. Bolts Installed in Concrete - NOT REQUIRED
    C. Structural Welding - Field Welds - NOT REQUIRED
    D. High Strength Bolting - NOT REQUIRED
    E. Structural Masonry - NOT REQUIRED
    F. Plated Wood Trusses w/ 60' or greater span or 60" or greater height - REQUIRED
    G. Shear Walls - REQUIRED

12. Unless otherwise indicated, all items noted to be demolished shall become the contractor's property and be removed from the site.

13. Contractors shall visit the site prior to bid to ascertain conditions which may adversely affect the work or cost thereof.

14. Ducts, plumbing and openings through engineered shear walls shall not exceed 6" in diameter except as noted on drawings. No perforations exceeding 3/4" in diameter shall be made in structural members except as noted on drawings. Perforations with 3/4" diameter and smaller shall be made in the center 1/3rd of the beam height and length. A maximum of (2) perforations per beam are allowed. Contact the engineer if additional perforations are required. A minimum of 6" horizontal distance between perforations is required.

DESIGN CRITERIA

Design Gravity Loads:

Roof DL - SEE COVER SHEET
Floor DL - SEE COVER SHEET

Design Live Loads:

Roof LL - 20 psf min
Snow - SEE COVER SHEET
Commercial Floor LL - 50 psf + 15 psf Partition
Residential LL - 40 psf

Lateral Live Loads:

Wind - SEE COVER SHEET
Seismic - SEE COVER SHEET
Equivalent Fluid Pressure - 45 pcf

CAST-IN-PLACE CONCRETE NOTES

1. Concrete mixes shall be designed per ACI 301, using Portland Cement conforming to ASTM C-150 or C-595, aggregate conforming to ASTM C-33, and admixtures conforming to ASTM C-494, C-1017, C-618, C-989 and C-260. Concrete shall be ready-mixed in accordance with ASTM C-94.

2. Concrete shall conform to the following compressive Strength, slump and air entrainment requirements:

    Concrete Compressive strength shall be 3000 psi. (3500 psi for slabs on grade permanently exposed to weather)

    Concrete permanently exposed to weather shall be air entrained to 6% (+/- 1%).

    Slump of concrete placed in removable forms shall be 6" max. Slump of concrete placed in stay-in-place forms shall be 6"-8".

3. All concrete work shall conform to the requirements of ACI 301, "Specification for Structural Concrete Buildings". Hot weather concreting shall be in accordance with ACI 305. Cold weather concreting shall be in accordance with ACI 306.

4. All reinforcing steel shall conform to ASTM A-615, Grade 60. All welding of reinforcing steel shall be in accordance with AWS D1.4. Epoxy coated reinforcing shall conform to ASTM A-775.

5. All welded wire fabric (WWF) shall conform to ASTM A-185.

6. All reinforcing steel shall be set and tied in place prior to pouring of concrete, except that vertical dowels for masonry wall reinforcing may be "floated" in place. Do not field bend bars partially embedded in hardened concrete unless specifically indicated or approved by the Engineer. Anchor bolts may be "wet set" while concrete is still plastic before it has set. Where anchor bolts resist placement or the consolidation of concrete around anchor bolts is impeded, the concrete shall be vibrated to ensure full contact between the anchor bolts and concrete.

7. All reinforcing steel indicated as being continuous (Cont.) shall be lapped 30" for #4 bars, 38" for #5 bars and 45" for #6 bars.

8. Unless noted otherwise, the following concrete cover shall be provided for reinforcement:

    A. Concrete cast against & permanently exposed to earth - 3"
    B. Concrete w/ removable forms exposed to earth or weather:
        #6 through #18 bars - 2"
        #5 bar, W31, D31 wire & smaller - 1 1/2"
    C. Concrete not exposed to earth or weather:
        Walls, elevated slabs - 3/4"
        Beams and columns - 1 1/2"

9. Bar supports and holding bars shall be provided for all reinforcing steel to ensure minimum concrete cover. Bar supports shall be plastic tipped or stainless steel.

10. All edges of permanently exposed concrete surfaces shall be chamfered 3/4" unless otherwise noted.

11. In order to avoid concrete shrinkage cracking, place concrete slabs in an alternating lane pattern. The maximum length of slab cast in any one continuous pour shall be limited to 80 feet. The maximum spacing of joints shall be 25 feet.

12. Formwork shall remain in place until concrete has obtained at least 90% of its 28 day compressive strength. The Contractor shall provide all shoring and reshoring.

13. Reinforcing steel hooks and bends shall be bent at a diameter of 6 times the diameter of the bar with an extension length of 12 times the diameter of the bar or alternative standard hook lengths as described in the referenced building codes.

FOUNDATION NOTES

1. See Cast-in-Place Concrete notes for additional requirements.

2. The building official shall determine whether to require a soil test to determine the soil's characteristics at a particular location.

3. Unless noted otherwise on the drawings, all footings shall bear on undisturbed, firm natural soil or compacted fill capable of supporting a minimum design bearing pressure as noted on the cover sheet. All foundation excavations shall be evaluated by a qualified geotechnical engineer/testing agency prior to pouring foundation concrete if required by the building official.

4. Top of footing elevations shall be as shown on elevation drawings and sections. Unless noted otherwise, the bottom of all exterior footings shall be placed 6" below local frost depth.

5. No unbalanced backfilling over 4'-0" shall be done against foundation walls unless walls are securely braced against overturning either by temporary bracing or by permanent construction.

6. Prior to commencing any foundation work, coordinate work with any existing utilities. Foundations shall be lowered where required to avoid utilities.

7. Unless noted otherwise, the centerlines of column foundations shall be located on column centerlines.

8. All retaining walls shall have at least 12" of free draining granular backfill, full height of wall. Provide control joints in retaining walls at approximately equal intervals not to exceed 25 feet nor 3 times the wall height. Provide expansion joints at every fourth control joint, unless otherwise indicated.

SLAB ON GRADE NOTES

1. See Cast-in-Place Concrete notes for additional requirements.

2. Provide concrete slabs over a 6 mil polyethylene vapor barrier and 4" of porous fill. Maximum slump for concrete slabs shall be 5", using Type II cement.

3. All porous fill material shall be a clean granular material with 100% passing a 1-1/2" sieve and no more than 5% passing a No. 4 sieve. Porous fill shall be compacted to 95% max. dry density per ASTM D-698.

4. Slab joints shall be filled with approved material. This should take place as late as possible, preferably 4 to 6 weeks after the slab has been cast. Prior to filling, remove all debris from the joints, then fill in accordance with the manufacturer's recommendations or as follows:

    6" slabs - fill with Epoxy resin
    Other slabs - fill with field molded of elastomeric sealant.

5. Unless approved otherwise, all reinforcing shall be blocked into the center of the slab with precast concrete blocks having a compressive strength equal to that of the slab.

6. Walk ways and other exterior slabs are not shown on the structural drawings. See the site plan and architectural drawings for location, dimensions, elevations, jointing details and finish details. Provide 4" walks reinforced with 6x6 - W1.4xW1.4 WWF unless otherwise noted.

7. See architectural drawings for exact locations of depressed slab areas and drains. Slope slab to drains where shown.

8. The finish tolerance of all slabs shall be in accordance with ACI 301, Type A.

9. Floor flatness and levelness tests shall be conducted if deemed necessary by the owner in accordance with ASTM E 1155. Results, including acceptance or rejection of the work will be provided to the contractor within 48 hours after data collection. Remedies for out of tolerance work may include removal and reconstruction at the contractors expense. Any other remediation requires the approval of the owner.

