FASTENER SCHEDULE

The following are minimum fastener requirements for the conditions specified. Other nailing and/or fastening conditions may govern (i.e. top plate splices for shearwalls). See plans and other notes for conditions locations and/or requirements which may exceed the minimums provided by this table. For fastening of manufactured products, refer to mfr recommendations. 3. Ix6 or less subfir to Ea jst, FN......(2) 8d 4. Wider than Ix6 subfir to Ea jst, FN......(3) 8d 5. 2x subflr to jst, bm or girder, blind & FN.......(2) 16d Sill IP to jst or blkg at braced wall panels.......(3) 16d @ 16", (4) Ø.131" x 3" nails @ 16", (4) 14 Ga x 3" staples @ 16"

II. Blkq btwn jsts or rafters to top IE, TN.......(3) 8d, (3) 0.131" x 3" nails, (3) 14 Ga x 3" staples 13. Top £'s, laps & intersections, FN.......(2) 16d, (3) Ø.131" x 3" nails, (3) 14 Ga x 3" staples 16. Cont har to stud, TN.....(4) 8d

18. Clng jst to parallel rafters, FN........(3) 16d, (4) 0.131" x 3" nails, (4) 14 Ga x 3" staples 20. Ix diag brace to Ea stud & P., FN........(2) 8d, (3) 0.131" x 3" nails, (3) 14 Ga x 3" staples 21. lx8 shtg to Ea brng, FN......(3) 8d Wider than Ix8 shtg to Ea brng, FN......(3) 8d 23. Built-up girders & bms, FN T&B, stgd, OS................. 20d @ 32", 0.131" x 3" nails @ 24", 14 Ga x 3" staples @ 24"

Built-up girders & bms, FN @ ends & Ea splice....... (2) 20d, (3) 0.131" x 3" nails, (3) 14 Ga x 3" staples 24. 2x planks @ Ea brng....... 16d 27. Rf rafter to 2x ridge bm, TN or FN........(2) 16d, (3) Ø.131" x 3" nails, (3) 14 Ga x 3" staples(3) 16d, (4) Ø.131" x 3" nails, (4) 14 Ga x 3" staples

29. Ledger strip, FN..... 30. Wd struct panels & particleboard^e, subflr, rf & wall shtg to framing ½ thk # less.....

1/8 to 1/4 thk..... .. 10d or 8d Wd struct panels & particleboard, single flr ³/₄ thk # less...... 6d def % to 1" thk..... ... 8d def

...... 10d common or 8d def 1/8 to 1/4 thk..... 31. Panel siding to framing ½ thk or less..... ... 6d corrosion-resistant siding or casing nail 5⁄8 thk.....

.......... 8d corrosion-resistant siding or casing nail 32. Fiberboard shtg to framing ... Il Ga x 1 $\frac{1}{2}$ " roofing nail h , 6d common, 16 Ga x 1 $\frac{1}{8}$ " staple i ½ thk^J..... || Ga x | $^3\!\!/_4$ " roofing nai h , 8d common, 16 Ga x | $^1\!\!/_2$ " staple 1 ²⁵/₃₂ thk^j... 33. Interior paneling to framing

..... 4d casing or finish nails @ 6" EN/12" FN . 6d Panel supports @ 24", casing or finish nails @ 6" EN/12" FN % thk....

Where EN 4 FN are noted, such may be designated as 6"/12" (EN @ 6", FN @ 12" Unless otherwise noted, common or box nails permitted for all conditions. All staples shall have 7/6 min crown width Unless otherwise noted, common or box nails permitted for all conditions. All staples shall have 1/16 min crown width Nails spaced 6" EN/12" FN except 6" FN where FN supports span 48" or more.

Common or deformed shank 8d nails reg'd min for wood struct panel rf shtg.

l. Detail Callout

5. Hold-Down Callout

6. Strap Callout

Pasteners spaced @ 4" EN, 8" FN, for rf shtg.
Fasteners spaced @ 4" EN, 8" FN, for subfir 4 wall shtg, @ 3" EN/6" FN for rf shtg.

Corrosion-resistant roofing nails w/1/16 \$ head

Corrosion-resistant staples w/% crown. FN supports @ 16". Fasteners spaced @ 3" EN/6" FN, when used as structural shtg. Spacing shall be @ 6" EN/2" FN for non-struct application.