RADON CONTROL

1. A minimum 6-mil (or 3-mil cross laminated) polyethylene or equivalent flexible sheeting material shall be placed on top of the gas permeable layer prior to pouring the slab. The sheeting should cover the entire floor area and separate sections of sheeting should be overlapped at least 12 inches.

2. To retard soil gas entry, large openings through concrete slabs, wood, and other floor assemblies in contact with the soil, such as spaces around bathtub, shower, or toilet drains, shall be filled or closed with materials that provide a permanent, airtight seal such as non-shrink mortar, grouts, expanding foam, or similar materials designed for such application.

3. A minimum 3-inch diameter PVC or other gas-tight pipe shall be embedded vertically into the sub slab aggregate or other permeable material before the slab is poured. A "T" fitting or other support on the bottom of the pipe shall be used to ensure that the pipe opening remains within the sub-slab permeable material. This gas tight pipe shall be extended vertically through the building floors, terminate at least 12 inches above the surface of the roof, in a location at least 10 feet away from any window or other opening into the conditioned spaces of the building that is less than 2 feet below the exhaust point, and 10 feet from any adjoining or adjacent buildings.

WOOD FRAMING NOTES

1. All wood framing material shall be surfaced dry and used at 19% maximum moisture content.

2. All wall framing shall be No. 2 grade Doug Fir unless noted otherwise.

3. All joists, rafters, headers & misc. framing shall be Select Str. grade Doug Fir UNO. Provide rim board or full depth blocking at bearing ends and provide full depth blocking at intermediate bearing supports. Provide full depth 1.25" min. thickness solid blocking or metal bridging at midspan and at a max. spacing of 8 ft o.c. between UNO.

4. All framing within 8" of grade or in contact with masonry or concrete shall be pressure treated in accordance with the American Wood Preservers Association Specifications where possible. All cuts and holes should be completed before treatment. Cuts and holes due to on-site fabrication shall be brushed with 2 coats of copper naphthenate solution containing a minimum of 2% metallic copper in solution (per AWPA STD. M4).

5. Provide single joists under all partition walls which run parallel with floor joists. Unless noted otherwise, provide double joists under all bearing walls which run parallel with floor joists. Provide 1" min. width solid blocking under all bearing walls which run perpendicular with joists. Provide solid blocking the width of the post under all concentrated loads from framing above.

6. Provide header beams of the same size as joists or rafters to frame around openings in the plywood deck unless otherwise indicated.

7. Structural steel plate connectors shall conform to ASTM A-36 specifications and be 1/4" thick unless noted otherwise. Bolts connecting wood members shall be ASTM A-307 and be 3/4" diameter unless otherwise indicated. Provide washers for all bolt heads and nuts in contact with wood surfaces.

8. Bolt holes shall be carefully centered and drilled not more than 1/16" larger than the bolt diameter. Bolted connections shall be snugged tight but not to the extent of crushing wood under washers.

9. Prefabricated metal joist hangers, hurricane clips, hold-down anchors and other accessories shall be as manufactured by "Simpson Strong-Tie Company", or approved equal. Install all accessories per the manufacturer's requirements. All steel shall have a minimum thickness of 0.04 inches (per ASTM A446, Grade A) and be galvanized (coating G60).

10. Holes and notches drilled or cut into wood framing shall not exceed the requirements of the referenced building code or the manufacturers specifications.

11. All plates, anchors, nails, bolts, washers and other miscellaneous hardware permanently exposed to weather or in treated wood shall be hot dip galvanized.

12. All 8d nails shall have a minimum shank diameter of 0.113". All 10d x 12d nails shall have a minimum shank diameter of 0.120". All 16d nails shall have a minimum shank diameter of 0.131".

13. All Douglas Fir shall be Douglas Fir-Larch (North) UNO

14. Bearing walls and shear walls require double top plates with either 24" laps or a steel splice plate. Butt joint splices require 3x16x0.036" min. straps w/ (18) 8d nails each side of the splice. Corner splices require 3x8x0.036" min. straps w/ (9) 8d nails each side of the splice.

15. Studs shall span continuously between floors and/or roofs. Studs shall not be spliced.

16. See IRC Table R602.3(1) for Fastening Schedule (UNO).

17. Wood Framed Decks shall be clad with 2x Spruce Pine Fir #2 or better unless noted otherwise. If Trex decking or similar is used, joist spacing over 16" o.c. may need to be tightened to 16" o.c. Check with the manufacturer for framing requirements.

18. Deck lateral support has been provided using hangers fastened with screws to resist pullout in accordance with recommendations in "Lateral Load Path and Capacity of Exterior Decks" published in Structural Magazine Wood Design Focus V. 23, N. 2. This method has been evaluated and tested to meet or exceed building code requirements for lateral deck tension straps.

PLYWOOD/GYPBOARD SHEATHING NOTES

1. All plywood construction shall be in accordance with the American Plywood Association (APA) specifications.

2. All roof panel sheathing shall be 5/8" (nom.) OSB 1 APA rated sheathing unless noted otherwise. Suitable edge support shall be provided by use of panel clips or 2x blocking between framing as required by local building codes. 2x blocking shall be installed between outlookers over exterior walls. Unless otherwise noted connect roof sheathing with 8d common nails at 6" o.c. at supported panel edges and 6" o.c. at intermediate supports. At gable ends provide 8d nails at 6" o.c. from rafter or blocking to top plate of wall.

3. All floor sheathing shall be 3/4" (nom.) APA rated STURD-I-FLOOR Exp. 1, with tongue and groove edge. Unless noted otherwise connect floor sheathing with 8d common nails spaced 6" o.c. at supported edges and 12" o.c. at intermediate supports. Field glue using adhesives meeting APA specification AFG-01, applied in accordance with the manufacturer's recommendations.

4. All wall sheathing shall be 7/16" OSB APA rated sheathing. Unless noted otherwise, connect wall sheathing with 8d common nails spaced at 6" o.c. at supported panel edges and 12" o.c at intermediate supports.

5. Install wall sheathing either vertically or horizontally with panel continuous over two or more spans. All other sheathing shall have long edges spanning over supports. Stagger panel end joints.

6. All nailing shall be carefully driven and not over-driven.

7. Provide 2x blocking at all unsupported panel edges at walls.

PRE-ENGINEERED TRUSS NOTES

1. Wood trusses shall be designed by the manufacturer to support the loads dictated by the governing jurisdiction.

2. Wood trusses shall be designed by the manufacturer in accordance with the applicable provisions of the latest edition of the National Design Specification of the National Forest Products Association and the design specification for metal plate connected wood trusses of the Truss Plate Institute.

3. Wood materials shall be Douglas Fir and shall be kiln dried and used at 19% maximum moisture content. Provide grade required to meet stress requirements.

4. Connector plates shall be not less than 0.036 inches (20 gage) in coated thickness, shall meet or exceed ASTM Grade A or higher and shall be hot dipped galvanized according to ASTM A-525 (coating G60). Minimum steel yield stress shall be 33,000 psi.

5. Trusses shall be fabricated in a properly equipped manufacturing facility of a permanent nature. Trusses shall be manufactured by experienced workmen, using precision cutting, jigging and pressing equipment under the requirements in quality control standard QST-88 of the Truss Plate Institute.