Sheet Number Scale

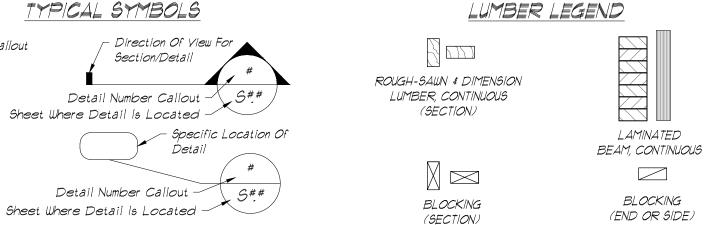
/HD# _ Hold-Down Per

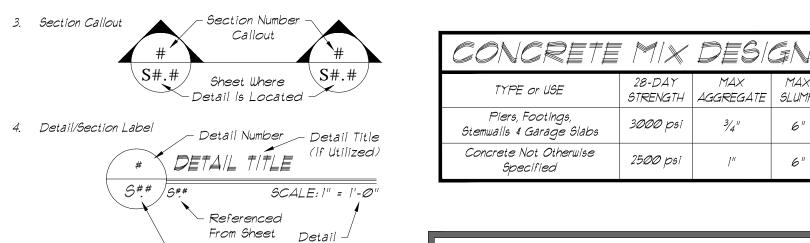
_G# Strap Per

Of Hold-Down

Schedule

Schedule Approximate Location





DEFERRED APPROVAL ITEMS

Deferred approval items shall be reviewed and approved by the Architect/Engineer of Record and by the building official prior to installation. the following items shall be permitted to be submitted for deferred approval:

Manufactured Roof Trusses

TYPICAL CONSTRUCTION NOTES

l. All manufactured hardware and framing materials identified in these plans may be substituted for similar materials manufactured by others, provided all such substitutions are with materials of at least equal capacity as those specified in these plans. All such substitutions shall be submitted to the EOR prior to their use and/or installation*.

*Hardware manufactured by USP Structural Connectors may be used without prior approval of the Engineer-Of-Record, provided the strength requirements as previously described are met.

2. As a minimum, the following shall be used, unless otherwise noted within these plans:

2.1. Sill Plates

- All sill plates in contact with concrete shall be PT DF #2 (Min), with a bolt between 6" and 9" from the end of each piece of sill plate, with two (2) bolts (Min) per sill plate. Sill plates of other wood products are prohibited.
- Sill plates shall be anchored to concrete $w/\frac{5}{8}$ " ϕ (min) anchor bolts embedded at least 10" and spaced at no more than 48" oc.
- 2.2. Double top plates shall be spliced with a minimum of (8) 10d nails at each side of each top- and bottom-piece joint splice. Concurrent splice joints shall be no closer than 48".
- 2.3. Shearwalls: All exterior walls shall be sheathed and nailed to match the minimum shearwall type per the shearwall schedule or plan notes, as applicable..
- 2.4. Post Alternate Unless otherwise noted, it is acceptable to use built-up 2x studs in place of solid-sawn posts not exposed to weather, provided the following criteria are met:
- The built-up section members are of the same material & grade as the post required, including pressure-treated where specified (see 'TIMBER SPECIFICATIONS,' this sheet).
- The built-up section is at least as large as the identified post section.
- The built-up section members are sistered together with 16d nails spaced at no more than 12" oc, staggered, and driven at varying angles to 'tie' each 2x ply to adjacent plys.
- The ends of the built-up member are cut flush for full and uniform bearing.
- Where a mechanical base or cap is required, the built-up section shall be either routed or shaved for proper fit-up. Another suitable base or cap may be used, provided capacities meet or exceed those of the base or cap specified.
- Construction adhesive is applied between plys in addition to the sistering nails.