6. Secondary bending stresses in truss top and bottom chords due to dead, live and wind loads shall be considered in the design. Load duration factors shall be per the "National Design Specification for Wood Construction" per referenced codes.

7. Wood trusses shall be erected in accordance with the truss manufacturer's requirements. This work shall be done by a qualified and experienced contractor.

8. The Contractor shall provide all temporary and permanent bracing as required for safe erection and performance of the trusses including permanent bracing supporting the bottom chord of the gable end truss and top plates of gable end walls. The guidelines set forth in the "BCSI-B1 Summary Sheet - Guide for Handling, Installing and Restraining and Bracing of Trusses" shall be a minimum requirement.

9. Truss member and components shall not be cut, notched drilled nor otherwise altered in any way without the written approval of the Engineer.

10. Submit complete shop drawings for all wood trusses if specified in General Structural Notes section 10.F. Drawings shall show member sizes, species, grade, moisture content, span, camber, dimensions, chord pitch, bracing requirements and loadings. Shop drawings shall be submitted to the Engineer and shall bear the seal of a Professional Engineer in the appropriate jurisdiction.

NOTE TO CONTRACTOR

1. TRUSS DRAWINGS SHALL BE ON SITE AT THE TIME OF FRAMING INSPECTION.
2. JOIST/RAFTER MANUFACTURER'S INSTALLATION MANUAL OF INSTRUCTIONS TO BE ON SITE AT THE TIME OF FRAMING INSPECTION.

MASONRY VENEERS

1. 2.5" max. thickness Stone or Cultured Stone Veneers - attach to framed walls per manufacturer's specifications.

2. Stone or Masonry Veneers over 3" thick - approved brick-ties shall be secured to studs with an approved water-resistant barrier. Studs spaced at 16" o.c. max require 24" o.c. vertical brick tie spacing. Studs spaced at 24" o.c. max require 12" o.c. vertical brick tie spacing. Brick ties shall be installed per manufacturer's specifications. Provide a 1" air gap between the barrier and the veneer. See details for bearing support.

FIRE BLOCKING

Fire blocking shall be provided in wood-frame construction in the following locations:

1. In concealed spaces of stud walls and partitions, including furred spaces and parallel rows of studs or staggered studs, as follows:
    1.1 Vertically at the ceiling and floor levels.
    1.2 Horizontally in intervals not exceeding 10 feet
2. At all intersections between concealed vertical and horizontal spaces such as occur at soffits, drop ceilings and cove ceilings.
3. In concealed spaces between stair stringers at the top and bottom of the run.
4. At openings around vents, pipes, ducts, cables and wires at ceiling and floor level, with an approved material to resist the free passage of flame and products of combustion.

STRUCTURAL STEEL NOTES

1. All structural steel shall conform to the latest edition of the "Manual of Steel Construction" of the AISC.

2. Unless noted otherwise, all materials shall be in conformance with the following ASTM specifications:

MEMBER	ASTM	MIN. STRENGTH
Structural Tubing	A500 Grade B	46 ksi
Steel Pipe	A53 (Type E or Grade B)	35 ksi
Wide Flange	A992	50 ksi
Other Rolled Shapes and Plates	A36	36 ksi
Connection Bolts	A325	92 ksi
Anchor Bolts	F1554	36 ksi
Threaded Rods	A36	36 ksi
Non-Shrink Grout	C1107	8000 psi

3. Minimum bolt diameter shall be 3/4" unless noted otherwise. All bolts shall be shear/bearing type bolts and be snug-tight.

4. All welding shall be in accordance with AWS D1.1 using E70XX electrodes. Unless noted otherwise, provide cont. min. sized fillet welds per AISC requirements. All filler material shall have a minimum yield strength of 58 ksi.

5. Where "Continuous Chord" angles are indicated, provide a continuous butt weld or full penetration weld at the splice connection detail for approval.

6. Where steel beams bear across building expansion joints or at wall control joints, provide a "slip" connection.

7. Holes in steel shall be drilled or punched. All slotted holes shall be provided with smooth edges. Burning of holes and torch cutting at the site is not permitted.

8. Unless otherwise noted, all structural steel permanently exposed to view shall be shop painted with one coat of SSPC 15-68, Type 1 (Red Oxide) paint.

9. Steel fabricators shall be an AISC certified shop for Category I steel structures and maintain detailed quality control procedures as required to satisfy the special inspection requirements of the International Building Code.

10. Unless otherwise noted, all structural steel permanently exposed to the weather, including all brick shelf angles shall be hot-dipped galvanized in accordance with ASTM A153.

11. Protective coatings damaged during the transporting, erecting and field welding processes shall be repaired in the field to match the shop applied coating.

12. The contractor shall hire an independent testing agency to provide special inspections of bolting, welding and other items in accordance with the International Building Code.

SITE PREPARATION NOTES

1. Excavate a minimum of 4" of existing soil for a minimum of 5 feet beyond the building limits. Remove all organics, pavement, roots, debris and otherwise unsuitable material.

2. The surface of the exposed subgrade shall be inspected by probing or testing to check for pockets of soft or unsuitable material. Excavate unsuitable soil as directed by the engineer.

3. Proof roll the surface of the exposed subgrade with a loaded tandem axle dump truck. Remove all soils which pump or does not compact properly as directed by the engineer.

4. Fill all excavated areas with approved controlled fill. Place in 8" loose lifts and compact to a minimum of 95% of the maximum dry density in accordance with ASTM D-698.

5. All controlled fill material shall be a select granular material free from all organics or otherwise deleterious material with not more than 20% by weight passing a no. 200 sieve and with a plasticity index not to exceed 6%.

6. Provide field density tests for each 3,000 SF of building area for each lift of controlled fill.

7. Surface drainage shall be diverted to a storm sewer conveyance or other approved point of collection that does not create a hazard. Lots shall be graded to drain surface water away from foundation walls. The grade shall fall a minimum of 6 inches within the first 10 feet. Finished grade slopes shall be limited to a maximum of 2:1 unless noted otherwise.

04/23/25

CONTRACTOR'S RESPONSIBILITY

IT IS THE CONTRACTOR'S RESPONSIBILITY TO REVIEW ALL ASPECTS OF THESE DRAWINGS, ARCHITECTURAL AND STRUCTURAL, PRIOR TO CONSTRUCTION. ANY CONFLICTS SHALL BE REPORTED TO THE ENGINEER FOR CORRECTION. CHANGES MAY BE PROPOSED BY THE CONTRACTOR IF HE FEELS THE CHANGE IS IN THE BEST INTEREST OF THE OWNER. CHANGES SHALL BE FORWARDED TO THE ENGINEER IN WRITING FOR APPROVAL PRIOR TO CONSTRUCTION.