TIMBER SPECIFICATIONS

- 1. All timber grades as specified in theses notes are minimum grades. It is acceptable to use grades of better quality (i.e. higher strength) without first obtaining approval from the EOR for any such
- 2. Timbers of nominal width equal to or larger than 4" shall not contain boxed heart (i.e. 'free-of-heartcenter,' or FOHC), unless noted 'No FOHC OK' in these plans.
- 3. All Douglas-Fir (DF) Products shall be graded by the Western Wood Products Association Grading Rules and any applicable ASTM standards (i.e. ASTM D245).
- 4. All load-bearing and shearwall framing shall be no less than Douglas-Fir #2. All studs 10'-0" to 14'-0" shall be Douglas-Fir #1. All studs 14'-0" and longer shall be manufactured 2.0E grade or equivalent.
- 5. All sawn beams 4 headers less than 10'-0" shall be Douglas-Fir #2 or Douglas-Fir #1 for spans 10'-0" or longer, unless noted otherwise.
- 6. All glue-laminated beams shall be 24F-V4 DF/DF. All multi-span & cantilever GLB's shall be 24F-V8 DF/DF.
- 7. All posts shall be Douglas-Fir #1 or Better, or manufactured 2.0E equivalent where manufactured studs are required.
- 8. No notching of timber products is allowed, unless otherwise noted in these plans or subsequentlyissued via approved addendums or sketches. Any such addendums or sketches shall be accompanied by the wet stamp and signature of the EOR.
- 9. Fasteners in preservative-treated and fireretardant-treated wood shall be of hot dipped zinc-coated galvanized steel, stainless steel, silicon bronze or copper.
- 10. Steel washers shall be provided under heads and nuts of all lag screws and bolts which bear on wood. The following minimum requirements shall be followed for sizing of washers to be used in sill plate applications*:

Steel PL

Bolt/Laa

<u>Diameter</u>	<u>Washer Size</u>	<u>Washer Size</u>
1/2"	2" Sq x 1/4"	2½" Dia x ¼"
5/8"	21/2" Śq x 1/4"	2 ³ / ₄ " Dia x ⁵ / ₁₆ "
<i>3/</i> 4"	3" Sa' x 3/16"	3" Dia x 1/6"
7/8"	$3\frac{1}{2}$ " $Sq \times \frac{3}{8}$ "	3 ½" Día x 1/6"
1"	3 3/4" Sq x 3/8"	4" Día x ½"
	,	

Malleable Iron

For structures classified as 'Seismic Design Category' D, E or F (See 'General Notes' this page), or for shearwalls where the design load exceeds 490 pounds per linear foot (plf), washers shall not be smaller than 3" $Sq \times \frac{3}{16}$ " PL

*Standard cut washers may be used for all other applications, unless noted otherwise in these plans.

CONCRETE NOTES

- I. Concrete mixing, placing and pouring shall be in accordance with ACI 318 and the project specifications. Mix design shall be in accordance with the applicable sections of the CBC and these plans. Mix designs must be submitted for approval prior to placement of concrete.
- 3. Concrete shall not be permitted to drop from a full height of
- 4. Footings (spread & continuous) are centered under posts, columns and walls, U.N.O.

steel (i.e. in formed cast-in-place concrete walls).

- 5. The finished surface of all horizontal construction joints shall be removed so as to expose clean, solidly embedded aggregate. All reinforcing steel dowels used at horizontal construction joints shall be free of flaking oxidation (rust) and any cured concrete (i.e. hard concrete adhered to the surface of the reinforcing dowels) prior to placement of new concrete at the construction joint.
- 6. Footings shall bear on firm undisturbed native soil or
- 7. Unless otherwise noted in these plans, concrete mixes shall meet the criteria as listed in the table titled 'CONCRETE MIX
- Reinforcing placement and splicing shall be in accordance with the 'Manual of Standard Practice' by the Concrete Reinforcing
- 2. Non-coated reinforcing steel shall be kept clean and free of
- 3. Splices of continuous steel reinforcement bars shall use Class B' lap splices (1'-6' min) with adjacent splices spaced at no

- 6. Wet-stabbing of reinforcing steel dowels or embedded anchor bolts shall not be permitted.
- 7. Protection (clearance from edge or face of concrete) for

7.1.	Concrete poured against earth
7.2.	Concrete formed but exposed to earth or weather
<i>7.3.</i>	#5 \$ smaller
7.4.	#6 \$ larger
7.5.	Columns & beams
7.6.	Interior walls ‡ slabs
7. 7.	Slab-on-Grade - from bottom
7. <i>8.</i>	Structural - from top
7.9.	Non-structural - from top

9. Welding of steel reinforcing bars shall comply with requirements of The American Welding Society (AWS) Dl.1 2008 and the

9. <i>l</i> .	Rebar to ASTM A36	E10XX electrodes
9.2.	Rebar to 450 or stronger	E90XX electrodes
9.3.		E90XX electrodes

1Ø.1.	Welded Wire Fabric	45
10.2.	Ties or stirrups	AS
IØ.3.	Other bars (not welded)	AS
10.4.	Welded bars	45

Diag Diagonal Dim,(s) Dimension(s)

Jnt Joint

- 2. All pipes and conduits passing through walls and footings shall utilize sleeves affixed prior to placing of concrete.
- more than six (6) vertical feet. Hoppers and/or vertical chutes shall be used to avoid segregation in and around reinforcing
- compacted engineered fill.
 - REQUIREMENTS' elsewhere within these plans.