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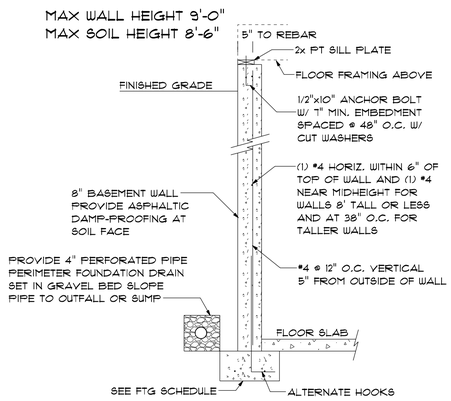
341 SNAKE RIVER DR LYTLE RESIDENCE
ALPINE, LINCOLN COUNTY, WYOMING

DATE 4/23/2025
SCALE AS NOTED
DRAWN BY MDW
2024-224

DESIGN INTELLIGENCE LLC THE RIGHT FIT

S0.1

S0.1



BASEMENT WALL

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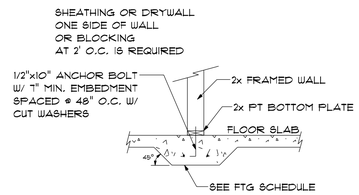
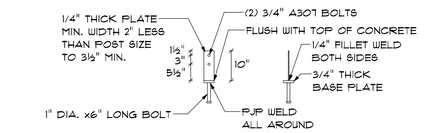
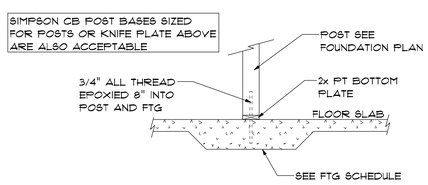


WALL TO INTERIOR FOOTING

2 S1.0



ALTERNATE KNIFE PLATE DETAIL



POST TO INTERIOR FOOTING

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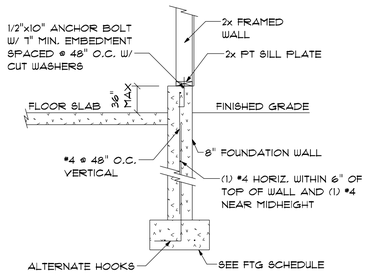
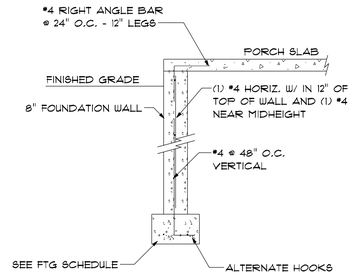


MAX WALL HEIGHT = 6'-0"

STEM WALL WITH SLAB FLOOR

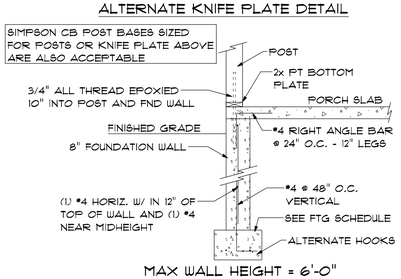
4 S1.0



MAX WALL HEIGHT = 6'-0"

STEM WALL AT PORCH

5 S1.0



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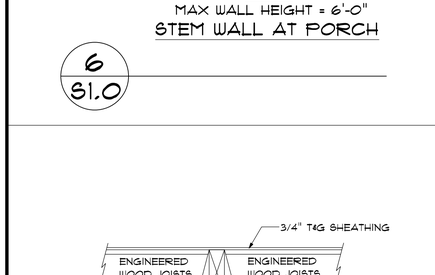
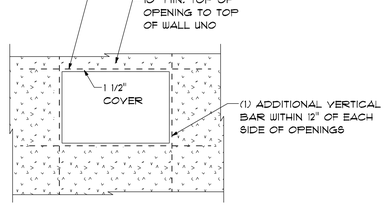


MAX WALL HEIGHT = 6'-0"

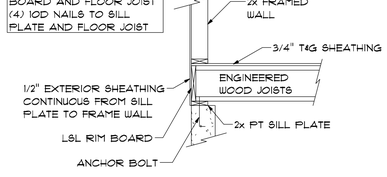
STEM WALL AT PORCH

6 S1.0



TYPICAL FOUNDATION WALL OPENING REINFORCING

7 S1.0



FLOOR JOIST TO STEM WALL

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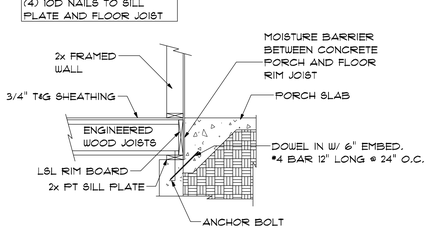

PORCH ATTACHMENT

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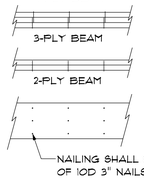

BUILT UP BEAM

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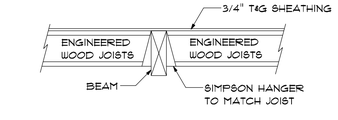

JOIST TO BEAM

11 S1.0

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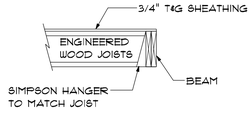

JOIST TO BEAM

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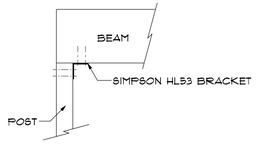

BEAM TO POST

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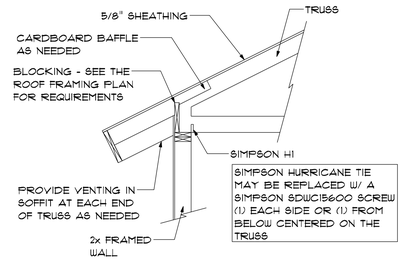

TRUSS TO FRAMED WALL

14 S1.0

15 S1.0

Professional Engineer
KRISTOPHER J. BENNETT
25475
Date
WYOMING
04/23/25

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341 SNAKE RIVER DR LYTLE RESIDENCE
ALPINE, LINCOLN COUNTY, WYOMING

S1.0

DATE 4/23/2025

DESIGN INTELLIGENCE LLC THE RIGHT FIT

SCALE AS NOTED
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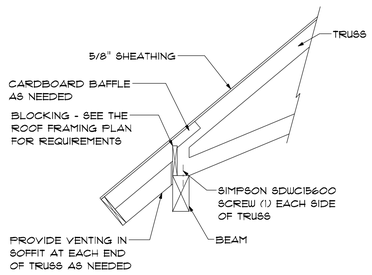
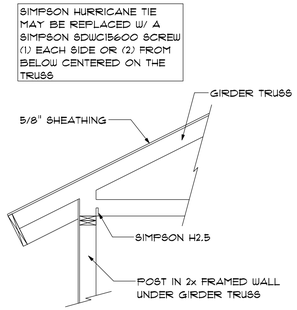
2024-224

S1.0



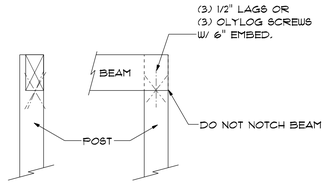
TRUSS TO BEAM

1
S1.1



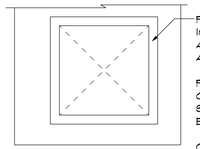
GIRDER TRUSS TO FRAMED WALL

2
S1.1



BEAM TO POST

3
S1.1



WINDOWS AND DOORS SHALL BE INSTALLED PER MANUFACTURERS APPROVED INSTALLATION DETAILS.

WEATHER RESISTIVE BARRIER FLASHING

4
S1.1

5
S1.1

6
S1.1

7
S1.1

8
S1.1

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S1.1

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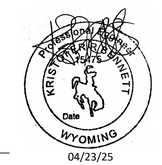
S1.1



04/23/25

CONTRACTOR'S RESPONSIBILITY

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S1.1

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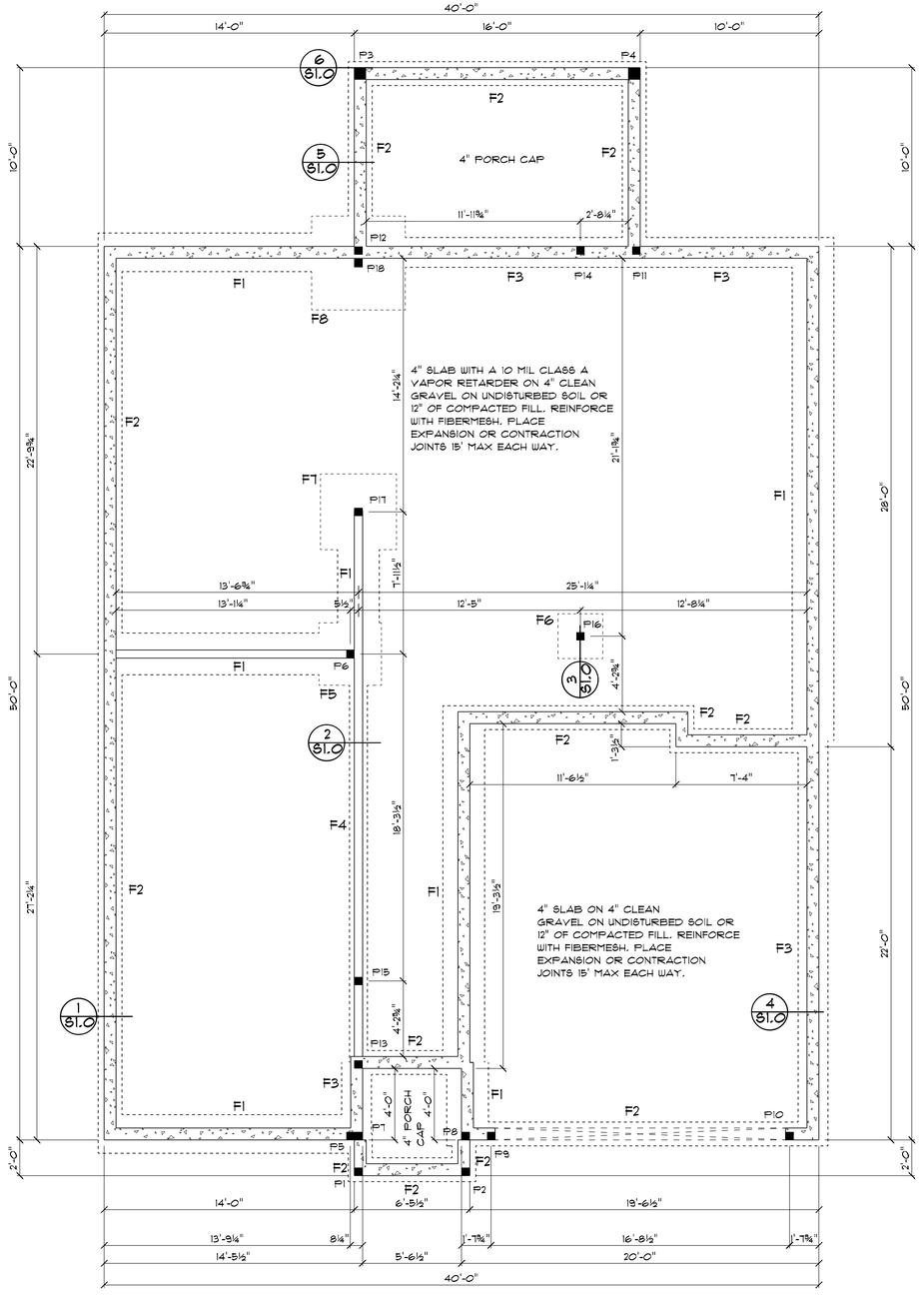


FOUNDATION PLAN

1/4" = 1'-0"

BLOCKOUTS

CONTRACTOR SHALL VERIFY ALL WINDOW AND DOOR ROUGH OPENING SIZES BEFORE FORMING BLOCKOUTS. SEE ARCHITECTURAL DRAWINGS FOR ALL WINDOW AND DOOR SIZES AND LOCATIONS.

SEE SHEET S5 FOR SHEAR WALLS, HOLD DOWNS AND KING STUD SCHEDULE

BOTTOM OF FOOTINGS & TOP OF STEM WALL HEIGHT MAY VARY SEE ARCHITECTURAL DRAWINGS

FOUNDATION NOTES

1. SEE SHEET S0.1 FOR ADDITIONAL GENERAL NOTES.
2. BOTTOM OF FOOTING SHALL BE BELOW LOCAL FROST LINE.

UP TO (3) 2x6 GANGSTUD POSTS EMBEDDED IN WALLS DO NOT REQUIRE POST BASES.

POST SCHEDULE

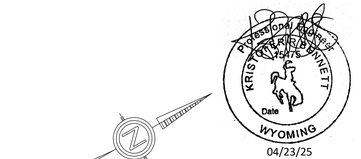
P1-P2 = DF #1 6x6
P3-P4 = DF #1 8x8
P5-P8 = (3) DF #2 2x6
P9-P10 = (2) DF #2 2x6
P11-P12 = (3) DF #2 2x6
P13-P14 = POINT LOAD
P15 = (2) DF #2 2x6
P16-P18 = (4) DF #2 2x6

FOOTING SCHEDULE

F1 = 28X10 CONT. FTG WITH (3) #4 CONT.
F2 = 16X10 CONT. FTG WITH (2) #4 CONT.
F3 = 20X10 CONT. FTG WITH (2) #4 CONT.
F4 = 12X10 CONT. FTG WITH (2) #4 CONT.
F5 = 42X42X10 FTG WITH (4) #4 EACH WAY
F6 = 30X30X10 FTG WITH (3) #4 EACH WAY
F7 = 51X51X10 FTG WITH (5) #4 EACH WAY
F8 = 63X63X10 FTG WITH (6) #4 EACH WAY