REINFORGING STEEL NOTES

- corrosion or rust prior to placement of concrete.
- less than 5'-0".
- 4. Welded wire fabric lap splices shall be lapped a minimum of 12".
- 5. Provide any and all accessories necessary to support the reinforcing steel and hardware in place as shown in these plans.

- 8. Each bar shall be wire-tied or attached by other approved method to ties, stirrups or cross-bars at a maximum of 24".

10. Reinforcing steel shall meet the following requirements:

1.	Welded Wire Fabric	ASTMI18
2.	Ties or stirrups	ASTM A615, Grade 60
3.	Other bars (not welded)	ASTM A615, Grade 60
4.	Welded bars	ASTM A706

GENERAL STRUCTURAL NOTES

1. The Contractor shall verify all field dimensions prior to fabrication & erection.

correction prior to continuing with construction.

Dead (Rafter)

- 2. If there are any omissions, errors or discrepancies discovered within these plans (i.e. dimension conflicts), contact the Architect or Engineer of Record for clarification and/or
- 3. All plan dimensions as indicated on these plans or on architectural plans are assumed to be from face of studs or face of concrete UNO.

12 psf

10 psf

4. Design Loading Criteria

4.1. Roof: Dead (Trusses)

	Live (Snow [‡] Per Ju	20/18 psf 100 psf [‡]	
	Floor:	Dead Live	12 psf 40 psf
	Deck:	Dead Live	8 psf 60 psf
	Walls:	Exterior & Thermal: Interior	12 psf 10 psf
4.2.	Risk Cat:	//	
4.3.	Wind:	Ult Speed: Exposure Roof Pitch K _{zt} = 1.00	105 mph 'C' 3/6:12 _w = 1.00
4.4.	Seismic:	Lat: 43.158529° N Site Class = D S _G = 1.114 S ₁ = 0.343 le = 1.00 R(1110 SW) = 6.5	Long: 111.003472° W Des. Cat. = D S _{DS} = 0.891 [‡] S _{DI} = 0.229 I _P = 1.00 C _{S(Wd SW)} = 0.137

FOUNDATION NOTES

I. Minimum allowable soil pressures, per IBC:

Dead + Live 1500 psf 1.2. Dead + Live + Wind/Earthquake

All recommendations shall be implemented as indicated within the Geotechnical report in it's entirety. Covenant Engineering shall not be responsible for any negative effects, damage or other detrimental results related to inadequate or uncompacted soil and/or backfill conditions or failure to properly implement Geotechnical recommendations...

SAFETY NOTES

1. It is the Contractors' responsibility to comply with all federal and state regulations regarding maintaining a safe work environment and performing work in a safe manner. It is the Contractors' responsibility to be aware and comply with all OSHA requirements that may apply to this construction project.

STRUCTURAL REFERENCE CODES

- ***** All work to be performed under these project plans shall conform to the following applicable codes and any applicable supplements and amendments:
- 2021 International Building Code 2. ASCE 1-16 Minimum Design Loads for Buildings and Other
- 3. ACI 318-14 Building Code And Commentary (Concrete) 4. ANSI/AF4PA NDS-2018 National Design Specification for Wood 5. ANSI/AF&PA SDPWS 2015 Special Design Provisions For Wind &