LEGEND

■ STRUCTURAL POST ABOVE



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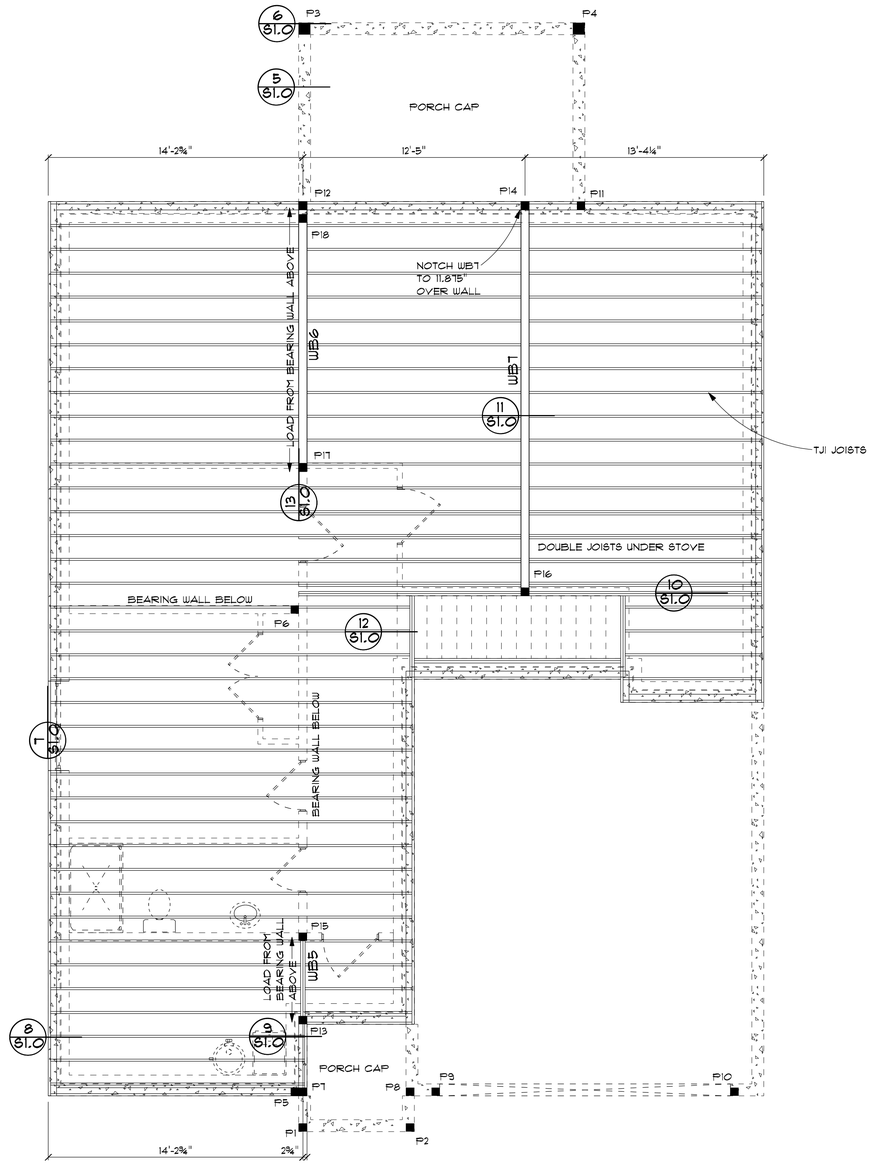
S2

S2



MAIN FLOOR FRAMING

1/4" = 1'-0"

FLOOR FRAMING NOTES

1. INSTALL JOISTS PER MANUFACTURER'S RECOMMENDATIONS INCLUDING ALL BRIDGING AND BRACING.
2. PROVIDE DBL JOISTS UNDER ALL BEARING WALLS THAT RUN PARALLEL TO FLOOR JOISTS.
3. FRAME AROUND CRAWL SPACE ACCESS AND STAIRS USING (2) 1.75x11.875 LVL W/ SIMPSON HUCQ412-6DS HANGERS OR GREATER WHERE APPLICABLE UNO.
4. DECK LEDGERS SHALL SUPPORT DECK JOISTS ONLY. DECK BEAMS SHALL HANG DIRECTLY TO OTHER DECK BEAMS, RIM BOARDS, GANGSTUDS AND FOUND. WALLS USING SIMPSON HUC28-2 FOR (2) 2X8 BEAMS AND HUCQ210-2-6DS FOR LARGER BEAM SIZES UNO.
5. ALL EXTERIOR WALLS ARE BEARING WALLS UNO.
6. DF #2 2X6 AT 16" O.C. INTERIOR BEARING WALLS UNO ON SHEAR WALL DRAWINGS.
7. BEARING WALL HEADERS SHALL BE (2) DF 2x10 OR (3) 1.5x5.5 LVL UNO WITH (1) DF 2x TRIMMER.
8. HEADERS SHOWN IN THE BEAM SCHEDULE REQUIRE (2) DF 2x TRIMMERS UNO.
9. JOIST COUNT SHOULD BE DETERMINED FROM JOIST SPACING NOT FROM DRAWING LAYOUT.
10. SEE SHEET S2 FOR STRUCTURAL POST SIZES.
11. SEE SHEET S4 FOR BEAM SCHEDULE.

SEE SHEET S5
FOR SHEAR WALLS,
HOLD DOWNS AND KING
STUD SCHEDULE

TJI JOISTS SHALL BE
11 7/8" TJI 110
OR EQUIVALENT
SPACED @ 16" O.C. UNO

LEGEND

■ STRUCTURAL POST

04/23/25

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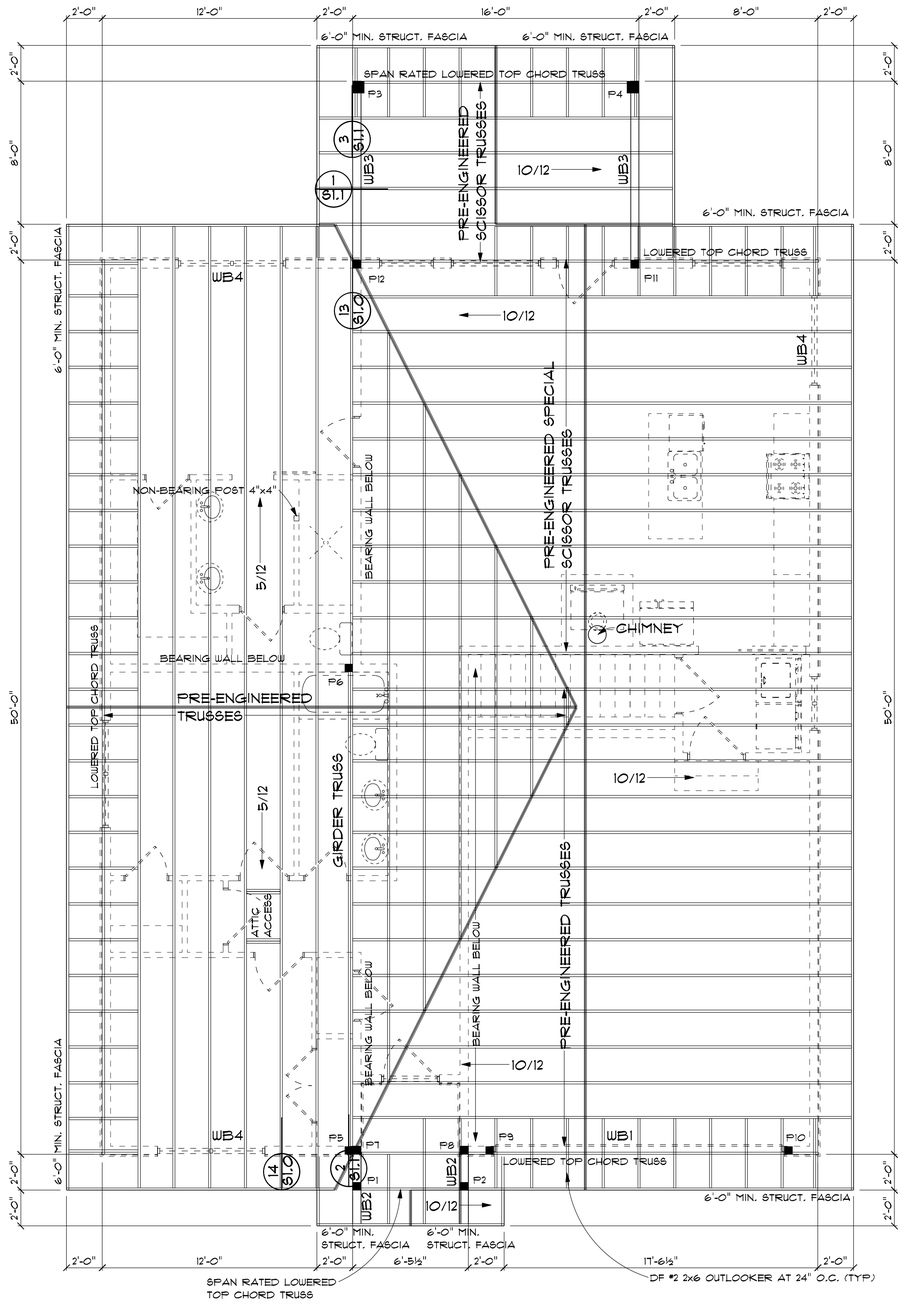
S3