STRUCTURAL SHEET INDEX

SO.O STRUCTURAL NOTES & REFERENCES SI.O FOUNDATION PLAN & DETAILS SI.I SHED ROOF FRAMING PLAN & DETAILS

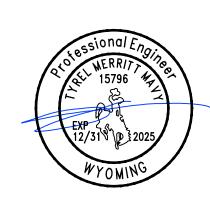
SC Saw Cut, Slip-Critical Wd Wood

Wy

Sched Schedule

		# \D/	CAL STRUCTURAL		BREVIATIONS		
		1 1 1 1					
AB	Anchor Bolt	Dwl	Dowel	LLV	Long Leg Vertical	SDST	Self Drilling Self
Abv	Above	(E)	Existing	LP	Lousiana Pacific Corp.		Tapping Screw
Adh	Adhesive	Ea	Each	$\perp t$	Light	Sht	Sheet
Adj	Adjacent, Adjust	Elev	Elevation	LWC	Lt Weight Concrete	Shtg	Sheathing
Add'l	Additional	EΝ	Edge or End Nailing	MB	Machine Bolt	Sim	Similar
$\mathcal{A}FF$	Above Finish Flr	Engr	Engineer(ed)	Max	Maximum	SLV	Short Leg Vertical
Arch	Architect(ural)	ΕÒ	Edge Of	Mfr	Manufacturer,	SMS	Sheet Metal Screw
BCl	Boise Cascade Inc.	EOR	Engineer Of Record		Manufactured	SOG	Slab-On-Grade
Bldg	Building	Eq	Equal	Min	Minimum	Spec	Specification(s)
Blk	Block	EĠ	Each Side	MtI	Metal	ŚS	Śtainless Steel
Blkg	Blocking	EW	Each Way	(N)	New	Std	Standard
Blui	Below	Exp	Expansion	No.,#	Number	Stgr	Stagger(ed)
BFF	Below Finish Flr	Ext	Ext <i>erior</i>	NS	Near Side	Stiff	Stiffener
Bm	Beam	FF	Finish Floor	NTS	Not To Scale	St1	St <i>eel</i>
BN	Boundary Nailing	FG	Finish Grade	NWC	Normal Weight Concrete	Struct	Structural
<i>B0</i>	Bottom Of	Flr	Floor	oc	On Center	Sq	Square
Bott	Bottom	FN	Field or Face Nailing	OD	Outside Diameter	T\$B	Top & Bottom
Brg	Bearing	Fndn	Foundation 2	0pn	Open(ing)	T#G	Tonque & Groove
Btwn	Between	FO	Face Of	Ópp	Ópposite	TF	Top Flange
Bynd	Beyond	FOHC	Free Of Heart Center	ÓŚ	Opposite Side(s)	Thk	Thick(en,ened,ness)
c	Camber	Frm	Frame, Framing	Par	Parallel	TJI	Trus-Joist Inc.
CJ	Construction Joint	FS	Far Side	Perp	Perpendicular	TO	Top Of
¢, CL	Center Line	Ftg	Footing	PD'F	Powder Driven Fastener	Trans	Transition,
Cing	Ceiling	Gã	Gage	Æ, PL	Plate	Transv	Transverse
Clr	Clear	GLB	Glu-Laminated Beam	Plywd	Plywood	Typ	Typical
CMU	Concrete Masonry Unit	(H)	Hilti Corp	Press	Pressure	(Ü)	Ünistrut Corp.
Col	Column	HD	Hold-Down	psf	Pounds per Square	UNO	Unless Note'd Otherwise
Conc	Concrete	Hdr	Header	1	Foot	URM	Unreinforced Masonry
Conn	Connect, Connection	Hgr	Hanger	psi	Pounds per Square Inch	OWJ	Open Web Joist
Cont	Continuous	HK	HOOK	'PT	Pressure Treated, Post	Vert	Vertical
Ctsnk	Countersink	Horiz	Horizontal		<i>Tension</i>	ω/	With
φ, Dia	Diameter	Ht	Height	Pur	Purlin	W/O	Without
Diag	Diagonal	ID	Inside Diameter	Rad	Radius	WF	Wide Flange
DbĨ	Double	Incl	Included	Reinf	Reinforcing,	WP	Work Point
Demo	Demolition	<i>lnt</i>	Interior		Reinforcement	WS	Wood Screw
Det	Detail	ITW	ITW Red Head Corp.	Reqd	Required	Wt	Weight
DF	Douglas Fir	JH	Joist Hanger	(5)	Simpson Strong Tie	WWF	Welded Wire Fabric
			. ~		, /		





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CONSULTANTS: Covenant ' ngineering PO Box 4260 Bedford, WY 83112

> ph: 916-838-1973 web: covenantengineers.com

ISSUES: REVISIONS: NO. DATE BY DESCRIPTION (1) 04/07/25 TMM STRUCTURAL PLANS Z SHEET DESCRIPTION: 7 REFERENCES JOB NUMBER: **P25004** AS NOTED 24 JAN 2025

TMM 1 OF # STRUCTURAL SHEETS

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2 OF # STRUCTURAL SHEETS

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POST SCHEDULE MARK POST CAPNOTES Wood (S) CCQ/ECCQ Or Sim PT @ Ext Los 6 x 6

NOTES: l. All sizes and grades are req'd minimums. See sheet SOO for wood specifications. 2. Solid posts may be replaced with built-up stud posts. See sheet SOO for requirements.

BASE SCHEDULE							
MARK	Туре	BASE	Anchorage	NOTES			
(ST)	Standoff Base	(S) ABA Or Sim	Wedge AB Per Mfr Kit	n/a			

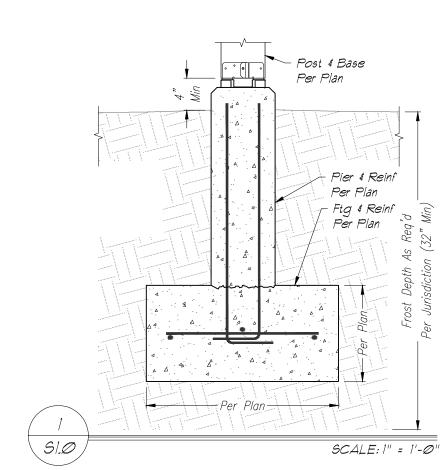
NOTES: l. All specified sizes and grades are req'd minimums. See sheet SOO for material specifications.

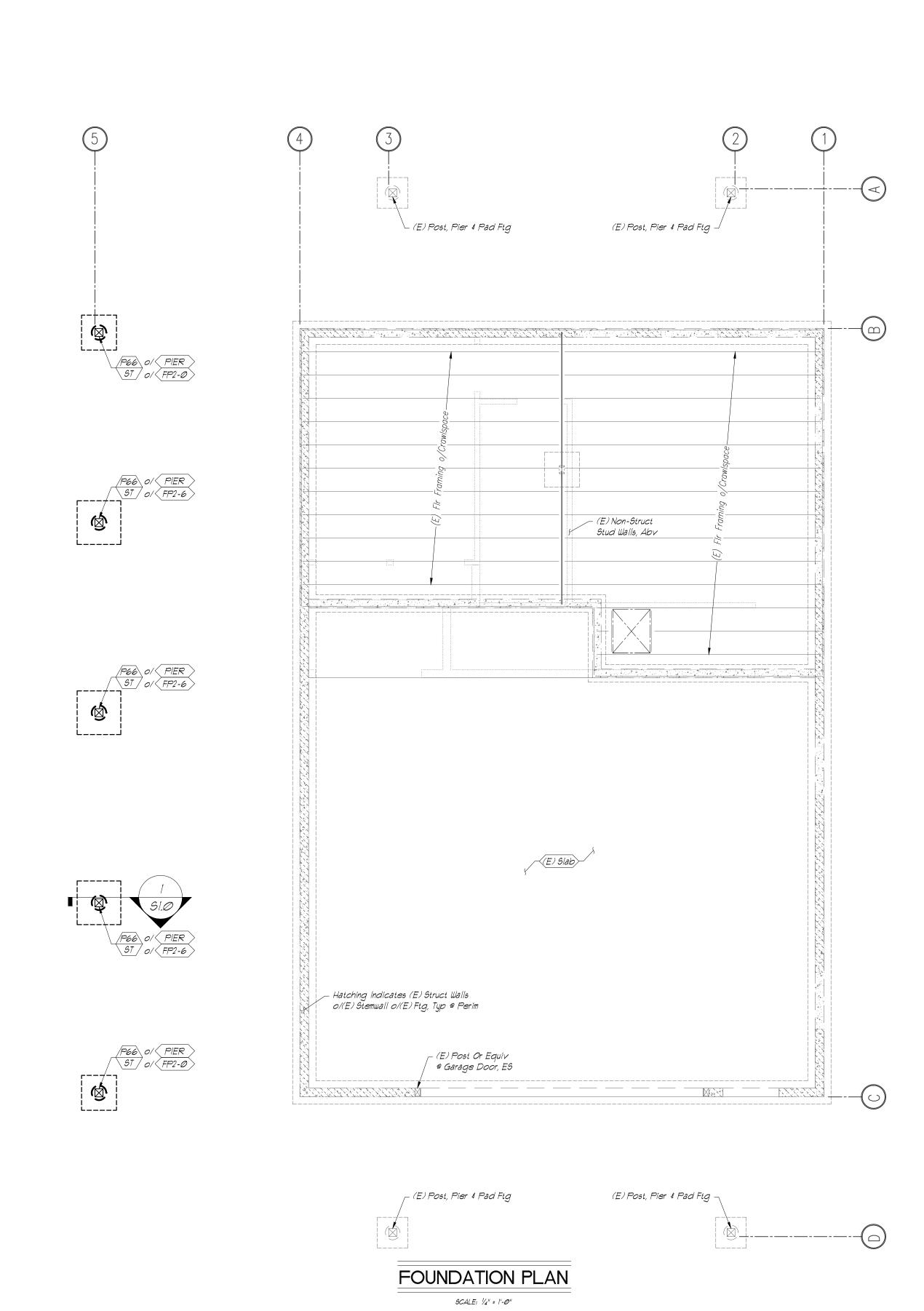
	FOUNDATION SCHEDULE								
MARK	TYPE	SIZE	REINFORCING	NOTES					
(FP2-Ø)	PAD	2'-0 SQ x 10" Thk	(3) #4 EW Centered Vert	BIGFOOT Or Sim Equiv Mfr Form OK					
(FP2-6)	PAD	2'-6 SQ x 10" Thk	(3) #4 EW Centered Vert	BIGFOOT Or Sim Equiv Mfr Form OK					
PIER	PIER	0'-8 ¢ Min	(3) #4 Or (4) #3	Recommend 10"					