ROOF FRAMING

1/4" = 1'-0"

LEGEND

■ STRUCTURAL POST

ROOF FRAMING BLOCKING:

BLOCKING IS REQUIRED BETWEEN RAFTERS OR TRUSSES OVER ALL END BEARING SUPPORTS AND OVER SHEAR WALLS RUNNING PERPENDICULAR TO RAFTERS OR TRUSSES.

BLOCKING SHALL NAIL TO ADJACENT RAFTERS OR TRUSSES AND TO THE WALL OR BEAM BELOW W/ 8D NAILS AT 6" O.C. STACKED BLOCKING IS NOT PERMITTED.

A 1" TO 2" VENTING GAP BETWEEN THE TOP OF BLOCKING AND THE BOTTOM OF ROOF SHEATHING OR (4) 1.5" VENTING HOLES IN THE BLOCKING SHALL BE PROVIDED.

BLOCKING MATERIAL OPTIONS:
1. 14" MAX HEIGHT BLOCKING - 2X OR LVL BLOCKING.
2. 48" MAX HEIGHT BLOCKING - FRAMED SHEAR WALLS BETWEEN TRUSSES. SHEATHING/NAILING SHALL MATCH THE SHEAR WALL BELOW.
3. TRUSS MANUFACTURER SUPPLIED BLOCKING

STRUCTURAL FASCIA NOTES:

UNLESS NOTED OTHERWISE:
1. STRUCTURAL FASCIAS ARE DF #2 2X6
2. STRUCTURAL FASCIAS SHALL ATTACH TO EACH SUPPORT PLY W/ (4) 10D NAILS
3. THE FIRST SUPPORT OVER THE WALL CORNER SHALL BE A (2) DF #2 2X6

OVERBUILDS SHALL BE VALLEY TRUSSES OR DF #2 2X4 AT 16" O.C. WALLS BUILT EVERY 24" O.C. OVERBUILD FRAMING SHALL ATTACH TO SUPPORT FRAMING BELOW W/ (3) 10D NAILS AT EACH SUPPORT

TRUSS FRAMED ROOF NOTES:

1. ALL EXTERIOR WALLS ARE BEARING WALLS.
2. DF #2 2X6 AT 16" O.C. INTERIOR BEARING WALLS UNO ON SHEAR WALL DRAWINGS.
3. BEARING WALL HEADERS SHALL BE (2) DF 2x10 OR (3) 1.5x5.5 LVL UNO WITH (1) DF 2x TRIMMER.
4. HEADERS SHOWN IN THE BEAM SCHEDULE REQUIRE (2) DF 2x TRIMMERS UNO.
5. ALL ROOF OVERHANGS SHALL BE AS NOTED.
6. INSTALL TRUSSES PER MANUFACTURER'S RECOMMENDATIONS INCLUDING ALL BRIDGING AND BRACING.
7. AT BRG. ENDS OF EACH TRUSS - PROVIDE SIMPSON H1 HURRICANE TIES OR SDWC15600 SCREWS - (1) EACH SIDE OR (1) FROM BELOW CENTERED ON THE TRUSS.
8. OUTLOOKERS SHALL ATTACH WITH (3) 10D NAILS TO THE COMMON TRUSS AND DROP CHORD TRUSS OR GABLE WALL. BACKSPANS SHALL MATCH OVERHANGS.
9. TRUSSES HAVE A TYPICAL 12" HEEL HEIGHT UNO.
10. PROVIDE ATTIC ACCESS (22"x30" MIN).
11. SEE THE ROOF FRAMING BLOCKING NOTE FOR BLOCKING REQUIREMENTS.

BEAM GRADING SHALL BE AS FOLLOWS UNO:
DF - SELECT STRUCTURAL
GLB - 24F-V4 DF/DF
LVL - 2.0, 2600Fb
FS - FULL SAWN

BEAM SCHEDULE

WB1 = (3) 1.75x11.875 LVL
WB2 = DF #1 6x8
WB3 = 5.125x10.5 GLB
WB4 = (2) 1.75x9.5 LVL
WB5 = (2) 1.75x11.875 LVL
WB6 = 6.75x18 GLB
WB7 = 6.75x18 GLB

ICE BARRIER NOTES:

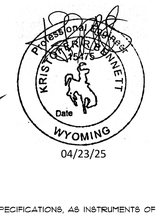
PROVIDE ICE AND WATER SHIELD TO COVER ENTIRE ROOF.

ROOF VENTILATION:

PROVIDE ROOF VENTILATION 1 SF FOR EVERY 300 SF OF ATTIC SPACE, 1/2 HIGH AND 1/2 LOW.

VALLEY FLASHING NOTES:

PROVIDE VALLEY FLASHING MINIMUM 28 GAUGE, GALVANIZED OR CORROSION RESISTANT METAL EXTENDING 10" FROM THE CENTER LINE EACH WAY.



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S4

S4

STAPLING ALTERNATIVE:
FOR 8D NAILS @ 6" O.C. EDGE NAILING
USE 3" O.C. STAPLES
FOR 8D NAILS @ 4" O.C. EDGE NAILING
USE 2" O.C. STAPLES

1.75", 16 GAGE STAPLES W/ 7/16" CROWN
FIELD SPACING SHALL BE 6" O.C.
DO NOT USE STAPLES FOR 2" O.C. NAILS

ALTERNATE HOLD DOWN SCHEDULE

STHD10 OR STHD10RJ USE HDU2-SDS2.5 W/ 5/8"
ALL-THREAD EPOXIED IN PLACE - 8" EMBEDMENT.

STHD14 OR STHD14RJ USE HDU4-SDS2.5 W/ 5/8"
ALL-THREAD EPOXIED IN PLACE - 8" EMBEDMENT.

WOOD SIDE MEMBER THICKNESS - 3" MIN.

SHEAR WALL NOTES

1. ALL FRAMED SHEAR WALLS SHALL BE DF #2 2X6 @ 16" O.C. WITH 7/16" APA RATED SHEATHING WITH 8D NAILS UNO. PROVIDE 12" O.C. FIELD NAILING TYP. STAGGER EDGE NAILING AT 3X BLOCKING. SEE THE SHEAR WALL DESIGN TABLE FOR EDGE NAILING AND ADDITIONAL SHEAR WALL REQUIREMENTS. SOME DESIGNS MAY NOT BE UTILIZED.
2. SHEAR BLOCKING (IF REQUIRED) SHALL BE PROVIDED AT ALL PANEL EDGES FOR EDGE NAILING.
3. ALL EXTERIOR WALLS SHALL BE NAILED PER S1 UNO.
4. ALL HOLD DOWNS ARE SIMPSON BRAND AND SHALL BE INSTALLED PER THE MANUFACTURER'S REQUIREMENTS.
5. WALL ID'S (LIKE H1-1) ARE FOR ENGINEERS REFERENCE.
6. ALL FRAMED WALLS SHALL BE SUPPORTED AT TOP AND BOTTOM BY FLOOR OR ROOF SYSTEMS. SPLICING WALLS AT UNSUPPORTED LOCATIONS IS NOT PERMITTED.