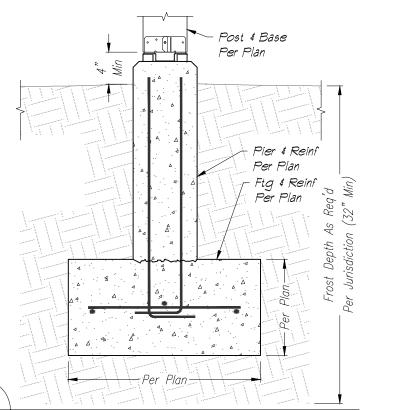
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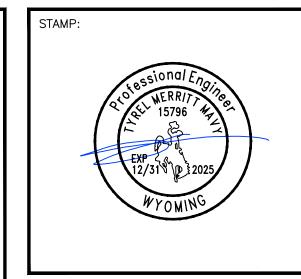
1. All sizes 4 quantities are reald minimums. UNO, it is acceptable to substitute larger sizes and/or more reinforcing without prior approval.

2. See sheet 800 for concrete and reinforcing specifications.









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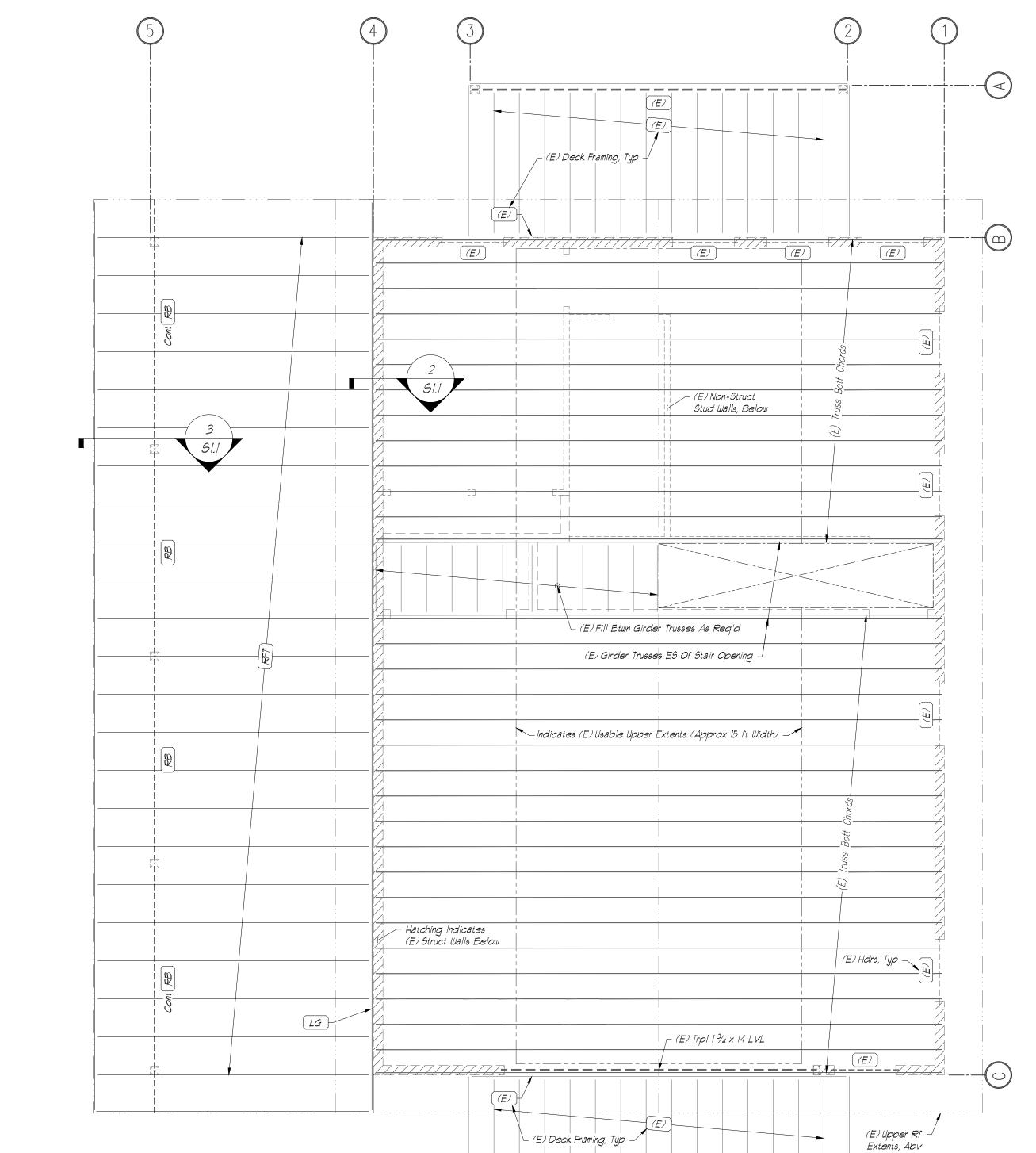
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5 OF # STRUCTURAL SHEETS

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(E) Deck Framing, Typ -

FRAMING PLAN

SCALE: 1/4" = 1'-0"

DIAPHRAGM SCHEDULE						
MARK	MATERIAL	TYPE	1 FASTENERS	ΕN	FN	NOTES
RD	WOOD	½" Min Wd Struct Panel Sthg	8d w/l ³ /s" Min Penetr	6" oc Max	12" 00	Blkg/Plywd Clip @ All Unsupported Panel Edges

FRAMING SCHEDULE						
MARK	FRAMING MEMBER	SUPPORT	NOTES			
LG	2x Or LVL, Match Jst/ Rafter Depth, Min	(2) ½ x 4 ½ SDS Ea Truss Vert Or @ 8" oc Stgd To 4x Blkg	TLOK Equiv OK			
(RB)	5 1/8 x 10 1/2 24F-V4 GLB Or 5 1/4 x 9 1/4 2.0E VLAM 3100	Post & Cap Per Plan	n/a			
(RFT)	1 ³ / ₄ × 11 ¹ / ₄ 2.0E LVL @ 24" Or 2x12 DF-L @ 16"	Brng On Bm	n/a			
	13/4 x 9 1/4 2.0E LVL @ 24" Or 2x10 DF-L No. 1 @ 16"	Hngr To Ledger	Alt Size OK If Large Defl Acceptable			

NOTES:
1. All sizes and grades are req'd minimums. See sheet \$00 for material specifications.
2. Solid Sawn beams may be replaced w/sistered built-up multi-ply sections as follows:
2.l. DF-L 4x may be replaced w/equivalent solid sawn Dbl 2x DF-L No. I/Btr
2.2. DFL 6x may be replaced w/equivalent solid sawn Trpl 2x DF-L
2.3. Mfr 6x nominal GLB/VLAM may be replaced w/equivalent Trpl 1 3/4x 20E LVL