SHEAR WALL DESIGN TABLE			
LABEL	EDGE NAILING SPACING	SHEAR BLOCKING	SHEATHING SIDES
S1	6" O.C.	NONE	SINGLE
S2	4" O.C.	2X	SINGLE
S3	2" O.C.	3X	SINGLE
S4	2" O.C.	3X	DOUBLE

2X6 FRAMED WALL (16.6 PSF)
KING STUD SCHEDULE

WITH 6' MAX HEADER LENGTHS:
STUD HEIGHT: KING STUD SIZE:
UP TO 9'-2" (1) DF #2 2X6
UP TO 12'-11" (2) DF #2 2X6
UP TO 16'-0" (2) DF SEL 2X6
UP TO 19'-5" (3) 1.5X5.5 LVL

WITH 12' MAX HEADER LENGTHS:
STUD HEIGHT: KING STUD SIZE:
UP TO 9'-7" (2) DF #2 2X6
UP TO 12'-1" (2) DF SEL 2X6
UP TO 14'-10" (3) DF SEL 2X6
UP TO 16'-0" (4) DF SEL 2X6
UP TO 18'-11" (5) 1.5X5.5 LVL

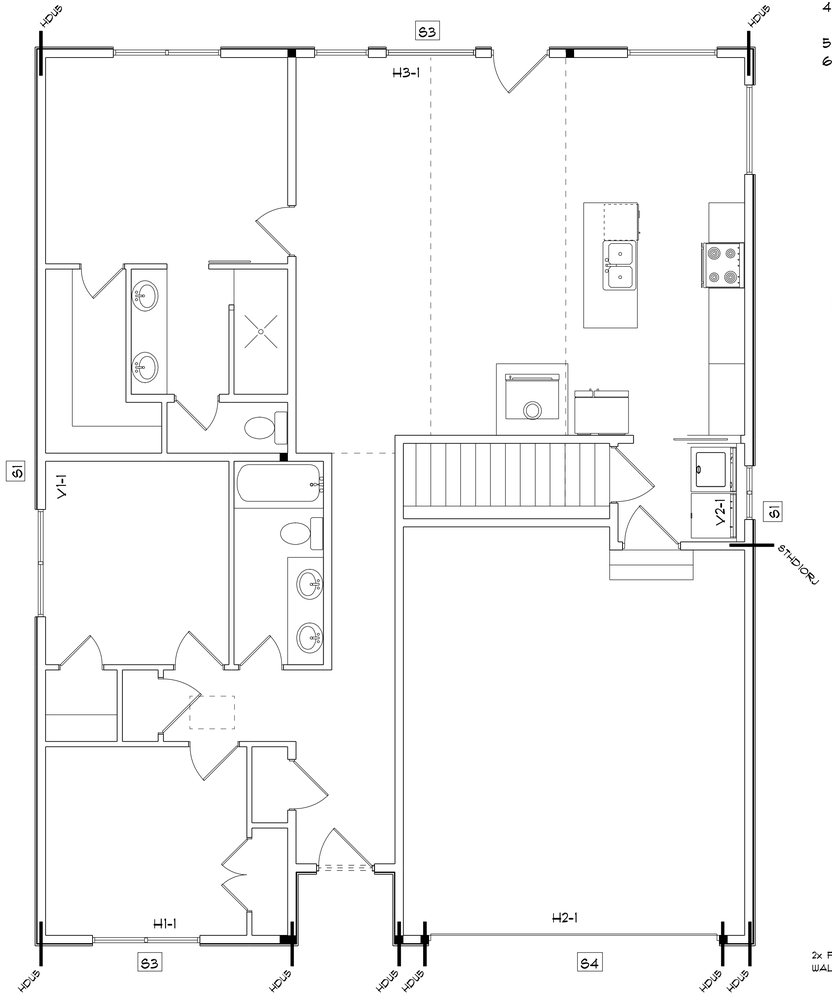
WITH 18' MAX HEADER LENGTHS:
STUD HEIGHT: KING STUD SIZE:
UP TO 8'-0" (2) DF #2 2X6
UP TO 10'-1" (2) DF SEL 2X6
UP TO 14'-3" (4) DF SEL 2X6
UP TO 15'-8" (4) 1.5X5.5 LVL
UP TO 16'-10" (5) 1.5X5.5 LVL
UP TO 18'-3" (5) 1.75X5.5 LVL (2.0, 2600FB)

LVL 18 (1.8, 2400FB) UNO

NAILING AT JOINTS AND BEAMS SHALL BE (10) 10D NAILS (OR #14 SCREWS) AT 2" O.C. ONE ROW TOP, ONE ROW BOTTOM AND ONE ROW CENTERED. SISTER TO TRIMMER/ POST W/ 10D NAILS AT 6" O.C.

IF APPLICABLE, ALSO SEE WINDOW WALL FRAMING FOR MORE INFO.

HEADER LENGTHS ARE THE SUM LENGTH OF HEADERS EACH SIDE OF A KING STUD.

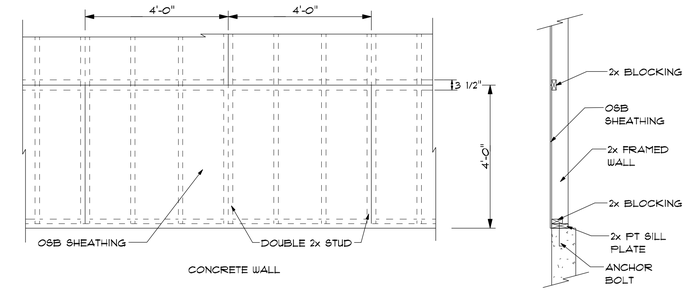


HDU5 = HDU5-SDS2.5 W/ 5/8" ANCHOR BOLT OR 5/8" ALL-THREAD EPOXIED IN PLACE - 8" EMBEDMENT.

WOOD SIDE MEMBER THICKNESS - 3" MIN.

MAIN FLOOR SHEAR WALLS

1/4" = 1'-0"



3" NOMINAL BLOCKING ALTERNATE DETAIL

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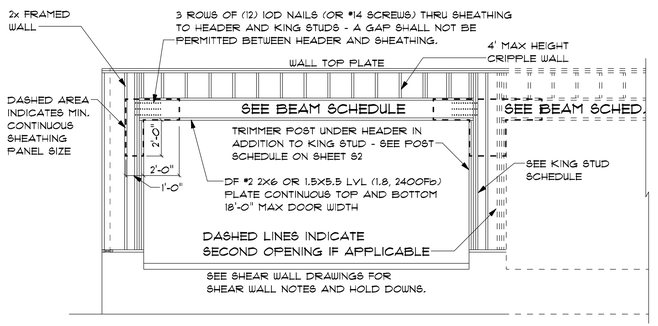

WALL FRAMING AT GARAGE DOORS

Professional Engineer
KRISTOPHER K. BENNETT
Date
WYOMING
04/23/25

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